DATE 03/08/2006 Columbia County	Building Permit PERMIT
This Permit Expires One Yea	ar From the Date of Issue 000024204
APPLICANT FREDRICK HAMMOND	PHONE 352-283-0000
ADDRESS PO BOX 1201	NEWBERRY FL 32669
OWNER WILLIAM & JOYCE CARTER	PHONE 497-1298
ADDRESS 968 SW BLUFF DR	FORT WHITE FL 32038
CONTRACTOR FREDRICK HAMMOND	PHONE <u>352-283-0000</u>
LOCATION OF PROPERTY 47 S, R HOLLINGSWORTH BLUE ON THE LEFT SIDE	FF, R BLUFF TO 968
TYPE DEVELOPMENT SFD,UTILITY EST	IMATED COST OF CONSTRUCTION 75000.00
HEATED FLOOR AREA TOTAL AREA	A 1500.00 HEIGHT 23.60 STORIES 1
FOUNDATION CONCRETE WALLS FRAMED RO	OOF PITCH 10/12 FLOOR SLAB
LAND USE & ZONING ESA-2	MAX. HEIGHT 35
Minimum Set Back Requirments: STREET-FRONT 30.00	REAR 10.00 SIDE 10.00
NO. EX.D.U. 0 FLOOD ZONE AE	DEVELOPMENT PERMIT NO. 06-005
PARCEL ID 18-7S-16-04236-062 SUBDIVISION	N CEDAR SPRINGS SHORES
LOT 33 BLOCK PHASE UNIT 5	
CGC017682	(1 1 1 1 1 2 1 1 2 1 1 1 1 1 1 1 1 1 1
Culvert Permit No. Culvert Waiver Contractor's License Numb	per Applicant/Owner/Contractor
EXISTING 06-0131-E BK	<u>JH Y</u>
Driveway Connection Septic Tank Number LU & Zoning	g checked by Approved for Issuance New Resident
COMMENTS: ONE FOOT RISE LETTER INCLUDED, FINISHED FL	OOR ELEVATION CERTIFICATION
MINIMUM OF 37 FEET BEFORE POWER, VARIENCE 235 APPROVE	
	Check # or Cash 4134
FOR BUILDING & ZONING	G DEPARTMENT ONLY (footer/Slab)
Temporary Power Foundation	Monolithic
date/app. by	date/app. by date/app. by
Under slab rough-in plumbing Slab	Sheathing/Nailing
Framing Rough-in plumbing abo	date/app. by date/app. by
date/app. by	ove slab and below wood floor date/app. by
Electrical rough-in Heat & Air Duct	Peri. beam (Lintel)
date/app. by	date/app. by date/app. by
Permanent power C.O. Final	Culvert
	tte/app. by date/app. by
M/H tie downs, blocking, electricity and plumbing date/app.	by Pool date/app. by
Reconnection Pump pole	Utility Pole
date/app. by date/a M/H Pole Travel Trailer	pp. by date/app. by Re-roof
	te/app. by date/app. by
DAMA DELICATION FEE	\$ 7.50 SURCHARGE FEE \$ 7.50
BUILDING PERMIT FEE \$ 375.00 CERTIFICATION FEE	
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00	FIRE FEE \$ _0.00 WASTE FEE \$
FLOOD DEVELOPMENT FEE \$ 50.00 FLOOD ZONE FEE \$ 25.00	CULVERT FEE \$ TOTAL FEE 515.00
INSPECTORS OFFICE J. J.	CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.



12853

Inst: 2003002955 Date: 02/12/2003 Time: 12:18

Doc Stamp-Deed: 542.50

_DC,P.DeWitt Cason,Columbia County B:974 P:1861

Warranty Deed Individual to Individual

THIS WARRANTY DEED made the 6th day of February A.D., 2003

William B. Carter, Jr., and Cassandra C. Hernandez and Linda Carter-Harfield hereinafter called the grantor, to

William B. Carter, Jr. and Joyce W. Carter, his wife whose post office address is: P.O. Box 10608, Riviera Beach, FL 33419 hereinafter called the grantee:

(Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporation)

Witnesseth: That the grantor, for and in consideration of the sum of \$10.00 and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys, and confirms unto the grantee, all that certain land situate in COLUMBIA County, Florida, viz: Parcel ID# R04236-062

Lot 33, Unit 5, Cedar Springs Shores, a subdivision as per plat recorded in Plat Book 4, Page 5, Public Records of Columbia County, Florida.

The above described property is not the constitutional homestead property of the grantors.

TOGETHER with all tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

TO HAVE AND TO HOLD, the same in fee simple forever.

AND the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2002.

IN WITNESS WHEREOF, the said grantor has signed and sealed these presents the day and year first above written.

Signed, sealed and delivered in our presence:

Witness: Angela M. Osborne

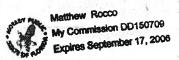
Matthew D. Rocco Witness:

STATE OF FLORIDA **COUNTY OF COLUMBIA** William B. Carter, Jr. M. Hand & Vance, A. Carrander C. Hernander

The foregoing instrument was acknowledged before me this 6th day of February, 2003 by William B. Carter, Jr., and Cassandra C. Hernandez, personally known to me or, if not personally known to me, who produced a Driver's License for identification and who did not take an oath. The B. Careter, TR. William B. Careter, TR. William B. Careter, TR. William B. Careter, TR.

Notary Public

(Notary Seal)



IN WITNESS WHEREOF, the said grantor has signed and sealed these presents the day and year first above written.

Signed, sealed and delivered in our presence:

Witness:

Witness: Fordon

Inst:2003002955 Date:02/13/2003 Time:12:18

Doc Stamp-Deed : 542.50

DC,P.DeWitt Cason,Columbia County B:974 P:1862

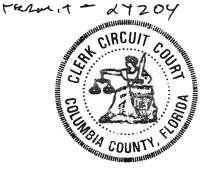
The foregoing instrument was acknowledged before me this 3 day of February, 2003 by Linda Carter-Hatfield, personally known to me or, if not personally known to me, who produced a Driver's License for identification and who did not take an oath.

STATE OF COUNTY OF

My Commission Expires 03/06/2004

Prepared by: Michael H. Harrell Abstract & Title Services, Inc. 420 W. Baya Avenue Lake City, FL 32055

Prepared by/Return to: Justin S.W. McMurray Haile Title Company, LLC P.O. Box 159 Newberry, FL (352) 472-7373 32669



STATE OF FLORIDA, COUNTY OF COLUMBIA I HEREBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office.
P. Dewyl CASON, CLERK OF COURTS

Date

NOTICE OF COMMENCEMENT
PERMIT NUMBER:
STATE OF FLORIDA COUNTY OF ALACHUA
THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Fla. Stats., the following information is provided in this notice of commencement.
DESCRIPTION OF PROPERTY Lot 33, UNIT 5 OF CEDAR SPRINGS SHORES, a subdivision as per plat thereof recorded in Plat Book 4, Page 5 of the Public Records of Columbia County, Florida
TO CONSTRUCT: GENERAL DESCRIPTION OF IMPROVEMENTS Single Family Dwelling
OWNER INFORMATION OWNER NAME: William B. Carter, Jr. and Joyce W. Carter ADDRESS: 968 SW BLUFF OR: FT WHITE, FL 32038 PHONE NUMBER: 386 491-1298 CITY: STATE: FL ZIP CODE:
INTEREST IN PROPERTY: fee simple FEE SIMPLE TITLEHOLDER NAME: same as above FEE SIMPLE TITLEHOLDER ADDRESS: (if other than owner)
CONTRACTOR NAME: FREDERICK G. HAMMOND. HAMMOND BUILLING AND DESIGN, INC. ADDRESS: PHONE NUMBER: 352-283-0000 CITY: STATE: FL ZIP CODE: LICENSEH CG C 017682
BONDING COMPANY: ADDRESS: PHONE NUMBER: CITY STATE: ZIP CODE:
LENDER NAME: Tri-County Bank ADDRESS: PO BOX & 99 CITY: NEWBERGY STATE: FL ZIP CODE: 32669
Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a) 7., Florida Statutes:
In addition to himself, Owner designates of of to receive a copy of the Lienor's Notice as provided in Section
713.13(1)(b), Florida Statutes. Expiration date is one (1) year from date of recording unless a different date is specified. William B. Carter, Jr.
Joyce W. Carter

STATE OF FLORIDA COUNTY OF ALACHUA

The foregoing was acknowledged before me this 18th day of May, 2006, by William B. Carter, Jr., and Joyce W. Carter, who produced a Florida Drivers License as identification.

JUSTIN S. W. MCMURRAY MY COMMISSION # DD439262
EXPIRES: June 09, 2009
RY
FI. Notary Discourse

Notary Public

Inst:2006013013 Date:05/30/2006 Time:14:29
_____DC,P.DeWitt Cason,Columbia County B:1085 P:400

Columbia County Building Permit Application (k 4)34 (F15.40) Revised 9-23-04

(p. 1.101130g 0 203.
For Office Use Only Application # 0602-40 Date Received 2-13-de By Ut Permit # 24204
Application Approved by - Zoning Official 6 Date 6 Date 6 Plans Examiner 6 Date 3 - 06 Dat
Flood Zone AE Development Permit YES Zoning ESA-2 Land Use Plan Map Category ESA
Comments V0235 sproved DP 06-005
. Soute Fe River 36St. Fleat 37ft 15 Floor Panel 0255B No Aluday
386-462-2566
Applicants Name <u>Fredrick Hammond</u> Phone <u>352-283-5000</u>
Address POBOXIZOI Newberry FL 32669
Owners Name WILLIAM B. + JOYCE W. CAKTER Phone 386-497-1298
911 Address 968 SW BLUFF DRIVE, FT. WHUE, FL 32038
Contractors Name 1/24 Mulli No Building & Design Phone 352-283-000
Address V.O. Box 1201 New Serry FL 32669
Fee Simple Owner Name & Address
Bonding Co. Name & Address
Architect/Engineer Name & Address WILLIAM N. JODYAM VE SAKAZOLA SI 34243
Mortgage Lenders Name & Address Angels, 25365 W Newsberry Rv. Newsberry FL 32669 KENNERS E. AKNOW
Circle the correct power company - FL Power & Light - Clay Elec Suwannee Valley Elec Progressive Energy
Property ID Number 1875 16 04236 062 Estimated Cost of Construction \$130,000
Subdivision Name CEDAR SARWES SMRES Lot 33 Block Unit 5 Phase
Driving Directions South From FI KIN ITE ON STATE RO 47 THEN RIGHT ON HOLLIAGESTA
RD, RIGHT ON BLUFF DKINE (DW KNEEKD), GO TO 968 BELIFF, SITE ON LEFT
Spray Blue (CARTER COVE)
Type of Construction RESIDENTAL, FRAME, WOOD Number of Existing Dwellings on Property
Total Acreage 1.5 Lot Size 100 x 1085 Do you need a - Culvert Permit or Culvert Walver or Have an Existing Drive
Actual Distance of Structure from Property Lines - Front
Total Building Height 23-6" Number of Stories \ Heated Floor Area 15005/F Roof Pitch 46-10/12
WILDET
Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.
OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.
(1/11/211/211/211/211/211/211/211/211/21
Owner Builder or Agent (Including Contractor) Contractor Signature
Contractors License Number <u>C6C017682</u>
STATE OF FLORIDA Competency Card NumberCOUNTY OF COLUMBIA
Sworn to (or affirmed) and subscribed before me MY COMMISSION # DD 333503
this / 3 day of February 20 de Bonded Thru Notary Public Underwriters
Personally known or Produced Identification Notary Signature

Columbia County Building Department Flood Development Permit

Development Permit F 023- 06-005

DATE 03/08/2006 BUILDING PERMI	Γ NUMBER <u>000024204</u>
APPLICANT FREDRICK HAMMOND	PHONE 352-283-0000
ADDRESS PO BOX 1201	NEWBERRY FL 32669
OWNER WILLIAM & JOYCE CARTER	PHONE 497-1298
ADDRESS 968 SW BLUFF DR	FORT WHITE FL 32038
CONTRACTOR FREDRICK HAMMOND	PHONE 352-283-0000
ADDRESS	
SUBDIVISION CEDAR SPRINGS SHORES	Lot 33 Block Unit Phase
TYPE OF DEVELOPMENT SFD,UTILITY	PARCEL ID NO. 18-7S-16-04236-062
FLOOD ZONE AE BY BK 1-6-88	FIRM COMMUNITY #. 120070 - PANEL #B
FIRM 100 YEAR ELEVATION 36'	PLAN INCLUDED YES or NO
REQUIRED LOWEST HABITABLE FLOOR ELEVA	
IN THE REGULATORY FLOODWAYYES or NO	RIVER Santa Fe
SURVEYOR / ENGINEER NAME William fix	LICENSE NUMBER 870/
ONE FOOT RISE CERTIFICATION INCL	UDED
ZERO RISE CERTIFICATION INCLUDE	
ZERO RISE CERTIFICATION INCLUDE	
SRWMD PERMIT NUMBER	
SRWMD PERMIT NUMBER(INCLUDING THE ONE FOOT RISE CER	TIFICATION)
	
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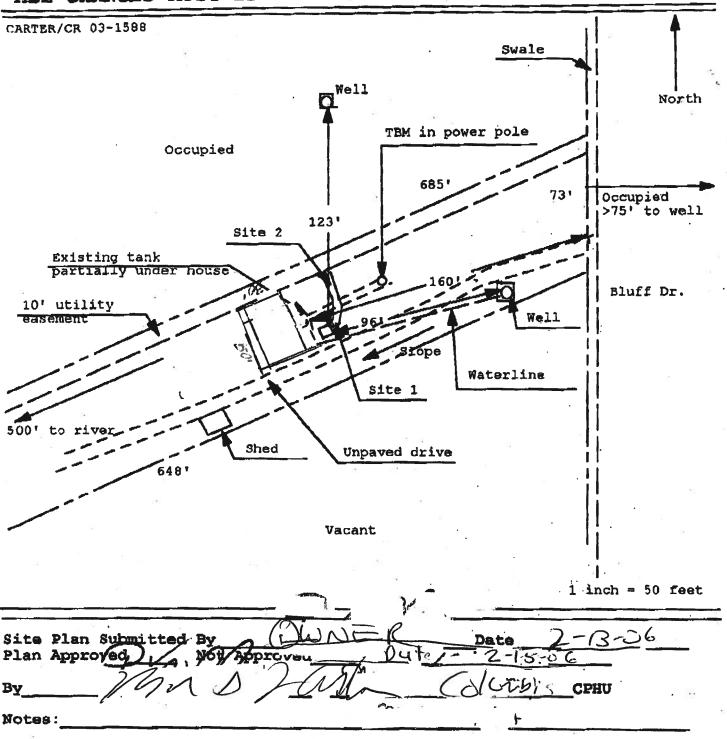
135 NE Hernando Ave., Suite B-21 Lake City, Florida 32055

Phone: 386-758-1008 Fax: 386-758-2160



Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan Permit Application Number: 06-031-E

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT





161 N.W. Madison St., Suite 102 Lake City, Florida 32055

> Tel: 386-758-4209 Fax: 386-758-4290

2/24/2006

Columbia County Building Department

To whom it may concern,

RE: Permit # 0602-40 and 0602-41

I have reviewed the conditions for the referenced property. The property is located in a flood zone (Zone AE). The required floor elevation (37.0') shall be set 1' above the 100 year flood elevation. The 100 year flood elevation is established at 36.0'. Please find a copy of the calculations verifying the flood rise to be less than 1'-0". If you have any questions, please call me at (386) 758-4209.

Sincerely,

William Freeman, P.E.

Certificate of Authorization # 00008701

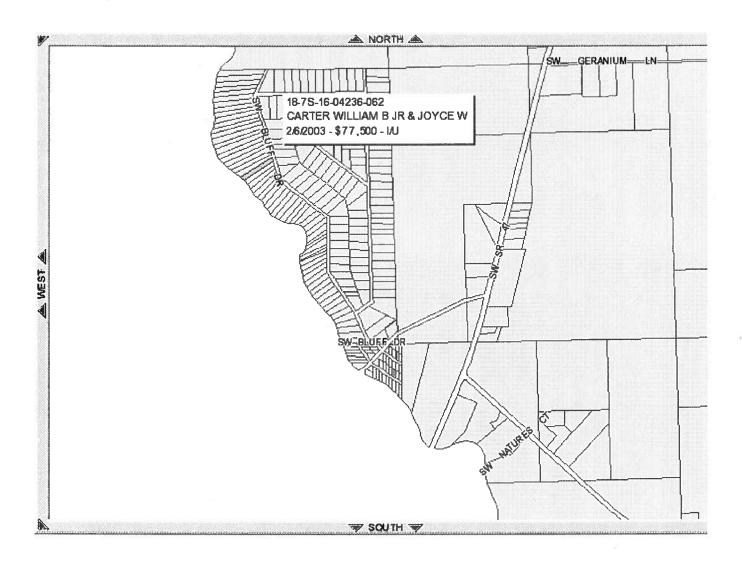
illian H. Fream

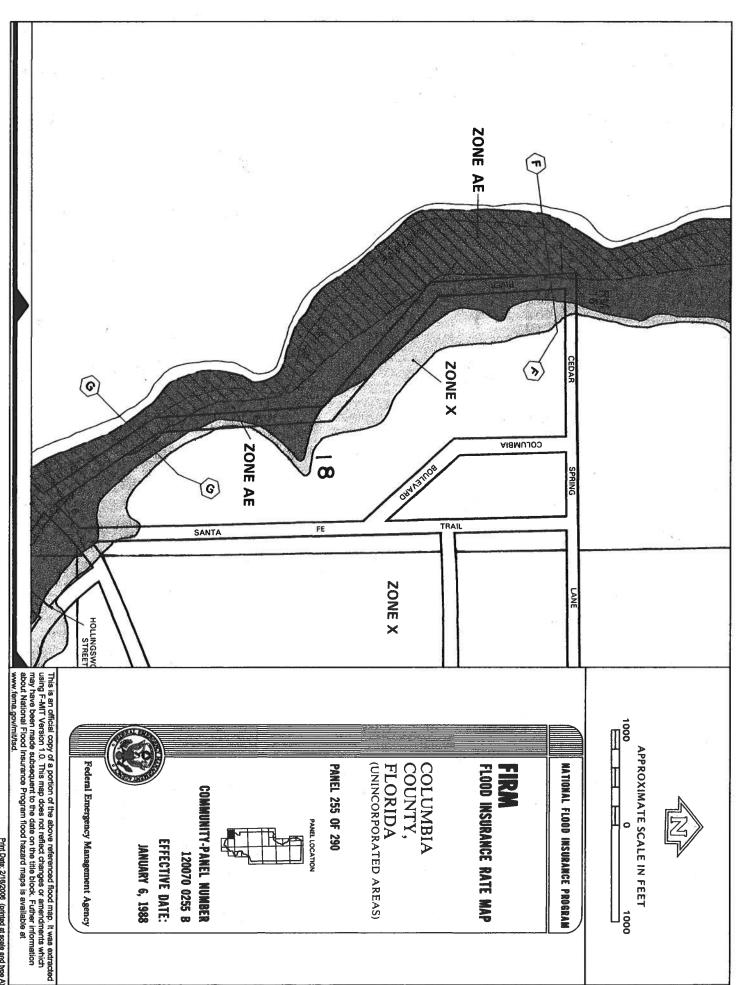
Freeman Design Group, Inc. 161 NW Madison St., Ste. # 102 Lake City, FL 32055 (386) 758-4209

1-ft Rise Flood Certification Calculations						
Project: Fred Hammond (Permit #0602-40 and #0602-41)						
	Home and	Detached Gar	rage			
Footing Area (sf): House	1500	30'x50' slab	1500.00	sf slab		
Rise Ht(ft):	3					
Footing Area (sf): Garage	720	24'x30' slab	720.00	sf slab		
Rise Ht(ft):	3					
Contributing Area:	1.33	acres>	57,934.80	sf		
New Ftg Area:		•	2220.000	sf		
Net Land Area (cont	ributing minu	is new):	55,714.80	sf		
Slab Volume Displa	cement:		6660.00	cf		
Amount of Rise (Sla	b volume / la	nd area) x 12:	1.434	in		

Base Flood Elevation 36.0 ft Min. Finished Floor Elevation 37.0 ft

Jellio H. Free CERT. OF MITH. 00008701





Print Date: 2/16/2006 (printed at scale and type A)

Columbia County Property

Appraiser
DB Last Updated: 9/16/2005

Parcel: 18-7S-16-04236-062 HX

2005 Proposed Values

Tax Record

Property Card

Interactive GIS Map

Print

Owner & Property Info

Search Result: 1 of 1

Owner's Name	CARTER WILLIAM B JR & JOYCE W			
Site Address	BLUFF			
Mailing Address	968 SW BLUFF DRIVE FT WHITE, FL 32038			
Brief Legal	LOT 33 UNIT 5 CEDAR SPRING SHORES. ORB 486- 184, 761-1859, DC 965-2593, PROB#02-214CP			

Use Desc. (code)	MOBILE HOM (000200)
Neighborhood	18716.01
Tax District	3
UD Codes	MKTA02
Market Area	02
Total Land Area	0.000 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (2)	\$36,098.00
Ag Land Value	cnt: (0)	\$0.00
Building Value	cnt: (1)	\$20,676.00
XFOB Value	cnt: (1)	\$1,155.00
Total Appraised Value		\$57,929.00

Just Value		\$57,929.00
Class Value		\$0.00
Assessed Value		\$57,929.00
Exempt Value	(code: HX)	\$25,000.00
Total Taxable Value		\$32,929.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price
2/6/2003	974/1861	WD	I	U	06	\$77,500.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value	
1	MOBILE HME (000800)	1980	Vinyl Side (31)	1344	1648	\$20,676.00	
Note: All S.F. calculations are based on exterior building dimensions.							

Extra Features & Out Buildings

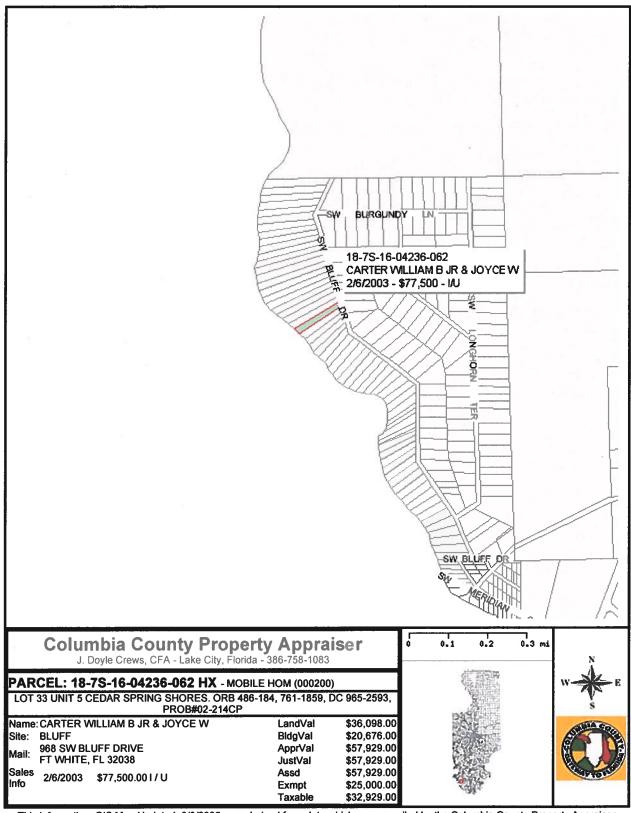
Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0040	BARN,POLE	0	\$1,155.00	770.000	22 x 35 x 0	(.00)

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000132	SFR RIVER (MKT)	100.290 FF - (.000AC)	1.00/1.00/1.00/1.00	\$340.00	\$34,098.00
009945	WELL/SEPT (MKT)	1.000 UT - (.000AC)	1.00/1.00/1.00/1.00	\$2,000.00	\$2,000.00

Columbia County Property Appraiser

DB Last Updated: 9/16/2005



This information, GIS Map Updated: 8/3/2005, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

From: The Columbia County Building Department

Plans Review

135 NE Hernando Av.

P. O Box 1529

Lake City Florida, 32056-1529

Reference to: Build permit application Number: 0602-41

Hammond Builders and Design Owner William Carter lot 33 of Cedar Springs Shores

On the date of February 16, 2006 application 0602-40 and plans for construction of a single family dwelling detached garage were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0602-40 when making reference to this application.

- 1. Lot 33 Unit 5 of Cedar Springs Shores subdivision as shown on the FIRM Flood Insurance Map Community-Panel Numbers 12007 0225 B defines that the Lot 33 is within an AE Flood Zone with an established elevation of 36 foot flood elevation. The second floor elevation of the proposed garage if (as the plan show to be a condition space) will be required to be at an elevation of 37 foot. An elevation certification from a surveyor will be required to be submitted prior to issuance of a certificate of occupancy.
- Columbia County regulations require a one foot rise analyses certified by an
 engineer be submitted to the Building and Zonings department prior to issuance of
 a building permit.

3. If any existing or additional earth will relocated within this parcel of land to help established the required 37 foot flood elevation a grading plan as detailed in Columbia County Florida Resolution No. 2005R-26 (attached) will be required to be submitted to this department for review by the Columbia County engineer.

4. Please show on the structural elevation plans the total height of the structure from the established finished grade to the highest roof peak of the structure.

5. On the floor plan please show the total amount of the condition and unconditioned square footage that will be under the roof area.

6. On the plans please identify all the exterior/interior shear walls.

7. Indicate on the foundation plans the load bearing soil capacity which be required to support the A2 foundation as designed.

Submit the Florida product approval numbers and manufacture information for the Tie Max System.

Thank you,

Joe Haltiwanger Plan Examiner

Columbia County Building Department

COLUMBIA COUNTY, FLORIDA RESOLUTION NO. 2005R-26

A RESOLUTION OF COLUMBIA COUNTY, FLORIDA, PROVIDING FOR ADDITIONAL REQUIREMENTS FOR A DEVELOPMENT PERMIT ON PROPERTY WHICH HAS BEEN IDENTIFIED AS "FLOOD PRONE;" AND PROVIDING FOR AN EFFECTIVE DATE.

WHEREAS, since the hurricane season of 2004, Columbia County has experienced significant flooding and related issues impacting the public health, safety and welfare of the residents and citizens of Columbia County as well as their property; and

WHEREAS, the Board of County Commissioners of Columbia County, Florida, finds it is necessary and in the best interest of Columbia County and its residents and citizens for the protection of the health, safety and welfare, together with the protection of property interests in Columbia County, to provide requirements in addition to those currently set forth in local, state and federal statutes, ordinances, rules and regulations, including but not limited to the Columbia County Comprehensive Plan and Columbia County Land Development Regulations (LDRs), for the application and issuance of a development permit.

NOW, THEREFORE, BE IT RESOLVED BY THE BOARD OF COUNTY COMMISSIONERS OF COLUMBIA COUNTY, FLORIDA AS FOLLOWS:

1. Properties, including lots and acreage, which have been identified in Columbia County as "flood prone" shall, in addition to all other local, state and federal requirements, prior to issuance of a development permit through the Columbia County Building Department provide the following:

- a. In addition to all other required submittals, the development permit applicant shall file a grading plan for the property proposed to be developed. The grading plan shall be signed and sealed by a Florida registered professional engineer.
- b. The grading plan shall delineate proposed changes from natural ground elevation, if any, including the amount of fill material to be added to the site. The grading plan shall clearly demonstrate that the natural flow of water shall not be altered nor will adjacent properties be negatively impacted by the proposed development.
- c. The grading plan shall further establish the lowest habitable floor elevation and building location on the lot or acreage.
- d. Upon its completion, the applicant shall obtain from a Florida licensed land surveyor and provide to Columbia County certification as to the actual height of the finished floor established by the grading plan.
- 2. Additionally, all "flood prone" properties shall require written certification by a competent Florida licensed professional or agency stating that the property is not defined as a wetland as defined in the Columbia County Land Development Regulations.
- 3. The term "flood prone" is defined as those lots, acreage or properties that can be demonstrated on existing FEMA or other maps as flood prone properties which competent personal testimony through affidavit or otherwise establishes the property has a history of flooding which would adversely impact development upon the property.
- 4. There shall be exempt from the requirements of this Resolution lots, acreage or properties otherwise defined as "flood prone" where the ratio of "non-flood prone" property

(numerator) to the square footage of impervious surface development on the property (denominator) is no less than 3-to-1. However, all other permitting requirements of the County must be satisfied.

- 5. Any interested party who is subject to these additional permitting requirements and believes they have been inappropriately applied to them may appeal the decision to the Board of County Commissioners of Columbia County. All such appeals must be in writing and mailed to the Board of County Commissioners of Columbia County, Post Office Box 1529, Lake City, Florida 32056-1529. At this time no appeal fee is assessed.
- 6. This Resolution shall remain in effect until the Board of County Commissioners has approved an appropriate ordinance addressing the flood prone issues of Columbia County or until further action of the Board.

unanimously passed and adopted by the Board of County Commissioners at its regular meeting on the 16th day of June, 2005.

BOARD OF COUNTY COMMISSIONERS COLUMBIA COUNTY, FLORIDA

w. / 1

mifer Flinn, Chairman

ATTEST:

P. DeWitt Cason, Clerk of Courts

(SEAL)

13521372 2721

03/03/2006 10:07 FAX 3526947733

GEOTECH

+ GEOTECH_GVILLE

001/002

Mar 02 06 10:36p

Geo-Tech, Inc

(352)372 2721

ENGINEERING CONSULTANTS IN GEOTECHNICAL . ENVIRONMENTAL . CONSTRUCTION MATERIALS TESTING

March 1, 2006 Project No. 062696.01G

Bill Carter 968 SW Bluff Drive Fort White, FL 32038

Attention:

Mr. Bill Carter

Project:

Proposed Residence and Garage, Cedar Springs Shores, Lot 33 - Unit 5

Columbia County, Florida Soil Bearing Capacity

Dear Mr. Carter:

As requested, Geo-Technologies, Inc. (Geo-Tech) has visited the above referenced project site. The purpose of our visit was to perform static cone penetrometer readings in the area that the proposed residence will be placed. Six (6) auger borings with static cone penetrometer readings were performed to four (4) feet below site grade. Based on the results of the penetrometer readings, the maximum allowable soil bearing pressures found at these locations are approximately 2, 500 pounds per square foot based.

Geo-Technologies, Inc. (Geo-Tech) trust this report is sufficient to meet your immediate needs. Should you have any questions concerning this report or if we may be of further assistance, please do not hesitate to contact the undersigned.

Sincerely.

Bulba Jornsbook F.
Donald "Bubba" Youngblood
Branch

Branch Manager

DY/DC: kw



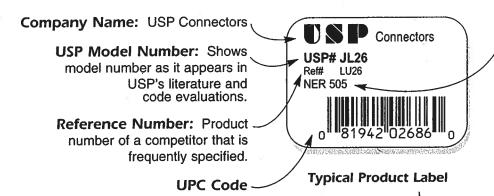
4000 SW 35TH TERRACE, SUITE C . GAINESVILLE, FLORIDA 32608 . PHONE: (352) 372-1274 . FAX: (352) 372-2721



Product Identification and Labeling

Code Evaluation Labeling Requirements

Each USP Lumber Connector is identified with the following information:

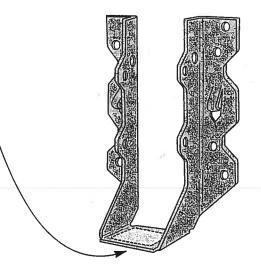


Code Evaluation Number: This is the code evaluation report number that the specific model number appears in. This number may appear as a listing for International Conference of Building Officials (ICBO), Southern Building Code Congress International (SBCCI), Officials Buildina and Code Administrators International Inc. (BOCA) or National Evaluation Service (NER) which covers all three agencies.

Code Evaluation Labeling Requirements:

Labeling of products for field identification is a requirement of the code evaluation reports. The code evaluation reports specifically state that each product manufactured and listed in a code evaluation report must be labeled with the manufacturer's name and/or trademark, the model number and code evaluation report number. Failure to do this would be a violation of the code evaluation report guidelines.

Currently, code agencies use a variety of descriptions when stipulating how this information will be applied to the lumber connector. The descriptions range from "labeled" "stamped" to "identified." USP currently labels all products which carry a code evaluation report number and is in the process of stamping this identification into some of our products. Either method is acceptable under guidelines set forth by the code evaluation agencies today.



Labels are positioned on products so they can be seen after installation

USP, Eastern Region 703 Rogers Drive Montgomery, MN 56069-1324 Livermore, CA 94550-9611 1-800-328-5934

Fax: 1-507-364-8762

USP, Western Region 2150 Kitty Hawk Road 1-800-227-0470

Fax: 1-925-373-9213

YOUR LOCAL USP DEALER/DISTRIBUTOR

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
- 2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

Applicant	Plans Examin	ENTS: Two (2) complete sets of plans containing the following:
	o .	All drawings must be clear, concise and drawn to scale ("Optional" details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
	ď	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
œ e	B	Site Plan including: a) Dimensions of lot
		b) Dimensions of building set backs
		c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.
		d) Provide a full legal description of property.
	Ø	Wind-load Engineering Summary, calculations and any details required
		Plans or specifications must state compliance with FBC Section 1609.
		The following information must be shown as per section 1603.1.4 FBC
		 a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table
		1-1, ASCE 7.
	194	c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated.
		d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient.
all gampa and		e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.
	/	Elevations including:
	8	a) All sides
		b) Roof pitch
D	G/	c) Overhang dimensions and detail with attic ventilation

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- d) Location, size and height above roof of chimneys.
- e) Location and size of skylights
- f) Building height
- e) Number of stories

Floor Plan including:

- a) Rooms labeled and dimensioned.
- b) Shear walls identified. MTC 6
- c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (see attach forms).
- d) Show safety glazing of glass, where required by code.
- e) Identify egress windows in bedrooms, and size.
- f) Fireplace (gas vented) (gas non-vented) or wood burning with hearth, (Please circle applicable type).
- g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- h) Must show and identify accessibility requirements (accessible bathroom) Foundation Plan including:
- a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
- b) All posts and/or column footing including size and reinforcing
- c) Any special support required by soil analysis such as piling
- d) Location of any vertical steel.

Roof System:

- a) Truss package including:
 - 1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
 - 2. Roof assembly (FBC 106.1.1.2)Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- b) Conventional Framing Layout including:
 - 1. Rafter size, species and spacing
 - 2. Attachment to wall and uplift
 - 3. Ridge beam sized and valley framing and support details
 - Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

- a) Masonry wall
 - 1. All materials making up wall
 - 2. Block size and mortar type with size and spacing of reinforcement
 - 3. Lintel, tie-beam sizes and reinforcement
 - 4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
 - All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.
 - 6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
 - 7. Fire resistant construction (if required)
 - 8. Fireproofing requirements
 - 9. Shoe type of termite treatment (termiticide or alternative method)
 - 10. Slab on grade
 - Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
 - b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
 - 11. Indicate where pressure treated wood will be placed
 - 12. Provide insulation R value for the following:

			b) Wood frame wall 1. All materials making up wall
			2. Size and species of studs
			3. Sheathing size, type and nailing schedule
			4. Headers sized
			5. Gable end showing balloon framing detail or gable truss and wall
			hinge bracing detail
			6. All required fasteners for continuous tie from roof to foundation
			(truss anchors, straps, anchor bolts and washers) shall be designed
			by a Windload engineer using the engineered roof truss plans.
			7. Roof assembly shown here or on roof system detail (FBC
			106.1.1.2) Roofing system, materials, manufacturer, fastening
			requirements and product evaluation with wind resistance rating)
			See Fire resistant construction (if applicable)
			589. Fireproofing requirements
			10. Show type of termite treatment (termiticide or alternative method) 11. Slab on grade
			a. Vapor retarder (6Mil. Polyethylene with joints lapped 6
			inches and sealed
			b. Must show control joints, synthetic fiber reinforcement or
			welded wire fabric reinforcement and supports
			12. Indicate where pressure treated wood will be placed
	<i>3</i>		13. Provide insulation R value for the following:
			a. Attic space
			b. Exterior wall cavityc. Crawl space (if applicable)
N/			c) Metal frame wall and roof (designed, signed and sealed by Florida Prof.
		U	Engineer or Architect)
			Floor Framing System:
0 " "			a) Floor truss package including layout and details, signed and sealed by Florida
			Registered Professional Engineer
	NA		b) Floor joist size and spacing
	بادر		c) Girder size and spacing
	•		d) Attachment of joist to girder
			e) Wind load requirements where applicable
♂			Plumbing Fixture layout
		_	Electrical layout including: a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
0 0 0 0			b) Ceiling fans
[]		0	c) Smoke detectors
10/ 13		0	d) Service panel and sub-panel size and location(s)
ro∕ .	.,	ם י	e) Meter location with type of service entrance (overhead or underground)
M =			f) Appliances and HVAC equipment
		0	g) Arc Fault Circuits (AFCI) in bedrooms
o l		0	h) Exhaust fans in bathroom
			HVAC information
		0	a) Energy Calculations (dimensions shall match plans)
	4.		b) Manual J sizing equipment or equivalent computation
	Alk		c) Gas System Type (LP or Natural) Location and BTU demand of equipment
0/	AK		Disclosure Statement for Owner Builders
3			*** Notice Of Commencement Required Before Any Inspections Will Be Done
3			Private Potable Water
			•

a. Attic spaceb. Exterior wall cavityc. Crawl space (if applicable)

- a) Size of pump motor
- b) Size of pressure tank
- c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

- 1. <u>Building Permit Application:</u> A current Building Permit Application form is to be completed and submitted for all residential projects.
- 2. <u>Parcel Number:</u> The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.
- Environmental Health Permit or Sewer Tap Approval: A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued.
 (386) 758-1058 (Toilet facilities shall be provided for construction workers)
- 4. <u>City Approval:</u> If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321

A

5. Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.

A development permit will also be required. Development permit cost is \$50.00

- 6. <u>Driveway Connection</u>: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial. <u>If the project is to be located on a F.D.O.T. maintained road, than an F.D.O.T. access permit is required.</u>
- 7. 911 Address: If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE — TIME WILL NOT ALLOW THIS —PLEASE DO NOT ASK

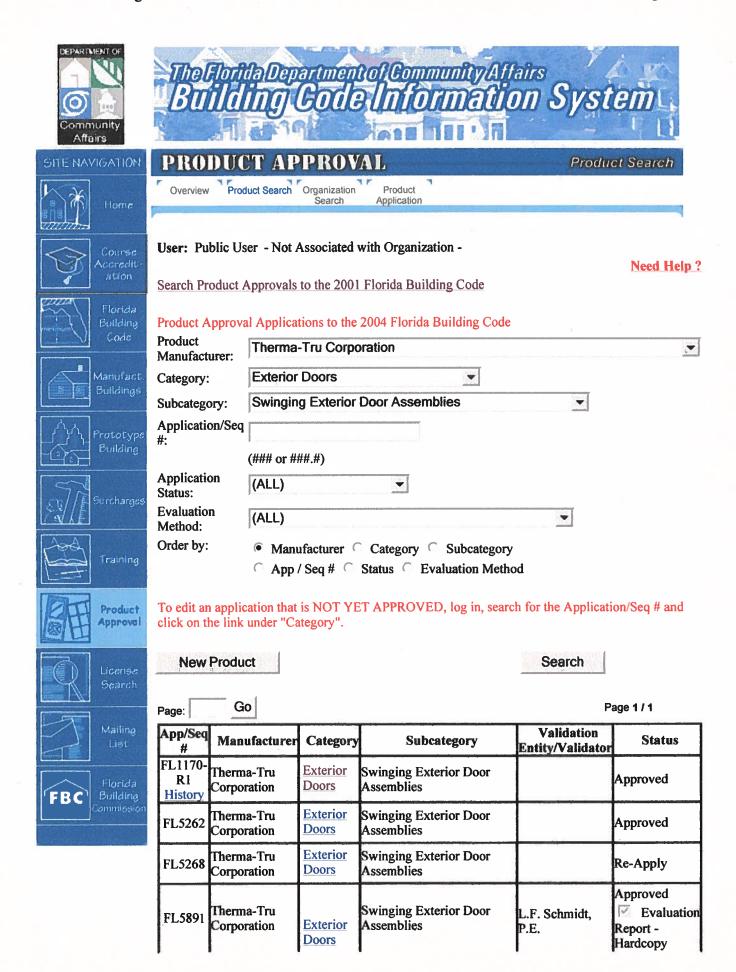
THE TOTAL PROPERTY OF THE PROP

Location: 968 SW BLUFF DR. FTWEEF, FL Project Name: CARSOR LOVE RESIDENCE

As required by Florida Statute 553.842 and Florida Ádministrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit on or after April 1, 2004. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

Ca	tegory/Subcategory	Manufacturer	Product Description	Approval Number(s)
A.	EXTERIOR DOORS			- Prover Hamper(8
	1. Swinging	THROA-TRU	FIBER LIASSIE	FL1170-R1
	2. Sliding			FEIIID-KI
	3. Sectional	WAYNE DALTON	GARAGE DAIR (FLZZREN)	FL3076-R1
	4. Roll up	11277	PERENT)	P-3010-K
	5. Automatic			
	6. Other GARAGE TOR			
В.	WINDOWS			
Г	1. Single hung			
	2. Horizontal Slider			
Г	3. Casement			
	4. Double Hung	SILVERLINE	DOUBLE YUNG WANDARS	C1 1111 - D1
	5. Fixed	2/4/200	DUADLE 13WA 1WAVES	FL4411-21
Г	6. Awning			
Г	7. Pass -through			
	8. Projected			
	9. Mullion			
	10. Wind Breaker			
	11 Dual Action			
Г	12. Other			
	PANEL WALL			
۳	1. Siding	74 mm (14.40 m	/ A 0	
-	2. Soffits	JAMES HOKDLE	HARDI PLANK SIDING	FL889-R2
H	3. EIFS			
H	4. Storefronts			
	5. Curtain walls			
\vdash	6. Wall louver	-		
\vdash	7. Glass block			
	8. Membrane			
_	9. Greenhouse			
⊢	10. Other			
L				
<u>D.</u>	ROOFING PRODUCTS			
-	Asphalt Shingles			
L	2. Underlayments			
_	3. Roofing Fasteners			
_	4. Non-structural Metal Rf	N HIEN	MASTER RYB MEIAL ROGENG .	FL4586
L	5. Built-Up Roofing			
<u> </u>	6. Modified Bitumen			
<u> </u>	7. Single Ply Roofing Sys			
<u> </u>	8. Roofing Tiles			
L	9. Roofing Insulation			
	10. Waterproofing			
	11. Wood shingles /shakes	B		
L	12. Roofing Slate			

Location		Permit # (FOR STAFF)	Permit # (FOR STAFF USE ONLY)		
Contractor or Contractor's Authorize 968 Sto Bluff Devot	ed Agent Signature	Print Name	Date		
understand these products	, may nave to D	e refuesed u abbiesai caminor ne demoi	istrated during inspect		
, ,		applicable manufacturers installation re be removed if approval cannot be demor	•		
• • • • •	• •	the performance characteristics which the performance characteristics	•		
me of inspection of these p	products, the fo	llowing information must be available to	the inspector on the		
he products listed below d	id not demonst	rate product approval at plan review. I u	understand that at the		
2.					
1.					
ENVELOPE PRODUCTS					
. NEW EXTERIOR					
13. Other					
12. Sheds					
11. Wall	1				
10. Deck-Roof	NIA				
9. Plastics	 				
8. Insulation Forms	 				
7. Material					
6. Concrete Admixtures	 				
4. Railing 5. Coolers-freezers					
3. Engineered lumber	BOISE	VERSA - LAM, LAMINATED VENSTR IN	MBER FL 1644-R1		
2. Truss plates	MITEK	TRUS PUNDES 20 BA	FL 2197-R1		
1. Wood connector/anchor	HURRI-BOCK	ANCHER SYDSEM	FL 1730-81		
COMPONENTS	11	1440 100 844	F. 125 51		
STRUCTURAL					
2. Other					
	 				
1. Skylight	N/A				
SKYLIGHTS	N/a				
7. Others					
5. Roll-up6. Equipment	<u> </u>				
4. Colonial					
3. Storm Panels					
Accordion Bahama					
SHUTTERS	NJA				
17. Other	NIIA				
Polyurethane Roof	3-12-				
16. Spray Applied					
15. Roof Tile Adhesive					
Coatings					
14. Cements-Adhesives –					
13. Liquid Applied Roof Sys			2,552 433 1443 1150 1/6		
		Li loddet bescription	Pubbliosai lantilibel/2		





Product

Approval

FBC

The Florida Department of Community Affairs Building Gode Information System

PRODUCT APPROVAL

Product Type Detail

Overview Product Search Organization Product Application

User: Public User - Not Associated with Organization -

Need Help?

Application #: FL1170-R1

Date Submitted: 08/06/2005

Code Version: 2004

Product Manufacturer: Therma-Tru Corporation
Address/Phone/email: 118 Industrial Drive

Edgerton, OH 43517 (419) 298-1740

Category: Exterior Doors

Subcategory: Swinging Exterior Door

Assemblies

Evaluation Method: Certification Mark or Listing

Referenced Standards from the Florida Building Code: Section Standard Year

ASTM E1996 2002 PA 201, 203 1994 PA 202 1994 ASTM E 330 2002 ASTM E1300 2002

Certification Agency: National Accreditation & Certification Agency:

Management Institute,

Quality Assurance Entity:

Validation Entity:

Authorized Signature: Steve Jasperson

sjasperson@tttechnologies.us

Evaluation/Test Reports Uploaded: Installation Documents Uploaded:

Product Approval Method: Method 1 Option A

Application Status:

Approved

Date Validated:

08/06/2005

Date Approved:

10/11/2005

Date Certified to the 2004 Code:

Page:

Go

Page 1 / 1

App/Seq #	Product Model # or Name	Model Description	Limits of Use
1170.1	"Classic-Craft"	Fiberglass Door w/wo sidelites	Maximum Design Pressure Rating: Single Door - + 100.0 -100.0 Double Door - + 70.0 - 70.0
1170.2	"Construction Series"	Steel Door w/wo sidelites	Maximum Design Pressure Rating: Single Door - + 67.0 -67.0 Double Door - + 60.0 - 60.0 (See Door Label for Variations)
1170.3	"Fiber-Classic"		Maximum Design Pressure Rating: Single Door - + 67.0 -67.0 Double Door - + 60.0 - 60.0 (See Door Label for Variations)
1170.4	"Premium Series"	Steel Door w/wo sidelites	Maximum Design Pressure Rating: Single Door - + 75.0 -75.0 Double Door - + 65.0 - 65.0
1170.5	"Smooth-Star"	Fiberglass Door w/wo sidelites	Maximum Design Pressure Rating: Single Door - + 67.0 -67.0 Double Door - + 60.0 - 60.0 (See Door Label for Variations)

Next

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R W Building Consultants, Inc.

Consulting and Engineering Services for the Building Industry

P.O. Box 230 Valrico, FL 33594 Phone 813.659.9197

Facsimile 813.659.4858

ENGINEER'S NOTICE OF EVALUATION # TTF-262IF

Therma-Tru Corp. 108 Mutzfeld Road Butler, IN 46721 Phone 260.868.5811 Facsimile 260.868.5190

DESCRIPTION OF UNIT

Model Designation: Fiber Classic/Smooth Star, Series Fiberglass Door (Opaque) Impact

Maximum Overall Nominal Size: up to 6'0 x 6'8 Usable Out-swing Configurations: X, XX

General Description: The head and jambs are wood measuring 4.5" x 1.25" with an extruded aluminum threshold. The door panels are 1.68" thick and consist of two SMC skins with a thickness of 0.065" for the Fiber Classic Series or the Smooth Star Series with a skin thickness of 0.070". Skins are glued to wood stiles and rails with a urethane core.

FBC Section 1707 Materials and Assembly Tests:

(1707.43 Exterior Door Assemblies: 1707.45 Mullions Door Assemblies)

Test	Description Description	Test Location	Date	Report No.	Certifying Engineer
ASTM E330	Uniform Static	ETC - Rochester, NY	July 14, 2001 June 22, 2001	ETC-01-741-10702.0 ETC-01-741-11008.0	Wendell Haney, P. E. #54158 Joseph Dolden, P.E. #42929
TAS 202	Air Pressure Forced Entry	ETC - Rochester, NY	July 14, 2001 June 22, 2001	ETC-01-741-10702.0 ETC-01-741-11008.0	Wendell Haney, P. E. #54158 Joseph Dolden, P.E. #42929
ASTM E331	Water Penetration	ETC - Rochester, NY	July 14, 2001 June 22, 2001	ETC-01-741-10702.0 ETC-01-741-11008.0	Wendell Haney, P. E. #54158 Joseph Dolden, P.E. #42929
ASTM E283	Air Infiltration	ETC - Rochester, NY	July 14, 2001 June 22, 2001	ETC-01-741-10702.0 ETC-01-741-11008.0	Wendell Hancy, P. E. #54158 Joseph Dolden, P.E. #42929
SSTD 12-99	Large Missile Impact/Cycling	CIL - Orlando, FL	October 3, 2002	CTLA - 980W	Ramesh Patel, P.E. #20224

Design Pressure Ratings:

Configuration Maximum Size Design Pressure Ratings					
Opaque Single	X	Up To 3'0 x 6'8	+ 67.00 - 67.00		
Орацие ощеге		*** \$7.7 . "	Alum Astragal Coastal Alum Astragal		
Onague Double	XX	Up To 6'0 x 6'8	+ 40.00 - 40.00 + 55.00 - 55.00		

Installation and Anchoring: See reverse side this page

- 1. Evaluated for use in locations adhering to the Florida Building Code and where pressure requirements as determined by ASCE 7 Minimum Design Loads for Buildings and Other Structures do not exceed the design pressure ratings listed above.
- 2. For Masonry installations where the sub-buck is less than 1-1/2 inches (FBC section 1707.4.4 Anchorage Methods and subsections 1707.4.4.1 and 1707.4.4.2) same diameter Tapcon type concrete anchors must be substituted and the length must be such that a minimum 1-1/4" engagement of the Tapcon into the masonry wall is obtained.

Certification:

Lyndon F. Schmidt

19506 French Lace Drive

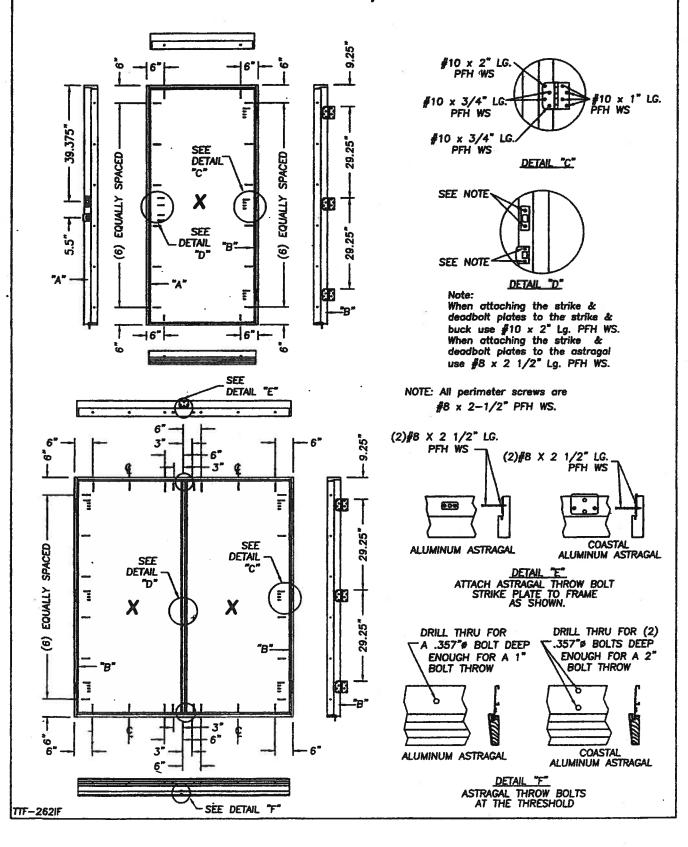
Lutz, FL 33558

Florida Professional Engineer

License No. 43409

December 5, 2002

Therma-Tru® FiberClassic / Smooth Star Out-swing Fiberglass Door Impact Rated Maximum Size Up To 6'0 x 6'8





mentol@mmunityAffairs ode_Information System



Product Type Detail

Product Application

User: Public User - Not Associated with Organization -

Buildina Code

Buildings

tototyp. Building

Liggree

Search,

Mailing

Florida Building

ommission

FBC

Application #: Date Submitted:

Product Manufacturer:

Address/Phone/email:

FL1170

11/20/2003

Therma-Tru Corporation

1687 Woodlands Drive

Maumee, OH 43537

Category:

Subcategory:

Exterior Doors

Swinging

Evaluation Method:

Certification Mark or Listing

Referenced Standards from the Florida Building Code:

Section <u>Standard</u> **ASTM E1996** 1606.1.4 1626.2 PA 201, 203 1707.4 PA 202

> ASTM E 330 1997

Year 2002

1994

1994

Certification Agency:

National Accreditation & Damp;

Management Institute,

Quality Assurance Entity:

Validation Entity:

Authorized Signature:

Steve Jasperson

sjasperson@tttechnologies.us

Evaluation/Test Reports Uploaded: Installation Documents Uploaded:

Product Approval Method:

Method 1 Option A

Application Status:

Date Validated:

Approved

11/20/2003

Page:

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App/Seq #	Product Model # or Name	Model Description	Limits of Use
1170.1	"Classic-Craft"	Fiberglass Door w/wo sidelites	Maximum Design Pressure Rating: Single Door - + 67.0 -67.0 Double Door - + 60.0 - 60.0 (See Door Label for Variations)
1170.2	"Construction Series"	Steel Door w/wo sidelites	Maximum Design Pressure Rating: Single Door - + 67.0 -67.0 Double Door - + 60.0 - 60.0 (See Door Label for Variations)
1170.3	"Fiber-Classic"	Fiberglass Door w/wo sidelites	Maximum Design Pressure Rating: Single Door - + 67.0 -67.0 Double Door - + 60.0 - 60.0 (See Door Label for Variations)
1170.4	"Premium Series"		Maximum Design Pressure Rating: Single Door - + 67.0 -67.0 Double Door - + 60.0 - 60.0 (See Door Label for Variations)
1170.5	"Smooth-Star"	Fiberglass Door w/wo sidelites	Maximum Design Pressure Rating: Single Door - + 67.0 -67.0 Double Door - + 60.0 - 60.0 (See Door Label for Variations)

Next



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The Florida Department of Community Affairs

PRODUCT APPROVAL SITE NAVIGATION

Product Type Detail



















Product Search Overview

Organization

Product Application

User: Public User - Not Associated with Organization -

Need Help?

Application #: Date Submitted: Code Version:

Product Manufacturer: Address/Phone/email:

Wayne-Dalton Corp. 3395 Addison Drive Pensacola, FL 32514 (850) 474-9890

FL3076-R1

02/22/2005

2004

Category:

Subcategory:

Exterior Doors

Sectional Exterior Door Assemblies

Evaluation Method:

Evaluation Report from a Florida Registered Architect or a Licensed Florida Professional Engineer

Referenced Standards from the Florida Building Code:

Section	Standard	Year
103.7	Alternate	2001
	Materials	
	and Methods	
1606	Wind Loads	2001
Chapter	Stuctural	2001
17	Tests and	
	Inspections	
1707.4	Exterior	2001
	Window and	
	Door	
	Assemblies	
1707.4.3	ASTM E-	1997
	330-97	
2204	Cold Formed	2001
	Steel	
	Construction	
Chapter	Steel	2001
22		
1707.4.3.1	DASMA	2002
	108	

	Chapter 26	Foam Plastic 2001
Updated Code Sections when certified to the current Florida Building Code:	2001 Section 103.7 1606 1707.4 1707.4.3 1707.4.3.1 2204 Chapter 26	2004 Section 104.11 1609 1714.5 1714.5.3 1714.5.3.1 2209 2603
Florida Engineer or Architect Name:	Jeffrey P. An	neson
Florida License:	PE- 58544	
Quality Assurance Entity:	Omega Point	Laboratories
Validation Entity:	Dole J. Kelle	у
Authorized Signature:	Wendi Frede	rick
	wfrederick@	wayne-dalton.com
Evaluation/Test Reports Uploaded:	wfrederick@wayne-dalton.com PTID 3076 R1 T 0500 P3.pdf PTID 3076 R1 T 0501 P3.pdf PTID 3076 R1 T 0502 P3.pdf PTID 3076 R1 T 0503 P3.pdf PTID 3076 R1 T 0504 P2.pdf PTID 3076 R1 T 0505 P2.pdf PTID 3076 R1 T 0506 P2.pdf PTID 3076 R1 T 0507 P2.pdf PTID 3076 R1 T 0508 P2.pdf PTID 3076 R1 T 0509 P2.pdf PTID 3076 R1 T 0510 P3.pdf PTID 3076 R1 T 0511 P3.pdf PTID 3076 R1 T 0511 P3.pdf PTID 3076 R1 T 0512 P3.pdf PTID 3076 R1 T 0513 P3.pdf PTID 3076 R1 T 0513 P3.pdf PTID 3076 R1 T 0515 P2.pdf PTID 3076 R1 T 0515 P2.pdf PTID 3076 R1 T 0516 P2.pdf PTID 3076 R1 T 0517 P2.pdf PTID 3076 R1 T 0518 P2.pdf PTID 3076 R1 T 0519 P1.pdf PTID 3076 R1 T 0520 P3.pdf PTID 3076 R1 T 0522 P3.pdf PTID 3076 R1 T 0524 P2.pdf PTID 3076 R1 T 0525 P2.pdf PTID 3076 R1 T 0525 P2.pdf PTID 3076 R1 T 0525 P2.pdf PTID 3076 R1 T 0527 P1.pdf PTID 3076 R1 T 0529 P1.pdf	

PTID 3076 R1 T Cert Ind.pdf PTID 3076 R1 T Evaluation Report.pdf

Installation Documents Uploaded:

Product Approval Method:

Method 1 Option D

Application Status: Approved

Date Validated: 02/24/2005

Date Approved: 03/16/2005

Date Certified to the 2004 Code: 04/27/2005

Page:

Page 1 / 2 > | >|

App/Seq #	Product Model # or Name	Model Description	Limits of Use
3076.1	5500 / 9700 #0500	Thru 9' wide. Design PSF +12.80 / -14.80	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.
3076.2	5500 / 9700 #0501	Thru 9' wide. Design PSF +12.80 / -14.80. Approved with glass in top section.	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.
3076.3	5500 / 9700 #0502	Thru 9' wide. Design PSF +22.90 / -26.30.	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.
3076.4	5500 / 9700 #0503	Thru 9' wide. Design PSF +22.90 / -26.30. Approved with glass in top section.	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.
3076.5	5500 / 9700 #0504	Thru 9' wide. Design PSF +26.90 / -30.80.	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.
3076.6	5500 / 9700 #0505	Thru 9' wide. Design PSF +26.90 / -30.80. Approved with glass in top section.	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.
3076.7	5500 / 9700 #0506	Thru 9' wide. Design PSF +31.20 / -35.80.	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.
3076.8	5500 / 9700 #0507	Thru 9' wide. Design PSF +31.20 / -35.80. Approved with glass in top	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.

		section.	
3076.9	5500 / 9700 #0508	Thru 9' wide. Design PSF +35.70 / -41.00.	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.
3076.10	5500 / 9700 #0509	Thru 9' wide. Design PSF +45.30 / -51.20.	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.
3076.11	5500 / 9700 #0510	10' wide. Design PSF +12.80 / - 14.80.	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.
3076.12	5500 / 9700 #0511	10' wide. Design PSF +12.80 / - 14.80. Approved with glass in top section.	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.
3076.13	5500 / 9700 #0512	10' wide. Design PSF +19.20 / - 22.00.	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.
3076.14	5500 / 9700 #0513	10' wide. Design PSF +19.20 / - 22.00. Approved with glass in top section.	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.
3076.15	5500 / 9700 #0514	10' wide. Design PSF +22.90 / - 26.30.	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.
3076.16	5500 / 9700 #0515	10' wide. Design PSF +22.90 / - 26.30. Approved with glass in top section.	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.
3076.17	5500 / 9700 #0516		As indicated in evaluation report and installation drawings. Not to be used in HVHZ.
3076.18	5500 / 9700 #0517	with class in toh	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.
3076.19	5500 / 9700 #0518	10' wide. Design	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.
3076.20	5500 / 9700 #0519	46 30	As indicated in evaluation report and installation drawings. Not to be used in HVHZ.

Next

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WINDLOAD SPECIFICATION 0125

MODEL:

8300, 8500, 5150 & 5200 16'-0" MAX WIDTH X 16'-0" MAX HEIGHT

STYLE:

RAISED PANEL

TEST PRESSURE: DESIGN PRESSURE: POS. 33.0 PSF/NEG. 37.0 PSF

POS. 22.0 PSF/NEG. 24.66 PSF

NOTES:

1. VINYL OR WOOD DOOR STOP (NAILED ON 6" CENTERS) MUST OVERLAP TOP AND BOTH ENDS OF PANELS MINIMUM 7/16" TO MEET NEGATIVE PRESSURÉS.

- 2. U-BAR TO HAVE A MINIMUM YIELD OF 80 KSI
- 3. TRACK STEEL TO HAVE A MINIMUM YIELD OF 33 KSI.
- 4. DOOR STEEL TO HAVE A MINIMUM YIELD OF 50.5 KSI.
- 5. OPTIONAL .080" (MIN) ALUMINUM LOUVERS WITH HIGH IMPACT STYRENE FRAME MAY BE LOCATED IN THE END PANELS OF THE BOTTOM SECTION.
- 6. LOCK OR OPERATOR IS REQUIRED.
- 7. THE DESIGN OF THE SUPPORTING STRUCTURAL ELEMENTS SHALL BE THE RESPONSIBILITY OF THE PROFESSION'AL OF RECORD FOR THE BUILDING OR STRUCTURE AND IN ACCORDANCE WITH CURRENT BUILDING CODES FOR THE LOADS LISTED ON THIS DRAWING.
- 8. DOOR JAMB TO BE 2X6 STRUCTURAL GRADE LUMBER. (MIN)

W.S. Wilson, P.E. 3395 ADDISON DR., PENSACCIA, FL 32514 FLORIDA CERTIFICATION NO. 0048489 GEORGIA CERTIFICATION NO. 018519 NORTH CAROLINA CERTIFICATION NO. 023836

7-24-02

13 GA HORIZ ANGLE 2" HORIZ TRACK (4) 1/4-20x9/16" TRACK BOLT & (4) 1/4-20 HEX NUT DRILL .281 OR 9/32"ø HOLE IN TRACK FOR JB-US BRACKET **ATTACHMENT** (1) JB-US BRACKET LOCATED -AT EACH ROLLER LOCATION EXCEPT TOP BRACKET ROLLER (P/N 125139) (1) JB-US BRACKET LOCATED AT THE MIDDLE OF EACH SECTION EXCEPT TOP SECTION 5/16x1-5/8" LAG SCREW AT EACH JB-US BRACKET .067" VERT TRACK-**©** 1/4-20x9/16" TRACK-BOLT & 1/4-20 HEX

APPROVED SIZES:

MAXIMUM SECTION WIDTH 24"

Approved:

Š

OPTION CODE: 0125

REV: P3

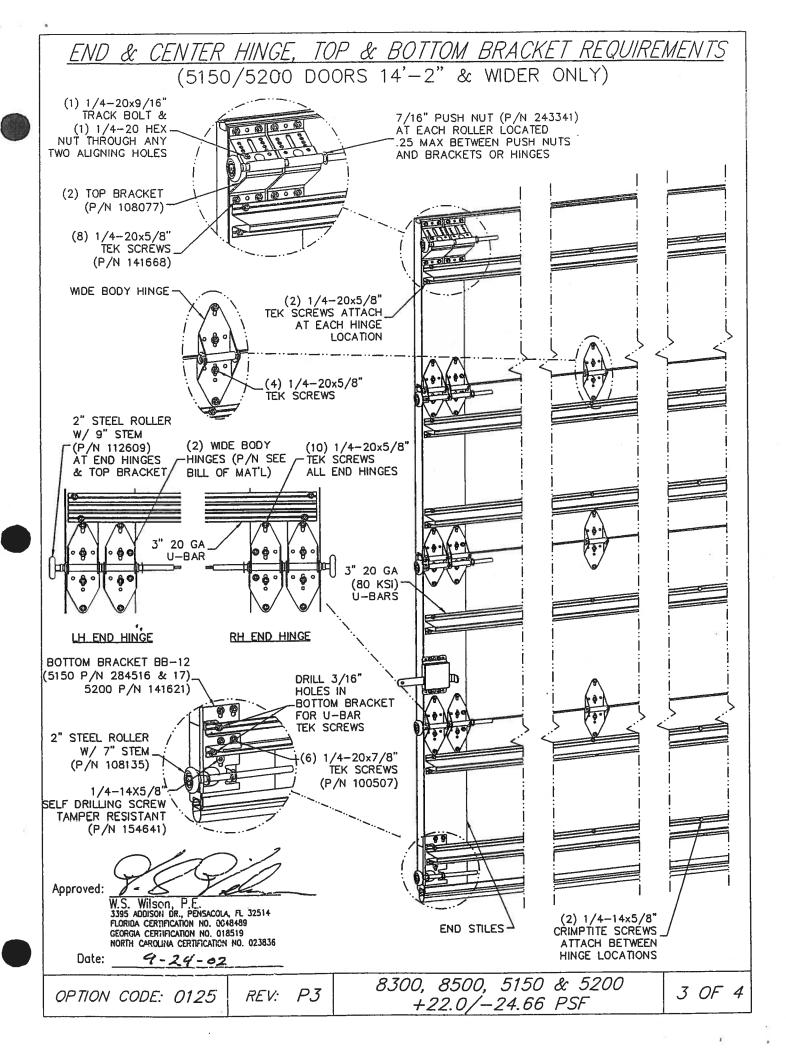
8300, 8500, 5150 & 5200 +22.0/-24.66 PSF

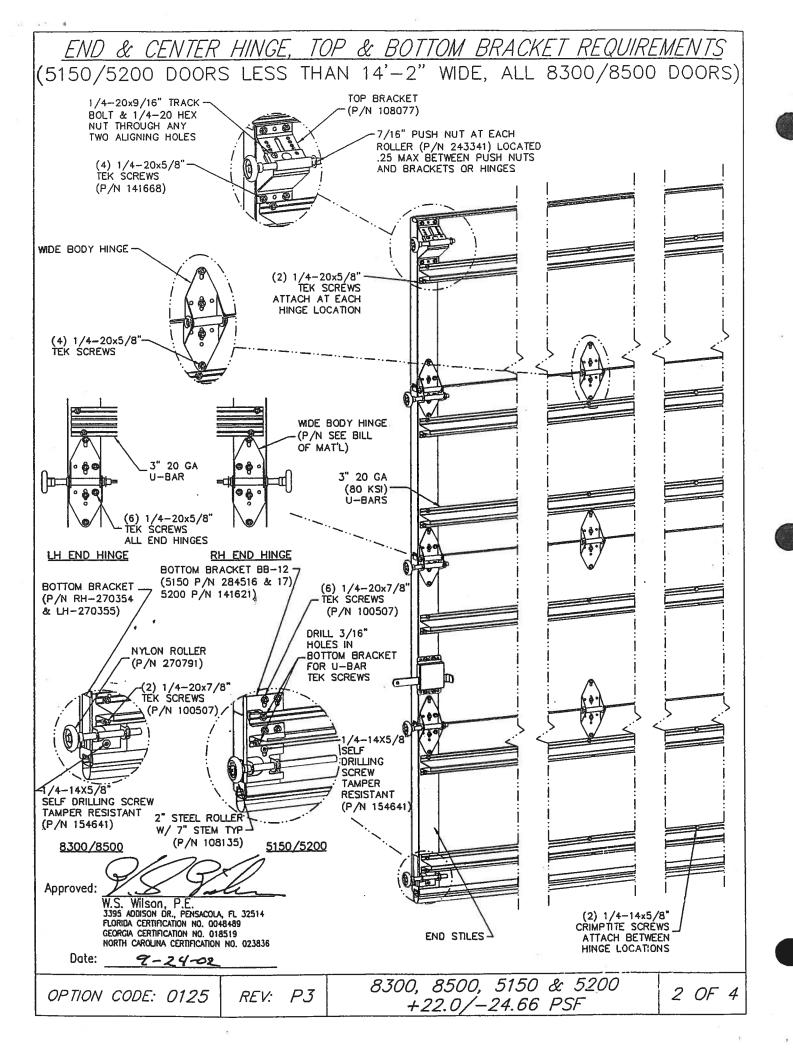
BRACKET

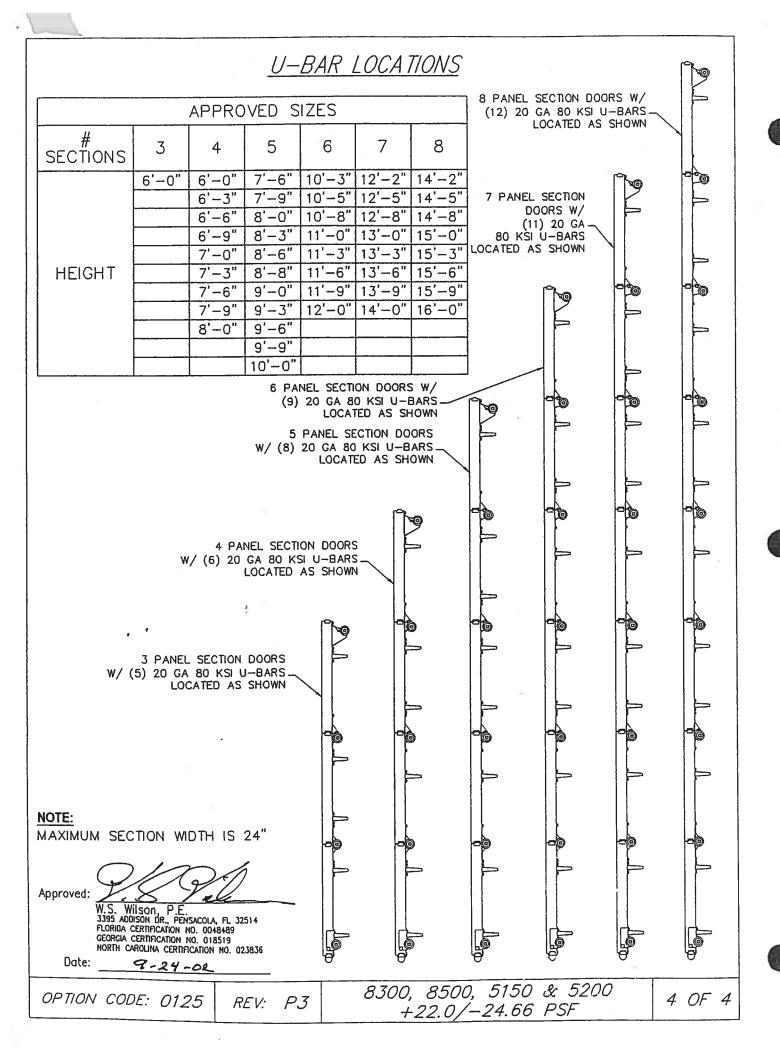
NUT AT EACH JB-US

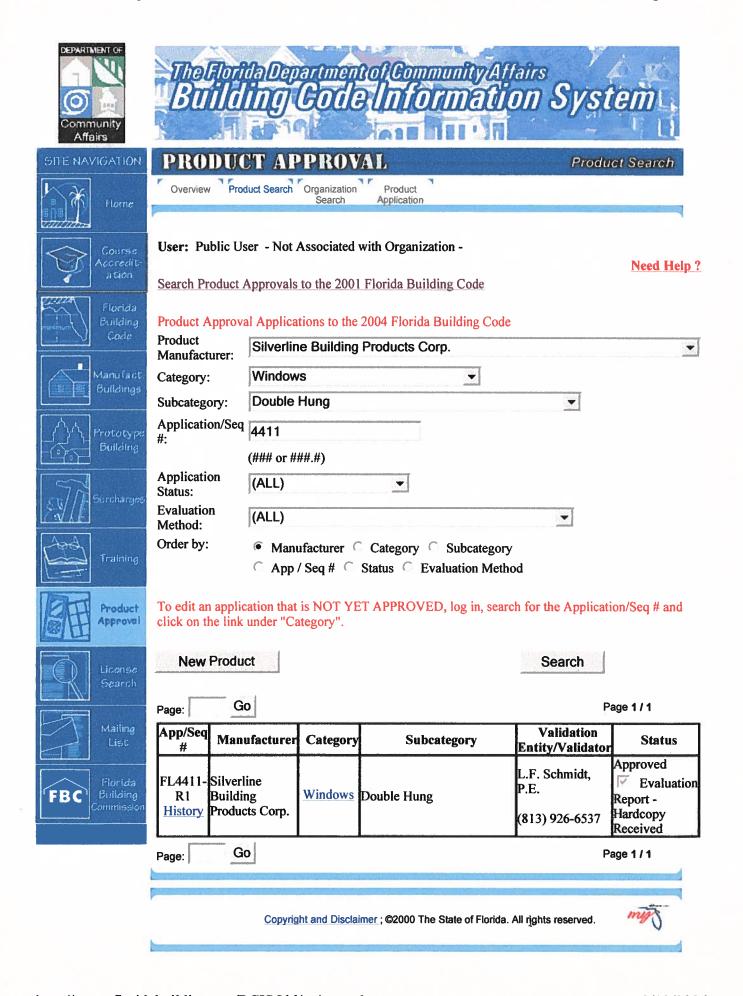
1 OF 4

-1/2"











Florida Department of Community Affairs



PRODUCT APPROVAL

Product Type Detail



Product Search Organization **Product** Overview Application

Accredit

User: Public User - Not Associated with Organization -

Need Help?



Florida Code

Manufact













FL4411-R1 Application #: Date Submitted: 07/26/2005 2004 Code Version:

Product Manufacturer: Silverline Building Products Corp.

Address/Phone/email: One Silverline Drive

North Brunswick, NJ 08902

(732) 435-1000

Windows Category:

Double Hung Subcategory:

Evaluation Report from a Florida **Evaluation Method:**

Registered Architect or a Licensed Florida

Professional Engineer

Referenced Standards from the Florida

Building Code:

Section

Standard Year ANSI/AAMA/NWWDA 1997

101 I.S.2 **ASTM E1300**

2002 2004 **Accepted Engineering**

Practice

Florida Engineer or Architect Name:

Wendell W. Haney

Florida License:

PE-54158

Quality Assurance Entity:

National Accreditation and Management

Institute

L.F. Schmidt, P.E. Validation Entity:

Pete Thornton Authorized Signature:

rickw@rwbldgconsultants.com

Evaluation/Test Reports Uploaded:

PTID 4411 R1 T 4411.1 EVAL.pdf

PTID_4411_R1_T_4411.1 INST.pdf

Installation Documents Uploaded:

Product Approval Method:

Method 1 Option D

Application Status:

Approved

Date Validated:

09/04/2005

Date Approved:

09/06/2005

Date Certified to the 2004 Code:

9500 Series, Model 9500

Tilt Double Hung

Page: Go

4411.1

Page 1 / 1

impact resistant covering

App/Seq	Product Model # or	Model	Limits of Use
#	Name	Description	
	• 100	Extraded Vipul	This product meets the requirements for the State of Florida excluding the "HVHZ". When used in wind-borne debris regions this product is required to be protected with an

that complies with Section Window - "Non-1609.1.4 of the Florida Impact" Building Code. Maximum Design Pressure Rating -Positive 45.0 PSF and Negative 45.0 PSF (see 4411.1 EVAL for any additional size and use limitations).

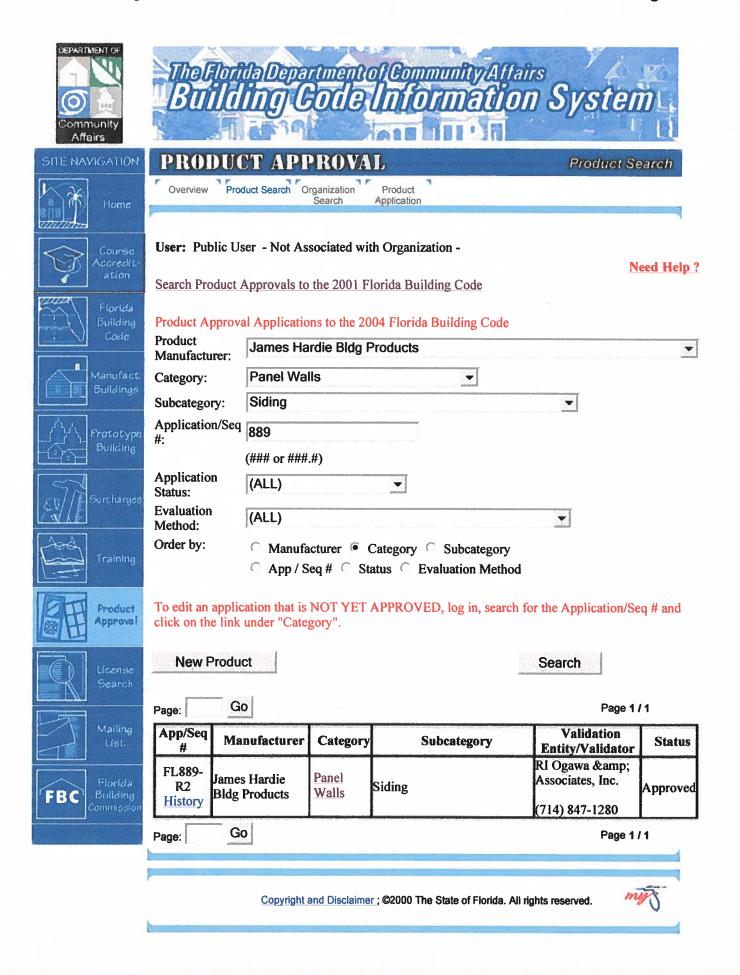
Next

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Extruded Vinyl

Tilt Double Hung







The Florida Department of Community Affairs 1 Britleting Code Intormation System

Product

Community Affairs SITE NAVIGATION

PRODUCT APPROVAL

Product Type Detail

Home

Florizis Building Code

Manufact Buildinge

Building

Training

Paritical Problem

Mailing List

Search

FBC Florida Building Commission Application

User: Public User - Not Associated with Organization -

Application #:

Date Submitted:

Product Manufacturer:

Address/Phone/email:

Category:

Subcategory:

Evaluation Method:

Evaluation Entity:

Quality Assurance Entity:

Validation Entity:

Authorized Signature:

Evaluation/Test Reports Uploaded: Installation Documents Uploaded:

Product Approval Method:

Application Status:

Date Validated:

FL889

11/03/2003

James Hardie Bldg Products

10901 Elm Avenue Fontana, CA 92337 (909) 356-6366

New/Innovative Envelope

Products

Other

claddings & amp; amp; a

Evaluation Report from a Product

Evaluation Entity

Referenced Standards from the Florida Building Code: Section Standard Year

National Evaluation Service, Inc.

Intertek Testing Services-ETL/Warnock Hersey

Inspection Concepts, Inc.

john mulder

jlm@jameshardie.com

PTID 889 T ner405.pdf

Method 2 Option A

Approved 11/18/2003





PRODUCT APPROVAL

Product Type Detail

Product Search Organization

Product Application



User: Public User - Not Associated with Organization -

Search

Need Help?

Florida

Manufact













FL889-R2 Application #: 11/10/2005 Date Submitted: 2004 Code Version:

Product Manufacturer:

10901 Elm Avenue Address/Phone/email: Fontana, CA 92337 (909) 356-6366

john mulder Technical Representative:

Technical Representative Address/Phone/email: 10901 elm avenue

fontana, CA 92337 (909) 356-6366 jlm@jameshardie.com

James Hardie Bldg Products

Panel Walls Category:

Subcategory: Siding

Evaluation Method: Evaluation Report from a Product

Evaluation Entity

Referenced Standards from the Florida Building Code: Section Standard Year 1405.15 **ASTM C1186** 1999

R703.10 **ASTM C1186** 1999

Evaluation Entity: ICC Evaluation Service, Inc.

Intertek Testing Services-Quality Assurance Entity: ETL/Warnock Hersey

Validation Entity: RI Ogawa & Samp; Associates, Inc.

iohn mulder Authorized Signature:

jlm@jameshardie.com

Evaluation/Test Reports Uploaded: PTID 889 R2 T ASCE 7-02

wind load calculation.pdf PTID_889_R2_T_ner-405 (April 2004).pdf PTID_889_R2_T_NOA No 02-0729-.02.pdf

Installation Documents Uploaded:

Product Approval Method:

Method 1 Option C

Application Status: Date Validated:

Approved 11/10/2005

Date Approved:

12/07/2005

Date Certified to the 2004 Code:

Page:

Go

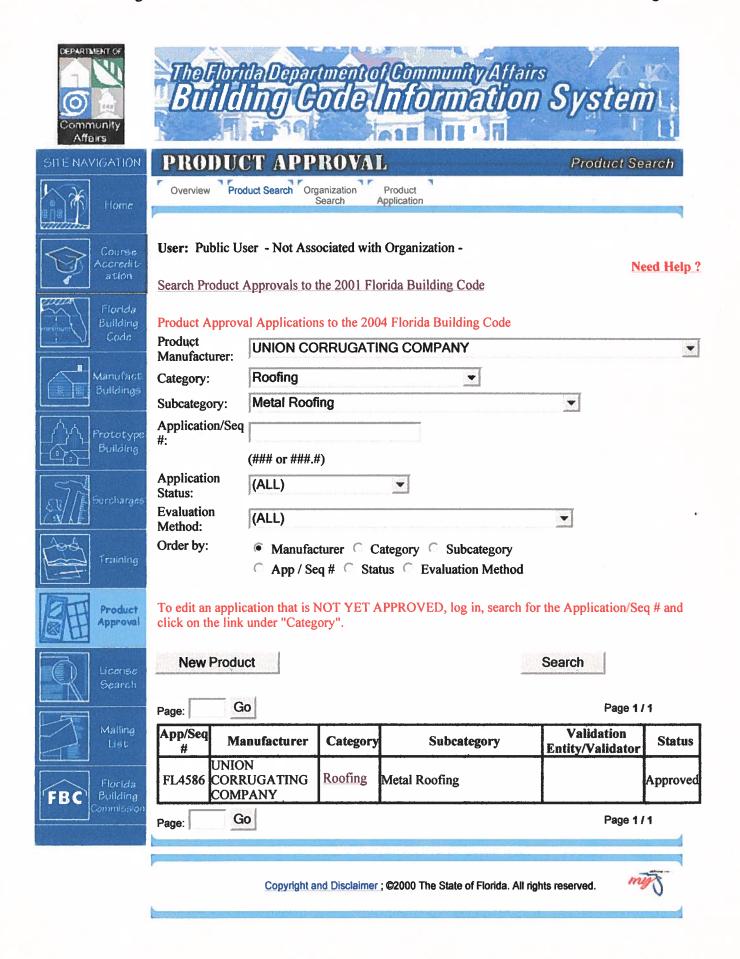
Page 1 / 1

App/Seq #	Product Model # or Name	Model Description	Limits of Use
889.1	Cempanel siding	fiber-cement cladding	For use in HVHZ install in accordance with NOA 02-0729-02
889.2	Cemplank lap siding	fiber-cement cladding	For use in HVHZ install in accordance with NOA 02- 0729-02
889.3	Cemsoffit panel	fiber-cement cladding	For use in HVHZ install in accordance with NOA 02- 0729-02
889.4	Hardipanel siding	fiber-cement cladding	For use in HVHZ install in accordance with NOA 02- 0729-02
889.5	Hardiplank lap siding	fiber-cement cladding	For use in HVHZ install in accordance with NOA 02-0729-02
889.6	Hardishingle cladding shingle	fiber-cement cladding	Not for use in HVHZ
889.7	Hardishingle notched panel	fiber-cement cladding	Not for use in HVHZ
889.8	Hardisoffit panel	fiber-cement cladding	For use in HVHZ install in accordance with NOA 02- 0729-02
889.9	Harditex baseboard	fiber-cement cladding	For use in HVHZ install in accordance with NOA 02-0729-02
889.10	Sentry lap sidig	fiber-cement cladding	For use in HVHZ install in accordance with NOA 02-0729-02

Next

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Product Approval

FBC



PRODUCT APPROVAL

Product Type Detail

Product Search Organization **Product** Search Application

User: Public User - Not Associated with Organization -

Need Help?

FL4586 Application #: 06/03/2005 Date Submitted: Code Version: 2004

Product Manufacturer: UNION CORRUGATING COMPANY

Address/Phone/email: 701 S. KING ST.

FAYETTEVILLE, NC 28301

(910) 483-0479

Technical Representative: Dave Hart Technical Representative Address/Phone/email: 701 S King St

Fayetteville, NC 28303

(910) 483-0479

dhart@unioncorrugating.com

Roofing Category:

Metal Roofing Subcategory:

Evaluation Method: Certification Mark or Listing

Referenced Standards from the Florida **Section Standard** Year **UL 580** 1994 **Building Code:** 1504.3.2

Underwriters Laboratories Inc. Certification Agency:

Quality Assurance Entity:

Validation Entity:

Glenn Hart Authorized Signature:

dhart@unioncorrugating.com

Evaluation/Test Reports Uploaded: PTID 4586 I 5VPanelProfile.pdf Installation Documents Uploaded:

PTID 4586 I AdLokPanelProfile.pdf

PTID_4586_I_MasterRibPanelProfile.pdf PTID_4586_I_Trim Installation.pdf

Product Approval Method:

Method 1 Option A

Application Status:

Approved

Date Validated:

06/03/2005

Date Approved:

06/29/2005

Date Certified to the 2004 Code:

Page:

Page 1 / 1

Go

App/Seq #	Product Model # or Name	Model Description	Limits of Use
4586.1	5∨	Min 29 ga. Through Fastened Metal Roof	UL Construction 579. Not for use in HVHZ. Design Pressure = 52.5 psf. Increased Design pressures at perimeter and corner areas, in compliance with FBC Chapter 16, may be met through rational analysis.
4586.2	Advantage Lok		UL Construction 529. Not for use in HVHZ. Design Pressure = 52.5 psf. Increased Design pressures at perimeter and corner areas, in compliance with FBC Chapter 16, may be met thorugh rational analysis.
4586.3	MasterRib	Min 29 ga. Through Fastened Metal Roof	UL Construction 584. Not for use in HVHZ. Design Pressure = 52.5 psf. Increased Design pressures at perimeter and corner areas, in compliance with FBC Chapter 16, may be met thorugh rational analysis.

Next

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The Florida Department of Community Affairs Building Gode Information System

PRODUCT APPROVAL

Product Type Detail

Overview Produc

Product Search Organization Search

Product Application

User: Public User - Not Associated with Organization -

Need Help?

Application #:
Date Submitted:
Code Version:

FL1730-R1 09/21/2005 2004

Product Manufacturer: Address/Phone/email:

Hurri-Bolt, Inc. 10704 N 46th Street Tampa, FL 33617

(813) 626-1676

Technical Representative:

Joe Hale

Technical Representative Address/Phone/email:

2720 N 46th Street Tampa, FL 33605 (813) 626-1676

hbolt@tampabay.rr.com

Category:

Structural Components

Subcategory:

Wood Connectors Anchors

Evaluation Method:

Evaluation Report from a Product Evaluation

Entity

License Search

FBC

burcharges

Product Approval

Referenced Standards from the Florida Building Code:

SectionStandardYear2104.9.5ASTM A 3619941606.1.1SBCCI Standard for Hurricane
Resistant SSTD10-991999

 1706.1
 ASTM D1761
 1988

 1706.3.1
 ASTM D1761
 1988

 2314.4.4
 ASTM D1761
 1988

Evaluation Entity:

SBCCI PST and ESI

Quality Assurance Entity:

PFS Corporation

Validation Entity:

R. D. Hall

Authorized Signature:

Joseph Hale

jhale@boltandnut.com

Evaluation/Test Reports Uploaded:

PTID_1730_R1_T_FL1730 Installation

Instruction.pdf

PTID 1730 R1 T fl1730Ind cert.pdf

PTID 1730 R1 T PE Cert Ind FL1730Rev1.pdf

PTID 1730 R1 T SBCCI9910A.pdf

Installation Documents Uploaded:

PTID_1730_R1_I_HW38Instructionsf11730.pdf

Product Approval Method:

Method 1 Option C

Application Status:

Denied

Date Validated:

11/21/2005

Date Approved:

Date Certified to the 2004 Code:

Page:

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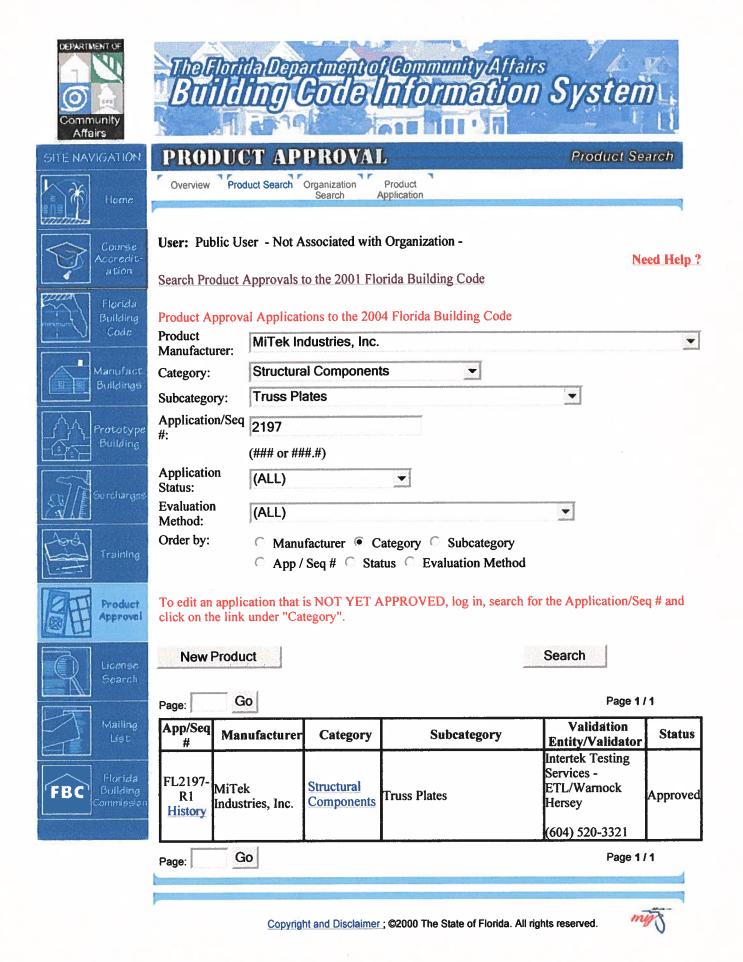
App/Seq #	Product Model # or Name	Model Description	Limits of Use
1730.1	1/2" Hurri-Bolt Assembly	Hurri-Bolt Top Plate Assembly 1/2" Diameter	Detailed Limitations are listed in SBCCI Report #9910A (Uploaded) The Design Capacity of the 1/2" Hurri-Bolt assembly is 3905 lbs. at 1-3/4" min. centerline distance to free edge of slab with 3000 psi normal weight concrete with 1/2" Hurri-Wedge Anchor. Size top plate washer according to bearing capacity of wood species.
1730.2	3/4" Hurri-Bolt Assembly	Hurri-Bolt Top Plate Assembly 3/4" Diameter	Detailed Limitations are listed in SBCCI Report #9910A (Uploaded) The Design Capacity of the 3/4" Hurri-Bolt assembly is 10,000 lbs. at 1-3/4" min. centerline distance to free edge of slab with 3000 psi normal weight concrete with 7/8" threaded stud with Ultrabond 1 epoxy and 7/8" to 3/4" reducer coupler. Size top plate washer according to bearing capacity of wood species.
			Detailed Limitations are listed in SBCCI Report #9910A (Uploaded) The Design Capacity of the

1730.3	3/8" Hurri-Bolt Assembly	Hurri-Bolt Top Plate Assembly 3/8" Diameter	3/8" Hurri-Bolt assembly is 2400 lbs. at 1-3/4" min. centerline distance to free edge of slab with 3000 psi normal weight concrete with HBA anchor and a 1/2" to 3/8" reducer coupler. Size top plate washer according to bearing capacity of wood species.
1730.4	5/8" Hurri-Bolt Assembly	Hurri-Bolt Top Plate Assembly 5/8" Diameter	Detailed Limitations are listed in SBCCI Report #9910A (Uploaded) The Design Capacity of the 5/8" Hurri-Bolt assembly is 7,050 lbs. at 1-3/4" min. centerline distance to free edge of slab with 3000 psi normal weight concrete with 3/4" threaded stud with Ultrabond 1 epoxy at 9" embedment and a 3/4" to 5/8" reducer coupler. Size top plate washer according to bearing capacity of wood species.

Next

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The Florida Department of Community Affairs

PRODUCT APPROVAL

Product Type Detail

Product Search Organization Overview

Product Application Search

User: Public User - Not Associated with Organization -

Need Help?

Application #: Date Submitted: Code Version:

FL2197-R1 09/14/2005 2004

Product Manufacturer: Address/Phone/email:

MiTek Industries, Inc.

14515 North Outer Fourty Drive

Suite 300

Chesterfield, MO 63017--574

(314) 851-7480



Technical Representative:

David Wert

Technical Representative Address/Phone/email:

14515 North Outer Fourty Drive

Suite 300

Chesterfield, MO 63017-5746

(314) 851-7480 dwert@mii.com



Category:

Structural Components



Subcategory:

Truss Plates



Evaluation Method:

Evaluation Report from a Product Evaluation Entity



FBC

Referenced Standards from the Florida Building Code:

Section Standard Year 2302.4. ANSI/TPI 1995 2319.17.2.1.

2302.4, ANSI/TPI 2002 2319.17.2.1. 1



ICC Evaluation Service, Inc.

Quality Assurance Entity:

Intertek Testing Services-ETL/Warnock Hersey

Validation Entity:

Intertek Testing Services -ETL/Warnock Hersey

Authorized Signature:

David Wert

david.c.wert@mii.com

Evaluation/Test Reports Uploaded:

PTID 2197 R1 T 95-43.01.pdf PTID 2197 R1 T 9604b.pdf PTID 2197 R1 T ESR-

1311.pdf

PTID 2197 R1 T ESR-

1352.pdf

PTID 2197 R1 T ICC ES Certificate of Independence.pdf PTID 2197 R1 T NOA 02-

042902.pdf

PTID 2197 R1 T NOA 02-

042910.pdf

Installation Documents Uploaded:

Product Approval Method:

Method 1 Option C

Application Status: Date Validated:

Approved 09/22/2005

Date Approved:

10/11/2005

Date Certified to the 2004 Code:

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Go

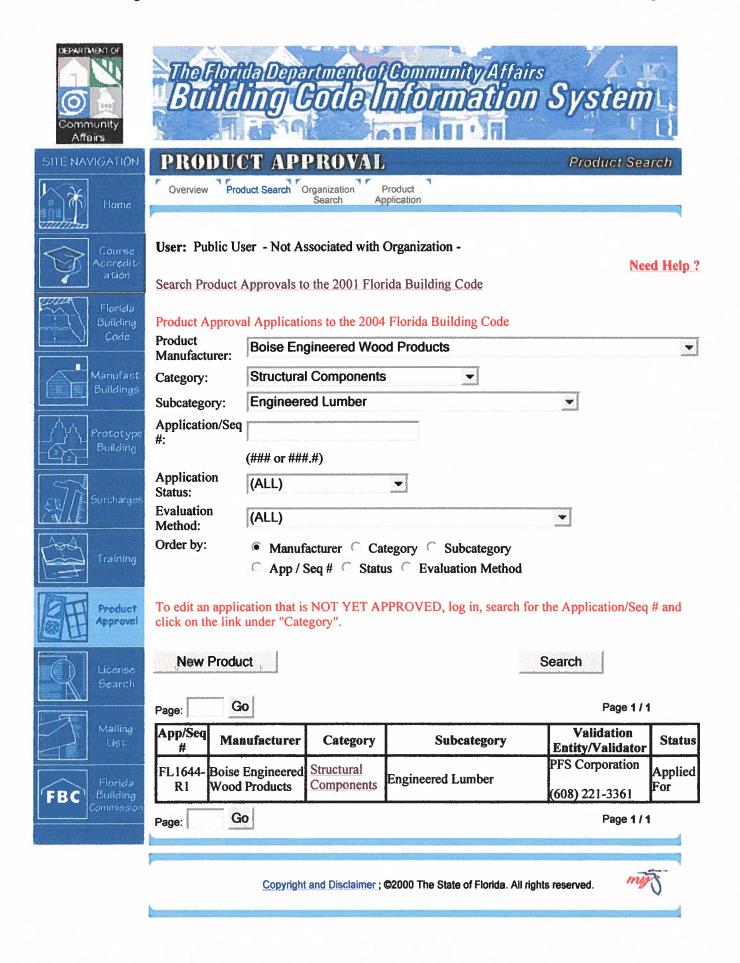
Page 1 / 1

App/Seq #	Product Model # or Name	Model Description	Limits of Use
2197.1	MT16	16 ga. truss connector plate	Not for use in HVHZ
2197.2	MT18	18 ga. truss connector plate	Can be used for HVHZ per NOA 02-0429.02
2197.3	MT20	20 ga. truss connector plate	Can be used for HVHZ per NOA 02-0429.10
2197.4	MT20HS	20 ga. high strength truss connector plate	Not for use in HVHZ

Next

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The Florida Department of Community Affairs

SITE NAVIGATION

Product Approval

FBC

PRODUCT APPROVAL Product Type Detail

Organization Product Search **Product** Overview Application

User: Public User - Not Associated with Organization -

Need Help?

Application #: FL1644-R1 Date Submitted: 11/01/2005 Code Version: 2004

Product Manufacturer: **Boise Engineered Wood Products**

Address/Phone/email: PO Box 2400

White City, OR 97503 (541) 826-0207

Category: Structural Components

Engineered Lumber Subcategory:

Evaluation Method: Evaluation Report from a Product

Evaluation Entity

Referenced Standards from the Florida Building Code: Section Standard Year

ASTM D 5456 2000

Evaluation Entity: ICC Evaluation Service, Inc.

Quality Assurance Entity: **PFS Corporation**

PFS Corporation Validation Entity:

Authorized Signature: Dan Cheney

dancheney@boisebuilding.com

Evaluation/Test Reports Uploaded: PTID 1644 R1 T 1040.pdf

Installation Documents Uploaded:

Product Approval Method: Method 1 Option C

Application Status: Applied For

Date Validated: Date Approved: Date Certified to the 2004 Code:

Page:			Page 1 / 1
App/Seq #	Product Model # or Name	Model Description	Limits of Use
1644.1	Versa-Lam	Laminated Veneer Lumber	Floor, Roof, and Wall Framing

Next

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NOTICE OF TREATMENT

Applicator Name	14 CH 2	or or a	
Address	Cold Mis	V' S.	Such 5
City	Coronle	11.	
Time	$\underline{-\sqrt{(x_1,0)}c_{x_1}\sqrt{N}}$	_ Date <u></u>	.10(-(16)-
	SITE LOCA	TION 2	4204
Lot #	Block #	Permit	#
Address	54 MULL	r. 7+	11/1/e
Name of Chemica	l Applied Nove C		Used <u>33</u> %
Area Treated	1510		
Gallons Used	1		
Remarks			
Applicator - White	e Permit File - Ca	anary F	Permit Holder - Pink M 1043



OCCUPANCY

COLUMBIA COUNTY, FLORIDA

tment of Building and Zoning

and premises at the below named location, and certifies that the work has been completed in This Certificate of Occupancy is issued to the below named permit holder for the building accordance with the Columbia County Building Code.

Parcel Number 18-7S-16-04236-062 Building permit No. 000024204

Use Classification SFD, UTILITY Fire:

Fire: 0.00

Waste: 0.00

Owner of Building WILLIAM & JOYCE CARTER

Location:

968 SW BLUFF DRIVE, FT. WHITE, FL

Date: 10/26/2006

Permit Holder FREDRICK HAMMOND

Hamy Brieke

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)

Bill Carter HVAC Load Calculations

for

Fredrick G Hammond P.O Box 1201 Newberry FI 32669



Prepared By:
Chuck Fischer
North Central Florida Air Conditioning
P.O Box 700
High Springs Fl 32655-0700
386-454-4767
Tuesday, February 07, 2006

Project Report

General Project Information

Project Filename: C:\Documents and Settings\Heat\My Documents\Projects\AutoLoad MJ8.rhv

Project Title: Bill Carter

Designed By: Chuck Fischer

Project Date: February 6th 2006

Client Name: Fredrick G Hammond

Client Address: P.O Box 1201
Client City: Newberry FI 32669
Client Phone: 352-283-0000

Client Comment:

Company Name: North Central Florida Air Conditioning

Company Representative: Chuck Fischer Company Address: P.O Box 700

Company City: High Springs FI 32655-0700

Company Phone: 386-454-4767 Company Fax: 386-454-4854

Company Comment: Bedroom 2&3 R/A are 10x10x8 Master bedroom R/A is 12x12x9 Main R/A is 20x24x18

Design Data

Reference City: Gainesville, Florida

Daily Temperature Range: Medium Latitude: 29 Degrees

Elevation: 152 ft. Altitude Factor: 0.995

Elevation Sensible Adj. Factor: 1.000
Elevation Total Adj. Factor: 1.000
Elevation Heating Adj. Factor: 1.000
Elevation Heating Adj. Factor: 1.000

Outdoor **Grains** Outdoor Indoor Indoor Dry Bulb Wet Bulb Rel.Hum **Dry Bulb** Difference Winter: 31 0 0 68 0 75 93 77 50 50 Summer:

Check FiguresTotal Building Supply CFM:462CFM Per Square ft.:0.458Square ft. of Room Area:1,009Square ft. Per Ton:921Volume (ft³) of Cond. Space:8,794Air Turnover Rate (per hour):3.2

Building Loads

Total Heating Required With Outside Air: 25,760 Btuh 25.760 MBH Total Sensible Gain: 10,119 Btuh 81 % Total Latent Gain: 2,445 Btuh 19 %

Total Cooling Required With Outside Air: 12,564 Btuh 1.05 Tons (Based On Sensible + Latent)

1.10 Tons (Based On 77% Sensible Capacity)

Notes

Calculations are based on 8th edition of ACCA Manual J.

All computed results are estimates as building use and weather may vary.

Rhvac - Residential & Light North Central Florida A/C Inc High Springs, FL 32643		VAC Loads			Elite Software D	evelopment, inc Bill Carter Page 3
Miscellaneous F	Report					
System 1 Main Floor		Outdoor	Outdoor	Indoor	indoor	Grains
Input Data		Dry Bulb	Wet Bulb	Rel.Hum	Dry Bulb	Difference
Winter:		31	Q	50	68	30.84
Summer:		93	77	50	75	50.06
Duct Sizing Inputs			ing the 200 states of the first one (figures).			
	Main Trunk		Runouts			
Calculate:	Yes		Yes			
Use Schedule:	No		No			
Roughness Factor:	0.00300		0.01000			
Pressure Drop:	0.1000	in.wg./100 ft.		in.wg./100 ft.		
Minimum Velocity:	650	ft./min		ft./min		
Maximum Velocity:	900	ft./min	750	ft./min		
Minimum Height:	0	in.	0	in.		
Maximum Height:	0	in.	0	in.		
Outside Air Data						
		Winter		<u>mmer</u>		
Infiltration:		0.900 AC/hr		0.400 AC/hr		
Volume of Conditioned	Space:	X 8794 Cu.ft.		8794 Cu.ft.		
		7,915 Cu.ft./h		3,518 Cu.ft./hr		
		<u> </u>	<u>X 0</u>	.0167		
Total Building Infiltration		132 CFM		59 CFM		
Total Building Ventilatio	n:	0 CFM		0 CFM		

19.69 = (1.10 X 0.995 X 18.00 Summer Temp. Difference) 33.85 = (0.68 X 0.995 X 50.06 Grains Difference) 40.48 = (1.10 X 0.995 X 37.00 Winter Temp. Difference)

---System 1--Infiltration & Ventilation Sensible Gain Multiplier:
Infiltration & Ventilation Latent Gain Multiplier:
Infiltration & Ventilation Sensible Loss Multiplier:

Rhyac : Residen North Central Flo High Springs, FL	ida A/C Inc	mercial HVAC I	oads				Elite Softv	vare Develop	oment, Inc Bill Carter Page 4
Load Prev	view Repo	rt							
		Sens	Lat	Net	Sens	Win	Sum	Sys	Duct
Scope	Area	Gain	Gain	Gain	Lóss	CFM	CFM	CFM	Size
Building: 1.05	Net Tons, 1.1	0 Recommer	ded Tons,	921 ft.3/Ton,	25,76 MBH I	leating		Military III	powers.
Building	1,009	10,119	2,445	12,564	25,760	336	462	462	
System 1: 1.05	Net Tons, 1	.10 Recomme	ended Tons,	921 ft.2/Ton,	25.76 MBH	Heating			i de la compa
System 1	1,009	10,119	2,445	12,564	25,760	336	462	462	11x10
Zone 1	1,009	10,119	2,445	12,564	25,760	336	462	462	
1-Garage	720	6,856	1,705	8,561	16,108	210	313	313	3-6
2-Storage	289	3,262	740	4,002	9,652	126	149	149	2-5

Rhvac - Residential & Light Commercial HVAC Loads North Central Florida A/C inc High Springs, FL 32643)		Elke Sc	iffware Develo	Bill Carter Page 5
Total Building Summary Loads					
Component	Area	Şen	Lat	Sën	Total
Description	Quạn	Loss	Gain	Gain	Gain
1D-cb-o: Glazing-Double pane, operable window, clear, metal frame with break, ground reflectance = 0.1, outdoor insect screen with 50% coverage, external shade screen coefficient of 0.45 and 50% coverage	25	600	0	576	576
11P: Door-Polyurethane Core	145.7	1,563	0	1,225	1,225
12B-4sw: Wall-Frame, R-11 insulation in 2 x 4 stud cavity, R-4 board insulation, siding finish, wood studs	1379	3,725	0	2,260	2,260
16C-30: Roof/Ceiling-Under attic or knee wall, Vented Attic, No Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel or Membrane, R-30 insulation	1009.2	1,194	0	1,389	1,389
22A-ph: Floor-Slab on grade, No edge insulation, no insulation below floor, any floor cover, passive, heavy moist soil	180	9,045	0	0	0
Subtotals for structure:		16,127	0	5,450	5,450
People:	2		460	600	1,060

Total Building Supply CFM: 462			CFM Per	r Square ft.:	0.458	
Square ft. of Room Area:	1,009			Square f	t. Per Ton:	921
Volume (ft³) of Cond. Space:	8,794			Air Turno	over Rate (per hour):	3.2
Building Loads	PERSONAL PROPERTY OF THE PROPE	energy of the state of the stat		relatively relativistic commendate of many relativistic commendate of the		
Total Heating Required With Outside A	ir:	25,760	Btuh	25.760	MBH	
Total Sensible Gain:		10,119	Btuh	81	%	
Total Latent Gain:		2,445	Btuh	19	%	
Total Cooling Required With Outside Ai	ir:	12,564	Btuh	1.05	Tons (Based On Sensible	+ Latent)
•		•			Tons (Based On 77% Sen	

360

4,294

5,339

25,760

0

1,228

1,687

1,154

10,119

1,985

2,445

1,228

1,687

3,139

12,564

0

Notes

Equipment:

Lighting:

Ductwork:

Calculations are based on 8th edition of ACCA Manual J.

Infiltration: Winter CFM: 132, Summer CFM: 59

Ventilation: Winter CFM: 0, Summer CFM: 0

Total Building Load Totals:

All computed results are estimates as building use and weather may vary.

Rhyan .	Residential ntral Florida	A Light Co	mmercial	HVAC I	oade"
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Nonn Ce	nual longa	AU Inc			

High Springs, EL 32643



Elite Software Development, Inc. Bill Certer Page 6

System 1 Main Floor Summary Loads (Average Method)

Component	Area	Sen	Lat	Sen	Total
Description	Quan	Loss	Gain	Gain	Gain
D-cb-o: Glazing-Double pane, operable window, clear, metal frame with break, ground reflectance = 0.1, outdoor insect screen with 50% coverage, external shade screen coefficient of 0.45 and 50% coverage	25	600	0	576	576
11P: Door-Polyurethane Core	145.7	1,563	0	1,225	1,225
12B-4sw: Wall-Frame, R-11 insulation in 2 x 4 stud cavity, R-4 board insulation, siding finish, wood studs	1379	3,725	0	2,260	2,260
16C-30: Roof/Ceiling-Under attic or knee wall, Vented Attic, No Radiant Barrier, White or Light Color Shingles, Any Wood Shake, Light Metal, Tar and Gravel or Membrane, R-30 insulation	1009.2	1,194	0	1,389	1,389
22A-ph: Floor-Slab on grade, No edge insulation, no insulation below floor, any floor cover, passive, heavy moist soil	180	9,045	0	0	0
Subtotals for structure:		16,127	0	5,450	5,450
People:	2	,	460	600	1,060
Equipment:			0	0	. 0
Lighting:	360			1,228	1,228
Ductwork:		4,294	0	1,687	1,687
Infiltration: Winter CFM: 132, Summer CFM: 59		5,339	1,985	1,154	3,139
Ventilation: Winter CFM: 0, Summer CFM: 0		0	. 0	0	. 0
System 1 Main Floor Load Totals:		25,760	2,445	10,119	12,564

Check Figures			
Supply CFM:	462	CFM Per Square ft.:	0.458
Square ft. of Room Area:	1,009	Square ft. Per Ton:	921
Volume (ft³) of Cond. Space:	8,794	Air Turnover Rate (per hour):	3.2

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The state of the s				CONTROL OF THE CONTRO
Total Heating Required With Outside Air:	25,760	Btuh	25.760	MBH
Total Sensible Gain:	10,119	Btuh	81	%
Total Latent Gain:	2,445	Btuh	19	%
Total Cooling Required With Outside Air:	12,564	Btuh	1.05	Tons (Based On Sensible + Latent)
			1.10	Tons (Based On 77% Sensible Capacity)

Notes

Calculations are based on 8th edition of ACCA Manual J.

All computed results are estimates as building use and weather may vary.

Rhvac - Residential & Light Commercial North Central Florida A/C inc High Springs, FU 32643	HVAC Loads			Elite S	ofiware Develo	opment, Inc. Bill Carter Page 7
System 1, Zone 1 Summ	ary Loads (Ave	erage Meth	od)			
Component		Area	Sen	Lát	Sen	Total
Description		Quan	Loss	Gain	Gain	Gain
1D-cb-o: Glazing-Double pane, open metal frame with break, ground r outdoor insect screen with 50% of shade screen coefficient of 0.45	reflectance = 0.1, coverage, external	25	600	0	576	576
11P: Door-Polyurethane Core		145.7	1,563	0	1,225	1,225
12B-4sw: Wall-Frame, R-11 insulation R-4 board insulation, siding finish		1379	3,725	0	2,260	2,260
16C-30: Roof/Ceiling-Under attic or le Attic, No Radiant Barrier, White of Shingles, Any Wood Shake, Ligh Gravel or Membrane, R-30 insula	knee wall, Vented or Light Color it Metal, Tar and	1009.2	1,194	0	1,389	1,389
22A-ph: Floor-Slab on grade, No edg insulation below floor, any floor of moist soil		180	9,045	0	0	0
Subtotals for structure:			16,127	0	5,450	5,450
People:		2	•	460	600	1,060
Equipment:				0	0	0
Lighting:		360			1,228	1,228
Ductwork:			4,294	0	1,687	1,687
Infiltration: Winter CFM: 132, Summ	er CFM: 59		5,339	1,985	1,154	3,139
System 1, Zone 1 Load Totals:			25,760	2,445	10,119	12,564
Check Figures						
Supply CFM:	462		M Per Square f			458
Square ft. of Room Area:	1,009		are ft. Per Tor		!	921
Volume (ft³) of Cond. Space:	8,794	Air '	Turnover Rate	(per hour):		3.2
Zone Loads						
Total Heating Required:	25,760		.760 MBH			
Total Sensible Gain:	10 119	Bhuh	81 %			

10,119 Btuh

2,445 Btuh

12,564 Btuh

81 %

19 %

1.05 Tons (Based On Sensible + Latent) 1.10 Tons (Based On 77% Sensible Capacity)

Notes

Total Sensible Gain:

Total Cooling Required:

Total Latent Gain:

Calculations are based on 8th edition of ACCA Manual J.

All computed results are estimates as building use and weather may vary.

Rhvac - Residential & Light Con North Central Florida A/C Inc High Springs, FU 32643								Elite Software Development, Inc. Bill Carter Page 8		
System 1 Room Loa	ad Sum	mary								
Room No Name	Area SF	Htg Sens Btuh	the state of the s	Rün Duct Size	Run Duct Vel	Glg Sens Btuh	Cig Lat Btuh	Clg Nom CFM	Ai Sys CFN	
—Zone 1— 1 Garage 2 Storage	720 289	16,108 9,652		3-6 2-5	532 547	6,856 3,262	1,705 740	313 149	313 149	
System 1 total	1,009	25,760	336			10,119	2,445	462	462	
System 1 Main Trunk Size: Velocity: Loss per 100 ft.:		11x10 701 0.098	ft./min							
Cooling System Summary										
	Cooling Töns	Sei	nsible/Latent Split		Sensible Btuh		Lätent Btuh		Tota Btul	
Net Required: Recommended: Actual:	1.05 1.10 1.50		81% / 19% 77% / 23% 71% / 29%		10,119 10,119 12,800		2,445 3,022 5,200		12,564 13,141 18,000	
Equipment Data										
Type: Model: Brand: Efficiency: Sound: Capacity: Sensible Capacity: Latent Capacity:		n/a	ng System				d Condense 181A*+AWE n tuh			



RE: CARTER - CARTER RESIDENCE

MiTek Industries, Inc.

1801 Massaro Blvd. Tampa, Fl 33619 Phone: 813/675-1200

Fax: 813/675-1148

Site Information:

Project Customer: BILL CARTER Project Name: CARTER RESIDENCE

Lot/Block:

Subdivision:

Address:

City: FT. WHITE

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002

Wind Speed: 110 mph

Design Program: MiTek 20/20 6.2 Design Method: User defined

Wind Code: N/A Roof Load: 47.0 psf

Floor Load: 55.0 psf

This package includes 17 individual, dated Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Job ID#	Truss Name	Date	No.	Seal#	Job ID#	Truss Name	Date
1	T1931922	CARTER	1A	12/29/05	17	T1931938	CARTER	PB1	12/29/05
2	T1931923	CARTER	1AG	12/29/05					1,00
3	T1931924	CARTER	1B	12/29/05					
4	T1931925	CARTER	1C	12/29/05					
5	T1931926	CARTER	1CG	12/29/05					
6	T1931927	CARTER	1D	12/29/05	Ĭ				
7	T1931928	CARTER	1DG	12/29/05	Ī				
8	T1931929	CARTER	1DS	12/29/05					
9	T1931930	CARTER	D1	12/29/05					
10	T1931931	CARTER	FG1	12/29/05					
11	T1931932	CARTER	M1	12/29/05					
12	T1931933	CARTER	M1S	12/29/05					
13	T1931934	CARTER	M2	12/29/05					
14	T1931935	CARTER	M3	12/29/05					
15	T1931936	CARTER	M4	12/29/05					
16	T1931937	CARTER	PB	12/29/05					

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Cox Lumber-Ocala, FL.

Truss Design Engineer's Name: Zhang, Guo-jie

My license renewal date for the state of is February 28, 2007.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Sec. 2.

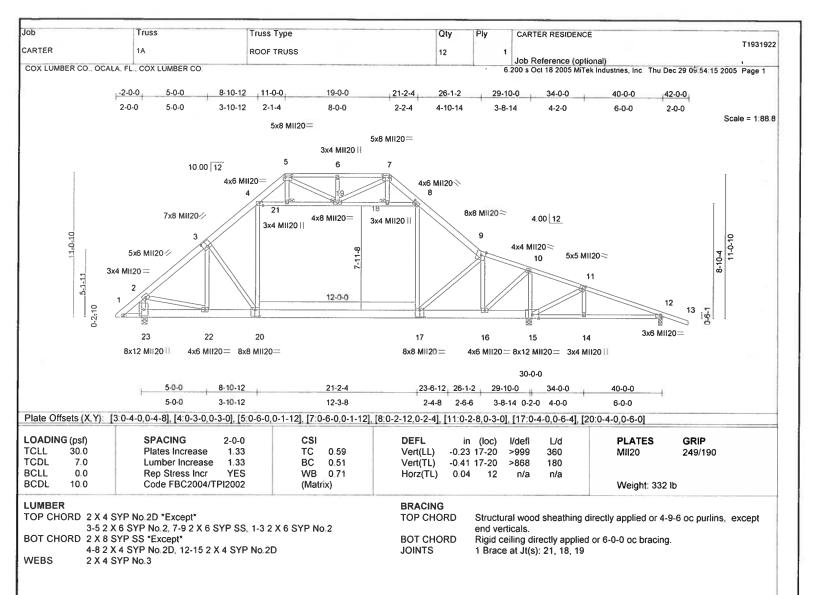
Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd

Tampa FL 33619

FL Cert.#6634

Zhang, Guo-jie

December 29,2005



REACTIONS (lb/size) 23=2355/0-5-8, 15=2836/0-5-8, 12=541/0-3-8

Max Horz 23=-413(load case 2)

Max Uplift23=-150(load case 4), 15=-280(load case 5), 12=-482(load case 3) Max Grav 23=2355(load case 1), 15=2944(load case 10), 12=541(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/91, 2-3=-2353/53, 3-4=-2615/29, 4-5=-948/241, 5-6=-945/332, 6-7=-945/332, 7-8=-908/219, 8-9=-2591/59,

9-10=-1501/281, 10-11=-595/785, 11-12=-563/624, 12-13=0/33, 2-23=-2212/189

22-23=-132/438, 20-22=-8/1683, 17-20=0/1925, 16-17=-0/1465, 15-16=-715/685, 14-15=-478/514, 12-14=-476/518

 $3-20=-79/586,\ 4-20=0/966,\ 5-21=0/211,\ 7-18=0/177,\ 8-17=-73/1001,\ 9-17=-1/842,\ 9-16=-1942/20,\ 10-16=-109/2301,\$

11-15=-645/230, 11-14=0/99, 4-21=-1239/19, 19-21=-1212/19, 18-19=-1291/70, 8-18=-1310/70, 6-19=-282/191,

7-19=-206/392, 5-19=-231/325, 10-15=-2388/207, 3-22=-813/0, 2-22=0/1585

NOTES

WEBS

BOT CHORD

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Ceiling dead load (5.0 psf) on member(s), 4-21, 19-21, 18-19, 8-18; Wall dead load (10.0 psf) on member(s), 4-20, 8-17 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room, 17-20

- 7) Bearing at joint(s) 15 considers parallel to grain value using ANSI/TPI1 angle to grain formula. Building designer should verify capacity of
- bearing surface.

 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 150 lb uplift at joint 23, 280 lb uplift at joint 15 and 482 lb uplift at joint 12.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert #6634

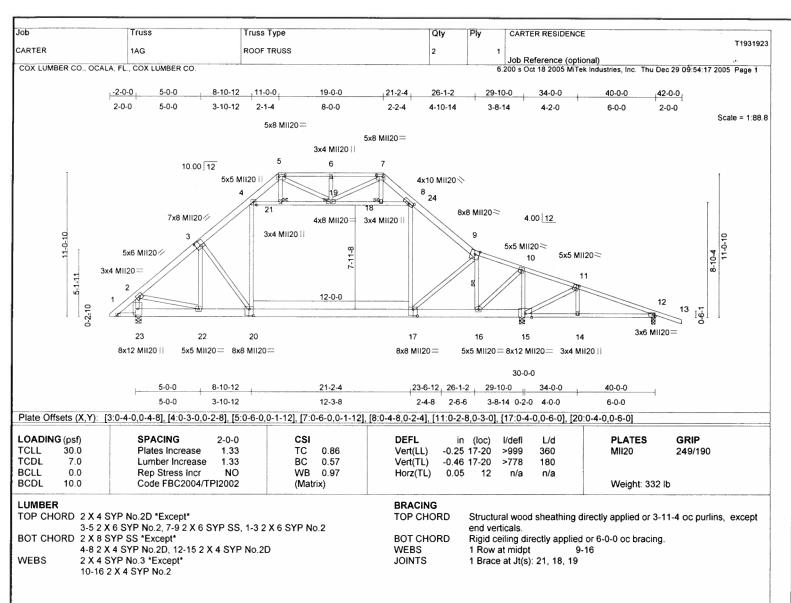
December 29,2005

MARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 Quality Criteria, DSB-89 and BCS1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

1801 Massaro Blvd. Tampa, FL 33619





REACTIONS (lb/size) 23=2968/0-5-8, 15=4283/0-5-8, 12=505/0-3-8

Max Horz 23=-413(load case 2)

Max Uplift23=-315(load case 4), 15=-296(load case 2), 12=-543(load case 3) Max Grav 23=2968 (load case 1), 15=4391 (load case 10), 12=505 (load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/91, 2-3=-3060/247, 3-4=-3407/231, 4-5=-1039/275, 5-6=-976/343, 6-7=-976/343, 7-8=-957/233, 8-24=-3007/269,

9-24=-3632/319, 9-10=-2420/526, 10-11=-419/863, 11-12=-456/796, 12-13=0/33, 2-23=-2813/354

BOT CHORD 22-23=-141/425, 20-22=-156/2217, 17-20=0/2534, 16-17=-225/2275, 15-16=-781/520, 14-15=-639/412, 12-14=-636/416 WEBS

3-20=-94/710, 4-20=-129/1493, 5-21=-1/285, 7-18=-6/242, 8-17=-139/1221, 9-17=0/567, 9-16=-2806/85, 10-16=-198/3468,

11-15=-600/221, 11-14=0/96, 4-21=-1782/142, 19-21=-1745/141, 18-19=-1903/233, 8-18=-1931/235, 6-19=-281/191,

7-19=-232/432, 5-19=-201/275, 10-15=-3815/243, 3-22=-960/10, 2-22=-59/2098

NOTES

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone;

cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Ceiling dead load (5.0 psf) on member(s). 4-21, 19-21, 18-19, 8-18; Wall dead load (10.0psf) on member(s). 4-20, 8-17 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 17-20

7) Bearing at joint(s) 15 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 315 lb uplift at joint 23, 296 lb uplift at joint 15 and 543 lb uplift at joint 12.

9) Load case(s) 1, 2, 3, 4, 5, 8, 9, 10 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 640 lb down and 244 lb up at 8-10-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)

LOAD CASE(S) Standard Continued on page 2

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December 29,2005

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.

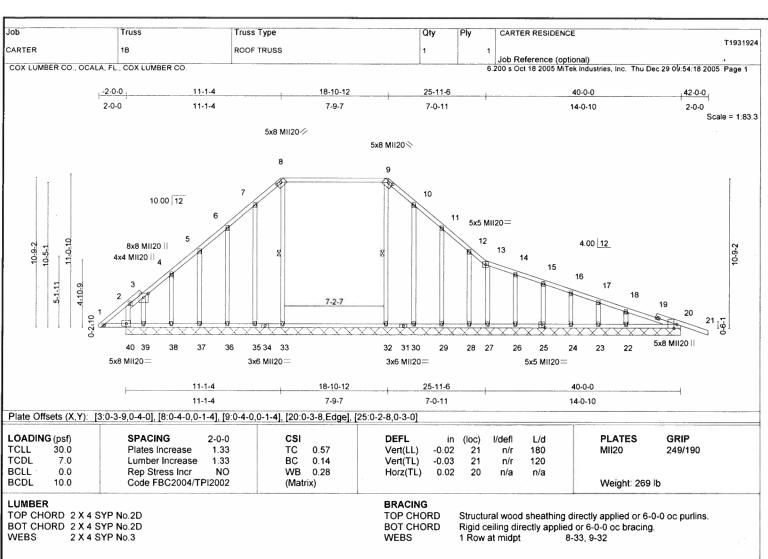
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



		W 19.				
Job	Truss	Truss Type	Qty	Ply	CARTER RESIDENCE	T19319
CARTER	1AG	ROOF TRUSS	2		Job Reference (optional)	st.
COX LUMBER CO., OCALA, F	L., COX LUMBER CO.				6.200 s Oct 18 2005 MiTek Industries, Inc. Thu De	
LOAD CASE(S) Standar 1) Regular: Lumber Incre Uniform Loads (plf) Vert: 1-2=-74, Drag: 4-20=-2(Concentrated Loads (II Vert: 20=-640(Trapezoidal Loads (plf) Vert: 24=-204(2) MWFRS Wind Left: Lu Uniform Loads (plf) Vert: 1-2=9, 2- Horz: 1-2=-18, Drag: 4-20=-2(Concentrated Loads (III Vert: 20=244(F Trapezoidal Loads (plf) Vert: 24=72(F= 3) MWFRS Wind Right: L Uniform Loads (plf)	d ase=1.33, Plate Increase: 2-5=-74, 5-7=-74, 7-24=-7 0, 8-17=-20 0) F=-130)-to-9=-248(F=-17-18) imber Increase=1.33, Plate 5=-12, 5-7=46, 7-24=22, 2-5=4, 7-9=31, 9-12=37, 0, 8-17=-20, 2-23=0 0) 1) 1=50)-to-9=28(F=6), 9=35(F=6), 9=	4, 10-13=-74, 20-23=-20, 17-20=-1. i), 9=-248(F=-174)-to-10=-286(F=-2e Increase=1.33 0-12=29, 12-13=21, 20-23=-10, 17-12-13=29, 2-23=22	212) -20=-22, 12-17=-10, 4	-21=-6,	1=-10, 8-18=-10 18-21=-6, 8-18=-6	622 09:04:17 2000 Page 2
Horz: 1-2=-23, Drag: 4-20=-20 Concentrated Loads (II Vert: 20=244(F Trapezoidal Loads (plf) Vert: 24=37(F= 4) MWFRS 1st Wind Para Uniform Loads (plf)	2-5=-31, 7-9=-4, 9-12=54 0, 8-17=-20, 2-23=-0 o) f) f) -50)-to-9=-6(F=6), 9=52(F allel: Lumber Increase=1.3	, 12-13=76, 2-23=-29 =6)-to-10=14(F=-32)				
Drag: 4-20=-20 Concentrated Loads (II Vert: 20=189(F Trapezoidal Loads (plf) Vert: 24=62(F= 5) MWFRS 2nd Wind Par Uniform Loads (plf) Vert: 1-2=17, 2 Horz: 1-2=-25,	2) 338)-to-9=18(F=-5), 9=18(F=-16), 9=18(F=-16), 9=18(F=-16), 9=18(F=-16), 9=18(F=-16), 9=18(F=-16), 8=17=-20, 2-23=-0, 9=17=-20, 2-23=-0, 9=18(F=-16), 9=18(F=-	F=-5)-to-10=-20(F=-43) 33, Plate Increase=1.33 10-12=38, 12-13=56, 20-23=-10, 17	7-20=-22, 12-17=-10, 4	J-21=-6,	18-21=-6, 8-18=-6	
Vert: 24=76(F= 8) Attic Floor: Lumber Inc Uniform Loads (plf) Vert: 1-2=-14, Drag: 4-20=-20 Concentrated Loads (lb	=38)-to-9=32(F=-5), 9=32(rease=1.00, Plate Increas 2-5=-14, 5-7=-14, 7-24=-1), 8-17=-20 p)		20, 12-17=-20, 4-21=-	10, 18-2	1=-10, 8-18=-10	
9) 1st unbalanced Regula Uniform Loads (plf) Vert: 1-2=-74, 2 Drag: 4-20=-20	=-47)-to-9=-105(F=-91), 9 ir: Lumber Increase=1.33, 2-5=-74, 5-7=-74, 7-24=-1 0, 8-17=-20	=-105(F=-91)-to-10=-143(F=-129) Plate Increase=1.33 4, 10-13=-14, 20-23=-20, 17-20=-12	20, 12-17=-20, 4-21=-	10, 18-2	1=-10, 8-18=-10	
 2nd unbalanced Regu Uniform Loads (plf) Vert: 1-2=-14, 	F) 130)-to-9=-188(F=-174 dar: Lumber Increase=1.3 2-5=-14, 5-7=-74, 7-24=-), 9=-188(F=-174)-to-10=-226(F=-2 3, Plate Increase=1.33 74, 10-13=-74, 20-23=-20, 17-20=-	,	-10, 18-	21=-10, 8-18=-10	
Drag: 4-20=-2 Concentrated Loads (Vert: 20=-640 Trapezoidal Loads (pl Vert: 24=-204	lb) i(F) f)	4), 9=-248(F=-174)-to-10=-286(F=-	212)			



T1931923



REACTIONS (lb/size) 40=319/40-0-0, 20=349/40-0-0, 27=157/40-0-0, 33=681/40-0-0, 32=682/40-0-0, 35=-88/40-0-0, 36=237/40-0-0, 37=179/40-0-0, 38=191/40-0-0, 39=29/40-0-0, 30=-90/40-0-0, 29=244/40-0-0, 28=135/40-0-0, 26=200/40-0-0,

25=181/40-0-0, 24=200/40-0-0, 23=149/40-0-0, 22=291/40-0-0

Max Horz 40=-368(load case 2)

Max Uplift40=-391(load case 4), 20=-272(load case 3), 27=-186(load case 3), 33=-215(load case 2), 32=-267(load case 2), 35=-211(load case 9), 36=-132(load case 4), 37=-120(load case 4), 38=-131(load case 4), 30=-212(load case 8), 29=-131(load case 5), 28=-99(load case 5), 26=-100(load case 3), 25=-77(load case 3), 24=-85(load case 3), 23=-81(load case 3), 22=-95(load case 3)

Max Grav 40=319(load case 8), 20=349(load case 9), 27=158(load case 9), 33=683(load case 8), 32=683(load case 9), 35=155(load case 2), 36=237(load case 8), 37=179(load case 1), 38=191(load case 8), 39=87(load case 4), 30=163(load case 3), 29=244(load case 9), 28=135(load case 1), 26=200(load case 9), 25=181(load case 1),

24=200(load case 9), 23=149(load case 1), 22=291(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/97, 2-3=-89/251, 3-4=-44/226, 4-5=-48/331, 5-6=-45/430, 6-7=-66/542, 7-8=-83/591, 8-9=-8/518, 9-10=-115/604,

10-11=-94/585, 11-12=-176/503, 12-13=-220/438, 13-14=-210/328, 14-15=-236/303, 15-16=-261/284, 16-17=-287/260,

17-18=-311/241, 18-19=-340/209, 19-20=-347/199, 20-21=0/33

BOT CHORD 39-40=-168/368, 38-39=-168/368, 37-38=-168/368, 36-37=-168/368, 35-36=-168/368, 34-35=-168/368, 33-34=-168/368,

32-33=-171/372, 31-32=-168/367, 30-31=-168/367, 29-30=-168/367, 28-29=-168/367, 27-28=-168/367, 26-27=-161/366,

25-26=-161/366, 24-25=-163/364, 23-24=-163/364, 22-23=-163/364, 20-22=-163/364

13-27=-123/204, 8-33=-540/286, 9-32=-541/338, 2-40=-312/395, 7-35=-165/191, 6-36=-181/160, 5-37=-144/138,

4-38=-149/152, 3-39=-69/15, 10-30=-173/191, 11-29=-186/160, 12-28=-108/112, 14-26=-160/120, 15-25=-142/96,

16-24=-155/108, 17-23=-122/94, 18-22=-221/132

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) All plates are 3x4 MII20 unless otherwise indicated.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection

6) Gable requires continuous bottom chord bearing.

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

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Continued on page 2

A WARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.

Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP11 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	CARTER RESIDENCE	
CARTER	1B	ROOF TRUSS	1	1		T193192
					Job Reference (optional)	110
COVILIMPED CO OCALA E	COVILINDED CO				000 0 1 10 0005 11 7 1 1 1 1 1 7 7	00 00 01 10 0000 0

NOTES

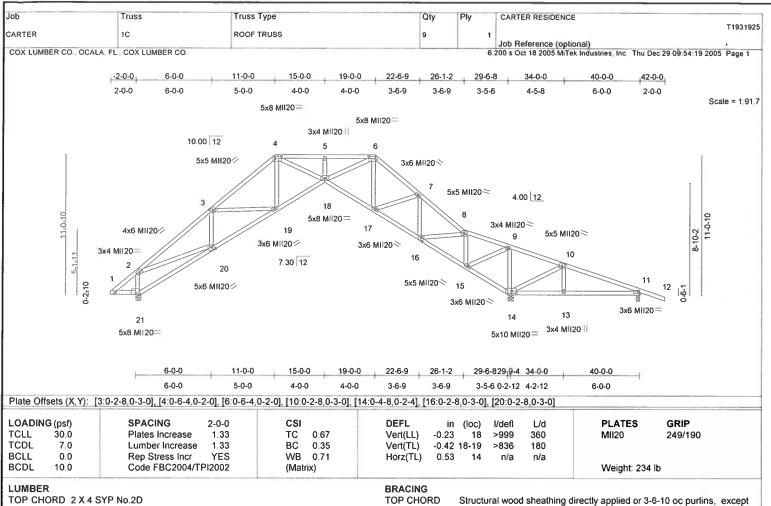
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 391 lb uplift at joint 40, 272 lb uplift at joint 20, 186 lb uplift at joint 27, 215 lb uplift at joint 33, 267 lb uplift at joint 32, 211 lb uplift at joint 35, 132 lb uplift at joint 36, 120 lb uplift at joint 37, 131 lb uplift at joint 38, 212 lb uplift at joint 30, 131 lb uplift at joint 29, 99 lb uplift at joint 26, 77 lb uplift at joint 25, 85 lb uplift at joint 24, 81 lb uplift at joint 23 and 95 lb uplift at joint 22.

LOAD CASE(S) Standard

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BOT CHORD 2 X 4 SYP No.2D

2 X 4 SYP No.3

TOP CHORD

BOT CHORD Rigid ceiling directly applied or 3-11-11 oc bracing

REACTIONS (lb/size) 14=2968/0-5-8, 11=-183/0-3-8, 21=1258/0-5-8

Max Horz 21=-420(load case 2)

Max Uplift14=-441(load case 4), 11=-428(load case 8), 21=-385(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/97, 2-3=-2180/558, 3-4=-2089/411, 4-5=-3105/280, 5-6=-3105/280, 6-7=-1359/243, 7-8=-527/359, 8-9=-1/1269,

9-10=-219/2465, 10-11=-162/1781, 11-12=0/33, 2-21=-1243/522

BOT CHORD 20-21=-226/546, 19-20=-436/1849, 18-19=-58/1774, 17-18=0/1133, 16-17=0/438, 15-16=-1317/250, 14-15=-2688/371,

13-14=-1598/190, 11-13=-1592/192

WEBS 3-20=-355/115, 3-19=-104/325, 4-19=-157/164, 4-18=0/1755, 5-18=-248/218, 6-18=-131/2360, 6-17=-507/87, 7-17=-57/702,

7-16=-1041/91, 8-16=-68/1425, 8-15=-1140/239, 9-15=-199/1251, 9-14=-1172/295, 10-14=-795/269, 10-13=0/130,

2-20=-189/1533

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

5) Bearing at joint(s) 14, 21 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 441 lb uplift at joint 14, 428 lb uplift at joint 11 and 385 lb uplift at joint 21.

LOAD CASE(S) Standard

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December 29,2005

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Job Truss Truss Type Qty Ply CARTER RESIDENCE T1931926 CARTER 1CG **ROOF TRUSS** 2 Job Reference (optional) COX LUMBER CO., OCALA, FL., COX LUMBER CO. 6 200 s Oct 18 2005 MiTek Industries, Inc. Thu Dec 29 09 54 21 2005 Page 1 -2-0-0 6-0-0 11-0-0 15-0-0 19-0-0 26-1-2 29-6-8 34-0-0 40-0-0 42-0-0 22-6-9 2-0-0 6-0-0 4-0-0 2-0-0 Scale = 1:91.7 5x8 MII20= 5x10 MII20= 3x4 MII20 || 10.00 12 4 5x5 MII20 // 3x6 MII20 > 5x6 MII20 > 4.00 12 18 6x8 MII20= 4x6 MII20 > 19 4x6 MII204 5x6 MII20~ 9 23 3x6 MII20 / 24 3x4 MII20= 5-1-11 3x6 MII20 > 16 7.30 12 20 7x8 MII20 ≥ 15 0-2:10 5x6 MII20/ 12 4x6 MII20 > 4x6 MII20 = 13 21 3x4 MII20 II 5x8 MII20= 8x12 MII20H= 26-1-2 29-6-829-9-4 34-0-0 6-0-0 11-0-0 15-0-0 19-0-0 22-6-9 40-0-0 5-0-0 3-6-9 6-0-0 4-0-0 4-0-0 3-6-9 6-0-0 Plate Offsets (X,Y): [3:0-2-8,0-3-0], [4:0-6-4,0-2-0], [6:0-8-4,0-2-0], [10:0-2-12,0-3-4], [16:0-4-0,0-3-4], [20:0-2-8,0-3-0] LOADING (psf) SPACING DEFL **PLATES** I/defl L/d **TCLL** 30.0 1.33 TC 0.83 MII20 249/190 Plates Increase Vert(LL) -0.31 18-19 >999 360 TCDL вС 7.0 1.33 0.68 -0.59 18-19 MII20H 187/143 Lumber Increase Vert(TL) >600 180 BCLL 0.0 Rep Stress Incr NO WB 0.85 Horz(TL) 0.74 14 n/a n/a Code FBC2004/TPI2002 **BCDI** 10.0 (Matrix) Weight: 250 lb LUMBER **BRACING** TOP CHORD 2 X 4 SYP No.2D *Except* TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins, except 8-10 2 X 4 SYP SS end verticals. BOT CHORD 2 X 4 SYP No.2D *Except* **BOT CHORD** Rigid ceiling directly applied or 3-6-8 oc bracing 11-14 2 X 6 SYP No.2, 14-16 2 X 6 SYP No.2 2 X 4 SYP No.3 *Except* WEBS

9-14 2 X 4 SYP SS

REACTIONS (lb/size) 14=5493/0-5-8, 11=-795/0-3-8, 21=1399/0-5-8

Max Horz 21=-419(load case 2)

Max Uplift14=-832(load case 3), 11=-1032(load case 8), 21=-417(load case 4) Max Grav 14=5493(load case 1), 11=80(load case 4), 21=1399(load case 1)

FORCES (Ib) - Maximum Compression/Maximum Tension

 $1-2=0/97,\ 2-3=-2521/636,\ 3-4=-2562/520,\ 4-5=-4175/525,\ 5-6=-4175/525,\ 6-22=-2053/438,\ 7-22=-2273/437,\ 7-8=-1692/594,\ 3-4=-2562/520,\ 4-5=-4175/525,\ 5-6=-4175/525,\ 6-22=-2053/438,\ 7-22=-2273/437,\ 7-8=-1692/594,\ 3-4=-2562/520,\ 4-5=-4175/525,\ 5-6=-4175/525,\ 6-22=-2053/438,\ 7-22=-2273/437,\ 7-8=-1692/594,\ 3-4=-2562/520,\ 4-5=-4175/525,\ 5-6=-4175/525,\ 6-22=-2053/438,\ 7-22=-2273/437,\ 7-8=-1692/594,\ 3-4=-2562/520,\ 4-5=-4175/525,\ 5-6=-4175/525,\ 6-22=-2053/438,\ 7-22=-2273/437,\ 7-8=-1692/594,\ 3-22=-2273/437,\ 7-8=-1692/594,\ 3-22=-2273/437,$ TOP CHORD

8-9=-169/1807, 9-23=-516/4330, 10-23=-531/4302, 10-11=-457/3578, 11-12=0/39, 2-21=-1386/555

BOT CHORD 20-21=-228/543, 19-20=-507/2155, 18-19=-154/2193, 17-18=-27/1876, 17-24=-293/1439, 16-24=-176/1089,

15-16=-1619/391, 14-15=-4747/736, 13-14=-3302/469, 11-13=-3295/471

3-20=-440/135, 3-19=-141/300, 4-19=-149/125, 4-18=-139/2535, 5-18=-223/209, 6-18=-274/2818, 6-17=-303/0, WEBS

7-17=-39/595, 7-16=-1302/157, 8-15=-2734/459, 9-15=-365/2699, 9-14=-2596/345, 10-14=-831/290, 10-13=0/160,

2-20=-250/1800, 8-16=-360/2465

NOTES

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

Provide adequate drainage to prevent water ponding.

4) All plates are MT20 plates unless otherwise indicated 5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

6) Bearing at joint(s) 14, 21 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

- capacity of bearing surface. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 832 lb uplift at joint 14, 1032 lb uplift at joint 11 and 417 lb uplift at joint 21.
- 8) Load case(s) 1, 2, 3, 4, 5, 8, 9 has/have been modified. Building designer must review loads to verify that they are correct for the intended use of this truss.

9) Uplift for first LC exceeds limits

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 640 lb down and 244 lb up at 21-1-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B). Continued on page 2

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December 29,2005

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Job Truss Truss Type Qty CARTER 1CG **ROOF TRUSS** 2 COX LUMBER CO., OCALA, FL., COX LUMBER CO. LOAD CASE(S) Standard 1) Regular: Lumber Increase=1.33, Plate Increase=1.33 Uniform Loads (plf) Vert: 1-2=-74, 2-4=-74, 4-6=-74, 6-22=-74, 12-23=-74, 18-21=-20, 14-18=-20, 11-14=-20 Concentrated Loads (lb) Vert: 24=-640(F) Trapezoidal Loads (plf) Vert: 22=-204(F=-130)-to-8=-248(F=-174), 8=-248(F=-174)-to-23=-287(F=-213) 2) MWFRS Wind Left: Lumber Increase=1.33, Plate Increase=1.33 Uniform Loads (plf) Vert: 1-2=9, 2-4=-12, 4-6=46, 6-22=22, 11-23=29, 11-12=21, 18-21=-10, 14-18=-10, 11-14=-10 Horz: 1-2=-18, 2-4=4, 6-8=31, 8-11=37, 11-12=29, 2-21=22 Drag: 4-5=-0, 5-6=0 Concentrated Loads (lb) Vert 24=244(F) Trapezoidal Loads (plf) Vert: 22=72(F=50)-to-8=28(F=6), 8=35(F=6)-to-23=-5(F=-33) 3) MWFRS Wind Right: Lumber Increase=1.33, Plate Increase=1.33 Uniform Loads (plf) Vert: 1-2=15, 2-4=22, 4-6=46, 6-22=-12, 11-23=46, 11-12=68, 18-21=-10, 14-18=-10, 11-14=-10 Horz: 1-2=-23, 2-4=-31, 6-8=-4, 8-11=54, 11-12=76, 2-21=-29 Drag: 4-5=-0, 5-6=0 Concentrated Loads (lb) Vert 24=244(F) Trapezoidal Loads (plf) Vert: 22=37(F=50)-to-8=-6(F=6), 8=52(F=6)-to-23=12(F=-33) 4) MWFRS 1st Wind Parallel: Lumber Increase=1.33, Plate Increase=1.33 Uniform Loads (plf) Vert. 1-2=56, 2-4=38, 4-6=24, 6-22=24, 11-23=24, 11-12=17, 18-21=-10, 14-18=-10, 11-14=-10 Horz: 1-2=-65, 2-4=-46, 6-8=32, 8-11=32, 11-12=25, 2-21=-23 Drag: 4-5=-0, 5-6=0 Concentrated Loads (lb) Vert: 24=189(F) Trapezoidal Loads (plf) Vert: 22=62(F=38)-to-8=18(F=-5), 8=18(F=-5)-to-23=-21(F=-45) 5) MWFRS 2nd Wind Parallel Lumber Increase=1.33, Plate Increase=1.33 Uniform Loads (plf) Vert. 1-2=17, 2-4=24, 4-6=24, 6-22=38, 11-23=38, 11-12=56, 18-21=-10, 14-18=-10, 11-14=-10 Horz: 1-2=-25, 2-4=-32, 6-8=46, 8-11=46, 11-12=65, 2-21=-23 Drag: 4-5=-0, 5-6=0 Concentrated Loads (lb) Vert. 24=189(F) Trapezoidal Loads (plf) Vert: 22=76(F=38)-to-8=32(F=-5), 8=32(F=-5)-to-23=-7(F=-45) 8) 1st unbalanced Regular. Lumber Increase=1.33, Plate Increase=1.33 Uniform Loads (plf) Vert: 1-2=-74, 2-4=-74, 4-6=-74, 6-22=-14, 12-23=-14, 18-21=-20, 14-18=-20, 11-14=-20 Concentrated Loads (lb) Vert: 24=-640(F) Trapezoidal Loads (plf) Vert: 22=-144(F=-130)-to-8=-188(F=-174), 8=-188(F=-174)-to-23=-227(F=-213) 9) 2nd unbalanced Regular: Lumber Increase=1.33, Plate Increase=1.33 Uniform Loads (plf) Vert. 1-2=-14, 2-4=-14, 4-6=-74, 6-22=-74, 12-23=-74, 18-21=-20, 14-18=-20, 11-14=-20 Concentrated Loads (lb) Vert 24=-640(F) Trapezoidal Loads (plf)

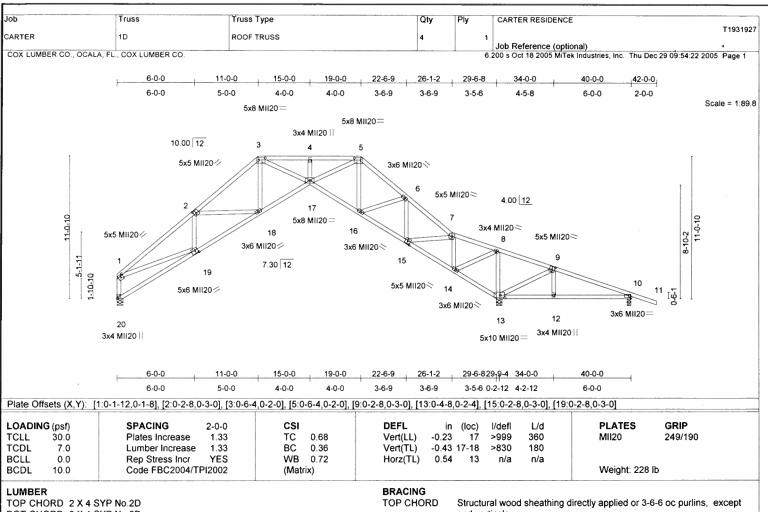
Vert 22=-204(F=-130)-to-8=-248(F=-174), 8=-248(F=-174)-to-23=-287(F=-213)

T1931926

Ply

CARTER RESIDENCE

Job Reference (optional) 6 200 s Oct 18 2005 MiTek Industries, Inc. Thu Dec 29 09:54:21 2005 Page 2



BOT CHORD 2 X 4 SYP No.2D

2 X 4 SYP No.3 WEBS

BOT CHORD

Rigid ceiling directly applied or 3-11-8 oc bracing

REACTIONS (lb/size) 13=2980/0-5-8, 10=-188/0-3-8, 20=1099/0-5-8

Max Horz 20=-450(load case 2)

Max Uplift13=-442(load case 4), 10=-433(load case 8), 20=-269(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2224/557, 2-3=-2113/414, 3-4=-3132/282, 4-5=-3132/282, 5-6=-1367/243, 6-7=-527/359, 7-8=-1/1280, 8-9=-220/2479,

9-10=-163/1795, 10-11=0/33, 1-20=-1125/396

BOT CHORD 19-20=-209/556, 18-19=-443/1894, 17-18=-58/1795, 16-17=0/1140, 15-16=0/438, 14-15=-1328/250, 13-14=-2705/372,

12-13=-1611/191, 10-12=-1605/193

 $2-19=-349/126,\ 2-18=-105/329,\ 3-18=-162/178,\ 3-17=0/1764,\ 4-17=-248/218,\ 5-17=-129/2383,\ 5-16=-513/86,\ 6-16=-57/708.$ WEBS

6-15=-1048/92, 7-15=-69/1434, 7-14=-1144/238, 8-14=-199/1256, 8-13=-1175/295, 9-13=-796/269, 9-12=0/130,

1-19=-205/1498

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

- 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 5) Bearing at joint(s) 13, 20 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 442 lb uplift at joint 13, 433 lb uplift at joint 10 and 269 lb uplift at joint 20.

LOAD CASE(S) Standard

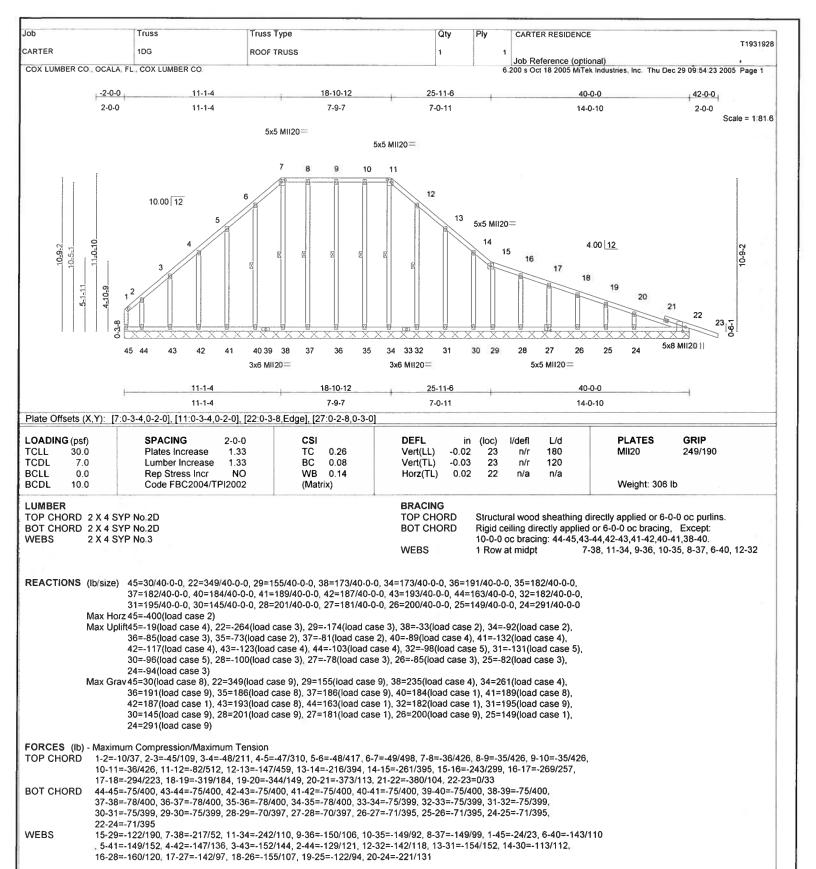
Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

December 29,2005

MARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





NOTES

1) Unbalanced roof live loads have been considered for this design

2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

Provide adequate drainage to prevent water ponding.

4) All plates are 3x4 MII20 unless otherwise indicated.

5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

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Tampa, FL 33619

1801 Massaro Blvd.

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MiTek®

Job	Truss	Truss Type	Qty	Ply	CARTER RESIDENCE	
CARTER	1DG	ROOF TRUSS	1	1		T1931928
			Ι΄		Job Reference (optional)	•

COX LUMBER CO., OCALA, FL., COX LUMBER CO.

6.200 s Oct 18 2005 MiTek Industries, Inc., Thu Dec 29 09 54:23 2005 Page 2

NOTES

6) Gable requires continuous bottom chord bearing.

38, 92 lb uplift at joint 34, 85 lb uplift at joint 36, 73 lb uplift at joint 35, 81 lb uplift at joint 37, 89 lb uplift at joint 40, 132 lb uplift at joint 41, 117 lb uplift at joint 42, 123 lb uplift at joint 43, 103 lb uplift at joint 44, 98 lb uplift at joint 32, 131 lb uplift at joint 31, 96 lb uplift at joint 30, 100 lb uplift at joint 28, 78 lb uplift at joint 27, 85 lb uplift at joint 26, 82 lb uplift at joint 30, 100 lb uplift at joint 28, 78 lb uplift at joint 27, 85 lb uplift at joint 26, 82 lb uplift at joint 30, 100 lb uplift at joint 28, 78 lb uplift at joint 27, 85 lb uplift at joint 28, 87 lb uplift at joint 29, 88 lb 25 and 94 lb uplift at joint 24.

LOAD CASE(S) Standard



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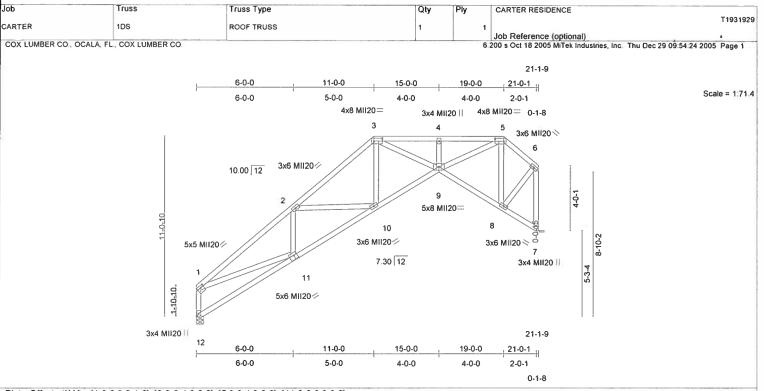


Plate Offsets (X,Y): [1:0-2-0,0-1-8], [3:0-6-4,0-2-0], [5:0-6-4,0-2-0], [11:0-2-8,0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) I/defl L/d	PLATES GRIP
TCLL 30.0	Plates Increase 1,33	TC 0.63	Vert(LL) 0.11 9-10 >999 360	MII20 249/190
TCDL 7.0	Lumber Increase 1,33	BC 0.29	Vert(TL) -0.20 9-10 >999 180	
BCLL 0.0	Rep Stress Incr YES	WB 0.59	Horz(TL) 0.26 7 n/a n/a	
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)	• •	Weight: 134 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No. 2D **WEBS** 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 4-3-8 oc purlins, except

BOT CHORD Rigid ceiling directly applied or 6-10-4 oc bracing

REACTIONS (lb/size) 7=979/Mechanical, 12=979/0-5-8

Max Horz 12=423(load case 3)

Max Uplift7=-318(load case 3), 12=-183(load case 4)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=-1938/664, 2-3=-1717/663, 3-4=-2217/1045, 4-5=-2217/1045, 5-6=-629/205, 1-12=-1003/437, 6-7=-974/343 TOP CHORD

BOT CHORD 11-12=-535/308, 10-11=-897/1634, 9-10=-769/1443, 8-9=-285/522, 7-8=-47/64

2-11=-278/160, 2-10=-154/297, 3-10=-145/204, 3-9=-534/1090, 4-9=-277/214, 5-9=-990/1960, 5-8=-724/405,

1-11=-326/1279, 6-8=-330/719

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) Provide adequate drainage to prevent water ponding.

4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

Refer to girder(s) for truss to truss connections.

- 6) Bearing at joint(s) 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 318 lb uplift at joint 7 and 183 lb uplift at joint

LOAD CASE(S) Standard

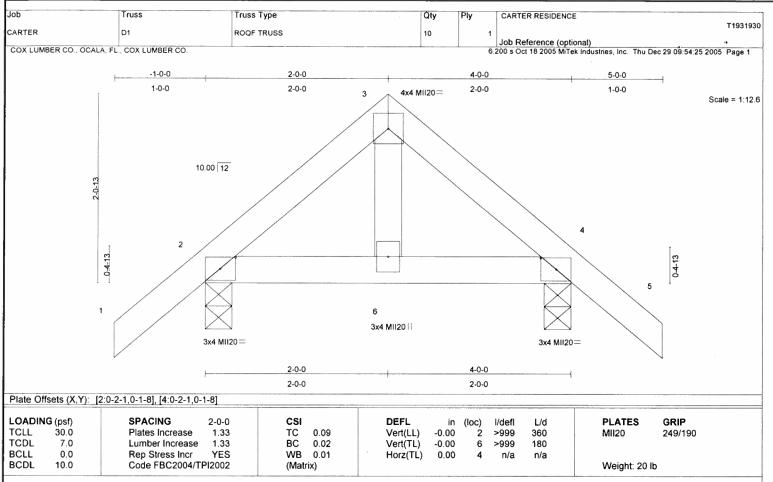
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December 29,2005

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Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D WEBS 2 X 4 SYP No.3 **BRACING**

TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=259/0-3-8, 4=259/0-3-8

Max Horz 2=64(load case 3)

Max Uplift2=-114(load case 4), 4=-114(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-158/16, 3-4=-158/15, 4-5=0/47

BOT CHORD 2-6=0/98, 4-6=0/98

WEBS 3-6=0/43

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 114 lb uplift at joint 2 and 114 lb uplift at joint 4

LOAD CASE(S) Standard

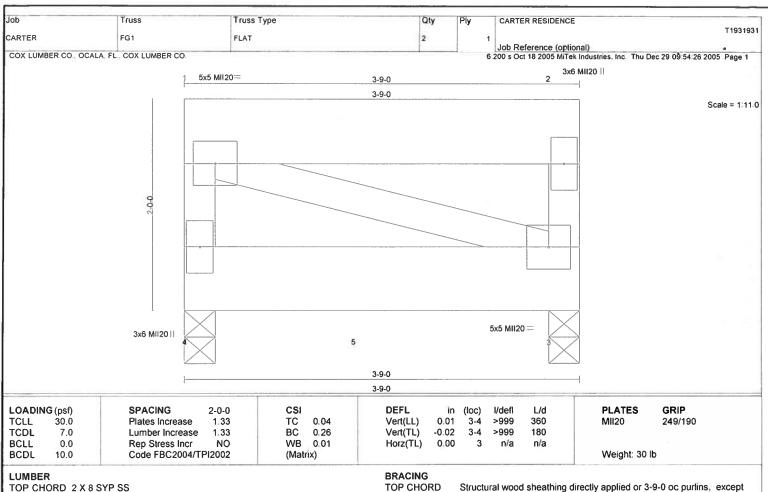
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December 29,2005

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TOP CHORD 2 X 8 SYP SS BOT CHORD 2 X 8 SYP SS

2 X 4 SYP No.3 WEBS

Structural wood sheathing directly applied or 3-9-0 oc purlins, except

end verticals:

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=685/0-3-8, 3=615/0-3-8

Max Horz 4=-60(load case 2)

Max Uplift4=-214(load case 2), 3=-193(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-4=-128/77, 1-2=-13/17, 2-3=-128/65

BOT CHORD 4-5=-43/47, 3-5=-43/47 1-3=-32/32

WEBS

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

2) Provide adequate drainage to prevent water ponding.

- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 214 lb uplift at joint 4 and 193 lb uplift at joint
- 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 975 lb down and 287 lb up at 1-9-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular, Lumber Increase=1.33, Plate Increase=1.33 Uniform Loads (plf) Vert 3-4=-20, 1-2=-74 Concentrated Loads (lb) Vert: 5=-975(F)

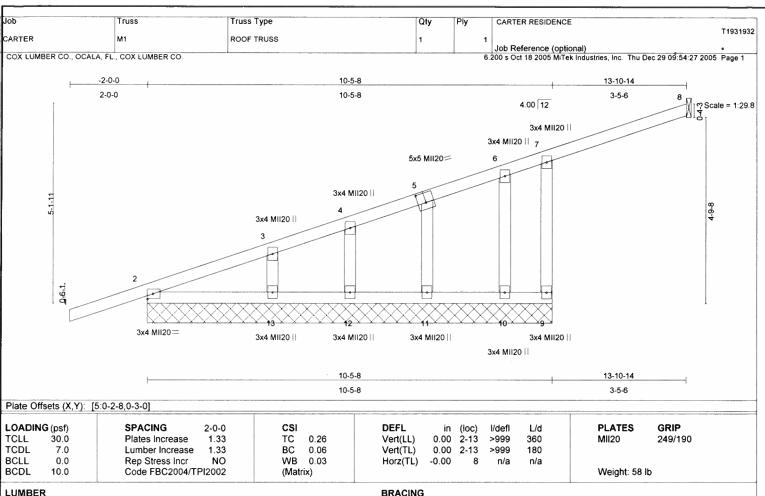
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TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

WEBS 2 X 4 SYP No.3 **OTHERS** 2 X 4 SYP No.3 **BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 8=105/Mechanical, 9=242/10-5-8, 2=331/10-5-8, 12=174/10-5-8, 13=221/10-5-8, 11=196/10-5-8, 10=110/10-5-8

Max Horz 2=232(load case 3)

Max Uplift8=-63(load case 2), 9=-116(load case 4), 2=-136(load case 2), 12=-76(load case 2), 13=-49(load case 4), 11=-63(load case 4), 10=-30(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-182/42, 3-4=-145/25, 4-5=-125/29, 5-6=-103/32, 6-7=-86/37, 7-8=-49/25, 7-9=-246/147

BOT CHORD 2-13=-34/45, 12-13=-34/45, 11-12=-34/45, 10-11=-32/48, 9-10=-32/48

WEBS 4-12=-131/81, 3-13=-185/103, 5-11=-157/92, 6-10=-64/37

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 8, 116 lb uplift at joint 9, 136 lb uplift at joint 2, 76 lb uplift at joint 12, 49 lb uplift at joint 13, 63 lb uplift at joint 11 and 30 lb uplift at joint 10.

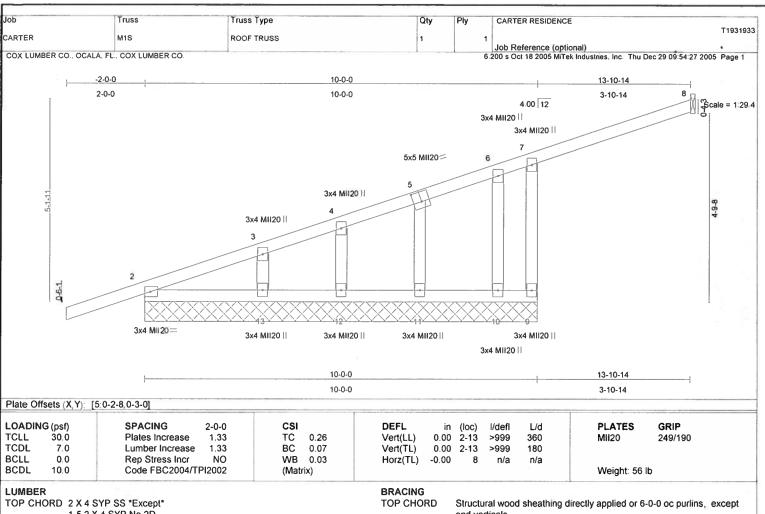
LOAD CASE(S) Standard

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1-5 2 X 4 SYP No. 2D

BOT CHORD 2 X 4 SYP No.2D **WEBS** 2 X 4 SYP No.3

2 X 4 SYP No.3 **OTHERS**

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing

REACTIONS (lb/size) 8=118/Mechanical, 9=282/10-0-0, 2=323/10-0-0, 12=188/10-0-0, 13=197/10-0-0, 11=191/10-0-0, 10=71/10-0-0

Max Horz 2=229(load case 3)

Max Uplift8=-70(load case 2), 9=-144(load case 4), 2=-134(load case 2), 12=-82(load case 2), 13=-42(load case 3), 11=-60(load case 4), 10=-8(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-184/46, 3-4=-148/35, 4-5=-127/36, 5-6=-105/35, 6-7=-92/49, 7-8=-56/28, 7-9=-296/180

BOT CHORD 2-13=-30/48, 12-13=-30/48, 11-12=-30/48, 10-11=-30/48, 9-10=-30/48

WEBS 4-12=-142/88, 3-13=-169/94, 5-11=-153/88, 6-10=-20/22

1) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable

3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

4) Gable studs spaced at 2-0-0 oc.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at joint 8, 144 lb uplift at joint 9, 134 Ib uplift at joint 2, 82 lb uplift at joint 12, 42 lb uplift at joint 13, 60 lb uplift at joint 11 and 8 lb uplift at joint 10.

LOAD CASE(S) Standard

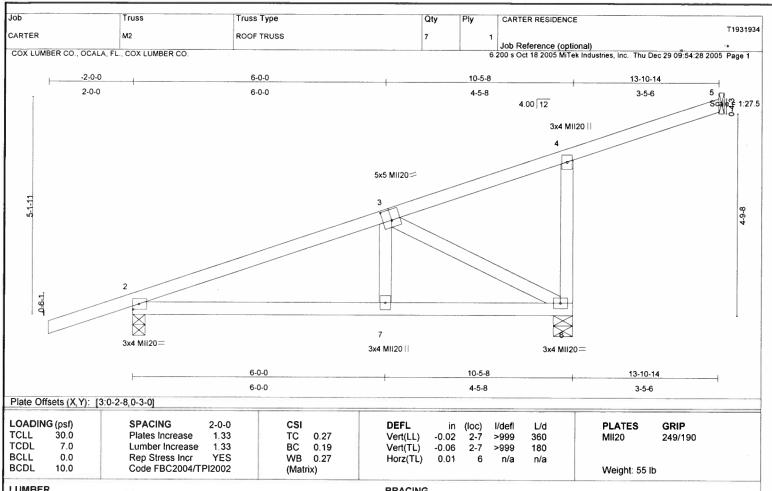
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LUMBER

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=100/Mechanical, 6=633/0-5-8, 2=643/0-3-8

Max Horz 2=232(load case 3)

Max Uplift5=-62(load case 2), 6=-239(load case 4), 2=-236(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/33, 2-3=-691/72, 3-4=-106/44, 4-5=-51/23, 4-6=-298/160

2-7=-139/574, 6-7=-142/568

BOT CHORD WEBS 3-7=0/123, 3-6=-644/210

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

2) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

3) Refer to girder(s) for truss to truss connections.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 62 lb uplift at joint 5, 239 lb uplift at joint 6 and 236 lb uplift at joint 2.

LOAD CASE(S) Standard

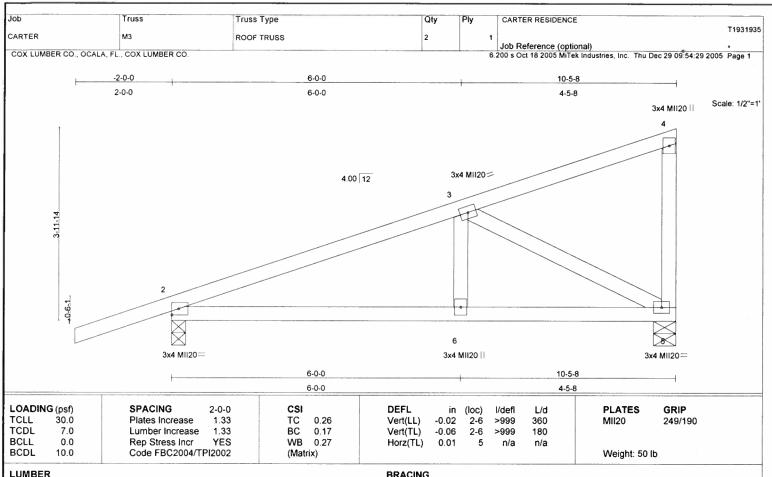
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TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D

WEBS 2 X 4 SYP No.3 **BRACING**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except

end verticals

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=461/0-5-8, 2=653/0-3-8

Max Horz 2=195(load case 3)

Max Uplift5=-142(load case 4), 2=-261(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/33, 2-3=-728/145, 3-4=-82/36, 4-5=-121/63 TOP CHORD **BOT CHORD**

2-6=-154/611, 5-6=-154/611 WEBS 3-6=0/115, 3-5=-662/211

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

2) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 142 lb uplift at joint 5 and 261 lb uplift at joint

LOAD CASE(S) Standard

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Truss Type CARTER RESIDENCE Truss Qty Ply T1931936 CARTER ROOF TRUSS M4 Job Reference (optional)
6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Dec 29 09:54:30 2005 Page 1 COX LUMBER CO., OCALA, FL., COX LUMBER CO. -2-0-0 5-0-0 8-9-8 2-0-0 5-0-0 3-9-8 3x4 MII20 II Scale = 1:56.3 3x6 MII20 // 10.00 12 3x6 MII20 // 1-10-10 \mathbf{x} 7 6 5 3x4 MII20 | 4x4 MII20 = 3x4 MII20= 5-0-0 8-9-8 5-0-0 3-9-8 LOADING (psf) SPACING 2-0-0 CSI **DEFL** in I/defl L/d **PLATES GRIP** (loc) **TCLL** 30.0 Plates Increase 1.33 TC 0.99 Vert(LL) -0.00 >999 360 MII20 249/190 6 TCDL 7.0 Lumber Increase 1 33 BC. 0.09 180 Vert(TL) -0.036-7 >999 BCLL 0.0 Rep Stress Incr YES WB 0.24 Horz(TL) -0.00 5 n/a n/a Code FBC2004/TPI2002 BCDL 10.0 (Matrix) Weight: 74 lb LUMBER **BRACING** TOP CHORD 2 X 4 SYP No.2D TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except BOT CHORD 2 X 4 SYP No.2D end verticals.

WEBS 2 X 4 SYP No.3

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing 4-5

WFBS

1 Row at midot

REACTIONS (lb/size) 7=578/0-5-8, 5=379/Mechanical

Max Horz 7=426(load case 3)

Max Uplift7=-144(load case 4), 5=-236(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-7=-539/163, 1-2=0/102, 2-3=-332/120, 3-4=-156/170, 4-5=-118/115

BOT CHORD 6-7=-406/265, 5-6=-167/198

2-6=-71/252, 3-6=-28/77, 3-5=-302/248 **WEBS**

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 2) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

3) Refer to girder(s) for truss to truss connections.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 7 and 236 lb uplift at joint

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

December 29,2005

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Qty Truss Type CARTER RESIDENCE Truss Ply T1931937 CARTER PB PIGGYBACK 26 Job Reference (optional)
6 200 s Oct 18 2005 MiTek Industries, Inc. Thu Dec 29 09:54:31 2005 Page 1 COX LUMBER CO., OCALA, FL., COX LUMBER CO 4-0-0 8-0-0 4-0-0 4-0-0 4x4 MII20= Scale = 1:21.8 3 10.00 12 4 5 3x4 MII20= 3x4 MII20 || 3x4 MII20 = 8-0-0 SPACING DEFL **PLATES GRIP** LOADING (psf) 2-0-0 CŞI in (loc) l/defl L/d 249/190 **TCLL** 30.0 Plates Increase 1.33 TC 0.16 Vert(LL) n/a n/a 999 MII20 1.33 BC 0.05 Vert(TL) 999 TCDL 7.0 Lumber Increase n/a n/a WB 0.03 BCLL 0.0 Rep Stress Incr YES 0.00 Horz(TL) n/a n/a Code FBC2004/TPI2002 Weight: 29 lb BCDL 10.0 (Matrix) LUMBER BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D **OTHERS** 2 X 4 SYP No.3

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=-215/8-0-0, 5=-215/8-0-0, 2=457/8-0-0, 4=457/8-0-0, 6=228/8-0-0

Max Horz 1=109(load case 3)

Max Uplift1=-215(load case 1), 5=-215(load case 1), 2=-337(load case 4), 4=-313(load case 5)

Max Grav 1=245(load case 4), 5=213(load case 5), 2=457(load case 1), 4=457(load case 1), 6=228(load case 1)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=-213/164, 2-3=-137/64, 3-4=-137/52, 4-5=-125/149 TOP CHORD

BOT CHORD 2-6=-25/65, 4-6=-25/65

3-6=-144/33 WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.

4) Gable requires continuous bottom chord bearing.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 215 lb uplift at joint 1, 215 lb uplift at joint 5, 337 lb uplift at joint 2 and 313 lb uplift at joint 4.

6) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 4, 6,

Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in. SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

December 29,2005

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Job Truss Truss Type Qty Ply CARTER RESIDENCE T1931938 CARTER PB1 PIGGYBACK 2 Job Reference (optional) 6.200 s Oct 18 2005 MiTek Industries, Inc. Thu Dec 29 09:54:31 2005 Page 1 COX LUMBER CO., OCALA, FL., COX LUMBER CO 3-10-11 7-9-7 3-10-11 3-10-12 4x4 MII20= Scale = 1:21.3 3 10.00 12 4 5 3x4 MII20= 3x4 MII20 || 3x4 MII20= 7-9-7 7-9-7 LOADING (psf) **SPACING** 2-0-0 CSI **DEFL** in I/defl L/d **PLATES GRIP TCLL** 30.0 1.33 TC 0.15 Vert(LL) 999 MII20 249/190 Plates Increase n/a n/a TCDL 7.0 BC. 0.05 999 Lumber Increase 1 33 Vert(TL) n/a n/a BCLL 0.0 Rep Stress Incr YES WB 0.03 Horz(TL) 0.00 n/a n/a Code FBC2004/TPI2002 BCDL 10.0 (Matrix) Weight: 28 lb LUMBER **BRACING** Structural wood sheathing directly applied or 6-0-0 oc purlins.

TOP CHORD 2 X 4 SYP No.2D BOT CHORD 2 X 4 SYP No.2D 2 X 4 SYP No.3 **OTHERS**

TOP CHORD **BOT CHORD**

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=-182/7-9-7, 5=-182/7-9-7, 2=419/7-9-7, 4=419/7-9-7, 6=219/7-9-7

Max Horz 1=-106(load case 2)

Max Uplift1=-182(load case 1), 5=-182(load case 1), 2=-303(load case 4), 4=-282(load case 5)

Max Grav 1=213(load case 4), 5=184(load case 5), 2=419(load case 1), 4=419(load case 1), 6=219(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-193/153, 2-3=-132/62, 3-4=-132/50, 4-5=-103/126

BOT CHORD 2-6=-24/63, 4-6=-24/63

WEBS 3-6=-138/32

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 182 lb uplift at joint 1, 182 lb uplift at joint 5, 303 lb uplift at joint 2 and 282 lb uplift at joint 4.
- 8) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 4, 6.
- 9) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744 MiTek Industries, Inc. 1801 Massaro Blvd Tampa FL 33619 FL Cert.#6634

December 29,2005

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FEDERAL EMERGENCY MANAGEMENT AGENCY NATIONAL FLOOD INSURANCE PROGRAM

O.M.B. No. 3067-0077 Expires December 31, 2005

ELEVATION CERTIFICATE

Important: Read the instructions on pages 1 - 7. SECTION A - PROPERTY OWNER INFORMATION For Insurance Company Use: **BUILDING OWNER'S NAME Policy Number** William B. & Joyce W. Carter Permit #2420**6** BUILDING STREET ADDRESS (Including Apt., Unit, Suite, and/or Bldg. No.) OR P.O. ROUTE AND BOX NO. Company NAIC Number 968 S.W. Bluff Drive CITY STATE ZIP CODE Fort White FI 32038 PROPERTY DESCRIPTION (Lot and Block Numbers, Tax Parcel Number, Legal Description, etc.) Lot 33, Cedar Springs Shores, Unit No. 5 BUILDING USE (e.g., Residential, Non-residential, Addition, Accessory, etc. Use a Comments area, if necessary.) Residential LATITUDE/LONGITUDE (OPTIONAL) HORIZONTAL DATUM: SOURCE: GPS (Type): (##° - ##' - ##.##" or ##.####") □ NAD 1927
□ NAD 1983 USGS Quad Map ☐ Other: SECTION B - FLOOD INSURANCE RATE MAP (FIRM) INFORMATION **B1. NFIP COMMUNITY NAME & COMMUNITY NUMBER B2. COUNTY NAME B3. STATE** Columbia Conty Unincorporated 120070 Columbia **B4. MAP AND PANEL B7. FIRM PANEL** B9. BASE FLOOD ELEVATION(S) NUMBER **B5. SUFFIX B6. FIRM INDEX DATE** EFFECTIVE/REVISED DATE **B8. FLOOD ZONE(S)** (Zone AO, use depth of flooding) 120070 0255 В 01/06/88 01/06//88 ΑE B10. Indicate the source of the Base Flood Elevation (BFE) data or base flood depth entered in B9. FIS Profile **⊠** FIRM ☐ Community Determined Other (Describe): B11. Indicate the elevation datum used for the BFE in B9: X NGVD 1929 ☐ NAVD 1988 ☐ Other (Describe): B12. Is the building located in a Coastal Barrier Resources System (CBRS) area or Otherwise Protected Area (OPA)? Yes No Designation Date SECTION C - BUILDING ELEVATION INFORMATION (SURVEY REQUIRED) C1. Building elevations are based on: Construction Drawings* ☐ Building Under Construction* □ Finished Construction *A new Elevation Certificate will be required when construction of the building is complete. C2. Building Diagram Number 1 (Select the building diagram most similar to the building for which this certificate is being completed - see pages 6 and 7. If no diagram accurately represents the building, provide a sketch or photograph.) C3. Elevations - Zones A1-A30, AE, AH, A (with BFE), VE, V1-V30, V (with BFE), AR, AR/A, AR/AE, AR/A1-A30, AR/AH, AR/AO Complete Items C3.-a-i below according to the building diagram specified in Item C2. State the datum used. If the datum is different from the datum used for the BFE in Section B, convert the datum to that used for the BFE. Show field measurements and datum conversion calculation. Use the space provided or the Comments area of Section D or Section G, as appropriate, to document the datum conversion. Datum NGVD 29 Conversion/Comments o a) Top of bottom floor (including basement or enclosure) 37. 1ft(m) o b) Top of next higher floor 45.93ft.(m) License Number, Embossed Signature, and Date o c) Bottom of lowest horizontal structural member (V zones only) N/A . ft.(m) o d) Attached garage (top of slab) <u>N/A</u>. __ft.(m) o e) Lowest elevation of machinery and/or equipment servicing the building (Describe in a Comments area) 37. 10 ft.(m) o f) Lowest adjacent (finished) grade (LAG) 33.7ft(m) o g) Highest adjacent (finished) grade (HAG) 34. 7ft(m) h) No. of permanent openings (flood vents) within 1 ft. above adjacent grade N/A o i) Total area of all permanent openings (flood vents) in C3.h N/A sq. in. (sq. cm) SECTION D - SURVEYOR, ENGINEER, OR ARCHITECT CERTIFICATION This certification is to be signed and sealed by a land surveyor, engineer, or architect authorized by law to certify elevation information. I certify that the information in Sections A, B, and C on this certificate represents my best efforts to interpret the data available. I understand that any false statement may be punishable by fine or imprisonment under 18 U.S. Code, Section 1001. CERTIFIER'S NAME KENNETH B. SARRIO LICENSE NUMBER 6348 TITLEPROFESSIONAL SURVEYOR & MAPPER COMPANY NAME DOVE & ASSOCIATES LAND SURVEYING INC. **ADDRESS** CITY STATE ZIP CODE 1762 FOLWER ST FORT MYERS FL 33901 SIGNATURE DATE TELEPHONE

99/18/06

239-332-7500

	the corresponding information from S			For Insurance Company Use:
	nit, Suite, and/or Bldg. No.) OR P.O. ROUTE AND B	OX NO.		Policy Number
968 S.W. Bluff Drive	STATE		IP CODE 2038	Company NAIC Number
Fort White SECTIO	FL ON D - SURVEYOR, ENGINEER, OR AF			
	for (1) community official, (2) insurance agent/o			
COMMENTS	or (1) community official, (2) insulative agentic	company, and (5) building owner		
Item in C3 e) is air conditioning equipment		- W		
				Check here if attachments
SECTION E - BUILDING EL	EVATION INFORMATION (SURVEY N	OT REQUIRED) FOR ZONE	AO AND ZONE	
For Zone AO and Zone A (without BFE), comp				
Section C must be completed.				
E1. Building Diagram Number _(Select the bu		which this certificate is being con	npleted – see pages	6 and 7. If no diagram accurately
represents the building, provide a sketch of E2. The top of the bottom floor (including base		in (cm) 🗆 above or 🗆 be	olow (check one) the	hinhest adiacent drade. (Lise
natural grade, if available).		ii.(cii) above oi ce	SON (CHECK ONE) THE	riignest aujaverit graue. 1000
E3. For Building Diagrams 6-8 with openings (see page 7), the next higher floor or elevated fl	oor (elevation b) of the building	isft.(m)in.(c	m) above the highest adjacent
grade. Complete items C3.h and C3.i on	front of form.			
E4. The top of the platform of machinery and/o	or equipment servicing the building isft.(m)	in.(cm) above or be	elow (check one) the	e highest adjacent grade. (Use
natural grade, if available). E5. For Zone AO only: If no flood depth numb	er is available, is the too of the hottom floor ele	vated in accordance with the co	mmunitv's floodolai	n management ordinance?
•	cal official must certify this information in Section			
	ON F - PROPERTY OWNER (OR OWNE		CERTIFICATION	
	presentative who completes Sections A, B, C			t a FEMA-issued or community-
11 11 11 11	ne statements in Sections A, B, C, and E are co	prect to the best of my knowled	gө.	D €
PROPERTY OWNER'S OR OWNER'S AU	THORIZED REPRESENTATIVE'S NAME			
ADDRESS		CITY	STATE	ZIP CODE
SIGNATURE		DATE	TELEPH	ONE
COMMENTS				
COMMENTS				
			12	Check here if attachments
	SECTION G - COMMUNITY IN			A. P. C.(or.E.) and C. of this Elevati
The local official who is authorized by law or or Certificate. Complete the applicable item(s) ar		ain management ordinance can	i complete Sections	A, B, C (OF E), and G OF IIIS Eleval
G1. The information in Section C was take		ned and embossed by a license	d surveyor, enginee	r, or architect who is authorized by
or local law to certify elevation information	ation. (Indicate the source and date of the elev	ration data in the Comments are	ea below.)	
G2. A community official completed Section			ued BFE) or Zone A	NO.
G3. The following information (Items G4-G		W CONTROL CO.		
G4. PERMIT NUMBER	G5. DATE PERMIT ISSUED	G6. DATE CERTI	IFICATE OF COMPLI	ANCE/OCCUPANCY ISSUED
G7. This permit has been issued for: New	Construction Substantial Improvement			
G8. Elevation of as-built lowest floor (including			ft(m)	Datum:
G9. BFE or (in Zone AO) depth of flooding at the	he building site is:		ft.(m)	Datum:
LOCAL OFFICIAL'S NAME		TITLE	··········	
COMMUNITY NAME	*2	TELEPHONE		
SIGNATURE		DATE		
COMMENTS				
		-		
				☐ Check here if attachments

520 (0) the signing party or parties is prohibited without the written consent of the signing party or parties.

The signing party or parties is prohibited without the written consent of the signing party or parties.

B) Underground utilities and structures were not field located. It is not a certification of title, zoning, easement, freedom of encumbrance, ownership, or rights-of-way.

10) Abstract not reviewed.

11) All Lot Lines are radial to the curve(s) unless otherwise noted. It is not partied to Lines are radial to the curve(s) unless otherwise noted.

12) Parcel lies in flood Zone AE Base Elevation = 36' as per FRH #120070 0255 B Dated: 06 JAN BB.

13) The elevations as shown hereon are based on NGVD 1929 Datum. REFE Cestificate of authorization .. B PARCEL DESCRIPTION:
Lot 33, Cedar Spring Shores Unit No. 5, accord
to the mcp or plot recorded in Plot 3ook 4, Pa5, 5A & 58, Public Records of Columbia County, .: <u>..</u> 9) = 81 WS SN&D = 2006 by This survey meets the minimum technical standards set forth by the Florida Board of Professional Land Surveyors pursuant to Section 472.027, Florida Statutes, and Chapter 61617-6, Florida Fl **LEGEND** Kenneth B. Sarrio Professional Surve Certificate No. 63 FAX (239) 332-8838 CONCRETE SEAWALL
ROXIMATE TOP BANK
PORARY BENCH MARK
LITY EASEMENT AGE FLOOR ELEVATION PROPERTY Dove & Associates OF PAVEKENT
D CONCRETE MONUMENT
D DRILL HOLE
SH FLOOR ELEVATION
D IRON ROD TRESS 5/5" TRON ROD BEAR : NGS LOCATION OF WWW.DOVESURVEY.COM Surveyor No. 6348 ROD & CAP 80 0.00 5000 Mapper \otimes Q \circ \circ \bullet [-1] W. 53901 = ELEVATION = WATER VALVE = FIRE HYDRANT FICE THOSE Surveying reserretions, POWER POLE SERVICE BOX ELECTRIC SERVICE WOOD FENCE SHAIN LINK FENCE STORM DRAIN WATER METER TELEPHONE SERVICE BOX i~ UD OVERHEAD B0X rding Pages Y, Fiorida REFERENCE LINES WHELL ST FIR PSM 5757 BIAEB WOOD DOC Q Ketc CERTIFIED T Co State And Series and Andrew 0 THE BOK LINE PER COLUMBIA COUNTY WASHING L'ADOSS ٣٠ ت Joyce 8 B . 7. oundar C Survey 010.57 LOT 36 PROPOSED DETATCHED 1 0731 Drawn 5. 01/3/5/ 5. 01/3/5/X ,'0.', 'e. 0/ PEWER 0. 0.0h .0 Sca FN IN POWER POLE
ELEVATION = (34.6) 9 -3 J 0 G C , q^Ç ANGLE BREAK A 5 DMO 0 0 ISTIBYCK TIME SS. [RON] \bigcirc 99 87 MARK Π TAM TO THOTAL OF 0 0) 383.72, (RIVER ROAD PER PLAT) X \bigcirc 7.92.10.21 BINEL 0 0 DRIVE S 17°00′56″E(M) 73.34'(M) S 17°01'43"E(P) 73,40'(P)

