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COA #0 278

Florida Certificate of Product Approval #FL 1999
03/06/2023

Alpine, an ITW Company
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025
Phone: (800)755-6001
www.alpineitw.com

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	*Job Number:* 23-9006
Job Description: Nickelson Shed	
Address: Nickelson Shed, FL	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	*IntelliVIEW Version:* 22.02.00 *JRef #:* 1XNR2150007
Wind Standard: ASCE 7-16 *Wind Speed (mph):* 130 *Building Type:* Closed	*Design Loading (psf):* 40.00, 55.00

This package contains general notes pages, 18 truss drawing(s) and 4 detail(s).

Item	Drawing Number	Truss
1	065.23.1509.21327	A1
3	065.23.1509.43430	B1
5	065.23.1509.58270	FL1
7	065.23.1511.16897	FT1
9	065.23.1512.07710	FT2
11	065.23.1512.58673	FT4
13	065.23.1458.20480	FT6
15	065.23.1513.24670	J1
17	065.23.1513.34887	J5
19	160TL	
21	BRCLBSUB0119	

Item	Drawing Number	Truss
2	065.23.1509.41137	A1E
4	065.23.1509.56177	B1E
6	065.23.1510.11873	FL2
8	065.23.1511.54933	FT1A
10	065.23.1512.43193	FT3
12	065.23.1513.14283	FT5
14	065.23.1513.22800	HGJ1
16	065.23.1513.33087	J3
18	065.23.1513.38710	J6
20	A14030ENC160118	
22	GBLLETIN0118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

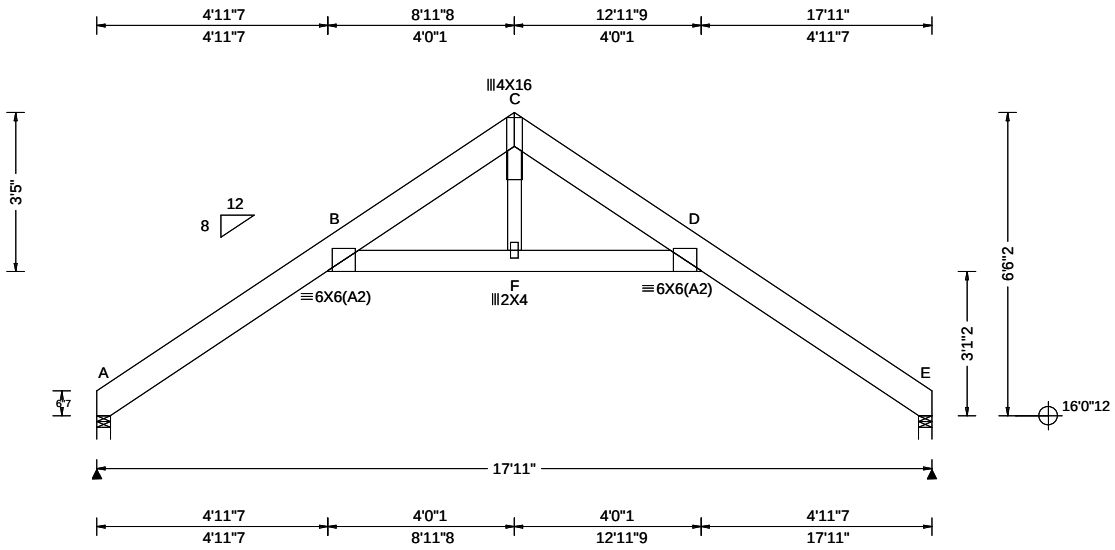
BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com.

SEQN: 543761 COMN FROM: RFG	Ply: 1 Qty: 8	**Job Number:** 23-9006 Nickelson Shed **Truss Label:** A1	Cust: R 215 JRef:1XNR2150007 T7 DrwNo: 065.23.1509.21327 KD / WHK 03/06/2023



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 19.59 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.424	C	491	360
VERT(CL): 0.774	C	269	240
HORZ(LL): 0.282	D	-	-
HORZ(TL): 0.514	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.473
Max BC CSI: 0.077
Max Web CSI: 0.023

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	654	/-	/-	/343	/218	/168
E	654	/-	/-	/343	/218	/-

Wind reactions based on MWFRS
A Brg Wid = 3.5 Min Req = 1.5 (Support)
E Brg Wid = 3.5 Min Req = 1.5 (Support)
Bearings A & E Fcperp = 565psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	279	-1203	C - D	279	-1203

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - F	1319	-115	F - D	1319	-115

Lumber

Top chord: 2x8 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Wind

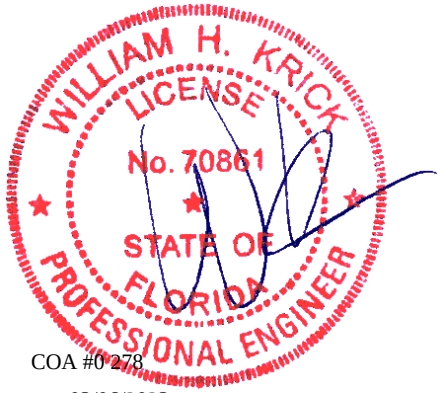
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

The overall height of this truss excluding overhang is 14-6-14.



COA #0 278
03/06/2023
Florida Certificate of Product Approval #FL 1999

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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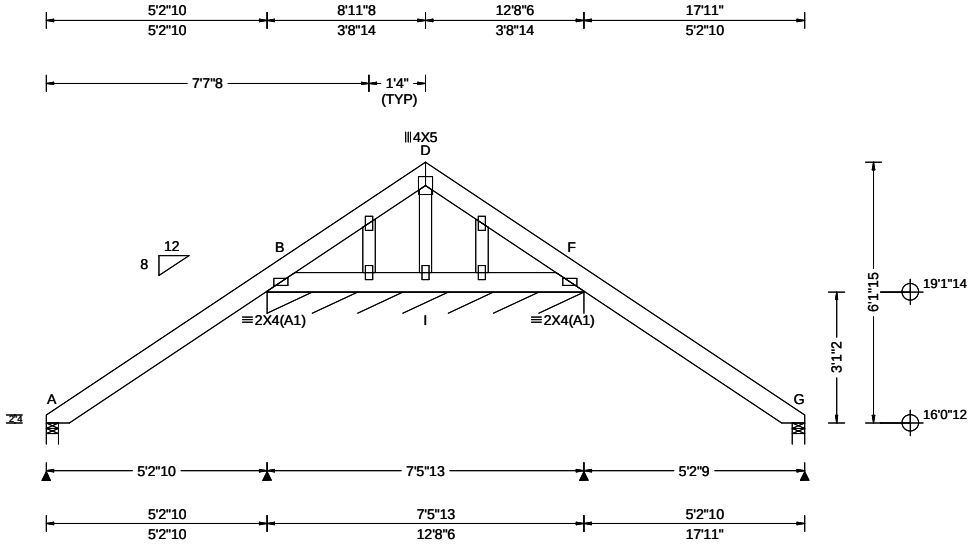
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543719 GABL FROM: RFG	Ply: 1 Qty: 2	**Job Number:** 23-9006 Nickelson Shed **Truss Label:** A1E	Cust: R 215 JRef:1XNR2150007 T8 DrwNo: 065.23.1509.41137 KD / WHK 03/06/2023



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 19.24 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 6.06 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.001	B	999	360
VERT(CL): 0.002	B	999	240
HORZ(LL): 0.002	F	-	-
HORZ(TL): 0.002	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.106
Max BC CSI: 0.011
Max Web CSI: 0.040

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
A	203	/-	/-	/170	/139	/234
B*	193	/-	/-	/103	/48	/-
G	203	/-	/-	/137	/139	/-

Wind reactions based on MWFRS
A Brg Wid = 3.5 Min Req = 1.5 (Support)
B Brg Wid = 89.8 Min Req = -
G Brg Wid = 3.5 Min Req = 1.5 (Support)
Bearing B is a rigid surface.
Bearings A & G Fcperp = 565psi.
Members not listed have forces less than 375#

Lumber

Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 7.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

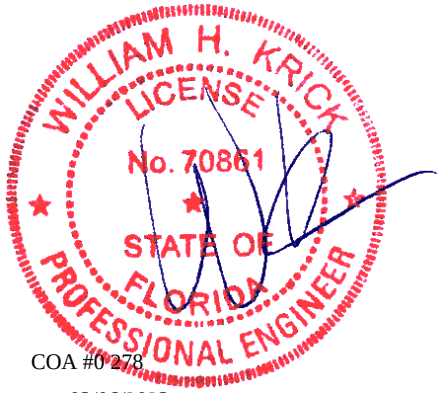
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

The overall height of this truss excluding overhang is 14-2-11.



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03/06/2023

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****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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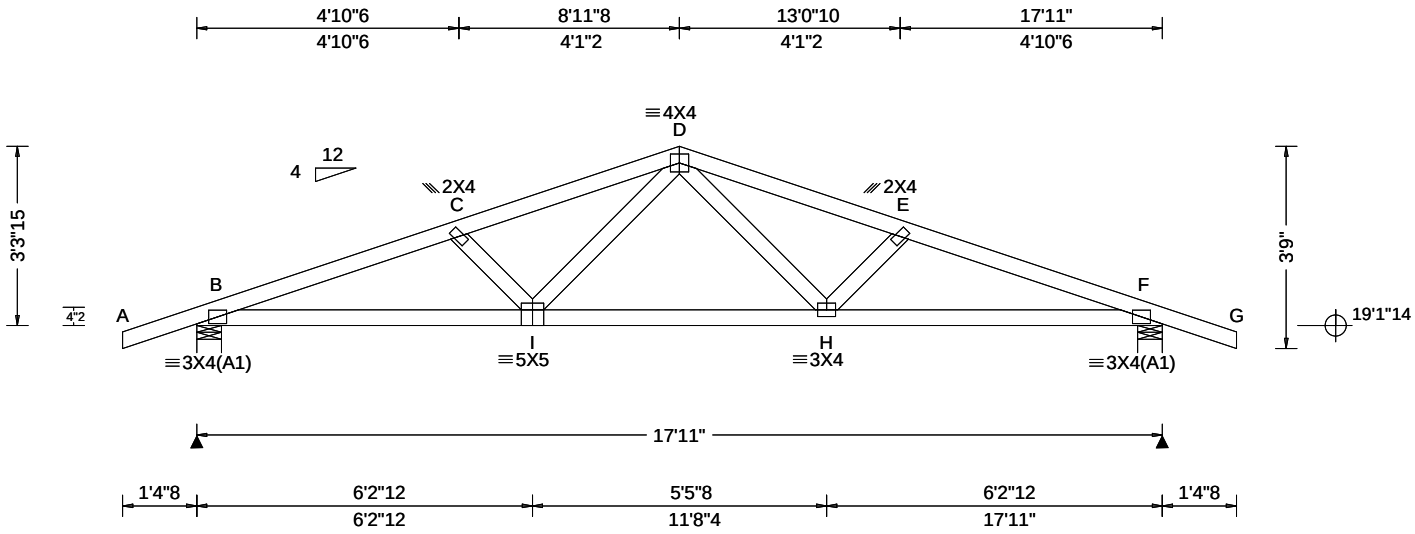
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543758 COMN FROM: RFG	Ply: 1 Qty: 6	**Job Number:** 23-9006 Nickelson Shed **Truss Label:** B1	Cust: R 215 JRef:1XNR2150007 T1 DrwNo: 065.23.1509.43430 KD / WHK 03/06/2023



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 20.76 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.062	D	999	360
VERT(CL): 0.122	D	999	240
HORZ(LL): 0.019	F	-	-
HORZ(TL): 0.037	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.209
Max BC CSI: 0.433
Max Web CSI: 0.156

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	816	/-	/-	/425	/155	/68
F	816	/-	/-	/425	/155	/-

Wind reactions based on MWFRS
B Brg Wid = 5.5 Min Req = 1.5 (Truss)
F Brg Wid = 5.5 Min Req = 1.5 (Truss)
Bearings B & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	309	- 1585	D - E	277	- 1411
C - D	278	- 1410	E - F	309	- 1586

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - I	1466	- 242	H - F	1467	- 242
I - H	1027	- 132			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
I - D	409	- 47	D - H	410	- 46

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

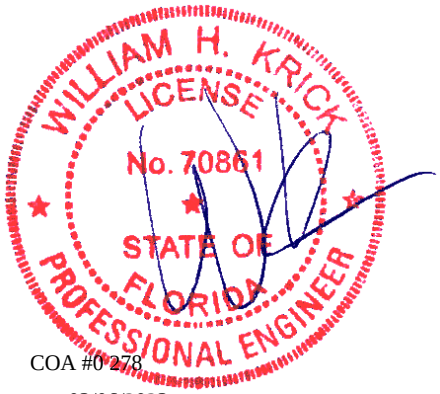
Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-3-15.



COA #0278
03/06/2023
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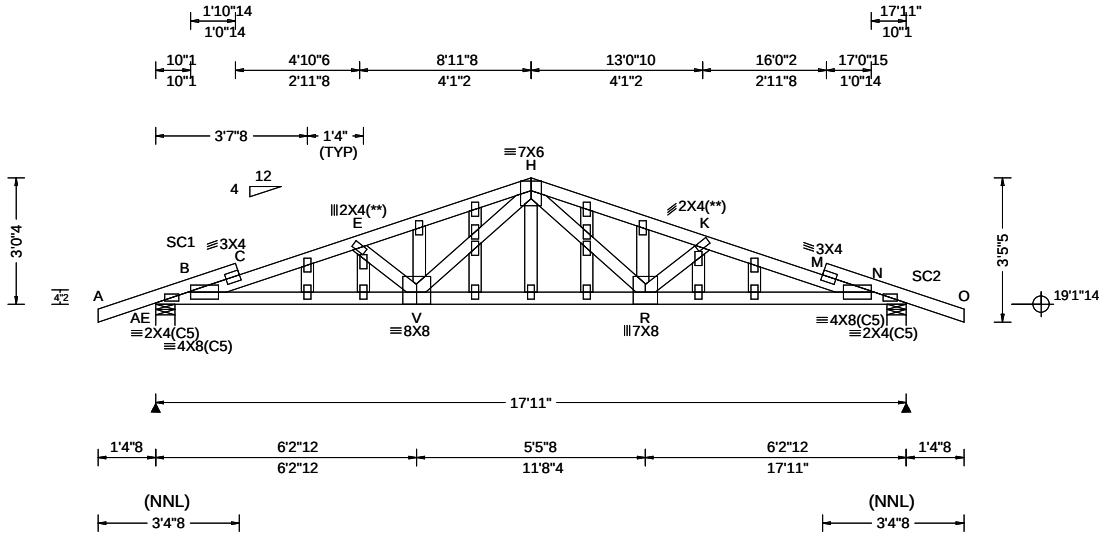
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543716 GABL FROM: RFG	Ply: 1 Qty: 2	**Job Number:** 23-9006 Nickelson Shed **Truss Label:** B1E		Cust: R 215 JRef:1XNR2150007 T2 DrwNo: 065.23.1509.56177 KD / WHK 03/06/2023	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 20.61 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 12.11 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.123	I	999	360
VERT(CL): 0.246	I	846	240
HORZ(LL): 0.019	D	-	-
HORZ(TL): 0.037	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.758
Max BC CSI: 0.528
Max Web CSI: 0.369

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AE	1145	/-	/-	/524	/461	/94
N	1145	/-	/-	/524	/461	/-

Wind reactions based on MWFRS
AE Brg Wid = 5.5 Min Req = 1.5 (Truss)
N Brg Wid = 5.5 Min Req = 1.5 (Truss)
Bearings AE & N are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	935	- 2695	H - K	883	- 2240
C - E	960	- 2640	K - M	960	- 2640
E - H	884	- 2241	M - N	935	- 2695

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - V	2510	- 804	R - N	2510	- 804
V - R	1603	- 435			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
E - V	232	- 581	H - R	681	- 278
V - H	683	- 279	R - K	233	- 583

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #2;
Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 7.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

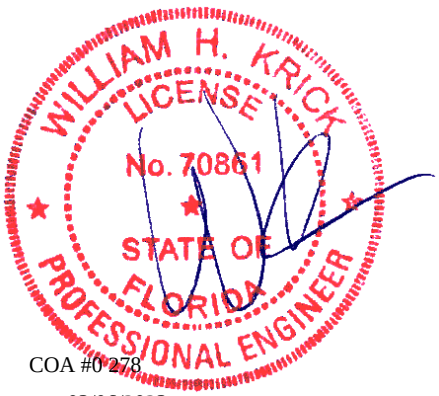
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 3-0-4.



COA #0 278
03/06/2023
Florida Certificate of Product Approval #FL 1999

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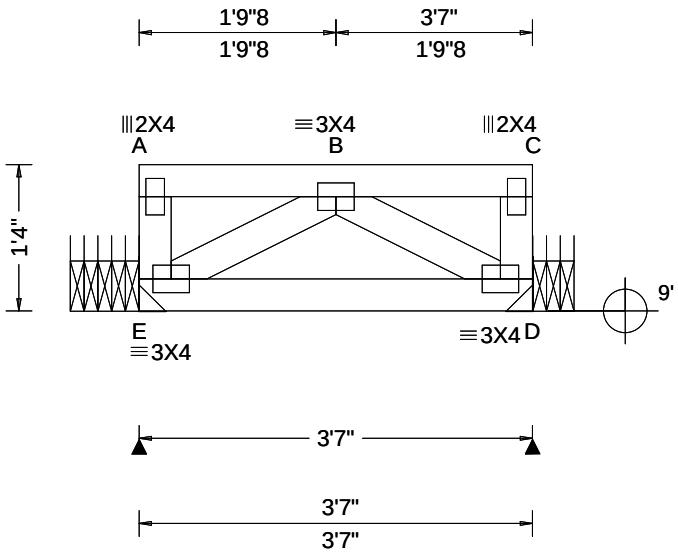
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543743 FLAT FROM: RFG	Ply: 1 Qty: 2	**Job Number:** 23-9006 Nickelson Shed **Truss Label:** FL1	Cust: R 215 JRef:1XNR2150007 T19 DrwNo: 065.23.1509.58270 KD / WHK 03/06/2023



Loading Criteria (psf)
TCLL: 40.00
TCDL: 10.00
BCLL: 0.00
BCDL: 5.00
Des Ld: 55.00
NCBCLL: 10.00
Soffit: 0.00
Load Duration: 1.00
Spacing: 24.0 "

Wind Criteria
Wind Std: NA
Speed: NA mph
Enclosure: NA
Category: NA
EXP: NA Kzt: NA
Mean Height: NA ft
TCDL: NA psf
BCDL: NA psf
MWFRS Parallel Dist: NA
C&C Dist a: NA ft
Loc. from endwall: NA
I: NA GCpi: NA
Wind Duration: NA

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.002	B	999	480
VERT(CL): 0.002	B	999	360
HORZ(LL): 0.001	D	-	-
HORZ(TL): 0.001	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.065
Max BC CSI: 0.106
Max Web CSI: 0.049

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

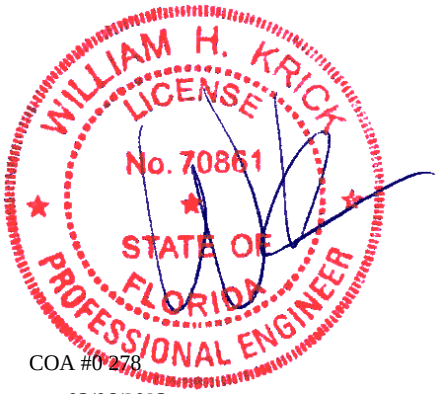
	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E	197	/-	/-	/-	/-	/-
D	197	/-	/-	/-	/-	/-

E Brg Wid = - Min Req = -
D Brg Wid = - Min Req = -
Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Hangers / Ties
(J) Hanger Support Required, by others

Additional Notes
Truss must be installed as shown with top chord up.
The overall height of this truss excluding overhang is 1-4-0.



COA #0 278
03/06/2023
Florida Certificate of Product Approval #FL 1999

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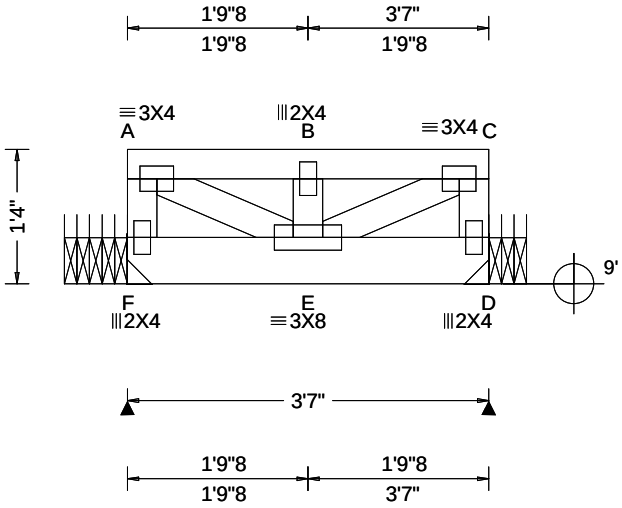
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543745 FLAT FROM: RFG	Ply: 2 Qty: 1	**Job Number:** 23-9006 Nickelson Shed **Truss Label:** FL2	Cust: R 215 JRef:1XNR2150007 T5 DrwNo: 065.23.1510.11873 KD / WHK 03/06/2023	

2 Complete Trusses Required



Loading Criteria (psf)
TCLL: 40.00
TCDL: 10.00
BCLL: 0.00
BCDL: 5.00
Des Ld: 55.00
NCBCLL: 10.00
Soffit: 0.00
Load Duration: 1.00
Spacing: 24.0 "

Wind Criteria
Wind Std: NA
Speed: NA mph
Enclosure: NA
Category: NA
EXP: NA Kzt: NA
Mean Height: NA ft
TCDL: NA psf
BCDL: NA psf
MWFRS Parallel Dist: NA
C&C Dist a: NA ft
Loc. from endwall: NA
I: NA GCpi: NA
Wind Duration: NA

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.004	B	999	480
VERT(CL): 0.005	B	999	360
HORZ(LL): 0.001	A	-	-
HORZ(TL): 0.001	A	-	-

Creep Factor: 2.0
Max TC CSI: 0.050
Max BC CSI: 0.087
Max Web CSI: 0.189

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
F	998	/-	/-	/-	/-	/-
D	762	/-	/-	/-	/-	/-

F Brg Wid = - Min Req = -
D Brg Wid = - Min Req = -
Members not listed have forces less than 375#

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
A - E	397	0	E - C	398	0

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

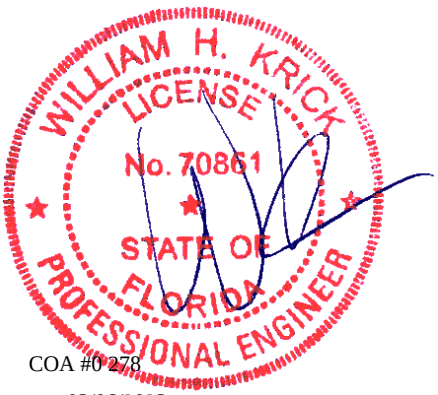
Nailnote
Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 2 Rows @ 6.00" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
------(Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)
TC: From 50 plf at 0.00 to 50 plf at 3.58
BC: From 5 plf at 0.00 to 5 plf at 3.58
BC: 781 lb Conc. Load at 0.52, 2.52

Hangers / Ties
(J) Hanger Support Required, by others

Purlins
The TC of this truss shall be braced with attached spans at 24" oc in lieu of structural sheathing.

Additional Notes
Truss must be installed as shown with top chord up.
The overall height of this truss excluding overhang is 1-4-0.



COA #0278
03/06/2023
Florida Certificate of Product Approval #FL 1999

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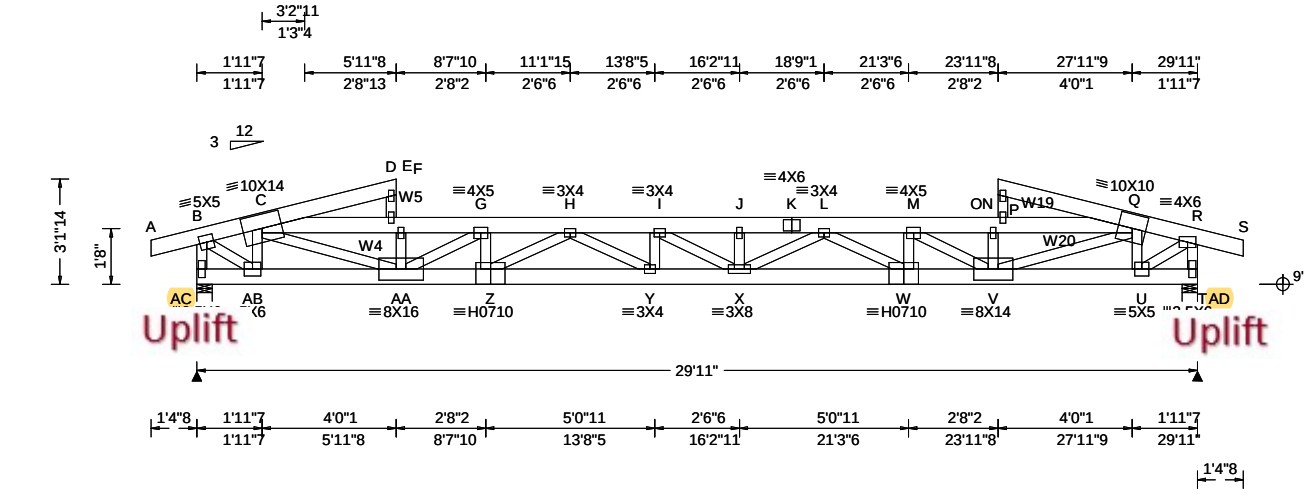
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543753 COMN FROM: RFG Page 1 of 2	Ply: 5 Qty: 1	**Job Number:** 23-9006 Nickelson Shed **Truss Label:** FT1	Cust: R 215 JRef:1XNR2150007 T10 DrwNo: 065.23.1511.16897 KD / WHK 03/06/2023

5 Complete Trusses Required



Loading Criteria (psf)

TCLL:	40.00
TCDL:	10.00
BCLL:	0.00
BCDL:	5.00
Des Ld:	55.00
NCBCLL:	0.00
Soffit:	2.00
Load Duration:	1.00
Spacing:	24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 3.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.409	I	878	360
VERT(CL): 0.761	I	471	240
HORZ(LL): 0.105	E	-	-
HORZ(TL): 0.197	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.284
Max BC CSI: 0.633
Max Web CSI: 0.931

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AC	6779	/-	/-	/106	/612	/-
AD	6468	/-	/-	/106	/613	/-

Wind reactions based on MWFRS
AC Brg Wid = 5.5 Min Req = 1.5 (Truss)
AD Brg Wid = 5.5 Min Req = 1.5 (Truss)
Bearings AC & AD are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	131	- 1510	J - K	604	- 7214
C - D	379	- 4644	K - L	604	- 7214
D - F	378	- 4662	L - M	505	- 5869
F - G	378	- 4662	M - N	379	- 4433
G - H	504	- 6258	N - P	379	- 4433
H - I	602	- 7403	P - Q	380	- 4415
I - J	604	- 7214	Q - R	131	- 1437

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
AB-AA	1596	- 132	X - W	6655	- 559
AA- Z	6115	- 491	W - V	5744	- 492
Z - Y	6961	- 558	V - U	1519	- 132
Y - X	7408	- 603			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B -AC	128	- 1431	X - L	673	- 52
B -AB	1809	- 152	L - W	65	- 954
AB- C	81	- 896	W - M	680	- 67
C -AA	3302	- 264	M - V	136	- 1576
AA- G	135	- 1744	V - Q	3138	- 265
G - Z	777	- 66	Q - U	81	- 855
Z - H	65	- 855	U - R	1721	- 152
H - Y	543	- 52	R - T	128	- 1363

Lumber

Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W4,W20 2x4 SP #2; W5,
W19 2x4 SP M-31;

Nailnote

Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @ 3.75" o.c.
Bot Chord: 1 Row @ 7.25" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.
In addition apply (1) 1/2" bolt, or 0.22"-0.25" min/max dia. X 4.5" length wood screw from each outside face, at each joint location.

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads and reactions based on MWFRS.

End verticals exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-1-14.

Note: Laterally brace top chord below filler at 2'0" O.C.Max. including a lateral brace at chord ends.

COA #0278
03/06/2023
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ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Special Loads

------(Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)

TC: From	101 plf at	-1.38 to	101 plf at	5.96
TC: From	259 plf at	5.96 to	341 plf at	14.96
TC: From	341 plf at	14.96 to	316 plf at	17.79
TC: From	316 plf at	17.79 to	260 plf at	23.93
TC: From	310 plf at	23.93 to	310 plf at	23.96
TC: From	101 plf at	23.96 to	101 plf at	31.29
BC: From	4 plf at	-1.38 to	4 plf at	0.00
BC: From	10 plf at	0.00 to	10 plf at	5.99
BC: From	5 plf at	5.99 to	5 plf at	23.93
BC: From	10 plf at	23.93 to	10 plf at	29.92
BC: From	4 plf at	29.92 to	4 plf at	31.29

TC: 689 lb Conc. Load at 6.10,23.81
BC: 767 lb Conc. Load at 5.99,23.93
BC: 230 lb Conc. Load at 8.02,10.02,12.02,14.02 15.90,17.90,19.90,21.90
BC: 998 lb Conc. Load at 10.83
BC: 197 lb Conc. Load at 12.56,14.56

COA #0278

03/06/2023

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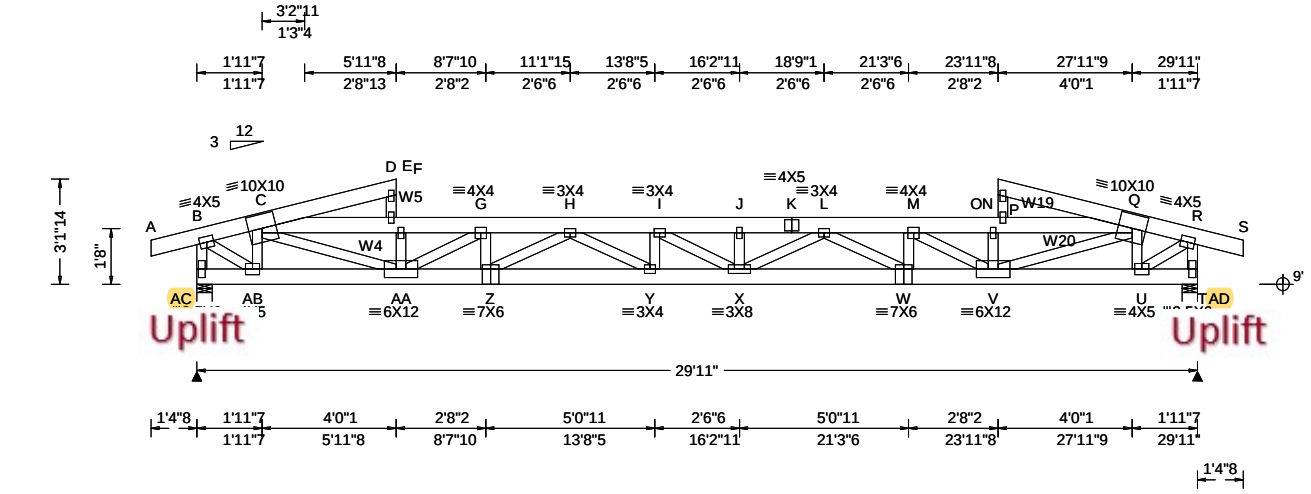
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543755 COMN FROM: RFG	Ply: 5 Qty: 1	**Job Number:** 23-9006 Nickelson Shed **Truss Label:** FT1A	Cust: R 215 JRef:1XNR2150007 T6 DrwNo: 065.23.1511.54933 KD / WHK 03/06/2023

5 Complete Trusses Required



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 40.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.315 I 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.594 I 604 240
BCDL: 5.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.081 E - -
Des Ld: 55.00	EXP: C Kzt: NA		HORZ(TL): 0.152 E - -
NCBCLL: 0.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.189
Load Duration: 1.00	BCDL: 3.0 psf	TPI Std: 2014	Max BC CSI: 0.446
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.701
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	VIEW Ver: 22.02.00.0914.12
	GCpi: 0.18	WAVE	
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AC	5232	/-	/-	/2	/614	/-
AD	5233	/-	/-	/2	/614	/-

Wind reactions based on MWFRS
AC Brg Wid = 5.5 Min Req = 1.5 (Truss)
AD Brg Wid = 5.5 Min Req = 1.5 (Truss)
Bearings AC & AD are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	131	- 1151	J - K	606	- 5750
C - D	381	- 3510	K - L	606	- 5750
D - F	379	- 3523	L - M	506	- 4733
F - G	379	- 3523	M - N	380	- 3523
G - H	506	- 4730	N - P	380	- 3523
H - I	605	- 5731	P - Q	381	- 3510
I - J	606	- 5750	Q - R	131	- 1151

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
AB-AA	1215	- 132	X - W	5361	- 560
AA- Z	4629	- 493	W - V	4630	- 493
Z - Y	5357	- 560	V - U	1216	- 132
Y - X	5761	- 605			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B -AC	129	- 1101	X - L	473	- 53
B -AB	1377	- 153	L - W	65	- 766
AB- C	82	- 686	W - M	560	- 67
C -AA	2485	- 265	M - V	136	- 1332
AA- G	136	- 1331	V - Q	2485	- 265
G - Z	555	- 66	Q - U	82	- 686
Z - H	66	- 764	U - R	1377	- 153
H - Y	463	- 52	R - T	129	- 1101

Lumber

Top chord: 2x6 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3; W4,W20 2x4 SP #2; W5, W19 2x4 SP M-31;

Nailnote

Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @ 4.50" o.c.
Bot Chord: 1 Row @10.00" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.
In addition apply (1) 1/2" bolt, or 0.22"-0.25" min/max dia. X 4.5" length wood screw from each outside face, at each joint location.

Special Loads

------(Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)

TC: From	101 plf at	-1.38 to	101 plf at	5.96
TC: From	126 plf at	5.96 to	127 plf at	6.00
TC: From	260 plf at	6.00 to	341 plf at	14.96
TC: From	341 plf at	14.96 to	316 plf at	17.79
TC: From	316 plf at	17.79 to	260 plf at	23.92
TC: From	127 plf at	23.92 to	127 plf at	23.93
TC: From	177 plf at	23.93 to	177 plf at	23.96
TC: From	101 plf at	23.96 to	101 plf at	31.29
BC: From	4 plf at	-1.38 to	4 plf at	0.00
BC: From	10 plf at	0.00 to	10 plf at	5.99
BC: From	5 plf at	5.99 to	5 plf at	23.93
BC: From	10 plf at	23.93 to	10 plf at	29.92
BC: From	4 plf at	29.92 to	4 plf at	31.29

BC: 767 lb Conc. Load at 5.99,23.93
BC: 230 lb Conc. Load at 8.02,10.02,12.02,14.02 15.90,17.90,19.90,21.90

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads and reactions based on MWFRS.

End verticals exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-1-14.

Note: Laterally brace top chord below filler at 24" O.C.Max. including a lateral brace at chord ends.

COA #0 278
03/06/2023
Florida Certificate of Product Approval #FL 1999

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
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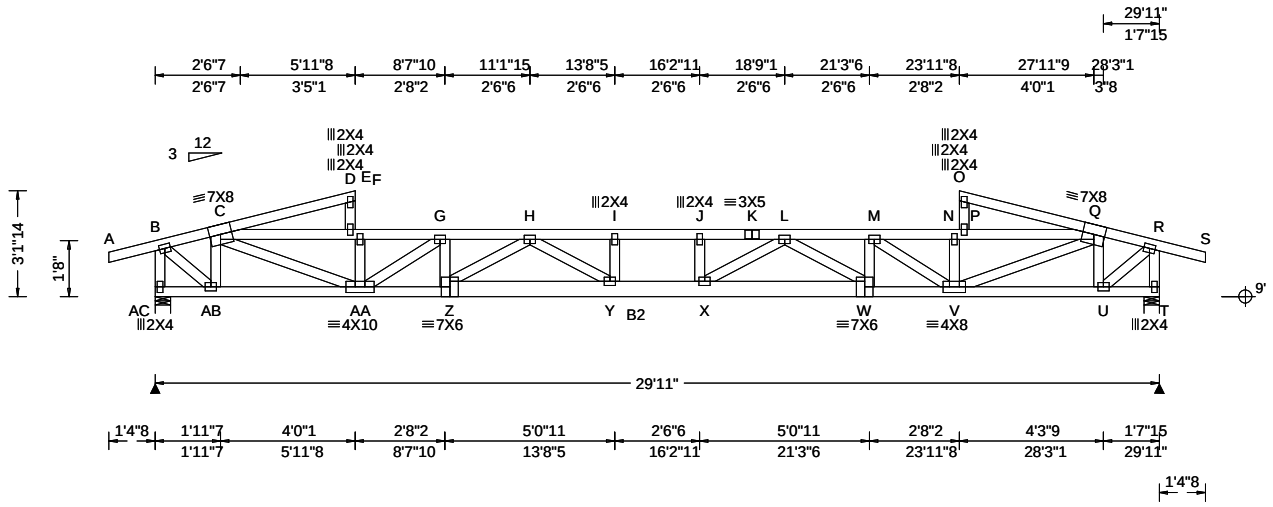
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543751 COMN FROM: RFG	Ply: 3 Qty: 1	**Job Number:** 23-9006 Nickelson Shed **Truss Label:** FT2	Cust: R 215 JRef:1XNR2150007 T20 DrwNo: 065.23.1512.07710 KD / WHK 03/06/2023	

3 Complete Trusses Required



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 40.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.341 I 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.467 I 769 240
BCDL: 5.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.098 E - -
Des Ld: 55.00	EXP: C Kzt: NA		HORZ(TL): 0.134 E - -
NCBCLL: 0.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.296
Load Duration: 1.00	BCDL: 3.0 psf	TPI Std: 2014	Max BC CSI: 0.686
Spacing: 16.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.608
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 22.02.00.0914.12
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AC	1821	/-	/-	/-	/197	/-
T	1595	/-	/-	/-	/201	/-

Wind reactions based on MWFRS
AC Brg Wid = 5.5 Min Req = 1.5 (Truss)
T Brg Wid = 5.5 Min Req = 1.5 (Truss)
Bearings AC & T are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	57	- 546	J - K	238	- 2812
C - D	154	- 1743	K - L	238	- 2812
D - F	152	- 1751	L - M	205	- 2025
F - G	152	- 1751	M - N	156	- 1490
G - H	198	- 2448	N - P	156	- 1490
H - I	238	- 2827	P - Q	159	- 1483
I - J	239	- 2823	Q - R	58	- 471

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
AB-AA	583	- 51	X - W	2442	- 231
AA- Z	2377	- 193	W - V	1974	- 199
Z - Y	2700	- 223	V - U	503	- 53
Y - X	2823	- 237			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B -AC	66	- 613	X - L	549	- 6
B -AB	682	- 64	L - W	34	- 528
AB- C	45	- 413	M - V	56	- 612
C -AA	1280	- 107	V - Q	1082	- 110
AA- G	54	- 787	U - R	587	- 66
G - Z	420	- 17	R - T	67	- 535

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2; B2 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #3;

Nailnote

Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

------(Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)

TC: From	67 plf at	-1.38 to	67 plf at	10.83
TC: From	34 plf at	10.83 to	34 plf at	14.56
TC: From	67 plf at	14.56 to	67 plf at	31.29
BC: From	3 plf at	-1.38 to	3 plf at	0.00
BC: From	7 plf at	0.00 to	7 plf at	10.83
BC: From	3 plf at	10.83 to	3 plf at	14.56
BC: From	7 plf at	14.56 to	7 plf at	29.92
BC: From	3 plf at	29.92 to	3 plf at	31.29

BC: 762 lb Conc. Load at 10.83
BC: 197 lb Conc. Load at 12.56,14.56

Plating Notes

All plates are 3X4 except as noted.

Additional Notes

The overall height of this truss excluding overhang is 3-1-14.

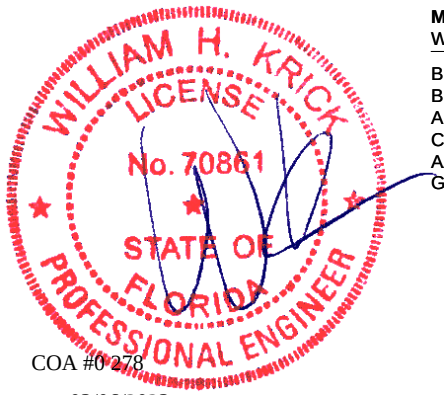
Wind

Wind loads and reactions based on MWFRS.

End verticals exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.

Note: Laterally brace top chord below filler at 2'0" O.C.Max. including a lateral brace at chord ends.



COA #0 278
03/06/2023
Florida Certificate of Product Approval #FL 1999

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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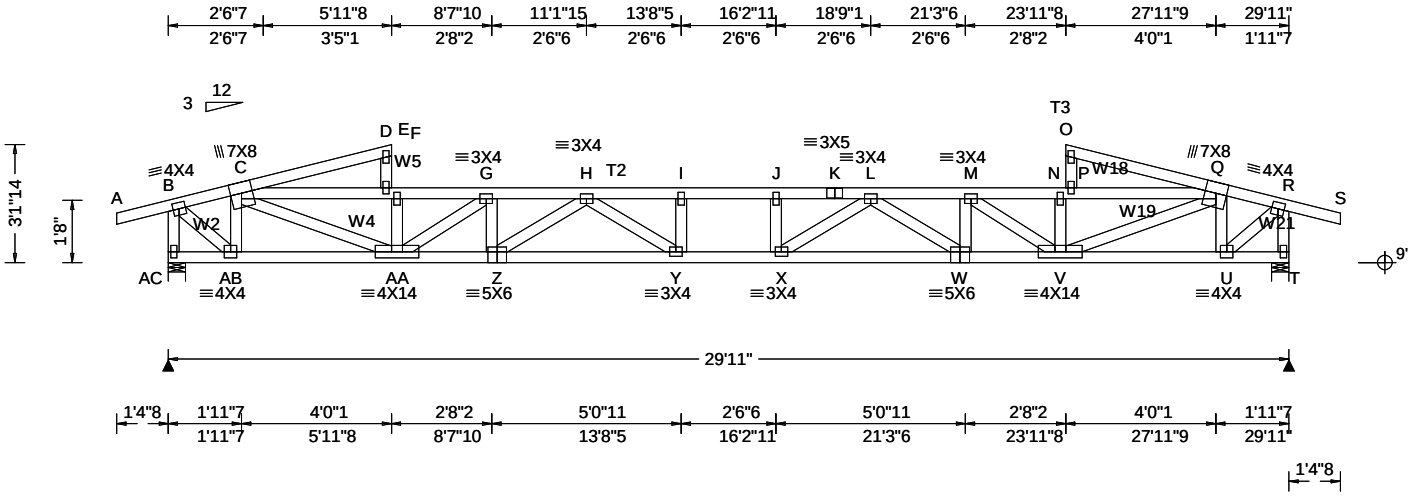
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543782 COMN FROM: RFG	Ply: 2 Qty: 7	**Job Number:** 23-9006 Nickelson Shed **Truss Label:** FT3	Cust: R 215 JRef:1XNR2150007 T9 DrwNo: 065.23.1512.43193 KD / WHK 03/06/2023	

2 Complete Trusses Required



Loading Criteria (psf)
TCLL: 40.00
TCDL: 10.00
BCLL: 0.00
BCDL: 5.00
Des Ld: 55.00
NCBCLL: 0.00
Soffit: 2.00
Load Duration: 1.00
Spacing: 16.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 3.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.331	J	999	360
VERT(CL): 0.545	J	659	240
HORZ(LL): 0.095	E	-	-
HORZ(TL): 0.157	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.255
Max BC CSI: 0.470
Max Web CSI: 0.298

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AC	1889	/-	/-	/341	/208	/85
T	1889	/-	/-	/341	/208	/-

Wind reactions based on MWFRS
AC Brg Wid = 5.5 Min Req = 1.5 (Truss)
T Brg Wid = 5.5 Min Req = 1.5 (Truss)
Bearings AC & T are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	98	- 847	J - K	427	- 3579
C - D	286	- 2728	K - L	427	- 3579
D - F	283	- 2745	L - M	359	- 3195
F - G	283	- 2745	M - N	284	- 2745
G - H	359	- 3195	N - P	284	- 2745
H - I	427	- 3579	P - Q	286	- 2728
I - J	429	- 3586	Q - R	98	- 847

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
AB-AA	904	- 91	X - W	3447	- 384
AA- Z	3160	- 330	W - V	3160	- 330
Z - Y	3447	- 384	V - U	904	- 86
Y - X	3586	- 404			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B -AC	105	- 950	M - V	88	- 540
B -AB	1056	- 106	N - V	34	- 525
AB- C	75	- 647	V - Q	2016	- 189
C -AA	2016	- 189	Q - U	75	- 647
F -AA	34	- 524	U - R	1056	- 106
AA- G	87	- 540	R - T	105	- 950

Lumber

Top chord: 2x4 SP #2; T2,T3 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3; W2,W21 2x4 SP #2; W4,W5,W18, W19 2x4 SP M-31;

Nailnote

Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

------(Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)

TC: From	67 plf at	-1.38 to	67 plf at	31.29
BC: From	3 plf at	-1.38 to	3 plf at	0.00
BC: From	7 plf at	0.00 to	7 plf at	29.92
BC: From	3 plf at	29.92 to	3 plf at	31.29

TC: 690 lb Conc. Load at 6.10,23.81

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

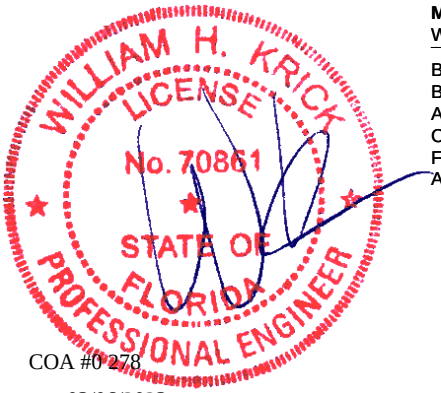
End verticals exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.

Note: Laterally brace top chord below filler at 2'0" O.C.Max. including a lateral brace at chord ends.

Additional Notes

The overall height of this truss excluding overhang is 3-1-14.



COA #0278
03/06/2023
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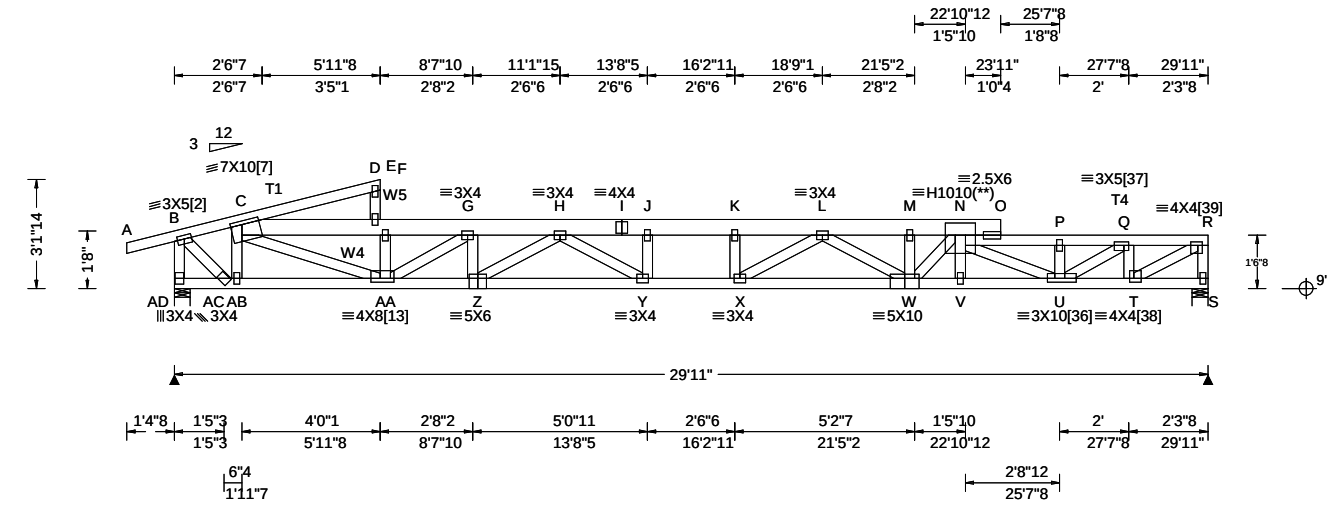
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543779 COMN FROM: RFG	Ply: 2 Qty: 10	**Job Number:** 23-9006 Nickelson Shed **Truss Label:** FT4	Cust: R 215 JRef:1XNR2150007 T21 DrwNo: 065.23.1512.58673 KD / WHK 03/06/2023	

2 Complete Trusses Required



Loading Criteria (psf)

TCLL:	40.00
TCDL:	10.00
BCLL:	0.00
BCDL:	5.00
Des Ld:	55.00
NCBCLL:	0.00
Soffit:	2.00
Load Duration:	1.00
Spacing:	16.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 3.0 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.324 K	999	360
VERT(CL):	0.535 K	670	240
HORZ(LL):	0.087 E	-	-
HORZ(TL):	0.145 E	-	-

Creep Factor: 2.0
Max TC CSI: 0.775
Max BC CSI: 0.490
Max Web CSI: 0.785

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AD	1890	/-	/-	/348	/204	/76
S	1790	/-	/-	/310	/192	/-

Wind reactions based on MWFRS
AD Brg Wid = 5.5 Min Req = 1.5 (Truss)
S Brg Wid = 5.5 Min Req = 1.5 (Truss)
Bearings AD & S are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	79	- 857	K - L	421	- 3732
C - D	262	- 2819	L - M	354	- 3350
D - F	266	- 2830	M - N	354	- 3350
F - G	266	- 2830	N - O	285	- 2879
G - H	346	- 3320	O - P	279	- 2829
H - I	421	- 3732	P - Q	279	- 2829
I - J	421	- 3732	Q - R	144	- 1430
J - K	423	- 3741			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
AC-AB	824	- 97	X - W	3565	- 408
AB-AA	863	- 101	W - V	3380	- 343
AA- Z	3288	- 349	V - U	3380	- 343
Z - Y	3566	- 404	U - T	1619	- 173
Y - X	3741	- 431			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B -AD	113	- 1071	N - U	72	- 633
B -AC	1137	- 100	P - U	53	- 536
AB- C	67	- 657	U - Q	1460	- 137
C -AA	2150	- 190	Q - T	92	- 828
AA- G	92	- 584	T - R	1648	- 171
F -AA	33	- 534	R - S	96	- 882

Lumber

Top chord: 2x6 SP 2400f-2.0E; T1 2x4 SP #2;
T4 2x4 SP M-31;
Bot chord: 2x4 SP M-31;
Webs: 2x4 SP #3; W4 2x4 SP #2; W5 2x4 SP M-31;

Nailnote

Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

------(Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)

TC: From	67 plf at	-1.38 to	67 plf at	29.92
BC: From	3 plf at	-1.38 to	3 plf at	0.00
BC: From	7 plf at	0.00 to	7 plf at	29.92

TC: 689 lb Conc. Load at 6.10,23.81

Plating Notes

All plates are 2X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Plate Shift Table

JT No	Plate Size	Lateral Shift		Chord Bite	JT No	Plate Size	Lateral Shift		Chord Bite
[2]	3X5	S		1.25	[7]	7X10	1.22	L	1.25
[13]	4X8	2.62	L	1.25	[36]	3X10	2.53	R	1.25
[37]	3X5	1.75	R	1.25	[38]	4X4	2.00	R	1.25
[39]	4X4	1.50	R	1.25					

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-1-14.

Note: Laterally brace top chord below filler at 2'0" O.C.Max. including a lateral brace at chord ends.

COA #0 278
03/06/2023
Florida Certificate of Product Approval #FL 1999

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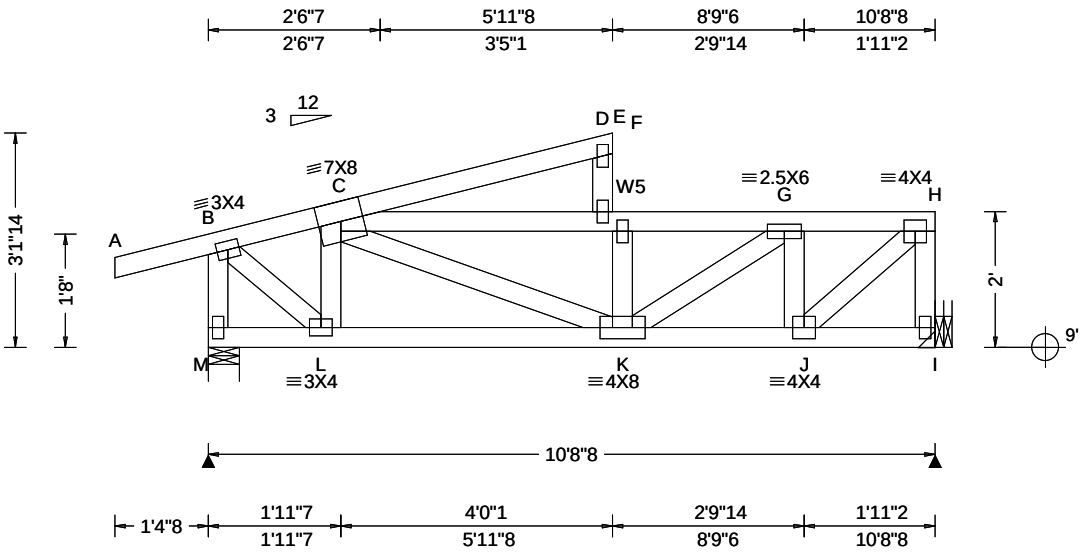
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543686 COMN	Ply: 1	**Job Number:** 23-9006	Cust: R 215 JRef:1XNR2150007 T4
FROM: RFG	Qty: 2	Nickelson Shed	DrwNo: 065.23.1513.14283
Page 1 of 2		**Truss Label:** FT5	KD / WHK 03/06/2023



Loading Criteria (psf)

TCLL:	40.00
TCDL:	10.00
BCLL:	0.00
BCDL:	5.00
Des Ld:	55.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.00
Spacing: 16.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 3.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.039	E	999	360
VERT(CL): 0.068	E	999	240
HORZ(LL): 0.014	E	-	-
HORZ(TL): 0.024	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.285
Max BC CSI: 0.346
Max Web CSI: 0.510

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
M	793	/-	/-	/149	/84	/69
I	781	/-	/-	/112	/77	/-

Wind reactions based on MWFRS
M Brg Wid = 5.5 Min Req = 1.5 (Truss)
I Brg Wid = - Min Req = -
Bearing M is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	68	- 635	F - G	127	- 1629
C - D	127	- 1620	G - H	73	- 806
D - F	127	- 1629			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
L - K	668	- 102	K - J	923	- 85

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - M	84	- 797	K - G	868	- 51
B - L	785	- 52	G - J	74	- 699
L - C	49	- 464	J - H	1072	- 97
C - K	1051	- 36	H - I	80	- 772
F - K	58	- 926			

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W5 2x4 SP #2;

Special Loads
------(Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)

TC: From	67 plf at	-1.38 to	67 plf at	10.71
BC: From	3 plf at	-1.38 to	3 plf at	0.00
BC: From	7 plf at	0.00 to	7 plf at	10.71

TC: 689 lb Conc. Load at 6.10

Plating Notes
All plates are 2X4 except as noted.

Wind
Wind loads based on MWFRS with additional C&C member design.
Left end vertical exposed to wind pressure. Deflection meets L/360.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 3-1-14.
Note: Laterally brace top chord below filler at 2'0" O.C.Max. including a lateral brace at chord ends.

COA #0 278
03/06/2023
Florida Certificate of Product Approval #FL 1999

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Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=10'5"8 uses the following support conditions: 10'5"8
Bearing I (10'5"8, 9') HUS26
Supporting Member: (2)2x6 SP 2400f-2.0E
(14) 0.148"x3" nails into supporting member,
(4) 0.148"x3" nails into supported member.

COA #0 278
03/06/2023
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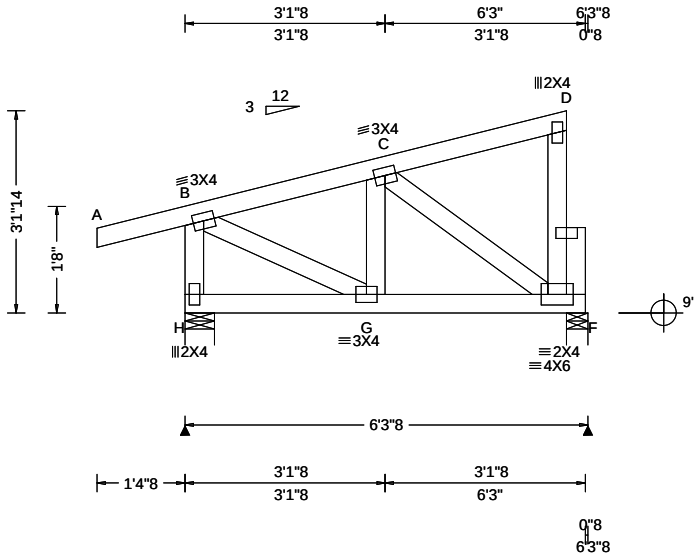
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543081 / MONO FROM: RFG	Ply: 1 Qty: 1	**Job Number:** 23-9006 Nickelson Shed **Truss Label:** FT6	Cust: R 215 JRef:1XNR2150007 T11 DrwNo: 065.23.1458.20480 SSB / YK 03/06/2023



Loading Criteria (psf)

TCLL:	40.00
TCDL:	10.00
BCLL:	0.00
BCDL:	5.00
Des Ld:	55.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.00
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 3.0 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.005	C	999	360
VERT(CL): 0.007	C	999	240
HORZ(LL): 0.001	F	-	-
HORZ(TL): 0.001	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.248
Max BC CSI: 0.098
Max Web CSI: 0.333

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
H	505	/-	/-	/122	/104	/103
F	301	/-	/-	/62	/93	/-

Wind reactions based on MWFRS
H Brg Wid = 5.5 Min Req = 1.5 (Truss)
F Brg Wid = 4.0 Min Req = 1.5 (Truss)
Bearings H & F are a rigid surface.
Members not listed have forces less than 375#

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
B - H	104	-491

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical exposed to wind pressure. Deflection meets L/360.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-1-14.

COA #0278
03/06/2023
Florida Certificate of Product Approval #FL 1999

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ALPINE
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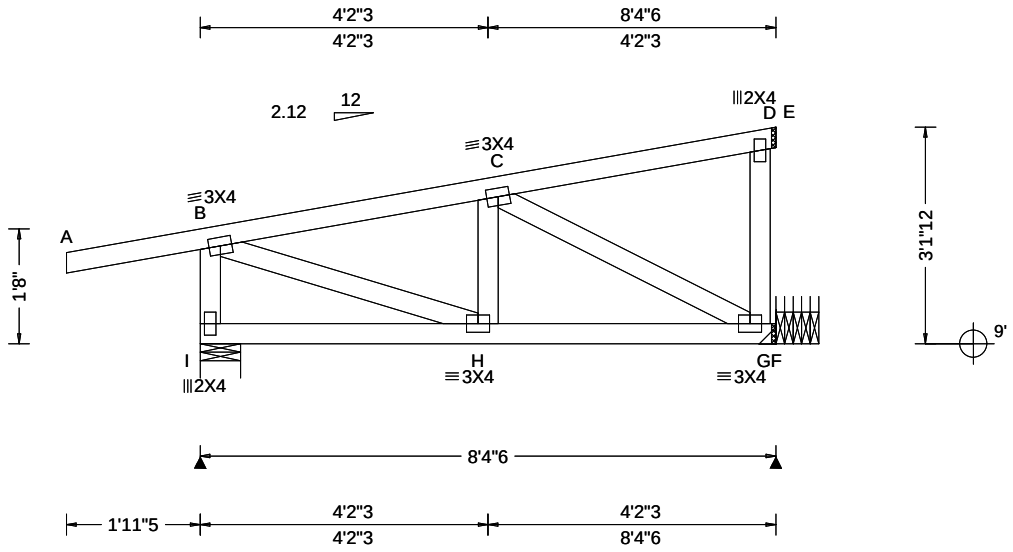
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543776 HIP_ FROM: RFG	Ply: 1 Qty: 4	**Job Number:** 23-9006 Nickelson Shed **Truss Label:** HGJ1	Cust: R 215 JRef:1XNR2150007 T18 DrwNo: 065.23.1513.22800 KD / WHK 03/06/2023



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.007	H	999	360
VERT(CL): 0.014	H	999	240
HORZ(LL): -0.002	E	-	-
HORZ(TL): 0.003	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.556
Max BC CSI: 0.510
Max Web CSI: 0.190

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
I	303	/-	/-	/-	/86	/41
F	537	/-	/-	/-	/43	/-

Wind reactions based on MWFRS
I Brg Wid = 7.0 Min Req = 1.5 (Truss)
F Brg Wid = - Min Req = -
Bearing I is a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - C	53	-450

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
H - G	437	-52

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
B - H	477	-21	C - G	58	-480

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Special Loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From	0 plf at	-1.94 to	60 plf at	0.00
TC: From	2 plf at	0.00 to	2 plf at	8.36
BC: From	0 plf at	-1.94 to	4 plf at	0.00
BC: From	2 plf at	0.00 to	2 plf at	8.36

TC: -31 lb Conc. Load at 1.32
TC: 136 lb Conc. Load at 4.15
TC: 274 lb Conc. Load at 6.98
BC: 37 lb Conc. Load at 1.32
BC: 117 lb Conc. Load at 4.15
BC: 197 lb Conc. Load at 6.98

Hangers / Ties
(J) Hanger Support Required, by others

Wind
Wind loads and reactions based on MWFRS.
Left end vertical exposed to wind pressure. Deflection meets L/360.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 3-1-12.

COA #0278
03/06/2023
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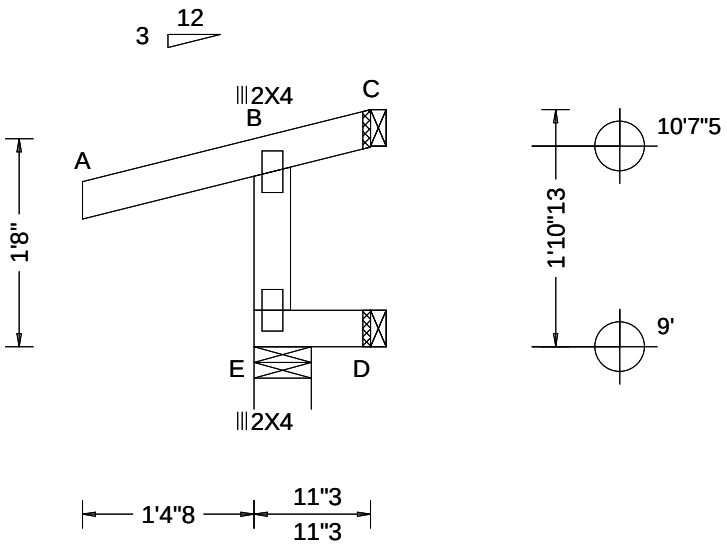
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543764 JACK FROM: RFG	Ply: 1 Qty: 8	**Job Number:** 23-9006 Nickelson Shed **Truss Label:** J1	Cust: R 215 JRef:1XNR2150007 T16 DrwNo: 065.23.1513.24670 KD / WHK 03/06/2023



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.000	B	999	360
VERT(CL): 0.000	B	999	240
HORZ(LL): -0.000	B	-	-
HORZ(TL): 0.000	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.123
Max BC CSI: 0.008
Max Web CSI: 0.052

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E	192	/-	/-	/125	/97	/-
D	19	/-	/-	/9	/-	/22
C	-	/-37	/-	/34	/40	/39

Wind reactions based on MWFRS
E Brg Wid = 5.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing E is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

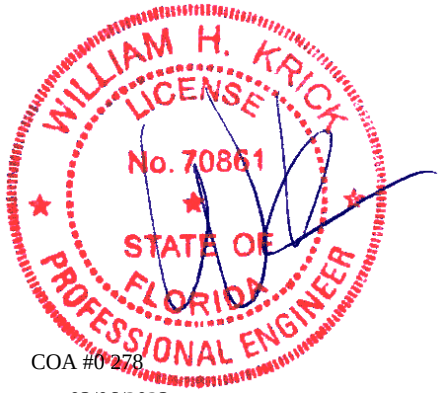
Wind loads based on MWFRS with additional C&C member design.

Left end vertical exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 1-10-13.



COA #0 278

03/06/2023

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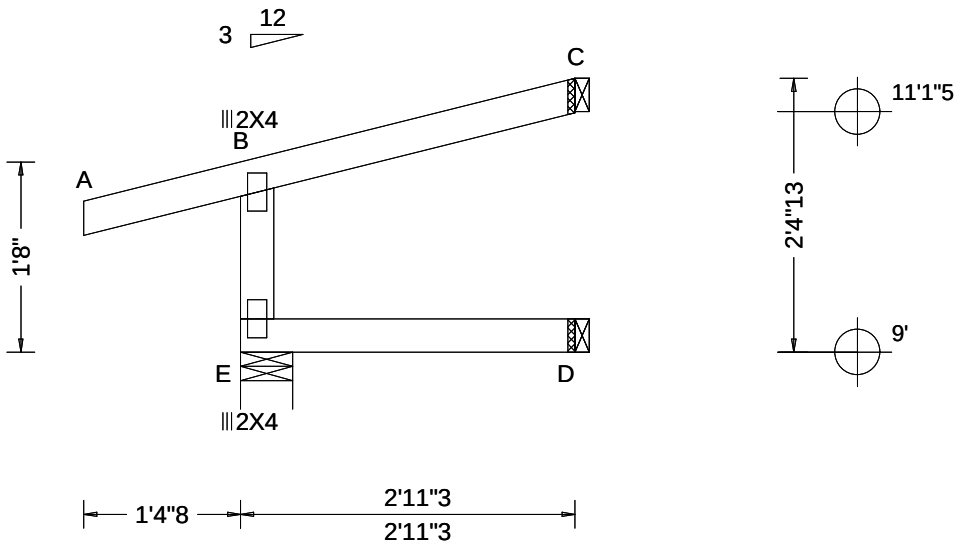
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543767 JACK FROM: RFG	Ply: 1 Qty: 8	**Job Number:** 23-9006 Nickelson Shed **Truss Label:** J3	Cust: R 215 JRef:1XNR2150007 T14 DrwNo: 065.23.1513.33087 KD / WHK 03/06/2023



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.000	B	999	360
VERT(CL): 0.000	B	999	240
HORZ(LL): -0.000	B	-	-
HORZ(TL): 0.000	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.134
Max BC CSI: 0.094
Max Web CSI: 0.053

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E	228	/-	/-	/138	/82	/-
D	59	/-	/-	/29	/-	/22
C	68	/-	/-	/30	/19	/57

Wind reactions based on MWFRS
E Brg Wid = 5.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing E is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

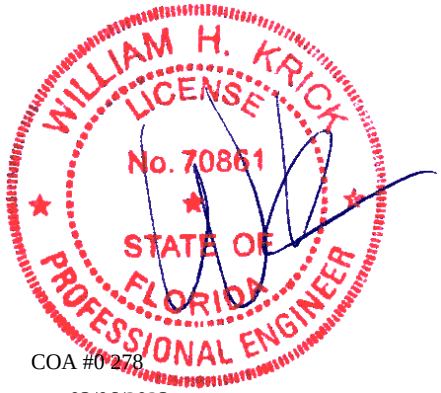
Wind loads based on MWFRS with additional C&C member design.

Left end vertical exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 2-4-13.



COA #0 278
03/06/2023
Florida Certificate of Product Approval #FL 1999

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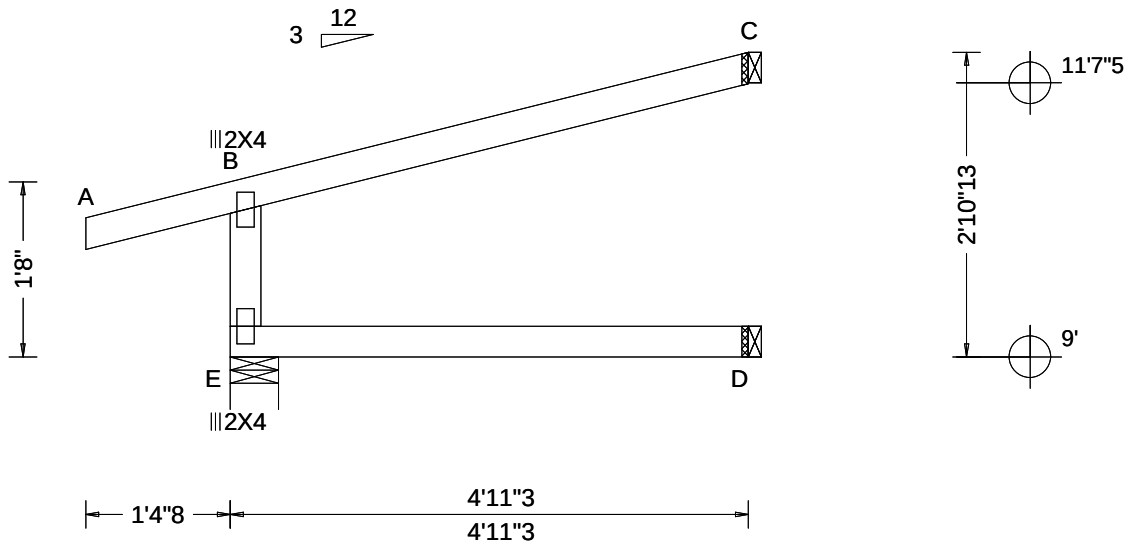
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543770 JACK FROM: RFG	Ply: 1 Qty: 8	**Job Number:** 23-9006 Nickelson Shed **Truss Label:** J5	Cust: R 215 JRef:1XNR2150007 T13 DrwNo: 065.23.1513.34887 KD / WHK 03/06/2023



Loading Criteria (psf)

TCLL:	20.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	40.00
NCBCLL:	10.00
Soffit:	2.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-16
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 5.0 psf
BCDL: 5.0 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 7th Ed. 2020 Res.
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.000	B	999	360
VERT(CL): 0.001	B	999	240
HORZ(LL): -0.000	B	-	-
HORZ(TL): 0.000	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.370
Max BC CSI: 0.289
Max Web CSI: 0.050

VIEW Ver: 22.02.00.0914.12

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E	300	/-	/-	/173	/87	/-
D	99	/-	/-	/49	/-	/22
C	137	/-	/-	/54	/45	/74

Wind reactions based on MWFRS
E Brg Wid = 5.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
C Brg Wid = 1.5 Min Req = -
Bearing E is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical exposed to wind pressure. Deflection meets L/360.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 2-10-13.

COA #0278
03/06/2023
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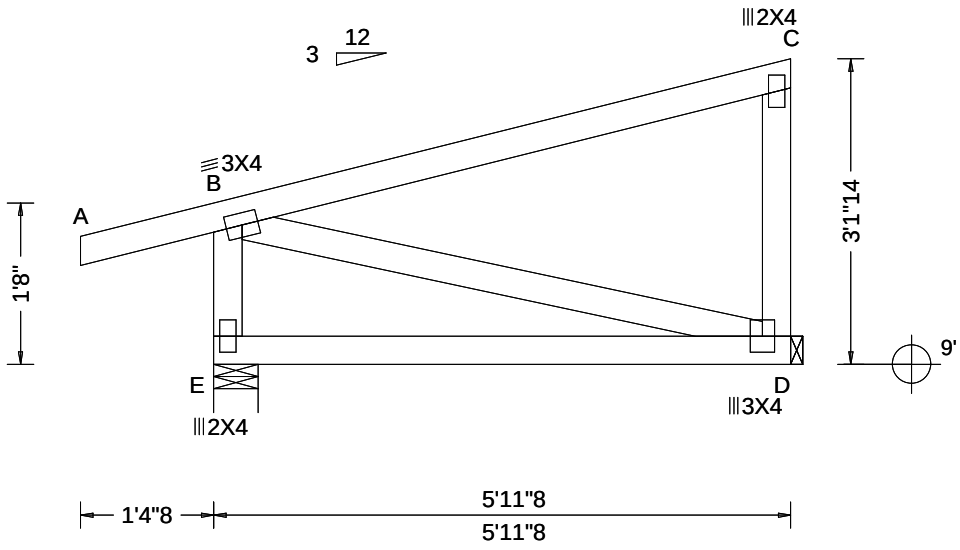
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 543773 EJAC FROM: RFG	Ply: 1 Qty: 20	**Job Number:** 23-9006 Nickelson Shed **Truss Label:** J6	Cust: R 215 JRef:1XNR2150007 T17 DrwNo: 065.23.1513.38710 KD / WHK 03/06/2023



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.000 C 999 360
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.001 C 999 240
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.001 C - -
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.002 C - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 2.00	TCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.503
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.374
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: No	Max Web CSI: 0.161
	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	
	Loc. from endwall: not in 4.50 ft	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 22.02.00.0914.12
	Wind Duration: 1.60		

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E	339	/-	/-	/178	/70	/103
D	230	/-	/-	/125	/68	/-

Wind reactions based on MWFRS
E Brg Wid = 5.5 Min Req = 1.5 (Truss)
D Brg Wid = 1.5 Min Req = -
Bearing E is a rigid surface.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical exposed to wind pressure. Deflection meets L/360.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 3-1-14.

COA #0278

03/06/2023

Florida Certificate of Product Approval #FL 1999

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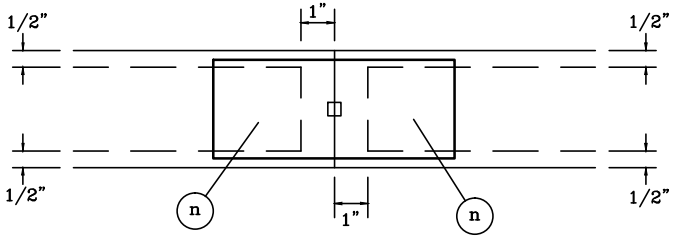
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

TRULOX INFORMATION DETAIL

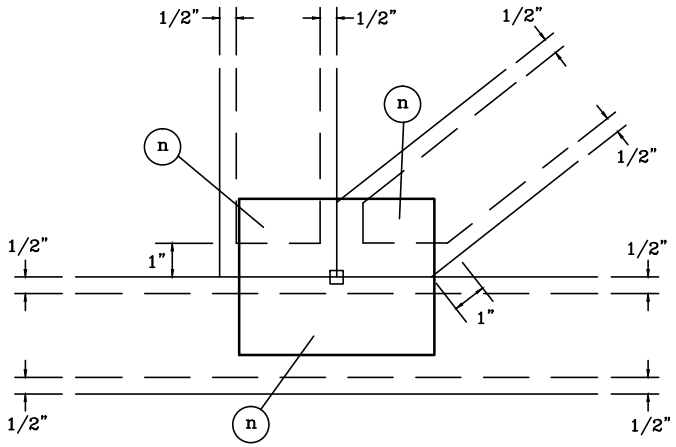
TYPICAL OFF PANEL SPLICE



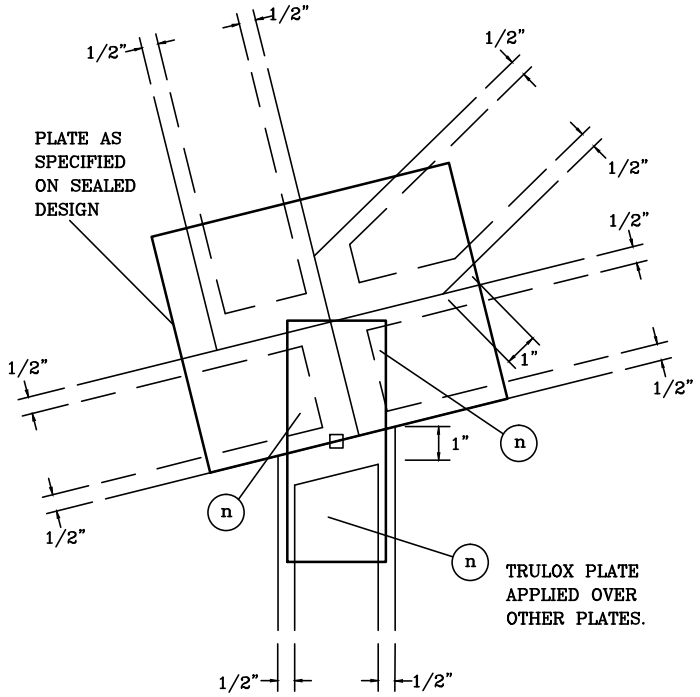
DO NOT APPLY NAILS WITHIN 1/2" OF LUMBER EDGES OR 1" OF LUMBER ENDS ON EACH FACE, AS SHOWN BY DASHED LINES.

NAILS MUST NOT SPLIT LUMBER.

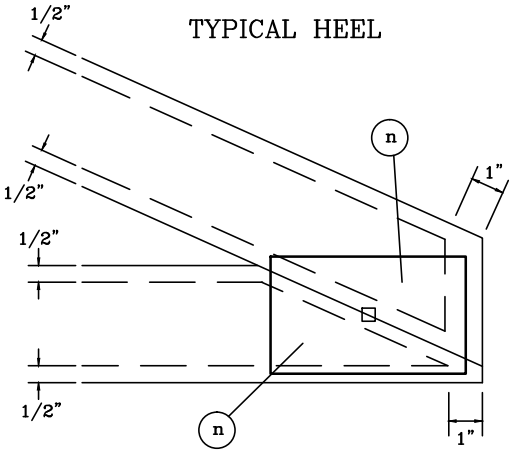
TYPICAL PANEL POINT WITHOUT SPLICE



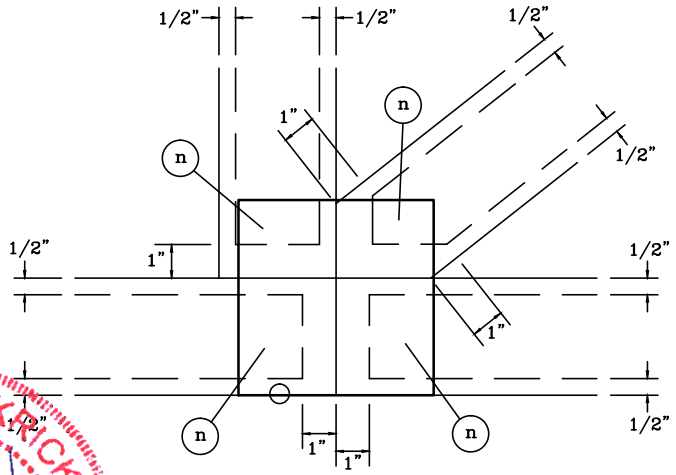
TYPICAL FILLER



TYPICAL HEEL



TYPICAL PANEL POINT SPLICE



NOTES:
(n) IS THE REQUIRED NUMBER OF 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY AS SPECIFIED ON THE SEALED DESIGN REFERENCING THIS DETAIL.
○ LOCATES PLATE CORNER OR FLUSH EDGE.
□ LOCATES PLATE CENTER.

WILLIAM H. KNICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

COA #0278 03/06/2023
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ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

TRULOX PLATING

160 TL

PAGE 1 OF 1

DATE 10/01/14

Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00
Or: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 "L" Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
		#3	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
		Stud	3' 10"	6' 2"	6' 6"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
		Standard	3' 10"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 0"	11' 0"	11' 10"	14' 0"	14' 0"
	SP DFL	#1	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"
		#2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
		#3	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
		Stud	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
		Standard	3' 9"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"
16" o.c.	SPF HF	#1 / #2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 5"	7' 6"	8' 3"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 5"	7' 6"	8' 0"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 5"	6' 5"	6' 10"	8' 7"	9' 2"	11' 0"	11' 6"	13' 6"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 10"	7' 5"	7' 11"	9' 11"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

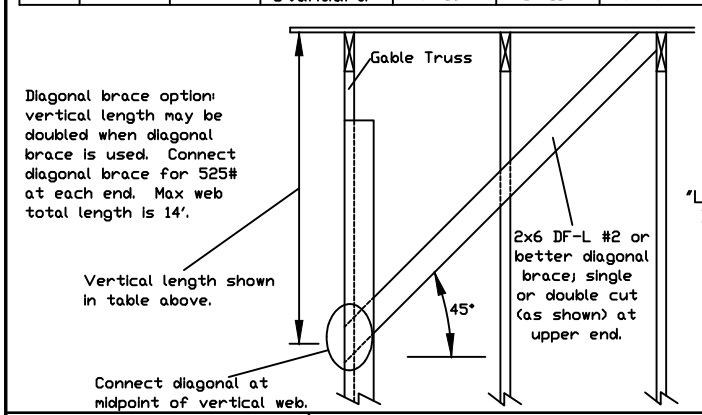
"L" bracing must be a minimum of 80% of web member length.

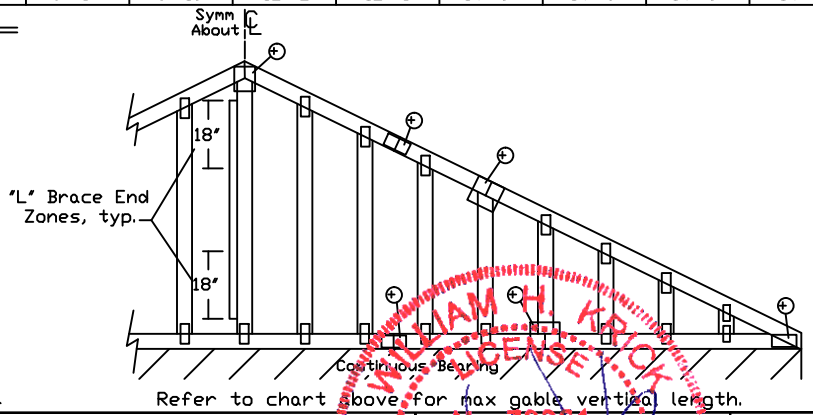
Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.





Refer to chart above for max gable vertical length.

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For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

COA #0 278 03/06/2023
Florida Certificate of Product Approval #FL 1999

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-16-GAB14030
DATE	01/26/2018
DRWG	A14030ENC160118

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

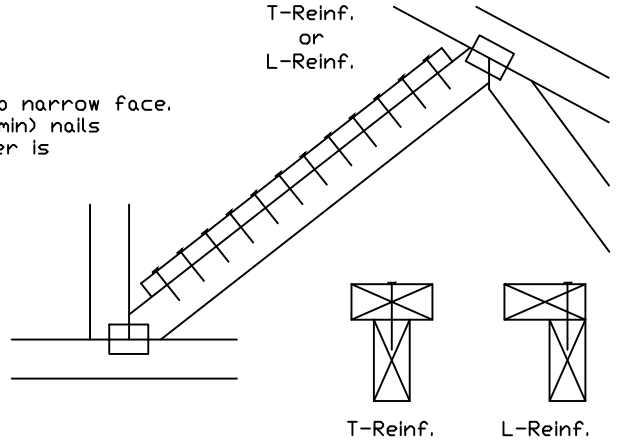
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf.	Alternative Reinforecement Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

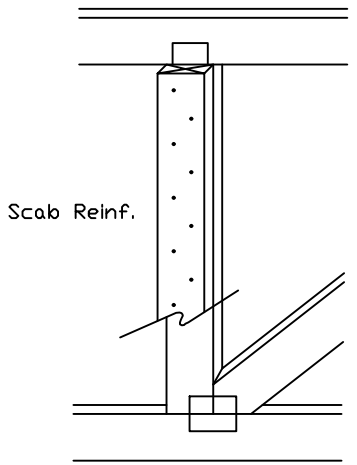
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

03/06/2023

COA #0278

Florida Certificate of Product Approval #FL 1999

ALPINE AN ITW COMPANY

155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

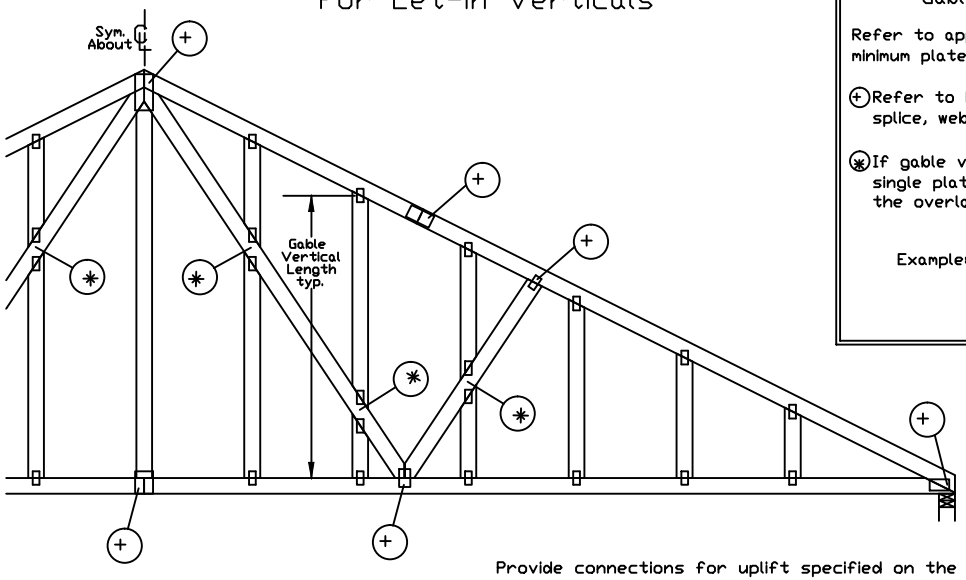
A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

TC LL	PSF	
TC DL	PSF	
BC DL	PSF	
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

REF	CLR Subst.
DATE	01/02/19
DRWG	BRCLBSUB0119

Gable Detail
For Let-in Verticals



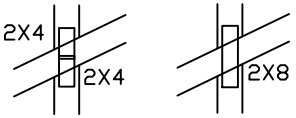
Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

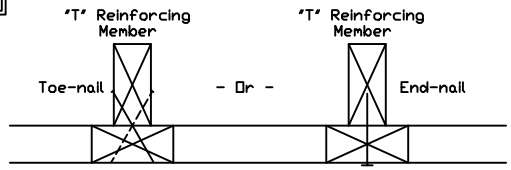
⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊛ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



"T" Reinforcement Attachment Detail



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:
10d Common (0.148"x 3.",min) Nails at 4" o.c. plus
(4) nails in the top and bottom chords.

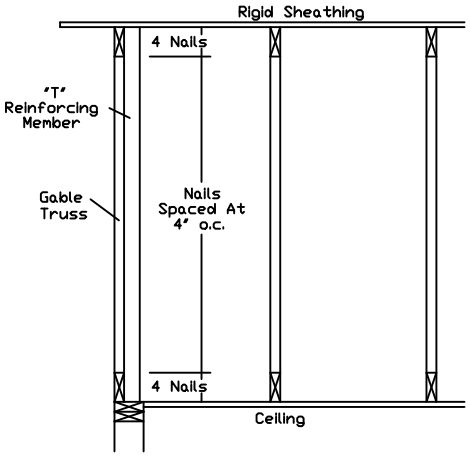
Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4" o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118, A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118, A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118, A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118, S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118, S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118, S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118, S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24"o.c. SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

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MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"

REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118