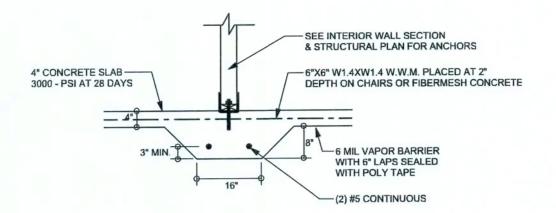
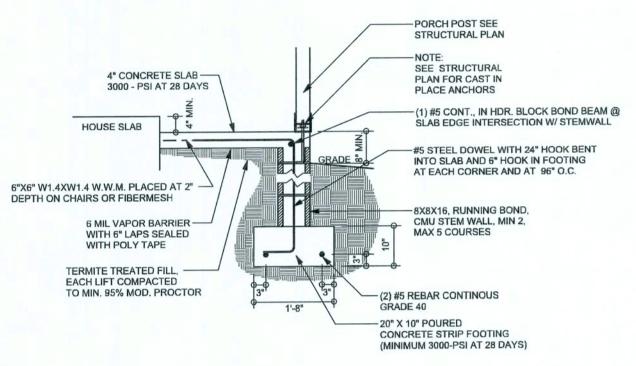


SCALE: 1/2" = 1'-0"

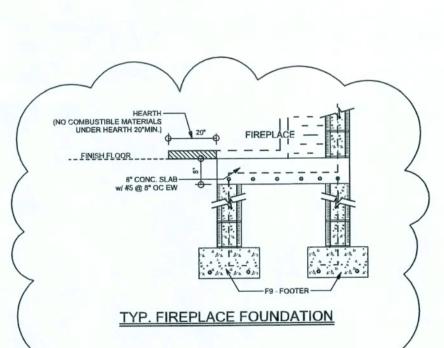






ALT. STEM WALL PORCH FOOTING



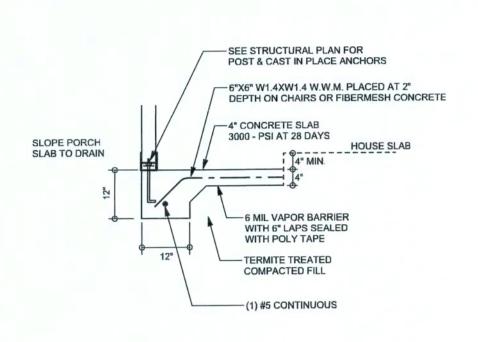


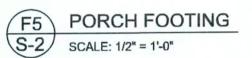
## TALL STEM WALL TABLE

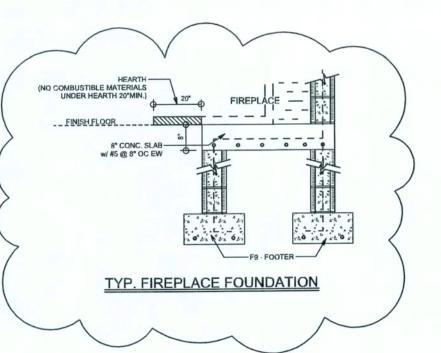
S-2 SCALE: 1/2" = 1'-0"

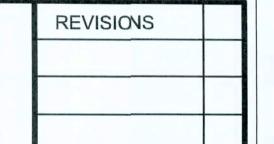
The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL I HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

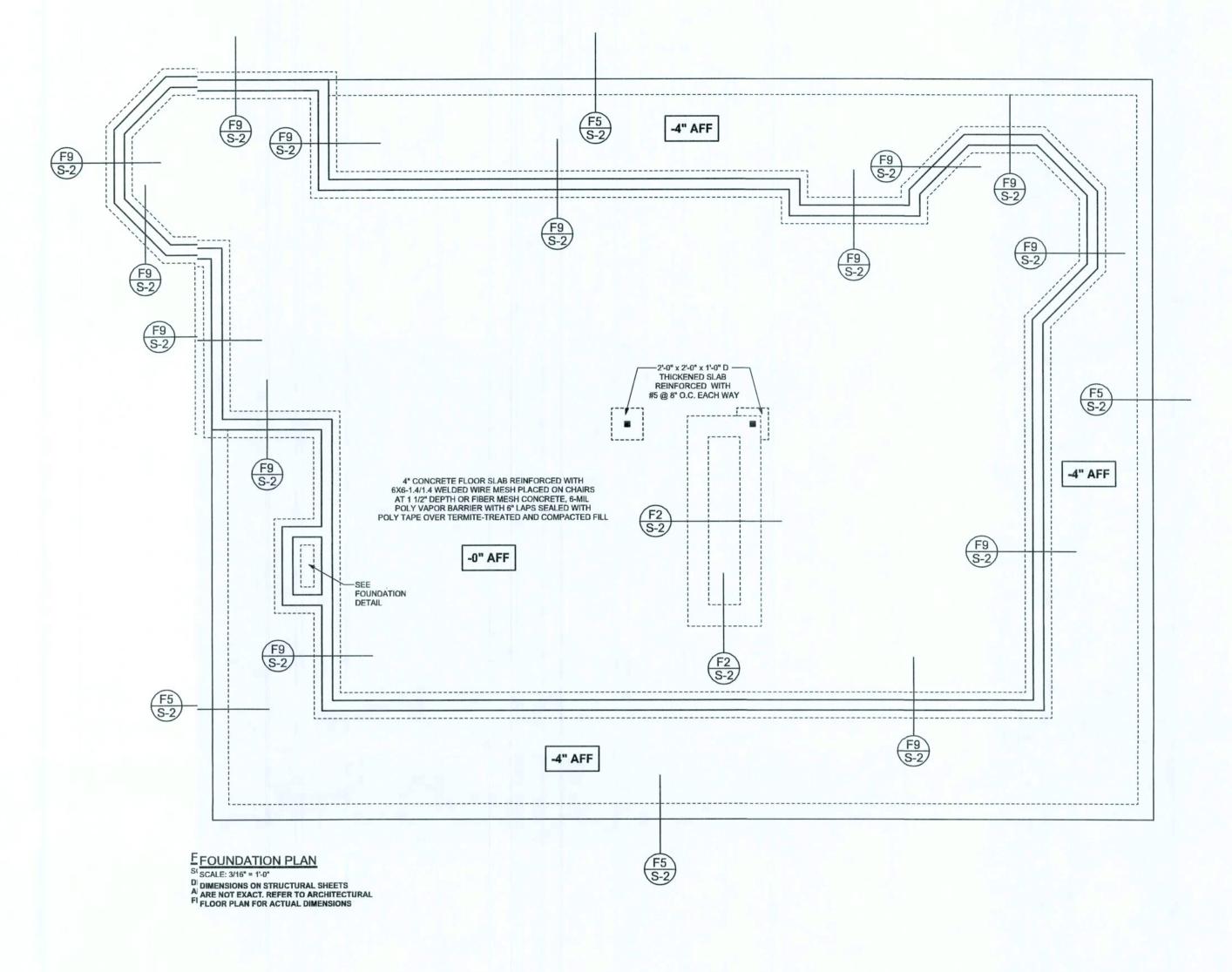








SOFTPION



WINDLOAD ENGINER: Mark Disosway, PE No.53915, POB 88, Lake City, FL 32056, 386-754-5419 DIMENSIONS: dimensions. Refer allquestions to Mark Disosway, P.E. or resolution. Do not proceed without clarification. COPYRIGHTS AND 'ROPERTY RIGHTS: Mark Disosway, P.E. rereby expressly reserve its common law copyights and property right in these instruments of ervice. This document is not to be reproduced altered or copied in any form or manner without first the express written permission and consnt of Mark Disosway. CERTIFICATION: I breby certify that I have examined this plan, and that the applicable portions of the plan, elating to wind engineering comply with section (301.2.1, florida building code residential 2004 to the best of my LIMITATION: This deign is valid for one building, at specified acation. MARKDISOSWAY P.I. 53915

> TIM & SARA PETERSEN

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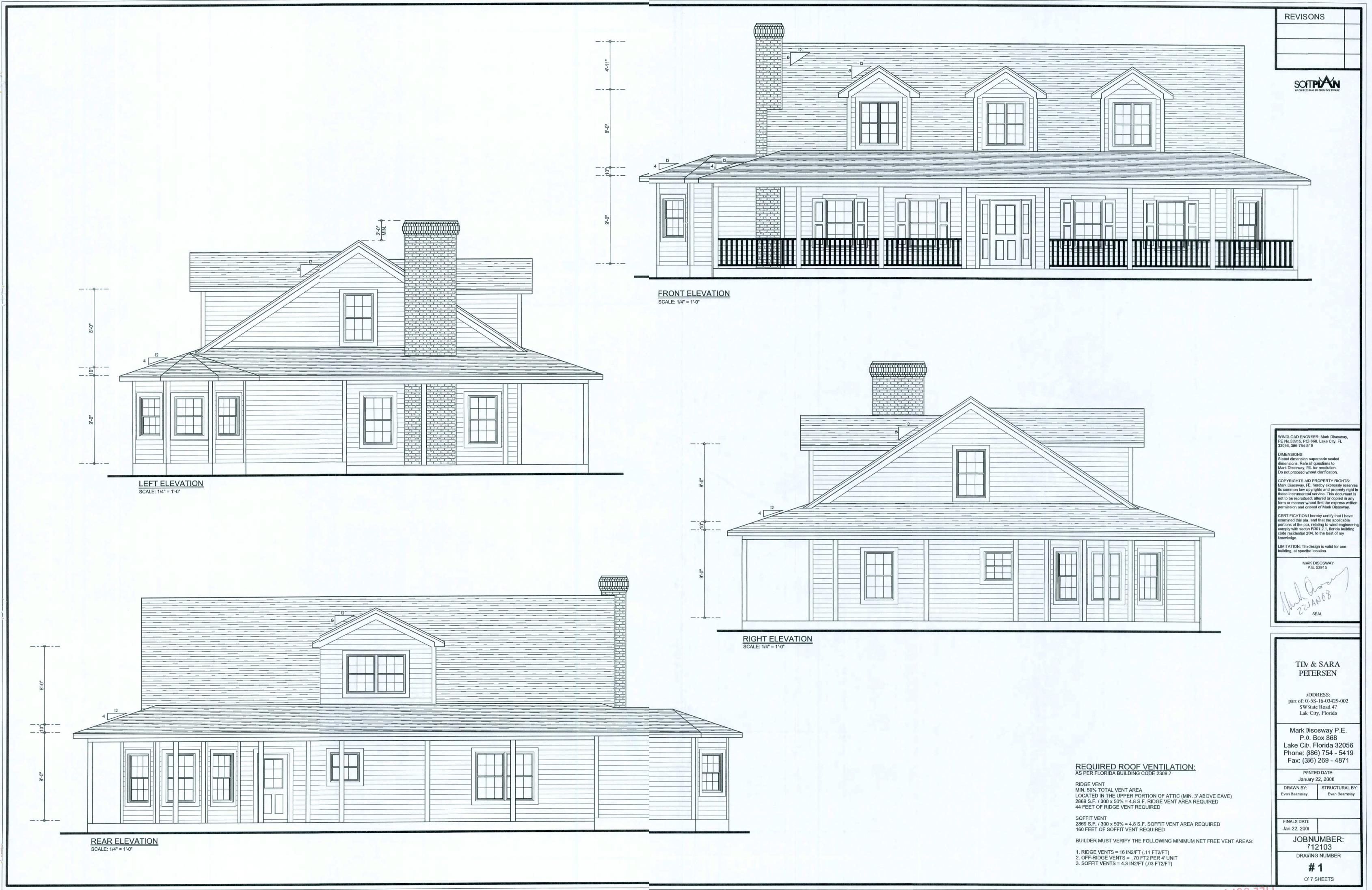
Mark Disosway P.E. P.O.Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (38f) 269 - 4871

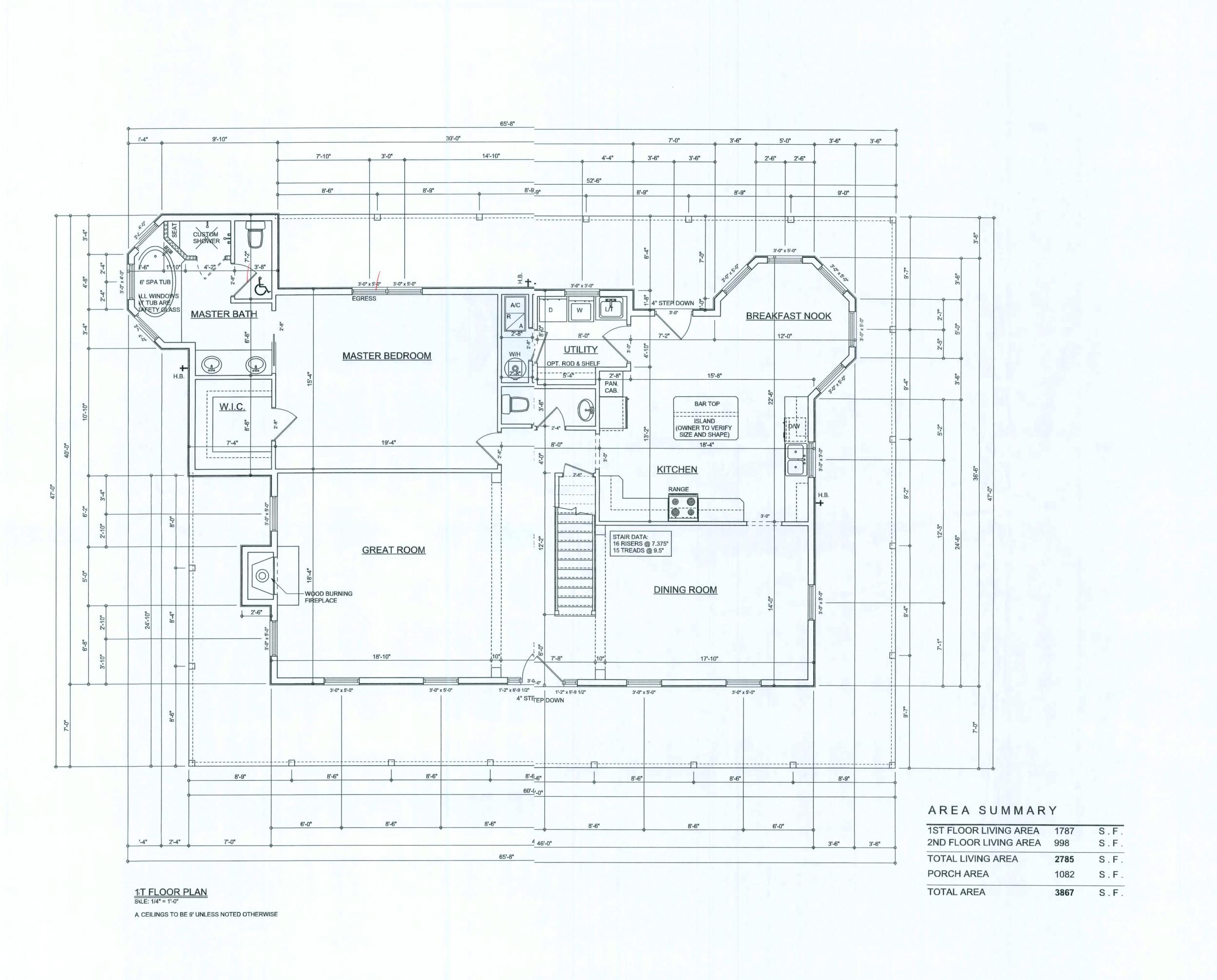
PRIN'ED DATE: Februay 11, 2008 DRAWN BY: STRUCTURAL BY **Evan Beamsley** Evan Beamsley

FINALS DATE: Jan 22, 2008 JOB NUMBER:

7.2103 DRAWING NUMBER **S-2** 

OF 7SHEETS





REVISIONS

SOTTPAN ARCHITETURAL DESIGN SOFTWARD

WINDLOAD ENGNEER: Mark Disosway, PE No.53915, PØ 868, Lake City, FL 32056, 386-754-419

DIMENSIONS: Stated dimension supercede scaled dimensions. Refe all questions to Mark Disosway, I.E. for resolution. Do not proceed vithout clarification.

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PHNTED DATE: January 22, 2008 DRAWN BY: STRUCTURAL BY: Evan Beamsley Evan Beamsley

FINALS DATE:

Jan 22, 2003 JOBNUMBER: 712103

DRAVING NUMBER

OF 7 SHEETS

HANDRAIL PROFILE:

- TYPE I: HANDRAILS WITH A CIRCULAR CROSS SECTION SHALL HAVE AN OUTSIDE DIA. OF 1 1/4" - 2". IF HANAIL IS NOT CIRCULAR IT SHALL HAVE A PERIMETER DIMENSION OF 4" - 6 1/4" AND MAX. CROSS SECTION OF 2 1/4".

- TYPE II: HANDRAILS WITH A PERIMETER GREATER THAN 6 1/4" SHALL PROVIDE A GRASPABLE FINGER RECS AREA ON BOTH SIDES OF THE PROFILE. THE FINGER RECESS SHALL BEGIN WITHIN A DISTANCE OF 3/4" VERTICALLY FROM THE TALLEST PTION OF THE PROFILE AND A DEPTH OF AT LEAST 5/8" WITHIN 7/8" BELOW THE WIDEST PORTION OF THE PROFILE. THIS REQUIRED DEPTHALL CONTINUE FOR AT LEAST 3/8" TO A LEVEL THAT IS NOT LESS THAN 1 3/4" BELOW THE TALLEST PORTION OF THE PROFILE. THE WIDTH THE HANDRAIL ABOVE THE RECESS SHALL BE 1 3/4" - 2 3/4". EDGES SHALL HAVE A MIN. RADIUS OF 0.01". ------A 4 3/8" SPHERE CANNOT PASS THROUGH OPENING IN GUARDS ON SIDES OF STAIR 36" MIN. CLEAR WIDTH ABOVE HANDRAIL SPINDLES / RAILS SPACED SO THAT A 4" SPHERE

TYPICAL STAIR AND GUARDRAIL REQUIRMENTS SCALE: 3/4" = 1'-0"

NOTE: THE SUM OF TWO RISERS AND

A TREAD, EXCLUDING NOSING,

SHALL BE BETWEEN 24" AND 25"

(2 RISERS + 1 TREAD = 24" - 25")

THE GREATEST DEPTH IN ANY FLIGHT OF STAIRS SHALL NOT EXCEED THE SMALLEST

9" MIN. TREAD DEPTH -

BY MORE THAN 3/8" MAX. TREAD SLOPE OF 2 PERCENT (1" VERT UNIT IN 48" HORZ. UNITS)

DPENING IN GUARD U.N.O.

A 6" SPHERE CANNOT PASS -

THROUGH THE TRIANGULAR AREA FORMED BY RISER,

LANDINGS:
A LANDING SHALL BE PROVIDED AT
THE TOP AND BOTTOM. LANDINGS
SHALL HAVE A MIN WIDTH OF NOT
LESS THAT STAIR WIDTH AND HAVE
UNDER ALL

LESS THAT STAIR WIDTH AND HAVE
A MIN 36" MEASURED IN DIRECTION
OF TRAVEL. A DOOR AT THE TOP OF
STAIR IS PERMITTED, PROVIDED

IT DOES NOT SWING OVER STAIR.

WINDER TREADS SHALL HAVE A MIN.

TREADDEPTH OF 10" MEASURED AT A POINT 12" FROM NARROWEST END.

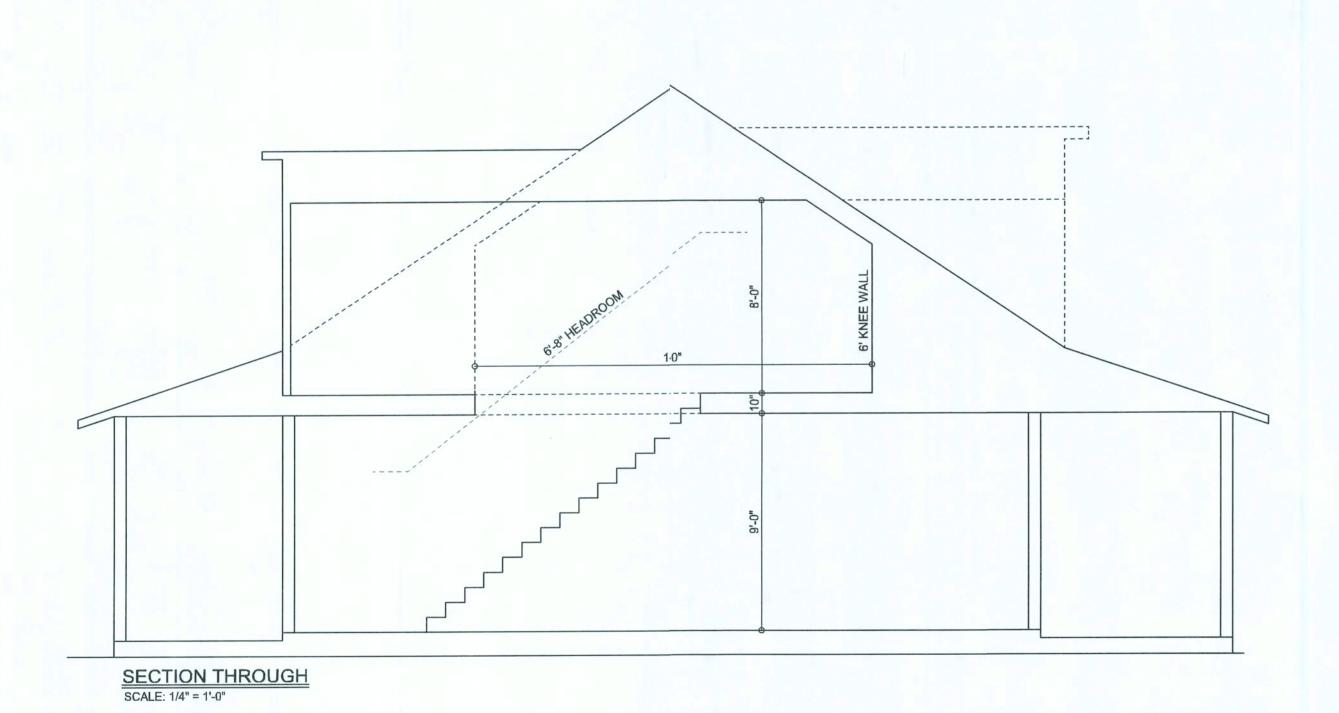
**GREATEST DEPTH AT 12" SHALL NOT** 

WINDER TREADS SHALL HAVE A MIN. DEPTH OF 6" AT ANY POINT. WITHIN A FLIGHT OF STAIRS THE

**EXCEED THE SMALLEST BY MORE THAN 3/8"** 

WINDER TREADS:

TREAD AND GUARD RAIL



NOTINGS: EVERY TREAD LESS THAT 10" SILL HAVE A NOSING OR EFFECTIVE PROJECTION OF APOX. 1" OVER THE LEVEL BELOW. A NOSING IS NOT REQUED WHERE THE TREAD

LEADING EDGE OF TREAD SHAIBE NO GRATER THAT 9/16"

-RISERS SHALL BE VERTIL OR SLOPED AT AN

ANGLE NOT MORE THAN DEGREES FROM VERTICAL.
OPEN RISERS ARE PERNTED, PROVIDED THAT THE
OPENING BETWEEN TRES DOES NOT PERMIT THE PASSAGE OF A 4" DIA. SERE. OPENING BETWEEN

TREADS IS NOT LIMITED TOTAL RISE IS 30" OR LESS

-74" MAX. RISER HEIGHT T GREATEST RISER IN ANY Firt of Stairs shall not

EEED THE SMALLEST

IS A MIN. 11". THE RADIUS OF CIVATURE AT THE

BEVELING OF NOSING SHALL N EXCEED 1/2"

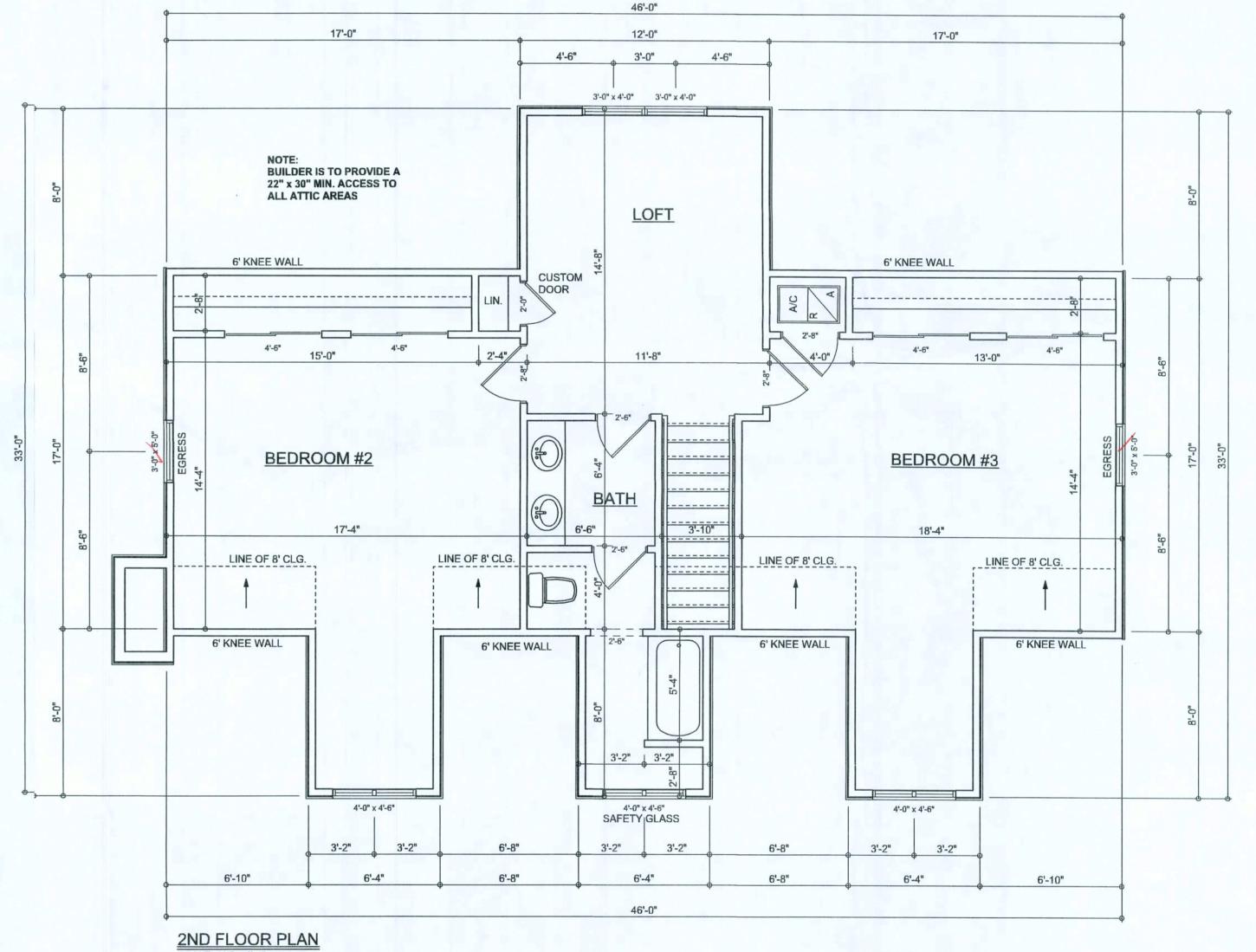
4 1/2" MAX.
HANDRAIL
PROJECTION

HANDRAIL

AND WALL

HANDRAILS SHALL BE CONT. ON AT LEAST ONE SIDE OF ALL STAIRS WITH 4 OR MORE RISERS

SECTION VIEW



2ND FLOOR PLAN SCALE: 1/4" = 1'-0"

ALL CEILINGS TO BE 8' UNLESS NOTED OTHERWISE

WINDLOAD ENGNEER: Mark Disosway, PE No.53915, PC3 868, Lake City, FL 32056, 386-754-519 DIMENSIONS:

REVISONS

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LIMITATION: Thisdesign is valid for one building, at specified location.

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TIM & SARA PEFERSEN

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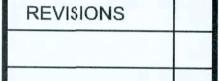
Mark Disosway P.E. P.0. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (336) 269 - 4871

PRNTED DATE: January 22, 2008 DRAWN BY: STRUCTURAL BY: Evan Beamsley

Jan 22, 200 JOBNUMBER: 12103

DRAVING NUMBER

OF 7 SHEETS



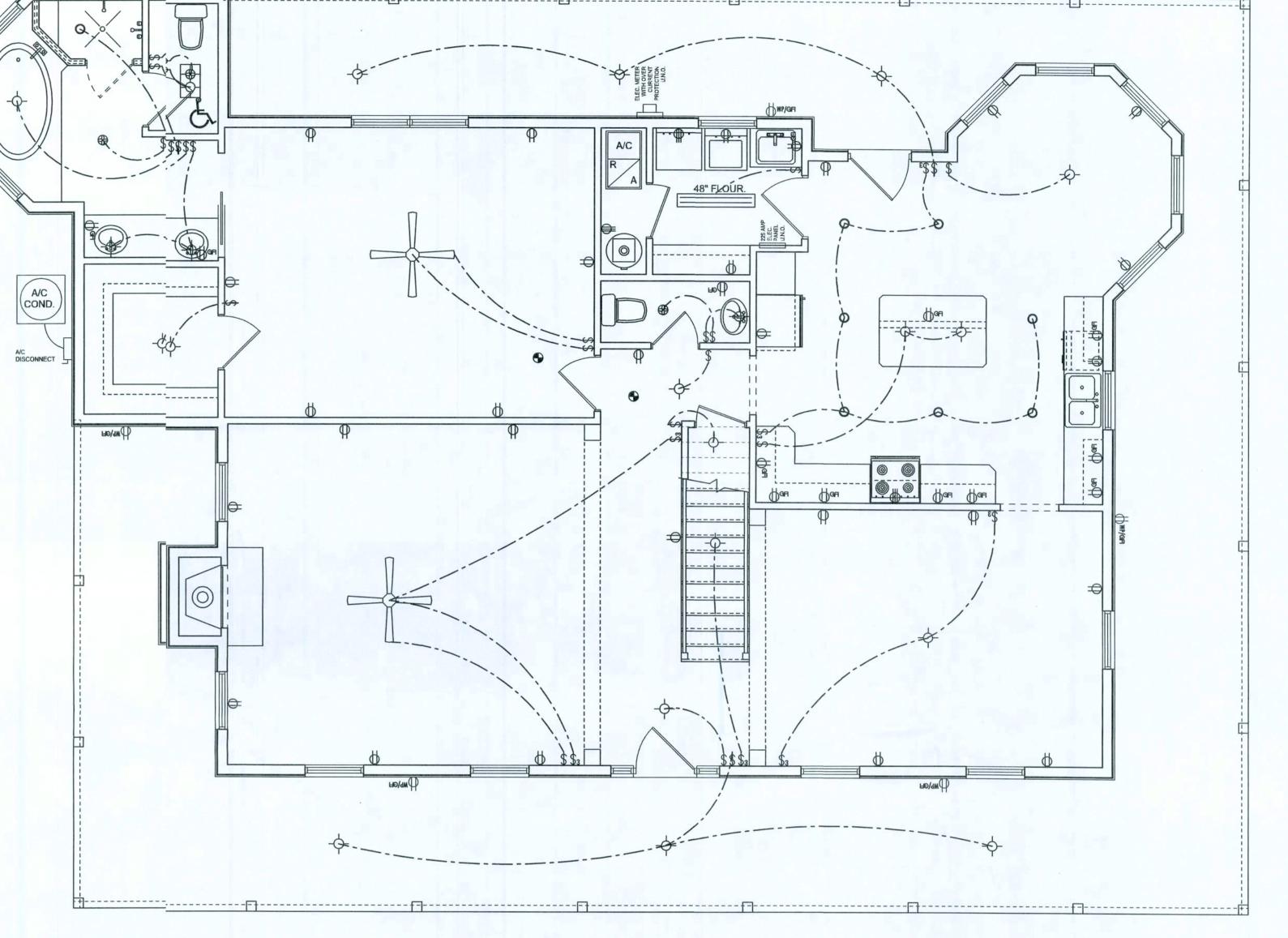
SCFTPIAN ARCHITETURAL DESIGN SOFTWARE

## **ELECTRICAL PLAN NOTES**

- WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY E -4 BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E -5
  TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE
  DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S
  DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE ( UNDERGROUND OR OVERHEAD )
  TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
- A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE
- LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC E -10 CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL

	ELECTRICAL LEGEND
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
90	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
0	RECESSED CAN LIGHT
-∳-₩	BATH EXAUST FAN WITH LIGHT
₩	BATH EXAUST FAN
-	LIGHT FIXTURE
Ф	DUPLEX OUTLET
•	220v OUTLET
Фан	GFI DUPLEX OUTLET
•	SMOKE DETECTOR
\$	WALL SWITCH
\$3	3 WAY WALL SWITCH
\$4	4 WAY WALL SWITCH
∯ <sub>WP/GFI</sub>	WATER PROOF GFI OUTLET
$\nabla$	PHONE JACK
0	TELEVISION JACK
更	GARAGE DOOR OPENER

WALL HEATER



1ST FLOOR ELECTRICAL PLAN SCALE: 1/4" = 1'-0" )"

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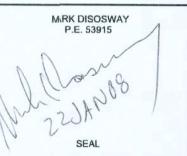
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WINDLOAD ENINEER: Mark Disosway, PE No.53915, P)B 868, Lake City, FL 32056, 386-754-419

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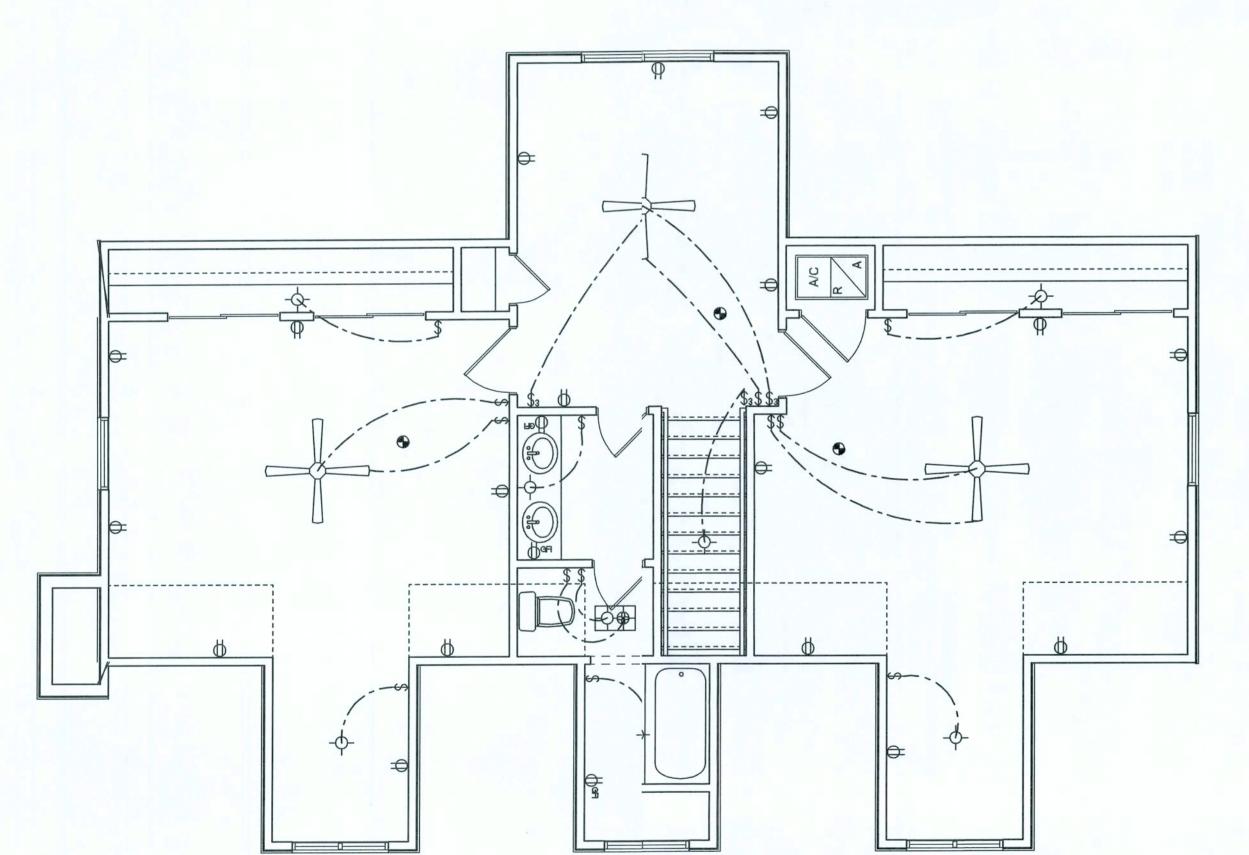
> PRINTED DATE: January 22, 2008

DRAWN BY STRUCTURAL BY: Evan Beamsley

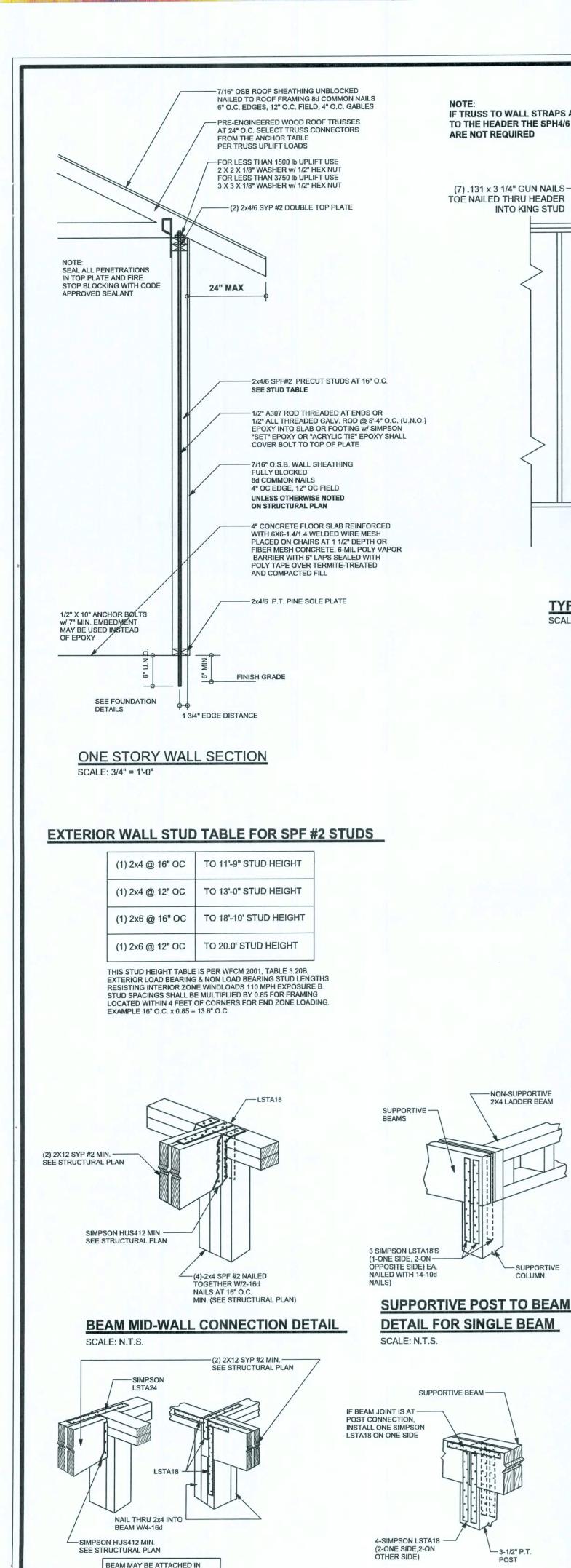
Jan 22, 2018

JOE NUMBER: 712103 DRAVING NUMBER

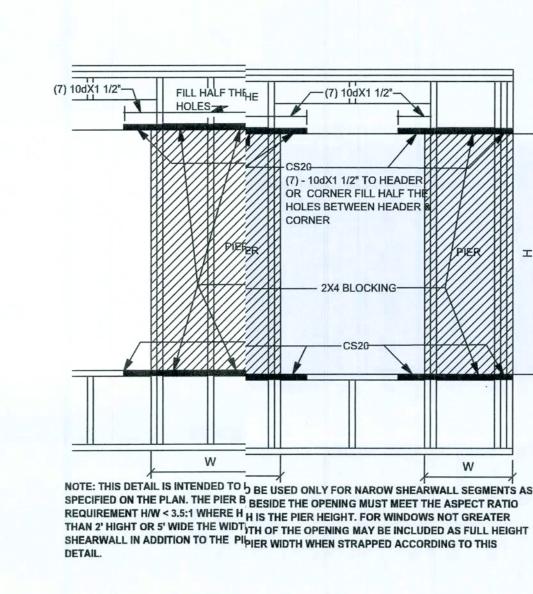
CF 7 SHEETS



2ND FLOOR ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"



**BEAM CORNER CONNECTION. DETAIL** 



FOR LESS THAN 1500 Ib UPLIFT USE

FOR LESS THAN 3750 Ib UPLIFT USE

-NAIL SHEATHING TO HEADER AND TOP

PLATE WITH 8d AT 3" O.C. FOR UPLIFT

-- SPH4/6 @ 48" O.C. (U.N.O.)/----(7) .131 x 3 1/4" GUN NAILS

-2X4 OUTRIGGER @ 24" O.C.

-7/16" OSB ROOF SHEATHING 8d 6" O.C.

EDGE, 12" O.C. FIELD, & 4" O.C. GABLES

-BLOCKING REQUIRED BETWEEN OUT RIGGERS

DIAGONAL BRACE MUST BE NAILED

OVER 12' IT MAY BE "T" BRACED UP

TO TRUSS WEBS FOR LENGTH

2X4X8' RAT RUN NAIL EACH

12dS = 12d SINKER

OR .135" X 3.125"

OR .131 X 3.25"

CONNECTION w/ (4) 12dS

TOE NAILED THRU HEADER

INTO KING STUD

2 X 2 X 1/8" WASHER

3 X 3 X 1/8" WASHER

CRIPPLES IF REQUIRED

(5) .131 x 3 1/4" GUN NAILS

—TOE NAILED THRU SILL —

INTO JACK STUD U.N.O.

TYPICAL STRAPPING (U.N.O.)

(1) 2X6 SPF #2 SILL UP TO 7'-6" U.N.O.

(2) 2X4 SPF #2 SILL UP TO 7'-8" U.N.O.

(1) 2X4 SPF #2 SILL UP TO 5'-1" U.N.O.

TYPIAL 1 STORY HEADER STRAPING DETAIL

H3 EACH -

EDGE & 12" O.C. FIELD

ATTACH RAT RUN TO -

TO TOP PLATE

SIMPSON LSTA21

@ 48" O.C. U.N.O.

-NON-SUPPORTIVE

2X4 LADDER BEAM

COLUMN

SUPPORTIVE BEAM -

SUPPORTIVE CENTER POST TO BEAM ETAIL

w/ (8) -16d TO TRUSS

SIMPSON H2.5A U.N.O.-

SEE STRUCTURAL PLAN

(2) SIMPSON LSTA21

w/(8)-16d TO HEADER

AND (8) -16d TO POST

AND (8) -16d TO WALL

12d @ 6" O.C.

(4) 12dS -

INSTALL 2X4 SPF #2 DIAGONAL BRACE AND NAIL TO BLOCKING AT TOP CHORD &

BOTTOM CHORD AND RAT RUN @ 6' O.C.

-(8) 12dS

-2X4 SPF #2 BLOCKING

SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.

GABLE BRACING DETAIL SCALE: 1/2" = 1'-0"

H3 INSTALLED HORIZONTALLY

FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED

-(2) 2X12 SYP #2 U.N.O.

-6X6 SYP #2 POST

-SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10"

-SEE FOOTING DETAILS

ANCHOR BOLT

(FOR: 120 MPH, 10'-0" WALL HEIGHT U.N.O.)

(SEE STRUCTURAL PLAN)

IF TRUSS TO WALL STRAPS ARNAILED

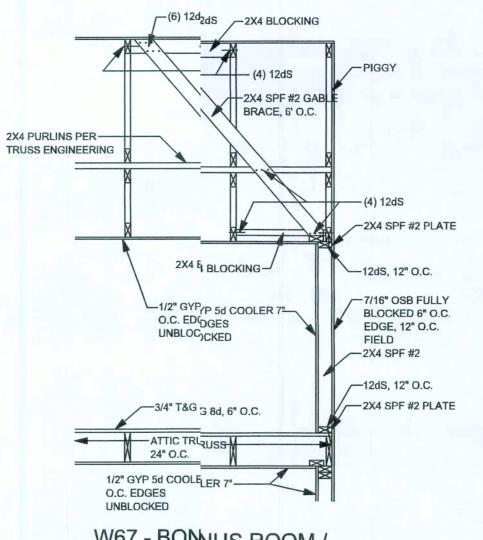
TO THE HEADER THE SPH4/6 @ " O.C.

(7) .131 x 3 1/4" GUN NAILS

INTO KING STUD

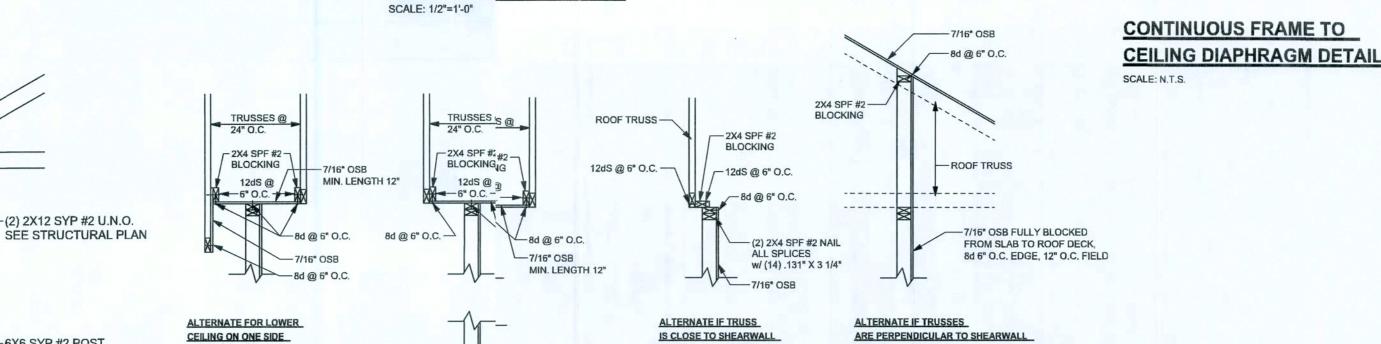
TOE NAILED THRU HEADER

ARE NOT REQUIRED



W67 - BONUS ROOM

GABLE ENID BRACING



- 2X4/6 SPF #2 STUDS

@ EACH END OF

SHEARWALL & @ 48" O.C. U.N.O.

EMBEDMENT IN SIMPSON AT U.N.O.

-1/2" X 8" ANCHOR BOLT @ 48" O.C. & 1/2" THD ROD EACH

END OF SHEARWALL w/ 6"

@ 16" O.C.

SEE FOUNDATION DETAILS (TYP.)

(NOTE: HAVE TRUSS DESIGNENER LOAD TRUSS FOR 400 PLF DRAG LOAD)

-7/16" OSB FULLY BLOCKED 8d 6" O.C. EDGE, 12" O.C. FIELD - STRAP STUDS w/ SPH4/6

ANCHOR TABLE

< 420

< 455

< 360

< 455

< 415

< 600

< 950

< 745

< 1465

< 990

< 760

< 1470

< 1470

< 1000

< 1450

< 2900

< 2050

< 10530

< 9250

< 435

< 455

< 825

< 825

< 885

< 1240

< 885

< 1240

< 1235

< 1235

< 1030

< 1705

< 1350

< 2310

< 2775

< 4175

< 1400

< 3335

< 2200

< 2300

< 2320

< 1465

MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP UPLIFT LBS. SPF

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

< 245

< 265

< 235

< 320

< 365

< 535

< 820

< 565

< 1050

< 1050

< 850

< 655

< 1265

< 1265

< 860

< 1245

< 2490

< 1785

< 9035

< 9250

< 435

< 420

< 825

< 600

< 760

< 1065

< 1065

< 1165

< 1235

< 1030

< 1705

< 1305

< 2310

< 2570

< 3695

< 1400

< 3335

< 2200

< 2300

< 2320

TRUSS CONNECTOR\*

H2.5

H2.5A

H6

H14-1

H14-2

H10-1

H10-2

H16-1

H16-2

MTS24C

2 - HTS24

LGT2

**HEAVY GIRDER TIEDOWNS** 

HGT-3

HGT-4

SSP DOUBLE TOP PLATE

SSP SINGLE SILL PLATE

DSP DOUBLE TOP PLATE

DSP SINGLE SILL PLATE

SPH4

SPH6

LSTA18

LSTA21

CS16

LTTI31

HTT16

PAHD42

HPAHD22

ABU66

ABU88

TO PLATES TO RAFTER/TRUSS

4-8d

4-8d

4-8d

5-8d

5-8d

5-10d, 1 1/2"

12-8d, 1 1/2"

12-8d, 1 1/2"

8-8d, 1 1/2"

6-10d

2-10d, 1 1/2"

2-10d, 1 1/2"

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

16 -10d

TO FOUNDATION

-5/8" THREADED ROD 12" EMBEDMENT

2-5/8" THREADED ROD

12" EMBEDMENT

2-5/8" THREADED ROD

12" EMBEDMENT

2-5/8" THREADED ROD

12" EMBEDMENT

TO STUDS

4-10d

8 -10d

8 -10d

6-10d, 1 1/2"

10-10d, 1 1/2"

6-10d, 1 1/2"

10-10d, 1 1/2"

TO FOUNDATION

1/2" AB

1/2" AB

5/8" AB

5/8" AB

1/2" AB

1/2" AB

2-5/8" AB

3-8d

4-8d

4-8d

5-8d

8-8d

5-10d, 1 1/2"

13-8d

15-8d

8-8d, 1 1/2"

6-10d

10-10d, 1 1/2"

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

1 -10d

6-10d

14-10d

16-10d

18-8d

28-8d TO STUDS

8-16d

18-10d, 1 1/2'

2-5/8" BOLTS

16-16d

16-16d

12-16d

12-16d

DOUBLE 2x4 SPF TOP PLATE NAILED -----

4' MIN. LAP w/ (12) - 16d OR 4" LAP w/

CS20 w/ (4) - 16d &(14) - 10d

INTERIOR CEILING AS SPECIFIED ON FLOOR PLAN

ALL STUDS TO BE 2x4 -

AND BOTTOM PLATES

CONTINUOUS FRAME TO

SPF NAILED TO TOP

WITH 2-16d NAILS

CONTINUOUS FRAME -

**BOTTOM CHORD OF TRUSS** 

TO TOP PLATE AT

PRE ENGINEERED ROOF TRUSS -

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

**GENERAL NOTES:** 

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO, SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY: 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

ASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

### **BUILDER'S RESPONSIBILITY**

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS. TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

#### ROOF SYSTEM DESIGN

THE WIND LOAD ENGINEER IMMEDIATELY.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF EBCR 3 LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

# **DESIGN DATA**

WIND LO	ADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
MEAN RO	ED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; OF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT R HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% ND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.
BUILDIN	IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUILDIN	IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH

WIND EXPOSURE = B

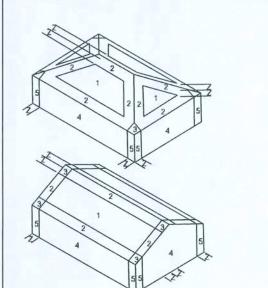
WIND IMPORTANCE FACTOR = 1.0

4.) BUILDING CATEGORY = II ROOF ANGLE = 10-45 DEGREES

MEAN ROOF HEIGHT = <30 FT

INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



25.5 40.6 25.5 68.3 23.6 29.1	18.1 18.1 18.1 18.5 18.5	-18.1 -21.8 -40.6 -21.8 -42.4 -20.4 -22.6
25.5 40.6 25.5 68.3 23.6 29.1	18.1 18.1 18.5	-21.8 -40.6 -21.8 -42.4 -20.4
40.6 25.5 68.3 23.6 29.1	18.1	-40.6 -21.8 -42.4 -20.4
25.5 68.3 23.6 29.1	18.5	-21.8 -42.4 -20.4
58.3 23.6 29.1	18.5	-42.4 -20.4
23.6		-20.4
29.1		-
	18.5	-22.6
ws :	21.8	-29.1
2)		
r	19.5	-22.9
or	18.5	-21.0
	r	?) r 19.5

Zone Effective Wind Area (ft2)

## DESIGN LOADS

LOOR	40 PSF (ALL OTHER DWELLING ROOMS)	
	30 PSF (SLEEPING ROOMS)	_
W	30 PSE (ATTICS WITH STORAGE)	-

10 PSF (ATTICS WITHOUT STORAGE, <3:12) ROOF 20 PSF (FLAT OR <4:12)

16 PSF (4:12 TO <12:12) 12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS) SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

SOFTPIAN

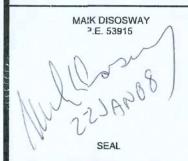
REVISONS

WINDLOAD ENGNEER: Mark Disosway PE No.53915, PO 868, Lake City, FL 32056, 386-754-519 Stated dimension:supercede scaled imensions. Refeall questions to Mark Disosway, PE. for resolution. Do not proceed whout clarification

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examined this plai, and that the applicable portions of the pla, relating to wind enginee comply with section R301.2.1, florida building code residential 204, to the best of my

LIMITATION: Thisdesign is valid for one building, at specifid location.



TIM & SARA PETERSEN

part of: 0:-5S-16-03429-002 SW State Road 47 Lake City, Florida

ADDRESS:

Mark Lisosway P.E. P.C. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (316) 269 - 4871

PRNTED DATE January 22, 2008 DRAWN BY: STRUCTURAL BY Evan Beamsley van Beamsley

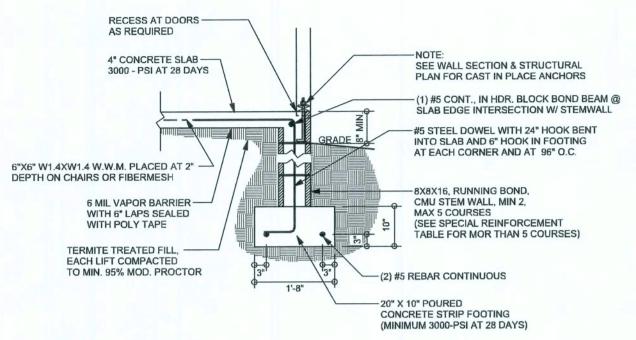
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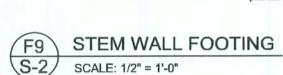
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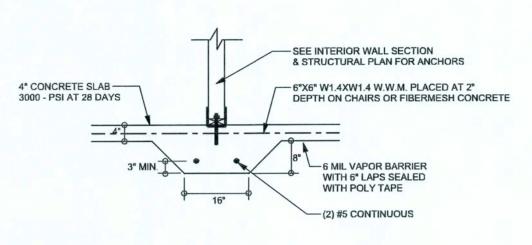
> > OI7 SHEETS

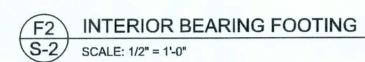
**GRADE & SPECIES TABLE** 

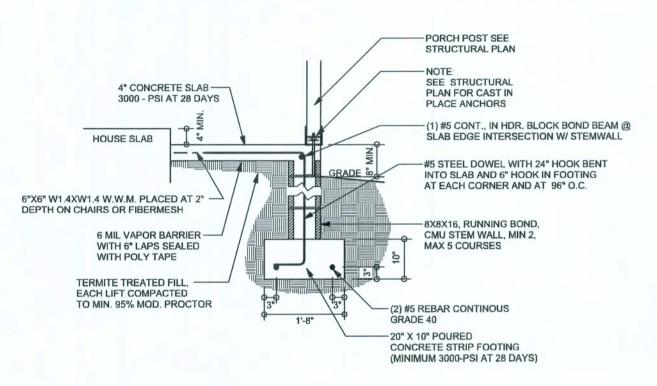
		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
DCI	DADALAM	2000	2.0



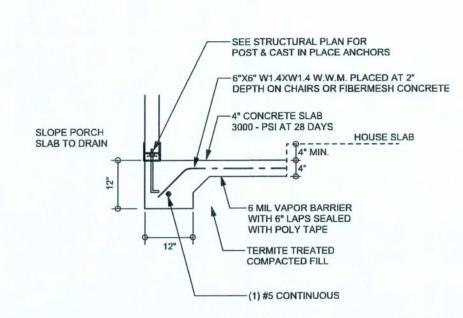










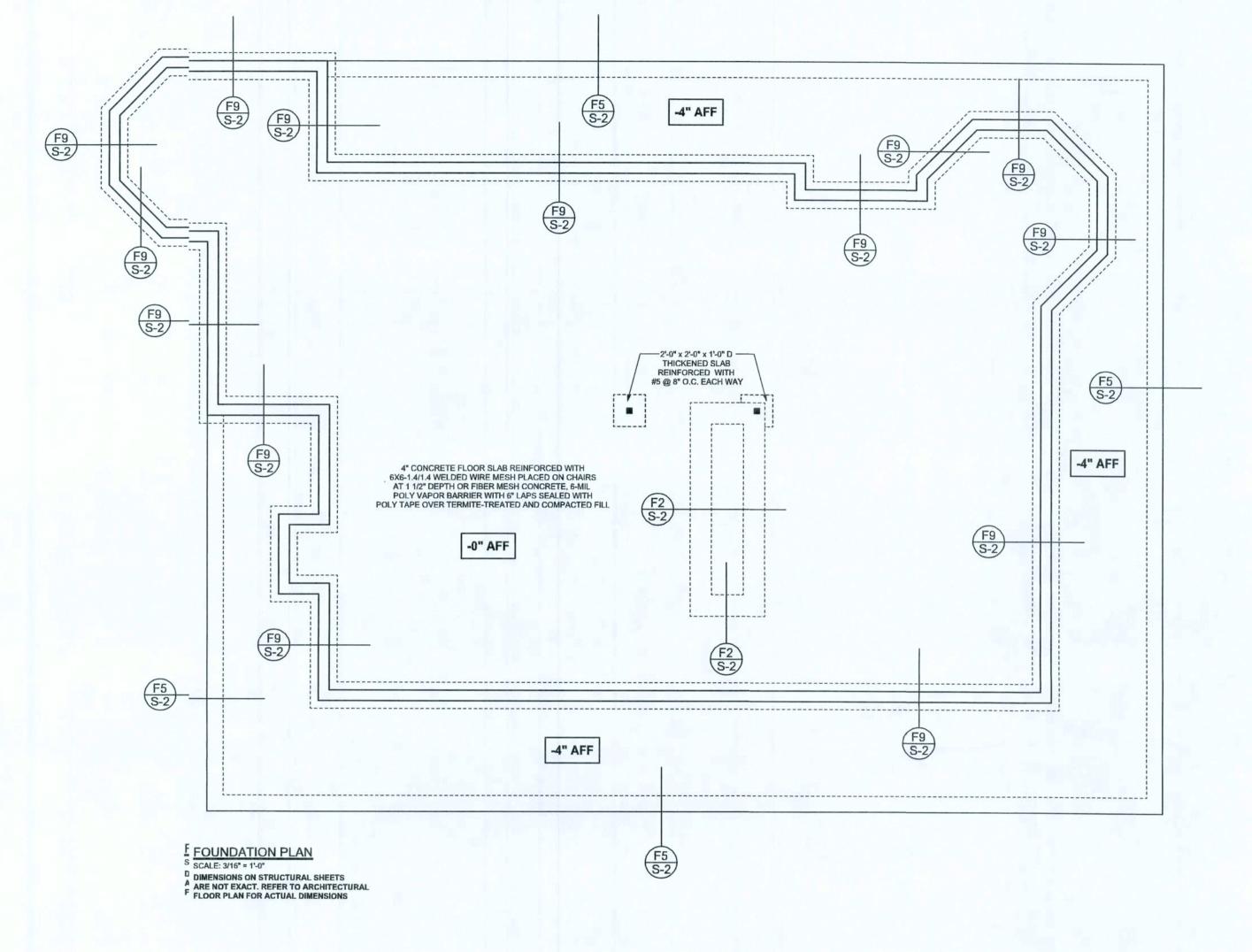


F5 PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"

## TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



REVISIONS

SOFIPIAN ARCHITECTUAL DESIGN SOFTWARE

WINDLOAD ENGINIER: Mark Disosway, PE No.53915, POB 68, Lake City, FL 32056, 386-754-541

Stated dimensions spercede scaled

dimensions. Refer alquestions to Mark Disosway, P.Efor resolution. Do not proceed withut clarification.

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CERTIFICATION: I breby certify that I have examined this plan, and that the applicable portions of the plan, elating to wind engineering comply with section \(^1301.2.1\), florida building code residential 200, to the best of my knowledge.

LIMITATION: This design is valid for one

MARKDISOSWAY
P.i. 53915

SEAL

TIM & SARA PETERSEN

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PRIN'ED DATE:
Januar 22, 2008

DRAWN BY: STRUCTURAL BY:

FINALS DATE: Jan 22, 2008

Evan Beamsley

JOB NUMBER: 7:2103 DRAWIIG NUMBER

**\$-2**OF 7SHEETS

Evan Beamsley

