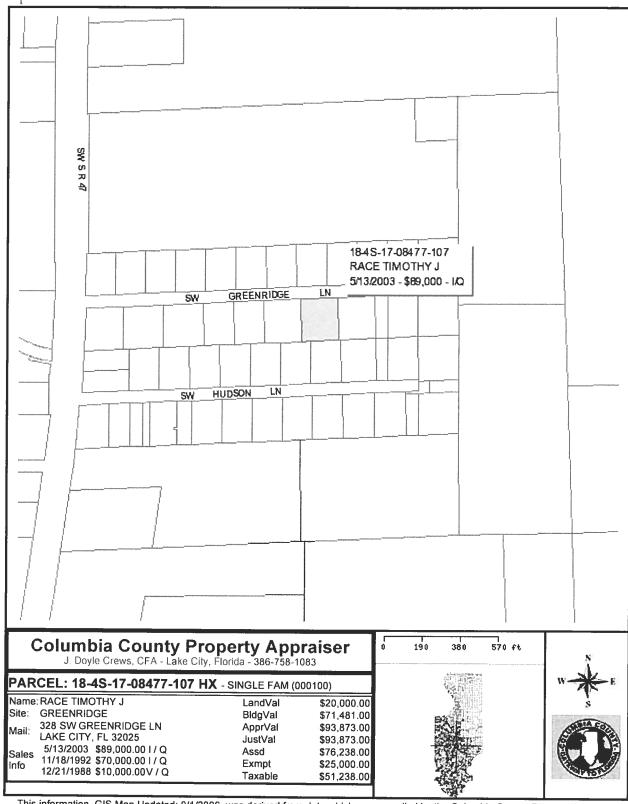
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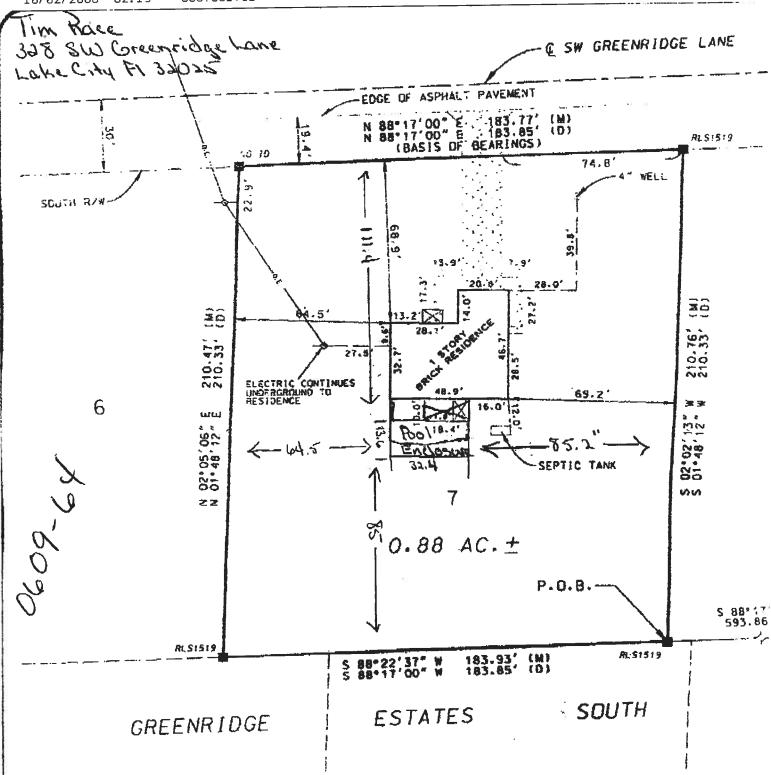
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Nadean G.S. McIntosh Commission # DD371494 Expires November 14, 2008 Bonded Troy Fain - Insurance, Inc. 800-385-7019

Notary Signature

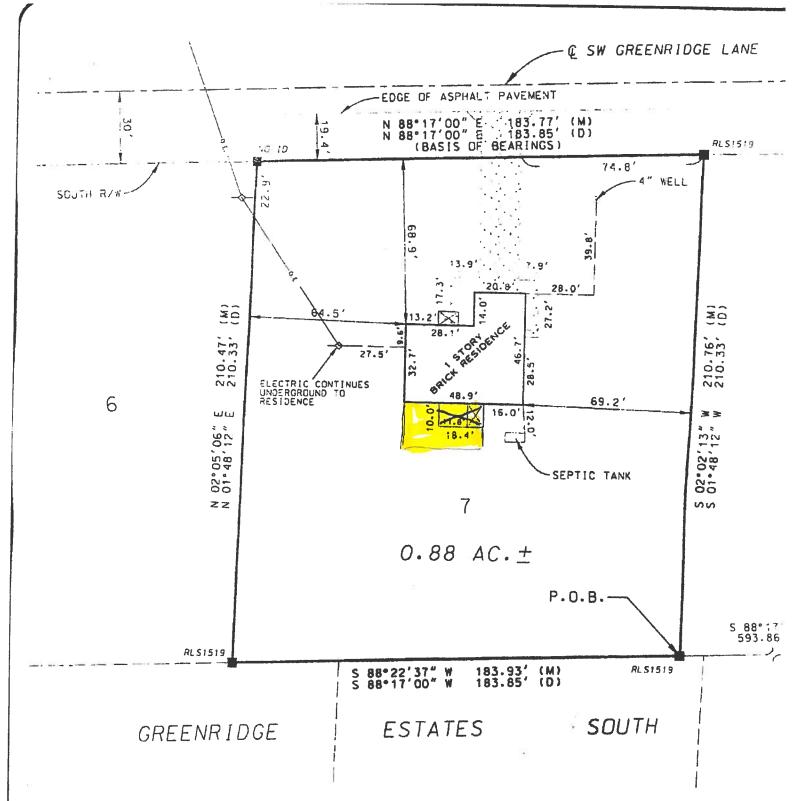


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NOTES:

- 1. BEARINGS ARE BASED ON THE SOUTH RIGHT OF WAY OF SW GREENRIDGE LANE. BEING N 88*17'00" E.
- 2. SUBJECT PROPERTY LIES IN FLOOD ZONE "X" (UN-SHADED), AN AREA OUTSIDE OF THE SOO-YEAR FLOOD PLAIN PER FLOOD INSLRANCE RATE MAP COMMUNITY PANEL NO. 120070 0175 B. LAST REVISION DATE JANUARY 6. 1988. FLOOD ZONE LINES, IF ANY, ARE SCALED FROM FLOOD INSURANCE RATE MAPS, PROVIDED BY FEMA.
- 3. ONLY THOSE VISIBLE INTERIOR IMPROVEMENTS AND IMPROVEMENTS PERTINENT TO THE SUBJECT PROPERTY HAVE BEEN LOCATED AS SHOWN HEREON, EXCEPTION IS MADE HEREON TO UNDERGROUND FACILITIES AND OTHER IMPROVEMENTS NOT VISIBLE OR KNOWN AT DATE OF SURVEY.
- 4. THIS SURVEY WAS PREPARED WITHOUT THE BENEFIT OF AN ABSTRACT OR TITLE POLICY. THEREFORE, EXCEPTION IS MADE HEREIN REGARDING EXSEMENTS. RESERVATIONS AND RESTRICTIONS OF RECORD NOT PROVIDED BY THE CLIE
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STATE OF FLORIDA

MENT OF BUSINESS AND PROFESSIONAL REGULATION
CONSTRUCTION INDUSTRY LICENSING BOARD

SEQ# L06070801467

LICENSE NBR BATCH NUMBER

07/08/2006 060011584 Scc056689:

The SPECIALTY STRUCTURE CONTRACTOR
Named below IS CERTIFIED
Under the provisions of Chapter 489 FS.

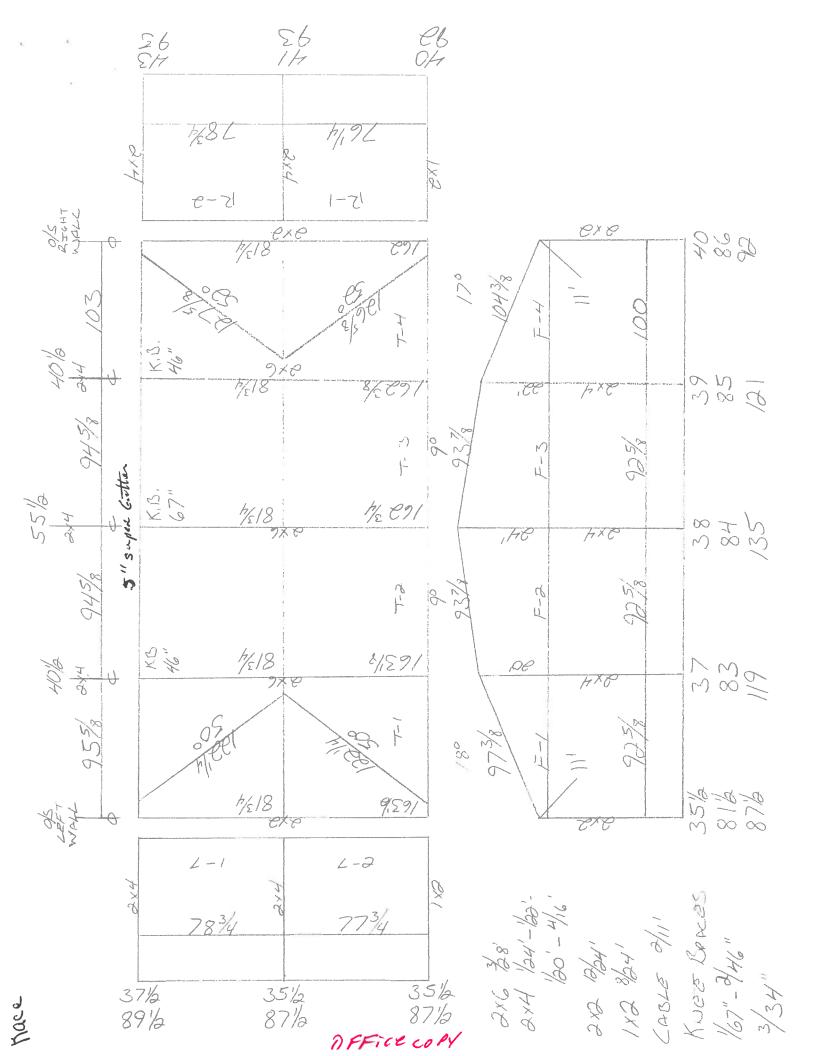
Under the provision 31, Expiration date: AUG 31, 2008

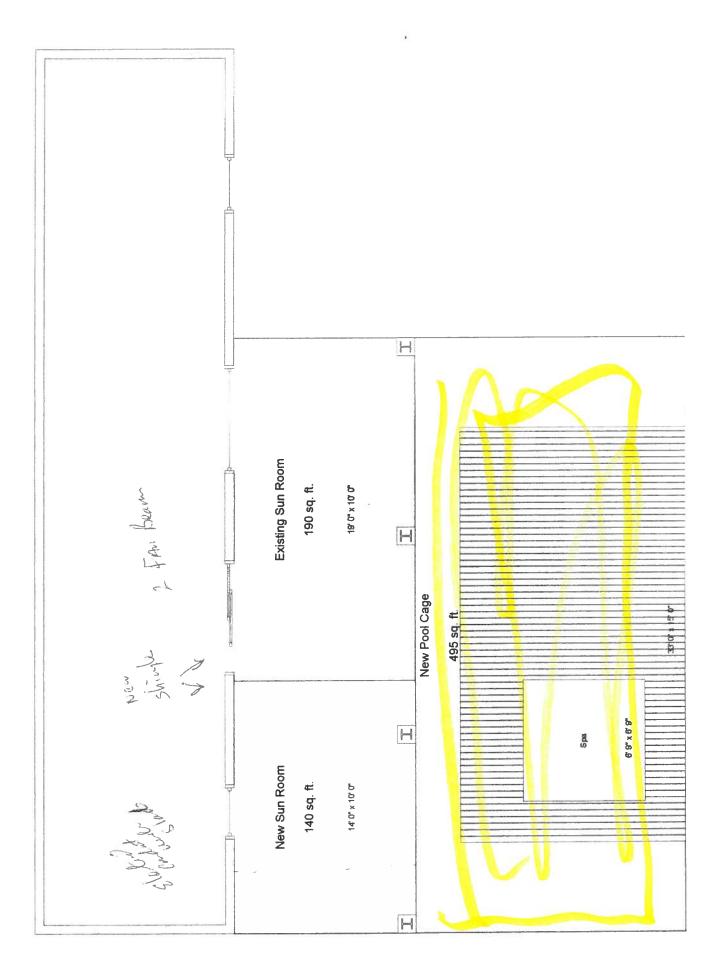
DELAHOZ, MICHAEL ANGEL FLORIDA POOL ENCLOSURES INC 927 HICKORY ST ALTAMONTE SPRINGS FL 32701

JEB BUSH (

DISPLAY AS REQUIRED BY LAW

SECRETARY





January 01, 2006

LAWRENCE E. BENNETT, P.E. P.O. BOX 214368 SOUTH DAYTONA, FL 32121 386-767-4774

TO ALL BUILDING DEPARTMENTS

Re: Master File Engineering
"ALUMINUM STRUCTURES DESIGN MANUAL"
2004 edition & 2006 edition

Dear Building Official/Plans Examiner,

This is to certify that the following contractor/company is hereby authorized to use my 2004 ed "ALUMINUM STRUCTURES DESIGN MANUAL" during the year 2006. When we publish and distribute the 2006 ed of the "ALUMINUM STRUCTURES DESIGN MANUAL", they will be authorized to use that manual for the remainder of 2006.

Our authorization is based on a January to January basis requardless of the edition of the manual. This authorization also applies to contractor master file drawings, " ONE PERMIT ONLY" drawings or any "site specific" drawings that I may furnish the contractor.

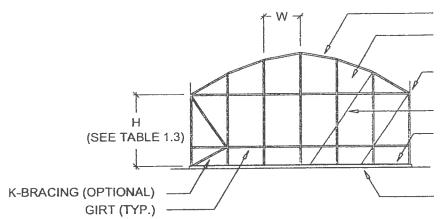
Mike Delahoz AAF Mid Fl Florida Pool Enclosures Inc P.O. Box 521136 Longwood, FL 32752

They are hereby added to my 2006 MASTERFILE LIST

Should you have any questions please contact me at your convenience.

Sincerely,

Lawrence E. Bennett, P.F. #16644



PURLINS (TYP.)

SCREEN (TYP.)

CABLE CONNECTION (SEE DETAILS SECTION 1)

ALTERNATE CABLE

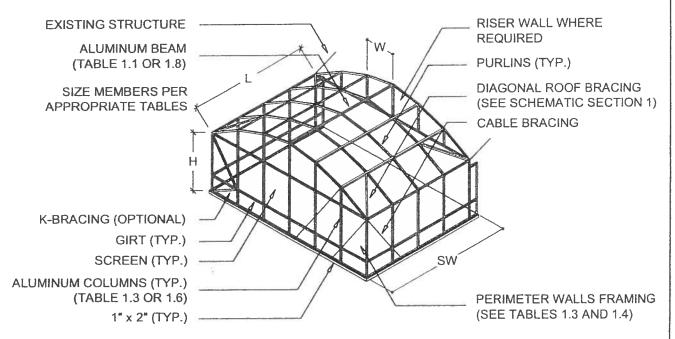
1" x 2" (TYP.)

GRADE

CABLE CONNECTION (SEE DETAILS SECTION 1)

TYPICAL DOME ROOF - ELEVATION

SCALE: N.T.S.



TYPICAL DOME ROOF - ISOMETRIC

SCALE: N.T.S.

CONNECTION DETAILS AND NOTES ARE FOUND IN THE SUBSEQUENT PAGES.

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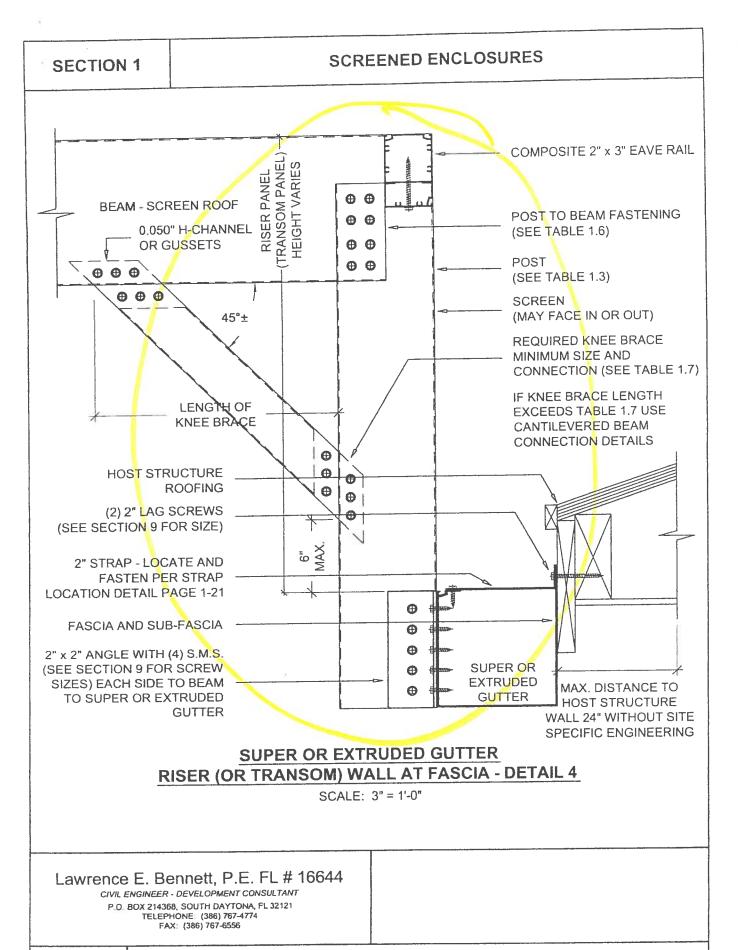
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PAGE

1-3

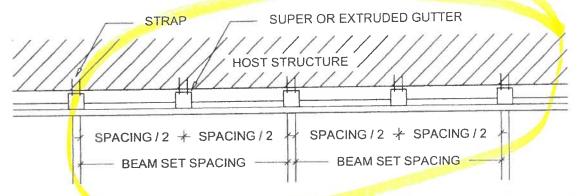


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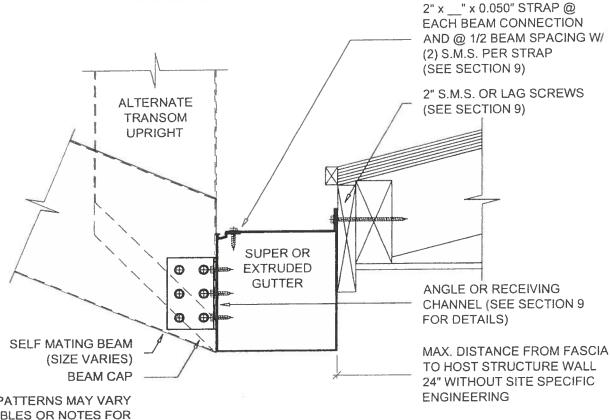
1-28

SECTION 1



STRAP LOCATION FOR SUPER OR EXTRUDED GUTTER REINFORCEMENT

SCALE: 3/8" = 1'-0"



SCREW PATTERNS MAY VARY (SEE TABLES OR NOTES FOR SIZE AND NUMBER OF SCREWS)

SELF MATING BEAM CONNECTION TO SUPER OR EXTRUDED GUTTER

SCALE: 3" = 1'-0"

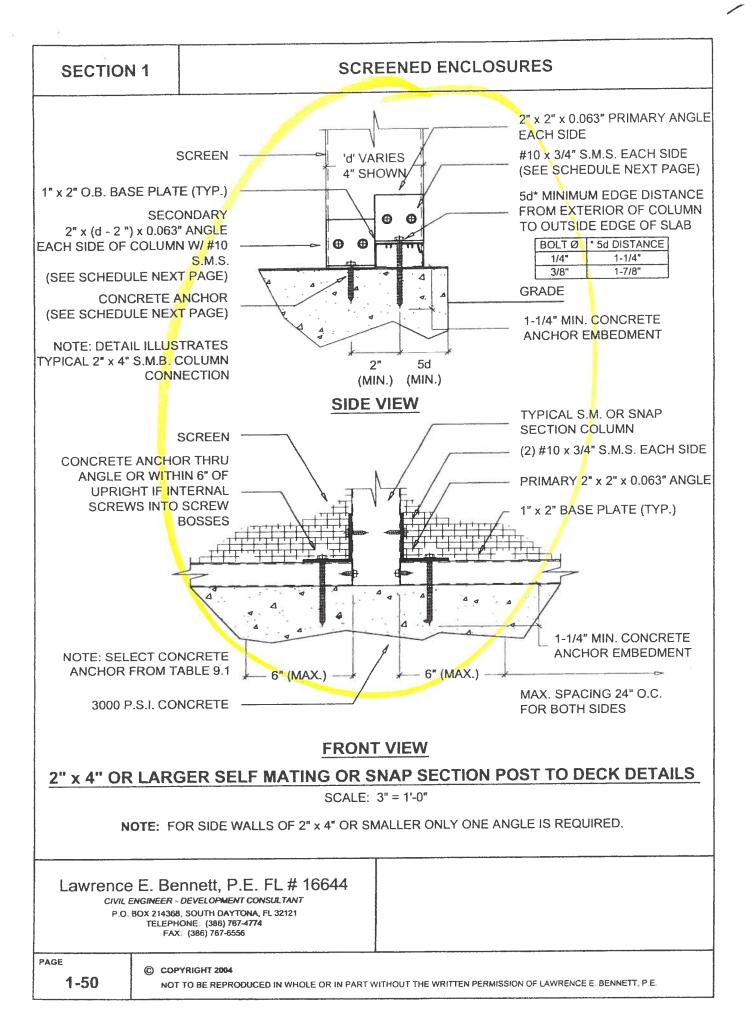
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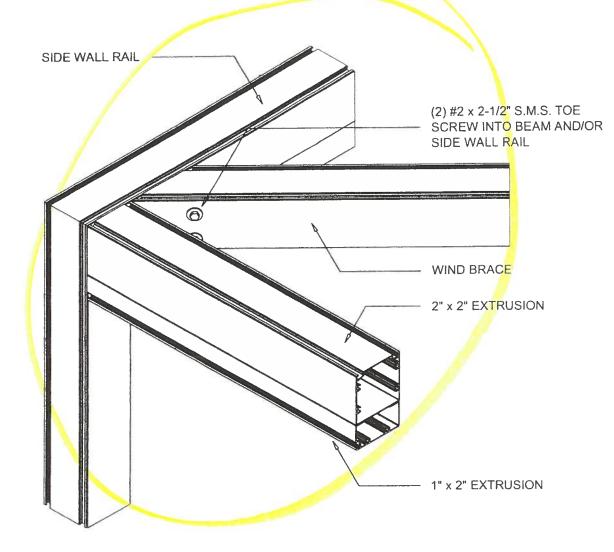
1-21

PAGE



SECTION 1

SCREENED ENCLOSURES



WIND BRACE CONNECTION DETAIL

SCALE: 3" = 1'-0"

NOTES:

1. Wind bracing shall be provided at each side wall panel when enclosure projects more than (4) panels from host structure.

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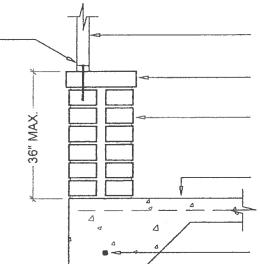
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1-36

SECTION 1

1/4" x 6" RAWL TAPPER THROUGH 1" x 2" AND ROWLOCK INTO FIRST COURSE OF BRICKS

ALTERNATE CONNECTION OF SCREENED ENCLOSURE FOR BRICK OR OTHER NON-STRUCTURAL KNEE WALL 1" WIDE x 0.063" THICK STRAP @ EACH POST FROM POST TO FOOTING W/ (2) #10 x 3/4" S.M.S. STRAP TO POST AND (1) 1/4" x 1-3/4" CONCRETE ANCHOR TO SLAB OR FOOTING



ALUMINUM FRAME SCREEN WALL

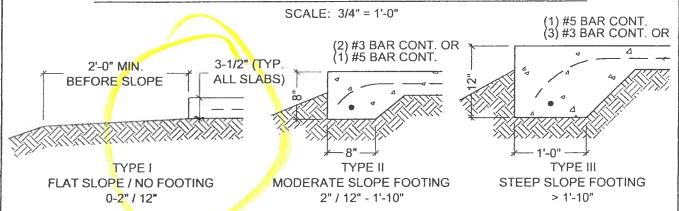
CAP BRICK

BRICK KNEEWALL TYPE 'S' MORTAR REQUIRED FOR LOAD BEARING BRICK WALL

4" (NOMINAL) PATIO CONCRETE SLAB (SEE NOTES CONCERNING FIBER MESH)

(3) #3Ø BARS OR (1) #5Ø BAR W/ 2-1/2" COVER (TYP.)

BRICK KNEEWALL AND FOUNDATION FOR SCREEN WALLS



Notes for all foundation types:

- 1. No footing required except when addressing erosion until the slab width in the direction of the primary exceeds 32 ft., then a Type II footing is required under the load bearing wall only unless the side wall exceeds 16 ft. in height or the enclosure is in a "C" exposure catagory in which case a Type II footing is required for all walls.
- 2. The foundations shown are based on a minimum soil bearing pressure of 1,500 PSF. Bearing capacity of soil shall be verified, using a pocket penetrometer, field soil test, or by a soil testing lab, to be above 1,500 PSF prior to placing the slab.
- 3. The slab / foundation shall be cleared of debris and roots and compacted prior to placement of concrete.
- 4. Monolithic slabs and footings shall be minimum 2,500 psi concrete with 6 x 6 10 x 10 welded wire mesh or crack control fiber mesh: Fibermesh ® Mesh, InForce™ e3™ (Formerly Fibermesh MD) per maufacturer's specification may be used in lieu of wire mesh.
- 5. If local building codes require a minimum footing, use Type II footing or footing sections required by local code. Local code governs.

SLAB-FOOTING DETAILS

SCALE: 3/4" = 1'-0"

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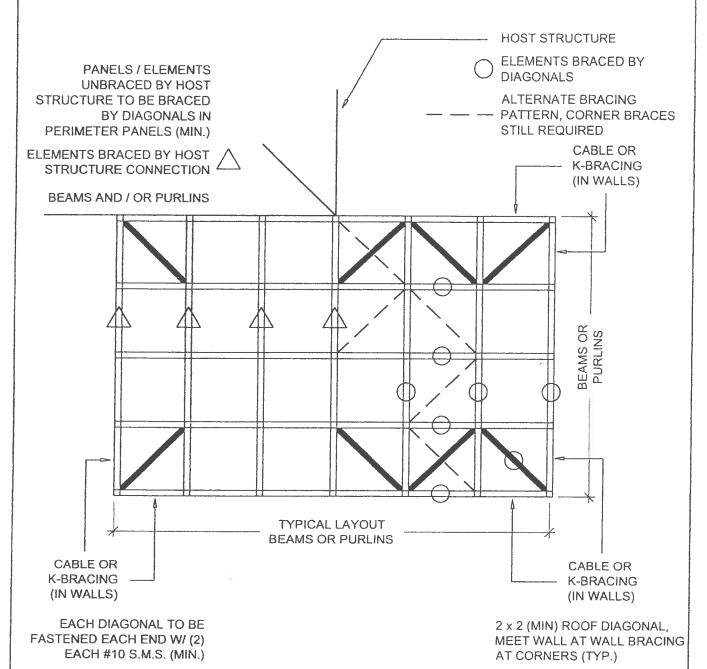
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1-55

SECTION 1



(POOL ENCLOSURE SCREEN ROOF MAY BE FLAT, GABLE, MANSARD, DOME, OR HIP)

POOL ENCLOSURE DIAGONAL BRACING - SCHEMATIC PLAN VIEW

SCALE: 3/8" = 1'-0"

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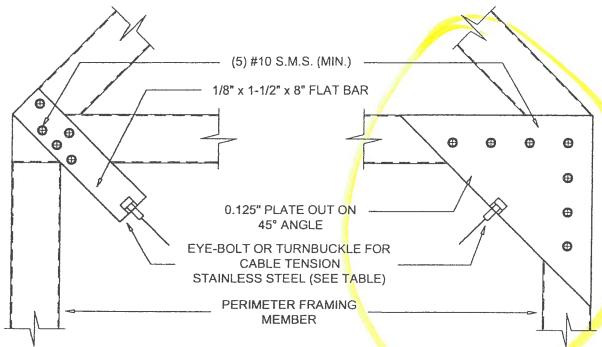
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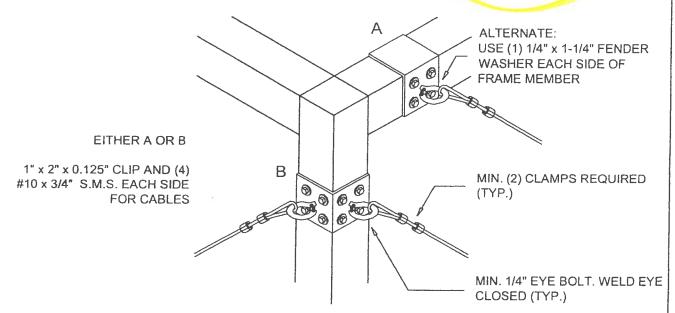
1-39

SECTION 1



TYPICAL CABLE CONNECTIONS AT CORNER - DETAIL 1

SCALE: 3" = 1'-0"



ALTERNATE TOP CORNER OF CABLE CONNECTION - DETAIL 1A

SCALE: 3" = 1'-0"

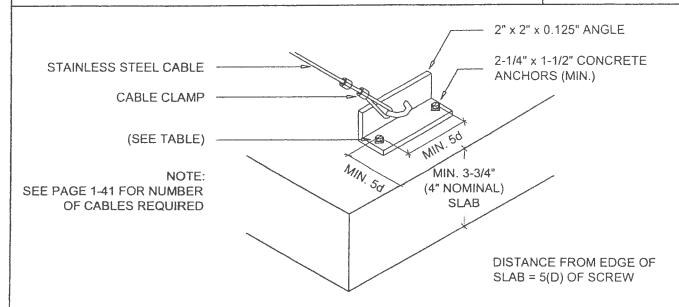
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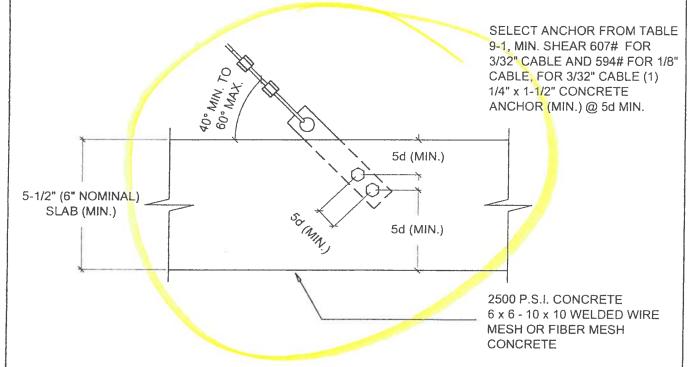
PAGE

SECTION 1



ALTERNATE CABLE CONNECTION AT SLAB DETAIL - DETAIL 2B

SCALE: 3" = 1'-0"



ALTERNATE CABLE CONNECTIONS AT FOUNDATION - DETAIL 2C

SCALE: 3" = 1'-0"

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PAGE

1-43

CABLE BRACING

General Notes and Specifications:

- 1) The following shall apply to the installation of cables as additional bracing to DIAGONAL bracing for pool enclosures:
 - a) FRONT WALL CABLES 7 x 7 OR 7 x 19 STAINLESS STEEL

CABLE DIAMETER	TOTAL ALLOWABLE WALL AREA *
3/32"	233 Sq. Ft. / PAIR OF CABLES
1/8"	445 Sq. Ft. / PAIR OF CABLES

^{*} TOTAL WALL AREA = 100% OF FRONT WALL + 50% OF ONE SIDE WALL

EXAMPLE:

FRONT WALL AREA @ 100% (8' x 32') = 256 Sq. Ft. SIDE WALL AREA @ 50% (8' x 20') = 80 Sq. Ft. TOTAL WALL AREA = 336 Sq. Ft.

233 Sq. Ft. x 2 sets = 466 Sq. Ft. > 336 Sq. Ft.; thus two sets of 3/32" cables is required.

b) SIDE WALL CABLES - 7 x 7 OR 7 x 19 STAINLESS STEEL

CABLE DIAMETER	SIDE WALL CABLE **
3/32"	ONE PER 233 Sq. Ft. OF WALL
1/8"	ONE PER 445 Sq. Ft. OF WALL

^{**} SIDE WALL CABLES ARE NOT REQUIRED FOR SIDE WALLS LESS THAN 233 Sq. Ft.

c) To calculate the required pair of cables for free standing pool enclosures use 100 % of each wall area & 50% of the area of one adjacent wall.

NOTES:

- 1. Where wall height is such that a girt is required between the top or eave rail and the chair rail, (i.e. a mid-rise girt), then the front wall shall have two cable pairs and they shall be attached to the top rail and the mid-rise rail. If more than one additional girt is required between the top or eave rail and the chair rail, then there shall be an additional front wall cable pair at that girt also.
- 2. Side walls do not require cables until the side wall area is greater than 233 Sq. Ft.. The side wall cable may be attached at the mid-rise girt or the top rail.
- 3. Standard rounding off rules apply, ie: if the number of cables calculated is less than 2.5 pairs use two cables; if the number of cables calculated is 2.5 pairs or greater use 3 pairs of cables.

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1-40

Table 1.6 Minimum Upright Sizes and Number of Screws for Connection of Roof Beams to Wall Uprights or Beam Splicing

Beam	Upright	Minimum Purlin, Girt	Deck	Notes	Minimu	ım Number of	Screws*	Beam Stitching
Size	Size	& Knee Brace Size**	Anchors		#8 x ½"	#10 x 1/2"	#12 x ½"	Screw @ 24" O.C.
2" x 3"	2" x 3"	2" x 2" x 0.044"	2	Full Lap	6	4	4	-
2" x 4"	2" x 3"	2" x 2" x 0.044"	2	Full Lap	8	6	4	#8
2" x 4"	2" x 4"	2" x 2" x 0.044"	2	Full Lap	8	6	4	#10
2" x 5"	2" x 3"	2" x 2" x 0.044"	2	Full Lap	8	6	4	#8
2" x 6"	2" x 3"	2" x 2" x 0.044"	4	Full Lap	10	8	6	#10
2" x 6"	2" x 4"	2" x 2" x 0.044"	4	Partial Lap	10	8	6	#10
2" x 7"	2" x 4"	2" x 2" x 0.044"	4	Partial Lap	14	12	10	#12
2" x 8"	2" x 5"	2" x 3" x 0.044"	6	Partial Lap	16	14	12	#14
2" x 9"	2" x 6"	2" x 3" x 0.045"	6	Partial Lap	18	16	14	#14
2" x 9"**	2" x 7"	2" x 4" x 0.050"	8	Partial Lap	20	18	16	#14
2" x 10"	2" x 8"	2" x 4" x 0.050"	10	Partial Lap	20	18	16	#14

Screw Size	Minimum Distance and S	Gusset Plate Thickness							
	Edge To Center	Center To Center	Beam Size	Thickness					
#8	5/16"	5/8"	2" x 7" x 0.055" x 0.120"	1/16" = 0.063"					
#10	3/8"	3/4"	2" x 8" x 0.072" x 0.224"	1/8" = 0.125"					
#12	1/2"	1"	2" x 9" x 0.072" x 0.224"	1/8" = 0.125"					
#14 or 1/4"	3/4"	1-1/2"	2" x 9" x 0.082" x 0306"	1/8" = 0.125"					
5/16"	7/8"	1-3/4"	2" x 10" x 0.092" x 0.369"	1/4" = 0.25"					
3/8"	1"	2"							

^{*} Refers to each side of the connection of the beam and upright and each side of splice connection.

Note:

- 1. Connection of 2" x 6" to 2" x 3" shall use a full lap cut or 1/16" gusset plate.
- 2. All gusset plates shall be a minimum of 5052 H-32 Alloy or have a minimum yield strength of 23 ksi.
- 3. For beam splice connections the number of screws shown is the total for each splice with 1/2 the screws on each side of the cut.
- 4. The number of screws is based on the maximum allowable moment of the beam.
- 5. The number of deck anchors is based on RAWL R Tapper allowable load data for 2,500 psi concrete and / or equal anchors may be used. The number shown is the total use 1/2 per side.
- 6. Hollow splice connections can be made provided the connection is approved by the engineer.
- 7. If a larger than minimum upright is used the number of screws is the same for each splice with 1/2 the screws on each side of the cut.
- 8. All beam to upright connections for 2" x 7" beams or larger shall have an internal or external gusset plates, Gusset plates are required for mansard or gable splice connections.
- 9. For gusset plate connections 2" x 9" beams or larger use 3/4" long screws.
- 10. The side wall upright shall have a minimum beam size as shown above, ie., a 2" x 4" upright shall have a 2" x 3" beam.
- 11. Connect beam to upright w/ H-bar, gusset plate, or angle clips for each splice with 1/2 the screws on each side of the cut.
- 12. For girt size use upright size (i.e. 2" x 6"). Read the 2" x 6" beam row for min. girt of 2" x 2" x 0.044".

Table 1.7 Minimum Size Screen Enclosure Knee Braces and Anchoring Required
Aluminum 6063 T-6

Brace Length	Extrusion	Anchoring System
0' - 2'-0"	2" x 2" x 0.044"	2" H-Channel With (3) #10 x 1/2" EACH SIDE
To 3'-0"	2" x 3" x 0.045"	2" H-Channel With (3) #10 x 1/2" EACH SIDE
To 4'-6"	2" x 4" x 0.044" x 0.12"	2" H-Channel With (4) 3/4" long screws (size to be determined by beam size, see table 9.6)

(See Table 1.6 For Number And Size Of Screws)

Note:

- 1. For required knee braces greater than 4'-6" contact engineer for specifications and details.
- 2. Cantilever beam detail shown on page 1-32 shall be used for host structure attachment when knee brace length exceeds 4'-6".

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^{** 0.082&}quot; wall thickness, 0.310" flange thickness

Table 1.4 Allowable Post / Girt / Chair Rail Spans, Header Spans & Upright Heights for Secondary Screen Wall Frame Members Aluminum Alloy 6063 T-6

For 3 second wind gust at velocity of 120 MPH or an applied load of 14 # / sq. ft.* A. Sections As Horizontals Fastened To Posts With Clips

					Tı	ribut	ary Loa	W	idth 'W'					
Hollow Sections	3'-6'		4'-0'	•	4'-6'	4	5'-0"		5'-6'	4	6'-0"	,	6'-8'	•
			All	owa	ble Hei	ghts	'H' / be	ndir	g 'b' or	ection 'c	<u>'</u>			
2" x 2" x 0.044"	6'-10"	d	6'-6"	b	6'-1"	ь	5'-9"	b	5'-6"	b	5'-3"	b	5'-0"	ь
2" x 2" x 0.055"	7'-3"	d	6'-11"	d	6'-8"	b	6'-4"	Ь	6'-0"	ь	5'-9"	b	5'-6"	b
3" x 2" x 0.045"	7'-9"	ď	7'-5"	d	7'-1"	d	6'-10"	ď	6'-7"	b	6'-4"	ь	5'-11"	b
2" x 3" x 0.045"	9'-4"	b	8'-9"	b	8'-3"	b	7'-10"	b	7'-5"	b	7'-2"	Ь	6'-9"	ь
2" x 4" x 0.050"	10'-3"	b	9'-7"	b	9'-0"	b	8'-7"	ь	8'-2"	b	7'-10"	ь	7'-5"	b

Snap Sections			IIA	owa	ble Hei	hts	'H' / be	ndin	g 'b' or	defle	ction '	d'		
2" x 2" x 0.044"	7'-6"	d	7'-2"	d	6'-11"	d	6'-8"	b	6'-4"	b	6'-1"	b	5'-9"	b

B. Sections As Horizontals Fastened To Posts Through Side Into Screw Bosses

					Tributary Load Width 'W'											
Hollow Sections	3'-6'		4'-0'	4'-6'	•	5'-0'		5'-6	16	6'-0'		6'-8	84			
			Allov		Allowable Hei		'H' / be	ndir	ig 'b' or	defl	ection 'c	ľ				
2" x 2" x 0.044"	7'-9"	b	7'-3"	b	6'-10"	ь	6'-6"	b	6'-2"	b	5'-11"	ь	5'-7"	ь		
2" x 2" x 0.055"	8'-5"	b	7'-11"	Ь	7'-5"	b	7'-1"	b	6'-9"	b	6'-5"	Ь	6'-1"	ь		
3" x 2" x 0.045"	9'-3"	ь	8'-8"	Ь	8'-2"	b	7'-9"	Ь	7'-5"	b	7'-1"	ь	6'-8"	b		
2" x 3" x 0.045"	10'-5"	b	9'-9"	Ь	9'-2"	Ь	8'-9"	b	8'-4"	Ь	7'-11"	ь	7'-7"	b		
2" x 4" x 0.050"	11'-6"	b	10'-9"	Ь	10'-1"	b	9'-7"	b	9'-2"	b	8'-9"	ь	8'-4"	b		
Snap Sections			All	owa	ble Hei	ghts	'H' / be	ndir	g 'b' or	defl	ection 'c	l'				
2" x 2" x 0.044"	9'-2"	ь	8'-7"	ь	8'-1"	ь	7'-8"	b	7'-4"	b	7'-0"	Ы	6'-8"	ь		

^{*} For allowable heights at wind velocities other than 120 MPH, see conversion table 1A on the specifications for tables page at the beginning of this section and example below. Notes:

- 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. Using screen panel width 'W' select girt lengths.
- 3. Site specific engineering required for pool enclosures over 20' in mean roof height.
- 4. Span is to be measured from center of beam and upright connection to fascia or wall connection.
- 5. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height can be considered as residential guardrails provided they are attached with min. (3) #10 x 1-1/2" S.M.S. into the screw bosses and do not exceed 8'-0" in span.
- 6. Girt spacing shall not exceed 6'-8".
- 7. Spans may be interpolated.

IF HEIGHTS FOR 'C' EXPOSURE CATAGORY AND/OR WINDZONES OTHER THAN 120 MPH ARE REQUIRED, SEE EXAMPLE ON SPECIFICATION PAGE FOR TABLES AT THE BEGINNING OF THIS SECTION.

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Table 1.3 Allowable Post / Upright Heights for Primary Screen Wall Frame Members
Aluminum Alloy 6063 T-6

For 3 second wind gust at velocity of 120 MPH or an applied load of 14 # / sq. ft.*

				Tri	butary L	oac	Width "	N ' =	Tributary Load Width 'W' = Upright Spacio											
Hollow Sections	3'-0"	'	4'-0"	•	5'-0'	*	6'-0"		7'-0'	1	8'-0'	•	9'-0"	,						
			Al	llow	able He	ight	'H' / ben	din	g 'b' or ı	defle	ction 'd	•								
2" x 2" x 0.044"	8'-4"	b	7'-3"	b	6'-6"	b	5'-11"	Ь	5'-6"	b	5'-1"	b	4'-10"	b						
2" x 2" x 0.055"	9'-1"	b	7'-11"	b	7'-1"	b	6'-5"	b	5'-11"	b	5'-7"	b	5'-3"	b						
2" x 3" x 0.045"	11'-3"	b	9'-9"	b	8'-9"	b	7'-11"	b	7'-5"	b	6'-11"	b	6'-6"	b						
2" x 4" x 0.050"	12'-5"	ь	10'-9"	b	9'-7"	b	8'-9"	b	8'-1"	b	7'-7"	b	7'-2"	b						

				Tri	butary L	oac	Width '\	N' =	= Upright	Sp	acing			
Self Mating Sections	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
			AJ	low	able Hei	ght	'H' / ben	din	g 'b' or d	efl	ection 'd'			
2" x 4" x 0.044 x 0.100"	16'-11"	b	14'-8"	b	13'-1"	Ь	11'-11"	b	11'-1"	b	10'-4"	b	9'-9"	b
2" x 5" x 0.050" x 0.100"	20'-11"	b	18'-1"	b	16'-2"	b	14'-9"	b	13'-8"	b	12'-10"	b	12'-1"	þ
2" x 6" x 0.050" x 0.120"	24'-2"	b	20'-11"	b	18'-9"	Ь	17'-1"	b	15'-10"	ь	14'-10"	b	13'-11"	b
2" x 7" x 0.055" x 0.120"	27'-3"	b	23'-7"	b	21'-1"	Ъ	19'-3"	b	17'-10"	b	16'-8"	b	15'-9"	b
2" x 7" x 0.055" w/ insert	36'-3"	b	31'-4"	Ь	28'-1"	b	25'-7"	b	23'-9"	b	22'-2"	b	20'-11"	b
2" x 8" x 0.072" x 0.224"	35'-2"	Ь	30'-6"	b	27'-3"	b	24'-10"	b	23'-0"	b	21'-6"	b	20'-4"	b
2" x 9" x 0.072" x 0.224"	38'-2"	b	33'-0"	b	29'-6"	b	26'-11"	b	24'-11"	b	23'-4"	b	22'-0"	b
2" x 9" x 0.082" x 0.310"	41'-10"	ь	36'-3"	b	32'-5"	b	29'-7"	ь	27'-5"	b	25'-8"	b	24'-2"	b
2" x 10" x 0.092" x 0.369"	50'-4"	b	43'-7"	b	38'-11"	b	35'-7"	b	32'-11"	b	30'-10"	b	29'-1"	b

	Tributary Load Width 'W'= Upright Spacing														
Snap Sections	3'-0'	,	4'-0"		5'-0"	,	6'-0'		7'-0"	,	8'-0"		9'-0"		
		Allowable Height 'H' / bending 'b' or deflection 'd'													
2" x 2" x 0.044"	9'-11"	b	8'-7"	ь	7'-8"	Ь	7'-0"	Ь	6'-6"	Ь	6'-1"	b	5'-9"	b	
2" x 3" x 0.045"	12'-9"	b	11'-0"	b	9'-10"	b	9'-0"	b	8'-4"	b	7'-10"	b	7'-4"	b	
2" x 4" x 0.045"	15'-7"	b	13'-6"	b	12'-1"	b	11'-0"	b	10'-2"	b	9'-7"	b,	8'-11"	Ь	
2" x 6" x 0.062"	26'-5"	b	22'-10"	b	20'-5"	b	18'-8"	b	17'-3"	b	16'-2"	b	15'-3"	b	
2" x 7" x 0.062"	29'-5"	b	25'-5"	b	22'-9"	b	20'-9"	Ь	19'-3"	b	17'-11"	þ	16'-11"	b	

^{*} For allowable heights at wind velocities other than 120 MPH, see conversion table 1A on the specification page for tables at the beginning of this section and example below.

Note:

- 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. Using screen panel width 'W' select upright length 'H'.
- 3. Above heights do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
- 4. Site specific engineering required for pool enclosures over 20' in mean roof height.
- 5. Height is to be measured from center of beam and upright connection to fascia or wall connection.
- 6. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height can be considered as residential guardrails provided they are attached with min. (3) #10 x 1-1/2" S.M.S. into the screw bosses and do not exceed 8'-0" in span.
- 7. Heights may be interpolated.

CHECK TABLE 1.6 FOR MINIMUM UPRIGHT SIZE FOR BEAMS.

IF SPANS FOR 'C' EXPOSURE CATAGORY AND/OR WINDZONES OTHER THAN 120 MPH ARE REQUIRED, SEE EXAMPLE ON SPECIFICATION PAGE FOR TABLES AT THE BEGINNING OF THIS SECTION.

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1-58

Table 1.1

Allowable Spans for Primary Screen Roof Frame Members Aluminum Alloy 6063 T-6

For Areas with Wind Loads up to 150 M.P.H. and Latitudes Below 30°-30'-00" North (Jacksonville, FL)

				Tr	ibutary l	oa	d Width	W'	= Beam	Spa	cing			
Hollow Sections	3'-0"		4'-0"		5'-0"		6'-0"		7'-0'		8'-0"		9'-0'	
110110# Occitorio			Al	low	able Spa	ın 'l	' / bend	ing	'b' or de	flec	tion 'd'			_
2" x 2" x 0.044"	9'-10"	b	8'-7"	b	7'-8"	b	6'-11"	b	6'-6"	b	6'-1"	Ь	5'-8"	_ b
2" x 2" x 0.055"	10'-9"	b	9'-4"	b	8'-4"	b	7'-7"	b	7'-1"	b	6'-7"	b	6'-3"	b
2" x 3" x 0.045"	13'-4"	b	11'-7"	b	10'-4"	b	9'-5"	b	8'-9"	Ъ	8'-2"	b	7'-8"	b
2" x 4" x 0.050"	14'-8"	ь	12'-8"	b	11'-4"	b	10'-4"	b	9'-7"	b	8'-11"	b	8'-5"	<u> </u>

	Tributary Load Width 'W' = Beam Spacing													
Self Mating Sections	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
Oct Mating Control	Allowable Span 'L' / bending 'b' or deflection 'd'													
2" x 4" x 0.044 x 0.100"	19'-11"	b	17'-4"	b	15'-6"	b	14'-2"	þ	13'-1"	b	12'-3"	b	11'-6"	b
2" x 5" x 0.050" x 0.100"	24'-9"	b	21'-5"	b	19'-2"	b	17'-6"	b	16'-2"	b	15'-2"	b	14'-3"	Ь
2" x 6" x 0.050" x 0.120"	28'-7"	b	24'-9"	ь	22'-2"	b	20'-3"	b	18'-9"	b	17'-6"	b	16'-6"	b
2" x 7" x 0.055" x 0.120"	32'-3"	ь	27'-11"	ь	24'-11"	b	22'-9"	Ь	21'-1"	b	19'-9"	b	18'-7"	b
2" x 7" x 0.055" w/ insert	42'-10"	ь	37'-1"	ь	33'-2"	b	30'-4"	b	28'-1"	b	26'-3"	b	24'-9"	b
2" x 8" x 0.072" x 0.224"	41'-7"	b	36'-1"	b	32'-3"	b	29'-5"	b	27'-3"	р	25'-6"	b	24'-0"	þ
2" x 9" x 0.072" x 0.224"	45'-1"	b	39'-1"	b	34'-11"	b	31'-11"	b	29'-6"	Ъ	27'-8"	b	26'-1"	Ь
2" x 9" x 0.082" x 0.310"	49'-6"	b	42'-11"	ь	38'-4"	b	35'-0"	Ь	32'-5"	Ь	30'-4"	b	28'-7"	b
2" x 10" x 0.092" x 0.369"	59'-6"	ь	51'-7"	b	46'-1"	Ь	42'-1"	b	38'-11"	b	36'-5"	b	34'-4"	b

Snap Sections	Tributary Load Width 'W' = Beam Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Span 'L' / bending 'b' or deflection 'd'													
2" x 2" x 0.044"	11'-9"	b	10'-2"	b	9'-1"	b	8'-4"	b	7'-8"	b	7'-2"	b	6'-9"	b
2" x 3" x 0.045"	15'-1"	b	13'-1"	b	11'-8"	b	10'-8"	Ь	9'-10"	b	9'-3"	b	8'-8"	_b
2" x 4" x 0.045"	18'-5"	b	15'-11"	b	14'-3"	b	13'-0"	Ь	12'-1"	b	11'-3"	b	10'-8"	b
2" x 6" x 0.062"	31'-3"	b	27'-1"	b	24'-2"	b	22'-1"	b	20'-5"	b	19'-2"	b	18'-0"	b
2" x 7" x 0.062"	34'-9"	b	30'-1"	b	26'-11"	b	24'-7"	b	22'-9"	b	21'-3"	b	20'-1"	b

- 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. The structures designed using this section shall be limited to a maximum combined span and upright height of 55' and a maximum upright height of 20. Structures larger than these limits shall have site specific engineering.
- 3. Spans are based on a minimum of 10# / Sq. Ft. for up to a 150 M.P.H. wind load.
- 4. Span is measured from center of beam and upright connection to fascia or wall connection.
- 5. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
- 6. Purlin spacing shall not exceed 6'- 8". For beam spans greater than 40'-0" the beam at the center purlin and one purlin for each 14'-0" on each side of the center purlin shall include lateral bracing as shown in detail (48'-0") span with purlins at 6'-8" o.c. center purlin and (2) purlins each side of center purlin need lateral bracing.
- 7. Spans may be interpolated.

Example: Max. 'L' for 2" x 4" x 0.050" hollow section with 'W' = 5'-0" = 11'-4"

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1-56

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25095	Notice of reatmen	t BLIST The
Address:	Pest Control & Chemical Co	. (www.flapest.com)
	ision Permit #	<u>N. 9. S</u>
Product used	Active Ingredient	% Concentration
Premise	Imidacloprid	0.1%
☐ <u>Termidor</u>	Fipronil	0.12%
□ Bora-Care I	Disodium Octaborate Tetrah	ydrate 23.0%
Type treatment: Area Treated	Soil Wood Square feet Linear fe	
As per Florida Building termite prevention is us to final building approv	g Code 104.2.6 – If soil chem red, final exterior treatment sh ral.	ical barrier method for nall be completed prior
If this notice is for the f	inal exterior treatment, initial	I this line 101.
8/3//04 • Date Remarks: 4/0 /20//00	Time Print	Technician's Name
Applicator White	Permit File - Canary	Permit Holder - Pink

NOTICE OF COMMENCEMENT FORM **COLUMBIA COUNTY, FLORIDA**

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 18-45-17-08477-107 HX

Description of property: (legal description of the prop	porty and street address or 911 address)
Description of property: (legal description of the prop	1917878+ W5777W988.5094
COMM DE COT COT DEC, MAN	91, M 210.3391, E 183.8591, 8 210.33
W 593.86 H FOR POB W 103.03	acrail a Falcia Mark SID
H to 600 (HKH 1011 O1	centidae Estates North SID
	0.11.
2. General description of improvement: <u>SCREEN</u>	Pool Enclosure
3. Owner Name & Address \\ \text{\text{Wothy Place}}	Interest in Property
Jaha City, F1 32025	Interest in Property
4. Name & Address of Fee Simple Owner (if other than	owner): 1 17
5. Contractor Name Wike Delatos	Phone Number <u>586-154-8618</u>
Address 189 MW Corinth Dr Lake C	Phone Number 386-754-8678
6. Surety Holders Name NA	Phone Number
Address	5t:2006022763 Date:09/25/2006 Time:10:27
Amount of Bond	DC,P.DeWitt Cason,Columbia County B:1096 P:2583
7. Lender Name NA	
A ddynes	
8. Persons within the State of Florida designated by the provided by section 718.13 (1)(a) 7; Florida S	ne Owner upon whom notices or other documents may be statutes:
Name N/A	Phone Number
A. I. Nov 2	
te the second of a lamakas	FI PoolEnclosures of
DEC WING OF WHOLVE TOKE 134 to receive a co	py of the Lienor's Notice as provided in Section 713.13 (1) -
(a) 7 Phone Number of the designee 386-36	4-8628
10 Expiration date of the Notice of Commencement (t	he expiration date is 1 (one) year from the date of recording,
(Unless a different date is specified)	
(Offices & different date of spirit	
NOTICE AS PER CHAPTER 713, Florida Statutes: The owner must sign the notice of commencement and	d no one else may be permitted to sign in his/her stead.
1110	Sworn to (or affirmed) and subscribed before
	day of 5001 13 2006
Ca	NOTARY STAMP 15 Commission # DD371494
Signature of Owner	Expires November 14, 200

Signature of Notary