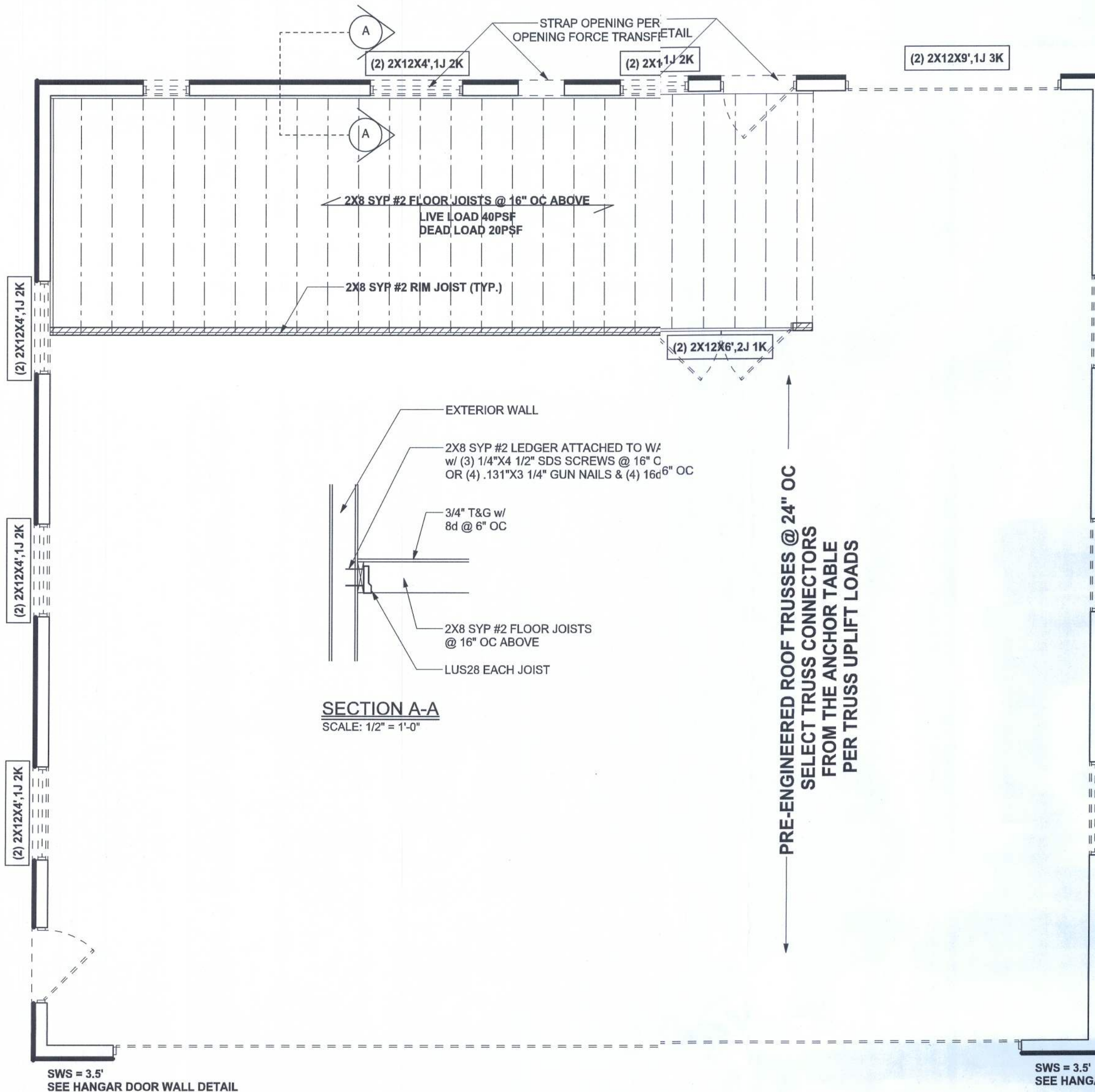




SECTION A-A
SCALE: 1/2\" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORT SHALL BE A MINIMUM OF (2) 2X12 SYP #2 ORCH HEADERS 1\" #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEAD SHALL HAVE (1) JACK STUD & (1) KING STADERS EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECT'S FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED TRUSS DRAWINGS. BCS1-B1, BCS1-B2, & BCS1-B3, BCS1-B1, BCS1-B2, & BCS1-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, BCS1-B2, & BCS1-B3 TRUSS PACKAGE

WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

HEADER LEGEND

	HEADER/BEAM CALL-OUT (U.N.O.)
	NUMBER OF KING STUDS (FULL LENGTH)
	NUMBER OF JACK STUDS (UNDER HEADER)
	SPAN OF HEADER
	SIZE OF HEADER MATERIAL
	NUMBER OF PLIES IN HEADER

TOTAL SHEAR WALL SEGMENTS

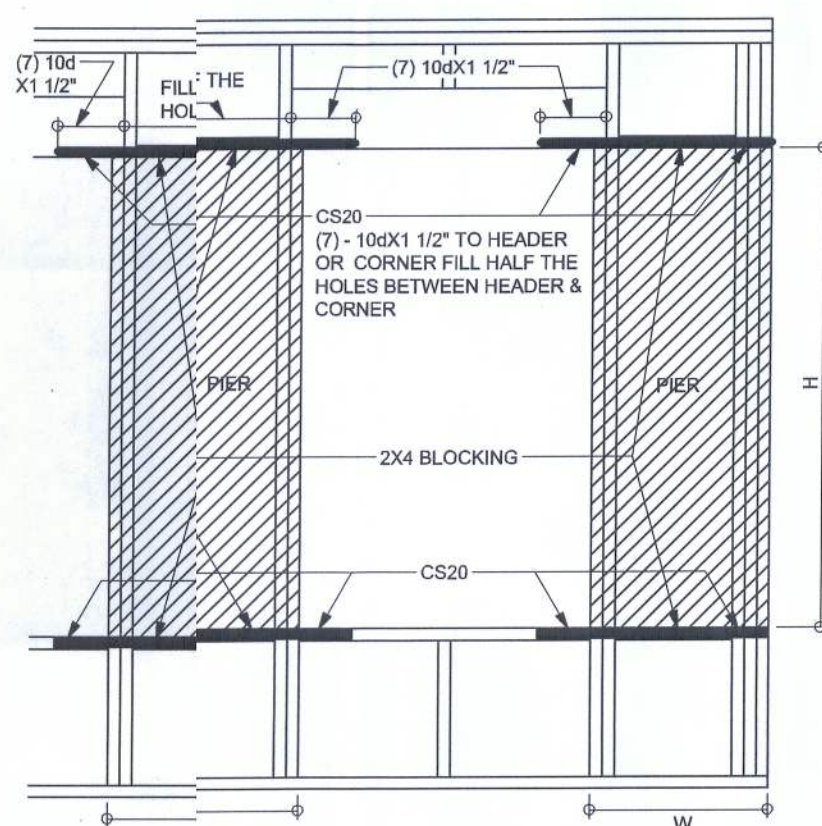
	REQUIRED	ACTUAL
TRANSVERSE	36.3'	37.5'
LONGITUDINAL	35.6'	51.5'

STRUCTURAL PLAN SCALE: 1/4\" = 1'-0"

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8\" black bearing walls F _m = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8\"x8\"x16\" running bond and 12\"x12\" or 16\"x16\" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5\"x2.75\"x11.5\"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 60, F _y = 60 ksi, Lap splices min 48 bar dia. (30\" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft ² or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft ² or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.



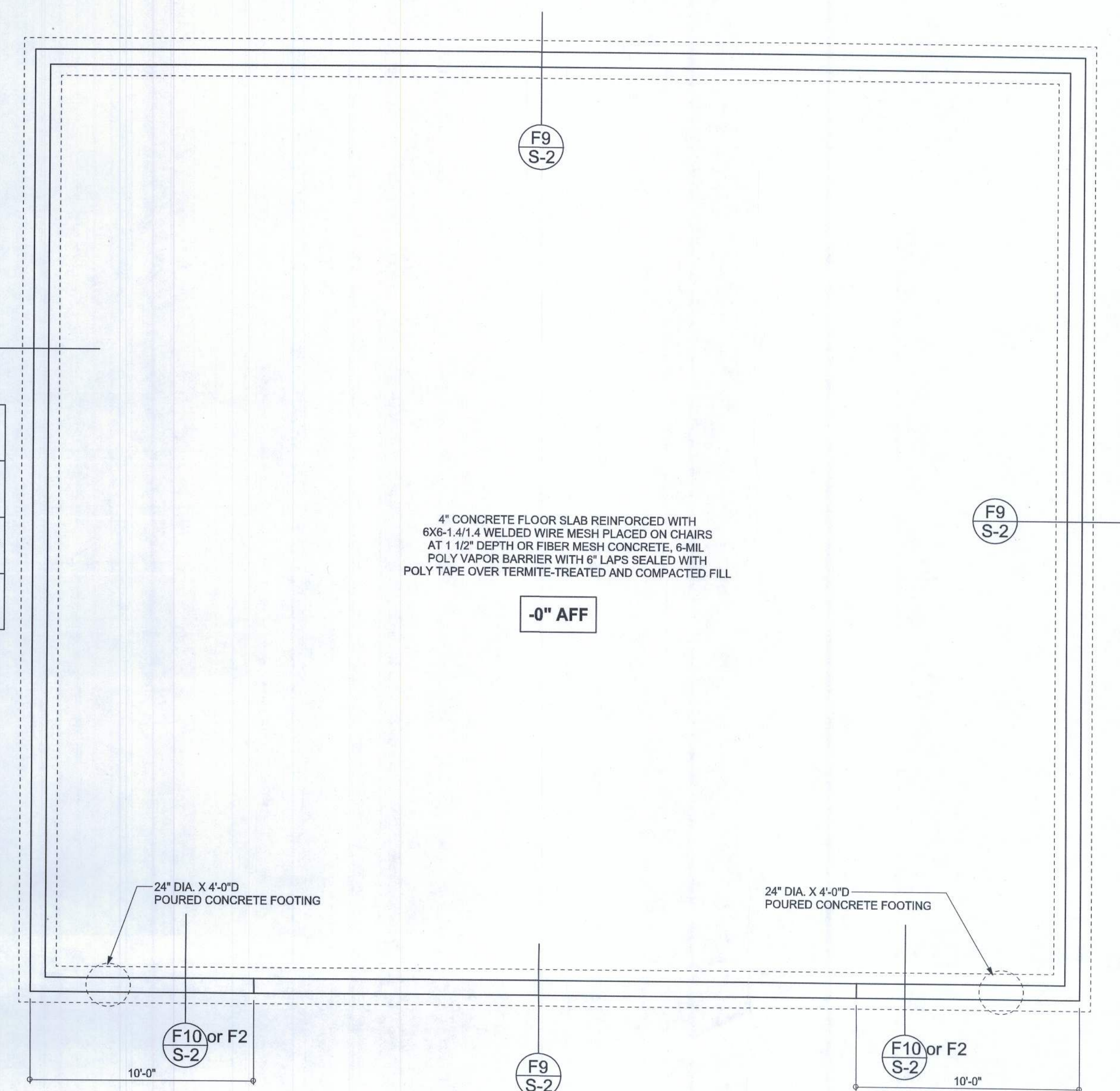
NOTE: THIS DETAIL IS INTENDED TO BE USED ONLY FOR NARROW SHEARWALL SEGMENTS AS SPECIFIED ON THE PIER BESIDE THE OPENING MUST MEET THE ASPECT RATIO REQUIREMENT H/W WHERE H IS THE PIER HEIGHT. FOR WINDOWS NOT GREATER THAN 2\" HIGH OR 4\" THE WIDTH OF THE OPENING MAY BE INCLUDED AS FULL HEIGHT SHEARWALL IN ADD TO THE PIER WIDTH WHEN STRAPPED ACCORDING TO THIS DETAIL.

OPENING FORCE TRANSFER
WOOD FRAME

TALL STEM WALL TABLE

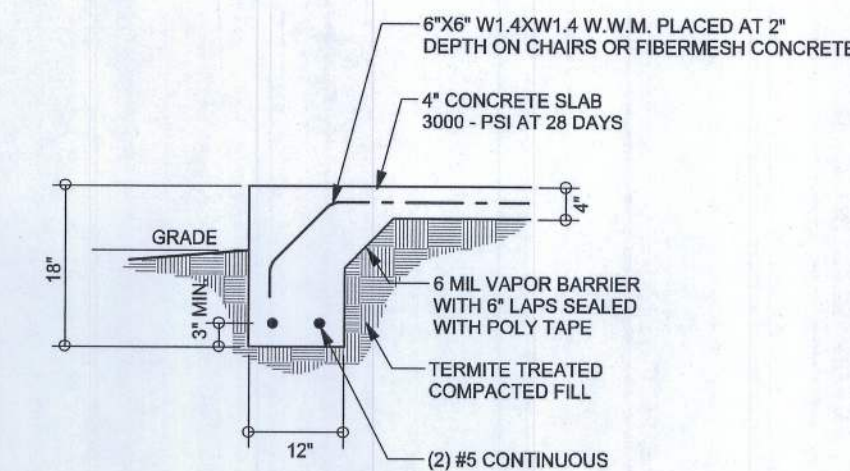
The table assumes 60 ksi reinforcing bars with 6\" hook in the footing and bent 24\" in reinforced slab at the top. The vertical steel is to be placed toward the tension side (24\" into the CMU wall away from the soil pressure, within 2\" of the exterior side of the wall). If side of the is over 8\" high, add Duowall ladder reinforcement at 16\" OC vertically or a horizontal. If the wall beam with 180 continuous at mid height. For higher parts of the wall 12\" CMU may total bond with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8\" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12\" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.7	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

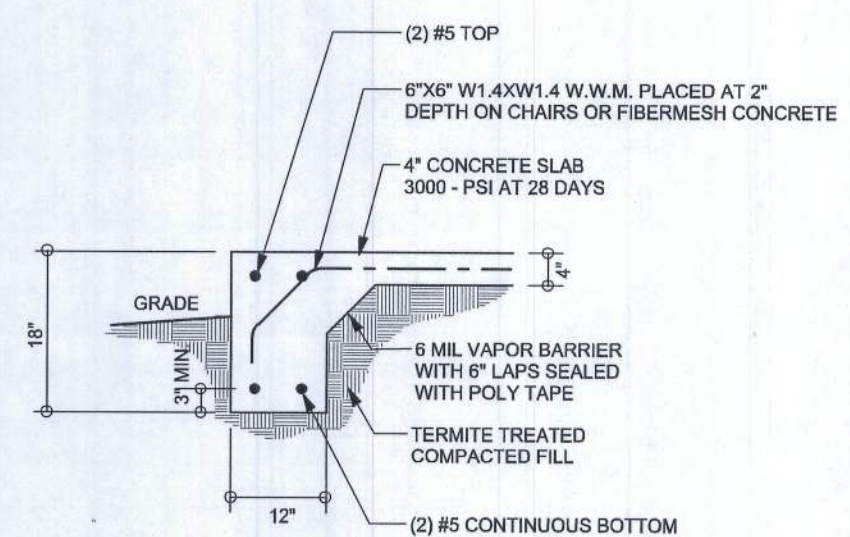


FOUNDATION PLAN SCALE: 1/4\" = 1'-0"

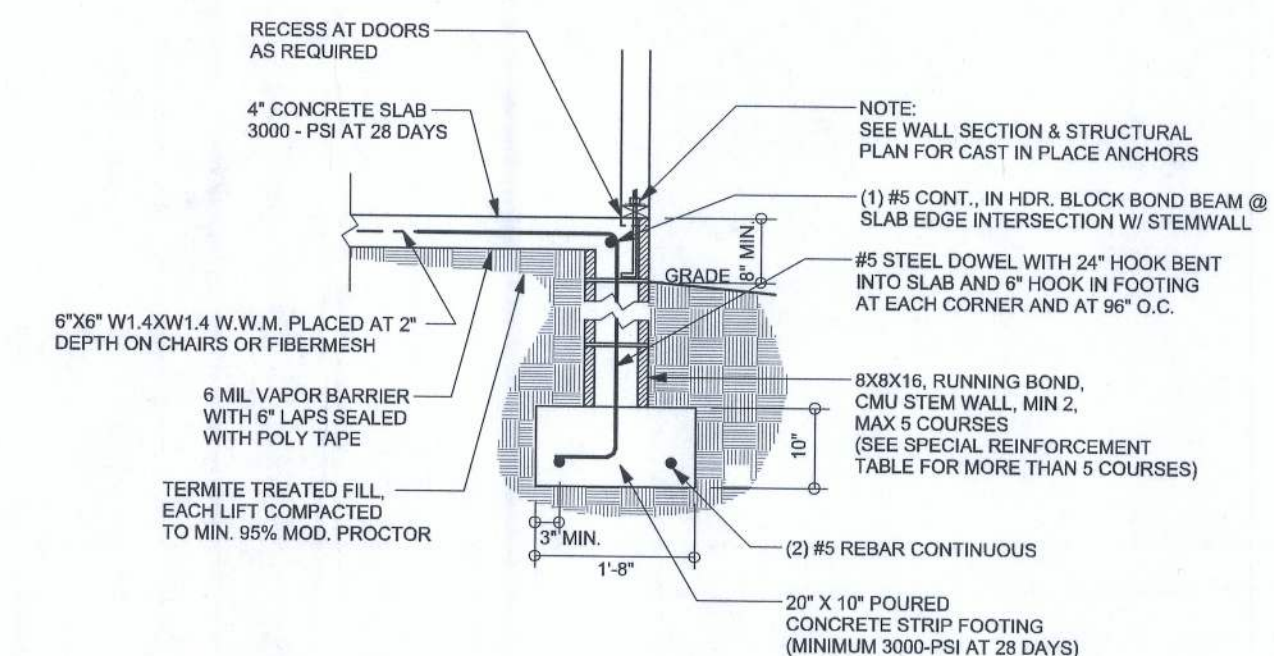
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS



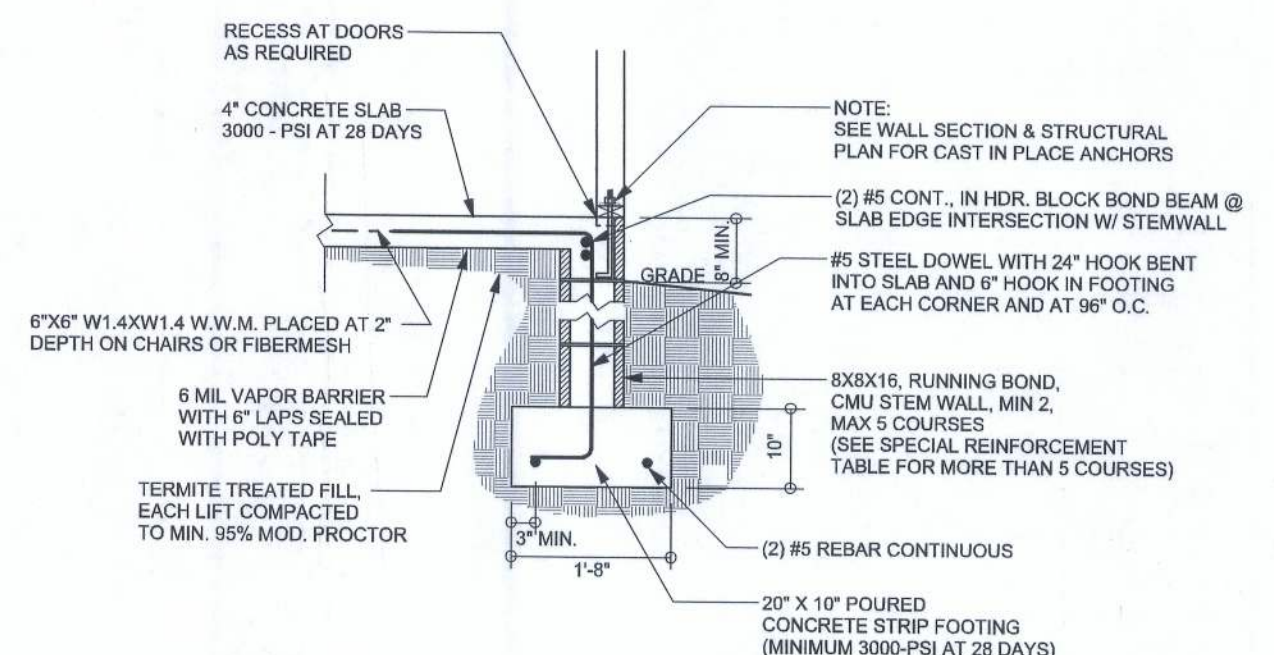
F1 S-2 MONOLITHIC FOOTING (OPTIONAL) SCALE: 1/2\" = 1'-0"



F2 S-2 MONOLITHIC FOOTING @ HANGAR DOOR WALL SCALE: 1/2\" = 1'-0"



F9 S-2 STEM WALL FOOTING SCALE: 1/2\" = 1'-0"



F10 S-2 STEM WALL FOOTING @ HANGAR DOOR WALL SCALE: 1/2\" = 1'-0"

REVISIONS
21Ct10

WINDLOAD ENGINEER:
Mark Disoway, P.E.
No. 5391, P.O. Box 868, Lake City, FL 32056,
385-754419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section RS01.2, Florida building code, residential 2007, and 2009 supplements to the best of my knowledge.

LIMITATION: This design is valid for one building, 1 specified location.

Edgley Construction

Donald R. Houston

ADDRESS:
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Lake City, Florida 32025

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Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
December 22, 2010

DRAWN BY: David Disoway
STRUCTURAL BY: David Disoway

FINAL DATE:
22Dec10

JOB NUMBER:
1012037

DRAWING NUMBER
S-2

OF 4 SHEETS