DATE 02/22/2011

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000029204

APPLICANT SCOTT ROSENBOOM PHONE 352-538-3877

ADDRESS 19802 NW 190TH AVE HIGH SPRINGS FL 32643

OWNER CLAUDIO & GRACIEF CONDE PHONE 386-454-8651

ADDRESS 245 SW WAFFLE GLN FORT WHITE FL 32038

CONTRACTOR SCOTT ROSENBOOM PHONE 352-538-3877

LOCATION OF PROPERTY 47 SOUTH, L 27, L WAFFLE GLN, 1ST HOUSE ON LEFT

TYPE DEVELOPMENT SFD, UTILITY ESTIMATED COST OF CONSTRUCTION 160300.00

HEATED FLOOR AREA 2138.00 TOTAL AREA 3206.00 HEIGHT 22.00 STORIES 1

FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 7/12 FLOOR SLAB

LAND USE & ZONING AG-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 1 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 18-7S-17-10021-009 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES 6.45

		CBC1257076		
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
EXISTING	11-0014	BK	TC	Y
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: FLOOR ONE FOOT ABOVE THE ROAD

NOC ON FILE

EXISTING MH TO BE REMOVED 45 DAYS OF CO DATE Check # or Cash 11903

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by Foundation _____ date/app. by Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by Slab _____ date/app. by Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by Insulation _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by Peri. beam (Lintel) _____ date/app. by Pool _____ date/app. by

Permanent power _____ date/app. by C.O. Final _____ date/app. by Culvert _____ date/app. by

Pump pole _____ date/app. by Utility Pole _____ date/app. by M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Reconnection _____ date/app. by RV _____ date/app. by Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 805.00 CERTIFICATION FEE $ 16.03 SURCHARGE FEE $ 16.03

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $ _____

FLOOD DEVELOPMENT FEE $ _____ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ _____ **TOTAL FEE** 912.06

INSPECTORS OFFICE _____ CLERKS OFFICE _____

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

NOTICE OF INSPECTION AND/OR TREATMENT

245 SW Wattle Glenn
Address

29204
Date of Inspection

3/1/11
Date of Treatment - Full ☑ Spot ☐

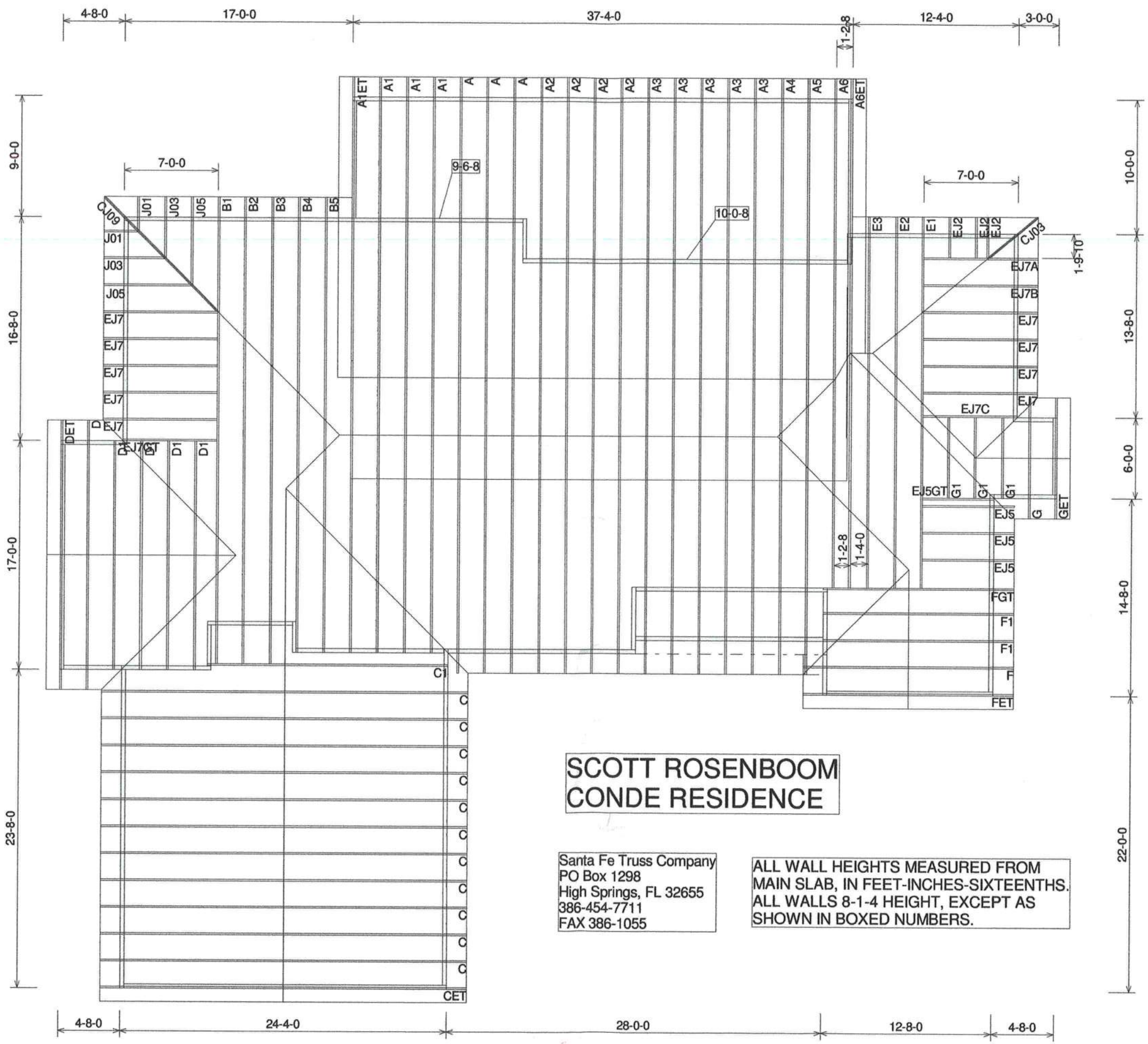
Aggressor @ .5%
Pesticide Used

Sub-Termites
Wood-Destroying Organisms Treated

It is a violation of Florida State Law (Chap. 482-226) for anyone other than the property owner to remove this notice.

- Lawn Spraying
- Household Pest Control
- Tree & Shrub Spraying
- Termite Control

DW Pest Control, Inc.
NW 270th Ave.



SCOTT ROSENBOOM
CONDE RESIDENCE

Santa Fe Truss Company
PO Box 1298
High Springs, FL 32655
386-454-7711
FAX 386-1055

ALL WALL HEIGHTS MEASURED FROM MAIN SLAB, IN FEET-INCHES-SIXTEENTHS. ALL WALLS 8-1-4 HEIGHT, EXCEPT AS SHOWN IN BOXED NUMBERS.

Schafer Engineering, LLC

14705 Main St. Alachua FL 32615

Prepared for:

ROSENBOOM CONSTRUCTION
THE CONDE RESIDENCE
ALACHUA COUNTY, FLORIDA

By:

Schafer Engineering, LLC

386-462-1340 / 352-375-6329

NO COPIES ARE TO BE PERMITTED

SCHAFER ENGINEERING, LLC

7104 NW 42ND LANE \ GAINESVILLE FL. 32606
PHONE: 386-462-1340 \ 352-375-6329

Trusses: Pre-engineered, pre-fabracated with the manufacturers required bracing system installed.

Roof Sheathing: Type: OSB Size: 7/16 Fastener type nails: 8d / .113 Ring Shank

Interior zone spacing: Interior: 6" Periphery: 4

Edge and end zone spacing: Interior: 6" Periphery: 4

Double Top Plate: Type: Spruce Grade: #1 #2 Size: 2 x 4 Nail Spacing: 8 in

Stud Type: Spruce Grade: #1 #2 Size: 2 x 4

Interior stud spacing: 16" End stud spacing: 16"

Shear Wall Siding: Type: OSB Thickness: 7/16

68 ft Trans: Fastener 8d/131 Spacing: Int: 8 Edge: 4

63 ft Trans: Fastener 8d/131 Spacing: Int: 8 Edge: 4

Allowable Unit Shear on Shear Walls: 314 pounds per linear foot

Unit Shear Transferred from Diaphragm: Trans: 252 Long: 62

Wall Tension Transferred by: Siding Nails: 8d/131 @ 4 O.C. Edges

Foundation Anchor Bolts: Concrete Strength: 3000 psi Size: 1/2"

Washer: 2" Embedment: 7" Location of first anchor bolt from corner: 8"

Anchor Bolts @ 48" o.c. Model: A307 Loc. from corner: 8"

Type of Foundation: (1) - #5 rebar continuous required in bond beam.
Floor Slab: 4" Cmu size: 8" x 16" Height: 24" Rein.: #5 at 72" o.c.

Monolithic Footing: Depth: 20" Bottom Width: 12 Rein.: 2 #5 rebars

Stemwall Footing: Width: 20 Depth: 10 Rein.: 2 #5 rebar

Interior Footings 16" Wide X 10" Deep with 2-#5 rebar continuous

Porch Columns: 6X6X8' Syp #2 PT @ 152" o.c. max Column Fasteners: Simpson ABU66/PC66 or equiv

Special Comments: Install ceiling diaphragm on screened porch using same nail, nail pattern, & same grade material as roof sheathing.

Notes:
1. Balloon frame all gable ends unless accompanied by gable end detail
2. All trusses must bear on exterior walls and porch beams.
3. All walls to be nailed with same nailing pattern as the shear walls.
4. This is a wind load ONLY not a structural analysis.
5. This wind load is not valid without a raised, embossed seal.
6. It is assumed that ideal soil conditions and pad preparations are provided.
7. Fiber mesh or WWM may be used in concrete slab.
8. Trusses must be installed and anchored in accordance to the truss engineering.
9. All headers spanning over 12' must be pre-engineered.
10. The foundatioin and walls are minimum design use, and may be increased.
11. Wind load is for one use only \ FBC-2007 \ No copies permitted

1-10-11

Bruce Schafer, P. E. #48984
7104 NW 42ND LN
GAINESVILLE, FL. 32606

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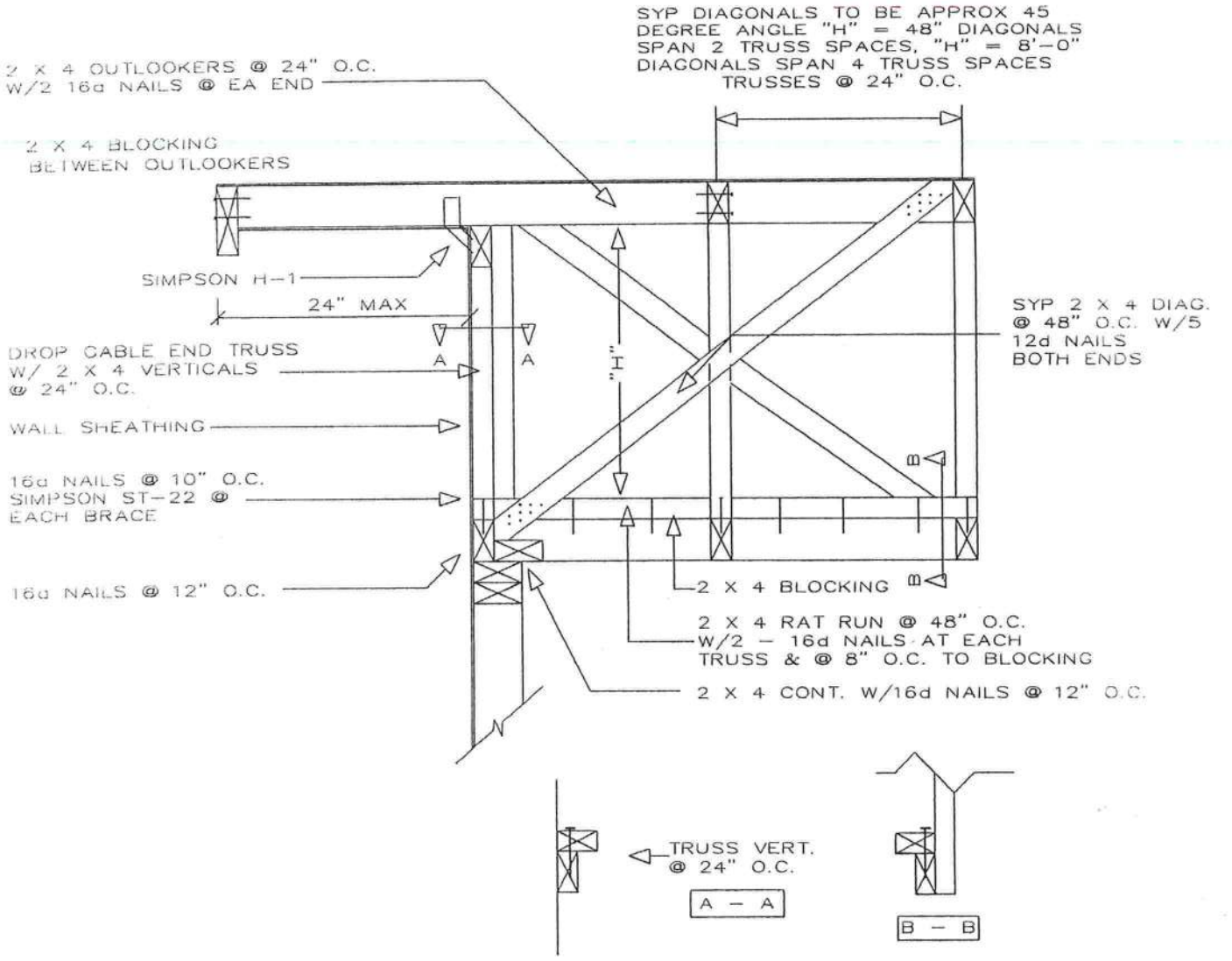
PHONE: 386-462-1340 \ 352-375-6329

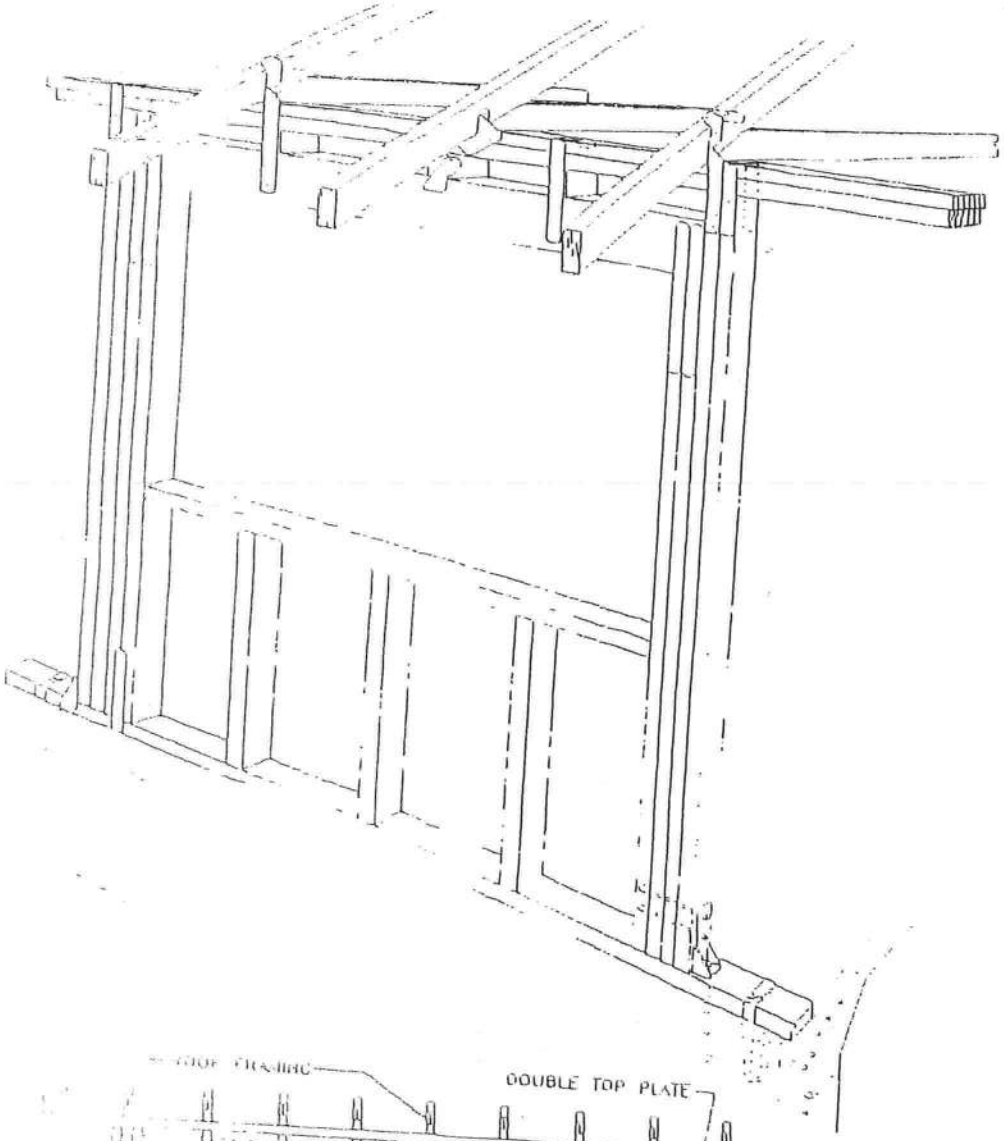


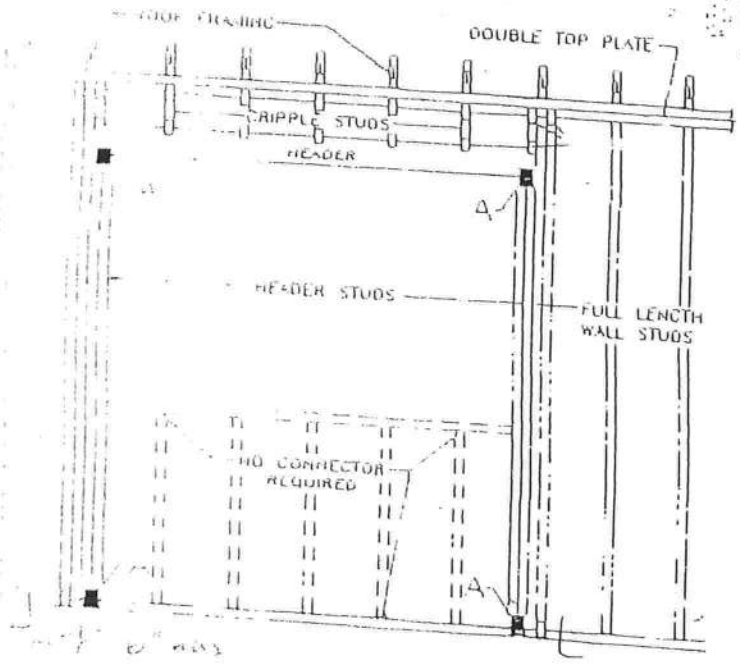
TYPICAL GABLE END BRACING

1-10-11

DETAIL MAY BE USED WITH INTERIOR CATH. CEILING BY INSTALLING A SYP 2 X 4 LEDGER IN PLANE WITH THE INTERIOR CEILING USING 2 - 16d NAILS ON EACH POINT WHERE THE LEDGER CROSSES THE GABLE END TRUSS VERTICALS

Bruce Schafer, P. E. #48984
7104 NW 42ND LN
GAINESVILLE, FL. 32606





Total each truss uplift on the header divide by 2 for header anchorage

SCHAFER ENGINEERING, LLC

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PHONE: 386-462-1340 \ 352-375-6329

HEADER STRAPPING				
Uplift Lbs	Top Connector	Rating Lbs	Bottom Connector	Rating Lbs
to 455	LSTA19	635	H3	320
to 910	LSTA12	795	2-H3	640
to 1265	LSTA18	1110	LTT19	1305
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2165
to 2865	3-LSTA18	3255	HD2A-3.5	2865
to 3700	3-LSTA24	3880	HD5A-3	3130
Total the uplift for each truss sitting on the header and divide by 2 to determine the uplift on the header. Use proper bolt anchors sufficient to support required uplift loads.				

TRUSSES \ GIRDERS			
Uplift Lbs	Top Connector	Bottom Connector	Rating Lbs
to 535	H2.5A	NA	
to 1015	H10A	NA	
to 1215	TS22	LTT19	1305
to 1750	2-TS22	LTT20	1750
to 2570	2-TS22	HD2A	2775
to 3665	3-TS22	HD5A	4010
to 5420	2-MST37	HTT22	5250
to 9660	2-MST60	HD10A	9540
Two 12d common toenials are required per truss for each bearing point into top plate. It is the contractors responsibility to provide a continuous load path from truss to foundation.			

	TOP CONNECTOR	RATING LBS	BOTTOM CONNECTOR	RATING LBS
BEAM SEATS	LSTA18	1110	LTT19	1305
POSTS	2-LSTA18	2220	ABU44	2300

1. Simpson or equivient hardware may be used. For nailing into spruce members, multiply table values by .86
2. See truss engineering for anchor uplift values.
3. This schedule is not meant to be a replacement to the specified values of any manufactures values.

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User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	30.3	Deg
Type of Roof	Hipped	
Eave Height (Eht)	8.00	ft
Ridge Height (RHt)	18.84	ft
Mean Roof Height (Ht)	13.51	ft
Width Perp. to Wind (B)	66.33	ft
Width Parallel to Wind (L)	74.33	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters		
Type of Structure		
Height/Least Horizontal Dim	0.20	
Flexible Structure	No	

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
lzm	Cc * (33/z)^0.167	0.3048	
Lzm	l*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((B+Ht)/Lzm)^0.63))^0.5	0.8880	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8589	
Gust Factor Category III: Flexible or Dynamically Sensitive Structures			
Vhref	V*(5280/3600)	161.33	ft/s
Vzm	bm*(zm/33)^Am*Vhref	70.89	ft/s
NF1	NatFreq*Lzm/Vzm	4.37	Hz
Rn	(7.47*NF1)/(1+10.302*NF1)^1.667	0.0552	
Nh	4.6*NatFreq*Ht/Vzm	0.88	
Nb	4.6*NatFreq*B/Vzm	4.30	
Nd	15.4*NatFreq*Depth/Vzm	16.15	
Rh	1/Nh-(1/(2*Nh^2)*(1-Exp(-2*Nh)))	0.6028	
Rb	1/Nb-(1/(2*Nb^2)*(1-Exp(-2*Nb)))	0.2054	
Rd	1/Nd-(1/(2*Nd^2)*(1-Exp(-2*Nd)))	0.0600	
RR	((1/Beta)*Rn*Rh*Rb*(0.53+0.47*Rd))^0.5	0.6175	
gg	+(2*LN(3600*n1))^0.5+0.577/(2*LN(3600*n1))^0.5	4.19	
Gust3	0.925*((1+1.7*Izm*(3.4^2*Q^2+GG^2*RR^2)^0.5)/(1+1.7*3.4*Izm))	1.02	

Gust Factor Summary			
Main Wind-force resisting system:		**Components and Cladding:**	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.86	Gust Factor (G)	0.86

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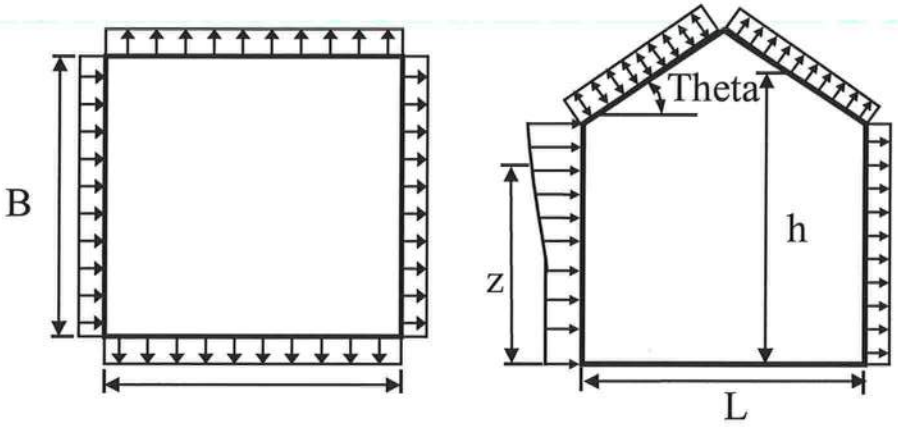
6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev.	Kz	Kzt	Kd	qz	Pressure (lb/ft^2)	
					Windward Wall*	
ft			1.00	lb/ft^2	+GCpi	-GCpi
18.84	0.70	1.00	1.00	21.70	11.71	18.12
15	0.70	1.00	1.00	21.70	11.71	18.12

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(15/zg)^(2/Alpha)	0.57	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*ImpFac*Kh*Kht*Kd	17.80	psf

Wall Pressure Coefficients, Cp	
Surface	**Cp**
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf)	
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 66.33 ft wall)	-0.48	-10.48	-4.07
Leeward Walls (Wind Dir Parallel to 74.33 ft wall)	-0.50	-10.85	-4.44
Side Walls	-0.70	-13.91	-7.50
Roof - Normal to Ridge (Theta>=10)			
Windward - Max Negative	-0.19	-6.08	0.33
Windward - Max Positive	0.31	1.47	7.88
Leeward Normal to Ridge	-0.60	-12.38	-5.97
Overhang Top	-0.19	-2.87	-2.87
Overhang Bottom	0.80	0.69	0.69
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 6.755 ft	-0.90	-16.97	-10.56
Dist from Windward Edge: 6.755 ft to 13.51 ft	-0.90	-16.97	-10.56
Dist from Windward Edge: 13.51 ft to 27.02 ft	-0.50	-10.85	-4.44

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Dist from Windward Edge: > 27.02 ft	-0.30	-7.79	-1.38

* Horizontal distance from windward edge

Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	17.80

Case A						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	0.56	0.18	-0.18	21.70	8.25	16.06
2	0.21	0.18	-0.18	21.70	0.65	8.46
3	-0.43	0.18	-0.18	21.70	-13.24	-5.43
4	-0.37	0.18	-0.18	21.70	-11.94	-4.12
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.69	0.18	-0.18	21.70	11.07	18.88
2E	0.27	0.18	-0.18	21.70	1.95	9.77
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

* p = qh * (GCpf - GCpi)

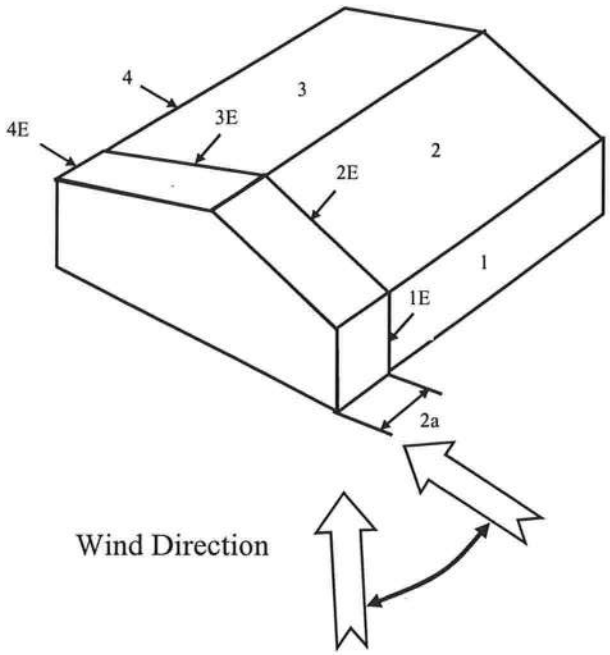

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Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	17.80

Case B						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86
5	0.40	0.18	-0.18	21.70	4.77	12.59
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.61	0.18	-0.18	21.70	9.33	17.14
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43

* p = qh * (GCpf - GCpi)

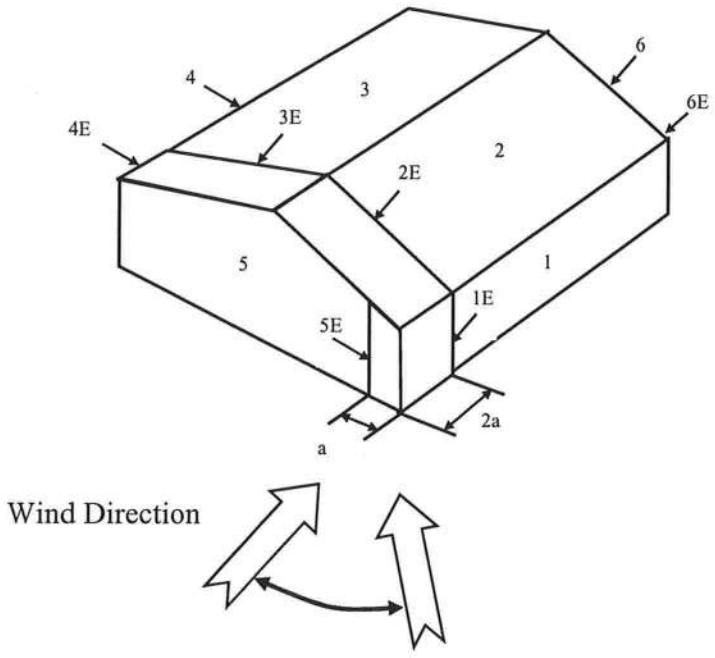

Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft

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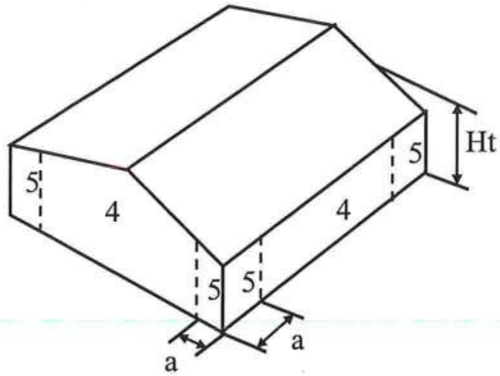

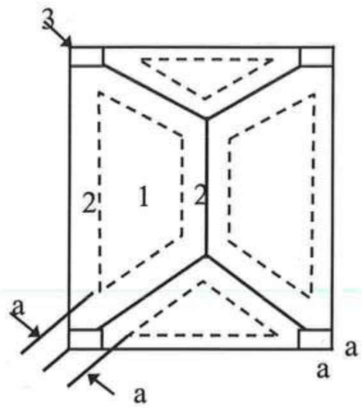

Hipped Roof
10 < Theta <= 30

a = 5.404 ==> 5.40 ft

Component	Width (ft)	Length (ft)	Area (ft^2)	Zone	GCp		Wind Press (lb/ft^2)	
					Max	Min	Max	Min
	16	7	112.00	5	0.81	-1.03	17.71	-21.53
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
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	0	0	0.00					
	0	0	0.00					
	0	0	0.00					

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

Condition	Gcpi	
	Max +	Max -

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Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	**0.18**	**-0.18**

Table 6-8 External Pressure Coefficients for Arched Roofs, Cp

r (Rise-to-Span Ratio) = 0.3

Condition	Variable	Cp		
		Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	Cp	0.13	-1	-0.5
	P (+GCpi) - psf	-1.29	-18.50	-10.85
	P (-GCpi) -psf	5.12	-12.09	-4.44
Roof Springing from Ground	Cp	0.42	-1	-0.5
	P (+GCpi) - psf	3.22	-18.50	-10.85
	P (-GCpi) -psf	3.22	-18.50	-10.85

Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings, Cf

Variable	Description	Value	
L	Roof dimension normal to wind direction	74.33	ft
B	Roof dimension parallel to wind direction	66.33	ft
L/B	Ratio of L to B	1.121	
Theta	Slope of Roof	30.3	Deg
Cf	Force Coefficient	0.00	
X	Distance to center of pressure from windward edge	0.00	ft

FORM 1100A-08

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Performance Method A

Project Name:	conde res	Builder Name:	rosenboom
Street:		Permit Office:	
City, State, Zip:	, fl ,	Permit Number:	
Owner:		Jurisdiction:	
Design Location:	FL, Gainesville		

1. New construction or existing		New (From Plans)		
2. Single family or multiple family		Single-family		
3. Number of units, if multiple family		1		
4. Number of Bedrooms		3		
5. Is this a worst case?		Yes		
6. Conditioned floor area (ft²)		2138		
7. Windows(324.0 sqft.)	Description		Area	
a. U-Factor: SHGC:	Dbl, U=0.55 SHGC=0.45		324.00	ft²
b. U-Factor: SHGC:	N/A			ft²
c. U-Factor: SHGC:	N/A			ft²
d. U-Factor: SHGC:	N/A			ft²
e. U-Factor: SHGC:	N/A			ft²
8. Floor Types (2138.0 sqft.)		Insulation	Area	
a. Slab-On-Grade Edge Insulation		R=0.0	2138.00	ft²
b. N/A		R=		ft²
c. N/A		R=		ft²

9. Wall Types (2214.0 sqft.)	Insulation	Area
a. Frame - Wood, Exterior	R=13.0	1944.00 ft²
b. Frame - Wood, Adjacent	R=13.0	270.00 ft²
c. N/A	R=	ft²
d. N/A	R=	ft²
10. Ceiling Types (2571.0 sqft.)	Insulation	Area
a. Under Attic (Vented)	R=30.0	2138.00 ft²
b. Knee Wall (Vented)	R=19.0	433.00 ft²
c. N/A	R=	ft²
11. Ducts		
a. Sup: Attic Ret: Attic AH: Garage Sup. R= 6, 224 ft²		
12. Cooling systems		
a. Central Unit		Cap: 48.0 kBtu/hr SEER: 14
13. Heating systems		
a. Electric Heat Pump		Cap: 48.0 kBtu/hr HSPF: 8.4
14. Hot water systems		
a. Electric		Cap: 40 gallons EF: 0.87
b. Conservation features None		
15. Credits		Pstat

Glass/Floor Area: 0.152

Total As-Built Modified Loads: 38.53
Total Baseline Loads: 45.30

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Dennis Gerlosy
DATE: 1-17-11

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____
DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____
DATE: _____

- **Compliance requires certification by the air handler unit manufacturer that the air handler enclosure qualifies as certified factory-sealed in accordance with N1110.A.3.**

COLUMBIA COUNTY BUILDING DEPARTMENT
Received for FILE COPY
Code Compliance
PLANS EXAMINER

PROJECT

Title:	conde res	Bedrooms:	3	Adress Type:	Street Address
Building Type:	FLAsBuilt	Conditioned Area:	2138	Lot #	
Owner:		Total Stories:	1	Block/SubDivision:	
# of Units:	1	Worst Case:	Yes	PlatBook:	
Builder Name:	rosenboom	Rotate Angle:	45	Street:	
Permit Office:		Cross Ventilation:		County:	alachua
Jurisdiction:		Whole House Fan:		City, State, Zip:	, fl ,
Family Type:	Single-family				
New/Existing:	New (From Plans)				
Comment:					

CLIMATE

√	Design Location	TMY Site	IECC Zone	Design Temp 97.5 %	Design Temp 2.5 %	Int Design Temp Winter	Int Design Temp Summer	Heating Degree Days	Design Moisture	Daily Temp Range
_____	FL, Gainesville	FL_GAINESVILLE_REGI	2	32	92	75	70	1305.5	51	Medium

FLOORS

√	#	Floor Type	Perimeter	R-Value	Area	Tile	Wood	Carpet
_____	1	Slab-On-Grade Edge Insulatio	247 ft	0	2138 ft²	0	0	1

ROOF

√	#	Type	Materials	Roof Area	Gable Area	Roof Color	Solar Absor.	Tested	Deck Insul.	Pitch
_____	1	Hip	Composition shingles	2316 ft²	0 ft²	Medium	0.96	No	0	22.6 deg

ATTIC

√	#	Type	Ventilation	Vent Ratio (1 in)	Area	RBS	IRCC
_____	1	Full attic	Vented	300	2138 ft²	N	N

CEILING

√	#	Ceiling Type	R-Value	Area	Framing Frac	Truss Type
_____	1	Under Attic (Vented)	30	2138 ft²	0.11	Wood
_____	2	Knee Wall (Vented)	19	433 ft²	0.11	Wood

WALLS

√	#	Ornt	Adjacent To	Wall Type	Cavity R-Value	Area	Sheathing R-Value	Framing Fraction	Solar Absor.
_____	1	N=>NE	Garage	Frame - Wood	13	270 ft²		0.23	0.01
_____	2	S=>SW	Exterior	Frame - Wood	13	648 ft²		0.23	0.75
_____	3	W=>NW	Exterior	Frame - Wood	13	648 ft²		0.23	0.75
_____	4	E=>SE	Exterior	Frame - Wood	13	648 ft²		0.23	0.75

DOORS

√	#	Ornt	Door Type	Storms	U-Value	Area
_____	1	N=>NE	Insulated	None	0.460000	21 ft²

WINDOWS

Orientation shown is the entered orientation (=>) changed to Worst Case.

√	#	Ornt	Frame	Panes	NFRC	U-Factor	SHGC	Storms	Area	Overhang Depth	Overhang Separation	Int Shade	Screening
_____	1	N=>NE	Metal	Low-E Double	Yes	0.55	0.45	N	90 ft²	0 ft 0 in	0 ft 0 in	HERS 2006	None
_____	2	S=>SW	Metal	Low-E Double	Yes	0.55	0.45	N	180 ft²	0 ft 0 in	0 ft 0 in	HERS 2006	None
_____	3	W=>NW	Metal	Low-E Double	Yes	0.55	0.45	N	30 ft²	0 ft 0 in	0 ft 0 in	HERS 2006	None
_____	4	E=>SE	Metal	Low-E Double	Yes	0.55	0.45	N	24 ft²	0 ft 0 in	0 ft 0 in	HERS 2006	None

INFILTRATION & VENTING

√	Method	SLA	CFM 50	ACH 50	ELA	EqLA	---- Forced Ventilation ---- Supply CFM	Exhaust CFM	Run Time Fraction	Fan Watts
_____	Default	0.00036	2019	7.08	110.8	208.4	0 cfm	0 cfm	0	0

GARAGE

√	#	Floor Area	Ceiling Area	Exposed Wall Perimeter	Avg. Wall Height	Exposed Wall Insulation
_____	1	382.8 ft²	382.8 ft²	64 ft	8 ft	1

COOLING SYSTEM

√	#	System Type	Subtype	Efficiency	Capacity	Air Flow	SHR	Ducts
_____	1	Central Unit	None	SEER: 14	48 kBtu/hr	1440 cfm	0.75	sys#0

HEATING SYSTEM

√	#	System Type	Subtype	Efficiency	Capacity	Ducts
_____	1	Electric Heat Pump	None	HSPF: 8.4	48 kBtu/hr	sys#0

HOT WATER SYSTEM

√	#	System Type	EF	Cap	Use	SetPnt	Conservation
_____	1	Electric	0.87	40 gal	60 gal	120 deg	None

SOLAR HOT WATER SYSTEM

√	FSEC Cert #	Company Name	System Model #	Collector Model #	Collector Area	Storage Volume	FEF
_____	None	None			ft²		

DUCTS

√	#	---- Supply ---- Location	R-Value	Area	---- Return ---- Location	Area	Leakage Type	Air Handler	CFM 25	Percent Leakage	QN	RLF
_____	1	Attic	6	224 ft²	Attic	106.9 ft	Default Leakage	Garage	(Default)	(Default) %		

TEMPERATURES

Programable Thermostat: Y

Ceiling Fans:

Cooling	[X] Jan	[X] Feb	[X] Mar	[X] Apr	[X] May	[X] Jun	[X] Jul	[X] Aug	[X] Sep	[X] Oct	[X] Nov	[X] Dec
Heating	[X] Jan	[X] Feb	[X] Mar	[X] Apr	[X] May	[X] Jun	[X] Jul	[X] Aug	[X] Sep	[X] Oct	[X] Nov	[X] Dec
Venting	[X] Jan	[X] Feb	[X] Mar	[X] Apr	[X] May	[X] Jun	[X] Jul	[X] Aug	[X] Sep	[X] Oct	[X] Nov	[X] Dec

Thermostat Schedule: HERS 2006 Reference

Schedule Type		Hours 1	2	3	4	5	6	7	8	9	10	11	12
Cooling (WD)	AM	78	78	78	78	78	78	78	78	80	80	80	80
	PM	80	80	78	78	78	78	78	78	78	78	78	78
Cooling (WEH)	AM	78	78	78	78	78	78	78	78	78	78	78	78
	PM	78	78	78	78	78	78	78	78	78	78	78	78
Heating (WD)	AM	66	66	66	66	66	68	68	68	68	68	68	68
	PM	68	68	68	68	68	68	68	68	68	68	66	66
Heating (WEH)	AM	66	66	66	66	66	68	68	68	68	68	68	68
	PM	68	68	68	68	68	68	68	68	68	68	66	66

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS:	PERMIT #:
, fl,	

INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	N1106.AB.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	N1106.AB.1.2	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	N1106.AB.1.2	Penetrations/openings > 1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	N1106.AB.1.2	Between walls & ceilings; penetrations of ceiling plane to top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	N1106.AB.1.2	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	N1106.AB.1.2	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	N1106.AB.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	N1112.AB.3	Comply with efficiency requirements in Table N1112.ABC.3 Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	N1112.AB.2.3	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%. Heat pump pool heaters shall have a minimum COP of 4.0.	
Shower heads	N1112.AB.2.4	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	N1110.AB	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section N1110.AB. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	N1107.AB.2	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	N1104.AB.1 N1102.B.1.1	Ceilings-Min. R-19. Common walls-frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE INDEX* = 85

The lower the EnergyPerformance Index, the more efficient the home.

, , fl,

1. New construction or existing		New (From Plans)
2. Single family or multiple family		Single-family
3. Number of units, if multiple family		1
4. Number of Bedrooms		3
5. Is this a worst case?		Yes
6. Conditioned floor area (ft²)		2138
7. Windows**	Description	Area
a. U-Factor: SHGC:	Dbl, U=0.55 SHGC=0.45	324.00 ft²
b. U-Factor: SHGC:	N/A	ft²
c. U-Factor: SHGC:	N/A	ft²
d. U-Factor: SHGC:	N/A	ft²
e. U-Factor: SHGC:	N/A	ft²

8. Floor Types	Insulation	Area
a. Slab-On-Grade Edge Insulation	R=0.0	2138.00 ft²
b. N/A	R=	ft²
c. N/A	R=	ft²

9. Wall Types	Insulation	Area
a. Frame - Wood, Exterior	R=13.0	1944.00 ft²
b. Frame - Wood, Adjacent	R=13.0	270.00 ft²
c. N/A	R=	ft²
d. N/A	R=	ft²

10. Ceiling Types	Insulation	Area
a. Under Attic (Vented)	R=30.0	2138.00 ft²
b. Knee Wall (Vented)	R=19.0	433.00 ft²
c. N/A	R=	ft²

11. Ducts
a. Sup: Attic Ret: Attic AH: Garage Sup. R= 6, 224 ft²

12. Cooling systems
a. Central Unit — Cap: 48.0 kBtu/hr SEER: 14

13. Heating systems
a. Electric Heat Pump — Cap: 48.0 kBtu/hr HSPF: 8.4

14. Hot water systems
a. Electric — Cap: 40 gallons EF: 0.87

b. Conservation features
None

15. Credits — Pstat

I certify that this home has complied with the Florida Energy Efficiency Code for Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

*Note: The home's estimated Energy Performance Index is only available through the EnergyGauge USA - FlaRes2008 computer program. This is not a Building Energy Rating. If your Index is below 100, your home may qualify for incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at (321) 638-1492 or see the Energy Gauge web site at energygauge.com for information and a list of certified Raters. For information about Florida's Energy Efficiency Code for Building Construction, contact the Department of Community Affairs at (850) 487-1824.

**Label required by Section 13-104.4.5 of the Florida Building Code, Building, or Section B2.1.1 of Appendix G of the Florida Building Code, Residential, if not DEFAULT.

Hogle's
Heating & Air

Project Summary
Entire House
HOGLE'S HEATING & AIR

Job: CONDE RESIDENCE
Date: Jan 04, 2011
By: W.D.HOGLE

13815 NW 39TH AVE, GAINESVILLE, FL 32606 Phone: 352-332-1508 Fax: 352-332-1501

Project Information

For: SCOTT ROSENBOOM CONSTRUCTION INC.

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Outside db	33 °F
Inside db	70 °F
Design TD	37 °F

Summer Design Conditions

Outside db	92 °F
Inside db	75 °F
Design TD	17 °F
Daily range	M
Relative humidity	50 %
Moisture difference	52 gr/lb

Heating Summary

Structure	22318 Btuh
Ducts	9574 Btuh
Central vent (0 cfm)	0 Btuh
Humidification	0 Btuh
Piping	0 Btuh
Equipment load	31892 Btuh

Sensible Cooling Equipment Load Sizing

Structure	23529 Btuh
Ducts	11821 Btuh
Central vent (0 cfm)	0 Btuh
Blower	0 Btuh
Use manufacturer's data	n
Rate/swing multiplier	0.97
Equipment sensible load	34290 Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	1 (Average)

	Heating	Cooling
Area (ft²)	2106	2106
Volume (ft³)	19210	19210
Air changes/hour	0.38	0.16
Equiv. AVF (cfm)	122	51

Latent Cooling Equipment Load Sizing

Structure	3401 Btuh
Ducts	2773 Btuh
Central vent (0 cfm)	0 Btuh
Equipment latent load	6174 Btuh
Equipment total load	40464 Btuh
Req. total capacity at 0.70 SHR	4.1 ton

Heating Equipment Summary

Make	AMANA
Trade	
Model	ASX130481
ARI ref no.	3186673
Efficiency	8.4 HSPF
Heating input	
Heating output	44000 Btuh @ 47°F
Temperature rise	25 °F
Actual air flow	1600 cfm
Air flow factor	0.050 cfm/Btuh
Static pressure	0 in H2O
Space thermostat	

Cooling Equipment Summary

Make	AMANA
Trade	
Cond	ASX130481
Coil	ASPF426016
ARI ref no.	3186673
Efficiency	14 SEER
Sensible cooling	32200 Btuh
Latent cooling	13800 Btuh
Total cooling	46000 Btuh
Actual air flow	1600 cfm
Air flow factor	0.045 cfm/Btuh
Static pressure	0 in H2O
Load sensible heat ratio	0.85

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Hogle's Heating & Air

AED Assessment
Entire House
HOGLE'S HEATING & AIR

Job: CONDE RESIDENCE
Date: Jan 04, 2011
By: W.D.HOGLE

13815 NW 39TH AVE, GAINESVILLE, FL 32606 Phone: 352-332-1508 Fax: 352-332-1501

Project Information

For: SCOTT ROSENBOOM CONSTRUCTION INC.

Design Conditions

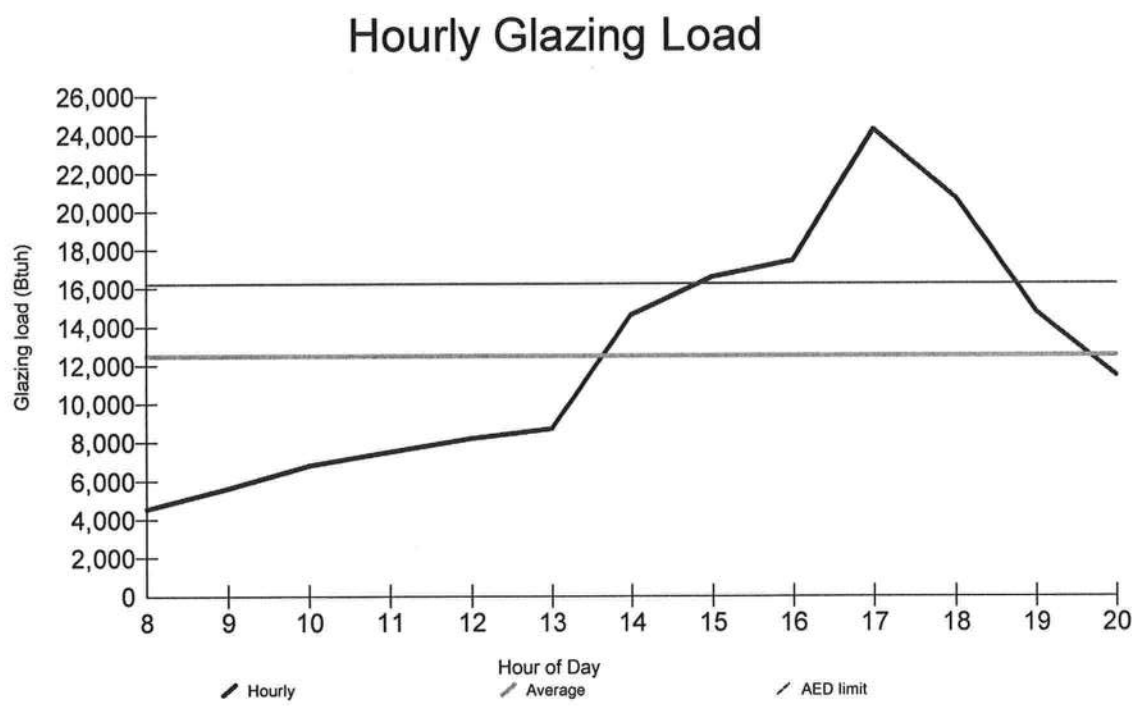
Location:
Gainesville, FL, US
Elevation: 151 ft
Latitude: 30 °N

Outdoor:	Heating	Cooling
Dry bulb (°F)	33	92
Daily range (°F)	-	19 (M)
Wet bulb (°F)	-	77
Wind speed (mph)	15.0	7.5

Indoor:	Heating	Cooling
Indoor temperature (°F)	70	75
Design TD (°F)	37	17
Relative humidity (%)	50	50
Moisture difference (gr/lb)	32.8	52.0

Infiltration:

Test for Adequate Exposure Diversity



Maximum hourly glazing load exceeds average by 94.2%.

House does not have adequate exposure diversity (AED), based on AED limit of 30%.

AED excursion: 8014 Btuh (PFG - 1.3*AFG)

Hogle's
Heating & Air

Right-J® Worksheet
Entire House
HOGLE'S HEATING & AIR

Job: CONDE RESIDENCE
Date: Jan 04, 2011
By: W.D.HOGLE

13815 NW 39TH AVE, GAINESVILLE, FL 32606 Phone: 352-332-1508 Fax: 352-332-1501

1	Room name						Entire House				BEDROOM 2			
2	Exposed wall						228.0 ft				34.0 ft			
3	Ceiling height						9.1 ft		d		8.0 ft		heat/cool	
4	Room dimensions										12.0 x 13.0 ft			
5	Room area						2106.0 ft²				156.0 ft²			
	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²)		Area (ft²) or perimeter (ft)		Load (Btuh)		Area (ft²) or perimeter (ft)		Load (Btuh)	
					Heat	Cool	Gross	N/P/S	Heat	Cool	Gross	N/P/S	Heat	Cool
6	W	12C-0sw	0.091	sw	3.37	2.20	0	-320	-1077	-703	0	-40	-135	-88
.	├─G	1D-c2ob	0.650	sw	24.05	32.09	35	55	842	1123	10	8	240	321
.	├─G	1D-c2ob	0.650	sw	24.05	24.99	18	36	433	450	0	0	0	0
.	├─G	1D-c2ob	0.650	sw	24.05	47.87	16	10	385	766	0	0	0	0
11	├─G	1D-c2ob	0.650	sw	24.05	49.98	90	45	2164	4498	30	8	721	1499
	├─G	1D-c2ob	0.650	sw	24.05	19.21	30	60	721	576	0	0	0	0
	├─G	1D-c2obd	0.650	sw	24.05	19.21	84	336	2020	1614	0	0	0	0
	├─G	1E-c2fw	0.560	sw	20.72	23.26	5	10	104	116	0	0	0	0
	└─D	11J0	0.600	sw	22.20	17.49	42	42	932	735	0	0	0	0
	C	16B-30ad	0.032	-	1.18	1.68	2106	2106	2494	3548	156	156	185	263
	F	22A-tpl	0.989	-	36.59	0.00	2106	228	8343	0	156	34	1244	0
6	c) AED excursion									8014				1109
	Envelope loss/gain								17361	20737			2256	3104
12	a) Infiltration								4957	953			644	149
	b) Room ventilation								0	0			0	0
13	Internal gains:	Occupants @ 230					8			1840	2			460
		Appliances/other								0				0
	Subtotal (lines 6 to 13)								22318	23529			2900	3713
	Less external load								0	0			0	0
	Less transfer								0	0			0	0
	Redistribution								0	0			0	0
14	Subtotal								22318	23529			2900	3713
15	Duct loads						43%	50%	9574	11821	43%	50%	1244	1865
	Total room load								31892	35350			4145	5578
	Air required (cfm)								1600	1600			208	252

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Hogle's
Heating & Air

Right-J® Worksheet
Entire House
HOGLE'S HEATING & AIR

Job: CONDE RESIDENCE
Date: Jan 04, 2011
By: W.D.HOGLE

13815 NW 39TH AVE, GAINESVILLE, FL 32606 Phone: 352-332-1508 Fax: 352-332-1501

					HALL BATHROOM 13.0 ft 8.0 ft heat/cool 12.0 x 6.0 ft 72.0 ft²				BEDROOM 3 27.0 ft 8.0 ft heat/cool 13.0 x 14.0 ft 182.0 ft²			
	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²)		Area (ft²) or perimeter (ft)		Load (Btuh)		Area (ft²) or perimeter (ft)		Load (Btuh)	
					Heat	Cool	Gross	N/P/S	Heat	Cool	Gross	N/P/S	Heat	Cool
6	W	12C-0sw	0.091	sw	3.37	2.20	0	-5	-17	-11	0	-25	-84	-55
.	G	1D-c2ob	0.650	sw	24.05	32.09	5	4	120	160	10	8	240	321
.	G	1D-c2ob	0.650	sw	24.05	24.99	0	0	0	0	0	0	0	0
.	G	1D-c2ob	0.650	sw	24.05	47.87	0	0	0	0	0	0	0	0
11	G	1D-c2ob	0.650	sw	24.05	49.98	0	0	0	0	15	4	361	750
	G	1D-c2ob	0.650	sw	24.05	19.21	0	0	0	0	0	0	0	0
	G	1D-c2obd	0.650	sw	24.05	19.21	0	0	0	0	0	0	0	0
	G	1E-c2fw	0.560	sw	20.72	23.26	0	0	0	0	0	0	0	0
	D	11J0	0.600	sw	22.20	17.49	0	0	0	0	0	0	0	0
	C	16B-30ad	0.032	-	1.18	1.68	72	72	85	121	182	182	215	307
	F	22A-tpl	0.989	-	36.59	0.00	72	13	476	0	182	27	988	0
6	c) AED excursion									148				703
	Envelope loss/gain								664	419			1721	2025
12	a) Infiltration								190	20			491	99
	b) Room ventilation								0	0			0	0
13	Internal gains:	Occupants @ 230					0			0	2			460
		Appliances/other								0				0
	Subtotal (lines 6 to 13)								854	439			2212	2584
	Less external load								0	0			0	0
	Less transfer								0	0			0	0
	Redistribution								0	0			0	0
14	Subtotal								854	439			2212	2584
15	Duct loads						43%	50%	366	221	43%	50%	949	1298
	Total room load								1220	660			3161	3882
	Air required (cfm)								61	30			159	176

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Hogle's
Heating & Air

Right-J® Worksheet
Entire House
HOGLE'S HEATING & AIR

Job: CONDE RESIDENCE
Date: Jan 04, 2011
By: W.D.HOGLE

13815 NW 39TH AVE, GAINESVILLE, FL 32606 Phone: 352-332-1508 Fax: 352-332-1501

							HALL AREA 0 ft 8.0 ft heat/cool 4.0 x 10.0 ft 40.0 ft²				FOYER 14.0 ft 8.0 ft heat/cool 14.0 x 5.0 ft 70.0 ft²			
	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²)		Area (ft²) or perimeter (ft)		Load (Btuh)		Area (ft²) or perimeter (ft)		Load (Btuh)	
					Heat	Cool	Gross	N/P/S	Heat	Cool	Gross	N/P/S	Heat	Cool
6	W	12C-0sw	0.091	sw	3.37	2.20	0	0	0	0	0	-51	-172	-112
.	G	1D-c2ob	0.650	sw	24.05	32.09	0	0	0	0	0	0	0	0
.	G	1D-c2ob	0.650	sw	24.05	24.99	0	0	0	0	0	0	0	0
.	G	1D-c2ob	0.650	sw	24.05	47.87	0	0	0	0	0	0	0	0
11	G	1D-c2ob	0.650	sw	24.05	49.98	0	0	0	0	0	0	0	0
	G	1D-c2ob	0.650	sw	24.05	19.21	0	0	0	0	30	30	721	576
	G	1D-c2obd	0.650	sw	24.05	19.21	0	84	0	0	0	0	0	0
	G	1E-c2fw	0.560	sw	20.72	23.26	0	0	0	0	0	0	0	0
	D	11J0	0.600	sw	22.20	17.49	0	0	0	0	21	21	466	367
	C	16B-30ad	0.032	-	1.18	1.68	40	40	47	67	70	70	83	118
	F	22A-tpl	0.989	-	36.59	0.00	40	0	0	0	70	14	512	0
6	c) AED excursion									0				909
	Envelope loss/gain								47	67			1611	1859
12	a) Infiltration								14	5			460	71
	b) Room ventilation								0	0			0	0
13	Internal gains:	Occupants @ 230					0			0	0			0
		Appliances/other								0				0
	Subtotal (lines 6 to 13)								61	72			2071	1930
	Less external load								0	0			0	0
	Less transfer								0	0			0	0
	Redistribution								0	0			0	0
14	Subtotal								61	72			2071	1930
15	Duct loads						43%	50%	26	36	43%	50%	889	969
	Total room load								87	109			2960	2899
	Air required (cfm)								4	5			148	131

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Hogle's
Heating & Air

Right-J® Worksheet
Entire House
HOGLE'S HEATING & AIR

Job: CONDE RESIDENCE
Date: Jan 04, 2011
By: W.D.HOGLE

13815 NW 39TH AVE, GAINESVILLE, FL 32606 Phone: 352-332-1508 Fax: 352-332-1501

							LIVING/DINING RM 24.0 ft 12.0 ft heat/cool 24.0 x 20.0 ft 480.0 ft²				KITCHEN 16.0 ft 10.0 ft heat/cool 13.0 x 17.0 ft 221.0 ft²			
	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²)		Area (ft²) or perimeter (ft)		Load (Btuh)		Area (ft²) or perimeter (ft)		Load (Btuh)	
					Heat	Cool	Gross	N/P/S	Heat	Cool	Gross	N/P/S	Heat	Cool
6	W	12C-0sw	0.091	sw	3.37	2.20	0	-84	-283	-185	0	-18	-61	-40
.	G	1D-c2ob	0.650	sw	24.05	32.09	0	0	0	0	0	0	0	0
.	G	1D-c2ob	0.650	sw	24.05	24.99	0	0	0	0	18	18	433	450
.	G	1D-c2ob	0.650	sw	24.05	47.87	0	0	0	0	0	0	0	0
11	G	1D-c2ob	0.650	sw	24.05	49.98	0	0	0	0	0	0	0	0
	G	1D-c2ob	0.650	sw	24.05	19.21	0	0	0	0	0	0	0	0
	G	1D-c2obd	0.650	sw	24.05	19.21	84	84	2020	1614	0	0	0	0
	G	1E-c2fw	0.560	sw	20.72	23.26	0	0	0	0	0	0	0	0
	D	11J0	0.600	sw	22.20	17.49	0	0	0	0	0	0	0	0
	C	16B-30ad	0.032	-	1.18	1.68	480	480	568	809	221	221	262	372
	F	22A-tpl	0.989	-	36.59	0.00	480	24	878	0	221	16	585	0
6	c) AED excursion									2202				789
	Envelope loss/gain								3184	4440			1219	1571
12	a) Infiltration								909	168			348	59
	b) Room ventilation								0	0			0	0
13	Internal gains:	Occupants @ 230					0			0	0			0
		Appliances/other								0				0
	Subtotal (lines 6 to 13)								4093	4608			1568	1630
	Less external load								0	0			0	0
	Less transfer								0	0			0	0
	Redistribution								0	0			0	0
14	Subtotal								4093	4608			1568	1630
15	Duct loads						43%	50%	1756	2315	43%	50%	672	819
	Total room load								5849	6923			2240	2449
	Air required (cfm)								293	313			112	111

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Hogle's
Heating & Air

Right-J® Worksheet
Entire House
HOGLE'S HEATING & AIR

Job: CONDE RESIDENCE
Date: Jan 04, 2011
By: W.D.HOGLE

13815 NW 39TH AVE, GAINESVILLE, FL 32606 Phone: 352-332-1508 Fax: 352-332-1501

							MASTER SUITE 34.0 ft 8.0 ft heat/cool 17.0 x 17.0 ft 289.0 ft²				MASTER BATHROOM 34.0 ft 8.0 ft heat/cool 12.0 x 17.0 ft 204.0 ft²			
	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²)		Area (ft²) or perimeter (ft)		Load (Btuh)		Area (ft²) or perimeter (ft)		Load (Btuh)	
					Heat	Cool	Gross	N/P/S	Heat	Cool	Gross	N/P/S	Heat	Cool
6	W	12C-0sw	0.091	sw	3.37	2.20	0	-40	-135	-88	0	-21	-71	-46
.	├─G	1D-c2ob	0.650	sw	24.05	32.09	10	8	240	321	0	0	0	0
.	├─G	1D-c2ob	0.650	sw	24.05	24.99	0	0	0	0	0	0	0	0
.	├─G	1D-c2ob	0.650	sw	24.05	47.87	0	0	0	0	16	5	385	766
11	├─G	1D-c2ob	0.650	sw	24.05	49.98	30	8	721	1499	0	0	0	0
	├─G	1D-c2ob	0.650	sw	24.05	19.21	0	0	0	0	0	0	0	0
	├─G	1D-c2obd	0.650	sw	24.05	19.21	0	0	0	0	0	0	0	0
	├─G	1E-c2fw	0.560	sw	20.72	23.26	0	0	0	0	5	5	104	116
	└─D	11J0	0.600	sw	22.20	17.49	0	0	0	0	0	0	0	0
	C	16B-30ad	0.032	-	1.18	1.68	289	289	342	487	204	204	242	344
	F	22A-tpl	0.989	-	36.59	0.00	289	34	1244	0	204	34	1244	0
6	c) AED excursion									1109				640
	Envelope loss/gain								2414	3328			1903	1820
12	a) Infiltration								689	166			543	88
	b) Room ventilation								0	0			0	0
13	Internal gains:	Occupants @ 230					2			460	0			0
		Appliances/other								0				0
	Subtotal (lines 6 to 13)								3103	3954			2447	1908
	Less external load								0	0			0	0
	Less transfer								0	0			0	0
	Redistribution								0	0			0	0
14	Subtotal								3103	3954			2447	1908
15	Duct loads						43%	50%	1331	1986	43%	50%	1050	959
	Total room load								4434	5940			3497	2867
	Air required (cfm)								222	269			175	130

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Hogle's
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Right-J® Worksheet
Entire House
HOGLE'S HEATING & AIR

Job: CONDE RESIDENCE
Date: Jan 04, 2011
By: W.D.HOGLE

13815 NW 39TH AVE, GAINESVILLE, FL 32606 Phone: 352-332-1508 Fax: 352-332-1501

							WIC 8.0 ft x 6.0 ft 78.0 ft² heat/cool 6.0 x 13.0 ft				LAUNDRY ROOM 8.0 ft x 8.0 ft 72.0 ft² heat/cool 8.0 x 9.0 ft			
	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²)		Area (ft²) or perimeter (ft)		Load (Btuh)		Area (ft²) or perimeter (ft)		Load (Btuh)	
					Heat	Cool	Gross	N/P/S	Heat	Cool	Gross	N/P/S	Heat	Cool
6	W	12C-0sw	0.091	sw	3.37	2.20	0	0	0	0	0	-21	-71	-46
.	—G	1D-c2ob	0.650	sw	24.05	32.09	0	0	0	0	0	0	0	0
.	—G	1D-c2ob	0.650	sw	24.05	24.99	0	0	0	0	0	0	0	0
.	—G	1D-c2ob	0.650	sw	24.05	47.87	0	0	0	0	0	0	0	0
11	—G	1D-c2ob	0.650	sw	24.05	49.98	0	0	0	0	0	0	0	0
	—G	1D-c2ob	0.650	sw	24.05	19.21	0	0	0	0	0	0	0	0
	—G	1D-c2obd	0.650	sw	24.05	19.21	0	0	0	0	0	0	0	0
	—G	1E-c2fw	0.560	sw	20.72	23.26	0	0	0	0	0	0	0	0
	—D	11J0	0.600	sw	22.20	17.49	0	0	0	0	21	21	466	367
	C	16B-30ad	0.032	-	1.18	1.68	78	78	92	131	72	72	85	121
	F	22A-tpl	0.989	-	36.59	0.00	78	6	220	0	72	8	293	0
6	c) AED excursion									0				0
	Envelope loss/gain								312	131			773	442
12	a) Infiltration								89	10			221	33
	b) Room ventilation								0	0			0	0
13	Internal gains:	Occupants @ 230					0			0	0			0
		Appliances/other								0				0
	Subtotal (lines 6 to 13)								401	141			994	476
	Less external load								0	0			0	0
	Less transfer								0	0			0	0
	Redistribution								0	0			0	0
14	Subtotal								401	141			994	476
15	Duct loads						43%	50%	172	71	43%	50%	427	239
	Total room load								573	212			1421	714
	Air required (cfm)								29	10			71	32

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Hogle's
Heating & Air

Right-J® Worksheet
Entire House
HOGLE'S HEATING & AIR

Job: CONDE RESIDENCE
Date: Jan 04, 2011
By: W.D.HOGLE

13815 NW 39TH AVE, GAINESVILLE, FL 32606 Phone: 352-332-1508 Fax: 352-332-1501

							MASTER HALL AREA 0 ft 8.0 ft heat/cool 20.0 x 4.0 ft 80.0 ft²				OFFICE 12.0 ft 8.0 ft heat/cool 12.0 x 9.0 ft 108.0 ft²			
	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²)		Area (ft²) or perimeter (ft)		Load (Btuh)		Area (ft²) or perimeter (ft)		Load (Btuh)	
					Heat	Cool	Gross	N/P/S	Heat	Cool	Gross	N/P/S	Heat	Cool
6	W	12C-0sw	0.091	sw	3.37	2.20	0	0	0	0	0	-15	-51	-33
.	G	1D-c2ob	0.650	sw	24.05	32.09	0	0	0	0	0	0	0	0
.	G	1D-c2ob	0.650	sw	24.05	24.99	0	0	0	0	0	0	0	0
.	G	1D-c2ob	0.650	sw	24.05	47.87	0	0	0	0	0	0	0	0
11	G	1D-c2ob	0.650	sw	24.05	49.98	0	0	0	0	15	4	361	750
	G	1D-c2ob	0.650	sw	24.05	19.21	0	0	0	0	0	0	0	0
	G	1D-c2obd	0.650	sw	24.05	19.21	0	0	0	0	0	0	0	0
	G	1E-c2fw	0.560	sw	20.72	23.26	0	0	0	0	0	0	0	0
	D	11J0	0.600	sw	22.20	17.49	0	0	0	0	0	0	0	0
	C	16B-30ad	0.032	-	1.18	1.68	80	80	95	135	108	108	128	182
	F	22A-tpl	0.989	-	36.59	0.00	80	0	0	0	108	12	439	0
6	c) AED excursion									0				406
	Envelope loss/gain								95	135			877	1304
12	a) Infiltration								27	10			250	67
	b) Room ventilation								0	0			0	0
13	Internal gains:	Occupants @ 230					0			0	2			460
		Appliances/other								0				0
	Subtotal (lines 6 to 13)								122	145			1128	1832
	Less external load								0	0			0	0
	Less transfer								0	0			0	0
	Redistribution								0	0			0	0
14	Subtotal								122	145			1128	1832
15	Duct loads						43%	50%	52	73	43%	50%	484	920
	Total room load								174	218			1611	2752
	Air required (cfm)								9	10			81	125

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Hogle's
Heating & Air

Right-J® Worksheet
Entire House
HOGLE'S HEATING & AIR

Job: CONDE RESIDENCE
Date: Jan 04, 2011
By: W.D.HOGLE

13815 NW 39TH AVE, GAINESVILLE, FL 32606 Phone: 352-332-1508 Fax: 352-332-1501

1	Room name					PWDR ROOM								
2	Exposed wall					6.0 ft								
3	Ceiling height					8.0 ft		heat/cool						
4	Room dimensions					6.0 x 9.0 ft								
5	Room area					54.0 ft²								
	Ty	Construction number	U-value (Btuh/ft²-°F)	Or	HTM (Btuh/ft²)		Area (ft²) or perimeter (ft)		Load (Btuh)		Area or perimeter		Load	
					Heat	Cool	Gross	N/P/S	Heat	Cool	Gross	N/P/S	Heat	Cool
6	W	12C-0sw	0.091	sw	3.37	2.20	0	0	0	0				
.	—G	1D-c2ob	0.650	sw	24.05	32.09	0	0	0	0				
.	—G	1D-c2ob	0.650	sw	24.05	24.99	0	0	0	0				
.	—G	1D-c2ob	0.650	sw	24.05	47.87	0	0	0	0				
11	—G	1D-c2ob	0.650	sw	24.05	49.98	0	0	0	0				
	—G	1D-c2ob	0.650	sw	24.05	19.21	0	0	0	0				
	—G	1D-c2obd	0.650	sw	24.05	19.21	0	0	0	0				
	—G	1E-c2fw	0.560	sw	20.72	23.26	0	0	0	0				
	—D	11J0	0.600	sw	22.20	17.49	0	0	0	0				
	C	16B-30ad	0.032	-	1.18	1.68	54	54	64	91				
	F	22A-tpl	0.989	-	36.59	0.00	54	6	220	0				
6	c) AED excursion									0				
	Envelope loss/gain								283	91				
12	a) Infiltration								81	7				
	b) Room ventilation								0	0				
13	Internal gains:	Occupants @ 230					0			0				
		Appliances/other								0				
	Subtotal (lines 6 to 13)								364	98				
	Less external load								0	0				
	Less transfer								0	0				
	Redistribution								0	0				
14	Subtotal								364	98				
15	Duct loads						43%	50%	156	49				
	Total room load								521	147				
	Air required (cfm)								26	7				

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PRODUCT APPROVAL SPECIFICATION SHEET

Location: _____ **Project Name:**

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are **applying for a building permit on or after April 1, 2004**. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging	MASONITE		FL 4940
2. Sliding			
3. Sectional			
4. Roll up			
5. Automatic			
6. Other			
B. WINDOWS	GENERAL	ALUMINUM	
1. Single hung			FL 8171
2. Horizontal Slider			
3. Casement			
4. Double Hung			
5. Fixed			FL 8164
6. Awning			
7. Pass -through			
8. Projected			
9. Mullion			
10. Wind Breaker			
11 Dual Action			
12. Other			
C. PANEL WALL			
1. Siding	HARDY		FL 8892
2. Soffits			
3. EIFS			
4. Storefronts			
5. Curtain walls			
6. Wall louver			
7. Glass block			
8. Membrane			
9. Greenhouse			
10. Other			
D. ROOFING PRODUCTS			
1. Asphalt Shingles	TAMKO	ARCHITECTURAL	FL 1956.1
2. Underlayments	UNDERCOAT	30lb	FL 2346
3. Roofing Fasteners			
4. Non-structural Metal			
Rf 5. Built-Up Roofing			
6. Modified Bitumen			
7. Single Ply Roofing Sys			
8. Roofing Tiles			
9. Roofing Insulation			
10. Waterproofing			
11. Wood shingles /shakes			
12. Roofing Slate			

COLUMBIA COUNTY BUILDING DEPARTMENT
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PLANS EXAMINER

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys			
14. Cements-Adhesives – Coatings			
15. Roof Tile Adhesive			
16. Spray Applied Polyurethane Roof			
17. Other			
E. SHUTTERS	N/A		
1. Accordion			
2. Bahama			
3. Storm Panels			
4. Colonial			
5. Roll-up			
6. Equipment			
7. Others			
F. SKYLIGHTS	N/A		
1. Skylight			
2. Other			
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor			
2. Truss plates			01-0327041
3. Engineered lumber			
4. Railing			
5. Coolers-freezers			
6. Concrete Admixtures			
7. Material			
8. Insulation Forms			
9. Plastics			
10. Deck-Roof			
11. Wall			
12. Sheds			
13. Other			
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspect

Contractor or Contractor's Authorized Agent Signature

Scott Rosenbaum JAN 26/2011

Print Name Date

Permit # (FOR STAFF USE ONLY)

AHRI CERTIFIED™
www.ahridirectory.org

Certificate of Product Ratings

AHRI Certified Reference Number: 3186673 Date: 1/6/2011

Product: Split System: Heat Pump with Remote Outdoor Unit-Air-Source
Outdoor Unit Model Number: ASZ130481A*
Indoor Unit Model Number: ASPF426016B*+TXV
Manufacturer: AMANA HEATING AND AIR CONDITIONING
Trade/Brand name: ASZ13 SERIES

Manufacturer responsible for the rating of this system combination is AMANA HEATING AND AIR CONDITIONING

Rated as follows in accordance with AHRI Standard 210/240-2006 for Unitary Air-Conditioning and Air-Source Heat Pump Equipment and subject to verification of rating accuracy by AHRI-sponsored, independent, third party testing:

Cooling Capacity (Btuh):	46000
EER Rating (Cooling):	11.50
SEER Rating (Cooling):	14.00
Heating Capacity(Btuh) @ 47 F:	44000
Region IV HSPF Rating (Heating):	8.40
Heating Capacity(Btuh) @ 17 F:	27000

* Ratings followed by an asterisk (*) indicate a voluntary rerate of previously published data, unless accompanied with a VWAS, which indicates an involuntary rerate.

CERTIFICATE VERIFICATION
The information for the model cited on this certificate can be verified at www.ahridirectory.org, click on "Verify Certificate" link and enter the AHRI Certified Reference Number and the date on which the certificate was issued, which is listed above, and the Certificate No., which is listed below.

AHRI Air-Conditioning, Heating, and Refrigeration Institute

CERTIFICATE NO.: 129387922389684446

ENGINEERING BY
TRENCO
A MiTek Affiliate

Important Notice: If visually graded lumber is used for the trusses covered by these designs, see "SPIB Important Notice, Dated July 28, 2010" (reprinted at www.mii.com) before use. Trenco does not warrant third-party lumber design values.

RE: RSNCONDE -

Trenco
818 Soundside Rd
Edenton, NC 27932

Site Information:
Project Customer: SCOTT ROSENBOOM Project Name: CONDE RES
Lot/Block: Subdivision:
Address: 245 SW WAFFLE GLEN
City: FT WHITE State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.
Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):
Design Code: FBC2007/TPI2002 Design Program: MiTek 20/20 7.2
Wind Code: ASCE 7-05 Wind Speed: 110 mph Floor Load: N/A psf
Roof Load: 40.0 psf

This package includes 43 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
This document processed per section 61G15-23.003 of the Florida Board of Professionals Rules

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	E5927665	A1	1/10/011	18	E5927682	CJ03	1/10/011
2	E5927666	A1ET	1/10/011	19	E5927683	CJ09	1/10/011
3	E5927667	A2	1/10/011	20	E5927684	D1	1/10/011
4	E5927668	A3	1/10/011	21	E5927685	D	1/10/011
5	E5927669	A4	1/10/011	22	E5927686	DET	1/10/011
6	E5927670	A5	1/10/011	23	E5927687	E1	1/10/011
7	E5927671	A6	1/10/011	24	E5927688	E2	1/10/011
8	E5927672	A6ET	1/10/011	25	E5927689	E3	1/10/011
9	E5927673	A	1/10/011	26	E5927690	EJ2	1/10/011
10	E5927674	B1	1/10/011	27	E5927691	EJ5	1/10/011
11	E5927675	B2	1/10/011	28	E5927692	EJ5GT	1/10/011
12	E5927676	B3	1/10/011	29	E5927693	EJ7	1/10/011
13	E5927677	B4	1/10/011	30	E5927694	EJ7A	1/10/011
14	E5927678	B5	1/10/011	31	E5927695	EJ7B	1/10/011
15	E5927679	C1	1/10/011	32	E5927696	EJ7C	1/10/011
16	E5927680	C	1/10/011	33	E5927697	EJ7GT	1/10/011
17	E5927681	CET	1/10/011	34	E5927698	F1	1/10/011

COLUMBIA COUNTY BUILDING DEPARTMENT
Received for FILE COPY
Code Compliance
PLANS EXAMINER

The truss drawing(s) referenced above have been prepared by TRENCO under my direct supervision based on the parameters provided by Santa Fe Truss.

Truss Design Engineer's Name: Strzyzewski, Marvin
My license renewal date for the state of is February 28, 2013.

MARVIN A. STRZYZEWSKI
LICENSE
No 43144
STATE OF FLORIDA
PROFESSIONAL ENGINEER

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.

FL Cert. #7239

January 10,2011
Strzyzewski, Marvin

RE: RSNCONDE -

Site Information:

Project Customer: SCOTT ROSENBOOM Project Name: CONDE RES
Lot/Block: Subdivision:
Address: 245 SW WAFFLE GLEN
City: FT WHITE State: FL

No.	Seal#	Truss Name	Date
35	E5927699	F	1/10/011
36	E5927700	FET	1/10/011
37	E5927701	FGT	1/10/011
38	E5927702	G1	1/10/011
39	E5927703	G	1/10/011
40	E5927704	GET	1/10/011
41	E5927705	J01	1/10/011
42	E5927706	J03	1/10/011
43	E5927707	J05	1/10/011

Job	Truss	Truss Type	Qty	Ply	E5927665
RSNCONDE	A1	SPECIAL	3	1	

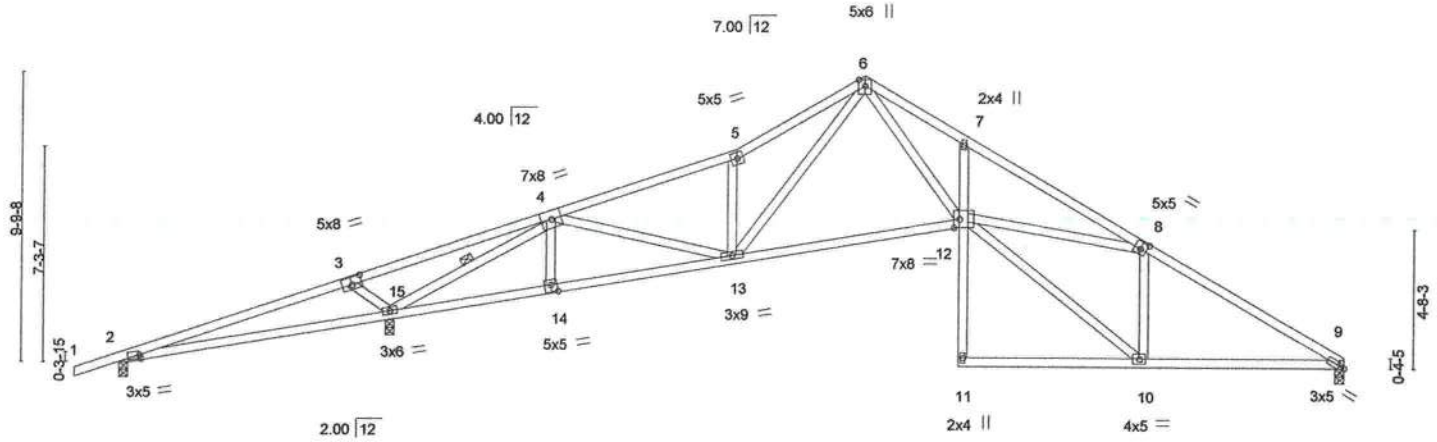
Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:10 2011 Page 1

ID:TEdFcz9TTEFDQ60dAp4fjazwlbN-TEdFcz9TTEFDQ60dAp4fja8MnQI9nAJkeEL7cgzwlbN

1-6-0	7-10-6	14-7-4	20-10-9	25-2-0	28-4-12	34-7-9	41-4-0
1-6-0	7-10-6	6-8-14	6-3-5	4-3-7	3-2-12	6-2-13	6-8-7

Scale = 1:75.7



9-1-12	14-7-4	20-10-9	28-4-12	34-7-9	41-4-0
9-1-12	5-5-8	6-3-5	7-6-3	6-2-13	6-8-7

Plate Offsets (X,Y): [2:0-0-4,0-1-8], [3:0-4-0,0-3-0], [8:0-2-8,0-3-0], [9:0-2-4,0-1-8], [12:0-2-4,0-3-8], [14:0-2-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.89	Vert(LL)	0.27	2-15	>407	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.74	12-13	>523	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.82	Horz(TL)	0.40	9	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 217 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
7-11: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 2-15.
WEBS 1 Row at midpt 4-15

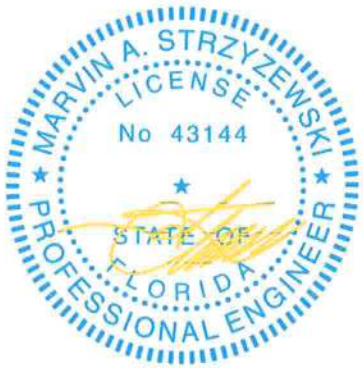
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=194/0-3-8 (min. 0-1-8), 9=1205/0-3-8 (min. 0-1-8), 15=1983/0-3-8 (min. 0-2-5)
Max Horz 2=329(LC 4)
Max Uplift 2=-261(LC 3), 9=-161(LC 6), 15=-459(LC 3)
Max Grav 2=245(LC 7), 9=1205(LC 1), 15=1983(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-109/1049, 3-4=-192/1456, 4-5=-2477/255, 5-6=-2756/374, 6-7=-3694/346, 7-8=-3745/274, 8-9=-2014/270
BOT CHORD 2-15=-929/104, 14-15=-134/1585, 13-14=-139/1599, 12-13=-29/1717, 11-12=0/99, 7-12=-262/139, 10-11=-0/14, 9-10=-153/1647
WEBS 3-15=-556/273, 4-15=-3412/373, 4-14=0/147, 4-13=-44/746, 5-13=-893/208, 6-13=-227/1082, 6-12=-226/2543, 10-12=-197/2086, 8-12=-10/1486, 8-10=-1105/150

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 261 lb uplift at joint 2, 161 lb uplift at joint 9 and 459 lb uplift at joint 15.
7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 15.
8) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	
RSNCONDE	A1ET	GABLE	1	1	E5927666
					Job Reference (optional)

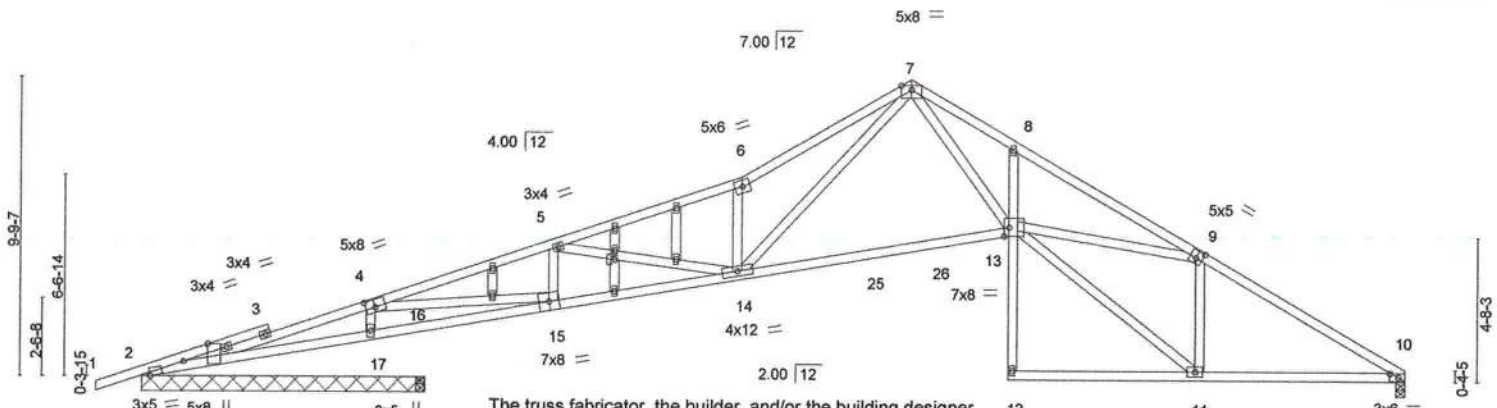
SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL, .

7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 15:48:24 2011 Page 1

ID:0vHngUiDrzk0JZqnhHb5GOzwjt3-G_yJOdqBjsagb?0sB1H84DR_IEyL_gWkhFJhi0zwjrb

1-6-0	7-6-1	13-6-1	19-7-13	25-2-0	28-4-12	34-7-9	41-4-0
1-6-0	7-6-1	6-0-0	6-1-12	5-6-3	3-2-12	6-2-13	6-8-7

Scale = 1:73.3



The truss fabricator, the builder, and/or the building designer are solely responsible for the review and approval of all pertinent data reflected on this drawing, such as, but not limited to: pitch, dimensions, load, concentrated floor load check, etc., before fabrication to ensure accuracy and full compliance with existing applicable building code requirements.

7-6-1	8-11-12	13-6-1	19-7-13	28-4-12	34-7-9	41-4-0
7-6-1	1-5-11	4-6-5	6-1-12	8-8-15	6-2-13	6-8-7

Plate Offsets (X,Y): [2:1-1-15,Edge], [2:0-6-14,Edge], [4:0-3-12,0-3-0], [9:0-2-8,0-3-0], [10:0-3-3,0-1-8], [13:0-2-0,0-3-8], [20:0-2-0,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	-0.48	13-14	>799	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-1.18	13-14	>325	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.93	Horz(TL)	0.48	10	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 226 lb	FT = 15%

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except* 8-12: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except* 4-15: 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-7-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-2-9 oc bracing.

REACTIONS (lb/size) 10=1319/0-3-8 (min. 0-1-9), 17=1686/9-3-8 (min. 0-2-2), 2=142/9-3-8 (min. 0-2-2), 16=324/0-3-8 (min. 0-1-8)
Max Horz 2=382(LC 4)
Max Uplift 10=-384(LC 6), 17=-553(LC 5), 2=-182(LC 3), 16=-73(LC 5)
Max Grav 10=1319(LC 1), 17=1686(LC 1), 2=177(LC 9), 16=324(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/25, 2-3=-571/1627, 3-4=-558/1721, 4-5=-2566/742, 5-6=-3415/891, 6-7=-3829/1148, 7-8=-4306/1016, 8-9=-4341/950, 9-10=-2235/632
BOT CHORD 2-17=-1601/457, 16-17=-1393/383, 15-16=-1442/399, 14-15=-702/2469, 14-25=-302/2028, 25-26=-297/2036, 13-26=-296/2057, 12-13=0/99, 8-13=-267/212, 11-12=-3/9, 10-11=-438/1837
WEBS 4-17=-1790/650, 4-15=-1061/3771, 5-15=-742/256, 5-14=-66/777, 6-14=-1167/487, 7-14=-677/1835, 7-13=-649/2911, 11-13=-558/2335, 9-13=-236/1808, 9-11=-1257/362

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
8) Bearing at joint(s) 17, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 384 lb uplift at joint 10, 553 lb uplift at joint 17, 182 lb uplift at joint 2 and 73 lb uplift at joint 16.
10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 16.
11) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

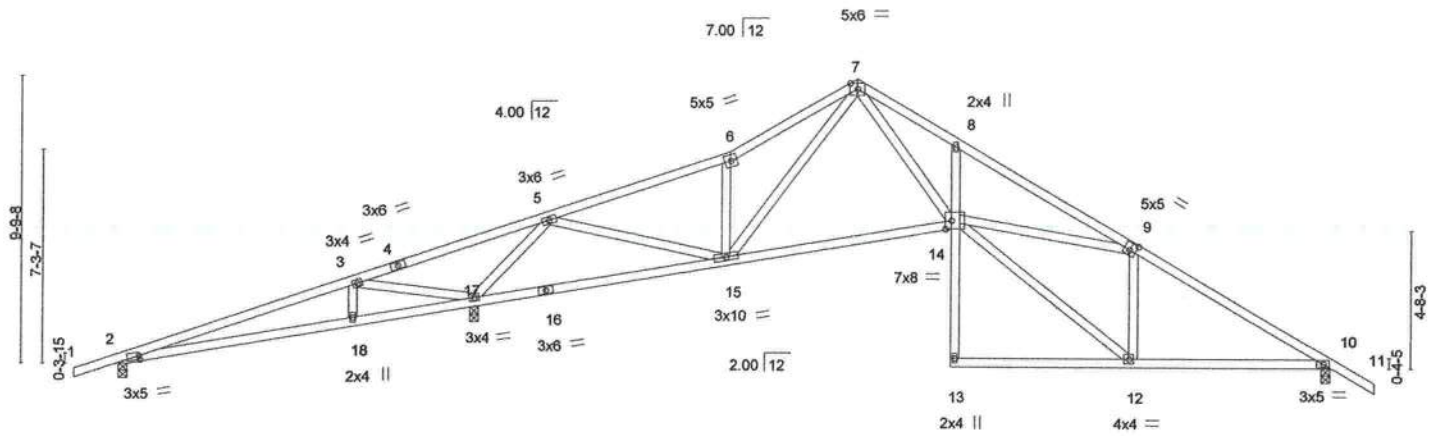
Job	Truss	Truss Type	Qty	Ply	E5927667
RSNCONDE	A2	SPECIAL	4	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:13 2011 Page 1

ID:upINF?CLm9dnHalCrxdMKDzwlbK-upINF?CLm9dnHalCrxdMKDmt4dP?_YZAKCZnD?zwlbK

1-6-0	8-0-5	14-8-10	20-10-9	25-2-0	28-4-12	34-7-9	41-4-0	42-10-0
1-6-0	8-0-5	6-8-4	6-1-15	4-3-7	3-2-12	6-2-13	6-8-7	1-6-0

Scale = 1:76.4



7-10-6	12-1-12	20-10-9	28-4-12	34-7-9	41-4-0
7-10-6	4-3-6	8-8-13	7-6-3	6-2-13	6-8-7

Plate Offsets (X,Y): [2:0-0-4,0-1-8], [9:0-2-8,0-3-0], [14:0-2-8,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.89	Vert(LL)	0.19	2-18	>746	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.47	14-15	>743	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.72	Horz(TL)	0.23	10	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 218 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
8-13: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 10=1139/0-3-8 (min. 0-1-8), 17=2057/0-3-8 (min. 0-2-7), 2=285/0-3-8 (min. 0-1-8)
Max Horz 2=327(LC 4)
Max Uplift 10=-216(LC 6), 17=-525(LC 3), 2=-312(LC 3)
Max Grav 10=1139(LC 1), 17=2057(LC 1), 2=336(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=0/570, 3-4=-464/1731, 4-5=-441/1845, 5-6=-1453/50, 6-7=-1619/146, 7-8=-2818/195, 8-9=-2868/112, 9-10=-1669/230, 10-11=0/45
BOT CHORD 2-18=-525/0, 17-18=-524/0, 16-17=-218/301, 15-16=-191/308, 14-15=0/1202, 13-14=0/100, 8-14=-269/144, 12-13=0/11, 10-12=-74/1349
WEBS 3-18=-194/254, 3-17=-1259/853, 5-17=-2345/419, 5-15=-124/1519, 6-15=-636/155, 7-15=-66/377, 7-14=-127/2106, 12-14=-96/1708, 9-14=0/1029, 9-12=-883/97

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 216 lb uplift at joint 10, 525 lb uplift at joint 17 and 312 lb uplift at joint 2.
7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 17.
8) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

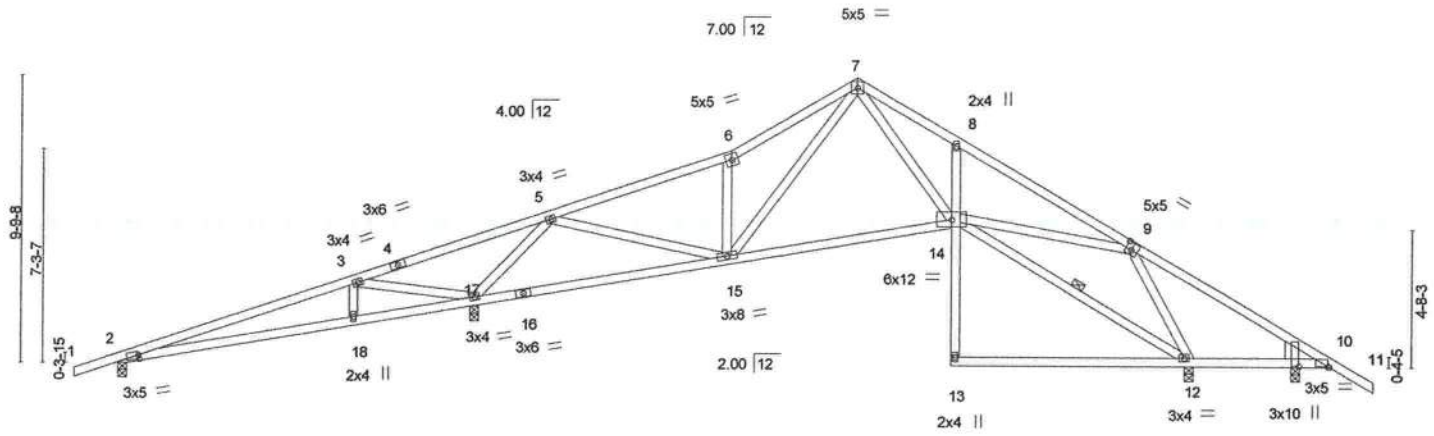
Job	Truss	Truss Type	Qty	Ply	E5927668
RSNCONDE	A3	SPECIAL	5	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:15 2011 Page 1

ID:qCQ8fgDbImtVXuvbzMfqQezwlbl-qCQ8fgDbImtVXuvbzMfqQesEOR0wSUBTnW2uluzwlbl

1-6-0	8-0-5	14-8-10	20-10-9	25-2-0	28-4-12	34-6-8	41-4-0	42-10-0
1-6-0	8-0-5	6-8-4	6-1-15	4-3-7	3-2-12	6-1-12	6-9-8	1-6-0

Scale = 1:76.4



7-10-6	12-1-12	20-10-9	28-4-12	36-6-4	40-4-0	41-4-0
7-10-6	4-3-6	8-8-13	7-6-3	8-1-8	3-9-12	1-0-0

Plate Offsets (X,Y): [2:0-0-4,0-1-8], [9:0-2-4,0-3-0], [10:0-0-5,Edge], [10:0-0-4,1-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.77	Vert(LL)	0.20	2-18	>727	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.30	12-13	>986	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.11	12	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 224 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
8-13: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
WEDGE
Right: 2 X 8 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 12-14

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 17=1654/0-3-8 (min. 0-1-15), 12=1712/0-3-8 (min. 0-2-0), 2=378/0-3-8 (min. 0-1-8), 10=-264/0-3-8 (min. 0-1-8)
Max Horz 2=327(LC 4)
Max Uplift 17=-527(LC 3), 12=-164(LC 6), 2=-311(LC 3), 10=-264(LC 1)
Max Grav 17=1654(LC 1), 12=1712(LC 1), 2=395(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-218/381, 3-4=-491/1175, 4-5=-468/1289, 5-6=-995/33, 6-7=-1113/127, 7-8=-1226/94, 8-9=-1266/37, 9-10=-35/1160, 10-11=0/45
BOT CHORD 2-18=-442/158, 17-18=-403/148, 16-17=-120/298, 15-16=-93/305, 14-15=0/653, 13-14=0/140, 8-14=-288/144, 12-13=0/16, 10-12=-902/74
WEBS 3-18=-195/252, 3-17=-1234/853, 5-17=-1733/398, 5-15=-104/941, 6-15=-530/151, 7-15=-72/490, 7-14=-35/655, 12-14=-262/13, 9-14=0/1217, 9-12=-1561/162

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever right exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 527 lb uplift at joint 17, 164 lb uplift at joint 12, 311 lb uplift at joint 2 and 264 lb uplift at joint 10.
7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 17.
8) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

MARVIN A. STRZYZEWSKI LICENSE No 43144 STATE OF FLORIDA PROFESSIONAL ENGINEER

FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

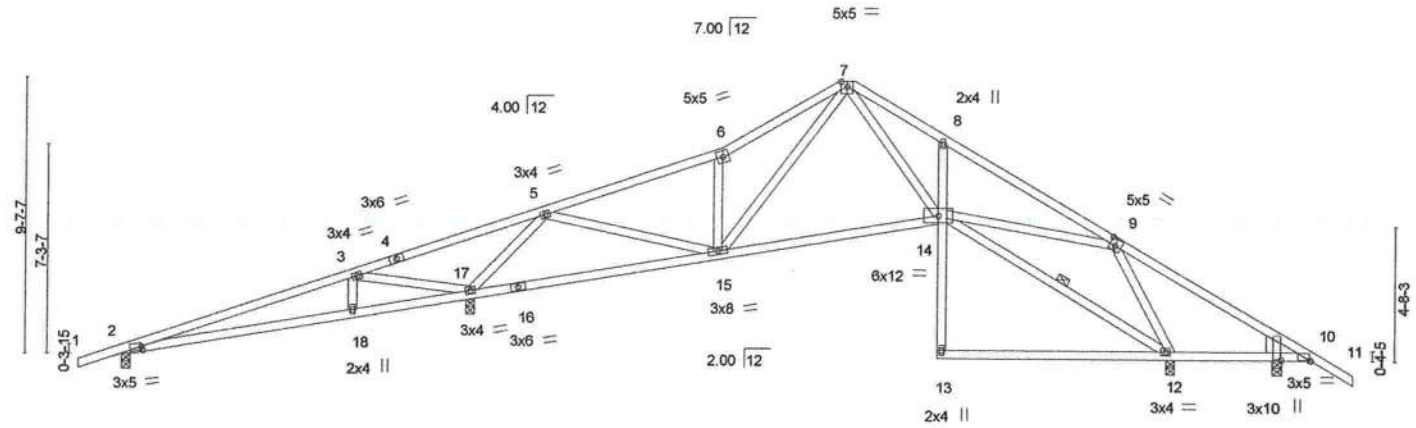
Job	Truss	Truss Type	Qty	Ply	E5927669
RSNCONDE	A4	SPECIAL	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:16 2011 Page 1

ID:IO_Wt0EE34?M82UnX3A3yrzwlbH-IO_Wt0EE34?M82UnX3A3yrOP8rL9BxRc0AoSqKzwlbH

1-6-0	8-0-5	14-8-10	20-10-9	25-2-0	28-4-12	34-6-8	41-4-0	42-10-0
1-6-0	8-0-5	6-8-4	6-1-15	4-3-7	3-2-12	6-1-12	6-9-8	1-6-0

Scale = 1:78.1



7-10-6	12-1-12	20-10-9	28-4-12	36-6-4	40-4-0	41-4-0
7-10-6	4-3-6	8-8-13	7-6-3	8-1-8	3-9-12	1-0-0

Plate Offsets (X,Y): [2:0-0-4,0-1-8], [9:0-2-4,0-3-0], [10:0-0-5,Edge], [10:0-0-4,1-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.77	Vert(LL)	0.20	2-18	>727	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.30	12-13	>986	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.11	12	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 224 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
8-13: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
WEDGE
Right: 2 X 8 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 12-14

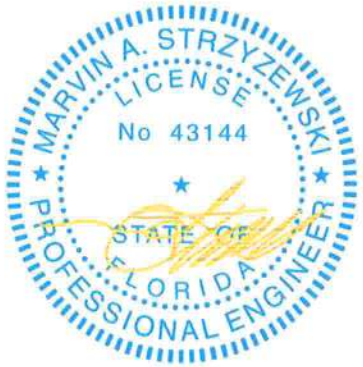
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 17=1654/0-3-8 (min. 0-1-15), 12=1712/0-3-8 (min. 0-2-0), 2=378/0-3-8 (min. 0-1-8), 10=-264/0-3-8 (min. 0-1-8)
Max Horz 2=327(LC 4)
Max Uplift 17=-527(LC 3), 12=-164(LC 6), 2=-311(LC 3), 10=-264(LC 1)
Max Grav 17=1654(LC 1), 12=1712(LC 1), 2=395(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-218/381, 3-4=-491/1175, 4-5=-468/1289, 5-6=-995/33, 6-7=-1113/127, 7-8=-1226/94, 8-9=-1266/37, 9-10=-35/1160, 10-11=0/45
BOT CHORD 2-18=-442/158, 17-18=-403/148, 16-17=-120/298, 15-16=-93/305, 14-15=0/653, 13-14=0/140, 8-14=-288/144, 12-13=0/16, 10-12=-902/74
WEBS 3-18=-195/252, 3-17=-1234/853, 5-17=-1733/398, 5-15=-104/941, 6-15=-530/151, 7-15=-72/490, 7-14=-35/655, 12-14=-262/13, 9-14=0/1217, 9-12=-1561/162

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever right exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 527 lb uplift at joint 17, 164 lb uplift at joint 12, 311 lb uplift at joint 2 and 264 lb uplift at joint 10.
7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 17.
8) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

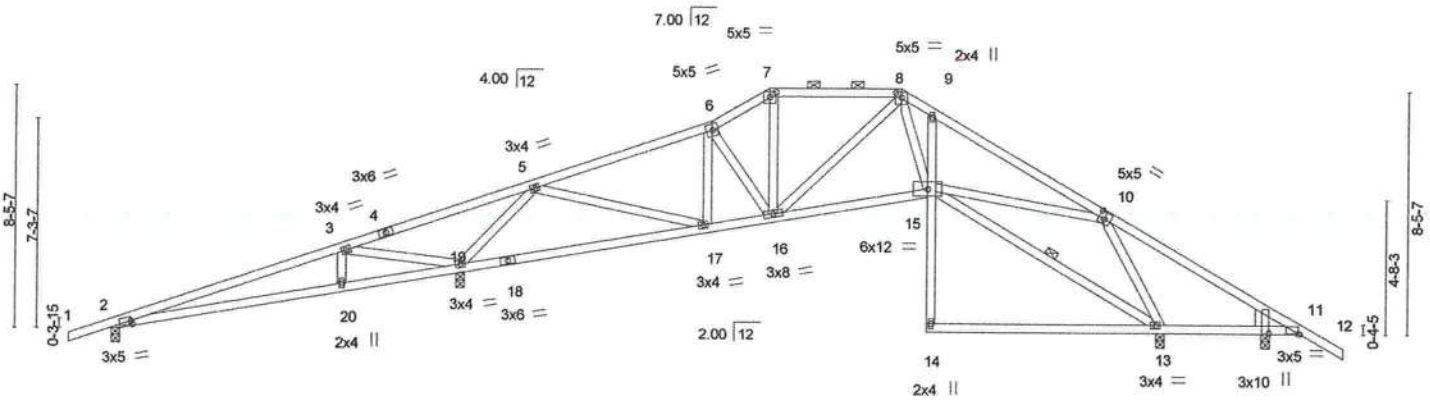
Job	Truss	Truss Type	Qty	Ply	E5927670
RSNCONDE	A5	SPECIAL	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:17 2011 Page 1

ID:maYu4MFsqN7DmB3_4nilV3zwlbG-maYu4MFsqN7DmB3_4nilV3xb5Ei?wOimFqX?MmzwlbG

1-6-0	8-0-5	14-8-10	20-10-9	22-10-8	27-5-8	28-4-12	34-6-8	41-4-0	42-10-0
1-6-0	8-0-5	6-8-4	6-1-15	1-11-15	4-7-0	0-11-4	6-1-12	6-9-8	1-6-0

Scale = 1:78.1



7-10-6	12-1-12	20-10-9	22-10-8	28-4-12	36-6-4	40-4-0	41-4-0
7-10-6	4-3-6	8-8-13	1-11-15	5-6-4	8-1-8	3-9-12	1-0-0

Plate Offsets (X,Y): [2:0-0-4,0-1-8], [7:0-2-8,0-2-1], [8:0-2-8,0-2-1], [10:0-2-4,0-3-0], [11:0-0-5,Edge], [11:0-0-4,1-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	0.20	2-20	>730	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.30	17-19	>982	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.11	13	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 229 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
9-14: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3
WEDGE
Right: 2 X 8 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-11 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 7-8.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 13-15

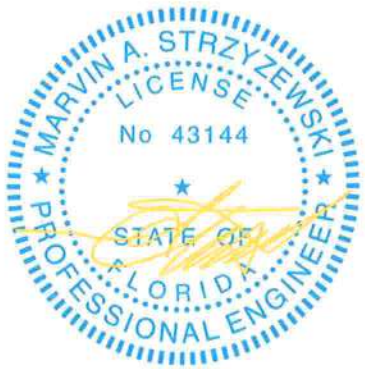
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 19=1646/0-3-8 (min. 0-1-15), 13=1677/0-3-8 (min. 0-2-0), 2=388/0-3-8 (min. 0-1-8), 11=-231/0-3-8 (min. 0-1-8)
Max Horz 2=290(LC 4)
Max Uplift 19=-585(LC 3), 13=-181(LC 3), 2=-296(LC 3), 11=-269(LC 7)
Max Grav 19=1646(LC 1), 13=1677(LC 1), 2=400(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-240/328, 3-4=-507/1110, 4-5=-484/1224, 5-6=-1022/51, 6-7=-974/91, 7-8=-820/70, 8-9=-1245/97, 9-10=-1320/59, 10-11=-33/1097, 11-12=0/45
BOT CHORD 2-20=-391/179, 19-20=-352/169, 18-19=-85/283, 17-18=-57/290, 16-17=-28/928, 15-16=-6/902, 14-15=0/140, 9-15=-222/140, 13-14=0/19, 11-13=-848/74
WEBS 3-20=-196/245, 3-19=-1223/858, 5-19=-1712/473, 5-17=-135/917, 6-17=-243/100, 6-16=-206/35, 7-16=-7/293, 8-16=-164/21, 8-15=-38/555, 13-15=-233/35, 10-15=0/1211, 10-13=-1556/165

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); cantilever right exposed ; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 585 lb uplift at joint 19, 181 lb uplift at joint 13, 296 lb uplift at joint 2 and 269 lb uplift at joint 11.
8) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 19.
9) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

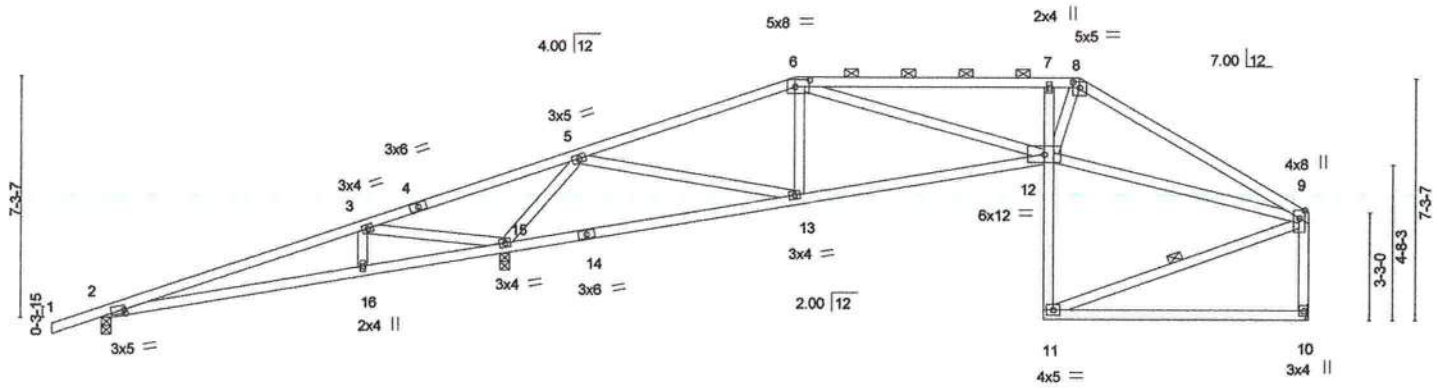
Job	Truss	Truss Type	Qty	Ply	E5927671
RSNCONDE	A6	SPECIAL	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:18 2011 Page 1

ID:En6GliGUbhF4OLdAeUDX1GzwlbF-En6GliGUbhF4OLdAeUDX1GUmqe65ft5vTUHYvDzwlbF

-1-6-0	7-10-6	14-4-6	20-10-7	28-4-12	29-5-8	36-4-8
1-6-0	7-10-6	6-6-1	6-6-1	7-6-5	1-0-12	6-11-0

Scale = 1:67.6



7-10-6	12-1-12	20-10-7	28-4-12	36-4-8
7-10-6	4-3-6	8-8-11	7-6-5	7-11-12

Plate Offsets (X,Y): [2:0-0-4,0-1-8], [6:0-5-4,0-2-8], [8:0-2-8,0-2-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	0.18	2-16	>811	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.37	10-11	>772	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.22	10	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 194 lb	FT = 15%

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
7-11: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
9-10: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-9-10 oc purlins, except end verticals, and 2-0-0 oc purlins (3-10-7 max.): 6-8.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 9-11

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 10=853/Mechanical, 15=1775/0-3-8 (min. 0-2-2), 2=359/0-3-8 (min. 0-1-8)
Max Horz 2=270(LC 4)
Max Uplift 10=-79(LC 3), 15=-530(LC 3), 2=-206(LC 5)
Max Grav 10=853(LC 1), 15=1775(LC 1), 2=370(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/26, 2-3=-96/264, 3-4=-544/1275, 4-5=-522/1386, 5-6=-1238/121, 6-7=-1948/255, 7-8=-1867/246, 8-9=-1923/212, 9-10=-774/102
BOT CHORD 2-16=-350/42, 15-16=-312/33, 14-15=-192/201, 13-14=-164/208, 12-13=-98/1122, 11-12=0/158, 7-12=-457/287, 10-11=-15/51
WEBS 3-16=-192/243, 3-15=-1242/719, 5-15=-1824/461, 5-13=-247/1274, 6-13=-276/145, 6-12=-130/929, 8-12=-264/903, 9-11=-29/18, 9-12=-124/1583

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Refer to girder(s) for truss to truss connections.
7) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 79 lb uplift at joint 10, 530 lb uplift at joint 15 and 206 lb uplift at joint 2.
9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 15.
10) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	
RSNCONDE	A6ET	GABLE	1	1	E5927672
					Job Reference (optional)

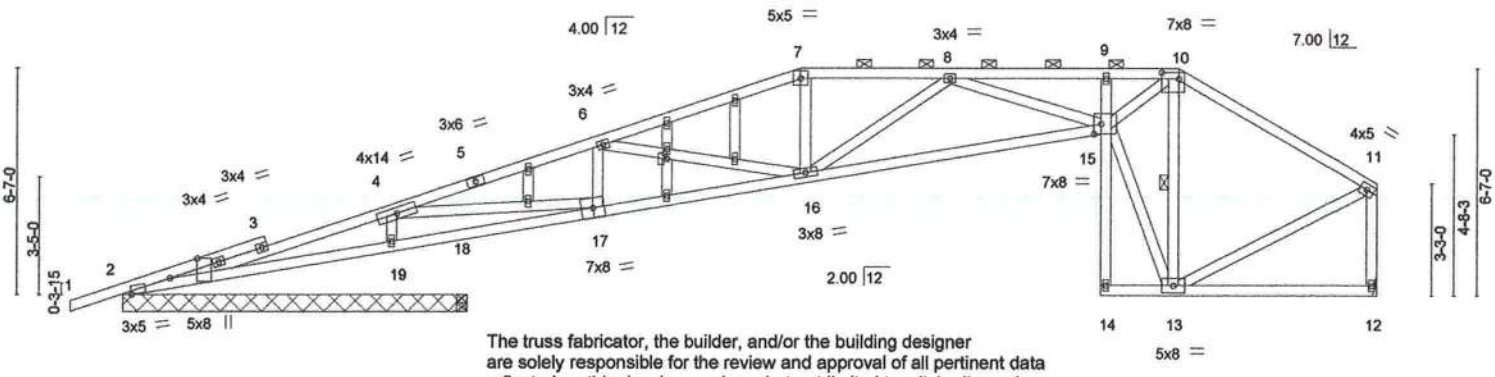
SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL, .

7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 15:52:51 2011 Page 1

ID:6wiZc2nEFqJ2sVglUGfMzgzwjnn-xMbFRw3fquDo71xAKbZlPW?dmt5s1Rq15lUJa2zwjnQ

-1-6-0	7-9-7	13-9-7	19-8-3	24-0-7	28-4-12	30-8-0	36-4-8
1-6-0	7-9-7	6-0-0	5-10-12	4-4-5	4-4-5	2-3-4	5-8-8

Scale = 1:65.0



The truss fabricator, the builder, and/or the building designer are solely responsible for the review and approval of all pertinent data reflected on this drawing, such as, but not limited to: pitch, dimensions, load, concentrated floor load check, etc., before fabrication to ensure accuracy and full compliance with existing applicable building code requirements.

7-9-7	9-8-2	13-9-7	19-8-3	28-4-12	30-8-0	36-4-8
7-9-7	1-10-11	4-1-4	5-10-12	8-8-9	2-3-4	5-8-8

Plate Offsets (X,Y): [2:1-1-15,Edge], [2:0-6-14,Edge], [10:0-6-0,0-2-4], [15:0-2-8,0-3-8], [22:0-2-0,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	0.24	15-16	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.74	Vert(TL)	-0.75	15-16	>424	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.92	Horz(TL)	0.50	12	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 207 lb	FT = 15%

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2 *Except* 9-14: 2 X 4 SYP No.3
WEBS	2 X 4 SYP No.3 *Except* 10-15,11-12: 2 X 4 SYP No.2
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-8-3 oc purlins, except end verticals, and 2-0-0 oc purlins (3-0-5 max.): 7-10.
BOT CHORD	Rigid ceiling directly applied or 5-5-0 oc bracing.
WEBS	1 Row at midpt 10-13

REACTIONS (lb/size) 12=1071/Mechanical, 19=1471/10-0-0 (min. 0-1-15), 2=224/10-0-0 (min. 0-1-15), 18=223/0-3-8 (min. 0-1-8)
Max Horz 2=259(LC 4)
Max Uplift 12=-284(LC 3), 19=-547(LC 3), 2=-103(LC 5), 18=-68(LC 3)
Max Grav 12=1071(LC 1), 19=1471(LC 1), 2=229(LC 9), 18=223(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/25, 2-3=-514/1041, 3-4=-501/1133, 4-5=-2031/668, 5-6=-1965/688, 6-7=-2474/835, 7-8=-2314/829, 8-9=-3689/1176, 9-10=-3571/1157, 10-11=-987/314, 11-12=-1015/297
BOT CHORD	2-19=-1029/331, 18-19=-1006/320, 17-18=-1037/335, 16-17=-719/1939, 15-16=-1123/3085, 14-15=-11/12, 9-15=-139/151, 13-14=-10/23, 12-13=-21/39
WEBS	4-19=-1446/622, 4-17=-1014/2867, 6-17=-552/256, 6-16=-122/528, 7-16=-95/481, 8-16=-931/383, 8-15=-92/729, 13-15=-486/1822, 10-15=-1207/3546, 10-13=-1984/599, 11-13=-244/836

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Provide adequate drainage to prevent water ponding.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) Refer to girder(s) for truss to truss connections.
10) Bearing at joint(s) 19, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 284 lb uplift at joint 12, 547 lb uplift at joint 19, 103 lb uplift at joint 2 and 68 lb uplift at joint 18.
12) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 18.
13) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

Continued on page 2

MARVIN A. STRZYZEWSKI LICENSE No 43144 STATE OF FLORIDA PROFESSIONAL ENGINEER

FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927672
RSNCONDE	A6ET	GABLE	1	1	Job Reference (optional)

NOTES

14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

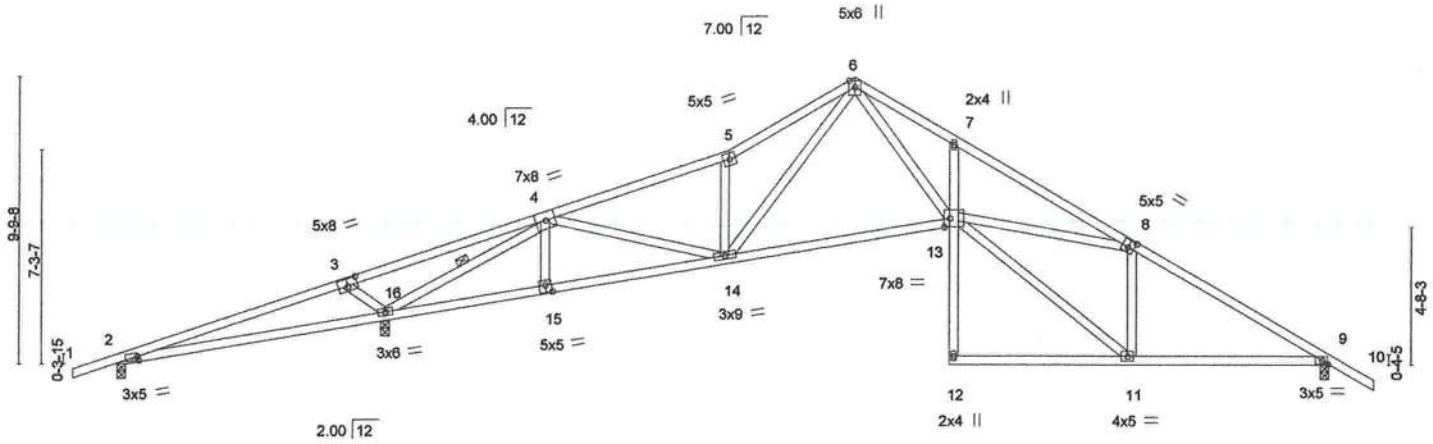
Job	Truss	Truss Type	Qty	Ply	E5927673
RSNCONDE	A	SPECIAL	3	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:09 2011 Page 1

ID:?23sPd9rjw7MpzRRc5YQANzwlbO-?23sPd9rjw7MpzRRc5YQANcB20zx2j8aPaba4EzwlbO

1-6-0	7-10-6	14-7-4	20-10-9	25-2-0	28-4-12	34-7-9	41-4-0	42-10-0
1-6-0	7-10-6	6-8-14	6-3-5	4-3-7	3-2-12	6-2-13	6-8-7	1-6-0

Scale = 1:76.4



9-1-12	14-7-4	20-10-9	28-4-12	34-7-9	41-4-0
9-1-12	5-5-8	6-3-5	7-6-3	6-2-13	6-8-7

Plate Offsets (X,Y): [2:0-0-4,0-1-8], [3:0-4-0,0-3-0], [8:0-2-8,0-3-0], [9:0-2-4,0-1-8], [13:0-2-4,0-3-8], [15:0-2-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.89	Vert(LL)	0.27	2-16	>406	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.73	13-14	>525	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.81	Horz(TL)	0.39	9	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 219 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
7-12: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 2-16.
WEBS 1 Row at midpt 4-16

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=195/0-3-8 (min. 0-1-8), 9=1306/0-3-8 (min. 0-1-9), 16=1979/0-3-8 (min. 0-2-5)
Max Horz 2=312(LC 4)
Max Uplift 2=-266(LC 3), 9=-231(LC 6), 16=-452(LC 3)
Max Grav 2=245(LC 7), 9=1306(LC 1), 16=1979(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-98/1044, 3-4=-181/1451, 4-5=-2470/249, 5-6=-2748/368, 6-7=-3677/299, 7-8=-3727/256, 8-9=-1992/260, 9-10=0/45
BOT CHORD 2-16=-925/107, 15-16=-123/1582, 14-15=-128/1596, 13-14=-11/1710, 12-13=0/100, 7-13=-265/142, 11-12=-0/15, 9-11=-99/1626
WEBS 3-16=-555/273, 4-16=-3403/361, 4-15=0/147, 4-14=-21/742, 5-14=-891/206, 6-14=-228/1082, 6-13=-160/2528, 11-13=-128/2058, 8-13=0/1493, 8-11=-1099/117

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 266 lb uplift at joint 2, 231 lb uplift at joint 9 and 452 lb uplift at joint 16.
7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 16.
8) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

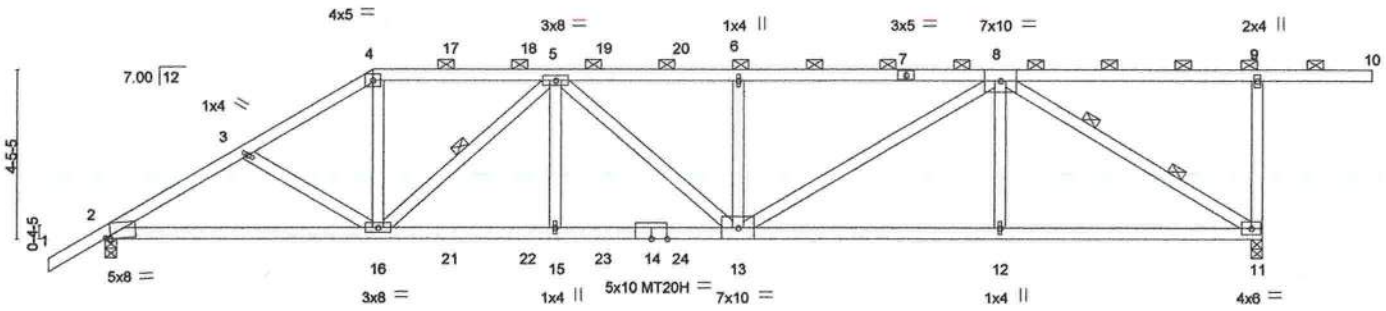
Job	Truss	Truss Type	Qty	Ply	E5927674
RSNCONDE	B1	MONO HIP	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:22 2011 Page 1

ID:7YLn73J_fwmWszxxtKHTC6zwlbB-7YLn73J_fwmWszxxtKHTC6eO2FLObaAVO6Fm2_zwlbB

-1-6-0	3-8-13	7-0-0	11-9-10	16-7-4	23-5-10	30-4-0	33-2-8
1-6-0	3-8-13	3-3-3	4-9-10	4-9-10	6-10-6	6-10-6	2-10-8

Scale = 1:58.8



7-0-0	11-9-10	16-7-4	23-5-10	30-4-0
7-0-0	4-9-10	4-9-10	6-10-6	6-10-6

Plate Offsets (X,Y): [2:0-1-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	-0.25	13-15	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	1.00	Vert(TL)	-0.64	13-15	>560	180	MT20H	187/143
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.95	Horz(TL)	0.20	11	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 173 lb	FT = 15%

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except* 4-7: 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-8-13 oc purlins, except end verticals, and 2-0-0 oc purlins (2-2-3 max.): 4-10.
BOT CHORD Rigid ceiling directly applied or 6-0-12 oc bracing.
WEBS 1 Row at midpt 5-16
2 Rows at 1/3 pts 8-11
JOINTS 1 Brace at Jt(s): 9

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 11=2425/0-3-8 (min. 0-2-7), 2=2683/0-3-8 (min. 0-2-11)
Max Horz 2=175(LC 5)
Max Uplift 11=-582(LC 4), 2=-590(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-4612/977, 3-4=-4480/994, 4-17=-3900/885, 17-18=-3900/885, 5-18=-3899/885, 5-19=-5559/1209, 19-20=-5559/1209, 6-20=-5559/1209, 6-7=-5559/1209, 7-8=-5559/1209, 8-9=-17/9, 9-10=0/0, 9-11=-393/196
BOT CHORD 2-16=-887/3865, 16-21=-1167/5193, 21-22=-1167/5193, 15-22=-1167/5193, 15-23=-1167/5193, 14-23=-1167/5193, 14-24=-1167/5193, 13-24=-1167/5193, 12-13=-717/3200, 11-12=-717/3200
WEBS 3-16=-112/51, 4-16=-253/1681, 5-16=-1732/471, 5-15=0/413, 5-13=-85/484, 6-13=-486/221, 8-13=-630/2756, 8-12=0/293, 8-11=-3751/832

NOTES

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 582 lb uplift at joint 11 and 590 lb uplift at joint 2.
7) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

FL Cert. #7239

January 10,2011

Continued on page 2

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927674
RSNCONDE	B1	MONO HIP	1	1	Job Reference (optional)

NOTES

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 230 lb down and 156 lb up at 7-0-0, 129 lb down and 88 lb up at 9-0-12, 129 lb down and 88 lb up at 11-0-12, 129 lb down and 88 lb up at 13-0-12, and 129 lb down and 88 lb up at 15-0-12, and 27 lb down and 34 lb up at 16-7-4 on top chord, and 390 lb down and 42 lb up at 7-0-0, 96 lb down at 9-0-12, 96 lb down at 11-0-12, 96 lb down at 13-0-12, and 96 lb down at 15-0-12, and 1104 lb down and 181 lb up at 16-7-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
      Vert: 1-4=-60, 4-9=-60, 9-10=-60, 2-11=-20
   Concentrated Loads (lb)
      Vert: 4=-230(F) 16=-357(F) 6=-27(F) 13=-1104(F) 17=-129(F) 18=-129(F) 19=-129(F) 20=-129(F) 21=-48(F) 22=-48(F) 23=-48(F) 24=-48(F)

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

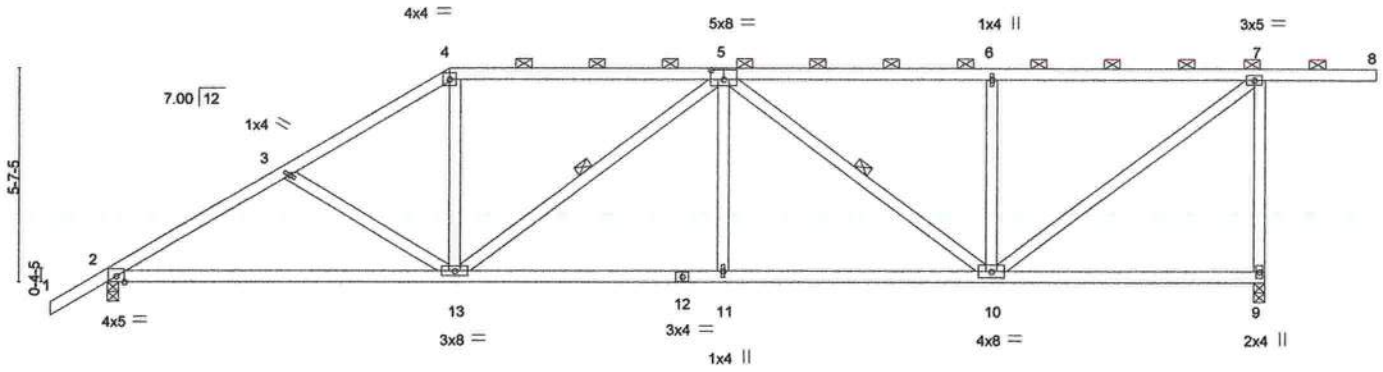
Job	Truss	Truss Type	Qty	Ply	E5927675
RSNCONDE	B2	MONO HIP	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:23 2011 Page 1

ID:bkv9LPJdQDuNU6W7R1oikKzwlbA-bkv9LPJdQDuNU6W7R1oikKBdufmQK8Zedm_JaQzwlbA

-1-6-0	4-9-4	9-0-0	16-1-15	23-2-1	30-4-0	33-2-8
1-6-0	4-9-4	4-2-12	7-1-15	7-0-3	7-1-15	2-10-8

Scale = 1:58.8



9-0-0	16-1-15	23-2-1	30-4-0
9-0-0	7-1-15	7-0-3	7-1-15

Plate Offsets (X,Y): [5:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	-0.14	2-13	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.39	2-13	>932	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.49	Horz(TL)	0.07	9	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 174 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-3 oc purlins, except end verticals, and 2-0-0 oc purlins (4-8-15 max.): 4-8.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-13, 5-10
JOINTS 1 Brace at Jt(s): 7

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 9=1389/0-3-8 (min. 0-1-10), 2=1294/0-3-8 (min. 0-1-8)
Max Horz 2=213(LC 5)
Max Uplift 9=-332(LC 4), 2=-174(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-1955/249, 3-4=-1717/244, 4-5=-1439/230, 5-6=-1230/243, 6-7=-1230/243, 7-8=0/0, 7-9=-1328/350
BOT CHORD 2-13=-286/1629, 12-13=-298/1720, 11-12=-298/1720, 10-11=-298/1720, 9-10=-6/13
WEBS 3-13=-238/127, 4-13=-29/543, 5-13=-353/196, 5-11=0/253, 5-10=-615/122, 6-10=-403/187, 7-10=-297/1536

NOTES
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
2) Provide adequate drainage to prevent water ponding.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 332 lb uplift at joint 9 and 174 lb uplift at joint 2.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927676
RSNCONDE	B3	MONO HIP	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:23 2011 Page 1

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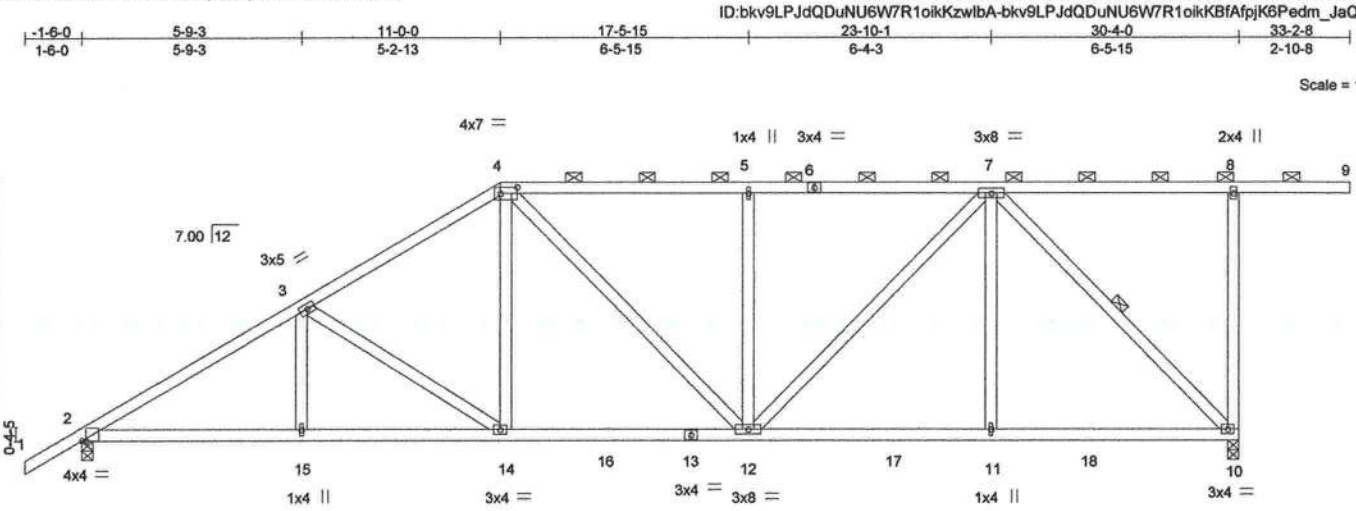

Plate Offsets (X,Y): [2:0-1-5,0-0-2], [4:0-5-4,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.10	12-14	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.22	12-14	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.63	Horz(TL)	0.08	10	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 189 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-10 oc purlins, except end verticals, and 2-0-0 oc purlins (4-8-0 max.): 4-9.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 7-10
JOINTS 1 Brace at Jt(s): 8

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 10=1594/0-3-8 (min. 0-1-14), 2=1398/0-3-8 (min. 0-1-10)
Max Horz 2=250(LC 5)
Max Uplift 10=-331(LC 4), 2=-182(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-2191/203, 3-4=-1779/219, 4-5=-1579/227, 5-6=-1578/227, 6-7=-1578/227, 7-8=-9/3, 8-9=0/0, 8-10=-386/196
BOT CHORD 2-15=-284/1804, 14-15=-284/1804, 14-16=-208/1471, 13-16=-208/1471, 12-13=-208/1471, 12-17=-161/1121, 11-17=-161/1121, 11-18=-161/1121, 10-18=-161/1121
WEBS 3-15=0/227, 3-14=-405/141, 4-14=-41/428, 4-12=-152/151, 5-12=-413/179, 7-12=-121/654, 7-11=0/318, 7-10=-1599/226

NOTES
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
2) Provide adequate drainage to prevent water ponding.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 331 lb uplift at joint 10 and 182 lb uplift at joint 2.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927677
RSNCONDE	B4	HIP	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:24 2011 Page 1

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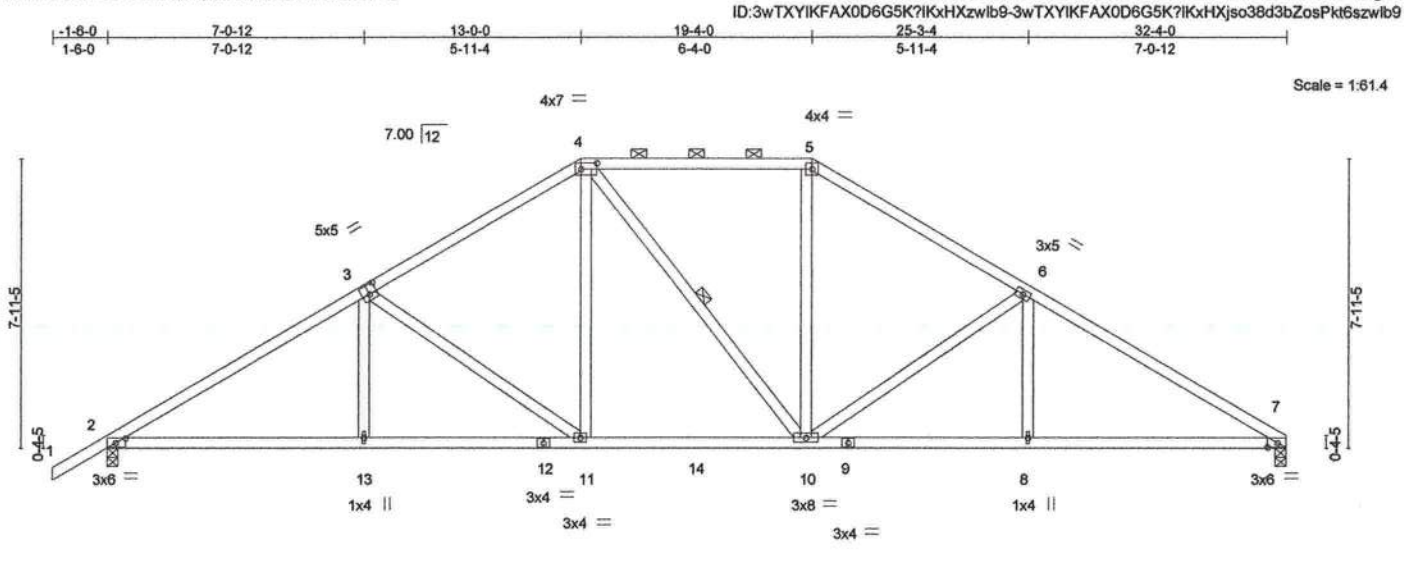

Plate Offsets (X,Y): [2:0-3-3,0-1-8], [3:0-2-8,0-3-0], [4:0-5-4,0-2-0], [7:0-3-3,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.12	10-11	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.27	10-11	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.10	7	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 174 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-13 oc purlins, except 2-0-0 oc purlins (4-9-14 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-10

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 7=1333/0-3-8 (min. 0-1-9), 2=1448/0-3-8 (min. 0-1-11)
Max Horz 2=228(LC 4)
Max Uplift 7=-147(LC 6), 2=-213(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-2242/225, 3-4=-1740/208, 4-5=-1405/217, 5-6=-1719/214, 6-7=-2239/244
BOT CHORD 2-13=-247/1835, 12-13=-248/1833, 11-12=-248/1833, 11-14=-147/1425, 10-14=-147/1425, 9-10=-127/1836, 8-9=-127/1836, 7-8=-127/1836
WEBS 3-13=0/284, 3-11=-514/172, 4-11=-62/509, 4-10=-168/128, 5-10=-43/472, 6-10=-537/188, 6-8=0/287

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 147 lb uplift at joint 7 and 213 lb uplift at joint 2.
7) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927678
RSNCONDE	B5	HIP	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:25 2011 Page 1

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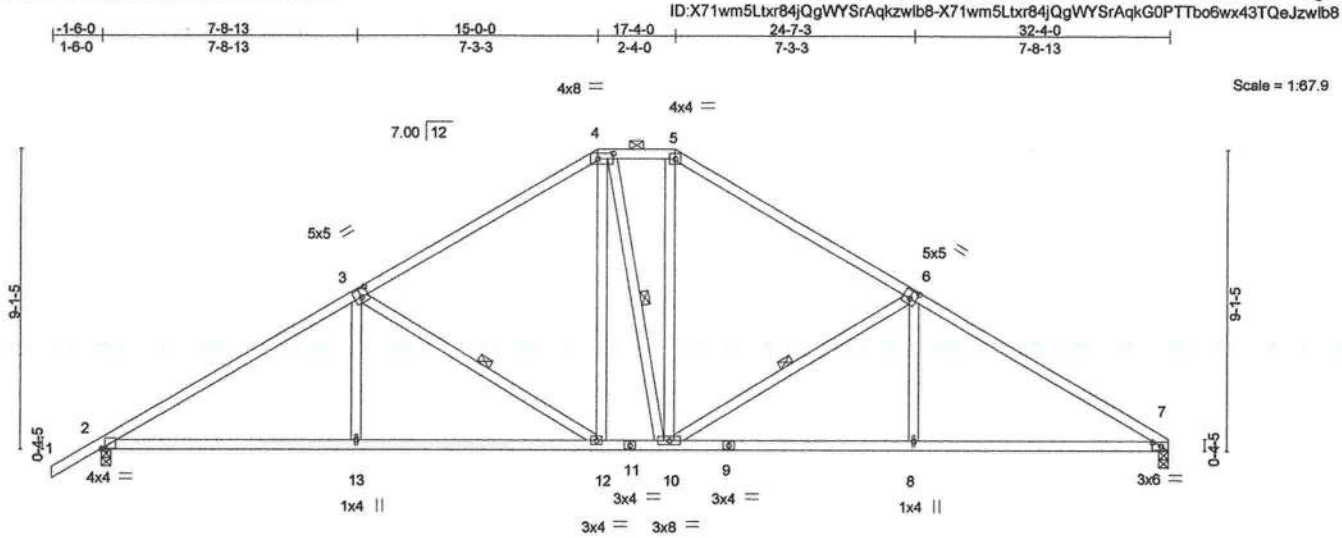

Plate Offsets (X,Y): [2:0-1-5,0-0-2], [3:0-2-8,0-3-0], [4:0-5-8,0-2-0], [6:0-2-8,0-3-0], [7:0-3-3,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.09	7-8	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.29	7-8	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.25	Horz(TL)	0.10	7	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 182 lb	FT = 15%

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-8-6 oc purlins, except 2-0-0 oc purlins (5-7-5 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 3-12, 4-10, 6-10

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1383/0-3-8 (min. 0-1-10), 7=1279/0-3-8 (min. 0-1-8)
Max Horz 2=260(LC 4)
Max Uplift 2=-223(LC 5), 7=-157(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-2105/244, 3-4=-1474/226, 4-5=-1173/231, 5-6=-1477/222, 6-7=-2118/260
BOT CHORD 2-13=-222/1715, 12-13=-223/1712, 11-12=-56/1170, 10-11=-56/1170, 9-10=-134/1725, 8-9=-134/1725, 7-8=-133/1728
WEBS 3-13=0/339, 3-12=-652/206, 4-12=-79/427, 4-10=-153/173, 5-10=-79/427, 6-10=-665/220, 6-8=0/340

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 223 lb uplift at joint 2 and 157 lb uplift at joint 7.
7) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927679
RSNCONDE	C1	COMMON	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:27 2011 Page 1

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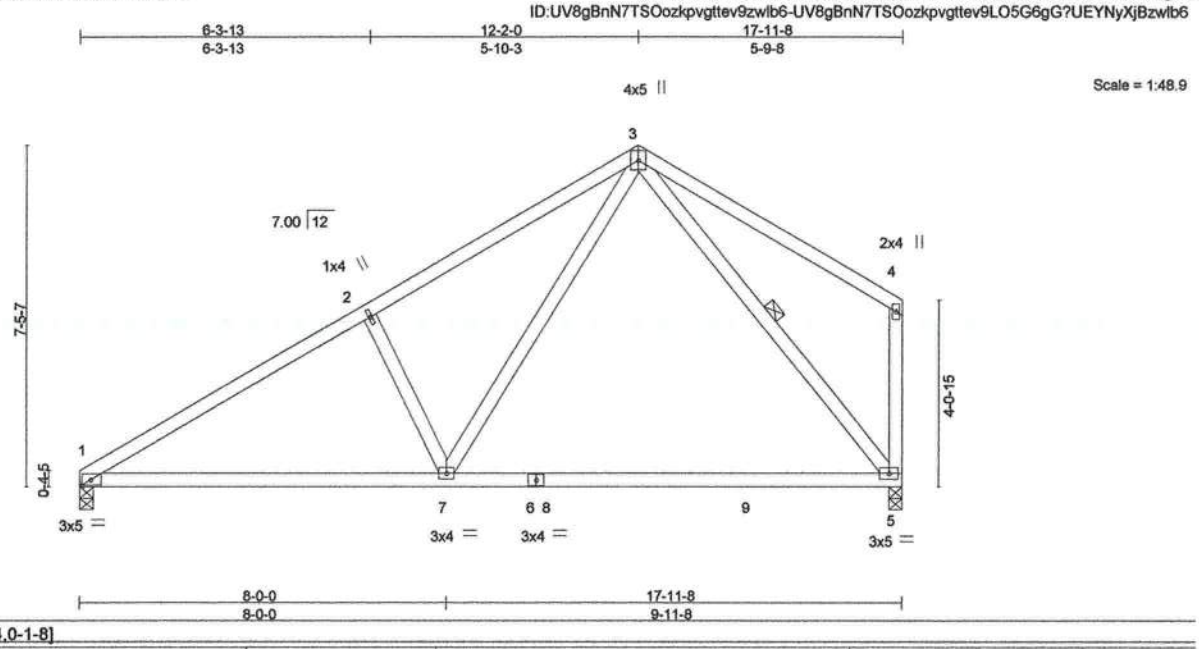

Plate Offsets (X,Y): [1:0-2-4,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.34	5-7	>633	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.64	5-7	>331	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.24	Horz(TL)	0.02	5	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 94 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
4-5: 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 3-5

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=759/0-3-8 (min. 0-1-8), 5=828/0-3-8 (min. 0-1-8)
Max Horz 1=193(LC 4)
Max Uplift 1=-88(LC 5), 5=-94(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1190/133, 2-3=-1031/186, 3-4=-162/83, 4-5=-211/89
BOT CHORD 1-7=-206/946, 6-7=-64/431, 6-8=-64/431, 8-9=-64/431, 5-9=-64/431
WEBS 2-7=-339/181, 3-7=-127/754, 3-5=-593/106

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 88 lb uplift at joint 1 and 94 lb uplift at joint 5.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

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January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927680
RSNCONDE	C	COMMON	11	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:26 2011 Page 1

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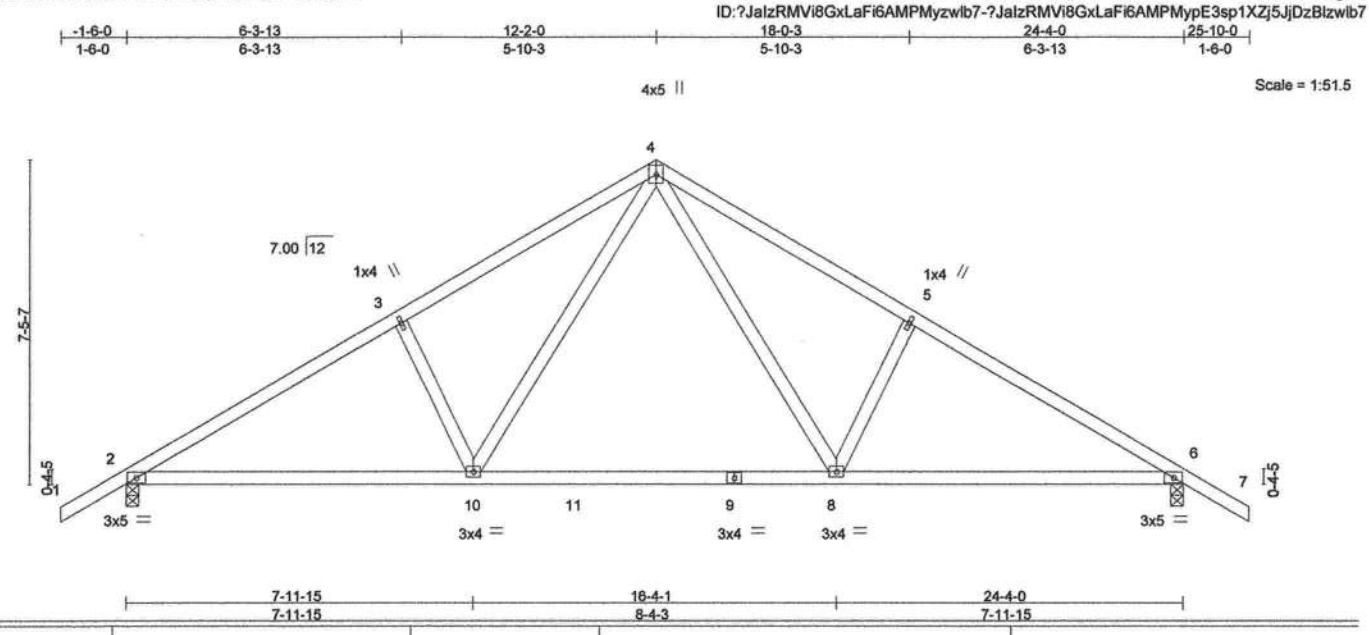


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.22	8-10	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.35	8-10	>820	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.21	Horz(TL)	0.05	6	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 119 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

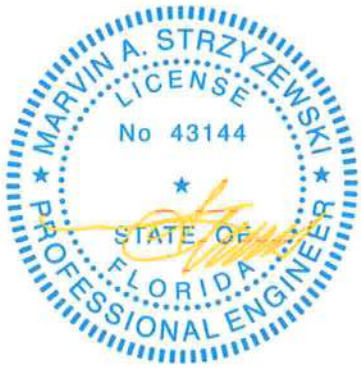
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1134/0-3-8 (min. 0-1-8), 6=1134/0-3-8 (min. 0-1-8)
Max Horz 2=-197(LC 3)
Max Uplift 2=-187(LC 5), 6=-187(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-1673/186, 3-4=-1514/238, 4-5=-1514/238, 5-6=-1673/186, 6-7=0/45
BOT CHORD 2-10=-157/1359, 10-11=-22/898, 9-11=-22/898, 8-9=-22/898, 6-8=-71/1358
WEBS 4-8=-122/654, 5-8=-330/172, 4-10=-122/655, 3-10=-330/172

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 187 lb uplift at joint 2 and 187 lb uplift at joint 6.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



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January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927681
RSNCONDE	CET	GABLE	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:28 2011 Page 1

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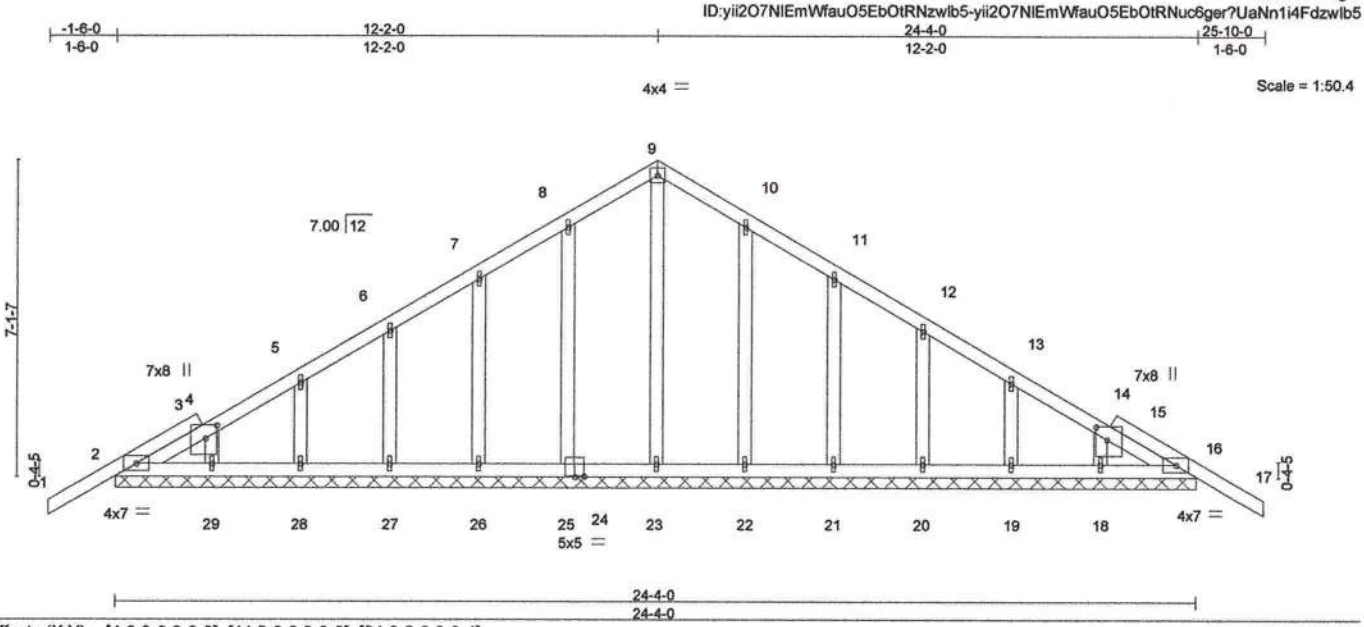


Plate Offsets (X,Y): [4:0-3-8,0-3-0], [14:0-3-8,0-3-0], [24:0-2-8,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	-0.01	17	n/r	120	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	-0.01	17	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.12	Horz(TL)	0.01	16	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 144 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

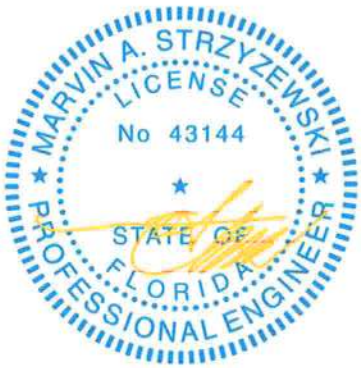
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=205/24-4-0 (min. 0-2-8), 16=205/24-4-0 (min. 0-2-8), 23=133/24-4-0 (min. 0-2-8), 25=166/24-4-0 (min. 0-2-8), 26=159/24-4-0 (min. 0-2-8), 27=160/24-4-0 (min. 0-2-8), 28=164/24-4-0 (min. 0-2-8), 29=144/24-4-0 (min. 0-2-8), 22=166/24-4-0 (min. 0-2-8), 21=159/24-4-0 (min. 0-2-8), 20=160/24-4-0 (min. 0-2-8), 19=164/24-4-0 (min. 0-2-8), 18=144/24-4-0 (min. 0-2-8)
Max Horz 2=-241(LC 3)
Max Uplift 2=-79(LC 5), 16=-102(LC 6), 25=-86(LC 5), 26=-90(LC 5), 27=-85(LC 5), 28=-89(LC 5), 29=-40(LC 5), 22=-84(LC 6), 21=-91(LC 6), 20=-85(LC 6), 19=-91(LC 6), 18=-41(LC 6)
Max Grav 2=205(LC 1), 16=205(LC 1), 23=164(LC 6), 25=168(LC 9), 26=159(LC 1), 27=160(LC 1), 28=164(LC 1), 29=144(LC 1), 22=168(LC 10), 21=159(LC 1), 20=160(LC 1), 19=164(LC 1), 18=144(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-199/153, 3-4=-167/140, 4-5=-163/146, 5-6=-128/145, 6-7=-87/140, 7-8=-46/179, 8-9=-50/218, 9-10=-50/211, 10-11=-46/150, 11-12=-47/86, 12-13=-47/48, 13-14=-67/50, 14-15=-87/61, 15-16=-104/50, 16-17=0/44
BOT CHORD 2-29=-32/163, 28-29=-32/163, 27-28=-32/163, 26-27=-32/163, 25-26=-32/163, 24-25=-32/163, 23-24=-32/163, 22-23=-32/163, 21-22=-32/163, 20-21=-32/163, 19-20=-32/163, 18-19=-32/163, 16-18=-32/163
WEBS 9-23=-152/0, 8-25=-128/99, 7-26=-119/102, 6-27=-120/98, 5-28=-123/98, 4-29=-107/64, 10-22=-128/96, 11-21=-119/103, 12-20=-120/98, 13-19=-123/100, 14-18=-107/64

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are 1x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.



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January 10,2011

Continued on page 2

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927681
RSNCONDE	CET	GABLE	1	1	Job Reference (optional)

NOTES

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 79 lb uplift at joint 2, 102 lb uplift at joint 16, 86 lb uplift at joint 25, 90 lb uplift at joint 26, 85 lb uplift at joint 27, 89 lb uplift at joint 28, 40 lb uplift at joint 29, 84 lb uplift at joint 22, 91 lb uplift at joint 21, 85 lb uplift at joint 20, 91 lb uplift at joint 19 and 41 lb uplift at joint 18.

10) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	E5927682
RSNCONDE	CJ03	JACK	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:29 2011 Page 1

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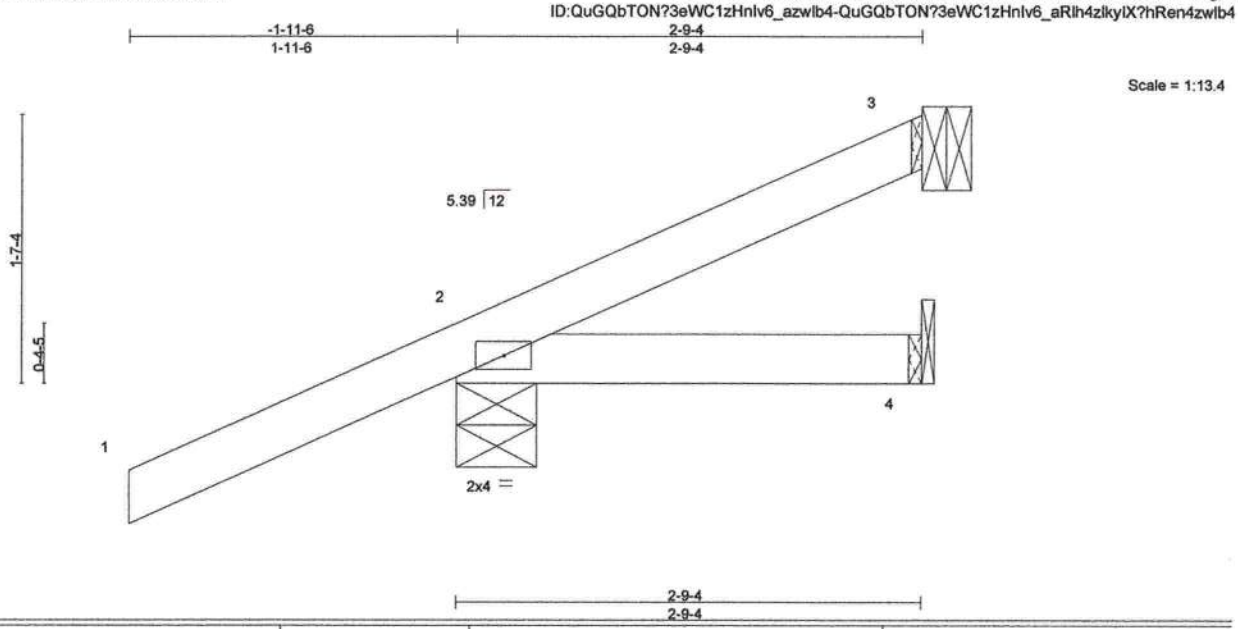


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 12 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

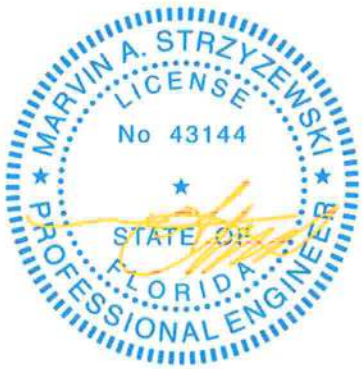
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=11/Mechanical, 2=288/0-5-12 (min. 0-1-8), 4=25/Mechanical
Max Horz 2=81(LC 5)
Max Uplift 3=-14(LC 4), 2=-137(LC 5)
Max Grav 3=17(LC 3), 2=288(LC 1), 4=50(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-60/6
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 14 lb uplift at joint 3 and 137 lb uplift at joint 2.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



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January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927683
RSNCONDE	CJ09	MONO TRUSS	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:30 2011 Page 1

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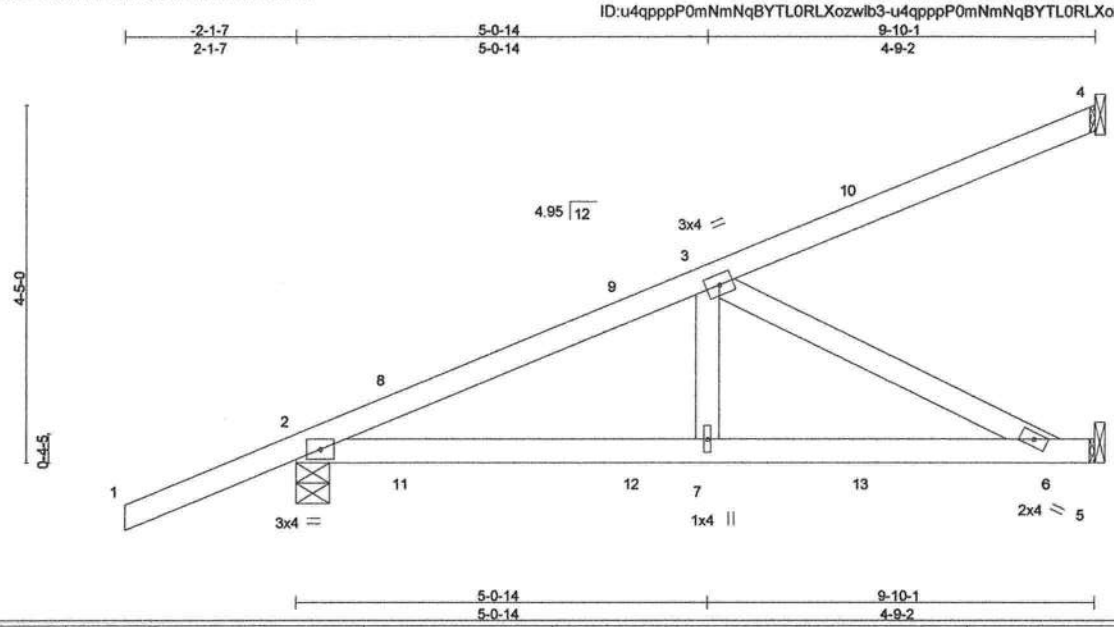

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.04	6-7	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.13	6-7	>912	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0.01	5	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 44 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=160/Mechanical, 2=517/0-4-15 (min. 0-1-8), 5=329/Mechanical
Max Horz 2=173(LC 5)
Max Uplift 4=-77(LC 5), 2=-133(LC 5), 5=-50(LC 4)
Max Grav 4=160(LC 1), 2=517(LC 1), 5=334(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-8=-684/69, 8-9=-615/88, 3-9=-611/81, 3-10=-96/7, 4-10=-42/50
BOT CHORD 2-11=-145/598, 11-12=-145/598, 7-12=-145/598, 7-13=-145/598, 6-13=-145/598, 5-6=0/0
WEBS 3-7=0/350, 3-6=-668/163

NOTES
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 77 lb uplift at joint 4, 133 lb uplift at joint 2 and 50 lb uplift at joint 5.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 19 lb down and 34 lb up at 1-4-9, 19 lb down and 34 lb up at 1-4-9, 14 lb up at 4-2-8, 14 lb up at 4-2-8, and 65 lb down and 53 lb up at 7-0-7, and 65 lb down and 53 lb up at 7-0-7 on top chord, and 20 lb up at 1-4-9, 20 lb up at 1-4-9, 17 lb down at 4-2-8, 17 lb down at 4-2-8, and 57 lb down at 7-0-7, and 57 lb down at 7-0-7 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-60, 2-5=-20

MARVIN A. STRZYZEWSKI LICENSE No 43144 STATE OF FLORIDA PROFESSIONAL ENGINEER

FL Cert. #7239

January 10,2011

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ENGINEERING BY TRENCO A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927683
RSNCONDE	CJ09	MONO TRUSS	1	1	Job Reference (optional)

LOAD CASE(S) Standard
Concentrated Loads (lb)
Vert: 8=68(F=34, B=34) 9=15(F=7, B=7) 10=-130(F=-65, B=-65) 11=20(F=10, B=10) 12=-17(F=-8, B=-8) 13=-57(F=-28, B=-28)

Job	Truss	Truss Type	Qty	Ply	E5927684
RSNCONDE	D1	COMMON	4	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:31 2011 Page 1

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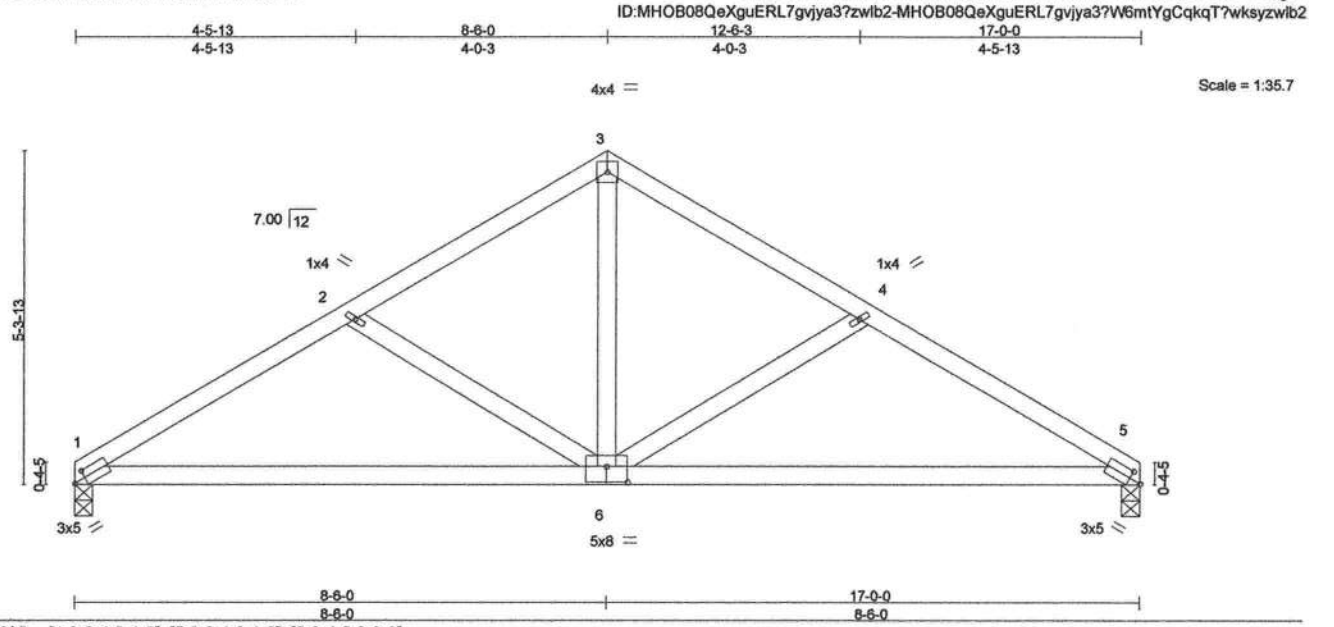


Plate Offsets (X,Y): [1:0-2-4,0-1-8], [5:0-2-4,0-1-8], [6:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.08	1-6	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.22	1-6	>919	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.16	Horz(TL)	0.02	5	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 76 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-11-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=668/0-3-8 (min. 0-1-8), 5=668/0-3-8 (min. 0-1-8)
Max Horz 1=139(LC 4)
Max Uplift 1=-84(LC 5), 5=-84(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1005/147, 2-3=-762/122, 3-4=-762/122, 4-5=-1005/147
BOT CHORD 1-6=-133/828, 5-6=-74/828
WEBS 3-6=-52/500, 4-6=-283/136, 2-6=-283/135

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 1 and 84 lb uplift at joint 5.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

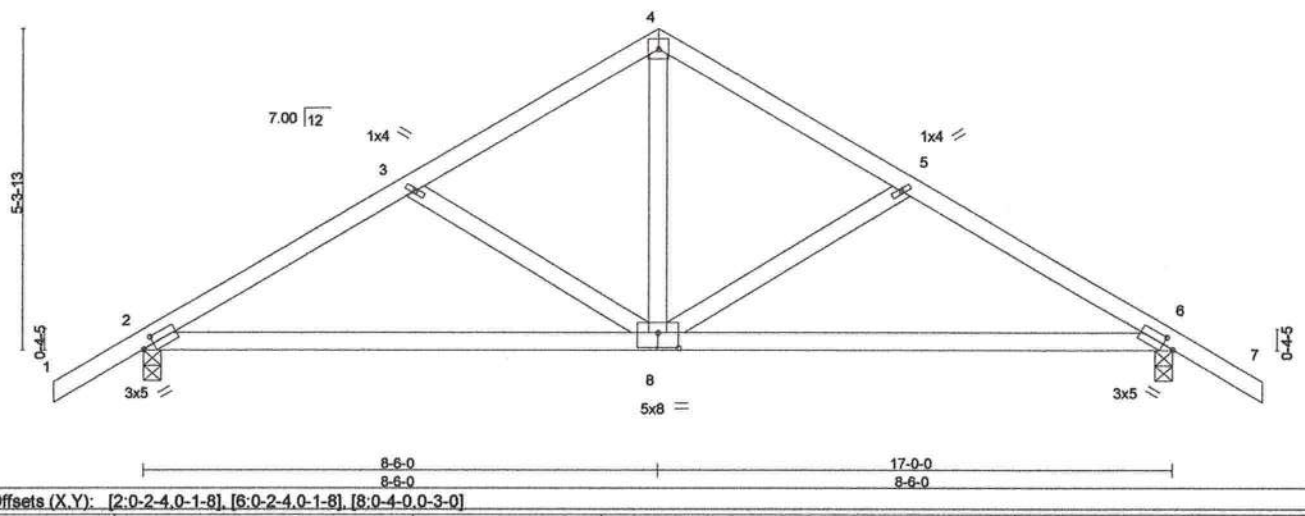
Job	Truss	Truss Type	Qty	Ply	E5927685
RSNCONDE	D	COMMON	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:31 2011 Page 1

ID:MHOB08QeXguERL7gvjya3?zwlb2-MHOB08QeXguERL7gvjya3?W6BtYqCqnqT?wksyzwlb2

-1-6-0	4-5-13	8-6-0	12-6-3	17-0-0	18-6-0
1-6-0	4-5-13	4-0-3	4-0-3	4-5-13	1-6-0

Scale = 1:37.1



8-6-0	17-0-0
8-6-0	8-6-0

Plate Offsets (X,Y): [2:0-2-4,0-1-8], [6:0-2-4,0-1-8], [8:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	-0.08	2-8	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.21	2-8	>954	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.16	Horz(TL)	0.02	6	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 81 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=767/0-3-8 (min. 0-1-8), 6=767/0-3-8 (min. 0-1-8)
Max Horz 2=138(LC 4)
Max Uplift 2=-151(LC 5), 6=-151(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-974/128, 3-4=-739/112, 4-5=-739/112, 5-6=-974/128, 6-7=0/45
BOT CHORD 2-8=-100/794, 6-8=-46/794
WEBS 4-8=-38/490, 5-8=-266/121, 3-8=-266/120

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 151 lb uplift at joint 2 and 151 lb uplift at joint 6.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

MARVIN A. STRZYZEWSKI LICENSE No 43144 STATE OF FLORIDA PROFESSIONAL ENGINEER

FL Cert. #7239

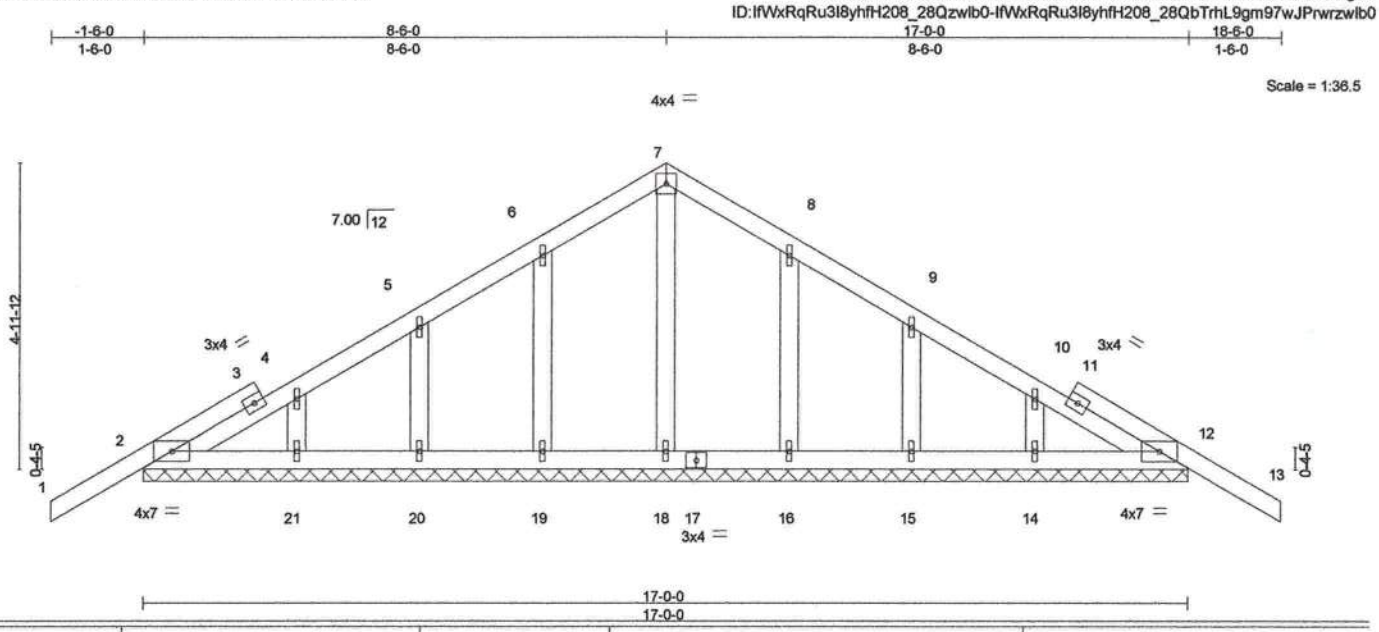
January 10,2011

ENGINEERING BY TRENCO A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927686
RSNCONDE	DET	GABLE	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:33 2011 Page 1

ID:IfWxRqRu3l8yhfH208_28QzwIb0-IfWxRqRu3l8yhfH208_28QbTrhL9gm97wJPrwrzwIb0



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	-0.01	13	n/r	120	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.01	13	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	12	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 90 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=213/17-0-0 (min. 0-1-13), 12=213/17-0-0 (min. 0-1-13), 18=134/17-0-0 (min. 0-1-13), 19=166/17-0-0 (min. 0-1-13), 20=160/17-0-0 (min. 0-1-13), 21=164/17-0-0 (min. 0-1-13), 16=166/17-0-0 (min. 0-1-13), 15=160/17-0-0 (min. 0-1-13), 14=164/17-0-0 (min. 0-1-13)
Max Horz 2=-166(LC 3)
Max Uplift 2=-110(LC 5), 12=-127(LC 6), 19=-88(LC 5), 20=-96(LC 5), 21=-48(LC 5), 16=-87(LC 6), 15=-97(LC 6), 14=-52(LC 6)
Max Grav 2=213(LC 1), 12=213(LC 1), 18=134(LC 1), 19=169(LC 9), 20=160(LC 1), 21=164(LC 1), 16=169(LC 10), 15=160(LC 1), 14=164(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-127/101, 3-4=-117/112, 4-5=-94/106, 5-6=-52/114, 6-7=-48/156, 7-8=-48/148, 8-9=-45/85, 9-10=-45/36, 10-11=-51/45, 11-12=-61/35, 12-13=0/44
BOT CHORD 2-21=-18/120, 20-21=-18/120, 19-20=-18/120, 18-19=-18/120, 17-18=-18/120, 16-17=-18/120, 15-16=-18/120, 14-15=-18/120, 12-14=-18/120
WEBS 7-18=-94/0, 6-19=-129/101, 5-20=-120/105, 4-21=-121/70, 8-16=-129/100, 9-15=-120/105, 10-14=-121/74

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are 1x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 110 lb uplift at joint 2, 127 lb uplift at joint 12, 88 lb uplift at joint 19, 96 lb uplift at joint 20, 48 lb uplift at joint 21, 87 lb uplift at joint 16, 97 lb uplift at joint 15 and 52 lb uplift at joint 14.
10) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

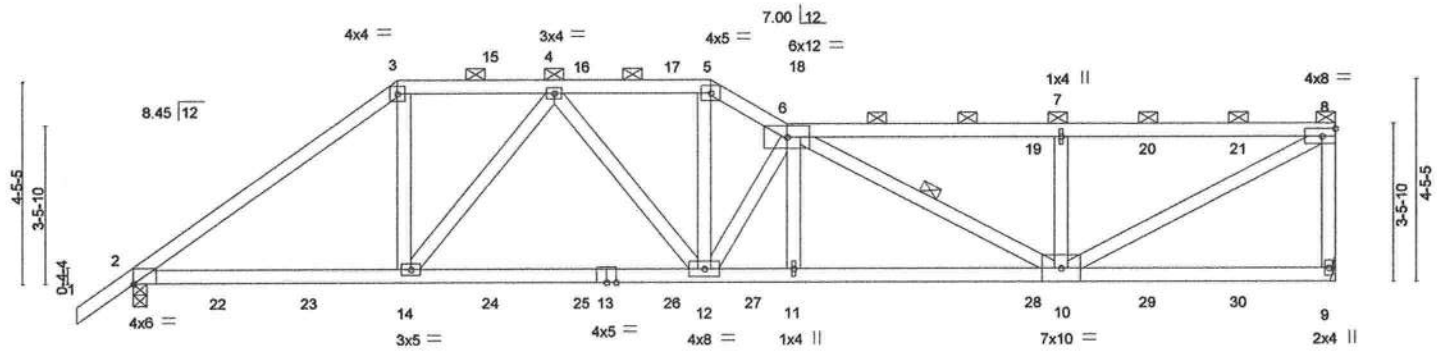
Job	Truss	Truss Type	Qty	Ply	E5927687
RSNCONDE	E1	SPECIAL	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:34 2011 Page 1

ID:ms3JfASWqbGpJpsFarVHhezwlb?-ms3JfASWqbGpJpsFarVHhe8SM5YAP3HG9z9PTHzwlb?

1-2-15	5-9-10	9-2-13	12-8-0	14-4-0	20-4-4	26-4-8
1-2-15	5-9-10	3-5-3	3-5-3	1-8-0	6-0-4	6-0-4

Scale = 1:49.2



5-9-10	12-8-0	14-4-0	20-4-4	26-4-8
5-9-10	6-10-6	1-8-0	6-0-4	6-0-4

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.91	Vert(LL)	-0.19	11-12	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.46	10-11	>677	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.69	Horz(TL)	0.12	9	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 146 lb	FT = 15%

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except* 6-8: 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP SS
WEBS 2 X 4 SYP No.3 *Except* 6-10,8-10: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-7-13 oc purlins, except end verticals, and 2-0-0 oc purlins (2-10-6 max.): 3-5, 6-8.
BOT CHORD Rigid ceiling directly applied or 6-1-15 oc bracing.
WEBS 1 Row at midpt 6-10
JOINTS 1 Brace at Jt(s): 8

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 9=2064/Mechanical, 2=2309/0-3-8 (min. 0-2-12)
Max Horz 2=156(LC 5)
Max Uplift 9=-524(LC 4), 2=-527(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/43, 2-3=-3207/755, 3-15=-2584/658, 4-15=-2583/658, 4-16=-3639/896, 16-17=-3640/896, 5-17=-3641/896, 5-18=-4113/1017, 6-18=-4136/1022, 6-19=-3315/836, 7-19=-3315/836, 7-20=-3315/836, 20-21=-3315/836, 8-21=-3315/836, 8-9=-1984/544
BOT CHORD 2-22=-651/2535, 22-23=-651/2535, 14-23=-651/2535, 14-24=-864/3273, 24-25=-864/3273, 13-25=-864/3273, 13-26=-864/3273, 12-26=-864/3273, 12-27=-1159/4648, 11-27=-1159/4648, 11-28=-1163/4663, 10-28=-1163/4663, 10-29=-14/52, 29-30=-14/52, 9-30=-14/52
WEBS 3-14=-261/1392, 4-14=-1165/373, 4-12=-77/619, 5-12=-404/1797, 6-12=-2155/555, 6-11=-81/345, 6-10=-1523/369, 7-10=-662/322, 8-10=-935/3711

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 524 lb uplift at joint 9 and 527 lb uplift at joint 2.
8) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

FL Cert. #7239

January 10,2011

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ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927687
RSNCONDE	E1	SPECIAL	1	1	Job Reference (optional)

NOTES

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 129 lb down and 88 lb up at 5-9-10, 129 lb down and 88 lb up at 7-10-6, 129 lb down and 88 lb up at 9-10-6, 129 lb down and 88 lb up at 11-10-6, 9 lb down and 34 lb up at 13-7-4, 131 lb down and 72 lb up at 19-8-12, 115 lb down and 59 lb up at 20-3-4, and 115 lb down and 59 lb up at 22-3-4, and 115 lb down and 59 lb up at 24-3-4 on top chord, and 168 lb down and 45 lb up at 1-10-6, 236 lb down and 55 lb up at 3-10-6, 96 lb down at 5-10-6, 96 lb down at 7-10-6, 96 lb down at 9-10-6, 96 lb down at 11-10-6, 498 lb down and 121 lb up at 13-7-4, 217 lb down and 21 lb up at 19-8-12, 63 lb down at 20-3-4, and 63 lb down at 22-3-4, and 63 lb down at 24-3-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
      Vert: 1-3=-60, 3-5=-60, 5-6=-60, 6-8=-60, 2-9=-20
   Concentrated Loads (lb)
      Vert: 3=-129(B) 14=-48(B) 7=-75(B) 10=-31(B) 15=-129(B) 16=-129(B) 17=-129(B) 18=34(B) 19=-91(B) 20=-75(B) 21=-75(B) 22=-168(B) 23=-236(B) 24=-48(B) 25=-48(B) 26=-48(B) 27=-498(B) 28=-217(B) 29=-31(B) 30=-31(B)

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

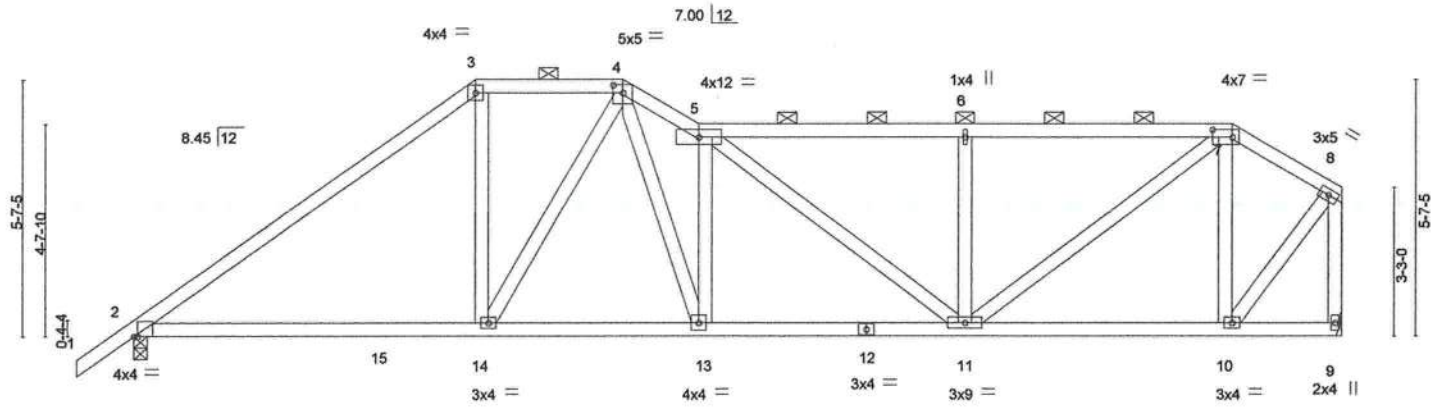
Job	Truss	Truss Type	Qty	Ply	E5927688
RSNCONDE	E2	SPECIAL	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:35 2011 Page 1

ID:F2disWT8bvOgwyRR8Z0WDrzwlb_-F2disWT8bvOgwyRR8Z0WDrhkmVt08bQPOduy?jzwlb_

1-2-15	7-5-8	10-8-0	12-4-0	18-2-0	24-0-0	26-4-8
1-2-15	7-5-8	3-2-8	1-8-0	5-10-0	5-10-0	2-4-8

Scale = 1:49.0



7-5-8	12-4-0	18-2-0	24-0-0	26-4-8
7-5-8	4-10-8	5-10-0	5-10-0	2-4-8

Plate Offsets (X,Y): [2:0-1-0,0-0-0], [4:0-2-8,0-2-1], [7:0-5-4,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.11	2-14	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.29	2-14	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.37	Horz(TL)	0.05	9	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 156 lb	FT = 15%

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
8-9: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-2-6 oc purlins, except end verticals, and 2-0-0 oc purlins (4-11-12 max.): 3-4, 5-7.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1196/0-3-8 (min. 0-1-8), 9=1063/Mechanical
Max Horz 2=166(LC 5)
Max Uplift 2=-150(LC 5), 9=-158(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/43, 2-3=-1562/213, 3-4=-1173/219, 4-5=-1969/357, 5-6=-1437/284, 6-7=-1437/284, 7-8=-656/111, 8-9=-1052/159
BOT CHORD 2-15=-222/1160, 14-15=-222/1160, 13-14=-234/1297, 12-13=-294/1637, 11-12=-294/1637, 10-11=-90/527, 9-10=-2/6
WEBS 3-14=-43/536, 4-14=-337/114, 4-13=-208/1162, 5-13=-1040/232, 5-11=-283/35, 6-11=-401/174, 7-11=-243/1134, 7-10=-598/150, 8-10=-156/875

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 150 lb uplift at joint 2 and 158 lb uplift at joint 9.
8) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927689
RSNCONDE	E3	CAL. DUAL PITCH	1	1	Job Reference (optional)

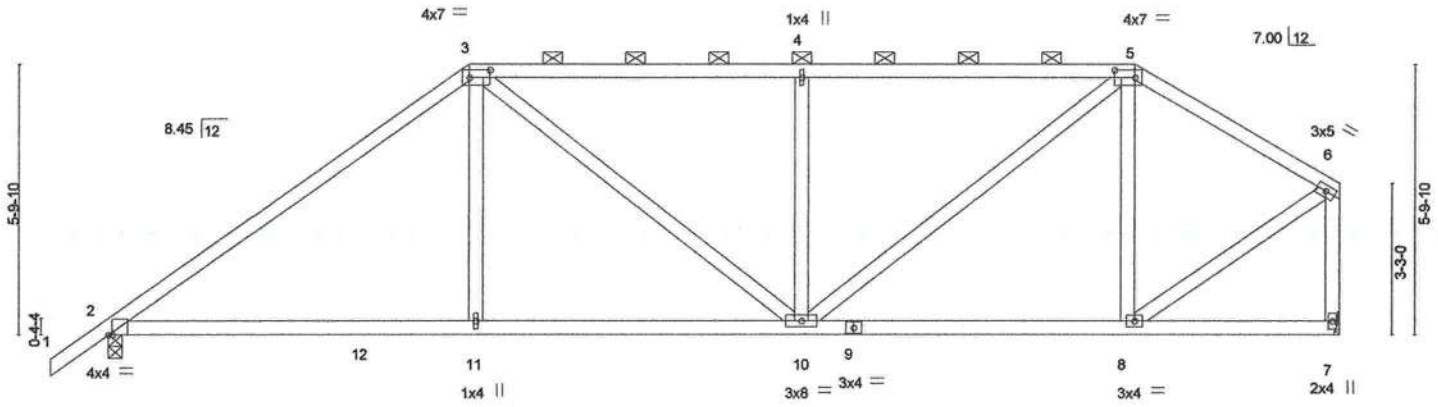
SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:36 2011 Page 1

ID:jEB43sTmMDWXY60diGXlm3zwlaz-jEB43sTmMDWXY60diGXlm3DupuDkt3qZcHeVWAzwlaz

-1-2-15 | 7-8-13 | 14-10-6 | 22-0-0 | 26-4-8

1-2-15 | 7-8-13 | 7-1-10 | 7-1-10 | 4-4-8

Scale: 1/4"=1'



7-8-13 | 14-10-6 | 22-0-0 | 26-4-8

7-8-13 | 7-1-10 | 7-1-10 | 4-4-8

Plate Offsets (X,Y): [2:0-1-0,0-0-0], [3:0-5-4,0-2-0], [5:0-5-4,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.13	2-11	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.31	2-11	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.30	Horz(TL)	0.04	7	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 145 lb	FT = 15%

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
6-7: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-0-12 oc purlins, except end verticals, and 2-0-0 oc purlins (5-0-0 max.): 3-5.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1204/0-3-8 (min. 0-1-8), 7=1066/Mechanical
Max Horz 2=169(LC 5)
Max Uplift 2=-165(LC 4), 7=-159(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/43, 2-3=-1563/253, 3-4=-1325/287, 4-5=-1325/287, 5-6=-906/162, 6-7=-1036/167
BOT CHORD 2-12=-256/1158, 11-12=-256/1158, 10-11=-256/1166, 9-10=-117/724, 8-9=-117/724, 7-8=-7/15
WEBS 3-11=0/329, 3-10=-188/301, 4-10=-485/220, 5-10=-218/785, 5-8=-365/122, 6-8=-154/873

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 165 lb uplift at joint 2 and 159 lb uplift at joint 7.
8) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927690
RSNCONDE	EJ2	JACK	3	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL

7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:37 2011 Page 1

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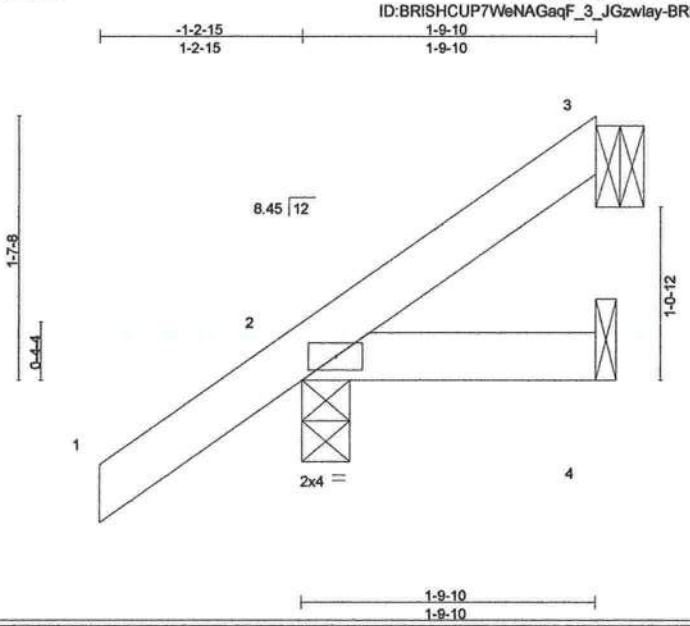


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.09	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.00	2-4	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 9 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-9-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

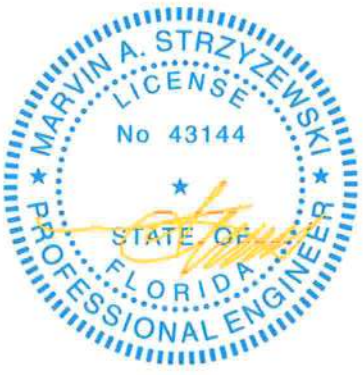
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=174/0-3-8 (min. 0-1-8), 4=18/Mechanical, 3=25/Mechanical
Max Horz 2=84(LC 5)
Max Uplift 2=-74(LC 5), 3=-15(LC 4)
Max Grav 2=174(LC 1), 4=35(LC 2), 3=25(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/42, 2-3=-47/11
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 74 lb uplift at joint 2 and 15 lb uplift at joint 3.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927691
RSNCONDE	EJ5	JACK	3	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL

7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:37 2011 Page 1

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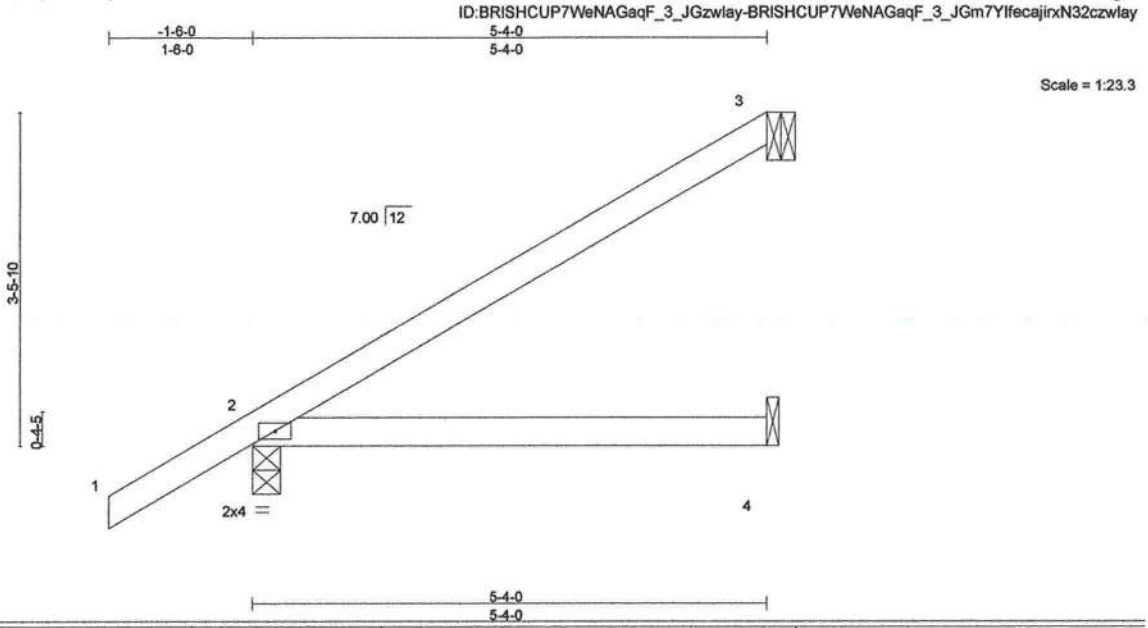

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.04	2-4	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	-0.09	2-4	>679	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 20 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=135/Mechanical, 2=317/0-3-8 (min. 0-1-8), 4=51/Mechanical
Max Horz 2=141(LC 5)
Max Uplift 3=-67(LC 5), 2=-83(LC 5)
Max Grav 3=135(LC 1), 2=317(LC 1), 4=102(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-87/55
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 67 lb uplift at joint 3 and 83 lb uplift at joint 2.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927692
RSNCONDE	EJ5GT	JACK	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:38 2011 Page 1

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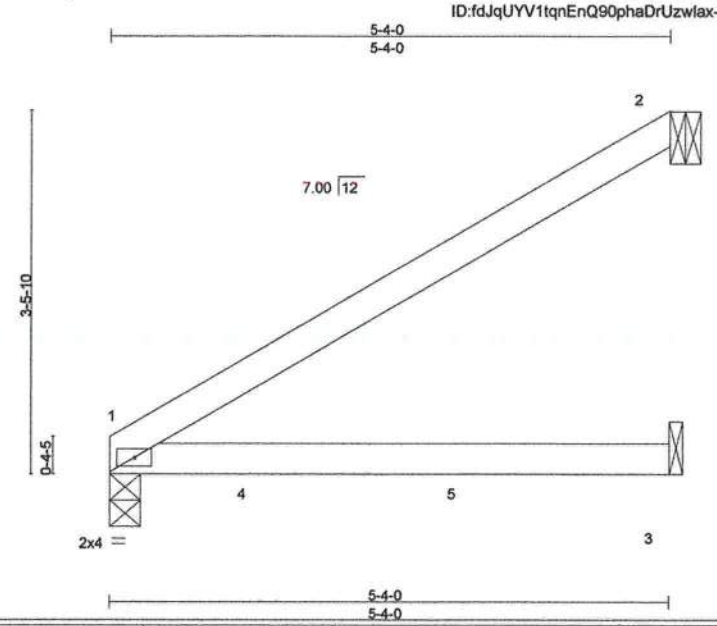

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.10	1-3	>596	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.29	1-3	>215	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 17 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-10-2 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=440/0-3-8 (min. 0-1-8), 2=151/Mechanical, 3=237/Mechanical
Max Horz 1=99(LC 5)
Max Uplift 1=-44(LC 5), 2=-81(LC 5), 3=-15(LC 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-70/63
BOT CHORD 1-4=0/0, 4-5=0/0, 3-5=0/0

NOTES
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 1, 81 lb uplift at joint 2 and 15 lb uplift at joint 3.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 212 lb down and 35 lb up at 1-4-12, and 212 lb down and 35 lb up at 3-4-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-60, 1-3=-20
Concentrated Loads (lb)
Vert: 4=-212(F) 5=-212(F)

FL Cert. #7239

January 10,2011

ENGINEERING BY
TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927693
RSNCONDE	EJ7	JACK	9	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:38 2011 Page 1

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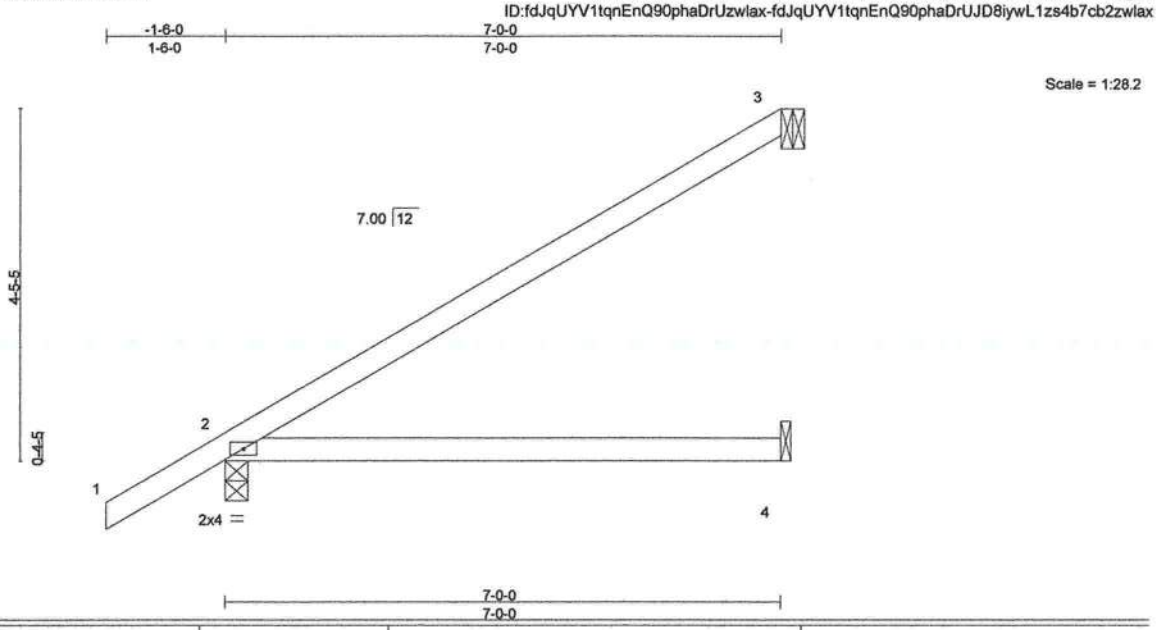


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.11	2-4	>730	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.28	2-4	>292	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 25 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

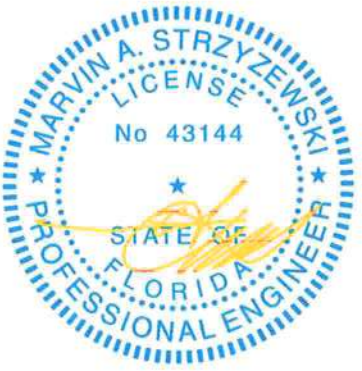
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=189/Mechanical, 2=380/0-3-8 (min. 0-1-8), 4=68/Mechanical
Max Horz 2=172(LC 5)
Max Uplift 3=-96(LC 5), 2=-82(LC 5)
Max Grav 3=189(LC 1), 2=380(LC 1), 4=136(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-110/77
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 96 lb uplift at joint 3 and 82 lb uplift at joint 2.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	
RSNCONDE	EJ7A	MONO HIP	1	1	E5927694

Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:39 2011 Page 1

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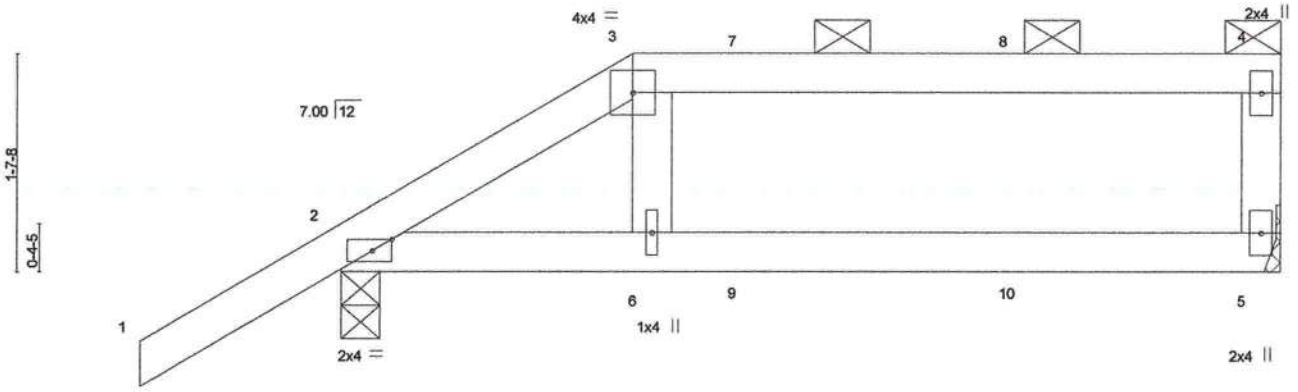
-1-6-0 / 1-6-0 | 2-2-1 / 2-2-1 | 7-0-0 / 4-9-15

Scale = 1:16.7



2-2-1 / 2-2-1 | 7-0-0 / 4-9-15

Plate Offsets (X,Y): [2:0-1-12,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.03	5-6	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.07	5-6	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.03	Horz(TL)	0.00	5	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 27 lb	FT = 15%

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 3-4.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 4

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=188/Mechanical, 2=291/0-3-8 (min. 0-1-8)
Max Horz 2=85(LC 5)
Max Uplift 5=-37(LC 4), 2=-111(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/45, 2-3=-155/17, 3-7=-118/18, 7-8=-118/18, 4-8=-118/19, 4-5=-125/49
BOT CHORD 2-6=-23/109, 6-9=-19/118, 9-10=-19/118, 5-10=-19/118
WEBS 3-6=0/87

NOTES

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
2) Provide adequate drainage to prevent water ponding.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 5 and 111 lb uplift at joint 2.
7) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 23 lb down and 84 lb up at 2-2-1, and 11 lb down and 35 lb up at 3-0-12, and 11 lb down and 35 lb up at 5-0-12 on top chord, and 5 lb down at 2-2-1, and 5 lb up at 3-0-12, and 5 lb up at 5-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

MARVIN A. STRZYZEWSKI LICENSE No 43144 STATE OF FLORIDA PROFESSIONAL ENGINEER

FL Cert. #7239

January 10,2011

Continued on page 2

ENGINEERING BY TRENCO A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927694
RSNCONDE	EJ7A	MONO HIP	1	1	Job Reference (optional)

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-60, 3-4=-60, 2-5=-20
Concentrated Loads (lb)
Vert: 3=84(F) 6=-3(F) 7=35(F) 8=35(F) 9=2(F) 10=2(F)

Job	Truss	Truss Type	Qty	Ply	E5927695
RSNCONDE	EJ7B	MONO HIP	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:39 2011 Page 1

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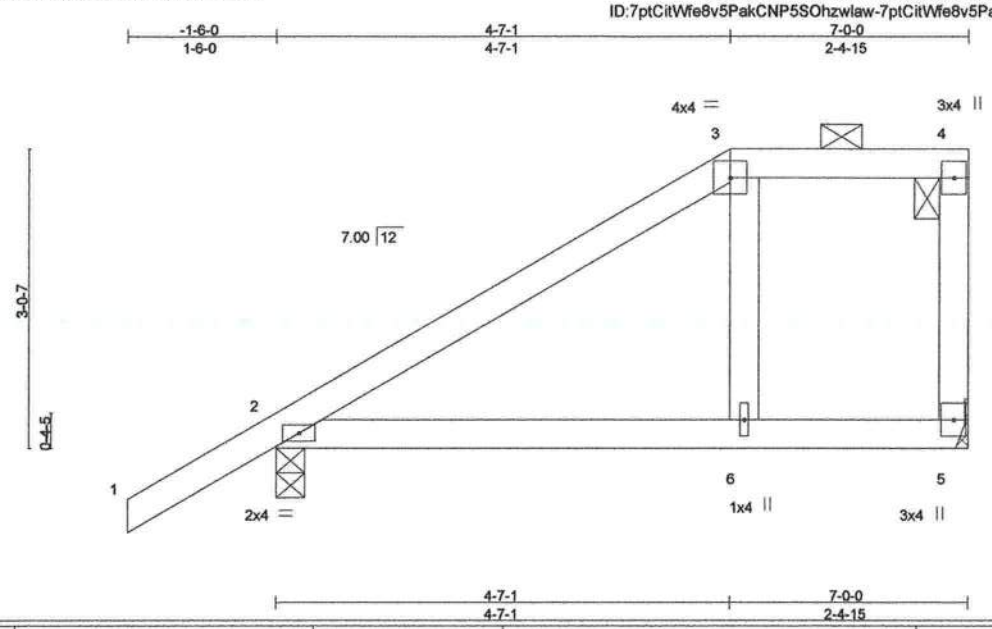


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.04	2-6	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.25	Vert(TL)	-0.13	2-6	>618	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.01	Horz(TL)	0.00	5	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 32 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 3-4.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 4

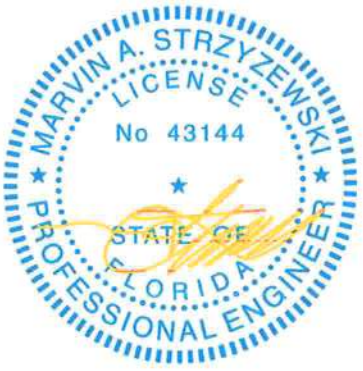
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=256/Mechanical, 2=379/0-3-8 (min. 0-1-8)
Max Horz 2=130(LC 5)
Max Uplift 5=-44(LC 4), 2=-99(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-186/0, 3-4=-87/13, 4-5=-126/29
BOT CHORD 2-6=-16/90, 5-6=-14/87
WEBS 3-6=-68/52

NOTES
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
2) Provide adequate drainage to prevent water ponding.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 5 and 99 lb uplift at joint 2.
7) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



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January 10,2011

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818 Soundside Road
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Job	Truss	Truss Type	Qty	Ply	E5927696
RSNCONDE	EJ7C	MONO HIP	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:40 2011 Page 1

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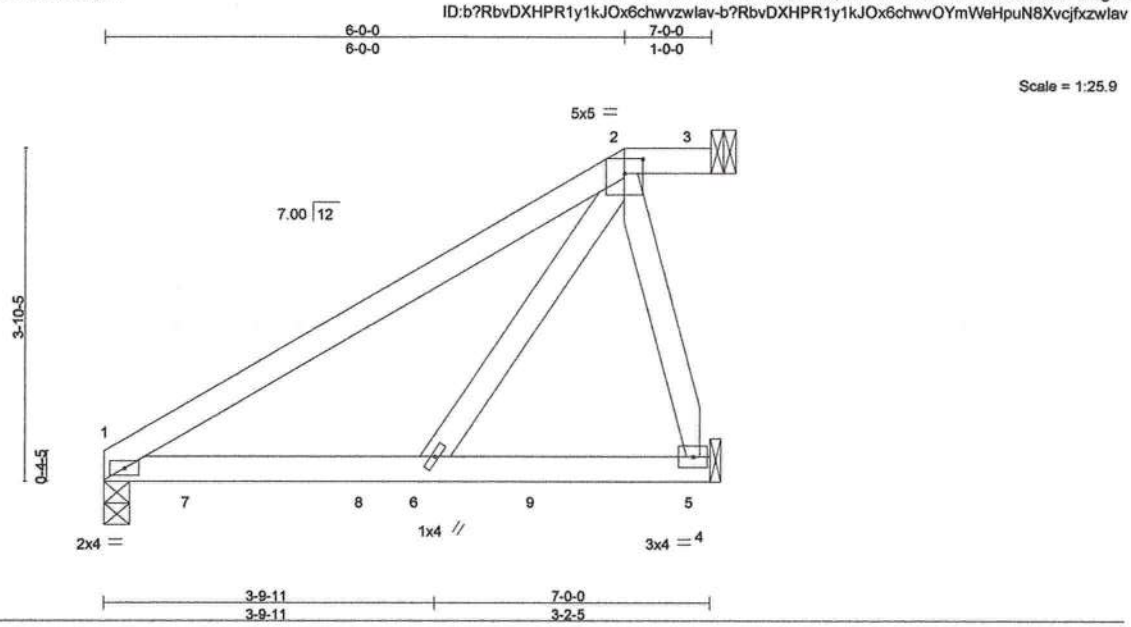

Plate Offsets (X,Y): [2:0-2-8,0-2-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	-0.02	1-6	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.05	1-6	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.20	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 33 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins: 2-3.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=629/0-3-8 (min. 0-1-8), 3=26/Mechanical, 5=518/Mechanical
Max Horz 1=114(LC 5)
Max Uplift 1=-76(LC 5), 3=-11(LC 3), 5=-110(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-595/53, 2-3=-0/0
BOT CHORD 1-7=-58/446, 7-8=-58/446, 6-8=-58/446, 6-9=-27/111, 5-9=-27/111, 4-5=0/0
WEBS 2-5=-464/113, 2-6=-57/619

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 76 lb uplift at joint 1, 11 lb uplift at joint 3 and 110 lb uplift at joint 5.
8) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 212 lb down and 35 lb up at 1-0-12, and 212 lb down and 35 lb up at 3-0-12, and 212 lb down and 35 lb up at 5-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Continued on page 2

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January 10,2011

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A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927696
RSNCONDE	EJ7C	MONO HIP	1	1	Job Reference (optional)

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
      Vert: 1-2=-60, 2-3=-60, 1-5=-20
   Concentrated Loads (lb)
      Vert: 7=-212(B) 8=-212(B) 9=-212(B)

Job	Truss	Truss Type	Qty	Ply	E5927697
RSNCONDE	EJ7GT	JACK	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:40 2011 Page 1

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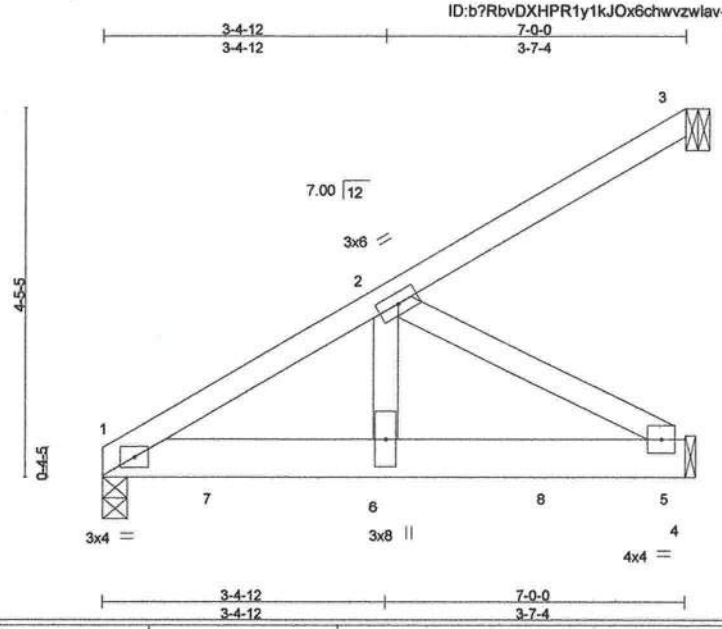


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	-0.02	5-6	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	-0.05	5-6	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.49	Horz(TL)	-0.01	3	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 37 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

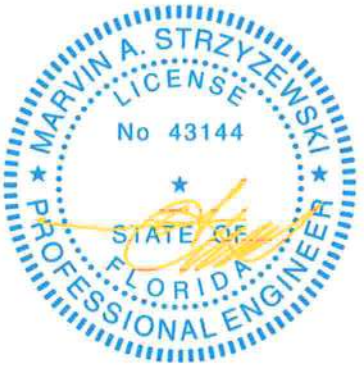
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1274/0-3-8 (min. 0-1-8), 3=87/Mechanical, 5=1124/Mechanical
Max Horz 1=127(LC 5)
Max Uplift 1=-150(LC 5), 3=-42(LC 5), 5=-172(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1663/179, 2-3=-61/34
BOT CHORD 1-7=-246/1401, 6-7=-246/1401, 6-8=-246/1401, 5-8=-246/1401, 4-5=0/0
WEBS 2-6=-162/1383, 2-5=-1604/282

NOTES
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 150 lb uplift at joint 1, 42 lb uplift at joint 3 and 172 lb uplift at joint 5.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 648 lb down and 90 lb up at 1-4-12, and 648 lb down and 90 lb up at 3-4-12, and 648 lb down and 90 lb up at 5-4-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-60, 1-4=-20
Concentrated Loads (lb)
Vert: 6=-648 7=-648 8=-648



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January 10,2011

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A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927698
RSNCONDE	F1	COMMON	2	1	Job Reference (optional)

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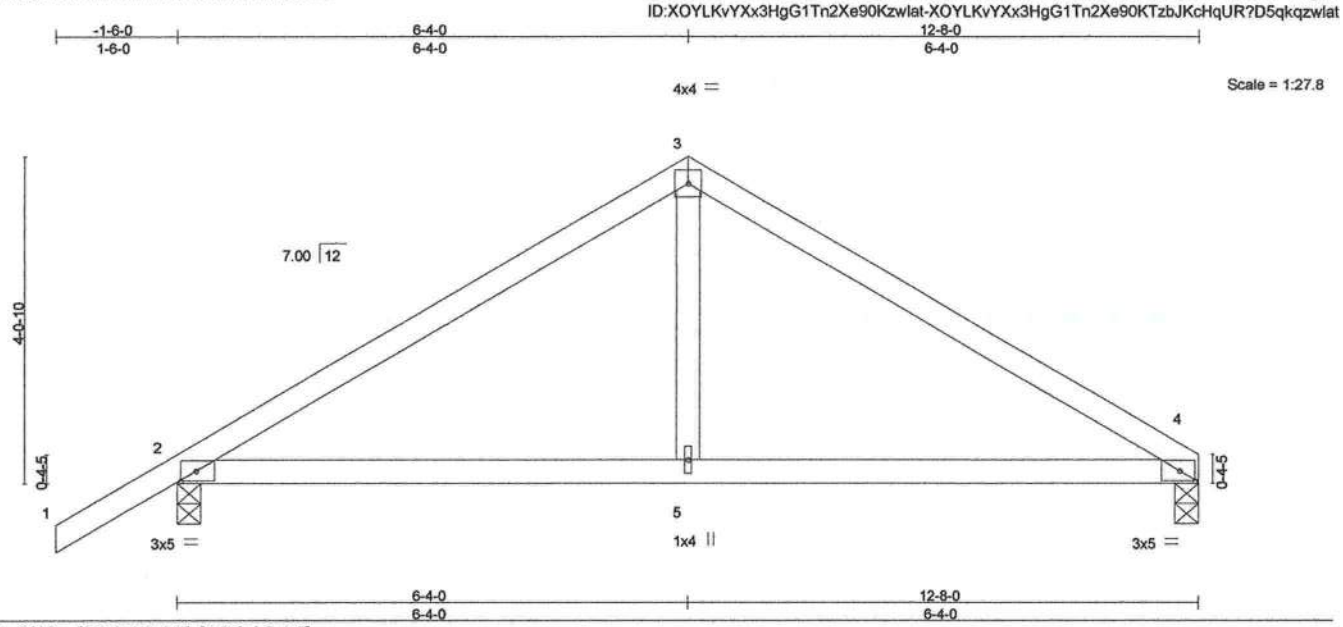

Plate Offsets (X,Y): [2:0-2-4,0-1-8], [4:0-2-4,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	-0.04	4-5	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.11	4-5	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.10	Horz(TL)	0.01	4	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 49 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=488/0-3-8 (min. 0-1-8), 2=600/0-3-8 (min. 0-1-8)
Max Horz 2=119(LC 4)
Max Uplift 4=-62(LC 6), 2=-131(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-643/95, 3-4=-640/81
BOT CHORD 2-5=-15/467, 4-5=-15/467
WEBS 3-5=0/298

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 62 lb uplift at joint 4 and 131 lb uplift at joint 2.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

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January 10,2011

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818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927699
RSNCONDE	F	COMMON	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:41 2011 Page 1

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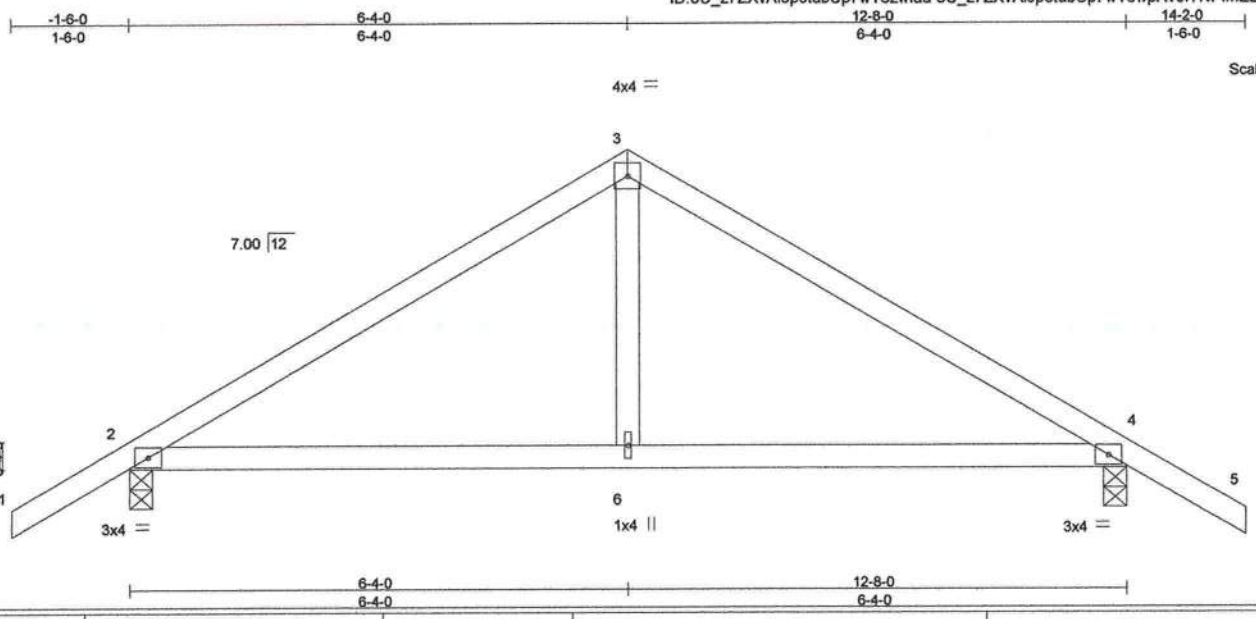

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.03	4-6	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.09	4-6	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.01	4	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 51 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=594/0-3-8 (min. 0-1-8), 4=594/0-3-8 (min. 0-1-8)
Max Horz 2=102(LC 4)
Max Uplift 2=-131(LC 5), 4=-131(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-626/80, 3-4=-626/80, 4-5=0/45
BOT CHORD 2-6=-2/452, 4-6=-2/452
WEBS 3-6=0/295

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 131 lb uplift at joint 2 and 131 lb uplift at joint 4.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

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818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927700
RSNCONDE	FET	GABLE	1	1	Job Reference (optional)

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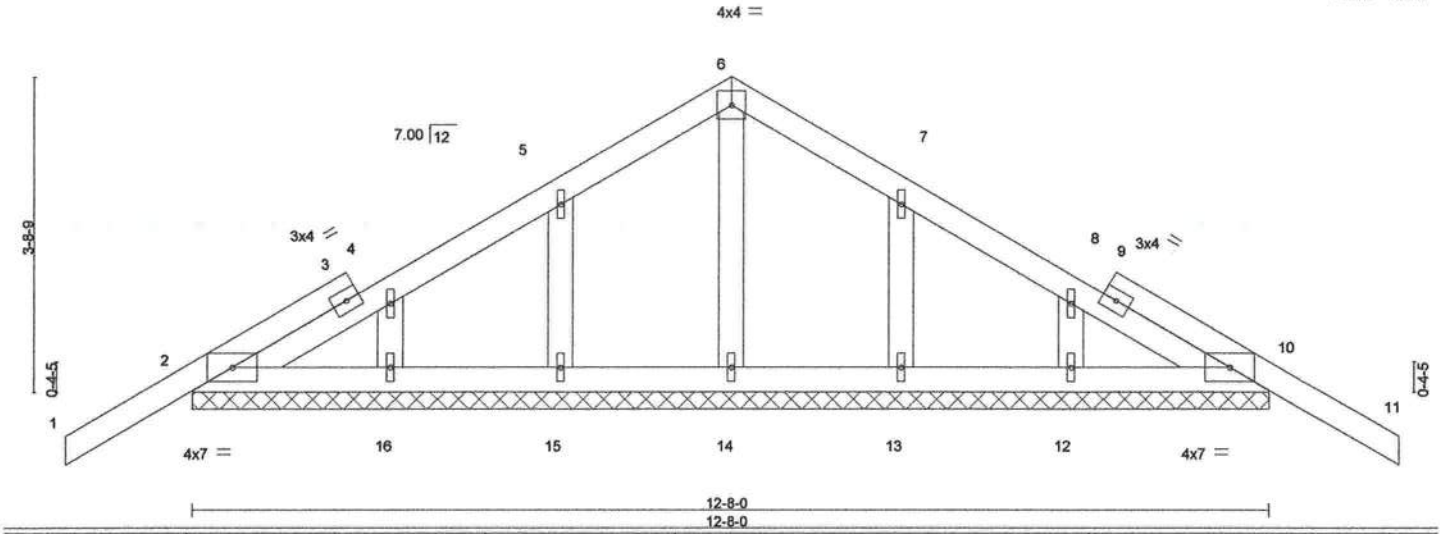


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	-0.01	11	n/r	120	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.01	11	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.00	10	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 64 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

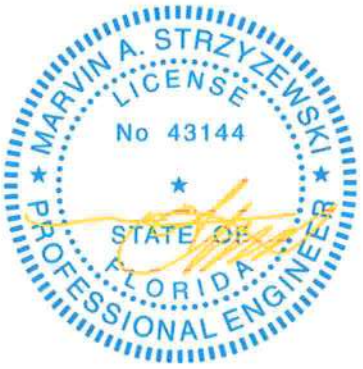
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=209/12-8-0 (min. 0-1-8), 10=209/12-8-0 (min. 0-1-8), 14=132/12-8-0 (min. 0-1-8), 15=170/12-8-0 (min. 0-1-8), 16=152/12-8-0 (min. 0-1-8), 13=170/12-8-0 (min. 0-1-8), 12=152/12-8-0 (min. 0-1-8)
Max Horz 2=122(LC 4)
Max Uplift 2=-124(LC 5), 10=-138(LC 6), 15=-101(LC 5), 16=-45(LC 6), 13=-100(LC 6), 12=-49(LC 6)
Max Grav 2=209(LC 1), 10=209(LC 1), 14=132(LC 1), 15=172(LC 9), 16=152(LC 1), 13=172(LC 10), 12=152(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-88/76, 3-4=-79/87, 4-5=-56/81, 5-6=-49/120, 6-7=-49/112, 7-8=-44/46, 8-9=-29/37, 9-10=-49/26, 10-11=0/44
BOT CHORD 2-16=-10/94, 15-16=-10/94, 14-15=-10/94, 13-14=-10/94, 12-13=-10/94, 10-12=-10/94
WEBS 6-14=-92/0, 5-15=-132/109, 4-16=-112/67, 7-13=-132/109, 8-12=-112/71

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are 1x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 2, 138 lb uplift at joint 10, 101 lb uplift at joint 15, 45 lb uplift at joint 16, 100 lb uplift at joint 13 and 49 lb uplift at joint 12.
10) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



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January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927701
RSNCONDE	FGT	COMMON	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:44 2011 Page 1

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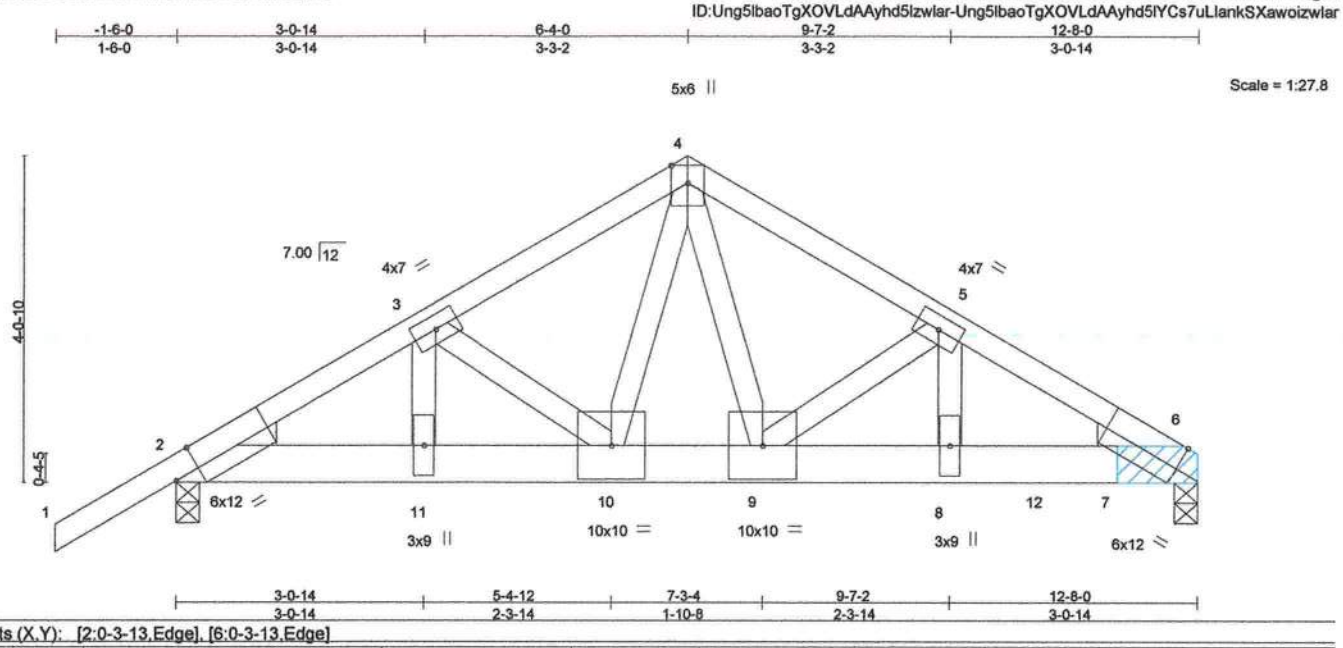

Plate Offsets (X,Y): [2:0-3-13,Edge], [6:0-3-13,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	-0.08	9	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	-0.19	9	>775	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.75	Horz(TL)	0.06	6	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 81 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP SS
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-11-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-4-5 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=4438/(0-3-8 + bearing block) (req. 0-5-4), 2=2678/0-3-8 (min. 0-3-3)
Max Horz 2=119(LC 4)
Max Uplift 6=-738(LC 6), 2=-582(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=-4603/915, 3-4=-4587/962, 4-5=-4627/919, 5-6=-6440/1150
BOT CHORD 2-11=-774/3903, 10-11=-774/3903, 9-10=-633/3392, 8-9=-954/5513, 8-12=-954/5513, 7-12=-954/5513, 6-7=-954/5513
WEBS 4-9=-401/2331, 5-9=-1945/322, 5-8=-258/1942, 4-10=-537/2196, 3-10=-108/138, 3-11=-134/68

NOTES
1) 2 X 6 SYP SS bearing block 12" long at jt. 6 attached to front face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners. Bearing is assumed to be SYP SS.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 738 lb uplift at joint 6 and 582 lb uplift at joint 2.
7) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2044 lb down and 530 lb up at 5-4-12, 1043 lb down and 179 lb up at 7-4-12, 1046 lb down and 165 lb up at 9-4-12, and 1063 lb down and 168 lb up at 10-8-12, and 832 lb down and 85 lb up at 11-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Continued on page 2

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ENGINEERING BY TRENCO
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818 Soundside Road
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Job	Truss	Truss Type	Qty	Ply	E5927701
RSNCONDE	FGT	COMMON	1	1	Job Reference (optional)

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-4=-60, 4-6=-60, 2-6=-20

Concentrated Loads (lb)

Vert: 9=-1043(F) 8=-1046(F) 10=-2044(F) 7=-832(F) 12=-1063(F)

Job	Truss	Truss Type	Qty	Ply	E5927702
RSNCONDE	G1	COMMON	3	1	Job Reference (optional)

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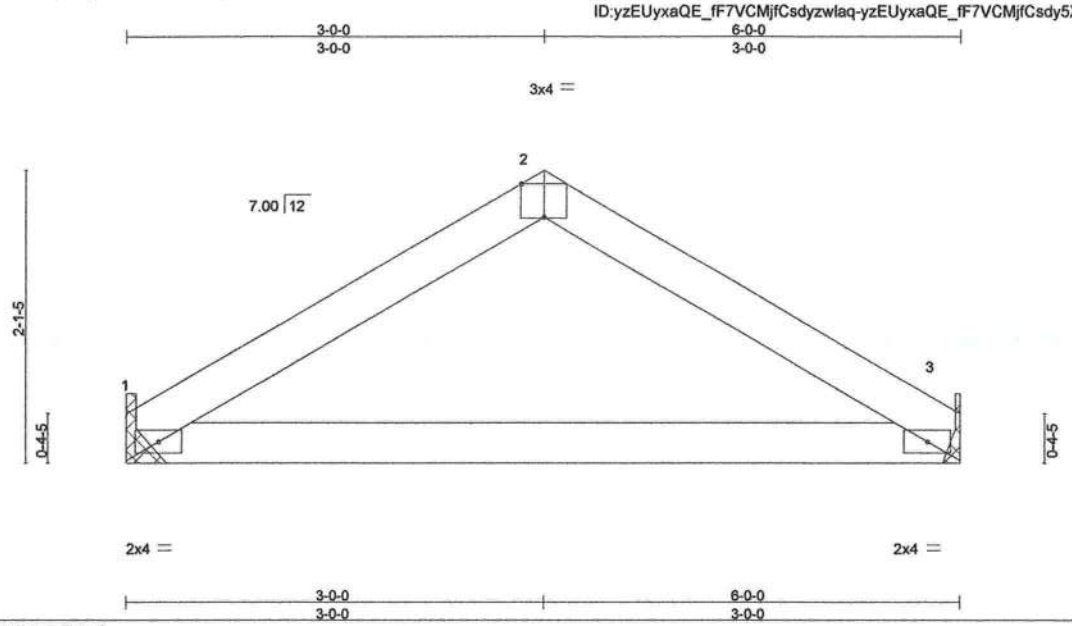

Plate Offsets (X,Y): [2:0-2-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	-0.06	1-3	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.15	1-3	>471	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 20 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=232/Mechanical, 3=232/Mechanical
Max Horz 1=50(LC 4)
Max Uplift 1=-29(LC 5), 3=-29(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-202/53, 2-3=-201/52
BOT CHORD 1-3=-20/140

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 1 and 29 lb uplift at joint 3.
7) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	
RSNCONDE	G	COMMON	1	1	E5927703

Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL

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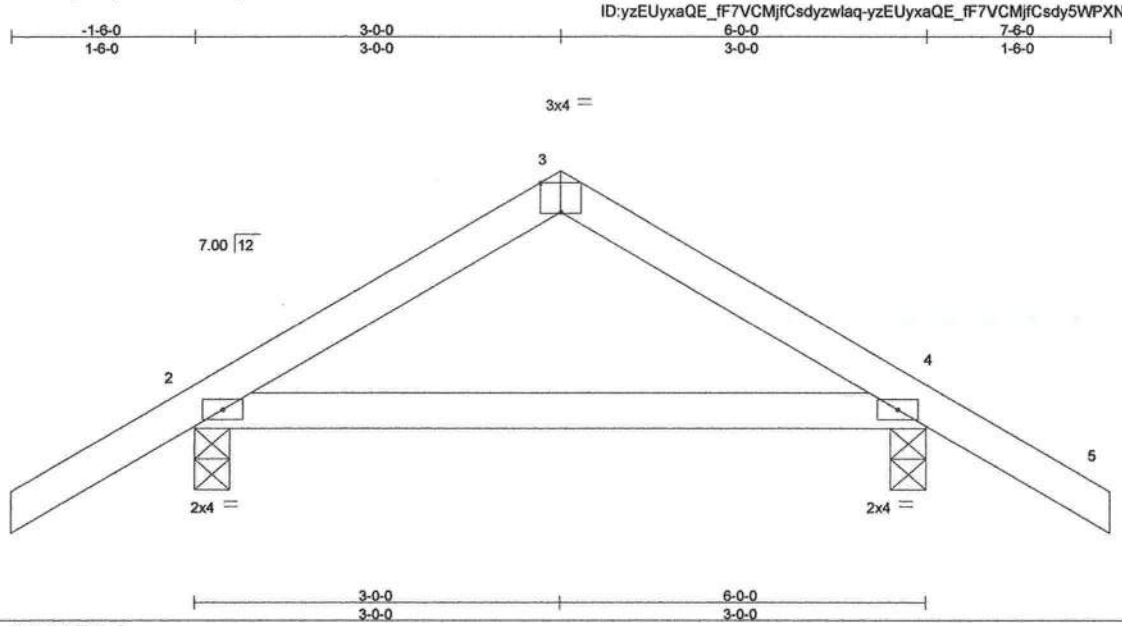


Plate Offsets (X,Y): [3:0-2-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	-0.06	2-4	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.14	2-4	>492	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	4	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 25 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

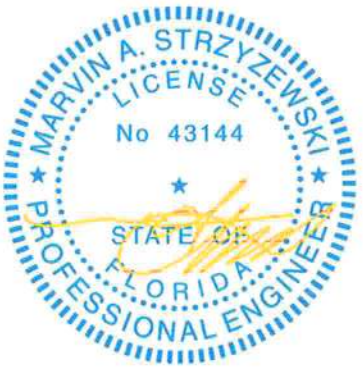
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=327/0-3-8 (min. 0-1-8), 4=327/0-3-8 (min. 0-1-8)
Max Horz 2=52(LC 5)
Max Uplift 2=-103(LC 5), 4=-103(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-168/28, 3-4=-168/28, 4-5=0/45
BOT CHORD 2-4=0/91

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 103 lb uplift at joint 2 and 103 lb uplift at joint 4.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927704
RSNCONDE	GET	GABLE	1	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL — 7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:46 2011 Page 1

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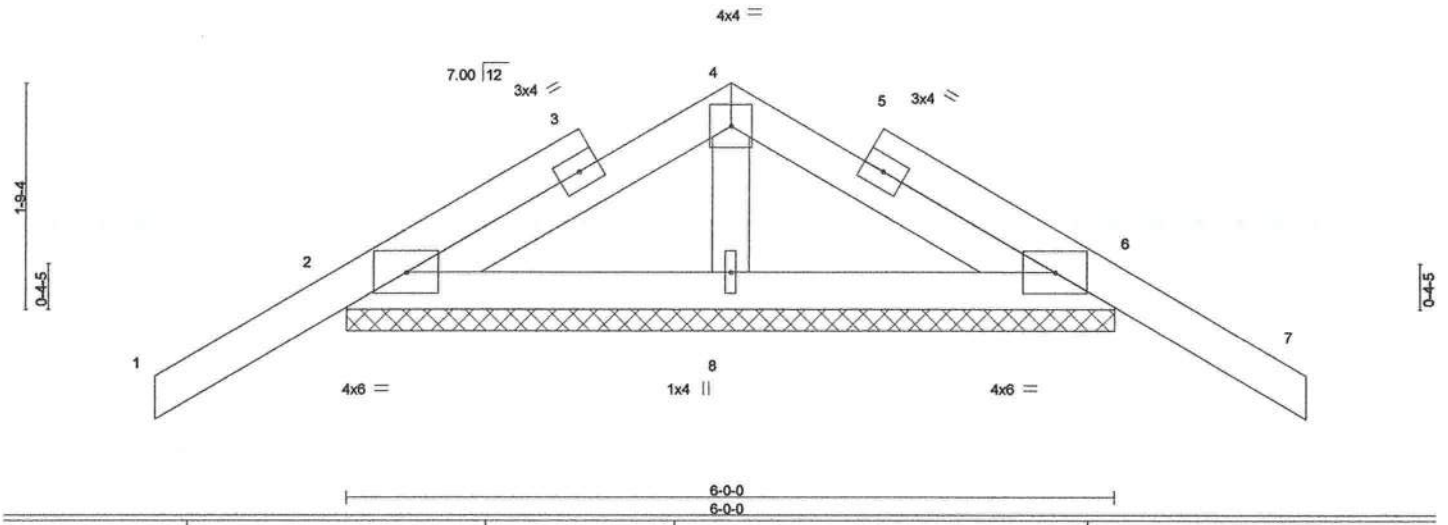


6-0-0 / 6-0-0

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	-0.00	7	n/r	120	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.01	7	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.00	6	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 31 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

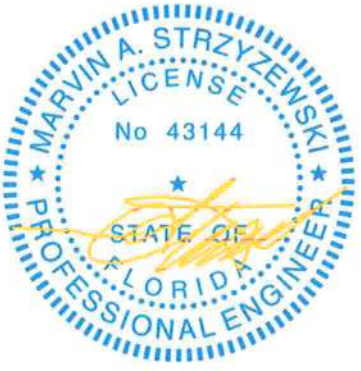
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=212/6-0-0 (min. 0-1-8), 6=212/6-0-0 (min. 0-1-8), 8=235/6-0-0 (min. 0-1-8)
Max Horz 2=55(LC 4)
Max Uplift 2=-148(LC 5), 6=-158(LC 6), 8=-24(LC 5)
Max Grav 2=217(LC 9), 6=217(LC 10), 8=235(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-21/39, 3-4=-12/54, 4-5=0/54, 5-6=-26/20, 6-7=0/44
BOT CHORD 2-8=-26/71, 6-8=-26/71
WEBS 4-8=-160/47

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 148 lb uplift at joint 2, 158 lb uplift at joint 6 and 24 lb uplift at joint 8.
9) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



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A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927705
RSNCONDE	J01	JACK	2	1	Job Reference (optional)

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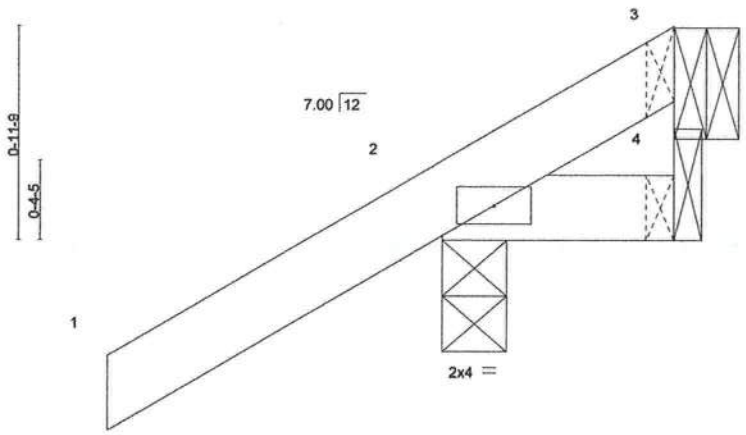

1-0-7 / 1-0-7

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 6 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=203/0-3-8 (min. 0-1-8), 4=10/Mechanical, 3=-42/Mechanical
Max Horz 2=62(LC 5)
Max Uplift 2=-123(LC 5), 3=-42(LC 1)
Max Grav 2=203(LC 1), 4=20(LC 2), 3=48(LC 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/44, 2-3=-55/25
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 123 lb uplift at joint 2 and 42 lb uplift at joint 3.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

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January 10,2011

ENGINEERING BY TRENCO
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818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927706
RSNCONDE	J03	JACK	2	1	Job Reference (optional)

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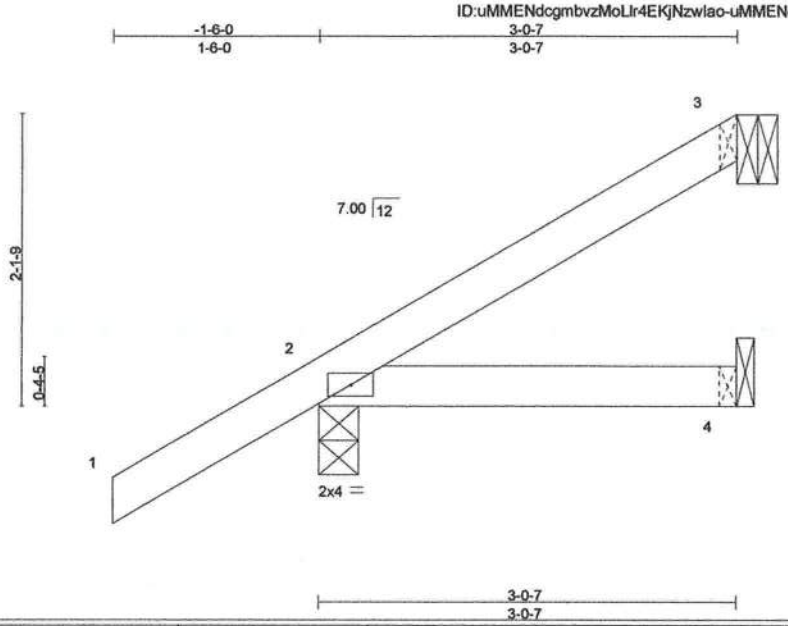

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	2-4	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 13 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=53/Mechanical, 2=239/0-3-8 (min. 0-1-8), 4=28/Mechanical
Max Horz 2=98(LC 5)
Max Uplift 3=-23(LC 4), 2=-90(LC 5)
Max Grav 3=53(LC 1), 2=239(LC 1), 4=57(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-60/19
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at joint 3 and 90 lb uplift at joint 2.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

FL Cert. #7239

January 10,2011

ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	E5927707
RSNCONDE	J05	JACK	2	1	Job Reference (optional)

SANTA FE TRUSS COMPANY, INC., HIGH SPRINGS, FL

7.250 s Nov 19 2010 MiTek Industries, Inc. Mon Jan 10 13:49:47 2011 Page 1

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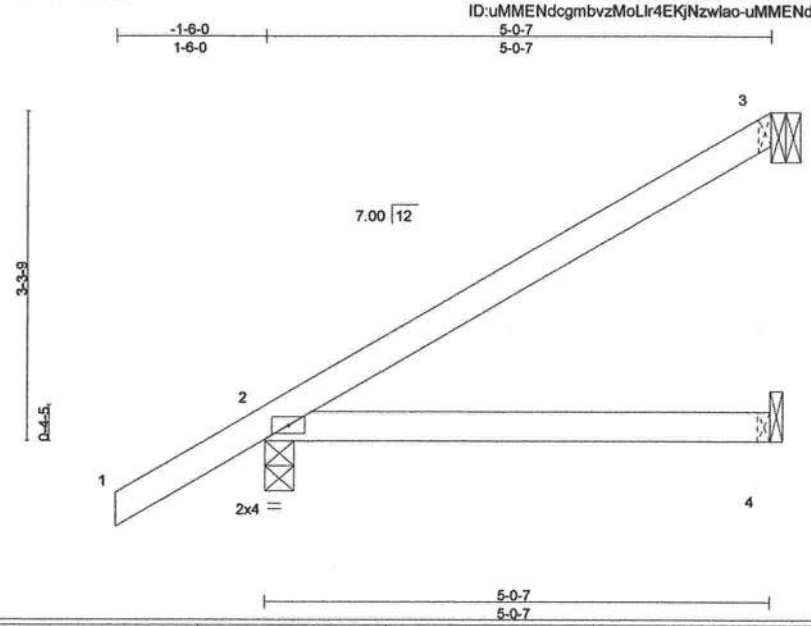


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	10.0	Lumber Increase	1.25	BC	0.22	Vert(TL)	-0.07	2-4	>813	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2007/TPI2002		(Matrix)							Weight: 19 lb	FT = 15%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

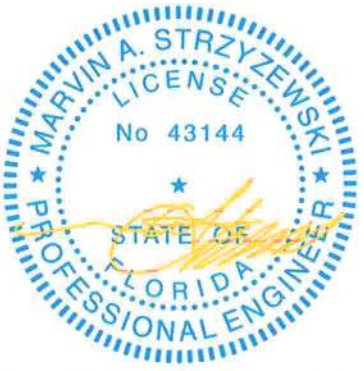
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=125/Mechanical, 2=307/0-3-8 (min. 0-1-8), 4=48/Mechanical
Max Horz 2=135(LC 5)
Max Uplift 3=-61(LC 5), 2=-83(LC 5)
Max Grav 3=125(LC 1), 2=307(LC 1), 4=97(LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/45, 2-3=-83/50
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=6.0psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 61 lb uplift at joint 3 and 83 lb uplift at joint 2.
6) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



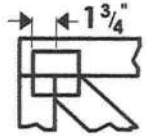
FL Cert. #7239

January 10,2011

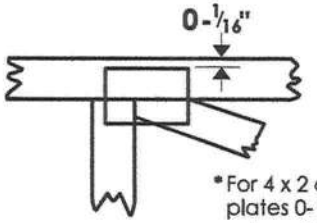
ENGINEERING BY TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

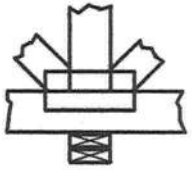
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

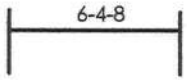


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

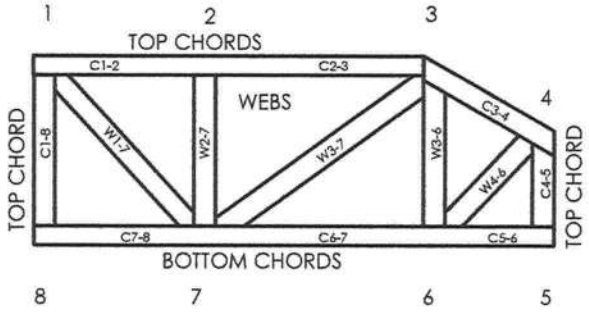
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B
9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

MiTek® POWER TO PERFORM.™
ENGINEERING BY TRENCO
A MiTek Affiliate

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

Columbia County Building Permit Application   CK# 11903

For Office Use Only Application # 1101-38 Date Received 1/28 By JW Permit # 29204

Zoning Official BLK Date 08.02.11 Flood Zone X Land Use A-3 Zoning A-3

FEMA Map # N/A Elevation N/A MFE 1' above rd River N/A Plans Examiner T.C. Date 2-2-11

Comments Existing MH to be removed 45 days after CO is issued

☑ NOC ☑ EH ☑ Deed or PA ☑ Site Plan □ State Road Info □ Parent Parcel #

□ Dev Permit # □ In Floodway □ Letter of Auth. from Contractor □ F W Comp. letter

IMPACT FEES: EMS Fire Corr Road/Code ☑ Garage Door: Sign

School = TOTAL Ø Suspended - VFD Insulation Vignette

Septic Permit No. 11-0014 Fax 386-454-2166

Name Authorized Person Signing Permit Scott Rosenboom Phone 352-538-3877

Address 19802 NW 190th AVE High Springs, FL 32643

Owners Name CLAUDIO & GRACIE F. CONDE Phone 386-454-8651

911 Address 245 SW WAFFLE GLN

Contractors Name Scott Rosenboom Phone 352-538-3877

Address 19802 NW 190th AVE High Springs FL 32643

Fee Simple Owner Name & Address CONDE, 245 SW WAFFLE GLN

Bonding Co. Name & Address

Architect/Engineer Name & Address SCHAFER ENG 7104 NW 42nd LANE Gville 32606

Mortgage Lenders Name & Address COMPASS BANK

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progress Energy   as per contractor

Property ID Number 18-7S-17-10021-009 Estimated Cost of Construction 225,000.00

Subdivision Name Lot Block Unit Phase

Driving Directions EAST FROM FT WHITE ON 27 TO WAFFLE GLN TURN LEFT, 1ST HOUSE ON LEFT

Number of Existing Dwellings on Property 1 (to be removed?)

Construction of NEW HOME - "SFD" as per contractor Total Acreage 6.45 Lot Size

Do you need a - Culvert Permit or Culvert Waiver or (Have an Existing Drive) off Waffle Gln Total Building Height

Actual Distance of Structure from Property Lines - Front 200 Side 200 Side 200 Rear 300

Number of Stories 1 Heated Floor Area 2138 Total Floor Area 3206 Roof Pitch 7

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction. **CODE:** Florida Building Code 2007 with 2009 Supplements and the 2008 National Electrical Code.

JW left message on VM 2.8.11

Columbia County Building Permit Application

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full.
This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE: ***YOU ARE HEREBY NOTIFIED*** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. It may be to your advantage to check and see if your property is encumbered by any restrictions.

(Owners Must Sign All Applications Before Permit Issuance.)

Gracie T Conde Cleora Ware

Owners Signature ****OWNER BUILDERS MUST PERSONALLY APPEAR AND SIGN THE BUILDING PERMIT.**

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

Scott Rozier

Contractor's Signature (Permitee)

Contractor's License Number CBC125-7076
Columbia County
Competency Card Number

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 28th **day of** January **20** 11.

Personally known ✓ **or Produced Identification**

Laurie Hodson

State of Florida Notary Signature (For the Contractor)

SEAL:

LAURIE HODSON
MY COMMISSION # DD 805657
EXPIRES: July 14, 2012
Bonded Thru Notary Public Underwriters

This Document Prepared By and Return to:
Darryl J. Tompkins, Esquire
Darryl J. Tompkins, P.A.
14420 NW 151st Blvd.
P.O. Box 519
Alachua, FL 32616

Inst:2006030165 Date:12/27/2006 Time:11:14
Doc Stamp-Deed : 630.00
DC,P.DeWitt Cason,Columbia County B:1105 P:2232

Parcel ID Number: R10021-009

Warranty Deed

This Indenture, Made this 22nd day of **December**, 2006 A.D., **Between**
William B. Erwin and Beverly M. Erwin, husband and wife

of the County of **Terrebonne**, State of **Louisiana**, **grantors**, and
Claudio Conde and Gracie F. Conde, husband and wife

whose address is: **245 SW Waffle Glen, Fort White, FL 32038**

of the County of **Columbia**, State of **Florida**, **grantees.**

Witnesseth that the GRANTORS, for and in consideration of the sum of
----------**TEN DOLLARS ($10)**---------- DOLLARS,
and other good and valuable consideration to GRANTORS in hand paid by GRANTEES, the receipt whereof is hereby acknowledged, have granted, bargained and sold to the said GRANTEES and GRANTEES' heirs, successors and assigns forever, the following described land, situate, lying and being in the County of **Columbia** State of **Florida** to wit:
See Exhibit "A" attached hereto.

SUBJECT TO THE FOLLOWING:

A. Zoning restrictions, prohibitions and other requirements imposed by governmental authority;
B. Restrictions and matters appearing on the plat and/or common to the subdivision;
C. Taxes for the year 2007 and subsequent years.

and the grantors do hereby fully warrant the title to said land, and will defend the same against lawful claims of all persons whomsoever.

In Witness Whereof, the grantors have hereunto set their hands and seals the day and year first above written.

Signed, sealed and delivered in our presence:

Sandra E Howe
Printed Name: Sandra E Howe
Witness

Linda J Perri
Printed Name: Linda J Perri
Witness

Linda Mulligan
Printed Name: Linda Mulligan
Witness

Cindy Holloway
Printed Name: Cindy Holloway
Witness

_____ (Seal)
William B. Erwin
P.O. Address: 102 Bayou Gardens Drive, Houma, LA 70364

Beverly M Erwin (Seal)
Beverly M. Erwin
P.O. Address: 102 Bayou Gardens Drive, Houma, LA 70364

STATE OF Florida
COUNTY OF Alachua

The foregoing instrument was acknowledged before me this 22nd day of **December**, 2006 by **William B. Erwin, a married man**

who is personally known to me or who has produced his **Florida driver's license as identification**.

Notary Public State of Florida
Sandra E Howe
My Commission DD481123
2009

Sandra E. Howe
Printed Name: Sandra E. Howe
Notary Public
My Commission Expires: 11/15/2009

RWIN06-233

SSS 11-0014

STATE OF FLORIDA
DEPARTMENT OF HEALTH
ON-SITE SEWAGE DISPOSAL SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT

PERMIT NO. 989874
DATE PAID: 1/11/11
FEE PAID: 495.00
RECEIPT #: 1533796

APPLICATION FOR:
[X] New System [ ] Existing System [ ] Holding Tank [ ] Innovative
[ ] Repair [ ] Abandonment [ ] Temporary [ ] ________

APPLICANT: CLAUDIO & GRACIE CONDE

AGENT: Scott Rosenboom TELEPHONE: 352-538-3817

MAILING ADDRESS: 19802 NW 140TH AVE High Springs, Fl 32643

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTED BY A PERSON LICENSED PURSUANT TO 489.105(3)(m) OR 489.552, FLORIDA STATUTES.

PROPERTY INFORMATION

LOT: ______ BLOCK: ______ SUBDIVISION: ______ PLATTED: ______

PROPERTY ID #: 18-7S-17-10021-009 ZONING: AG I/M OR EQUIVALENT: (Y / N)

PROPERTY SIZE: 6.45 ACRES WATER SUPPLY: [✓] PRIVATE PUBLIC [ ]<=2000GPD [ ]>2000GPD

IS SEWER AVAILABLE AS PER 381.0065, FS? [Y / (N)] DISTANCE TO SEWER: ______FT

PROPERTY ADDRESS: 245 SW WAFFLE GLEN FT WHITE 32038

DIRECTIONS TO PROPERTY: FRM FT WHITE GO EAST ON 27 TO WAFFLE GLN TURN LEFT IT'S FIRST PLACE ON LEFT

BUILDING INFORMATION [ ] RESIDENTIAL [ ] COMMERCIAL

(3R)

Unit No	Type of Establishment	No. of Bedrooms	Building Area Sq Ft	Commercial/Institutional System Design Table 1, Chapter 64E-6, FAC
1	Mobile Home	2	924	*Size system for
2				future use = 3BR SFR
3				2138ft²
4				

[ ] Floor/Equipment Drains [ ] Other (Specify) ______

SIGNATURE: Scott Rosenboom DATE: 1/10/11

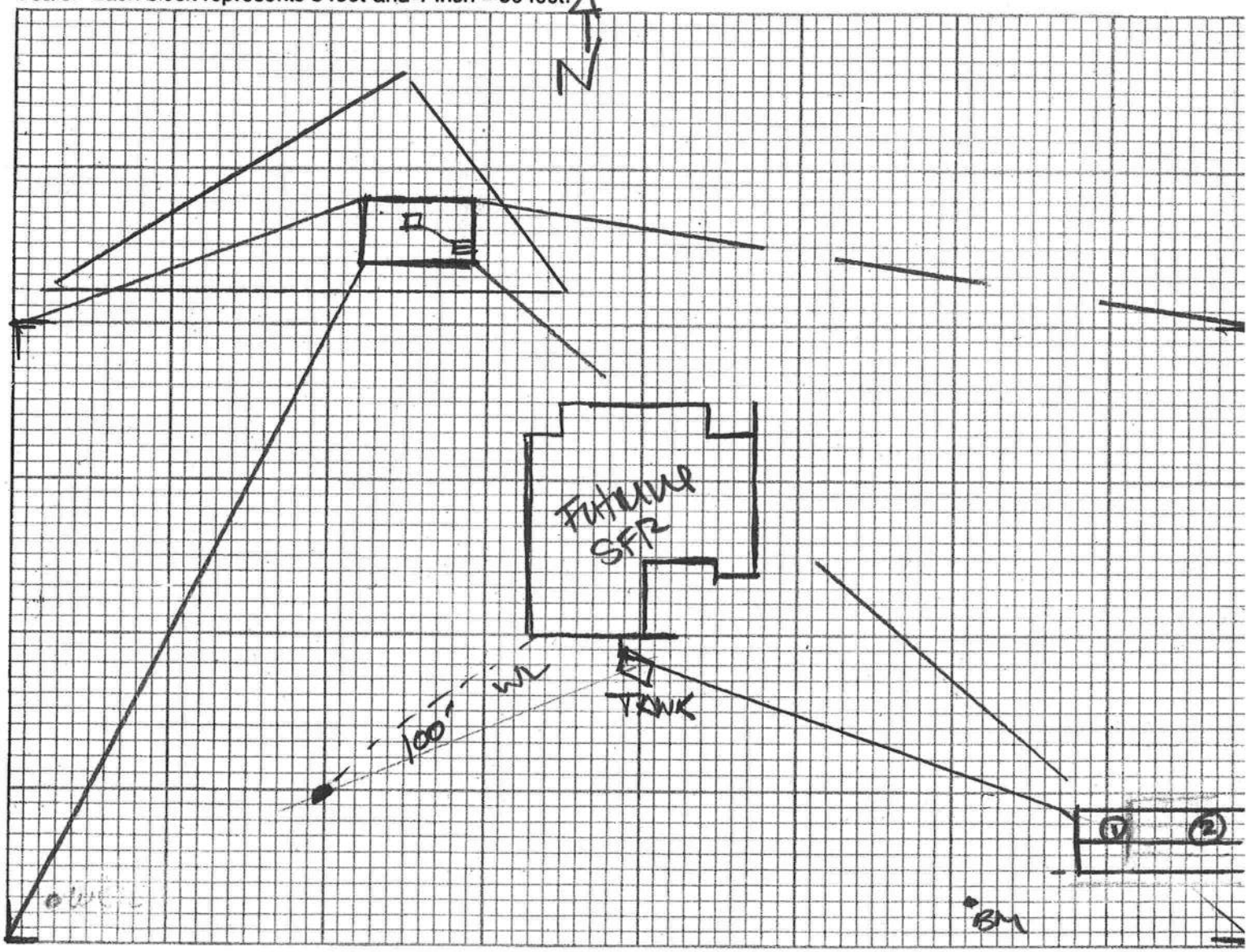
STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number _____

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes: _____

Site Plan submitted by: _____ _____
Signature Title

Plan Approved X Not Approved _____ Date 1/25/11

By Scott Rowland County Health Departme

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

DH 4015, 10/96 (Replaces HRS-H Form 4015 which may be used)
(Stock Number: 5744-002-4015-6)

SUBCONTRACTOR VERIFICATION FORM

APPLICATION NUMBER 1101-38 CONTRACTOR SCOTT ROSENBOOM PHONE 352.

THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

In Columbia County one permit will cover all trades doing work at the permitted site. It is REQUIRED that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

ELECTRICAL 380	Print Name DONALD DAVIS	Signature Donald Davis
	License #: EC 000 2306	Phone #: 386-623-0499
MECHANICAL/ A/C 960	Print Name William Hogle	Signature Will Hogle
	License #: CAC057124	Phone #: 352-332-1508
PLUMBING/ GAS 441	Print Name JOE DAVIS	Signature Joe Davis
	License #: CFC 057304	Phone #: 386-623-3487
ROOFING 373	Print Name Tim McKee	Signature Tim McKee
	License #: CCC 58050	Phone #: 352-339-4135
SHEET METAL	Print Name	Signature
	License #:	Phone #:
FIRE SYSTEM/ SPRINKLER	Print Name	Signature
	License#:	Phone #:
SOLAR	Print Name	Signature
	License #:	Phone #:

Specialty License	License Number	Sub-Contractors Printed Name	Sub-Contractors Signature
MASON	—	—	—
CONCRETE FINISHER	CBC125 2076	Scott Rosenboom	Scott Rosenb
FRAMING	000033	MICHAEL L HADDEN	Michael L Hadden
INSULATION	[illegible]	SEE ATTACHED - V2	
STUCCO	—	—	—
DRYWALL	—	—	—
PLASTER	CBC 125 2076	Scott Rosenboom	Scott Rosenb
CABINET INSTALLER	CBC 125 2076	Scott Rosenboom	Scott Rosenb
PAINTING	CBC 125 2076	Scott Rosenboom	Scott Rosenb
ACOUSTICAL CEILING	—	—	—
GLASS	—	—	—
CERAMIC TILE	000997	MIKE WRIGHT	Mike Wright
FLOOR COVERING	000998	MIKE WRIGHT	Mike Wright
ALUM/VINYL SIDING	CBC029025	DAVID MEYER	David A Meyer
GARAGE DOOR 1092	CBC1251673	LESTER'S GARAGE DOORS	[illegible]
METAL BLDG ERECTOR	—	—	—

F. S. 440.103 Building permits; identification of minimum premium policy.--Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each time the employer applies for a building permit.

Prepared by:
Record and Return to: T. Moody
Integrity Title & Escrow
4731 NW 53rd Avenue, Suite 2
Gainesville, FL 32653
File # IT 101218

Inst. 201112000008 Date:1/3/2011 Time:10:47 AM
DC,P.DeWitt Cason,Columbia County Page 1 of 4 B:1207 P:1536

Permit No. _____ Tax Folio No._____

NOTICE OF COMMENCEMENT

State of FLORIDA
County of COLUMBIA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property: *[legal description of the property and street address if available]*

SEE EXHIBIT "A" ATTACHED HERETO AND INCORPORATED HEREIN FOR ALL PURPOSES.

(more commonly known as 245 SW Waffle Glen
Fort White, FL 32038)

2. General description of improvement:

3. Owner information

a. Name and address CLAUDIO CONDE and GRACIE F. CONDE
245 SW Waffle Glen
Fort White, FL 32038

b. Interest in property

c. Name and address of fee simple titleholder *[if other than owner]*

4.

a. Contractor: *[name and address]* *ROSENBOOM, INC.*
18266 US 441
High Springs, FL 32643

b. Contractor's phone number:

5. Surety

a. Name and address

b. Phone number

c. Amount of bond $ _____

6.

a. Lender *[name and address]* *COMPASS BANK*
P. O. Box 10687
Birmingham, AL 35202

b. Lender's phone number:

7.

a. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)(7)., Florida Statutes:
_____ *[name and address]*

b. Phone number

8.

a. In addition to himself, Owner designates Shane White of Compass Bank, 401 West Valley Avenue, Homewood, AL 35209 Mail Code ALBIHWGEX to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

b. Phone number of person or entity designated by owner: 205-524-5721

9. Expiration of date of Notice of Commencement: (the expiration date is 1 year from the date of recording unless a different date is specified)

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

_____

(Signature of Owner or Owner's Authorized Officer/Director/Partner/Manager)

The foregoing instrument was acknowledged before me this 30 day of Dec, 2010, (year), by Claudio Conde and Gracie F. Conde (name of person) as Owners (type of authority, e.g. officer, trustee, attorney in fact) for _____ (name of party on behalf of whom instrument was executed).

_____
(Signature of Notary Public - State of Florida)

NOTARY PUBLIC-STATE OF FLORIDA
Terri Moody
Commission #DD674033
Expires: JUNE 02, 2013
BONDED THRU ATLANTIC BONDING CO., INC

(Print, Type, or Stamp Commissioned Name of Notary Public)

Personally Known _____

OR Produced Identification drivers License

Type of Identification Produced drivers License

Verification pursuant to Section 92.525, Florida Statutes.

Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

Gracie F. Conde

(Signature of Natural Person Signing Above)

Escrow File No.: IT101218

EXHIBIT "A"

Part of Section 18, Township 7 South, Range 17 East, Columbia County, Florida, and being part of those lands described in Official Records Book 741, Page 898 of the Official Records of Columbia County, Florida, more particularly described as follows:

Commence at the Southeast corner of Section 18, Township 7 South, Range 17 East, Columbia County, Florida, and thence South 88°00'29" West, along the South line of said Section 18, a distance of 2641.60 feet to a concrete monument marking the Southeast 1/4 of the Southwest 1/4 of said Section 18; thence North 01°30'02" West, along the East line of said Southeast 1/4 of the Southwest 1/4, a distance of 210.00 feet to a concrete monument marking the Northeast corner of the South 210.00 feet of said Southeast 1/4 of the Southwest 1/4; thence South 88°00'29" West, along the North line of said South 210.00 feet, a distance of 166.33 feet to the Point of Beginning; thence North 45°01'45" West, along the Easterly line of those lands described in Official Records Book 741, Page 898, a distance of 660.72 feet; thence South 54°15'42" West, 790.37 feet; thence South 45°01'45" East, along the Northeasterly right of way line of State Road No. 20 (A.K.A. US Highway 27) to the aforementioned North line of the South 210.00 feet of the Southeast 1/4 of the Southwest 1/4; thence North 88°00'29" East, along said North line, 1067.16 feet to the Point of Beginning.

Together with and subject to an easement for ingress and egress over and across the South 40.00 feet of the above described lands.

Parcel ID#R10021-009

COLUMBIA COUNTY BUILDING DEPARTMENT 6-25-09
RESIDENTIAL CHECK LIST REQUIRMENTS

MINIMUM PLAN REQUIREMENTS FOR THE FLORIDA BUILDING CODE RESIDENTIAL 2007 EFFECTIVE 1 MARCH 2009 & 2009 SUPPLEMENTS EFFECTIVE 1 MARCH 2009, ONE (1) AND TWO (2) FAMILY DWELLINGS with Supplements and Revision, OF THE NATIONAL ELECTRICAL 2008

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE with the Current 2007 FLORIDA BUILDING CODES RESIDENTIAL EFFECTIVE 1 MARCH 2009 & 2009 SUPPLEMENTS EFFECTIVE 1 MARCH 2009. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FIGURE R301.2(4) of the FLORIDA BUILDING CODES RESIDENTIAL (Florida Wind speed map) **SHALL BE USED.**

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE -------- 100 MPH
ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------110 MPH
NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL			Items to Include- Each Box shall be Circled as Applicable		
				Yes	No	N/A
1	Two (2) complete sets of plans containing the following:			✓		
2	All drawings must be clear, concise, drawn to scale, details that are not used shall be marked void			✓		
3	Condition space (Sq. Ft.)		Total (Sq. Ft.) under roof	IIIIIII	IIIIIII	IIIII

X Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES RESIDENTIAL R101.2.1

Site Plan information including:

4	Dimensions of lot or parcel of land	✓		
5	Dimensions of all building set backs	✓		
6	Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.	✓		
7	Provide a full legal description of property.	✓		

45	Show required amount of ventilation opening for under-floor spaces			✓
46	Show required covering of ventilation opening			✓
47	Show the required access opening to access to under-floor spaces			✓
48	Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & inter of the areas structural panel sheathing			✓
49	Show Draftstopping, Fire caulking and Fire blocking			✓
50	Show fireproofing requirements for garages attached to living spaces, per FBCR section 309	✓		
51	Provide live and dead load rating of floor framing systems (psf).			✓

FBCR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable		
		YES	NO	N/A
52	Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls	✓		
53	Fastener schedule for structural members per table FBCR 602.3 are to be shown	✓		
54	Show Wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing	✓		
55	Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems	✓		
56	Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FBCR Table 502.5 (1) HEADER SHEET	✓		
57	Indicate where pressure treated wood will be placed			
58	Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas	✓		
59	A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail	✓		

FBCR :ROOF SYSTEMS:

60	Truss design drawing shall meet section FBCR 802.10 Wood trusses	✓		
61	Include a layout and truss details, signed and sealed by Florida Professional Engineer	✓		
62	Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters	✓		
63	Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	✓		
64	Provide dead load rating of trusses	✓		

FBCR 802:Conventional Roof Framing Layout

65	Rafter and ridge beams sizes, span, species and spacing			✓
66	Connectors to wall assemblies' include assemblies' resistance to uplift rating			✓
67	Valley framing and support details			✓
68	Provide dead load rating of rafter system			✓

FBCR Table 602,3(2) & FBCR 803 ROOF SHEATHING

69	Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness			✓
70	Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas			✓

FBCR ROOF ASSEMBLIES FRC Chapter 9

71	Include all materials which will make up the roof assembles covering	✓		
72	Submit Florida Product Approval numbers for each component of the roof assembles covering	✓		

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FBCR Chapter 11 Energy Efficiency Code for residential building

Residential construction shall comply with this code by using the following compliance methods in the FBCR chapter11 Residential buildings compliance methods. **Two of the required forms are to be submitted**, *N1100.1.1.1 As an alternative to the computerized Compliance Method A, the Alternate Residential Point System Method hand calculation, Alternate Form 600A, may be used. All requirements specific to this calculation are located in Sub appendix C to Appendix G. Buildings complying by this alternative shall meet all mandatory requirements of this chapter. Computerized versions of the Alternate Residential Point System Method shall not be acceptable for code compliance.*

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable		
		YES	NO	N/A
73	Show the insulation R value for the following areas of the structure	✓		
74	Attic space	✓		
75	Exterior wall cavity	✓		
76	Crawl space			✓

HVAC information

77	Submit two copies of a Manual J sizing equipment or equivalent computation study	✓		
78	Exhaust fans shown in bathrooms **Mechanical exhaust capacity of 50 cfm intermittent or 20 cfm continuous required**	✓		
79	Show clothes dryer route and total run of exhaust duct 12'	✓		

Plumbing Fixture layout shown

80	All fixtures waste water lines shall be shown on the foundation plan	✓		
81	Show the location of water heater	✓		

Private Potable Water

82	Pump motor horse power 1HP	✓		
83	Reservoir pressure tank gallon capacity Dis 32gal ~~135~~ 120gal	✓		
84	Rating of cycle stop valve if used NONE	✓		

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Electrical layout shown including

85	Show Switches, receptacles outlets, lighting fixtures and Ceiling fans	✓		
86	Show all 120-volt, single phase, 15- and 20-ampere branch circuits outlets required to be protected by **Ground-Fault Circuit Interrupter (GFCI) Article 210.8 A**	✓		
87	Show the location of smoke detectors & Carbon monoxide detectors	✓		
88	Show service panel, sub-panel, location(s) and total ampere ratings	✓		
89	On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type. **For structures** with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an Grounding electrode system. Per the National Electrical Code article 250.52.3			
90	Appliances and HVAC equipment and disconnects			
91	Show all 120-volt, single phase, 15- and 20-ampere branch circuits supplying outlets installed in dwelling unit family rooms, dining rooms, living rooms, parlors, libraries, dens, bedrooms, sunrooms, recreation rooms, closets, hallways, or similar rooms or areas shall be protected by a listed **Combination arc-fault circuit interrupter,** Protection device.			

Disclosure Statement for Owner Builders *If you as the applicant will be acting as an owner/builder under section 489.103(7) of the Florida Statutes, submit the required owner builder disclosure statement form.*

Notice Of Commencement

A notice of commencement form **recorded** in the Columbia County Clerk Office is required to be filed with the building department Before Any Inspections can be preformed.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

		YES	NO	N/A
92	**Building Permit Application** A current Building Permit Application form is to be completed and submitted for all residential projects			
93	**Parcel Number** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested			
94	**Environmental Health Permit or Sewer Tap Approval** A copy of a approved Columbia County Environmental Health (386) 758-1058			
95	**City of Lake City** A permit showing an approved waste water sewer tap			
96	**Toilet facilities shall be provided for all construction sites**			
97	**Town of Fort White** (386) 497-2321 If the parcel in the application for building permit is within the Corporate city limits of Fort White an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit.			

98	**Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations			✓
99	CERTIFIED FINISHED FLOOR ELEVATIONS will be required on any project where the base flood elevation (100 year flood) has been established			✓
100	A development permit will also be required. Development permit cost is **$50.00**			✓
101	**Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (**$25.00**) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (**$50.00**). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.			✓
102	**911 Address:** If the project is located in an area where a 911 address has not been issued, then application for a 911address must be applied for and **received** through the Columbia County Emergency Management Office of 911 Addressing Department (386) 758-1125			✓

Section R101.2.1 of the Florida Building Code Residential:

The provisions of Chapter 1, Florida Building Code, Building shall govern the administration and enforcement of the Florida Building Code, Residential.

Section 105 of the Florida Building Code defines the:

Time limitation of application.

An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

Single-family residential dwelling.

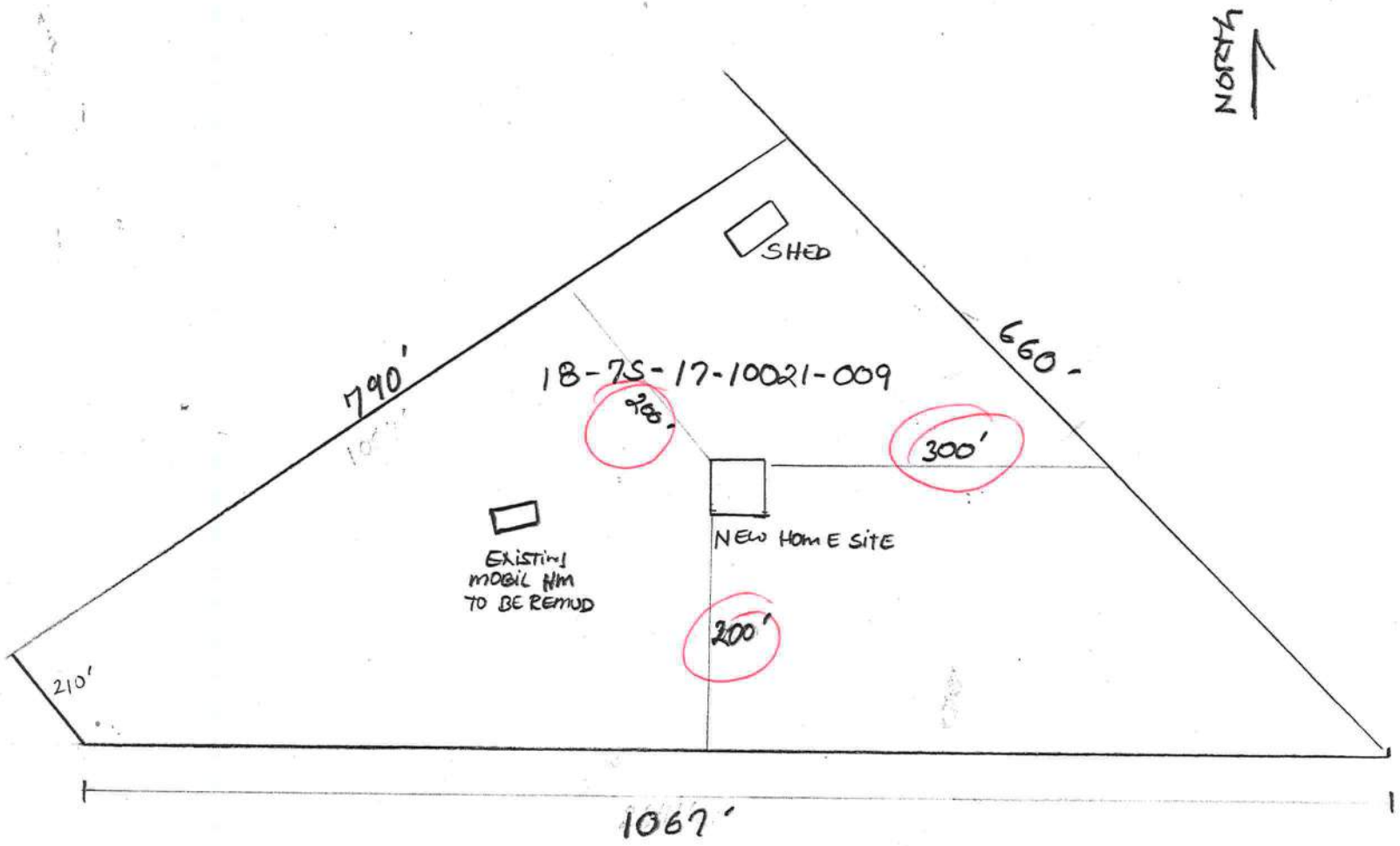
Section 105.3.4 A building permit for a single-family residential dwelling must be issued within 30 working days of application therefor unless unusual circumstances require a longer time for processing the applicatior or unless the permit application fails to satisfy the Florida Building Code or the enforcing agency's laws or ordinances.

Permit intent.

Section 105.4.1: A permit issued shall be constructed to be a license to proceed with the work and not as authority to violate, cancel, alter or set aside any of the provisions of the technical codes, nor shall issuance of a permit prevent the building official from thereafter requiring a correction of errors in plans, construction or violations of this code. Every permit issued shall become invalid unless the work authorized by such permit is commenced within six months after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of six months after the time the work is commenced.

Rosenboom Inc Construction

By Scott Rosenboom



FROM : ROSENBOOM INC. PHONE NO. : 904 454 2894 Jan. 31 2011 12:11PM P1

SUBCONTRACTOR VERIFICATION FORM

APPLICATION NUMBER 1101-38 CONTRACTOR Scott Rosenboom PHONE 352-538-3877

THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

In Columbia County one permit will cover all trades doing work at the permitted site. It is REQUIRED that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

ELECTRICAL	Print Name _____ License #:	Signature _____ Phone #:
MECHANICAL/ A/C _____	Print Name _____ License #:	Signature _____ Phone #:
PLUMBING/ GAS	Print Name _____ License #:	Signature _____ Phone #:
ROOFING	Print Name _____ License #:	Signature _____ Phone #:
SHEET METAL	Print Name _____ License #:	Signature _____ Phone #:
FIRE SYSTEM/ SPRINKLER	Print Name _____ License#:	Signature _____ Phone #:
SOLAR	Print Name _____ License #:	Signature _____ Phone #:

Specialty License	License Number	Sub-Contractors Printed Name	Sub-Contractors Signature
MASON			
CONCRETE FINISHER			
FRAMING			
✓ INSULATION 1694	CGC1512179	Paul Wilson Hash/ Gale Insulation	Paul Wilson Hash
STUCCO			
DRYWALL			
PLASTER			
CABINET INSTALLER			
PAINTING			
ACOUSTICAL CEILING			
GLASS			
CERAMIC TILE			
FLOOR COVERING			
ALUM/VINYL SIDING			
GARAGE DOOR			
METAL BLDG ERECTOR			

F. S. 440.103 Building permits; identification of minimum premium policy.--Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each time the employer applies for a building permit.

Contractor forms: subcontractor form: 6/09

Reference – Paul Hash –

2:03:18 PM 2/2/2011

Data Contained In Search Results Is Current As Of 02/02/2011 02:01 PM.

Search Results

Please see our glossary of terms for an explanation of the license status shown in these search results.

For additional information, including any complaints or discipline, click on the name.

License Type	Name	Name Type	License Number/ Rank	Status/Expires
Professional Engineer	SCHAFER, BRUCE M	Primary	48984 Prof Engineer	Current, Active 02/28/2011

Main Address*: 7104 NW 42ND LANE GAINESVILLE, FL 326060000

Back New Search

*** denotes**
Main Address - This address is the Primary Address on file.
Mailing Address - This is the address where the mail associated with a particular license will be sent (if different from the Main or License Location addresses).
License Location Address - This is the address where the place of business is physically located.

Contact Us :: 1940 North Monroe Street, Tallahassee FL 32399 :: Call.Center@dbpr.state.fl.us :: Customer Contact Center: 850.487.1395

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 18-7S-17-10021-009 **Building permit No.** 000029204

Use Classification SFD, UTILITY **Fire:** 0.00

Permit Holder SCOTT ROSENBOOM **Waste:**

Owner of Building CLAUDIO & GRACIEF CONDE **Total:** 0.00

Location: 245 SW WAFFLE GLN, FORT WHITE, FL 32038

Date: 07/29/2011

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

DW Pest Control, Inc.

13618 NW 270th Ave.
Alachua, FL 32615
(386) 418-4387

29204

CERTIFICATE OF COMPLIANCE FOR TERMITE PROTECTION

(As required by Florida Building Code (FBC) 1816.1.7)

Address of treatment or lot/block of treatment: 245 SW Waffle Glen High Springs, FL 32643
Describe method of termite prevention treatment: Trench & Treat around structure

The building has received a complete treatment for the prevention of subterranean termites. Treatment is in accordance with rules and laws, established by the Florida Department of Agriculture and Consumer Services.

Authorized Signature

GEO-TECH, INC.

Engineering Consultants in Geotechnical • Environmental • Construction Materials Testing

29204

FIELD DENSITY WORKSHEET

CLIENT Rosenboom — DATE 2/28/11

PROJECT NO. _____

PROJECT NAME Condo Pds — PERMIT NO. 29204

EARTH CONTRACTOR 245 Site Work Inc — TESTED BY DH

❑ Standard Proctor

COMPACTION REQUIREMENT (%) 95 ❑ Modified Proctor — _____ FIELD CONTACT

TOTAL ON-SITE TIME _____ — MILES FROM OFFICE _____

❑ Limerock ❑ Subgrade ❑ Pipe Backfill ❑ Building Pad ❑ Building Footing ❑ Other _____

TEST LOCATION	LAB PROCTOR DENS.	LAB PROCTOR OMC	TEST DEPTH	PROBE DEPTH	% MOIST.	WET DENSITY (PCF)	DRY DENSITY (PCF)	% COMP.
S. Side of Pad	107.8	10.6	FG	12"	4.9	109.3	103.1	95.6
	↓	↓	↓	↓				
N. Side of Pad					7.0	112.6	105.2	97.6
							~~104.2~~	
N. Bldg.					6.4	110.9	104.2	96.7
					1			

REMARKS _____

* Density failed to meet minimum project requirement

** Retest indicates minimum density requirement was obtained.

( ) Client is aware of unsatisfactory test results.

1016 S.E. 3RD AVENUE • OCALA, FLORIDA 34471 • (352) 694-7711