

GENERAL NOTES:

FABRICATION SHALL BE IN ACCORDANCE WITH ASI STANDARD PRACTICES AND IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1 AND D1.3"

MATERIALS	ASTM DESIGNATION	MIN YIELD STRENGTH
Hot Rolled Steel Shupe	A572	Fy=50 KSI
Steel pipes	A500	Fy=42 KSI
Structural Tubing	A500	Fy=46 KSI
Structural Steel Web Plate	A572/A1011	Fy=50 KSI
Structural Steel Flange Plate	A529/A572	Fy=55 KSI
Cold Form Light Gage	A653/A1011	Fy=50/55 KSI
Roof and Wall Sheets	A792/A653	Fy=50/80 KSI
Cable Broce	A475-Type 1	Extra HighSt
Rod Broce	A36	Fy=36 KSI
Mill Section	A50	Fy=50 KSI
Machine Bolt & Nuts	A327	Min Tensile Strength
High Strength Bolts(1" Dia Or Less)	A325-Type 1	Fu=60 KSI
High Strength Bolts(>1" to 1 1/2" Dia)	A325-Type-1	Fu=120 KSI
		Fu=105 KSI

PRIMER

Shop Primer Point is rust inhibitive primer,which meets the end performance of Federal specification TT-P-636 and is SFR Red oxide color. This point is not intended for long-term exposure to the elements. ASI is not responsible for any deterioration of the shop point as a result of improper handling and/or storage. ASI shall not be responsible for any field applied point and/or coatings.(Section 6.5 AISC Code of Standard Practice 9th Edition). Normal thickness of primer shall be 1 mil unless otherwise specified in contract documents.

A325 BOLT TIGHTENING REQUIREMENTS:

All High strength bolts are A325-N Unless noted otherwise.

Structural bolts shall be tightened by the turn-of-the-nut method in accordance with the 9th Edition AISC "Specification for Structural Joints" using ASTM A325 or A490 Bolts. When Specifically required, A325-N bolts are Supplied without washer unless noted on the drawings as provided by ADEL STEEL,INC.

All bolted connections unless noted are designed as bearing type connections with threads not excluded from the shear plane.

ERECTION NOTE:

All bracing,stripping shown and provided by ADEL STEEL,INC for this building is required and shall be installed by the erector as a permanent part of the structure. If additional bracing required for stability during erection of building, it shall be the Erectors responsibility to determine the amount of such bracing and to produce and install as needed.

ADEL STEEL,INC is not responsible for Unloading and Erection of Building.

SHORTAGES:

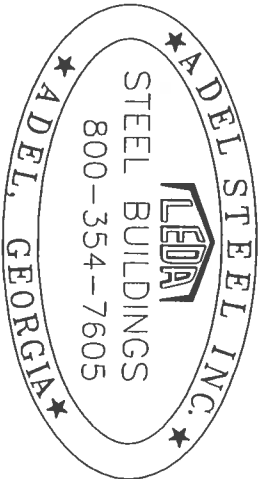
Any Claims or shortages by buyer must be reported to Customer Service Department within 48 Hours of Delivery or such claims shall be waived by the customer and disallowed.

CORRECTION OF ERRORS AND REPAIR 6.10):

Claims for correction of alleged misfit will be disallowed unless ASI shall have received prior notice thereof and allowed reasonable inspection of such misfits. The correction of minor misfits by the use of drift pins to draw the components into line amounts of reaming,chiping and cutting and the replacement of minor shortages of material are normal part of erection and are not subject to claim. No part of the building may be returned for alleged misfit without the prior approval of Adel steel,inc.

WARNING:

In no cose should Galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the Galvalume alloy coating when they are in contact with Galvalume steel panels. Even run-off from copper flashing,wiring to tubing onto Galvalume should be avoided. This also includes any metal shovings on Roof Panels must be swept clean to avoid corrosion



"The 1st Choice in Steel Buildings"

ADEL STEEL, INC.

601 SOUTH ELM STREET ----- ADEL, GEORGIA 31620
PHONE:(229)896-2263 ----- FAX:(229)896-4658

PURCHASER: Williams Dannocker

PROJECT:

JOB NUMBER: 6002

BUILDING LOADS/DESCRIPTION

WIDTH: 90 LENGTH: 70 HEIGHT: 16.25 / 12
(BUILDING DIMENSIONS ARE NOMINAL REFER TO PLAN)
BUILDING CODE: FBC 04
ROOF DEAD LOAD: 2 PSF
ROOF LIVE LOAD: 20 PSF
COLLATERAL LOAD: 3 PSF
ROOF SNOW LOAD: 0 PSF
WIND SPEED: 120 MPH
WIND EXPOSURE: B
WIND IMP FACT: 1.00
SEISMIC COEFF: 0.220
SIEMIC IMP FACT: 1.00
OTHER LOADS:

THE CONTRACTOR IS TO CONFIRM THAT THESE LOADS COMPLY WITH THE REQUIREMNTS OF THE LOCAL BUILDING DEPARTMENT.

ROOF/WALL PANEL DETAIL

	ROOF	WALL
PANEL TYPE:	PBR	PBR
GAGE:	26	26
COLOR:	Galvalume	NEED COLOR
INSULATION:	3	3
TYPE:	VR	VR

TRIM COLORS:

BASE TRIM	: NEED COLOR
CORNER TRIM	: NEED COLOR
RAKE TRIM	: NEED COLOR
EAVE/GUTTER	: NEED COLOR
DOWNSPOUTS	: NEED COLOR
FRAMED OPEN	: NEED COLOR

BUILDER/CONTRACTOR RESPONSIBILITY

It is the Responsibility of the Builder/Contractor to insure that all project plans,specifications and loads comply with the applicable requirements of any governing building authorities. The supplying sealed Engineering dato & drawings for Metal building system does not imply or constitute an agreement that the ADEL Steel or its Design engineer is acting as the Engineer of Record or design professional for o construction project.

The contractor must secure all required approvals and permit from the appropriate agency as required. Approval of the ADEL STEEL,INC drawings and calculations indicate that the ADEL STEEL,INC. correctly interpreted and applied the requirements of the contract drawings and specifications.(Sect.4.2.1 AISC code of standard practices,9th Edition).

Where discrepancies exist between, the Manufacturer's structural steel plans and the plans for other trades, the structural steel plan shall govern.(Sect.3.3 AISC Code of standard practice 9th Edition.) Design Considerations of any materials in the structure which are not furnished by the Building manufacturer are the responsibility of the contractors engineers other than the building manufacturer's engineers unless specifically indicated.

SAFETY COMMITMENT:

The building manufacturer has a commitment to manufacture quality building components that can be safely erected. However,the safety commitment and job practices of the erector are beyond the control of the building manufacturer. It is strongly recommended that safe working conditions and accident prevention practices be the top priority of any job site. Local,state and federal safety and health standards should always be followed to help insure worker safety. Make certain all employees know the safest and most productive way of erecting o building.Emergency procedures should be known to all employees. Daily meetings highlighting safety procedures are also recommended. The use of hard hats,rubber sole shoes for roof work,proper equipment for handling material, and safety nets where applicable, are recommended.

SPECIAL NOTE:

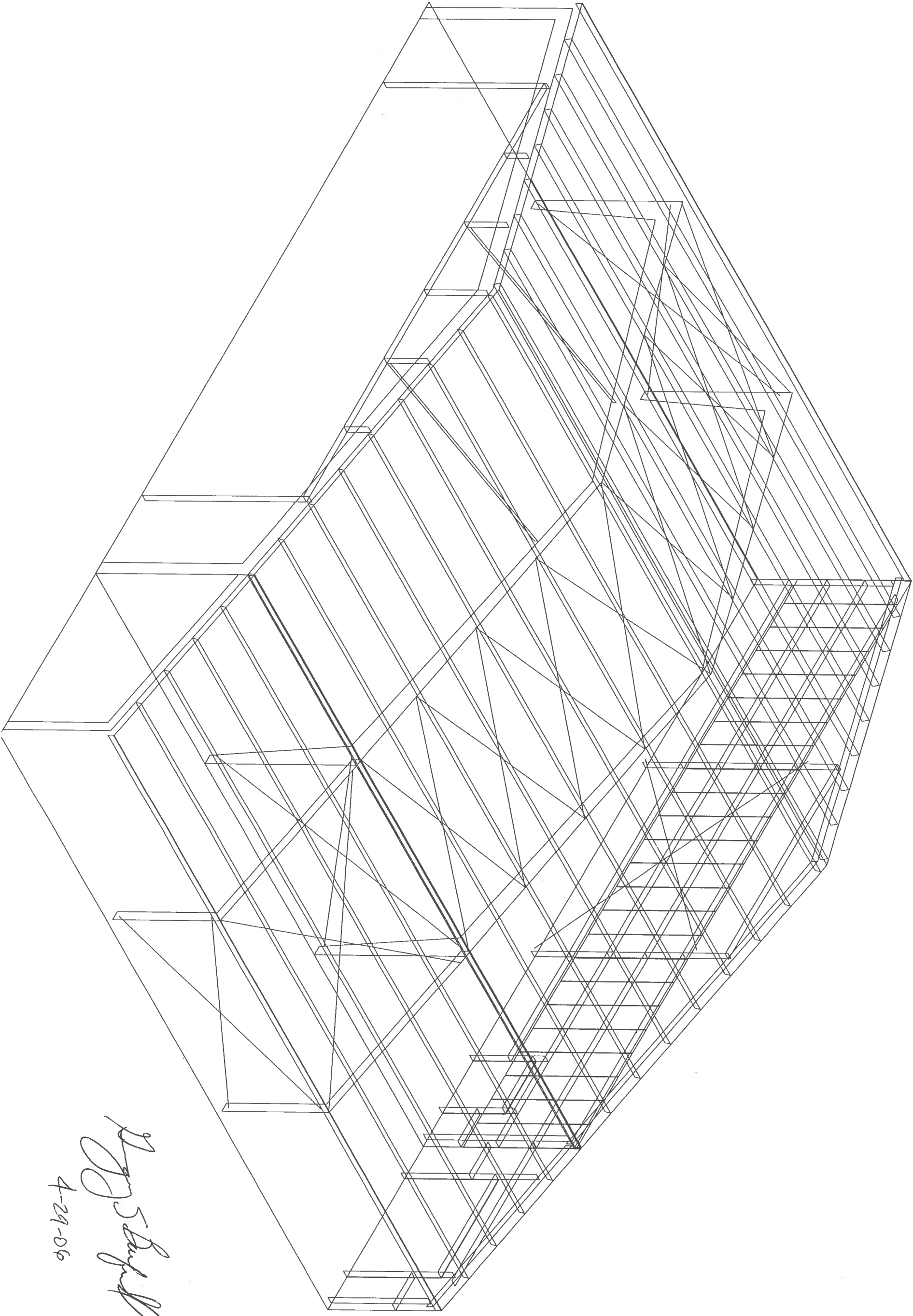
- (1) ADEL STEEL RESERVES THE RIGHT TO SPLICE OR MODIFY PARTS FOR BEST UTILIZATION OF MATERIALS.
- (2) FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF ADEL STEEL,INC.
- (3) ADEL STEEL,INC DOES NOT PROVIDE ANCHOR BOLT LENGTH, CONSULT YOUR FOUNDATION ENGINEER FOR ANCHOR BOLT LENGTH.
- (4) ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLERANCE OF +/-1/8" IN BOTH ELEVATION AND LOCATION.

1 Approval Changes 4/20/2006

DRAWING NOTE

Field locate 1--each 3070 walkdoor W/Std hardware.
Factory locate 1--each 12'x10' Rolldoor W/Std Hardware.
Field locate 1--each 6'-5"x6'-8 1/2" F/O For Door
By others.
Factory locate 1--each 50'x14' F/O For Bi-fold Door
By others.

GREGORY S. BARRFIELD, P.E.
2149 NELL PURVIS ROAD
ADEL, GA 31620
P E # 54419



Gregory S. Barfield

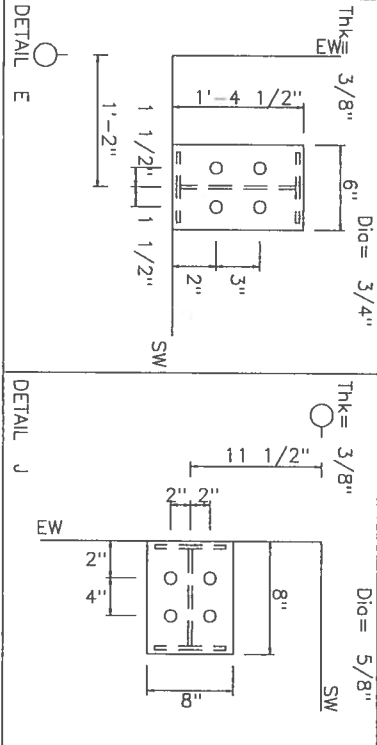
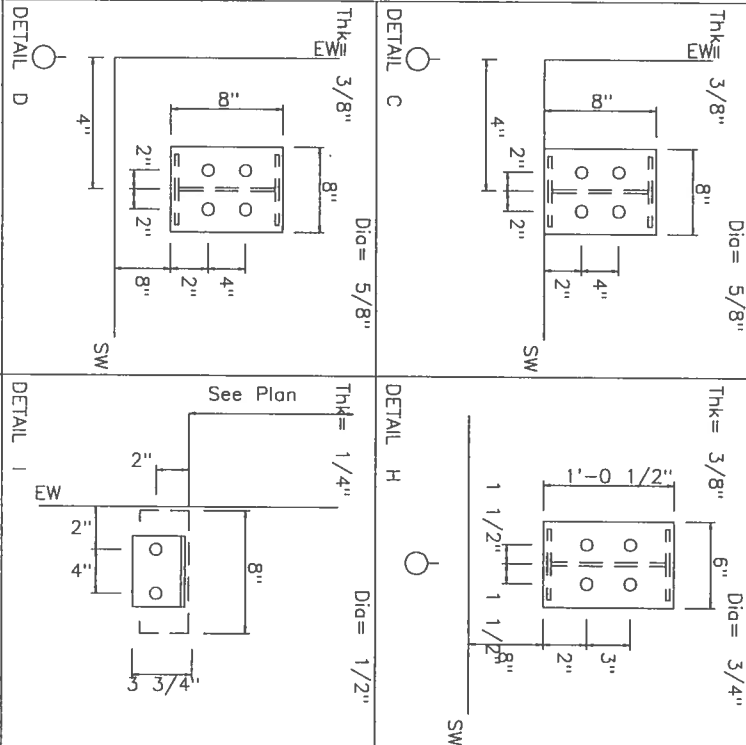
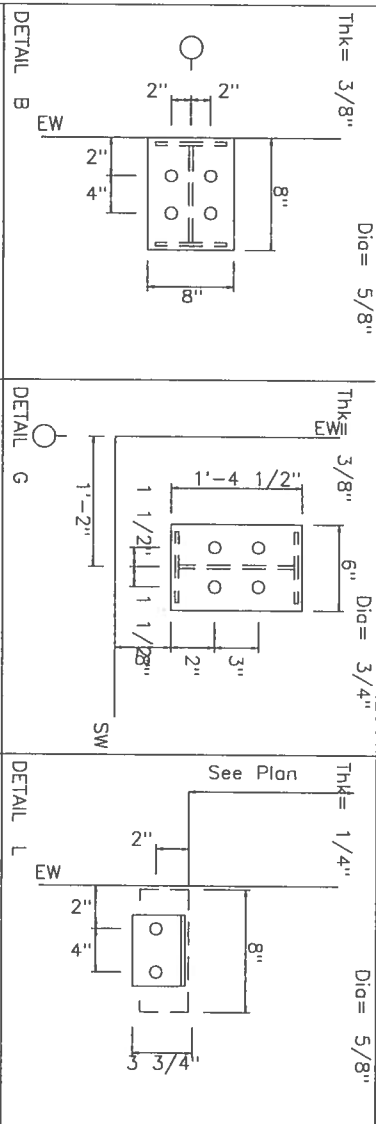
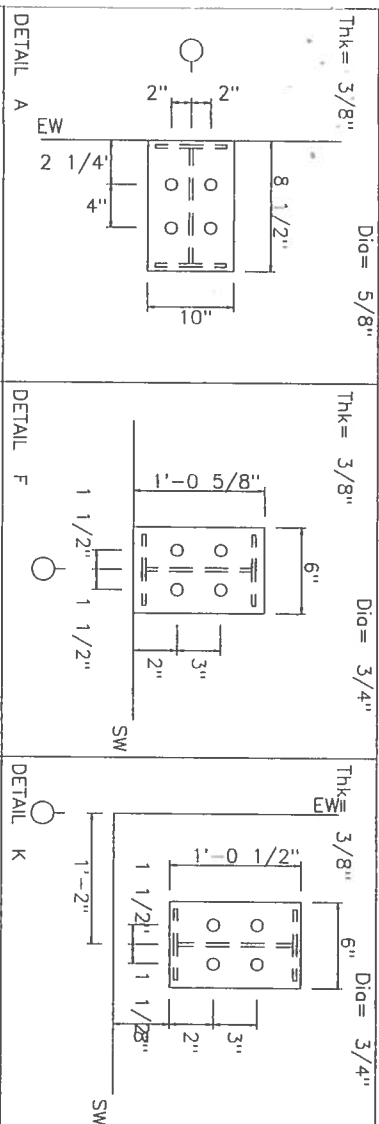
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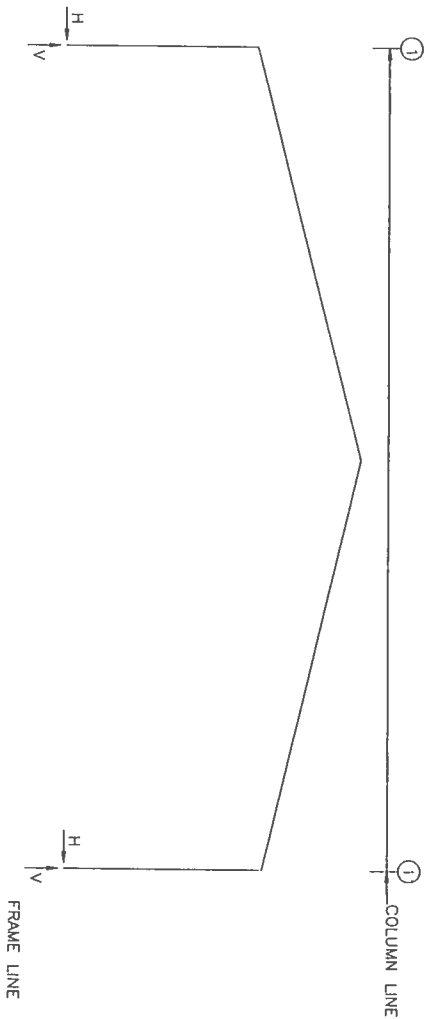
NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft)	=	70.0
Length (ft)	=	70.0
Eave Height (ft)	=	16.3/5.7
Roof Slope (rise, 12)	=	3.0/2.68
Dead Load (psf)	=	2.0
Collected Load (psf)	=	2.0
Roof Live Load (psf)	=	20.0
Frame Live Load (psf)	=	12.0
Wind Speed (mph)	=	120.0
Wind Code	=	FCB
Exposure	=	04
Closed/Open	=	C
Importance	=	1.00
Importance - Seismic	=	1.00
Seismic Zone	=	1
Seismic Coeff (F _o -S _s)	=	0.22

ANCHOR BOLT SUMMARY

Ont	Loc	Dig (in)	Total Len (in)	Proi (in)
4	DJ	1/2"	5.00	0.50
24	EW	5/8"		1.50
24	RF	3/4"		2.50



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Col Line Line	Load Id	Column Reactions (k)			Anc. Bolt No D(in)	Base Plate W/d Len	Groat (in)					
		Hmax V Vmax	Hmin V Vmin									
A 1	1	6.0	10.6	2	-4.2	-5.6	4	0.750	6.000	16.50	0.375	0.0
A 11	3	-2.7	-6.9	1	-5.9	13.5	4	0.750	6.000	16.50	0.375	0.0

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Firm Col Line Line	Load Id	Column Reactions (k)				Arc. Bolt No D(in)	Base Plate Wid Len	Plate (in) Thk	Grout (in)		
		Hmax H	V Vmax	Hmin H	Vmin V						
B * 1	1	9.6	15.2	2	-11.6	4	0.750	6.000	12.50	0.375	0.0
B * 11	3	8.9	-17.2	1	-4.3	4	0.750	6.000	12.63	0.375	0.0
	1	-9.5	19.4	5	-1.2						
Frame lines: B C											

RIGID FRAME:

BASIC COLUMN REACTIONS (k)

Frame		Column		Line		Wind		R2		Seismic		R1		Wind		L2		Unst	
B	1	Horiz	Deo	Collateral	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
		2.67	3.89	0.76	1.36	2.93	5.29	5.36	8.32	2.35	6.47	4.40	4.72						
		1.55	1.72	0.65	1.42	2.06	5.29	1.15	8.01	3.48	9.87	0.49	3.79						
		1.53	3.31	1.63	3.18	6.33	3.66	1.358	13.79	3.41	12.05	8.79	8.78						
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		1.55	1.72	0.65	1.42	2.06	5.29	1.15	8.01	3.48	9.87	0.49	3.79						
		1.53	3.31	1.63	3.18	6.33	3.66	1.358	13.79	3.41	12.05	8.79	8.78						
B	1	Horiz	Deo	Collateral	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
		2.67	3.89	0.76	1.36	2.93	5.29	5.36	8.32	2.35	6.47	4.40	4.72						
		1.55	1.72	0.65	1.42	2.06	5.29	1.15	8.01	3.48	9.87	0.49	3.79						
		1.53	3.31	1.63	3.18	6.33	3.66	1.358	13.79	3.41	12.05	8.79	8.78						
B	1	Horiz	Deo	Collateral	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
		2.67	3.89	0.76	1.36	2.93	5.29	5.36	8.32	2.35	6.47	4.40	4.72						
		1.55	1.72	0.65	1.42	2.06	5.29	1.15	8.01	3.48	9.87	0.49	3.79						
		1.53	3.31	1.63	3.18	6.33	3.66	1.358	13.79	3.41	12.05	8.79	8.78						
B	1	Horiz	Deo	Collateral	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
		2.67	3.89	0.76	1.36	2.93	5.29	5.36	8.32	2.35	6.47	4.40	4.72						
		1.55	1.72	0.65	1.42	2.06	5.29	1.15	8.01	3.48	9.87	0.49	3.79						
		1.53	3.31	1.63	3.18	6.33	3.66	1.358	13.79	3.41	12.05	8.79	8.78						
B	1	Horiz	Deo	Collateral	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
		2.67	3.89	0.76	1.36	2.93	5.29	5.36	8.32	2.35									

ENDWALL COLUMNS REACTIONS (k)

MAXIMUM DOWN	= 8.1
MAXIMUM UP	= ~10.7
MAXIMUM HORIZONTAL	= 7.9

BRACING REACTIONS, PANEL SHEAR

--Wall--		± Reactions (k)		Panel Shear (lb/ft)
Loc	Line	--Wind--	--Seismic--	
		Horz	Vert	Horz
EW	1	Rigid	At Endwall	
SW	A	7.5	4.7	1.1
EW	D	2.8	2.7	0.4
SW	1	6.4	3.8	0.5

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P E # 54419

PE# 54419



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Anchor Bolt Layout

DESIGN CRITERIA	
FBC 04	BUILDING CODE
12 PSF	LIVE LOAD TO FRAMES
20 PSF	LIVE LOAD TO PURLINS
120 MPH	WIND LOAD
PSF	SNOW LOAD
3 PSF	COLLATERAL LOAD
1.00	IMPORTANCE USE FACTOR
B	EXPOSURE
2.0	SOIL COEFFICIENT
0.220	A_v
0.00	A_s

TAGNA CONSTRUCTION, INC.

CITY FL,32055

6002	4/20/06
Scale N.T.S.	Contact
Drawing Number	

6002 AB2

4-29-06

4-29-06

ANCHOR BOLT SUMMARY

Qnt	Loc	Dia (in)	Proj (in)
0 4	DJ	5/8"	1.50
0 4	EW	5/8"	1.50
0 12	RF	3/4"	2.50

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft) = 20.0
Length (ft) = 70.0
Eave Height (ft) = 12.0/16.7
Roof Slope (rise/12) = 2.8
Dead Load (psf) = 2.0
Collateral Load (psf) = 3.0
Roof Live Load (psf) = 20.0
Frame Live Load (psf) = 14.7
Wind Speed (mph) = 120.0
Wind Code = FBC 04
Exposure = B
Closed/Open = C
Importance = 1.00
Importance - Wind = 1.00
Seismic Zone = B
Seismic Coeff (Fo*Fs) = 0.22

Id	Description
1	DL+CL+LL
2	DL+CL+1.30WR1
3	DL+CL+1.30LWnDR
4	DL+CL+1.30LWnDR
5	1.02DL+1.02CL+SeisL
6	DL+CL+1.30WR1+1.30WS



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frame Col Line	Load Id	Hmax H	V Vmax	Column Reactions (k)	Id	H Hmin	V Vmin	Anchor Bolt No D(in)	Base Plate (in)	Wid	Len	Thk	GROUT (in)
C * 13	1	4	2.2	-5.1	3	-0.1	-4.9	4	0.750	6.000	12.50	0.375	0.0
C * 13	1	0.1	5.2	5.2	3	2.2	-6.0						

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frame Col Line	Load Id	Hmax H	V Vmax	Column Reactions (k)	Id	H Hmin	V Vmin	Anchor Bolt No D(in)	Base Plate (in)	Wid	Len	Thk	GROUT (in)
A 13	4	1.2	-3.0	2	-0.1	-2.6	4	0.750	6.000	12.50	0.375	0.0	
A 13	1	0.1	3.6	4	1.2	-3.0							

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Column Line	Dead Horiz Vert	Collateral Horiz Vert	Live Horiz Vert	Wind L1 Horiz Vert	Wind R1 Horiz Vert	Wind L2 Horiz Vert
A 13	0.01	0.02	0.07	-0.08	-2.79	-0.04
C * 13	0.02	0.02	0.10	-0.14	-4.97	-0.08
Frame Column Line	Wind R2 Horiz Vert	Seismic L1 Horiz Vert	Seismic R1 Horiz Vert	LnWind L1 Horiz Vert	LnWind R1 Horiz Vert	LnSeis Horiz Vert
A 13	-0.04	-1.64	0.00	0.92	3.87	0.00
C * 13	-0.08	-2.91	0.00	1.63	-3.84	0.00
C * 13						-0.13

ENDWALL COLUMNS REACTIONS (k)

MAXIMUM DOWN = 3.2
MAXIMUM UP = -2.9
MAXIMUM HORIZONTAL = 0.0

BRACING REACTIONS, PANEL SHEAR

Loc Line	Col Line	± Reactions (k)	Panel Shear (lb/ft)
LEW D	Brocing Not Used		507
F-SW 12	Rigid Frame At Endwall		0
B-SW 13		0.9	0.3

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ADEL, GA 31620
P E # 54419



DESIGN CRITERIA
FBC 04 BUILDING CODE
18.7/23.7 L&DE LOAD TO FRAMES
20 PSF LINE LOAD TO PURLINS
0 PSF SNOW LOAD
3 PSF COLLATERAL LOAD
1.00 IMPORTANCE USE FACTOR
2.0 SOIL COEFFICIENT
0.00 A5

Anchor Bolt Layout

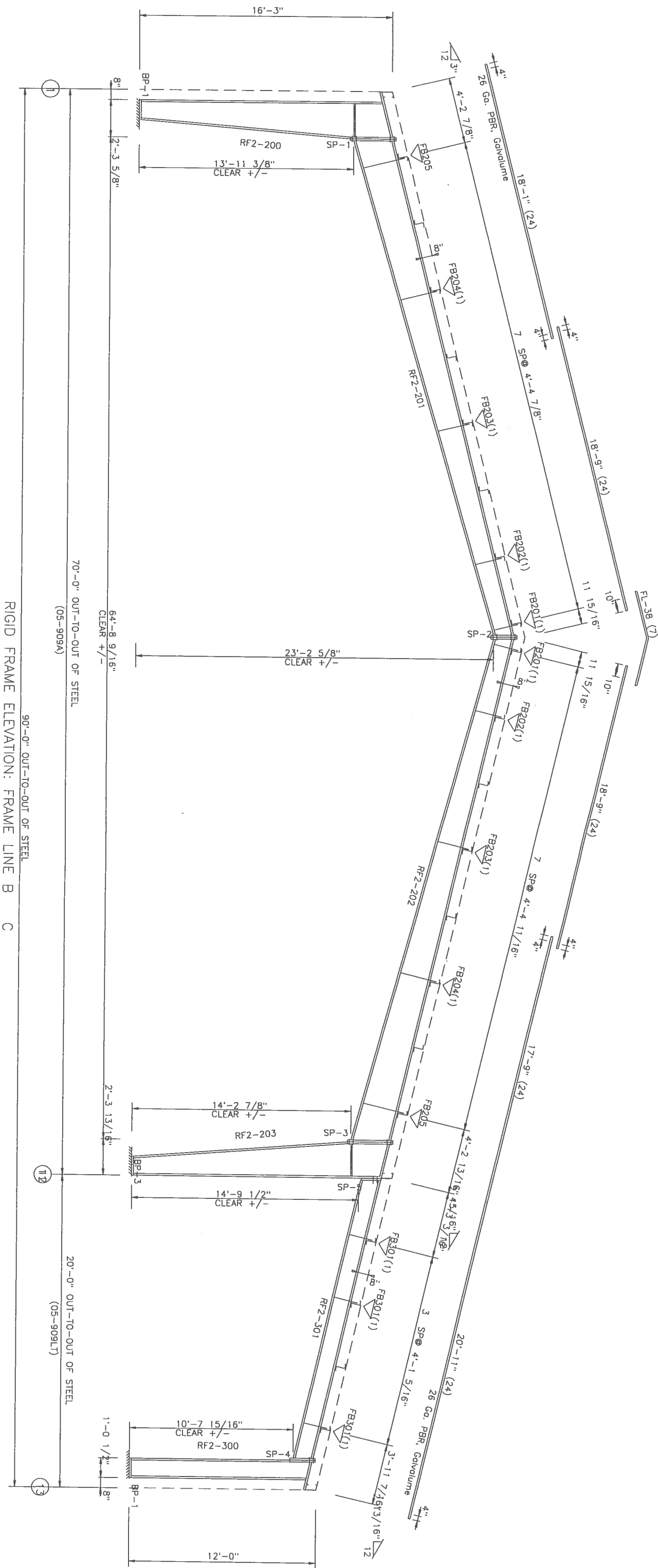
CONSTAGNA CONSTRUCTION, INC.
521 NW OLD MILL ROAD
LAKE CITY FL 32055
386-755-6867

Project	Date
6002	4/20/06
Issued	Revised
N.I.S.	
Drawing Number	
6002	AB3

SPURCE BOLTS			
Splice Mark	Quon Top/Bol/Int Type	Bolt Dia	Len
SP-1	4	0	A325 3/4" 2"
SP-2	4	0	A325 3/4" 2"
SP-3	4	0	A325 3/4" 2 1/4"
SP-4	4	0	A325 3/4" 2"
SP-5	4	0	A325 3/4" 2"

FLANGE BRACES: Both Sides (U.N.)
FBxA(1)
A - L2X2X1/8

BASE PLATES		
Col Id	Plate Size	Wld Thick Length
BP-1	6" 3/8"	1'-0 1/2"
BP-2	6" 3/8"	1'-0 5/8"
BP-3	6" 3/8"	1'-0 5/8"
BP-4	"	1'-0"



MEMBER SIZE TABLE (in)			
PIECE	WEIGHT	WEB DEPTH START/END	WEB PLATE THICK LENGTH
RF2-200	416	12.0/17.5	0.134 60.0
RF2-201	713	12.0/17.5	0.134 134.8
RF2-202	732	12.0/17.5	0.134 101.9
RF2-203	479	12.0/17.5	0.134 238.0
RF2-300	186	12.0/12.0	0.134 62.0
RF2-301	298	12.0/12.0	0.134 238.0

Handwritten signature and date:
4-29-04

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P E # 54419

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E-MAIL: STEEL@ADELSTEEL.COM

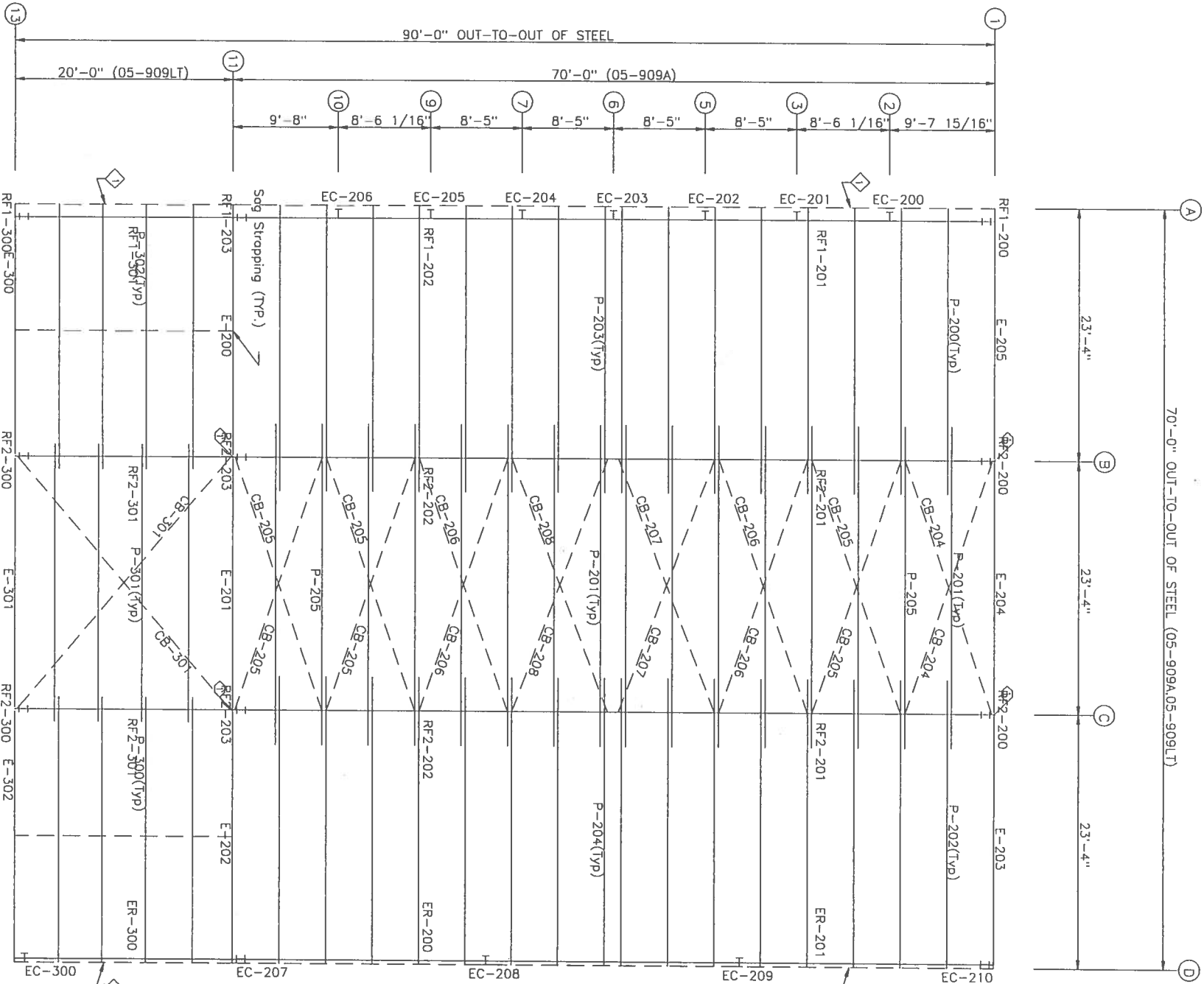
DESIGN CRITERIA

ASCE 04 BUILDING CODE
18.7699' MADE LOAD TO FRAMES
20 PSF LIVE LOAD TO PURVIS
0 PSF WIND LOAD
3 PSF COLLATERAL LOAD
1.00 IMPORTANCE USE FACTOR
B EXPOSURE
2 SOIL COEFFICIENT
0.00 A_s

Rigid Frame Cross Section

CONSTRAGNA CONSTRUCTION, INC.
521 NW OLD MILL ROAD
LAKE CITY FL 32055
386-755-6867

Project: 6002 Date: 4/20/06
Name: N.T.S. Checked:
Drawing Number: 6002 XS2



PURLIN
LAP (05-909A)
PURLIN
LAP (05-909LT)

3'-1 3/4"
1'-1 3/4"
1'-1 3/4"

3'-1 3/4"
3'-1 3/4"
1'-1 3/4"

ROOF FRAMING PLAN

MEMBER TABLE			
ROOF PLAN			
QUAN	MARK	PART	LENGTH
8	05-909A	P-200 BX25x14	26'-5 1/2"
14		P-201 BX25x16	29'-7 1/2"
8		P-202 BX25x14	26'-5 1/2"
8		P-203 BX25x14	26'-5 1/2"
2		P-204 BX25x14	26'-5 1/2"
2		P-205 BX25x14	26'-5 1/2"
1		E-200 BE14	23'-3 1/2"
1		E-201 BE14	23'-3 1/2"
1		E-202 BE14	23'-3 1/2"
1		E-203 BE14	23'-3 1/2"
1		E-204 BE14	23'-3 1/2"
1		E-205 BE14	23'-3 1/2"
2		CB-204 3/4" ROD	25'-1"
2		CB-205 3/4" ROD	25'-3"
2		CB-206 1/2" ROD	25'-3"
2		CB-207 1/2" ROD	25'-7"
2		CB-208 1/2" ROD	25'-6"
4		P-300 BX35x14	24'-5 1/2"
4		P-301 BX35x14	25'-7 1/2"
4		P-302 BX35x14	24'-5 1/2"
1		E-300 BE14	23'-3 1/2"
1		E-301 BE14	23'-3 1/2"
1		E-302 BE14	23'-3 1/2"
2		CB-301 1/4" CBL	27'-1 1/2"

SPECIAL BOLTS			
QUAN	TYPE	DIA	LENGTH WASH
1	4	A325	1 1/2" 1 1/4" 1

ANGLE TABLE			
ROOF PLAN			
QUAN	MARK	LENGTH	
1	RA1	20'-0"	

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4-29-06

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P E#54419

Roof Framing

CONSTRACON CONSTRUCTION, INC.
521 NW OLD MILL ROAD
LAKE CITY, FL 32055
386-755-6867

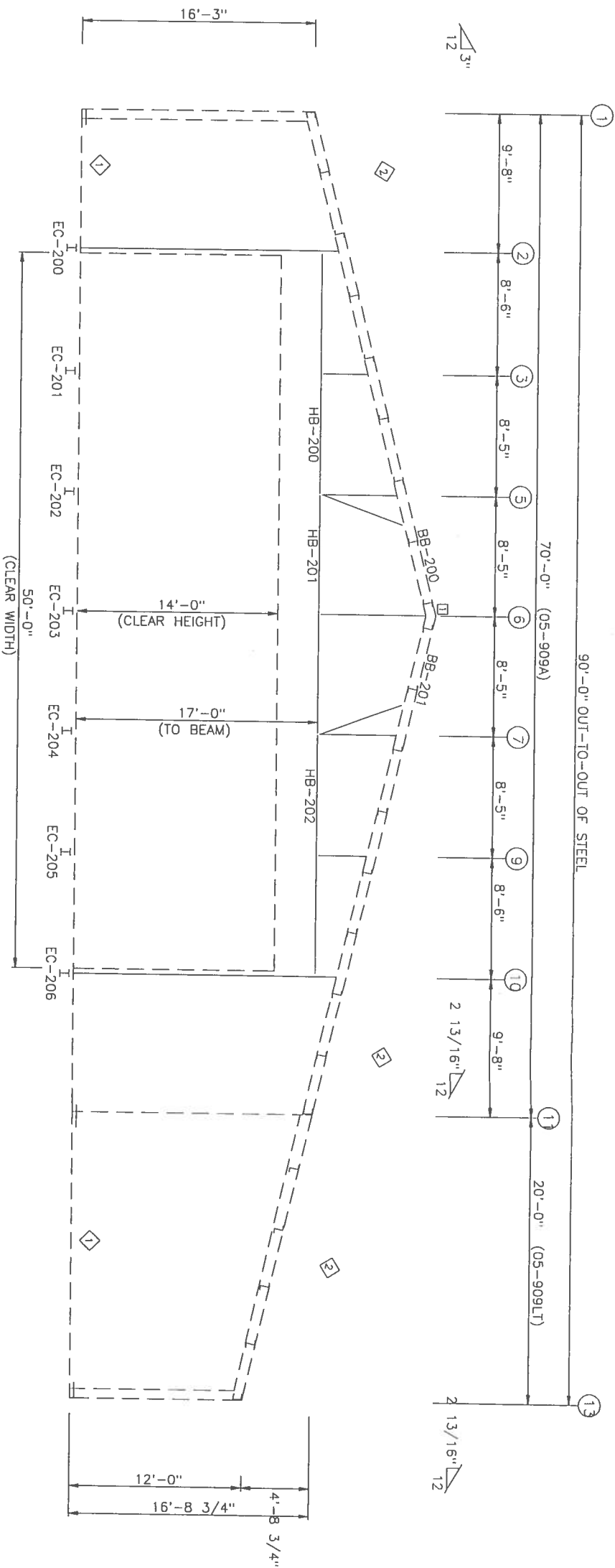
6002 FR1



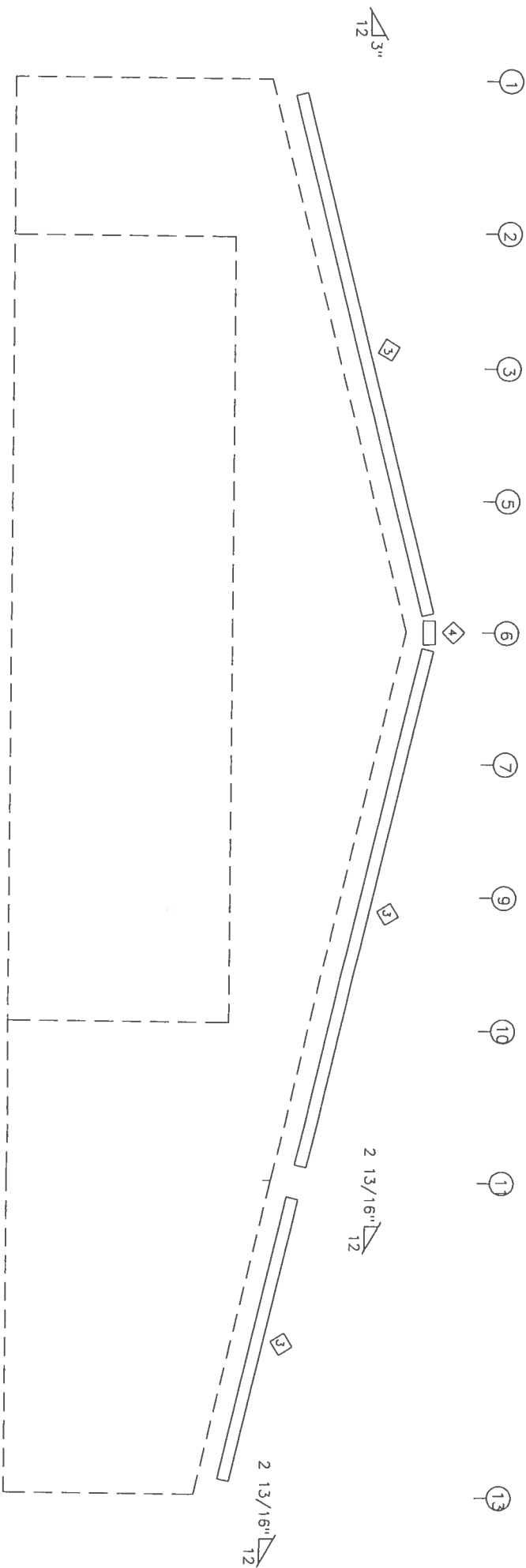
LEDA
ADEL STEEL, INC.
601 S. ELM STREET ADEL, GEORGIA 31620
229-896-2263 229-896-4658
HTTP://WWW.ADELSTEEL.COM/
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DESIGN CRITERIA	
FBC 04	BUILDING CODE
12 PSF	LIVE LOAD TO FRAMES
20 PSF	LIVE LOAD TO PURLINS
0	WIND LOAD
3 PSF	COLLATERAL LOAD
1.00	IMPORTANCE USE FACTOR
20	EXPOSURE
0.220	WIND COEFFICIENT
As	

90'-0" OUT-TO-OUT OF STEEL



ENDWALL FRAMING: FRAME LINE A



PANELS: 26 Ga. PBR
NEED COLOR

BOLT TABLE			
FRAME LINE A		QUAN	TYPE DIA LENGTH
LOCATION			
05-909A	Int Column/Ref	2	A325 1/2" 1 1/4"
05-909A	Int Column/Ref	8	A325 1/2" 1 1/4"

MEMBER TABLE		
FRAME LINE A	PART	LENGTH
05-909A		

HB-200	WBX18	16'-6 5/8"
HB-201	WBX18	16'-9 11/16"
HB-202	WBX18	16'-6 5/8"
BB-200	P4X14.98	22'-8 13/16"
BB-201	P4X14.98	22'-9 1/16"
EC-200	WBX40	17'-3 1/2"
EC-201	WBX24	2'-1 1/4"
EC-202	WBX24	4'-2 1/2"
EC-203	WBX24	6'-3 3/4"
EC-204	WBX24	4'-3 15/16"
EC-205	WBX24	2'-4 1/16"
EC-206	WBX40	17'-7 11/16"

TRIM TABLE	
FRAME LINE A	LENGTH
QID MARK	
3 FL-13D	20'-3"
4 FL-13C	1'-4"

ANGLE TABLE	
FRAME LINE A	LENGTH
QID MARK	
1 BA1	20'-0"
2 RA1	20'-0"

CONNECTION PLATES	
FRAME LINE A	LENGTH
QID MARK/PART	
1 n200	

ENDWALL TRIM: FRAME LINE A

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4-29-06
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ADEL, GA 31620
P E #54419



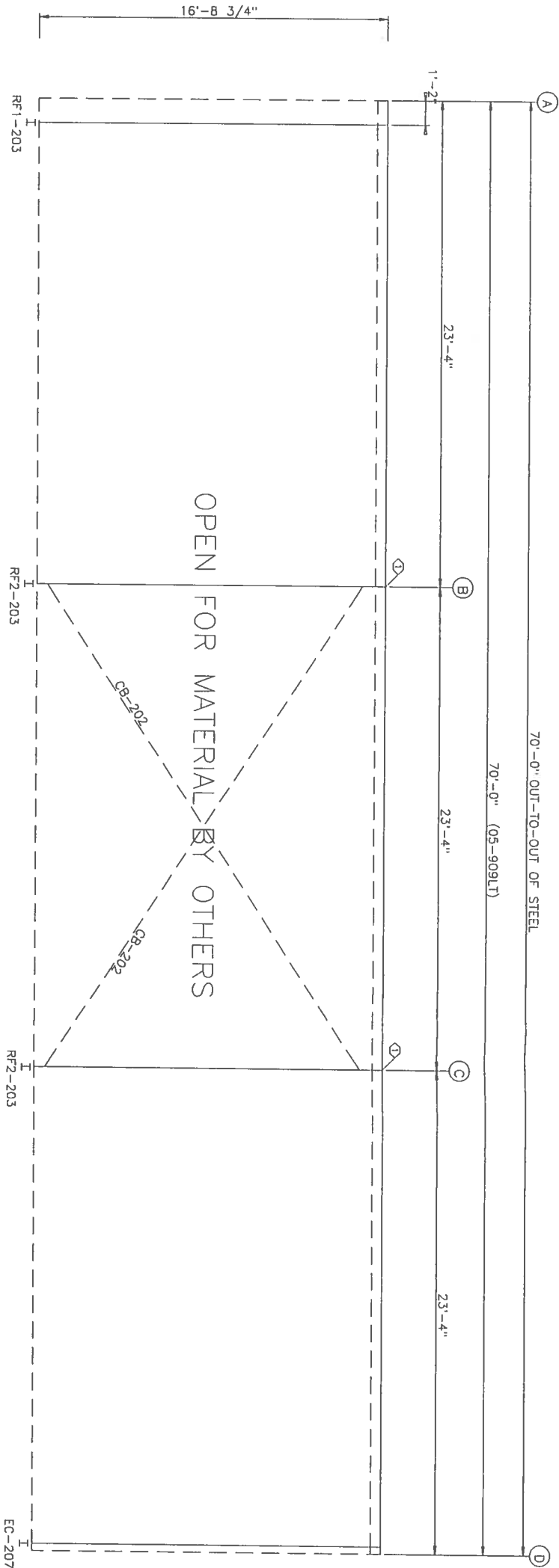
DESIGN CRITERIA	
FBC 04	BUILDING CODE
12 PSF	LIVE LOAD TO FRAMES
20 PSF	LIVE LOAD TO PURLINS
0 PSF	SNOW LOAD
3 PSF	COLLATERAL LOAD
1.00	IMPORTANCE USE FACTOR
2.0	WIND COEFFICIENT
0.220	SOIL COEFFICIENT
A _s	

Endwall Framing & Sheeting
CONSTAGNA CONSTRUCTION, INC.
521 NW OLD MILL ROAD
LAKE CITY FL 32055
386-755-6867
Project: 6002 Date: 4/20/06
Scale: N.T.S. Drawing Number: 6002 FR2

MEMBER TABLE			
FRAME LINE	12	PART	LENGTH
QUAN	MARK		
2	CB-202	7/8" ROD	28'-5"

SPECIAL BOLTS			
Q ID	QUAN	TYPE	DIA
1	4	A325	1/2"

ANGLE TABLE			
QID	MARK	LENGTH	WASH
1	BA1	20'-0"	1



A

B

C

D

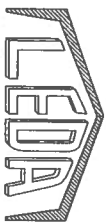
OPEN FOR MATERIAL BY OTHERS

SIDEWALL TRIM: FRAME LINE 11

Gregory S. Barfield

4-29-04

GREGORY S. BARFIELD, P.E.
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P E # 54419



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DESIGN CRITERIA	
IBC 04	BUILDING CODE
120	PSF LIVE LOAD TO FRAMES
20	PSF LIVE LOAD TO PURLINS
120	MPH WIND LOAD
0	PSF SNOW LOAD
1.00	PSF IMPORTANCE USE FACTOR
B	EXPOSURE
2.0	SOIL COEFFICIENT
0.200	A _s

Sidewall Framing & Trim

CONSTAGNA CONSTRUCTION, INC.

521 NW OLD MILL ROAD
LAKE CITY FL 32055
386-755-6867

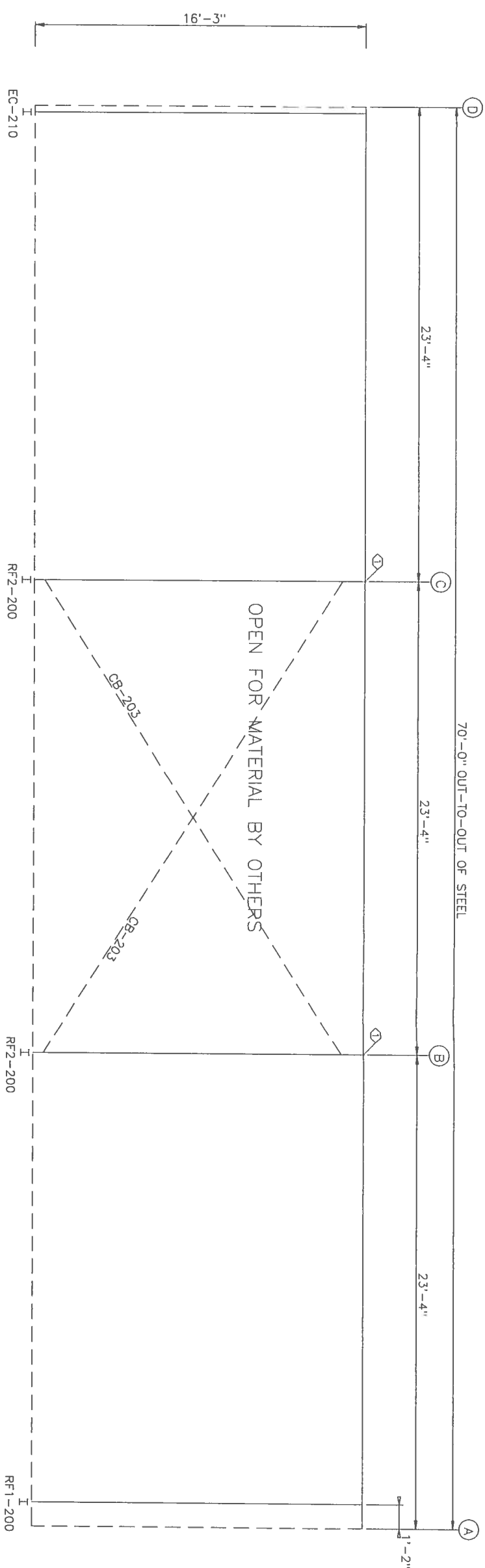
Project	Date
6002	4/20/06
Drawn	Checked
N.T.S.	
Drawing Number	
6002 FR4	

MEMBER TABLE			
FRAME LINE 1			
QUAN	MARK	PART	LENGTH
2	CB-203	3/4" ROD	28'-3"

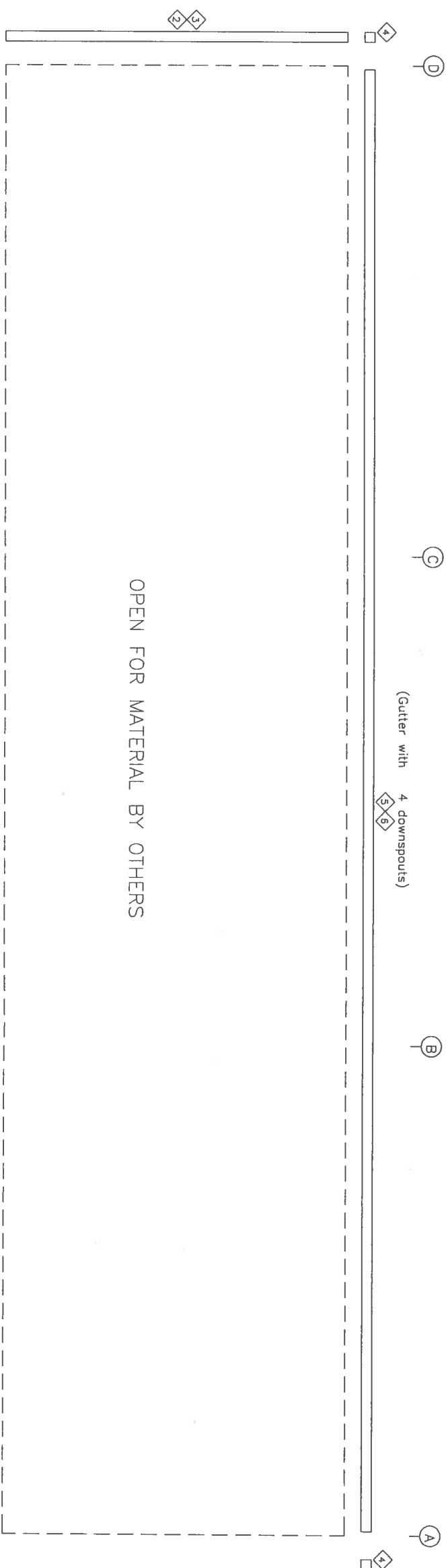
SPECIAL BOLTS			
Q	ID	QUAN	TYPE DIA LENGTH WASH
1	4	A325	1/2" 1 1/4" 1

ANGLE TABLE	
FRAME LINE 1	
OLD MARK	LENGTH
1 BA1	20'-0"

TRIM TABLE		
FRAME LINE 1		
QID	MARK	LENGTH
2	FL-22	7'-3"
3	FL-23	10'-2"
4	FL-14A	6"
5	FL-74B	10'-2"
6	FL-74C	20'-2"



SIDEWALL FRAMING: FRAME LINE 1



SIDEWALL TRIM: FRAME LINE 1

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2149 NEL PURVIS ROAD
ADEL, GA 31620
PE#54419



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DESIGN CRITERIA	
FRC 04	BUILDING CODE
12 PSF	LOAD TO FRAMES
20 PSF	LIVE LOAD TO PLAINS
120 MPH	WIND LOAD
0 PSF	SNOW LOAD
3 PSF	LOAD
1.00	IMPERMEANCE USE FACTOR
B	EXPOSURE
2.0	SOIL COEFFICIENT
0.220	
As	

CONSTAGNA CONSTRUCTION ,INC.

521 NW OLD MILL ROAD

LAKE CITY FL 32055

386-755-6867

Sidewall Framing & Trim

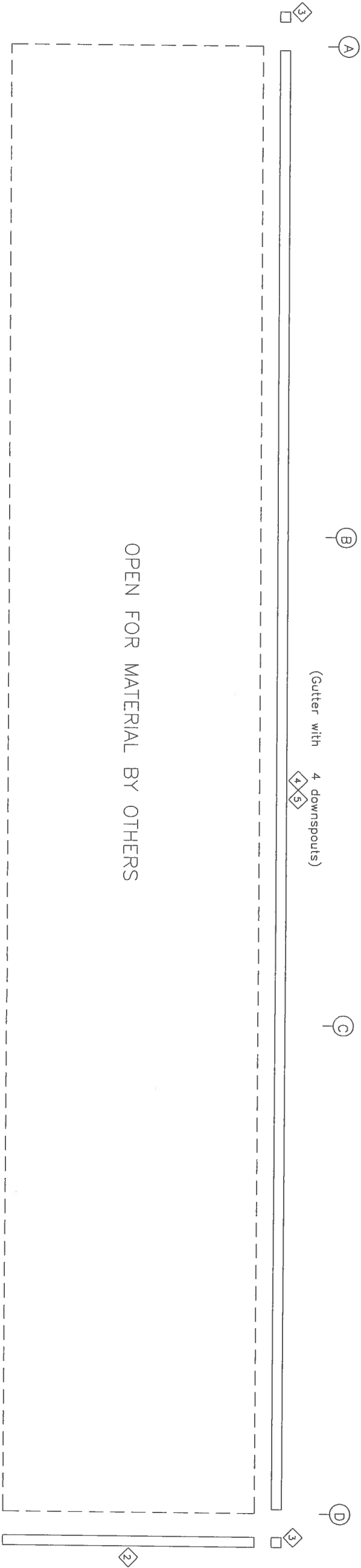
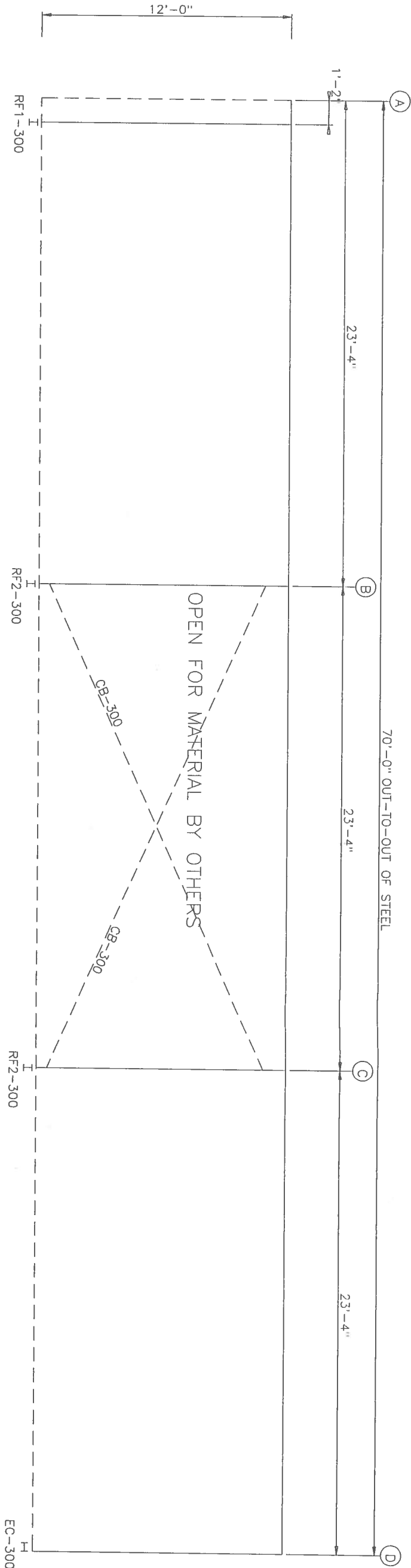
Project 6002 Scope N.T.S. Drawing Number	Issue 4/20/06 Comment
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6002 FR5

MEMBER TABLE		
FRAME LINE	PART	LENGTH
13		
QUAN	MARK	LENGTH
2	CB-300 1/4 CBL	24'-1"

TRIM TABLE	
Q/D MARK	LENGTH
2	FL-23B 12'-2"
3	FL-14A 6"
4	FL-74B 10'-2"
5	FL-74C 20'-2"

ANGLE TABLE	
FRAME LINE	LENGTH
13	
Q/D MARK	LENGTH
1	BAT 20'-0"



OPEN FOR MATERIAL BY OTHERS

SIDEWALL TRIM: FRAME LINE 13

Gregory S. Barfield
4-29-06

GREGORY S. BARFIELD, P.E.
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ADEL, GA 31620
P E # 54419



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DESIGN CRITERIA	
FSC OR	BUILDING CODE
120	PSF
20	PSF
120	PSF
0	PSF
1.00	PSF
B	2.0
0.220	0.220
0.220	0.220
A _s	A _s

Sidewall Framing & Trim

CONSTAGNA CONSTRUCTION, INC.
521 NW OLD MILL ROAD
LAKE CITY, FL 32055
386-755-6867

Project	Date
6002	4/20/06
Scale	Comments
N.T.S.	
Drawing Number	
6002 FR6	

Ⓕ Ⓖ Ⓖ Ⓐ

3"
12

2 13/16"
12

9'-0" (WI-200)	9'-0" (WI-200)
9'-0" (WI-202)	9'-0" (WI-202)
9'-0" (WI-202)	9'-0" (WI-202)
9'-0" (WI-202)	9'-0" (WI-202)
9'-0" (WI-202)	9'-0" (WI-202)
9'-0" (WI-202)	9'-0" (WI-202)
9'-0" (WI-202)	9'-0" (WI-202)
9'-0" (WI-202)	9'-0" (WI-202)
9'-0" (WI-200)	9'-0" (WI-200)

ENDWALL INSULATION: FRAME LINE 1

WALL INSULATION TABLE FRAME LINE 1 & 4		
MARK	WIDTH	LENGTH
WI-200	4'-0"	18'-0"
WI-201	4'-0"	18'-6"
WI-202	6'-0"	63'-0"
WI-203	6'-0"	106'-6"
WI-204	6'-0"	138'-0"

ROOF INSULATION TABLE FRAME LINE 1 & 4		
MARK	WIDTH	LENGTH
RI-200	4'-0"	74'-6"
RI-201	6'-0"	74'-6"

Ⓐ Ⓒ Ⓓ Ⓕ

2 13/16"
12

3"
12

18'-2 1/16" (WI-201)	19'-7 1/16" (WI-204)	21'-0 1/8" (WI-204)	22'-5 1/8" (WI-204)	23'-10 1/8" (WI-204)	25'-3 3/16" (WI-204)	25'-6" (WI-204)	24'-3" (WI-203)	22'-9" (WI-203)	21'-3" (WI-203)	19'-9" (WI-203)	18'-3" (WI-203)
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OPEN FOR WALL LITES

ENDWALL INSULATION: FRAME LINE 4

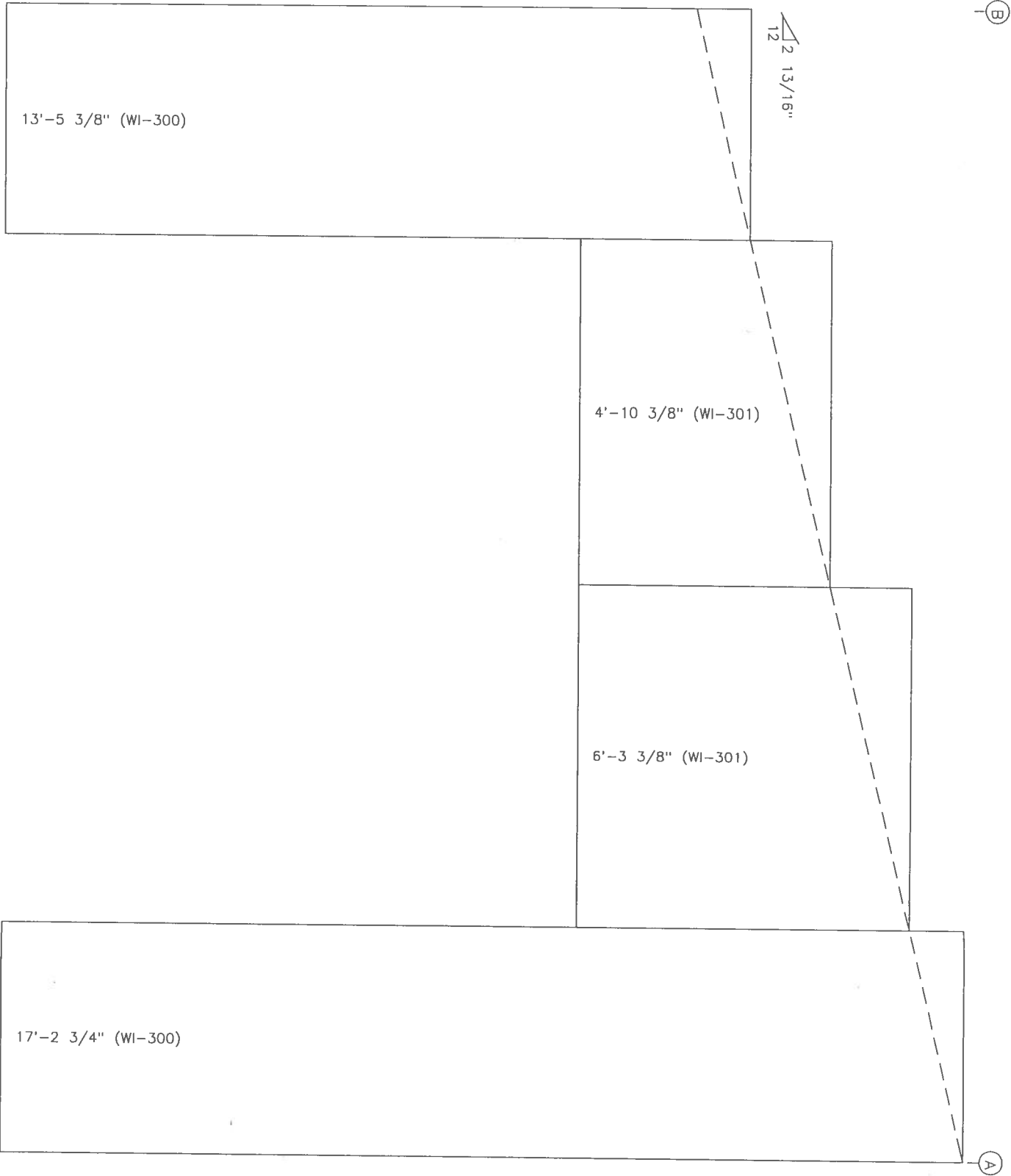
Gregory S. Barfield
4-29-06

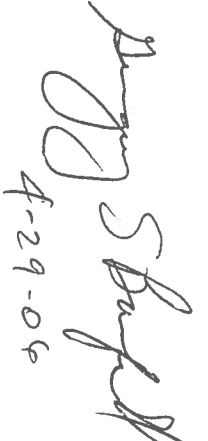
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2149 NELL PURVIS ROAD
ADEL, GA 31620
P.E.#54419

Adel Steel, Inc.	Constagna Construction
PROJECT	WALL INSULATION-1
ID	6002
PROJECT	DATE: 4/20/06
ADDRESS	SHEET
	OF

WALL INSULATION TABLE		
FRAME LINE 1		
MARK	WIDTH	LENGTH
WI-300	4'-0"	31'-0"
WI-301	6'-0"	11'-6"

ROOF INSULATION TABLE		
FRAME LINE 1		
MARK	WIDTH	LENGTH
RI-300	4'-0"	23'-0"
RI-301	6'-0"	23'-0"





GREGORY S. BARFIELD, P.E.
2149 NELL PURVIS ROAD
ADEL, GA 31620
P E # 54419

4-29-06

Adel Steel, Inc.		Constagna Construction	
PROJECT		WALL INSULATION-2	
ID	6002	DESIGN:	CHECK:
PROJECT		DATE: 4/20/06	SHEET OF
ADDRESS	1 Adel Steel, Inc.		