



COA #0 278 Florida Certificate of Product Approval #FL1999 12/20/2024 Alpine, an ITW Company 155 Harlem Ave North Building, 4th Floor Glenview, IL 60025 Phone: (800)755-6001 www.alpineitw.com

This item has been digitally signed by Douglas Fleming on the date adjacent to the seal.

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Site Information:	Page 1:	
Customer: W. B. Howland Company, Inc.	Job Number: 24-2041	
Job Description: AMBER WILLIS		
Address: Lake City, FL		

Job Engineering Criteria:			
Design Code: FBC 8th Ed. 2023 Res.	IntelliVIEW Version: 23.02.04		
	JRef #: 1Y5X2150008		
Wind Standard: ASCE 7-22 Wind Speed (mph): 130	Design Loading (psf): 40.00		
Building Type: Closed			

This package contains general notes pages, 14 truss drawing(s) and 3 detail(s).

Item	Drawing Number	Truss
1	354.24.1533.17577	A01
3	354.24.1533.14103	A03
5	354.24.1533.11240	B02
7	354.24.1532.43317	C02
9	354.24.1532.40627	C04
11	354.24.1532.37087	V02
13	354.24.1532.35203	V04
15	VAL180220723	
17	BRCLBSUB0119	

Item	Drawing Number	Truss
2	354.24.1533.15177	A02
4	354.24.1533.12880	B01
6	354.24.1532.44593	C01
8	354.24.1532.42037	C03
10	354.24.1532.37950	V01
12	354.24.1532.36240	V03
14	354.24.1532.33960	V05
16	VALTN220723	

#### **General Notes**

## Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

#### **Temporary Lateral Restraint and Bracing:**

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

#### Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

#### **Connector Plate Information:**

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

## **Bearing Information:**

The bearing area factor, Cb, is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

## **General Notes** (continued)

#### **Coated Lumber:**

Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

#### Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

#### **Key to Terms:**

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

C = Coated lumber.

C-AT = AtTEK coated lumber.

C-FX = FX Lumber Guard coated lumber.

C -TE = TechWood 4400 coated lumber.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-BF = Boraflame Fire Retardant Treated lumber

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-ON = OnWood Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

## **General Notes** (continued)

#### Key to Terms (continued):

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

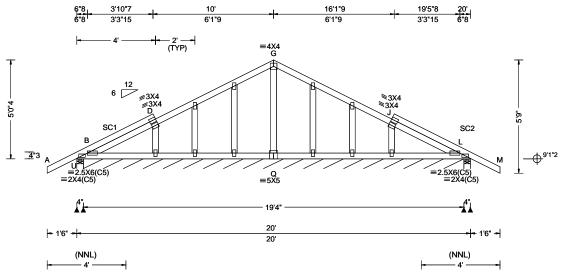
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

#### References:

- 1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
- 2. ICC: International Code Council; www.iccsafe.org.
- 3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
- 4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
- 5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com

SEQN: 792271 GABL Ply: 1 Job Number: 24-2041 Cust: R 215 JRef: 1Y5X2150008 T8 FROM: CDM AMBER WILLIS DrwNo: 354.24.1533.17577 Qty: 1 Truss Label: A01 SSB / DF 12/19/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.001 B 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.003 B 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.002 J
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.002 J
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.270
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.066
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.994
	Loc. from endwall: Any	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 23.02.04.0123.14
1		A. I. Peter and Market	

#### ▲ Maximum Reactions (lbs), or \*=PLF Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL U 300 /-/182 /165 В\* 65 /-/35 /11 /-300 /208 /56 Wind reactions based on MWFRS Brg Wid = 4.0 Min Req = 1.5 (Truss) Brg Wid = 232 Min Req = Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings U, B, & L are a rigid surface. Members not listed have forces less than 375#

#### Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Stack Chord: SC1 2x4 SP #2; Stack Chord: SC2 2x4 SP #2;

#### **Plating Notes**

All plates are 2X4 except as noted.

#### Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind loads based on MWFRS with additional C&C member design.

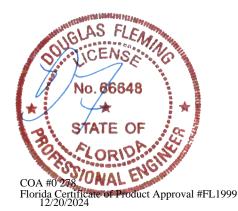
Wind loading based on both gable and hip roof types. Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/233.

#### **Additional Notes**

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is



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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

SEQN: 792274 COMN Ply: 1 Job Number: 24-2041 Cust: R 215 JRef: 1Y5X2150008 T6 FROM: CDM AMBER WILLIS DrwNo: 354.24.1533.15177 Qty: 2 Truss Label: A02 SSB / DF 12/19/2024 5'3"4 10' 14'8"12 20' 5'3"4 4'8"12 4'8"12 5'3"4 ≡4X4 D **%2X4** C <del>+</del>9'1"2 <u>4</u>"3 H ≡3X4 I ≡5X5

20'

6'3"11

13'1"13

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	1
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	١.
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.041 H 999 240	L
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.082 H 999 180	le
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.016 F	F
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.032 F	٧
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	E
Soffit: 2.00	BCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.223	ŀ
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.462	E
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.161	1
' '	Loc. from endwall: Any	FT/RT:20(0)/10(0)		"
	GCpi: 0.18	Plate Type(s):		] -
	Wind Duration: 1.60	WAVE	VIEW Ver: 23.02.04.0123.14	E
Lumber				- (

6'10"3

6'10"3

▲ Maximum Reactions (lbs)							
	Gravity		N	on-Grav	∕ity		
Loc R	+ /R-	/ Rh	/ Rw	/ U	/ RL	_	
B 924	4 /-	/-	/560	/167	/167		
F 924	4 /-	/-	/560	/167	/-		
Wind re	eactions b	ased on	<b>MWFRS</b>				
B Br	g Wid = 4	.0 Min	Req = 1.5	5 (Truss	s)		
F Br	g Wid = 4	.0 Min	Req = 1.5	5 (Truss	s)		
			d surface.	`	,		
	•	•	forces les	s than 3	375#		
Maxim	um Top	Chord Fo	orces Per	Ply (lb	s)		
Chords	Tens.C	omp.	Chords	Tens.	Ćomp.		
B-C	539	1356	D-E	540	- 1200		
C-D		1199	Ē-F	538	- 1357		

<del>|-</del> 1'6" <del>-</del>|

6'10"3

20'

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

#### Wind

Wind loads based on MWFRS with additional C&C member design.

=3X4(A1)

<del>-</del> 1'6" <del>-</del>

Wind loading based on both gable and hip roof types.

#### **Additional Notes**

The overall height of this truss excluding overhang is

## Maximum Bot Chord Forces Per Ply (lbs)

Cnoras	rens.comp.		Cnoras	rens. Comp.		
B - I		- 355	H-F	1156	- 361	
I - H	789	- 147				

#### Maximum Web Forces Per Ply (lbs)

webs rens.Comp.		vvebs	Tens. Comp.	
I - D	422 - 165	D-H	423	- 164



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For more information see these web sites: Alpine: alpineitw.com: TPI: binst.org: SBCA: sbcacomponents.com: ICC: iccsafe.org: AWC: awc.org



SEQN: 792277 COMN Ply: 1 Job Number: 24-2041 Cust: R 215 JRef: 1Y5X2150008 T9 FROM: CDM AMBER WILLIS DrwNo: 354.24.1533.14103 Qty: 1 Truss Label: A03 SSB / DF 12/19/2024 5'3"4 10' 14'8"12 20 5'3"4 4'8"12 4'8"12 5'3"4 ≡4X4 C **⊕**9'1"2 <u>4</u>"3 G ≡5X5 F ≡3X4 =3X4(A1)  $\equiv 3X4(A1)$ 

20'

6'3"11

13'1"13

Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	▲ Ma
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.039 F 999 240 VERT(CL): 0.081 F 999 180 HORZ(LL): 0.016 E HORZ(TL): 0.033 E Creep Factor: 2.0 Max TC CSI: 0.254 Max BC CSI: 0.474 Max Web CSI: 0.170  VIEW Ver: 23.02.04.0123.14	Loc A & Bear Mem Max Choi
Lumber				- `

6'10"3

6'10"3

▲ Maximum Reactions (lbs)								
	Gravity		No	on-Grav	vity			
Loc R	- /R-	/ Rh	/ Rw	/ U	/ RL			
A 824	. /-	/-	/475	/141	/129			
E 824	. /-	/-	/475	/141	/-			
Wind re	actions b	ased on	MWFRS					
A Brg	Wid = 4	.0 Min	Req = 1.5	(Trus	s)			
E Brg	Wid = 4	.0 Min	Req = 1.5	(Trus	s)			
Bearing	s A & E a	are a rigio	d surface.					
Membe	rs not list	ed have	forces less	s than 3	375#			
Maximu	ım Top (	Chord Fo	rces Per	Ply (lb	s)			
Chords	Tens.Co	omp.	Chords	Tens.	Comp.			
A - B	389 -	1393	C-D	397	- 1235			
B-C	397 -	1235	D-F	389	- 1394			

6'10"3

20'

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

#### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

#### **Additional Notes**

The overall height of this truss excluding overhang is

## Maximum Bot Chord Forces Per Ply (lbs)

Cilolus	i ciis.c	onip.	Cilolus	i ciis. V	Jonep.	
A - G	1193		F-E	1194	- 284	

## Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.	
G-C	444 - 116	C-F	446 - 115	



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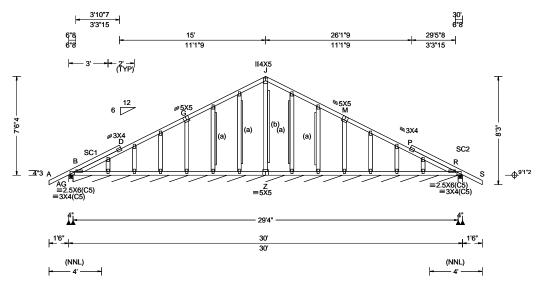
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SEQN: 792285 GABL Ply: 1 Job Number: 24-2041 Cust: R 215 JRef: 1Y5X2150008 T2 DrwNo: 354.24.1533.12880 FROM: CDM AMBER WILLIS Qty: 1 Truss Label: B01 SSB / DF 12/19/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00	Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA	PP Deflection in loc L/defl L/# VERT(LL): 0.002 D 999 240 VERT(CL): 0.004 D 999 180 HORZ(LL): 0.005 P -
Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any	Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0)	HORZ(TL): 0.006 P Creep Factor: 2.0  Max TC CSI: 0.275  Max BC CSI: 0.041  Max Web CSI: 0.902
	GCpi: 0.18 Wind Duration: 1.60	Plate Type(s):  WAVE	VIEW Ver: 23.02.04.0123.14

#### ▲ Maximum Reactions (lbs), or \*=PLF Non-Gravity Gravity Loc R+ /Rh /Rw /U /RL AG 292 /159 /45 /229 /-/38 **B**\* 71 /13 /-292 /193 /42 Wind reactions based on MWFRS AG Brg Wid = 4.0 Min Req = 1.5 (Truss) Brg Wid = 352 Min Req = Brg Wid = 4.0 Min Req = 1.5 (Truss) Bearings AG, B, & R are a rigid surface. Members not listed have forces less than 375#

#### Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Stack Chord: SC1 2x4 SP #2; Stack Chord: SC2 2x4 SP #2;

#### **Plating Notes**

All plates are 2X4 except as noted.

#### Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/278.

#### **Gable Reinforcement**

(a) 1x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.

(b) 2x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.

#### **Additional Notes**

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is



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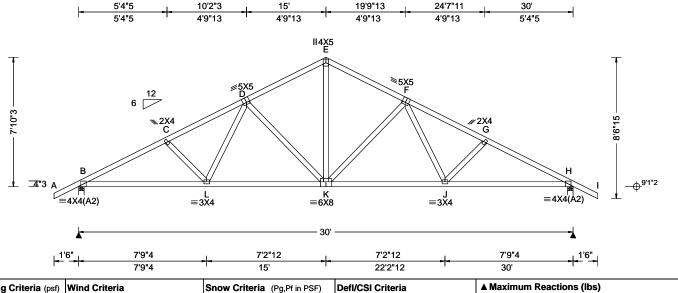
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Glenview, IL 60025

155 Harlem Ave North Building, 4th Floor SEQN: 792301 COMN Ply: 1 Job Number: 24-2041 Cust: R 215 JRef: 1Y5X2150008 T1 FROM: CDM DrwNo: 354.24.1533.11240 Qty: 12 AMBER WILLIS Truss Label: B02 SSB / DF 12/19/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
Loading Criteria (psf)   TCLL: 20.00   TCDL: 10.00   BCLL: 0.00   BCDL: 10.00   Des Ld: 40.00   NCBCLL: 10.00   Soffit: 2.00   Load Duration: 1.25   Spacing: 24.0   "	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCbi: 0.18	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s):	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.101 K 999 240 VERT(CL): 0.205 K 999 180 HORZ(LL): 0.041 H HORZ(TL): 0.084 H Creep Factor: 2.0 Max TC CSI: 0.299 Max BC CSI: 0.662 Max Web CSI: 0.506
	Wind Duration: 1.60	WAVE	VIEW Ver: 23.02.04.0123.14
Lumber			

#### Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

#### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

#### **Additional Notes**

The overall height of this truss excluding overhang is

Wind reactions based on MWFRS

Brg Wid = 4.0 Min Req = 1.6 (Truss) Bearings B & H are a rigid surface. Members not listed have forces less than 375#

Gravity

/R

Brg Wid = 4.0

Loc R+

1336 /-

1336

Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

/Rh

/-

Non-Gravity

/239 /232

/239 /-

/RL

/Rw /U

/797

/797

Min Reg = 1.6 (Truss)

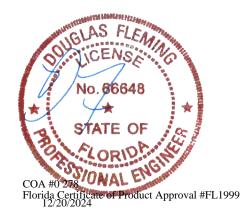
B-C C-D	865 - 2221	E-F	687	- 1433
C-D	825 - 2013	F-G	825	- 2013
D-E	687 - 1433	G - H	865	- 2221

#### Maximum Bot Chord Forces Per Ply (lbs) Chords Tens. Comp. Chords Tens.Comp.

B-L	1923	- 644	K-J	1588	- 452
L-K	1588	- 447	J - H	1923	- 648

## Maximum Web Forces Per Ply (lbs)

Webs	Tens.C	Comp.	Webs	Tens. (	Comp.
L-D	416	-96	K-F	321	- 531
D-K	321	- 531	F-J	416	-96
F-K	917	- 388			



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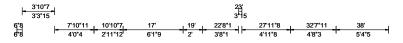
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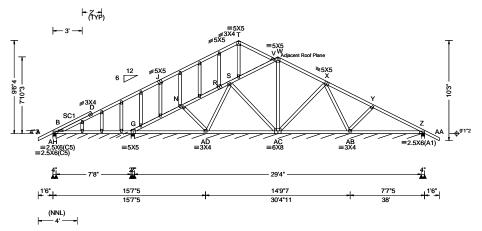
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155 Harlem Ave North Building, 4th Floor Glenview, IL 60025





Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.020 J 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.041 J 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.012 K
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.025 K
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.315
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.432
Spacing: 24.0 "	C&C Dist a: 3.80 ft	Rep Fac: Yes	Max Web CSI: 0.385
, -	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 23.02.04.0123.14

## Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Stack Chord: SC1 2x4 SP #2;

#### **Plating Notes**

All plates are 2X4 except as noted.

#### Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types. Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/688.

Laterally brace top chord below filler and bottom chord above filler at 24" o.c.,including a lateral brace at chord ends (If no rigid diaphragm exists at that point)

## **Additional Notes**

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is

-							
	▲ M	laxim	um Rea	actions (	lbs), or *=	PLF	
		(	3ravity		No	n-Gra	vity
	Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
	АН	324	/-	/-	/174	/13	/294
	В*	56	/-	/-	/36	/11	/-
	G	352	/-	/-	/248	/22	/-
	G*	62	/-	/-	/34	/0	/-
	Z	419	/-	/-	/307	/26	/-
	Wir	nd rea	ctions b	ased on	MWFRS		
	ΑН	Brg \	Nid = 4	.0 Min	Req = 1.5	(Trus	s)
	В	Brg \	Nid = 9	2.0 Min	Req = -		
	G	Brg \	Nid = 4	.0 Min	Req = 1.5	5	
_	G	Brg \	Nid = 3	52 Min	Req = -		
	Z	Brg \	Nid = 4	.0 Min	Req = 1.5	(Trus	s)
			AH, B,	G, G, & 2	Z are a rigi	id	
	sur	face.					

Members not listed have forces less than 375#



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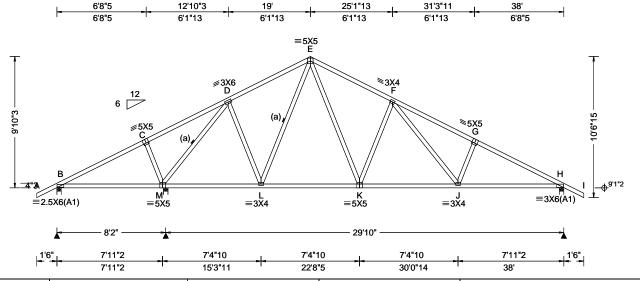
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155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

SEQN: 792259 COMN Ply: 1 Job Number: 24-2041 Cust: R 215 JRef: 1Y5X2150008 Т3 FROM: CDM AMBER WILLIS DrwNo: 354.24.1532.43317 Qty: 13 Truss Label: C02 SSB / DF 12/19/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA	PP Deflection in loc L/defl L/# VERT(LL): 0.111 F 999 240 VERT(CL): 0.201 F 999 180 HORZ(LL): 0.038 E HORZ(TL): 0.069 E
NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.80 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Creep Factor: 2.0 Max TC CSI: 0.491 Max BC CSI: 0.917 Max Web CSI: 0.600  VIEW Ver: 23.02.04.0123.14

<b>▲</b> N	▲ Maximum Reactions (IDS)						
	G	ravity		Non-Gravity			
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
В	452	/-	/-	/300	/83	/294	
М	1821	/-	/-	/896	/271	/-	
Н	1477	/-	/-	/855	/238	/-	
Wi	nd read	ctions b	ased on I	MWFRS			
В	Brg V	Vid = 4.	0 Min	Req = 1.5	(Truss	s)	
М	Brg V	Vid = 4.	0 Min	Req = 2.1	Ŭ (Truss	s)	
Н	Brg V	Vid = 4.	0 Min	Req = 1.7	' (Truss	s)	
Bea	arings	B, M, &	H are a	rigid surfa	ce.		
Me	mbers	not list	ed have f	orces less	than 3	375#	
Ma	ximun	Top C	hord Fo	rces Per	Ply (lb	s)	
Ch	ords 7	Tens.Co	omp.	Chords	Tens.	Ćomp.	
D-	E	387 -	1247	F-G	524	- 2353	

## Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

(a) Continuous lateral restraint equally spaced on

## Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

#### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

#### **Additional Notes**

The overall height of this truss excluding overhang is 9-10-3.

E-F	467 - 1678	G-H	475 - 2492

## Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.C	Comp.	Chords	Tens. (	Comp.
M - L	1817	- 145	K-J	1653	- 167
L-K	1065	-9	J - H	2152	- 309

## Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. C	Comp.
С - М	201 - 387	K-F	274	- 634
M - D	272 - 1613	F-J	626	- 126
E-K	986 - 199			



COA #0 278
Florida Certificate of Product Approval #FL1999
12/20/2024

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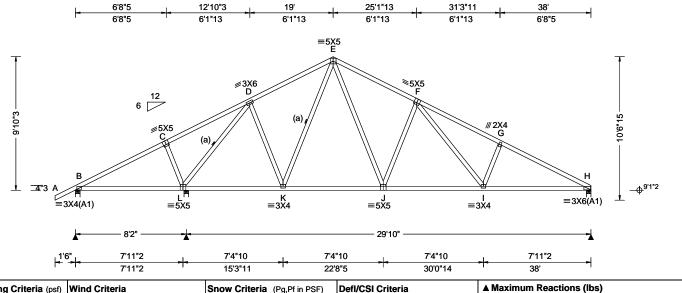
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SEQN: 792262 COMN Ply: 1 Job Number: 24-2041 Cust: R 215 JRef: 1Y5X2150008 T5 FROM: CDM Qty: 12 AMBER WILLIS DrwNo: 354.24.1532.42037 Truss Label: C03 SSB / DF 12/19/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	l
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.111 F 999 240	
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.200 F 999 180	
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.038 H	
Des Ld: 40.00	EXP: C Kzt: NA Mean Height: 15.00 ft		HORZ(TL): 0.069 H	
NCBCLL: 10.00	TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	
Soffit: 2.00	BCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.491	
Load Duration: 1.25	MWFRS Parallel Dist: h to 2h	TPI Std: 2014	Max BC CSI: 0.918	
Spacing: 24.0 "	C&C Dist a: 3.80 ft	Rep Fac: Yes	Max Web CSI: 0.600	
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)		
	GCpi: 0.18	Plate Type(s):		1
	Wind Duration: 1.60	WAVE	VIEW Ver: 23.02.04.0123.14	
Lumber				-

#### Chords Tens.Comp. Chords Tens. Comp. D - E F-G 540 - 2382 385 - 1252 E-F 467 - 1683 G-H 491 - 2520

Non-Gravity

/29

/RL

/279

/Rw /U

/303

/892 /35 /-

/768

Min Req = 1.5 (Truss)

Min Req = 2.1 (Truss)

Min Req = 1.6 (Truss)

Top chord: 2x4 SP #2;

Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

(a) Continuous lateral restraint equally spaced on

## Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

#### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

#### **Additional Notes**

The overall height of this truss excluding overhang is 9-10-3.

## Maximum Bot Chord Forces Per Ply (lbs)

/Rh

/-

Wind reactions based on MWFRS Brg Wid = 4.0

Bearings B, L, & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)

Gravity

Brg Wid = 4.0

Brg Wid = 4.0

Loc R+

1821 /-

1374 /-

В 453 /-

Chords	Tens.C	comp.	Chords	Tens.	Comp.
L-K	1826	- 167	J - I	1661	- 206
K - J	1070	- 51	I - H	2181	- 364

Maximum web Forces Per Ply (lbs)					
Webs	Tens.Comp.	Webs	Tens. (	Comp.	
C-L	201 - 387	J - F	276	- 639	
L - D	282 - 1613	F-I	652	- 157	
E-J	989 - 200				



COA #0 278
Florida Certificate of Product Approval #FL1999
12/20/2024

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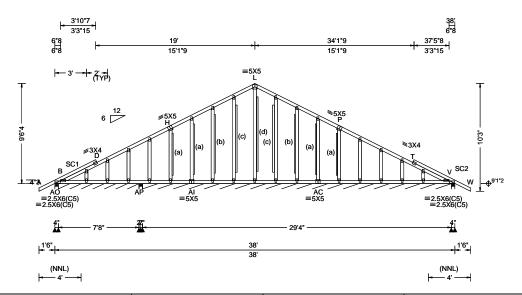
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SEQN: 792267 GABL Ply: 1 Job Number: 24-2041 Cust: R 215 JRef: 1Y5X2150008 T4 Qty: 1 FROM: CDM AMBER WILLIS DrwNo: 354.24.1532.40627 Page 1 of 2 Truss Label: C04 SSB / DF 12/19/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.002 T 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.004 T 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.007 T
Des Ld: 40.00	EXP: C Kzt: NA Mean Height: 15.00 ft		HORZ(TL): 0.008 T
NCBCLL: 10.00	TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.275
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.042
Spacing: 24.0 "	C&C Dist a: 3.80 ft	Rep Fac: Yes	Max Web CSI: 0.992
	Loc. from endwall: Any	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 23.02.04.0123.14

#### Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Stack Chord: SC1 2x4 SP #2; Stack Chord: SC2 2x4 SP #2;

#### **Plating Notes**

All plates are 2X4 except as noted.

#### Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types. Gable meets L/120 deflection criteria for wind load applied to face. Calculated deflection ratio is L/214.

#### **Gable Reinforcement**

(a) 1x4 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.

(b) 1x4 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder. (c) 2x4 "L" reinforcement. Same species and grade as web. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4" oc for the remainder.
(d) 2x6 "L" reinforcement. Any species and grade. 80% length of web member. Attach with 10d (0.131"x3",min.) nails @ 2" oc at each end for the first 18" and then 4"

	▲ M	laxim	num Rea	ection	s (lb:	s), or *:	-PLF		
ı		(	Gravity			N	on-Gra	vity	
ı	Loc	R+	/ R-	/RI	h	/Rw	/ U	/ RL	
ı	ΑO	294	/-	/-		/147	/43	/291	
ı	В*	60	/-	/-		/41	/15	/-	
ı	AΡ	27	/-	/-		/14	/-	/-	
ı	AP*	77	/-	/-		/42	/14	/-	
ı	٧	294	/-	/-		/190	/30	/-	
ı	Win	d rea	actions b	ased o	on M	WFRS			
ı	ΑO	Brg	Wid = 4	.0 M	lin Re	eq = 1.5	5 (Trus	s)	
ı	В	Brg	Wid = 9	2.0 N	lin Re	eq = -	•	•	
ı	AΡ	Brg	Wid = 4	.0 M	lin Re	eq = 1.5	5 (Trus	s)	
_			Wid = 3				•	,	
ı	٧	Brg	Wid = 4	.0 M	lin Re	eq = 1.5	5 (Trus	s)	
_	Bea		AO, B,					,	
			face.	,	,				
	Mor	mhor	not list	ad bas	o for	ooo loo	a than	275#	

Members not listed have forces less than 375# Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp.

B-AI 419 - 194



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SEQN: 792267 GABL Ply: 1 Job Number: 24-2041 Cust: R 215 JRef: 1Y5X2150008 FROM: CDM AMBER WILLIS DrwNo: 354.24.1532.40627 Qty: 1 Page 2 of 2 Truss Label: C04 SSB / DF 12/19/2024

#### Additional Notes

Exposed portion of gable face shall be reinforced with sheathing and the wind pressures shall be transferred into lateral diaphragms. Connections and designs for diaphragms is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 9-6-4.



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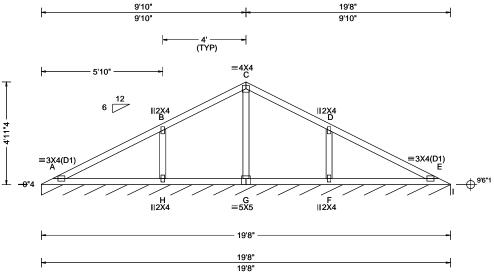
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SEQN: 792288 VAL Ply: 1 Job Number: 24-2041 Cust: R 215 JRef: 1Y5X2150008 T10 FROM: CDM AMBER WILLIS Qty: 1 DrwNo: 354.24.1532.37950 Truss Label: V01 SSB / DF 12/19/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.020 E 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.041 E 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.007 E
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.014 E
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.451
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.263
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.156
	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 23.02.04.0123.14
1 1			

#### ▲ Maximum Reactions (lbs), or \*=PLF Gravity Non-Gravity Loc R+ /R /Rh /Rw /U /RL 82 /-/-/42 /13 Wind reactions based on MWFRS Brg Wid = 235 Min Req = Bearing A is a rigid surface. Members not listed have forces less than 375#

#### Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

#### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

#### **Additional Notes**

See DWGS VALTN220723 and VAL180220723 for valley details.

The overall height of this truss excluding overhang is 4-11-4.



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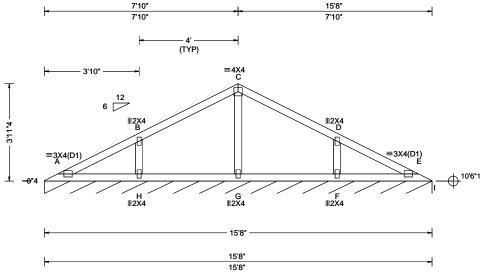
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155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

SEQN: 792290 VAL Ply: 1 Job Number: 24-2041 Cust: R 215 JRef: 1Y5X2150008 T11 FROM: CDM Qty: 1 AMBER WILLIS DrwNo: 354.24.1532.37087 Truss Label: V02 SSB / DF 12/19/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.004 E 999 240
DCLL. 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.009 E 999 180
	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.002 E
ID⇔Id∙ 40 00	EXP: C Kzt: NA Mean Height: 15.00 ft		HORZ(TL): 0.004 E
NCBCLL: 10.00	TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 8th Ed. 2023 Res.	Max TC CSI: 0.283
	MWFRS Parallel Dist: h/2 to h	TPI Std: 2014	Max BC CSI: 0.134
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.071
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	
	GCpi: 0.18	Plate Type(s):	
	Wind Duration: 1.60	WAVE	VIEW Ver: 23.02.04.0123.14
Lumban			

#### ▲ Maximum Reactions (lbs), or \*=PLF Gravity Non-Gravity Loc R+ /R /Rh /Rw /U /RL 82 /-/-/42 /12 Wind reactions based on MWFRS Brg Wid = 187 Min Req = Bearing A is a rigid surface. Members not listed have forces less than 375#

#### Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

#### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

#### **Additional Notes**

See DWGS VALTN220723 and VAL180220723 for valley details.

The overall height of this truss excluding overhang is 3-11-4.



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SEQN: 792292 VAL Ply: 1 Job Number: 24-2041 Cust: R 215 JRef: 1Y5X2150008 T12 FROM: CDM Qty: 1 AMBER WILLIS DrwNo: 354.24.1532.36240 Truss Label: V03 SSB / DF 12/19/2024 5'10" 11'8" 5'10' 5'10' =3X4(D1) =3X4(D1) 11'6"1 \_\_\_\_\_\_ ∥2X4 11'8" 5'10' 5'10" 5'10' 11'8' Loading Criteria (psf) Wind Criteria Snow Criteria (Pg,Pf in PSF) Defl/CSI Criteria ▲ Maximum Reactions (lbs), or \*=PLF Gravity Non-Gravity Wind Std: ASCE 7-22 Pg: NA Ct: NA CAT: NA TCLL: 20.00 PP Deflection in loc L/defl L/# Loc R+ /R /Rw /U /RL Speed: 130 mph TCDL: 10.00 Pf: NA Ce: NA VERT(LL): 0.023 C 999 240 Enclosure: Closed VERT(CL): 0.048 C BCII: 0.00 Lu: NA Cs: NA 999 180 E\* 82 /-/-Risk Category: II BCDL: 10.00 Snow Duration: NA HORZ(LL): -0.009 C Wind reactions based on MWFRS EXP: C Kzt: NA Brg Wid = 139 Min Req = HORZ(TL): 0.019 C Des Ld: 40.00 Mean Height: 15.00 ft Bearing A is a rigid surface. **Building Code:** Creep Factor: 2.0 NCBCLL: 10.00 TCDL: 5.0 psf Members not listed have forces less than 375# FBC 8th Ed. 2023 Res. Max TC CSI: 0.463 Soffit: 2.00 BCDL: 5.0 psf Maximum Top Chord Forces Per Ply (lbs) TPI Std: 2014 Max BC CSI: 0.393 Load Duration: 1.25 MWFRS Parallel Dist: h/2 to h Chords Tens.Comp. Chords Tens. Comp. Rep Fac: Yes Max Web CSI: 0.142 Spacing: 24.0 " C&C Dist a: 3.00 ft FT/RT:20(0)/10(0) Loc. from endwall: not in 9.00 ft A - B 482 - 220 B-C 482 - 233 GCpi: 0.18 Plate Type(s): VIEW Ver: 23.02.04.0123.14 Wind Duration: 1.60 Maximum Web Forces Per Ply (lbs) <u>WA</u>VE Tens.Comp. Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2: Webs: 2x4 SP #3;

#### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

#### **Additional Notes**

See DWGS VALTN220723 and VAL180220723 for valley details.

The overall height of this truss excluding overhang is 2-11-4

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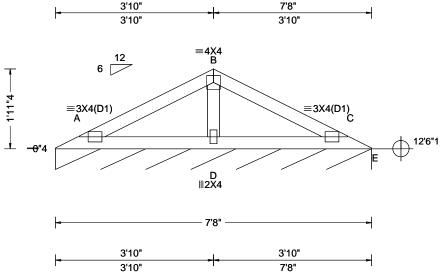
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155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

SEQN: 792294 VAL Ply: 1 Job Number: 24-2041 Cust: R 215 JRef: 1Y5X2150008 T13 FROM: CDM AMBER WILLIS Qty: 1 DrwNo: 354.24.1532.35203 Truss Label: V04 SSB / DF 12/19/2024



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF) Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-22	Pg: NA Ct: NA CAT: NA PP Deflection in loc L/defl L/#
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA VERT(LL): 0.006 C 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA VERT(CL): 0.013 C 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA HORZ(LL): -0.003 C
Des Ld: 40.00	EXP: C Kzt: NA	HORZ(TL): 0.005 C
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code: Creep Factor: 2.0
Soffit: 2.00	BCDL: 5.0 psf	FBC 8th Ed. 2023 Res. Max TC CSI: 0.169
Load Duration: 1.25	MWFRS Parallel Dist: h/2 to h	TPI Std: 2014 Max BC CSI: 0.154
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes Max Web CSI: 0.076
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)
	GCpi: 0.18	Plate Type(s):
	Wind Duration: 1.60	WAVE VIEW Ver: 23.02.04.0123.14
Lumber		

▲ Maximum Reactions (lbs), or \*=PLF Gravity Non-Gravity Loc R+ /R /Rh /Rw /U /RL E\* 82 /-/-/40 /10 Wind reactions based on MWFRS Brg Wid = 92.0 Min Req = Bearing A is a rigid surface. Members not listed have forces less than 375#

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

#### Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

#### **Additional Notes**

See DWGS VALTN220723 and VAL180220723 for valley details.

The overall height of this truss excluding overhang is 1-11-4



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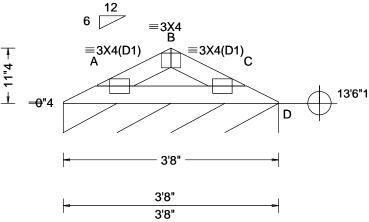
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SEQN: 792296 VAL Ply: 1 Job Number: 24-2041 Cust: R 215 JRef: 1Y5X2150008 T14 FROM: CDM AMBER WILLIS Qty: 1 DrwNo: 354.24.1532.33960 Truss Label: V05 SSB / DF 12/19/2024





Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg Pf in PSF)	DefI/CSI Criteria
Continue	Wind Criteria Wind Std: ASCE 7-22 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: FBC 8th Ed. 2023 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.003 C 999 240 VERT(CL): 0.006 C 999 180 HORZ(LL): -0.001 A HORZ(TL): 0.002 A Creep Factor: 2.0 Max TC CSI: 0.061 Max BC CSI: 0.087 Max Web CSI: 0.000
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#### ▲ Maximum Reactions (lbs), or \*=PLF Gravity Non-Gravity Loc R+ /R /Rh /Rw /U /RL D\* 82 /-/-Wind reactions based on MWFRS D Brg Wid = 44.0 Min Req = Bearing A is a rigid surface. Members not listed have forces less than 375#

#### Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2;

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

#### **Additional Notes**

See DWGS VALTN220723 and VAL180220723 for valley details.

The overall height of this truss excluding overhang is



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## Valley Detail - ASCE 7-22: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better. Bot Chord 2x4 SP #2N or SPF #1/#2 or better. Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

\*\* Attach each valley to every supporting truss with:
535# connection or with (1) Simpson H2.5A or
equivalent connector for

ASCE 7-22 180 mph. 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00

ASCE 7-22 160 mph. 30' Mean Height, Part. Enc. Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128"  $\times$  3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with: properly attached, rated sheathing applied prior to valley truss installation.

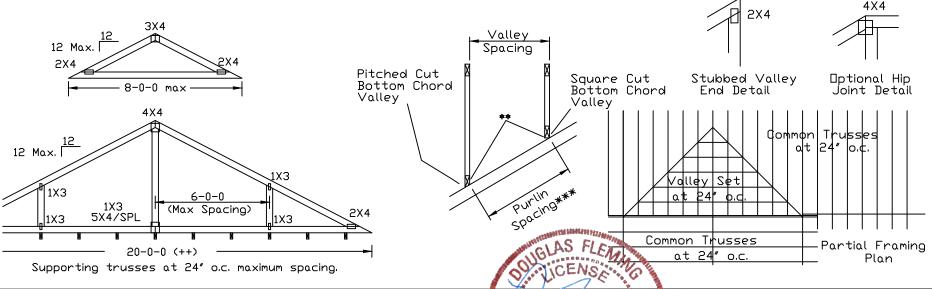
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Purlins at 24" o.c. or as otherwise specified on engineer's sealed design  $\Box r$ 

SPACING

By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

- \*\*\* Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.
- ++ Larger spans may be built as long as the vertical height does not exceed 14'-0''.





155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

## \*\*\*VARNING\*\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING \*\*\*IMPORTANT\*\*\* FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviation of any installation as the state of the state of

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TC	LL	30	30	40PSF	REF	VALLEY DETAIL
TC	DL	20	15	7PSF	DATE	07/03/2023
ВС	DL	10	10	10 PSF	DRWG	VAL180220723
ВC	LL	0	0	0 PSF		
TD.	Γ. LD.	60	55	57PSF		
					l	
DUR.FAC.1.25/1.33		1.15	1.15			

24.0"

## Valley Detail - ASCE 7-22: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better. Bot Chord 2x4 SP #2N or SPF #1/#2 or better. Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

\*\* Attach each valley to every supporting truss with: (2) 16d box (0.135" x 3.5") nails toe-nailed for ASCE 7-22, 30' Mean Height, Enclosed Building, Exp. C. Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on supporting truss material at connection location: 140 mph for SP (G = 0.55, min.), 125 mph for DF-L (G = 0.50, min.), or 105 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses below valley trusses.

Bottom chord of valley trusses may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.

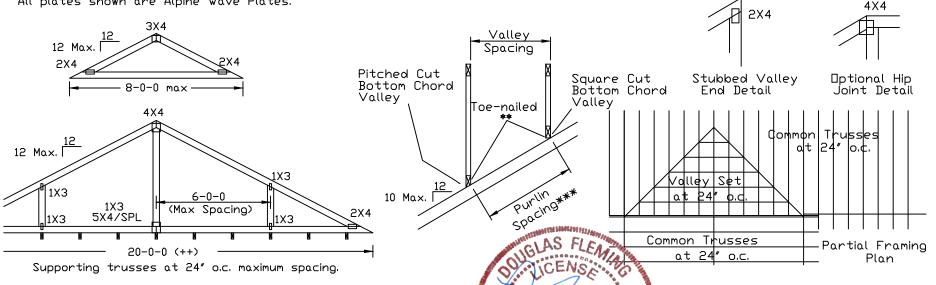
Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box  $(0.128" \times 3.0")$  nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with: properly attached, rated sheathing applied prior to valley truss installation.

Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

- \*\*\* Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.
- ++ Larger spans may be built as long as the vertical height does not exceed 14'-0".





155 Harlem Ave North Building, 4th Floor Glenview II 60025

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maintainement in Brawing TI ALL Contractors including the Install Rs.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. If fer to and follow the latest edition of BCSI (Bullaing Component Safety Information, by TPI and SBCA for sefety practices prior to performing these functions. Installers shall provide temporary bracin prop PSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and betton chor shall have a properly attached rigid celling. Locations shown for permanent lateral restrict int of else shall have bracing installed per BCSI sections B3, B7 or BiO, as applicable. Apply plates the earl fice of truss and position as shown above and on the Joint Betails, unless noted otherwise.

Refer to drawings IGOA-I for standard plate positions.

Alpine, a division of ITV Building Components Group Inc. shall not be responsible for any deviations of a division of ITV Building Components Group Inc. shall not be responsible for any deviation a bracing of trusses.

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No. 66648

TC	LL	30	30	40PSF	REF
TC	DL	20	15	7PSF	DAT
ВС	DL	10	10	10 PSF	DRW
ВC	LL	0	0	0 PSF	
TOT	Γ. LD.	60	55	57PSF	
DUR.FAC.1.25/1.33			1 15	1 15	

24.0"

SPACING

07/03/2023 SFIDATE SF DRWG VALTN220723 SF SF

VALLEY DETAIL

# CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

#### Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

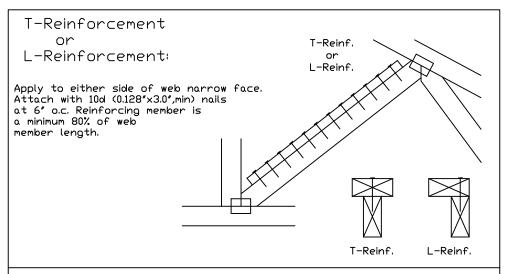
Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member	Specified CLR	Alternative Reir	
Size	Restraint	T- or L- Reinf.	
2x3 or 2x4	1 row	2×4	1-2×4
2x3 or 2x4	2 rows	2×6	2-2×4
2×6	1 row	2×4	1-2×6
2×6	2 rows	2×6	2-2×4( <b>*</b> )
2×8	1 row	2×6	1-2×8
	2 rows	2×6	2-2×6( <del>*/</del> )

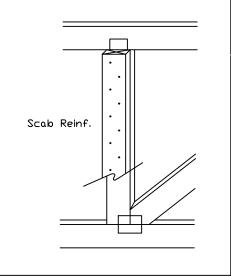
T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(\*) Center scab on wide face of web. Apply (1) scab to each face of web.



#### Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nalls at 6" o.c. Reinforcing member is a minimum 80% of web member length.



GLAS FLEM

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Refer to drawings 160A-Z for standard plate positions.

Albine. A division of ITV Building Components Group Inc. shall not be responsible for any deviation from

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shippin installation & bracing of trusses.

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STATE OF CONTROL OF CO

TC LL	PSF	REF	CLR Subst.
TC DL	PSF	DATE	01/02/19
BC DL	PSF	DRWG	BRCLBSUB0119
BC LL	PSF		
тот. ср.	PSF		
DUR. FAC.			
SPACING			

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