

ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

REVISIONS

SOFTPIXN

REQUIRED ROOF VENTILATION:
AS PER FLORIDA BUILDING CODE 2309.7

RIDGE VENT MIN. 50% TOTAL VENT AREA LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3' ABOVE EAVE) 484 S.F. / 300 x 50% = 1 S.F. RIDGE VENT AREA REQUIRED 10 FEET OF RIDGE VENT REQUIRED

SOFFIT VENT 484 S.F. / 300 x 50% = 1 S.F. SOFFIT VENT AREA REQUIRED 34 FEET OF SOFFIT VENT REQUIRED

BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

1. RIDGE VENTS = 16 IN2/FT (.11 FT2/FT)
2. OFF-RIDGE VENTS = .70 FT2 PER 4' UNIT
3. SOFFIT VENTS = 4.3 IN2/FT (.03 FT2/FT)

AREA SUMMARY

GARAGE AREA	484	S.F.
TOTAL AREA	484	S.F.

ELECTRICAL PLAN NOTES

E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.

ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY E -4 BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE E -5 DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.

E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.

E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.

E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)

E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION

A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC E -10 CONDUCTORS ENTER THE BUILDING.

SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is

not to be reproduced, altered or copied in any form or manner without first the express writte permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my



JOHN JOHNSON **CONSTRUCTION**

DONALD & SHIRLEY △SHIRLEY GARAGE

Pellicer ADDRESS: lot #13 Russwood Estates Unit 3 171 SW Emily Glen Lake City FL 32024

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PRINTED DATE: May 22, 2006 DRAWN BY: STRUCTURAL BY

Ben Sparks

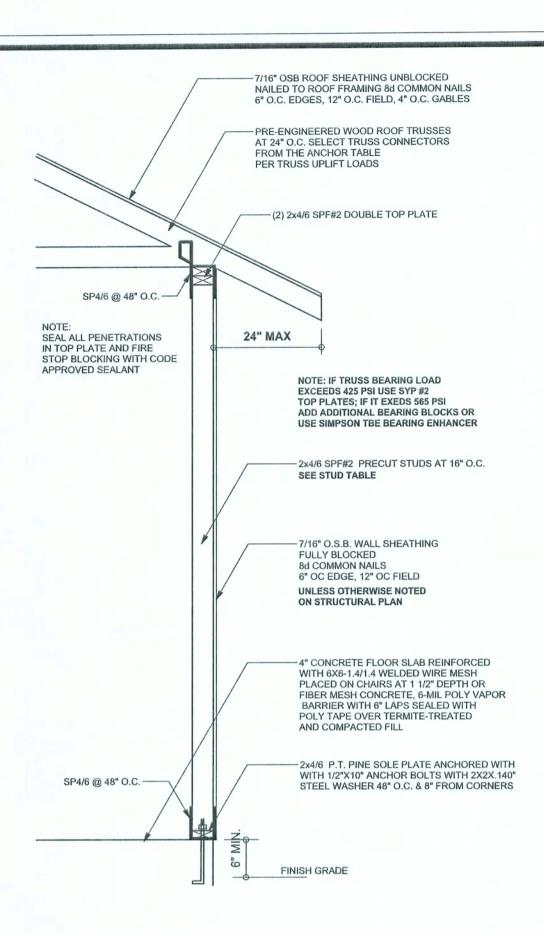
Ben Sparks

22 / May / 06 JOB NUMBER: 605222

DRAWING NUMBER

A-1 OF 3 SHEETS

OFFICE COR



SCALE: 3/4" = 1'-0"

ONE STORY WALL SECTION

EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

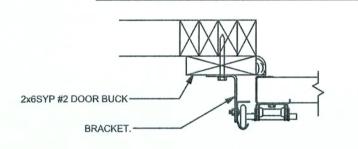
(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.

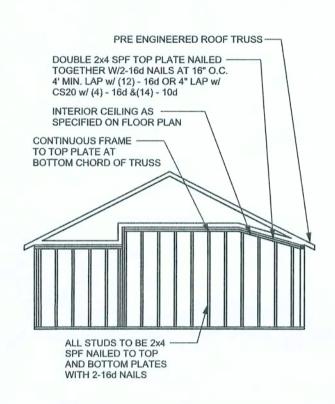
2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"

DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.

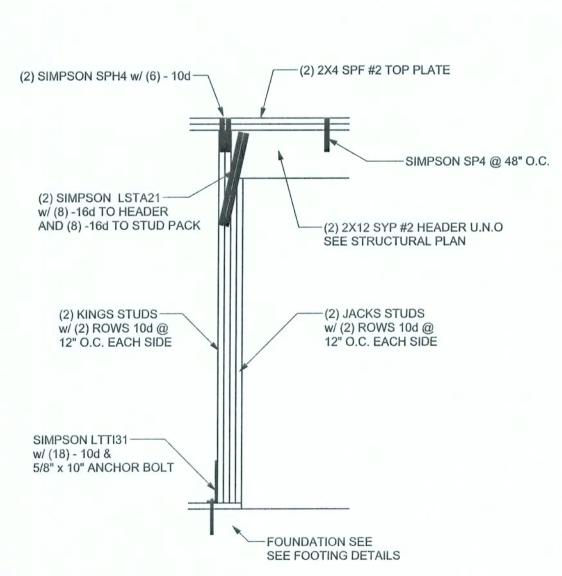


GN PER TABLE BELOW:

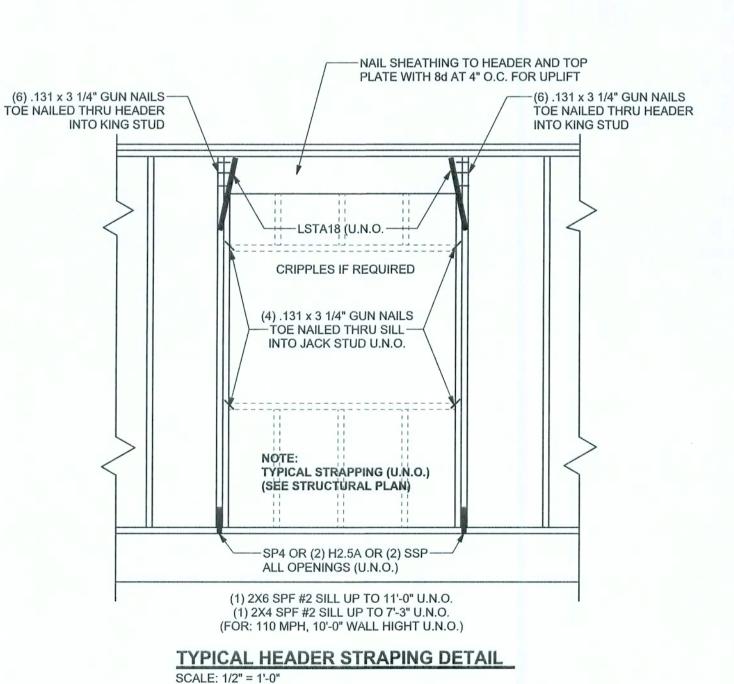
GARAGE DOOR BUCK INSTALLATION DETAIL SCALE: N.T.S.



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL SCALE: N.T.S.



TYPICAL GARAGE DOOR HEADER STRAPING DETAIL SCALE: 1/2" = 1'-0"



GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8;	SYP #2	1200	1.6
2x10)	SYP #2	1050	1.6
2x122	SYP #2	975	1.6
GLB ₃	24F-V3 SP	2400	1.8
LSL.	TIMBERSTRAND	1700	1.7
LVL.	MICROLAM	1600	1.9
PSL_	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEE GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

	ND OWNER ARE RESPONSIBLE FOR THE FOLLOWING NOT PART OF THE WIND LOAD ENGINEER'S SCOPE O	
	DITIONS, FOUNDATION BEARING CAPACITY, GRADE AND WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.	
	S AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.	CR 2004
BELIEVE THE PLA	UOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL GINEER IMMEDIATELY.	
DESIGN, PLACEM	MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS IT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL NS.	

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301,2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements	
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi	
2.1	Mortar	ASTM C 270, Type N, UNO	
2.2	Grout	ASTM C 476, admixtures require approval	
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow medium surface finish, 8"x8"x16" runn bond and 12"x12" or 16"x16" column block	
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"	
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)	
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS	
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS	
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.	
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.	

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

JPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED RO 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6		-	10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

WIND LOADS PER FLORIDA BUILDING CODE	2004 RESIDENTIA	L, SE	CTIO	N R30	1.2.1
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS MEAN ROOF HEIGHT NOT EXCEEDING LEAST ON UPPER HALF OF HILL OR ESCARPMENT & SLOPE AND UNOBSTRUCTED UPWIND FOR	T HORIZONTAL D SOFT IN EXP. B. 3	IMEN OFT I	SION	OR 60	0 FT; N
BUILDING IS NOT IN THE HIGH VELOCITY HUI	RRICANE ZONE				
BUILDING IS NOT IN THE WIND-BORNE DEBR	IS REGION				
1.) BASIC WIND SPEED = 110 MPH					
2.) WIND EXPOSURE = B					
3.) WIND IMPORTANCE FACTOR = 1.0					
4.) BUILDING CATEGORY = II					
5.) ROOF ANGLE = 10-45 DEGREES					
6.) MEAN ROOF HEIGHT = <30 FT					
7.) INTERNAL PRESSURE COEFFICIENT = N	I/A (ENCLOSED B	UILDI	NG)		
8.) COMPONENTS AND CLADDING DESIGN				R301.	.2(2))
Z. A	Zone	_	0		ea (ft2) 100
	1		-21.8		-18.1
3 2	2		-25.5		-21.8
1 7	2 O'hg		-40.6		-40.6
2 2 2 1	3	19.9	-25.5	18.1	-21.8
4 1 2 3	3 Oʻhg		-68.3		-42.4
1 1	4	21.8	-23.6	18.5	-20.4
55	5	21.8	-29.1	18.5	-22.6
The state of the s	Doors	& Wind	dows	21.8	-29.1
[3]	1,17,700	st Cas	-		
	(Zone	5, 10	ft2)		
5 3	8x7 Gar	age D	oor	19.5	-22.9
		rage [-	-

13		Doors & Windows Worst Case (Zone 5, 10 ft2)	21.8	-29.1
5	2,	8x7 Garage Door	19.5	-22.9
1	4 3 2 4 5 2 2	16x7 Garage Door	18.5	-21.0
DESIGN	LOADS			
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)			
	30 PSF (SLEEPING ROOMS)	100		
	30 PSF (ATTICS WITH STORAGE)			
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)			
ROOF	20 PSF (FLAT OR <4:12)			
	16 PSF (4:12 TO <12:12)			
	12 PSF (12:12 AND GREATER)			
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)			
SOIL BE	ARING CAPACITY 1000PSF			
NOTINE	FLOOD ZONE (BUILDER TO VERIFY)			

REVISIONS

SOFTPILAIN

WINDLOAD ENGINEER: Mark Disoswa PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 DIMENSIONS:

dimensions. Refer all questions to
Mark Disosway, P.E. for resolution.
Do not proceed without clarification.

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permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineerin comply with section R301.2.1, florida building code residential 2004, to the best of my

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

JOHN JOHNSON CONSTRUCTION

DONALD & SHIRLEY

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PRINTED DATE:
May 22, 2006

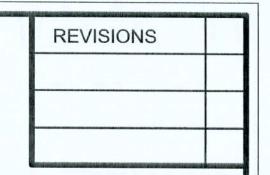
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Ben Sparks Ben Sparks

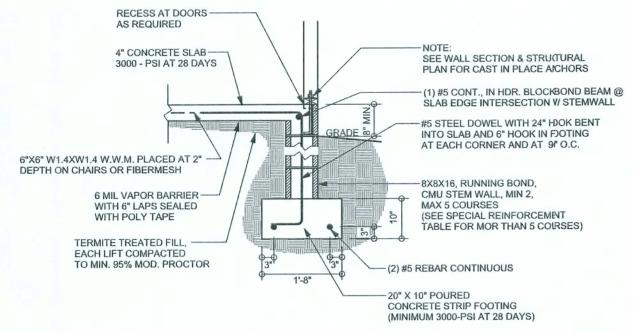
22 / May / 06

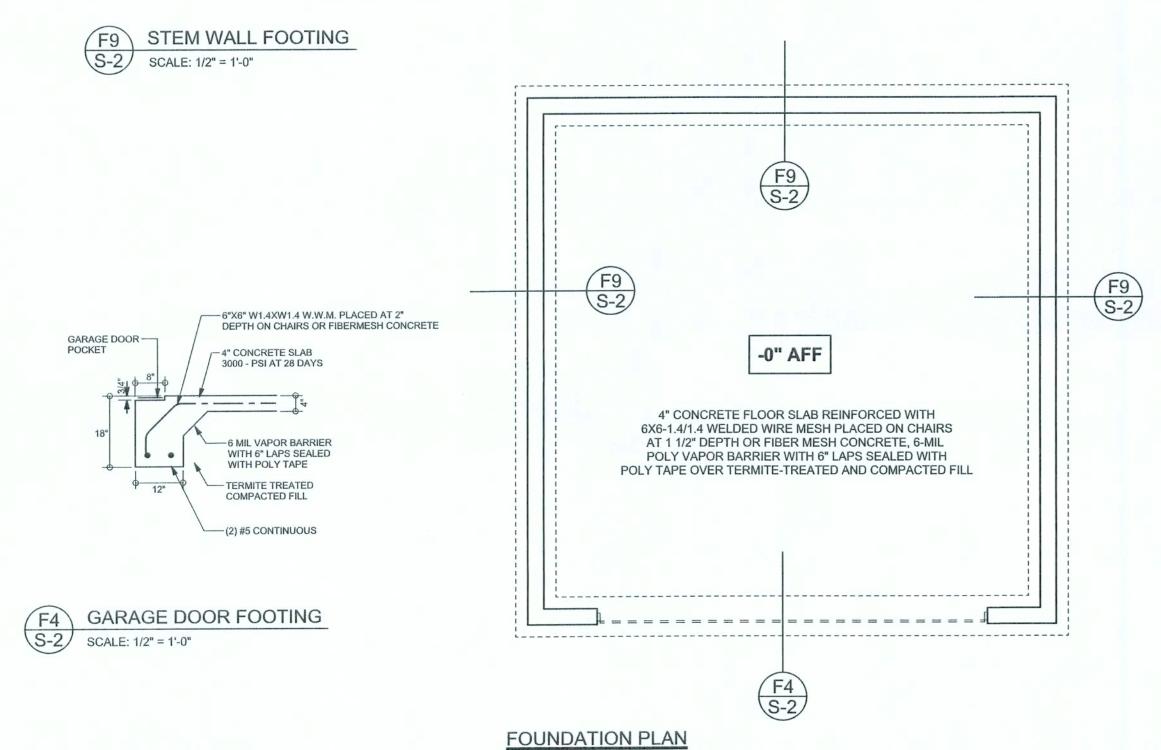
FINALS DATE:

JOB NUMBER: 605222 DRAWING NUMBER

S-1
OF 3 SHEETS

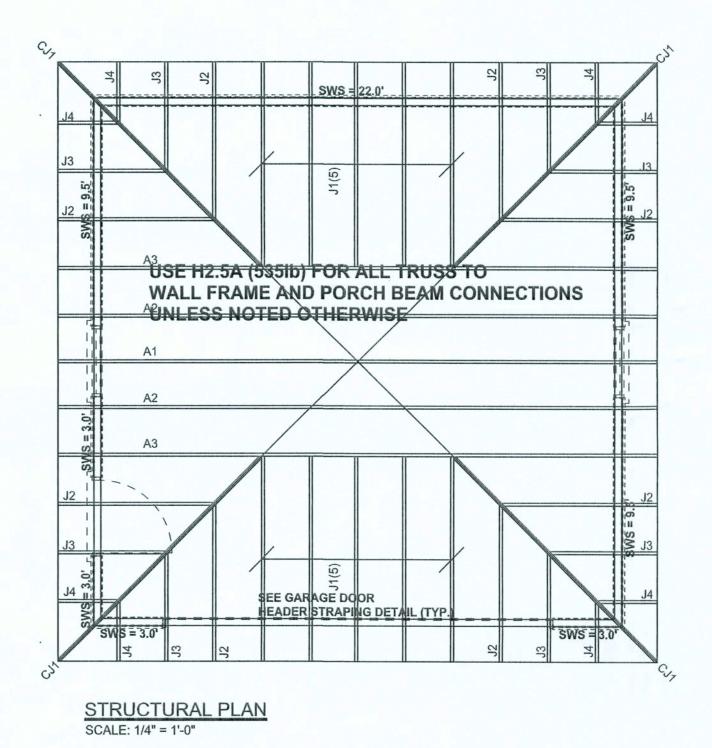


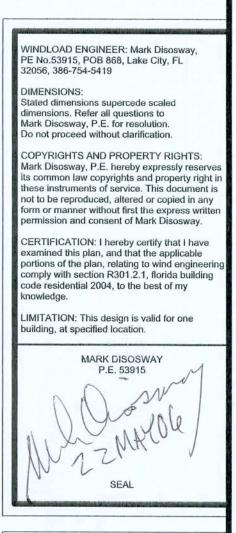




SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS





JOHN JOHNSON **CONSTRUCTION**

DONALD & SHIRLEY SHIRLEY GARAGE

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PRINTED DATE:

605222

S-2

OF 3 SHEETS

Ben Sparks

May 22, 2006 DRAWN BY: STRUCTURAL BY

FINALS DATE:

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING F:RAME WALL & PORCH HEADERS SHALL BE A MINIMUMI OF (2) 2X12 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING F-RAME WALL HEADERS SHALL HAVE (1) JACK(STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRIUCTURAL SHEETS ARE NOT EXACT. REF-ER TO ARCHITECTURAL FLOOR PLAN FOR AC; TUAL DIMENSIONS
- PERMANENT TRUSS EBRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

 SN-4

 LATERAL BRACING IS, TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & E3CSI-B3, BCSI-B1, BCSI-B2, & BCSI-B3
 ARE FURNISHED BY TITHE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
SWS = 0.0'	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

HEADER LEGEND

IL/ (DLIV LL	NAME OF THE PROPERTY OF THE PR
(2) 2X10X0',1J 1K	HEADER/BEAM CALL-OUT (U.N.O.)
	NUMBER OF KING STUDS (FULL LENGTH)
	NUMBER OF JACK STUDS (UNDER HEADER)
	SPAN OF HEADER
	SIZE OF HEADER MATERIAL
	NUMBER OF PLIES IN HEADER

TOTAL SHEAR WALL SEGMENTS

W	S = 0.0' INDICATES	S SHEAR WA	LL SEGM
		REQUIRED	ACTUAL
	TRANSVERSE	11.0'	34.5'
	LONGITUDINAL	9.0'	28.0'

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. MAYO TRUSS CO.

Ben Sparks

22 / May / 06 JOB NUMBER: DRAWING NUMBER