

JULIUS LEE PE.

RE: 536765 - REGAN ADDITION

**1109 COASTAL BAY BLVD,
BOYNTON BEACH, FL 33435**

Site Information:

Project Customer: Adams Const. Project Name 536765 Model: Regan Additions
Lot/Block: Subdivision:
Address: 615 Lona Loop
City: Columbia Cty State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: Unknown at time of seal License #: Unknown at time of seal
Address: Unknown at time of seal
City: Unknown at time of seal State: Unknown at time of seal

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

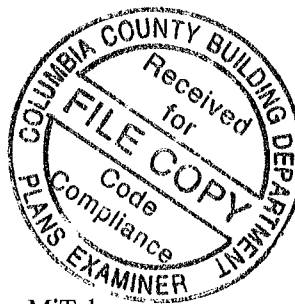
Design Code: FBC2010/TPI2007 Design Program: MiTek 20/20 7.3
Wind Code: ASCE 7-10 Wind Speed: 130 mph Floor Load: N/A psf
Roof Load: 32.0 psf

This package includes 21 individual, dated Truss Design Drawings and 0 Additional Drawings
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet
conforms to 61G15-31 003, section 5 of the Florida Board of Professional Engineers Rules.

This document processed per section 16G15-23 003 of the Florida Board of Professionals Rules

**In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany
this coversheet. The latest approval dates supersede and replace the previous drawings.**

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I7534587	CJ01	11/22/013	18	I7534604	T10	11/22/013
2	I7534588	CJ02	11/22/013	19	I7534605	T11	11/22/013
3	I7534589	CJ03	11/22/013	20	I7534606	T12	11/22/013
4	I7534590	CJ03T	11/22/013	21	I7534607	T13	11/22/013
5	I7534591	EJ01	11/22/013				
6	I7534592	EJ01T	11/22/013				
7	I7534593	HJ01	11/22/013				
8	I7534594	HJ01T	11/22/013				
9	I7534595	T01	11/22/013				
10	I7534596	T02	11/22/013				
11	I7534597	T03	11/22/013				
12	I7534598	T04	11/22/013				
13	I7534599	T05	11/22/013				
14	I7534600	T06	11/22/013				
15	I7534601	T07	11/22/013				
16	I7534602	T08	11/22/013				
17	I7534603	T09	11/22/013				

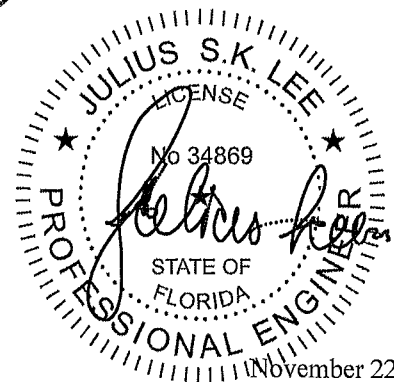


The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Jax).

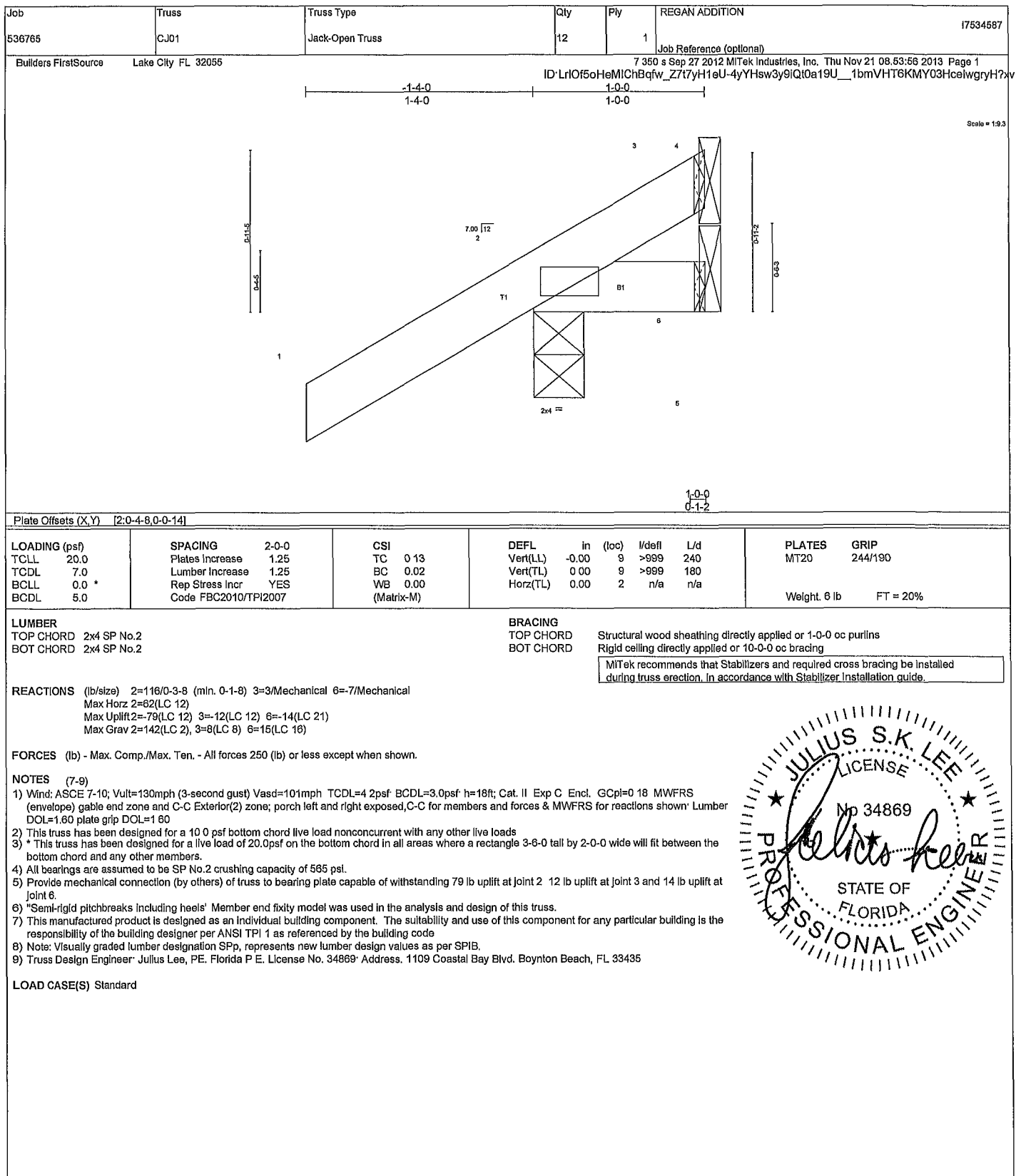
Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2015.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



November 22, 2013



November 22, 2013

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BC311 Building Component Safety Information, available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE.
1109 Coastal Bay
Boynton Beach, FL 33435

Job 536765	Truss CJ03	Truss Type Jack-Open Truss	Qty 8	Ply 1	REGAN ADDITION Job Reference (optional) ID LrOf5oHeMlChBqfw_Z7t7yH1eU-Y86f4F4aw0Ykejcm1hVG8_2Q?WcLHTIRr1TDHyH?x	17534589
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Builders FirstSource, Lake City FL 32055

7.360 s Sep 27 2012 MITek Industries, Inc. Thu Nov 21 08:53:57 2013 Page 1

Scale = 1:21.4

Plate Offsets (X,Y). [2-0-6-0-0-1-2]								
LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.27	Vert(LL) 0.07	4-7	>895	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.23	Vert(TL) 0.08	4-7	>999	180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00	2	n/a	n/a		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)						
						Weight: 19 lb FT = 20%		

LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2	BRACING TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing
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MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=81/Mechanical, 2=220/0-3-8 (min. 0-1-8) 4=24/Mechanical
 Max Horz 2=173(LC 12)
 Max Uplift 3=103(LC 12) 2=-114(LC 12), 4=-36(LC 9)
 Max Grav 3=111(LC 21) 2=264(LC 2) 4=57(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown
 TOP CHORD 2-3=-502/699
 BOT CHORD 2-4=-1169/891

NOTES (7-9)
 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II, Exp C, Encl. GCpi=0.18 MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 103 lb uplift at joint 3, 114 lb uplift at joint 2 and 36 lb uplift at joint 4.
 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
 9) Truss Design Engineer: Julius Lee PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard



November 22, 2013

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
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Julius Lee PE,
 1109 Coastal Bay
 Boynton Beach, FL 33435

Job 536765	Truss EJ01	Truss Type Jack-Partial Truss	Qty 5	Ply 1	REGAN ADDITION Job Reference (optional)	17534591
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Builders FirstSource, Lake City FL 32055

ID LrOf5oHeMICHBqfw_Z7i7yH1eU-VXEPVx6qSdoSt1mk96XkDP7hNKEFIMoklcWaHAyH?xs

7.350 s Sep 27 2012 MITek Industries, Inc. Thu Nov 21 08:53:59 2013 Page 1

Scale = 1:27.2

Plate Offsets (X,Y). [2:0-2-3,0-1-8]									
LOADING (psf)		SPACING		CSI		DEFL		PLATES GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	0.24	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.46	Vert(TL)	0.20		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.01		
BCDL	5.0	Code	FBC2010/TP12007	(Matrix-M)				Weight: 25 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS (lb/size) 3=115/Mechanical 2=287/0-3-8 (min 0-1 8) 4=32/Mechanical
Max Horz 2=158(LC 12)
Max Uplift 3=95(LC 12) 2=125(LC 9) 4=48(LC 9)
Max Grav 3=152(LC 21) 2=342(LC 2) 4=80(LC 3)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1048/1511
BOT CHORD 2-4=-2465/1855

NOTES (7-9)
1) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C; Encl GCpi=0.18 MWFRS (envelope) and C-C Exterior(2) zone porch left and right exposed C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 95 lb uplift at Joint 3, 125 lb uplift at Joint 2 and 48 lb uplift at joint 4.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
8) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
9) Truss Design Engineer: Julius Lee, PE, Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435

LOAD CASE(S) Standard

November 22, 2013

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Julius Lee PE,
1109 Coastal Bay
Boynton Beach, FL 33435

Job 538785	Truss HJ01	Truss Type Diagonal Hip Girder	Qty 4	Ply 1	REGAN ADDITION Job Reference (optional)	I7534593
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Builders FirstSource Lake City FL 32055
7,350 s Sep 27 2012 MITek Industries, Inc. Thu Nov 21 08:54:01 2013 Page 1

ID LrOf5oHeMlChBqfw_Z7t7yH1eU-RwM9wd74zE296Lv7GXZCqC2G7x?DCL0lw?hM2yH7xq

Scale = 1/28.8

Plate Offsets (X,Y). [2:0-6-4,Edge]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d
TCLL 20.0	Plates Increase	1.25	TC 0.58	Vert(LL)	0.08 6-7	>999	240
TCDL 7.0	Lumber Increase	1.25	BC 0.44	Vert(TL)	-0.10 6-7	>999	180
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.32	Horz(TL)	-0.01 5	n/a	n/a
BCDL 5.0	Code FBC2010/TPI2007		(Matrix-M)				
				PLATES		GRIP	
				MT20		244/190	
				Weight. 44 lb		FT = 20%	

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS (lb/size) 4=121/Mechanical 2=302/0-4-15 (min. 0-1-8), 5=206/Mechanical

Max Horz 2=230(LC 8)

Max Uplift 4=-147(LC 8) 2=-282(LC 4) 5=-245(LC 8)

Max Grav 4=148(LC 2) 2=380(LC 2), 5=239(LC 2)

FORCES (lb) - Max. Comp./Max. Ten - All forces 250 (lb) or less except when shown.

TOP CHORD 2-11=-557/453, 11-12=-529/472 3-12=-527/459

BOT CHORD 2-14=-584/526, 14-15=-584/526 7 15=-584/526 7 16=-584/526, 6-16=-584/526

WEBS 3-8=-568/631

NOTES (9-11)

- 1) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=18ft, Cat. II Exp C, Encl , GCpl=0.18, MWFRS (envelope) gable end zone Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 147 lb uplift at joint 4, 282 lb uplift at joint 2 and 245 lb uplift at joint 5.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 28 lb down and 51 lb up at 1-5-12, 28 lb down and 51 lb up at 1-5-12, 4 lb down and 59 lb up at 4-3-11 4 lb down and 59 lb up at 4-3-11 and 45 lb down and 103 lb up at 7-1-10, and 45 lb down and 103 lb up at 7-1-10 on top chord and 19 lb up at 1-5-12 19 lb up at 1-5-12, 5 lb down and 32 lb up at 4-3-11 5 lb down and 32 lb up at 4-3-11 and 27 lb down and 46 lb up at 7-1-10, and 27 lb down and 46 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
- 10) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
- 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869 Address 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-44, 5-8=-10

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins

BOT CHORD Rigid ceiling directly applied or 7-3-15 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



Continued on page 2

November 22, 2013

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Julius Lee PE
1109 Coastal Bay
Boynton Beach, FL 33435

Job 536765	Truss HJ01T	Truss Type Diagonal Hip Girder	Qty 2	Ply 1	REGAN ADDITION	17534594
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Builders FirstSource, Lake City FL 32055

Job Reference (optional)
7 350 s Sep 27 2012 MITek Industries, Inc. Thu Nov 21 08:54:03 2013 Page 1

ID: LrIOf5oHeMICHBqfw_Z717yH1eU-NITwLJ9LVsJtMf3WOycgNFHQNbfn3fJDEUnQxyH7xo

1-10-10 4-7-14 4-11-0 9-10-13
1-10-10 4-7-14 0-3-2 4-11-13

Scale = 1/26.6

Plate Offsets (X,Y): [3:0-4.6,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.41	Vert(LL)	0.10	7-8	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.56	Vert(TL)	-0.12	7-8	>954	180		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.46	Horz(TL)	-0.03	6	n/a	n/a		
BCDL 5.0	Code FBC2010/TPI2007		(Matrix-M)						Weight: 50 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2 *Except*
B3: 2x4 SP No.3
WEBS 2x4 SP No.3

BRACING
TOP CHORD
BOT CHORD
Structural wood sheathing directly applied or 5-9-8 oc purlins.
Rigid ceiling directly applied or 6-1-3 oc bracing.
MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=103/Mechanical, 2=315/0-4-15 (min. 0-1-8) 6=216/Mechanical
Max Horz 2=230(LC 8)
Max Uplift 5=-121(LC 8) 2=-268(LC 4) 6=-199(LC 8)
Max Grav 5=126(LC 2) 2=375(LC 2) 6=251(LC 2)

FORCES (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
TOP CHORD 2-14=-387/303, 3-14=-395/274, 3-15=-1003/798 4-15=-959/786
BOT CHORD 2-17=-418/389 10-17=-374/328, 3-9=-824/892, 6-9=-880/927 6-18=-880/927 7-18=-880/927
WEBS 4-7=-951/902, 4-8=-230/361 3-10=-323/351

NOTES (9-11)
1) Wind: ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph TCCL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C Encl GCpi=0.18 MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 121 lb uplift at joint 5, 268 lb uplift at joint 2 and 199 lb uplift at joint 6.
6) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 28 lb down and 51 lb up at 1-5-12, 28 lb down and 51 lb up at 1-5-12, 4 lb down and 59 lb up at 4-3-11, 4 lb down and 59 lb up at 4-3-11 and 18 lb down and 71 lb up at 7-1-10 and 18 lb down and 71 lb up at 7-1-10 on top chord, and 19 lb up at 1-5-12, 19 lb up at 1-5-12, 5 lb down and 32 lb up at 4-6-2, 5 lb down and 32 lb up at 4-6-2, and 45 lb down and 36 lb up at 7-1-10, and 45 lb down and 36 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B)
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code
10) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular Lumber Increase=1.25 Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-44 10-11=-10 6-9=-10



Continued on page 2

November 22, 2013

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Julius Lee PE
1109 Coastal Bay
Boynton Beach, FL 33435

Job 538765	Truss T01	Truss Type Hip Truss	Qty 1	Ply 1	REGAN ADDITION	I7534595																																													
Builders FirstSource Lake City FL 32055		7.350 s Sep 27 2012 MITek Industries, Inc. Thu Nov 21 08:54:05 2013 Page 1 ID LrOf5oHeMICHBqfw_Z7I7yH1eU-Jhbgl?Ab1T2bbyDuVNe8TgNh7lBe9swcgYzuVpyH7xm																																																	
<p>Scale = 1:38.8</p> <p>Plate Offsets (X,Y): [2-0-2-3-0-1-8], [10-0-2-3-0-1-8]</p>																																																			
<table border="1" style="width:100%; border-collapse: collapse;"> <tr> <td style="width:15%;">LOADING (psf)</td> <td style="width:15%;">SPACING</td> <td style="width:10%;">CSI</td> <td style="width:10%;">DEFL</td> <td style="width:10%;">In (loc)</td> <td style="width:10%;">I/defl</td> <td style="width:10%;">L/d</td> <td style="width:10%;">PLATES</td> <td style="width:10%;">GRIP</td> </tr> <tr> <td>TCLL 20.0</td> <td>Plates Increase 2-0-0</td> <td>TC 0.71</td> <td>Vert(LL) 0.18</td> <td>15</td> <td>>999</td> <td>240</td> <td>MT20</td> <td>244/190</td> </tr> <tr> <td>TCDL 7.0</td> <td>Lumber Increase 1.25</td> <td>BC 0.84</td> <td>Vert(TL) -0.31</td> <td>15</td> <td>>784</td> <td>180</td> <td></td> <td></td> </tr> <tr> <td>BCLL 0.0 *</td> <td>Rep Strass Incr NO</td> <td>WB 0.92</td> <td>Horz(TL) 0.24</td> <td>10</td> <td>n/a</td> <td>n/a</td> <td></td> <td></td> </tr> <tr> <td>BCDL 5.0</td> <td>Code FBC2010/TPI2007</td> <td>(Matrix-M)</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> </tr> </table> <p style="text-align: right;">Weight: 117 lb FT = 20%</p>							LOADING (psf)	SPACING	CSI	DEFL	In (loc)	I/defl	L/d	PLATES	GRIP	TCLL 20.0	Plates Increase 2-0-0	TC 0.71	Vert(LL) 0.18	15	>999	240	MT20	244/190	TCDL 7.0	Lumber Increase 1.25	BC 0.84	Vert(TL) -0.31	15	>784	180			BCLL 0.0 *	Rep Strass Incr NO	WB 0.92	Horz(TL) 0.24	10	n/a	n/a			BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)						
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REACTIONS (lb/size) 2=1008/0-3-8 (min 0-1-8), 10=1008/0-3-8 (min 0-1-8) Max Horz 2=104(LC 5) Max Uplift 2=538(LC 8) 10=538(LC 9) Max Grav 2=1191(LC 2) 10=1191(LC 2)																																																			
FORCES (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1753/830, 3-4=-3052/1474, 4-5=-2137/1080, 5-25=-1874/995, 6-25=-1874/979, 7-26=-1874/979, 7-8=-2137/1063, 8-9=-3052/1426, 9-10=-1753/792 BOT CHORD 2-18=-1201/2472, 17-18=-298/634, 4-17=-214/506, 18-17=-1391/2872, 16-27=-959/2055, 15-27=-959/2055, 15-28=-959/2055, 14-28=-959/2055, 13-14=-1284/2872, 12-13=-267/634, 8-13=-192/506, 10-12=-1105/2472 WEBS 3-18=-1088/528, 3-17=-1185/2415, 4-16=-1084/538, 5-16=-392/785, 8-16=-337/197, 6-14=-337/194, 7-14=-399/785, 8-14=-1084/506, 9-13=-1071/2415, 9-12=-1088/481																																																			
NOTES (11 13) 1) Unbalanced roof live loads have been considered for this design 2) Wind: ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C, Encl GCpl=0.18, MWFRS (envelope) Lumber DOL=1.60 plate grip DOL=1.60 3) Provide adequate drainage to prevent water ponding. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members 6) All bearings are assumed to be SP No.2 crushing capacity of 585 psi 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 538 lb uplift at joint 2 and 538 lb uplift at joint 10. 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 57 lb down and 79 lb up at 7-0-0, 57 lb down and 79 lb up at 9-0-12, 57 lb down and 79 lb up at 10-3-0, and 57 lb down and 79 lb up at 11-5-4, and 57 lb down and 79 lb up at 13-6-0 on top chord and 241 lb down and 209 lb up at 7-0-0, 41 lb down and 11 lb up at 7-0-12, 41 lb down and 11 lb up at 9-0-12, 41 lb down and 11 lb up at 10-3-0, 41 lb down and 11 lb up at 11-5-4, and 41 lb down and 11 lb up at 13-5-4, and 241 lb down and 209 lb up at 13-6-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B) 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code 12) Note: Visually graded lumber designation SPP, represents new lumber design values as per SPIB. 13) Truss Design Engineer: Julius Lee PE: Florida P.E. License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435																																																			
LOAD CASE(S) Standard																																																			



November 22, 2013

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE,
 1109 Coastal Bay
 Boynton Beach, FL 33435

Job 538785	Truss T02	Truss Type Hip Truss	Qty 1	Ply 1	REGAN ADDITION	17534596
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Builders FirstSource, Lake City FL 32055

7.350 s Sep 27 2012 MiTek Industries, Inc. Thu Nov 21 08:54:06 2013 Page 1
ID LrOf5oHeMICHbQfw_Z7i7yH1eU-ot92zKBDonhSD6o4349N?twp8cSujAmvCjS1GyH7x

Scale = 1/32"

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0	TC 0.46	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.55	Vert(LL) 0 11 12-13 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.92	Vert(TL) -0 21 12-13 >999 180		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)	Horz(TL) 0.15 9 n/a n/a		
				Weight: 118 lb	FT = 20%

LUMBER
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2 *Except*
 B2: 2x4 SP No.3
 WEBS 2x4 SP No.3

BRACING
 TOP CHORD
 BOT CHORD

Structural wood sheathing directly applied or 3-11-1 oc purlins.
 Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
 6-10-14 oc bracing: 14-15
 6-10-2 oc bracing: 12-13

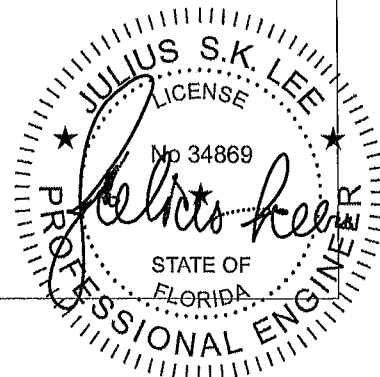
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection. In accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=612/0-3-8 (min. 0-1-8) 9=612/0-3-8 (min. 0-1-8)
 Max Horz 2=-151(LC 10)
 Max Uplift 2=-178(LC 12) 9=-178(LC 13)
 Max Grav 2=728(LC 2) 9=728(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-1465/487 3-4=-2045/766, 4-5=-1078/435, 5-6=-928/440 6-7=-1082/437 7 8=-2065/779, 8-9=-1488/503
 BOT CHORD 2-16=-680/2083, 15-16=-82/366 4-15=-33/302 14-15=-733/2072 13-14=-168/794, 12-13=-746/2092, 11-12=-92/382 7-12=-43/315 9-11=-719/2143
 WEBS 3-16=-638/170, 3-15=-639/1637 4-14=-1303/575, 7-13=-1320/586, 8-12=-546/1648 8-11=-658/183

NOTES (10-12)
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=18ft; Cat. II, Exp C, Encl , GCpl=0 18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown Lumber DOL=1.60 plate grip DOL=1.60
 3) Provide adequate drainage to prevent water ponding.
 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 178 lb uplift at joint 2 and 178 lb uplift at joint 9.
 8) Following joints to be plated by qualified designer: Joint(s) 9 not plated
 9) Semi-rigid pitchbreaks including heels. Member end fixity model was used in the analysis and design of this truss.
 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 11) Note: Visually graded lumber designation SP, represents new lumber design values as per SPIB.
 12) Truss Design Engineer: Julius Lee, PE: Florida P E License No 34869 Address 1109 Coastal Bay Blvd Boynton Beach, FL 33435

LOAD CASE(S) Standard



November 22, 2013

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSS-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE,
 1109 Coastal Bay
 Boynton Beach, FL 33435

Job 536765	Truss T04	Truss Type Common Truss	Qty 3	Ply 1	REGAN ADDITION	17534598
Builders FirstSource, Lake City FL 32055		7,360 s Sep 27 2012 MITek Industries, Inc. Thu Nov 21 08:54:09 2013 Page 1				
		ID LrIOF5oHeMICHbQfw_Z7t7yH1eU-CSrBbME55i314aWkDj4dVWXUcMit5ssCbAx6ebyH7x				
Scale = 1:39.1						
Plate Offsets (X,Y): [2,0-2-3,0-1-8], [6,0-2-3,0-1-8]						

LOADING (psf) TCCL 20.0 TCDL 7.0 BCCL 0.0 * BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2010/TPI2007	CSI TC 0.30 BC 0.31 WB 0.16 (Matrix-M)	DEFL in (loc) l/defl L/d Vert(LL) -0.07 8-10 >999 240 Vert(TL) -0.11 8-10 >999 180 Horz(TL) 0.03 6 n/a n/a	PLATES MT20 GRIP 244/190 Weight: 100 lb FT = 20%
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LUMBER
 TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 4-10-6 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=645/0-3-8 (min. 0-1-8) 6=645/0-3-8 (min. 0-1-8)
 Max Horz 2=168(LC 11)
 Max Uplift 2=182(LC 12) 6=182(LC 13)
 Max Grav 2=728(LC 2), 6=728(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown
 TOP CHORD 2-3=-1364/510, 3-4=-1222/504, 4-5=-1222/504, 5-6=-1364/509
 BOT CHORD 2-10=-332/1112 9-10=-107/682 9-17=-107/682, 17 18=-107/682 6-18=-107/682, 6-8=-334/1115
 WEBS 4-8=-176/455, 5-8=-375/243, 4-10=-176/455, 3-10=-376/244

NOTES (8-10)

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C Encl GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members with BCDL = 5.0psf
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 182 lb uplift at joint 2 and 182 lb uplift at joint 6.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Note Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
- 10) Truss Design Engineer Julius Lee PE, Florida P.E. License No. 34869- Address, 1109 Coastal Bay Blvd, Boynton Beach, FL 33435

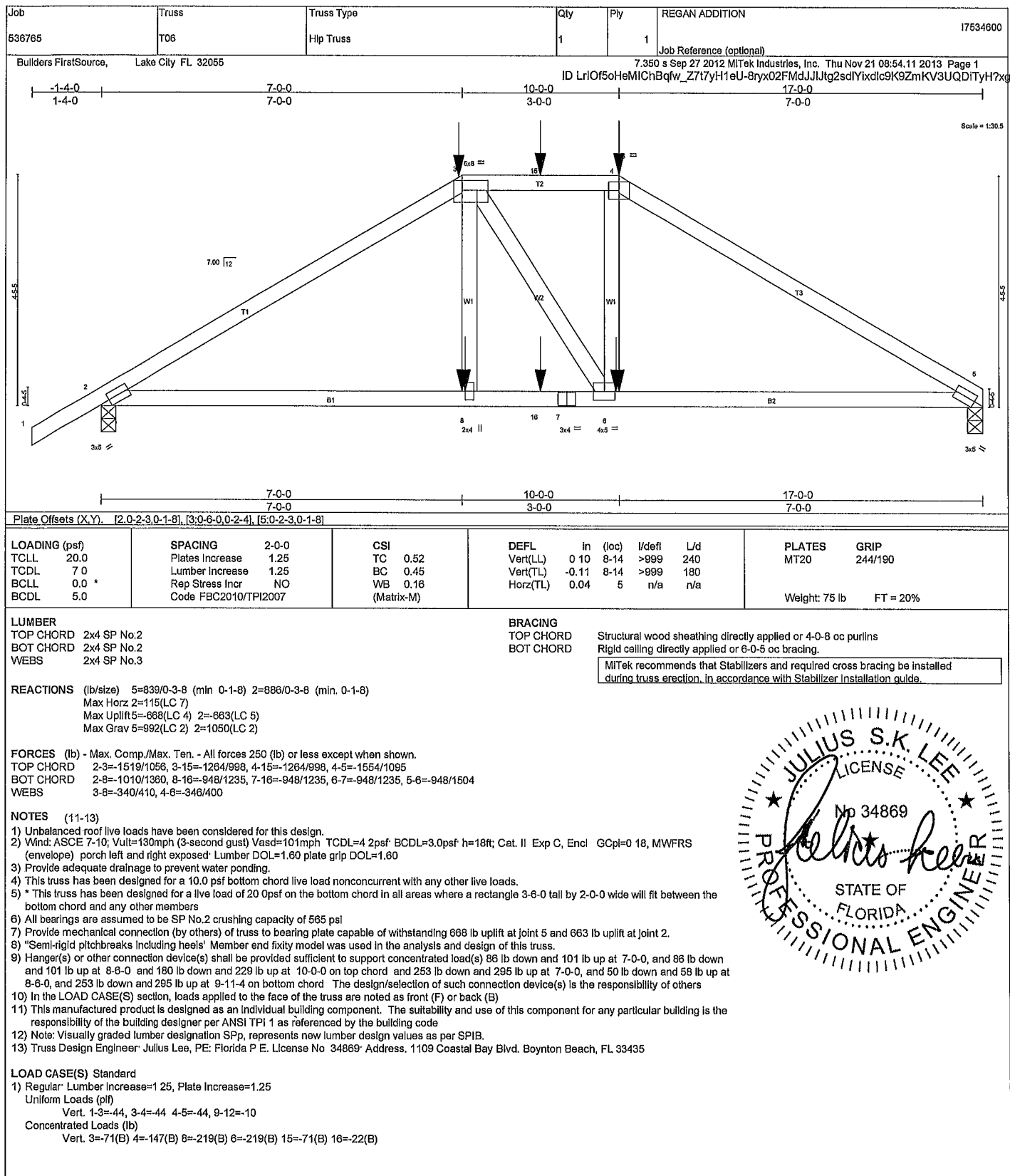
LOAD CASE(S) Standard



November 22, 2013

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719

Julius Lee PE
 1109 Coastal Bay
 Boynton Beach, FL 33435



November 22, 2013

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Julius Lee PE,
 1109 Coastal Bay
 Boynton Beach, FL 33435

Job 536765	Truss T08	Truss Type Special Truss	Qty 2	Ply 1	REGAN ADDITION Job Reference (optional)	17534602
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Buildlrs FirstSource Lake City FL 32055
7.350 s Sep 27 2012 MiTek Industries Inc. Thu Nov 21 08:54:14 2013 Page 1

ID LrIOf5oHeMICHbqfw_Z7t7yH1eU-ZQe4e4HEwEhJALPdXmIFKZF16NF0m15xISftJoyH7xd

Plate Offsets (X,Y): [2:0-2-3,0-1-8], [6:Edge,0-0-4]					
LOADING (psf)	SPACING	CSI	DEFL		PLATES GRIP
TCLL 20.0	Plates Increase 2-0-0 1.25	TC 0.44	in (loc) l/defl L/d		MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.93	Vert(LL) 0.12 8-9 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.54	Vert(TL) -0.22 9-10 >920 180		
BCDL 5.0	Code FBC2010/TPI2007	(Matrix-M)	Horz(TL) 0.20 6 n/a n/a		
					Weight: 83 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3

REACTIONS (lb/size) 6=457/Mechanical 2=520/0-3-8 (min. 0-1-8)

Max Horz 2=137(LC 11)

Max Uplift 6=131(LC 13), 2=-157(LC 12)

Max Grav 6=541(LC 2) 2=619(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1399/454 3-4=-863/334, 4-5=-862/336, 5-6=-1286/480

BOT CHORD 2-11=-670/2024, 9-10=-573/1551 8-9=-546/1479, 6-7=-710/1898

WEBS 3-9=-968/460, 4-9=-136/429, 5-9=-899/434

NOTES (8-11)

- 1) Unbalanced roof live loads have been considered for this design
- 2) Wind: ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II Exp C, Encl GCpi=0.18; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown, Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 131 lb uplift at joint 6 and 157 lb uplift at joint 2
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Note: Visually graded lumber designation SPp, represents new lumber design values as per SPIB.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P. E. License No. 34869 Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- 11) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

JULIUS S.K. LEE

LICENSE

No 34869

Julius S.K. Lee

STATE OF FLORIDA

PROFESSIONAL ENGINEER

November 22, 2013

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 erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding
 fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component
 Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

Julius Lee PE
 1109 Coastal Bay
 Boynton Beach, FL 33435

Job 536765	Truss T10	Truss Type Common Truss	Qty 1	Ply 1	REGAN ADDITION	17634604
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Builders FirstSource, Lake City FL 32055

7.350 s Sep 27 2012 MITek Industries, Inc. Thu Nov 21 08:54 16 2013 Page 1
ID LrOf5oHeMICHBqfw_Z7i7yH1eU-Vomq3lJURrx1QeZ7fBLJP_KelA2AE13ECI8zOhYH7x0

Scale = 1:34.9

Plate Offsets (X,Y). [2-0-2-3-0-1-8], [2-0-0-0-11-13], [6-0-2-3-0-1-8], [6-0-0-0-11-13]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.43	Vert(LL)	-0 07	8-12	>999	240	MT20	244/190
TCDL 7 0	Lumber Increase	1.25	BC 0.40	Vert(TL)	-0 15	8-12	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.19	Horz(TL)	0 02	6	n/a	n/a		
BCDL 5 0	Code FBC2010/TP12007		(Matrix-M)							

Weight: 91 lb FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
WEDGE
Left: 2x8 SP No.2, Right: 2x8 SP No.2

BRACING
TOP CHORD
BOT CHORD
Structural wood sheathing directly applied or 5-6-9 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=518/0-3-8 (min. 0-1-8) 6=571/0-3-8 (min. 0-1-8)
Max Horz 2=150(LC 10)
Max Uplift 2=158(LC 12) 6=171(LC 13)
Max Grav 2=617(LC 2) 6=679(LC 2)

FORCES (lb) Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-984/400, 3-4=-751/319, 4-5=-739/314, 5-6=-914/368, 6-16=-297/385
BOT CHORD 2-9=-260/997 8-9=-236/820, 6-8=-185/698, 6-17=-367/341
WEBS 3-8=-382/225, 4-8=-150/394

NOTES (8-10)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7 10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf BCDL=3.0psf h=18ft; Cat. II Exp C Encl. GCpi=0.18, MWFRS (envelope) and C-C Exterior(2) zone; cantilever right exposed C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
5) All bearings are assumed to be SP No.2 crushing capacity of 565 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 158 lb uplift at joint 2 and 171 lb uplift at joint 6.
7) 'Semi-rigid pitchbreaks including heels' Member end fixity model was used in the analysis and design of this truss
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
9) Note: Visually graded lumber designation SPp, represents new lumber design values as per SP1B.
10) Truss Design Engineer Julius Lee, PE: Florida P.E. License No. 34869 Address 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

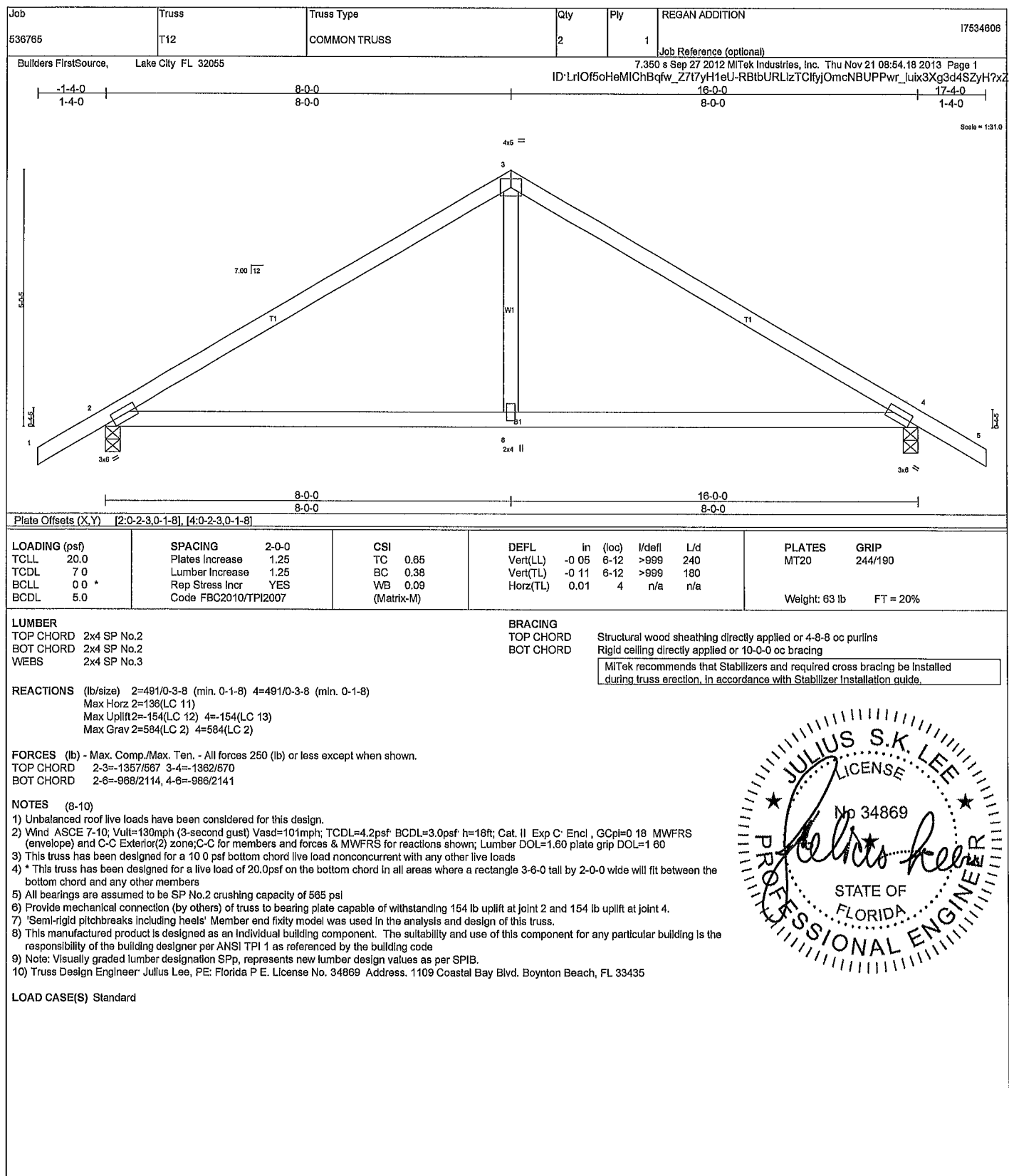
LOAD CASE(S) Standard



November 22, 2013

WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719

Julius Lee PE
1109 Coastal Bay
Boynton Beach, FL 33435



November 22, 2013

Job 536765	Truss T13	Truss Type Common Truss	Qty 1	Ply 2	REGAN ADDITION Job Reference (optional)	17534607
Builders FirstSource Lake City FL 32055		7.350 s Sep 27 2012 Mitek Industries, Inc. Thu Nov 21 08:54:20 2013 Page 2 ID LrIOf5oHeMICHBqfw_Z7l7yH1eU-Na7Lv7M7V4STuGsnuOPfZqVNxoN5AlXp7N6BXSyH7xX				
LOAD CASE(S) Standard 1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-44, 3-5=-44, 1-5=-10 Concentrated Loads (lb) Vert: 12=-432(F) 13=-448(F) 14=-448(F) 15=-447(F) 16=-447(F) 17=-432(F) 18=-432(F) 19=-432(F)						



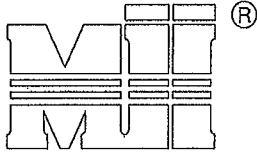
WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive Madison, WI 53719

Julius Lee PE
1109 Coastal Bay
Boynton Beach FL 33435

August 10, 2010

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

ST - T-BRACE 2



MiTek Industries, Inc.

MiTek Industries, Chesterfield, MO Page 1 of 1

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

Brace Size
for One-Ply Truss

Specified Continuous
Rows of Lateral Bracing

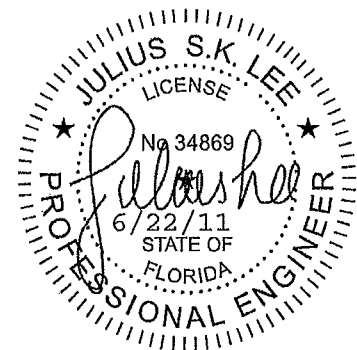
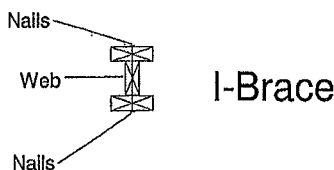
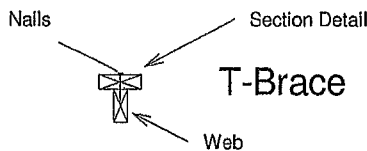
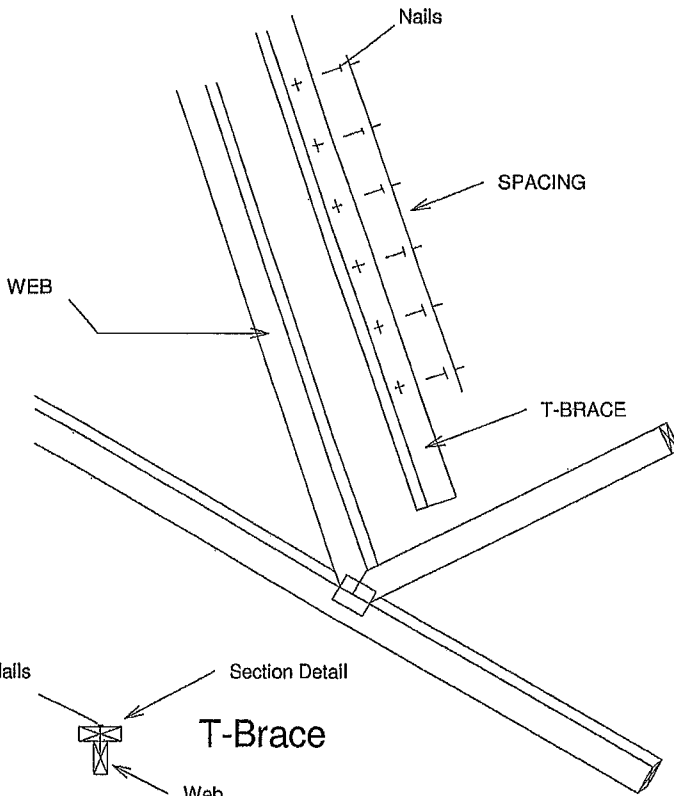
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

Brace Size
for Two-Ply Truss

Specified Continuous
Rows of Lateral Bracing

Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.

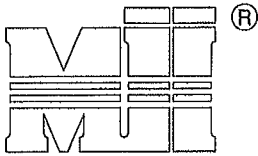


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BOYNTON BC, FL 33435

JANUARY 1, 2009

LATERAL TOE-NAIL DETAIL

ST-TOENAIL_SP



MITek Industries, Inc.

MITek Industries, Chesterfield, MO Page 1 of 1

NOTES

1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.)
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)

	DIAM.	SYP	DF	HF	SPF	SPF-S
3.5" LONG	.131	88.0	80.6	69.9	68.4	59.7
	.135	93.5	85.6	74.2	72.6	63.4
	.162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57.6	50.3
	.131	75.9	69.5	60.3	59.0	51.1
	.148	81.4	74.5	64.6	63.2	52.5

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
 APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED

EXAMPLE

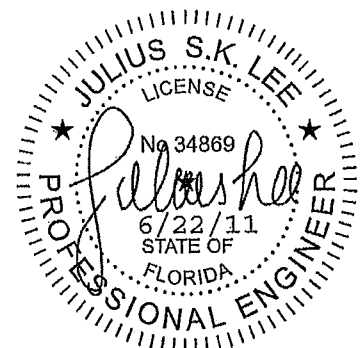
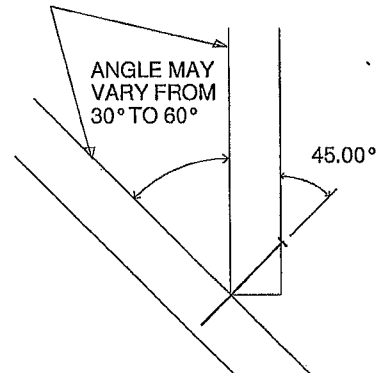
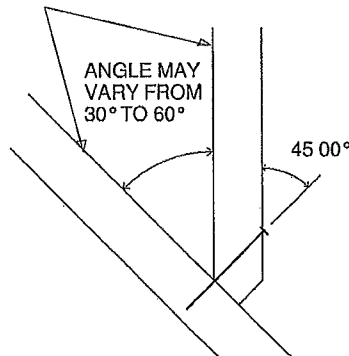
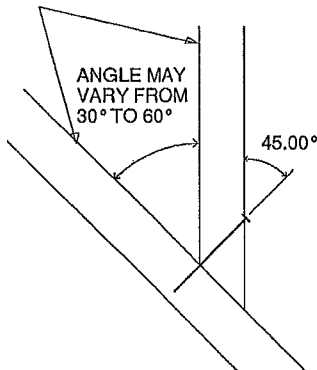
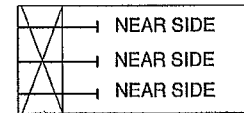
(3) - 16d NAILS (.162" diam. x 3.5") WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15

3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

SIDE VIEW

3 NAILS



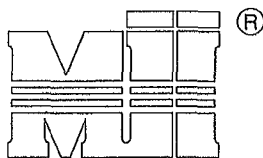
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 BOYNTON BC, FL 33435

FEBRUARY 14, 2012

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

ST-PIGGY-7-10

MITek Industries, Chesterfield, MO

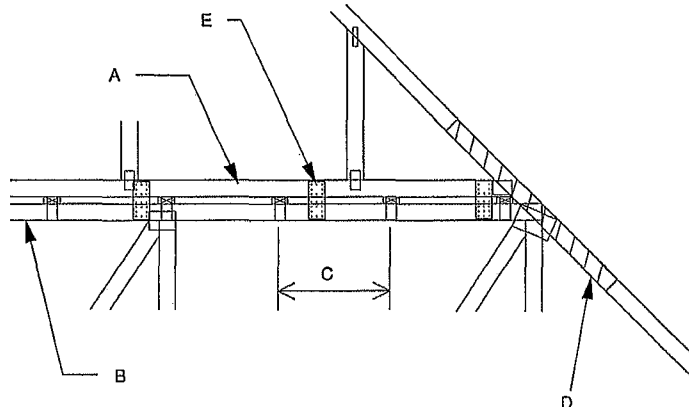


MITek Industries, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E
MAX MEAN ROOF HEIGHT = 30 FEET
MAX TRUSS SPACING = 24" O.C.
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-10
DURATION OF LOAD INCREASE 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES
TRANSFERING DRAG LOADS (SHEAR TRUSSES)
ADDITIONAL CONSIDERATIONS BY BUILDING
ENGINEER/DESIGNER ARE REQUIRED.

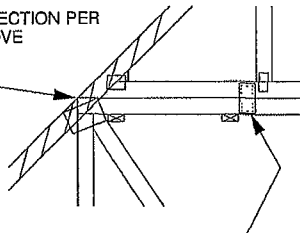
- A - PIGGYBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5" TOE NAILED
- B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0.131" X 3.5" NAILS EACH.
- D - 2 X 4-0" SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
2. WIND SPEED OF 116 MPH TO 160 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.
- E - FOR WIND SPEEDS BETWEEN 126 AND 160 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) 0.131" X 1.5" PER MEMBER STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



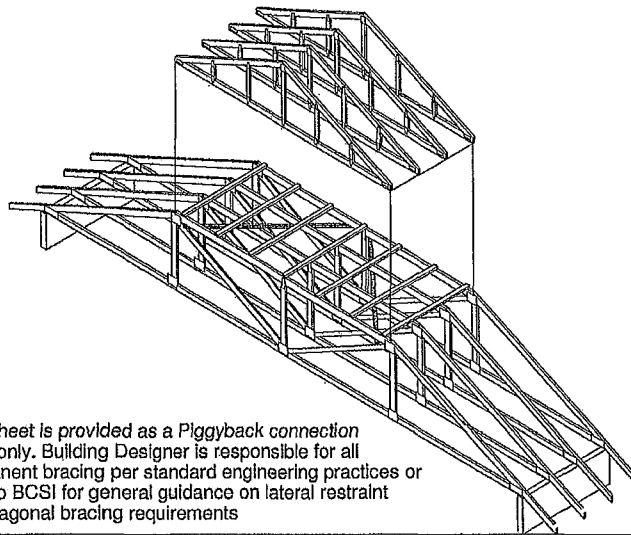
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS.

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING

SCAB CONNECTION PER
NOTE D ABOVE

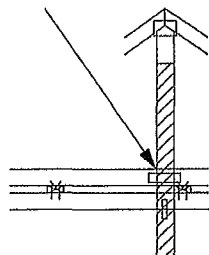


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) 0.131" X 1.5" PER MEMBER STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE



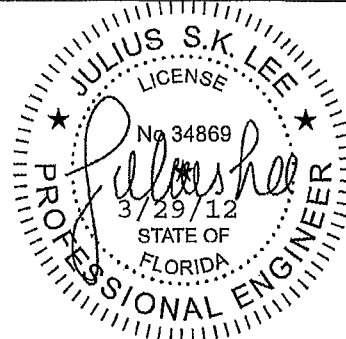
This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements

VERTICAL WEB TO
EXTEND THROUGH
BOTTOM CHORD
OF PIGGYBACK

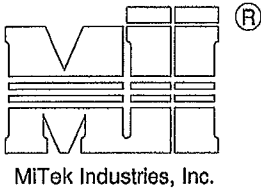


FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

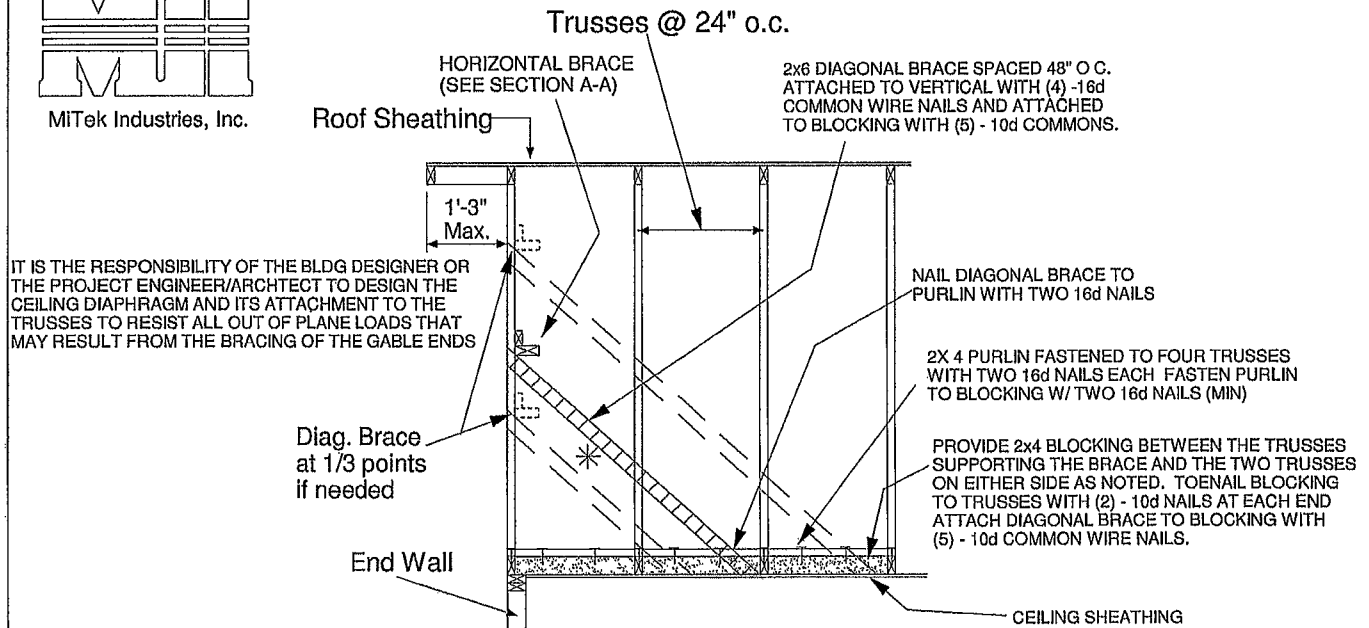
- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL
- 2) ATTACH 2 x 4-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS
- 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN



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ALTERNATE DIAGONAL BRACING TO THE BOTTOM CHORD



BRACING REQUIREMENTS FOR STRUCTURAL GABLE TRUSSES

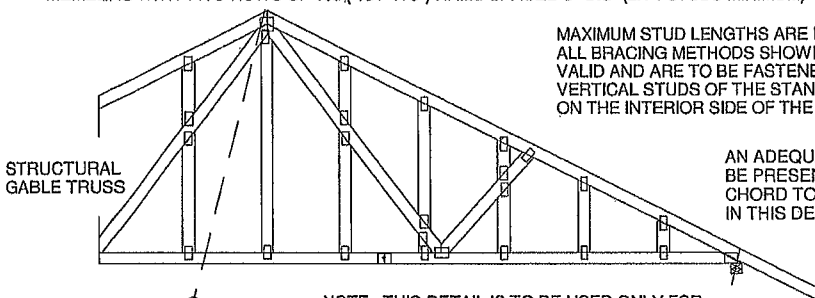
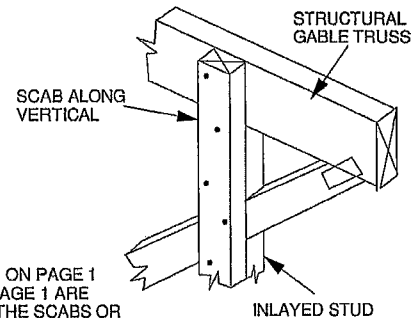
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED.

METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

METHOD 2: ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

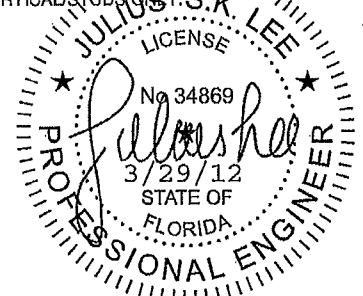
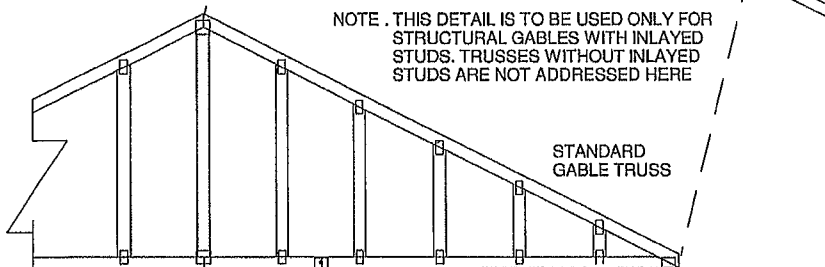
- FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (131" X 3") NAILS SPACED 6" O.C
- FOR WIND SPEEDS GREATER 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (131" X 3") NAILS SPACED 6" O.C (2X 4 STUDS MINIMUM)



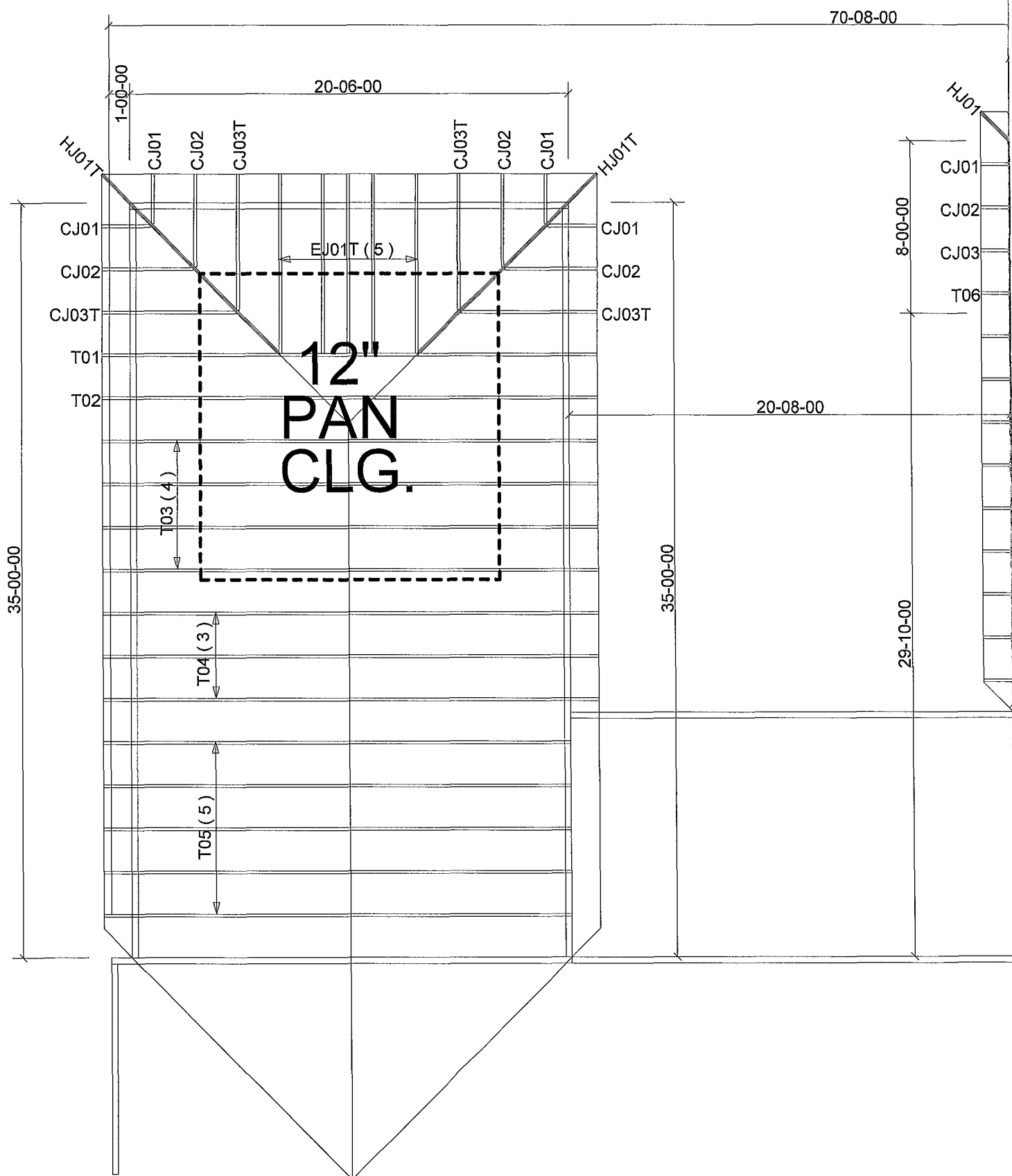
MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1
ALL BRACING METHODS SHOWN ON PAGE 1 ARE VALID AND ARE TO BE FASTENED TO THE SCABS OR VERTICAL STUDS OF THE STANDARD GABLE TRUSS ON THE INTERIOR SIDE OF THE STRUCTURE.

AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL STUDS ONLY.

NOTE: THIS DETAIL IS TO BE USED ONLY FOR STRUCTURAL GABLES WITH INLAVED STUDS. TRUSSES WITHOUT INLAVED STUDS ARE NOT ADDRESSED HERE



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MITEK PLATE APPROVAL #'s 2197.2 - 2197.4, WEYERH