Builders FirstSource

HLAKMF

0807-48

Project Information:

Builder: Norton Home Imp. Co. Inc.
Model: custom
Builders FirstSource Job #: L279547
Street: 249 SW LOGSTON CT.
City: Fort White
County: Columbia
Building Code: FBC2004/TPI2002
Computer Program Used: MiTek 6.3

Builders FirstSource
2525 E. Duval St.
Lake City, FL 32055

Truss Design Information:

Gravity Loads
Roof: 32 psf Total
Floor: 55 psf Total

Wind
Wind Standard: ASCE 7-02
Wind Speed: 110 mph
Mean Roof Ht: 14 ft
Exposure: B

Note: Refer to individual truss design drawings for special loading conditions, design criteria, truss geometry, lumber, and plate information.

Design Professional Information:

Design Professional Of Record: **License # :**
Delegated Truss Engineer: Julius Lee **License # :** 34869

This truss specification package consists of this index sheet and 11 truss design drawings. This signed and sealed index sheet indicates acceptance of my professional engineering responsibility solely for listed truss design drawings. The suitability and use of each truss component for any particular building is the responsibility of the building designer per ANSI/TPI 1-1995, Section 2.

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By julius lee at 3:47 pm, Jul 10, 2008

JULIUS LEE'S CONSULT. ENGR INC
1455 SW 4TH AVE , DELRAY BEACH
FLORIDA. 33444

Truss #	Truss Label	Drawing #	Seal Date
1	T01	L279547001	7/10/2008
2	T01G	L279547002	7/10/2008
3	T04	L279547003	7/10/2008
4	T04A	L279547004	7/10/2008
5	T04G	L279547005	7/10/2008
6	T05	L279547006	7/10/2008
7	T05G	L279547007	7/10/2008
8	T06	L279547008	7/10/2008
9	T07	L279547009	7/10/2008
10	T08	L279547010	7/10/2008
11	T08G	L279547011	7/10/2008

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T01	COMMON	15	1	Job Reference (optional)

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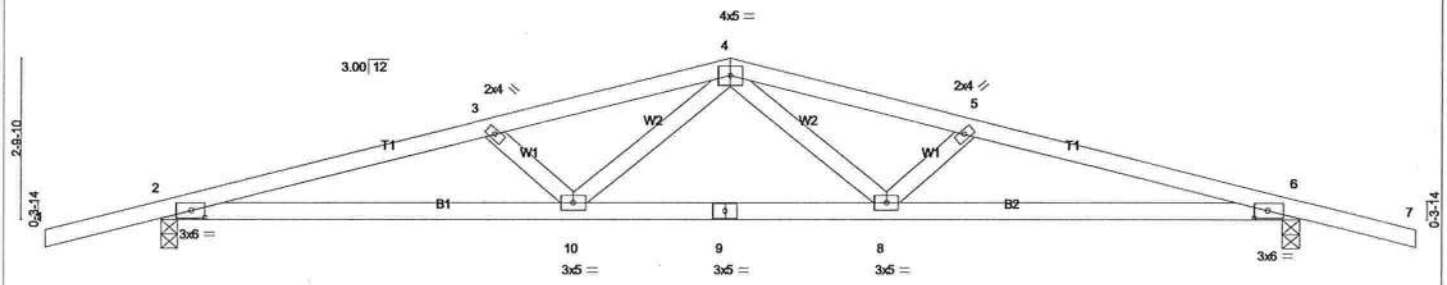
-2-0-0	5-9-14	9-11-0	14-0-2	19-10-0	21-10-0
2-0-0	5-9-14	4-1-2	4-1-2	5-9-14	2-0-0

Scale = 1:38.6
Camber = 1/8 in



7-2-6	12-7-10	19-10-0
7-2-6	5-5-3	7-2-6

Plate Offsets (X,Y): [2:0-2-12,0-1-8], [6:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	0.33	2-10	>704	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.22	8-10	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.19	Horz(TL)	-0.07	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 82 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-11-7 oc bracing.

REACTIONS (lb/size) 2=741/0-3-8, 6=741/0-3-8
Max Horz 2=-47(load case 7)
Max Uplift2=-488(load case 4), 6=-488(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-1762/2439, 3-4=-1541/2324, 4-5=-1541/2324, 5-6=-1762/2439, 6-7=0/25
BOT CHORD 2-10=-2280/1667, 9-10=-1596/1191, 8-9=-1596/1191, 6-8=-2280/1667
WEBS 3-10=-296/234, 4-10=-700/395, 4-8=-700/395, 5-8=-296/234

NOTES (5)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 488 lb uplift at joint 2 and 488 lb uplift at joint 6.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T01G	GABLE	1	1	Job Reference (optional)

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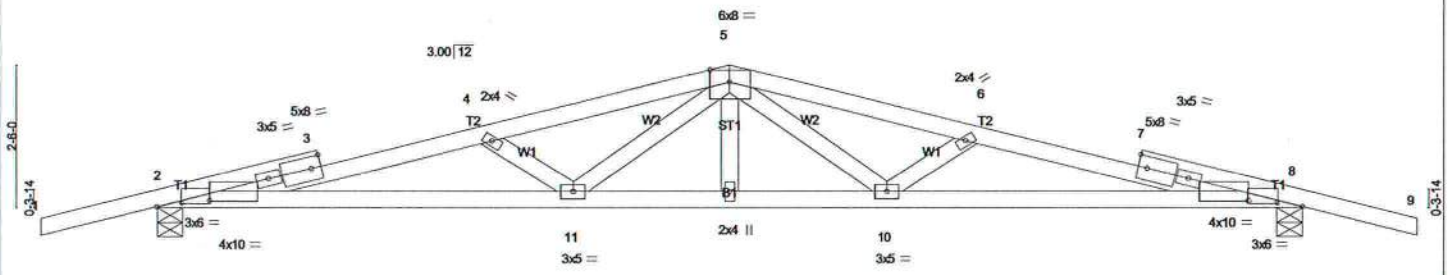
-2-0-0	5-9-14	9-11-0	14-0-2	19-10-0	21-10-0
2-0-0	5-9-14	4-1-2	4-1-2	5-9-14	2-0-0

Scale = 1:38.6
Camber = 3/16 in



7-2-6	12-7-10	19-10-0
7-2-6	5-5-3	7-2-6

Plate Offsets (X,Y): [2:0-5-4,0-0-12], [2:0-11-4,0-1-4], [8:0-5-4,0-0-12], [8:0-11-4,0-1-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.95	Vert(LL)	0.61	10-11	>379	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.72	Vert(TL)	-0.44	10-11	>526	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.29	Horz(TL)	-0.12	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 89 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except* T1 2 X 4 SYP No.2, T1 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-10-0 oc bracing.

REACTIONS (lb/size) 2=1038/0-5-8, 8=1038/0-5-8
Max Horz 2=-56(load case 5)
Max Uplift2=-909(load case 6), 8=-909(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-13/37, 2-3=-2975/4186, 3-4=-2942/4175, 4-5=-2492/3694, 5-6=-2492/3694, 6-7=-2942/4175, 7-8=-2975/4186, 8-9=-13/37
BOT CHORD 2-11=-4025/2876, 10-11=-2521/1830, 8-10=-4025/2876
WEBS 4-11=-624/726, 5-11=-1185/700, 5-10=-1185/700, 6-10=-624/726

NOTES (8)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 909 lb uplift at joint 2 and 909 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-79(F=-25), 5-9=-79(F=-25), 2-8=-10

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T04	COMMON	6	1	Job Reference (optional)

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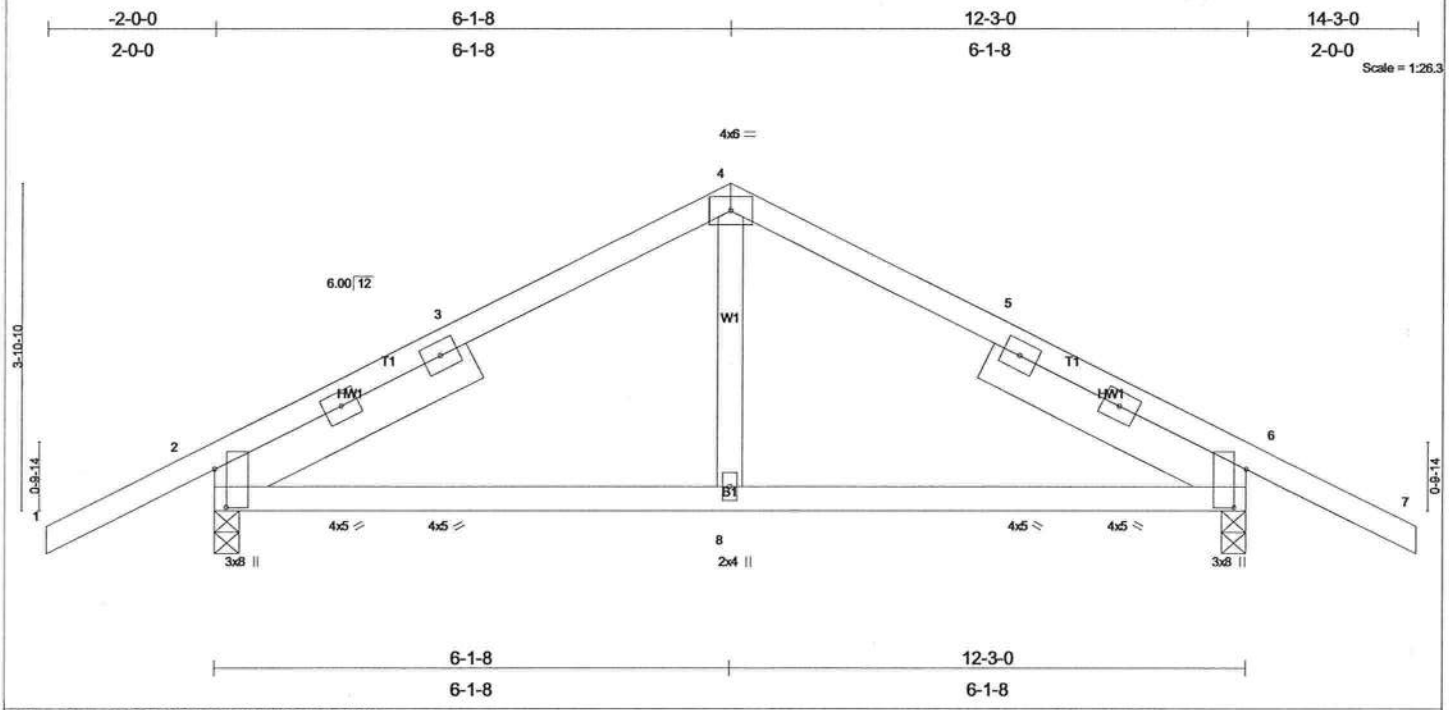

Plate Offsets (X,Y): [2:0-5-7,0-1-11], [6:0-5-7,0-1-11]

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.22	Vert(LL)	-0.02	2-8	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.03	2-8	>999	240		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 67 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 3-5-3, Right 2 X 6 SYP No.1D 3-5-3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=500/0-3-8, 6=500/0-3-8
Max Horz 2=60(load case 6)
Max Uplift2=-175(load case 6), 6=-175(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-496/260, 3-4=-392/282, 4-5=-392/282, 5-6=-496/260, 6-7=0/26
BOT CHORD 2-8=-80/351, 6-8=-80/351
WEBS 4-8=0/188

NOTES (5)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 175 lb uplift at joint 2 and 175 lb uplift at joint 6.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T04A	COMMON	1	2	Job Reference (optional)

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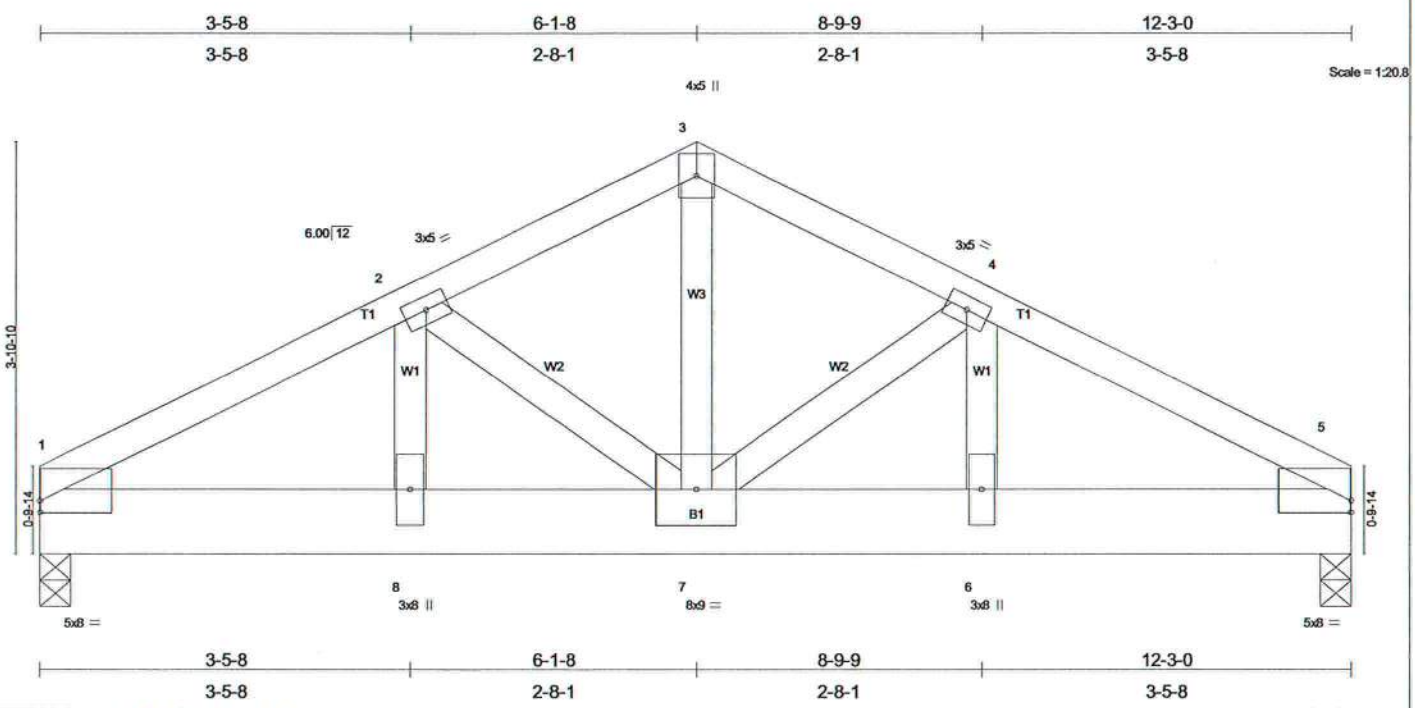

Plate Offsets (X,Y): [1:0-0-1,0-1-6], [5:0-0-1,0-1-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.05	7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.09	7	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.61	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 153 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=4433/0-3-8, 5=4433/0-3-8
Max Horz 1=44(load case 4)
Max Uplift1=-1202(load case 5), 5=-1202(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5924/1599, 2-3=-4513/1245, 3-4=-4513/1245, 4-5=-5924/1600
BOT CHORD 1-8=-1354/4980, 7-8=-1354/4980, 6-7=-1314/4980, 5-6=-1314/4980
WEBS 2-8=-465/1752, 2-7=-1229/375, 3-7=-1044/3802, 4-7=-1229/376, 4-6=-467/1752

NOTES (8)
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1202 lb uplift at joint 1 and 1202 lb uplift at joint 5.
7) Girder carries tie-in span(s): 44-4-0 from 0-0-0 to 12-3-0
8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-54, 1-5=-687(F=-677)

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T04G	GABLE	1	1	Job Reference (optional)

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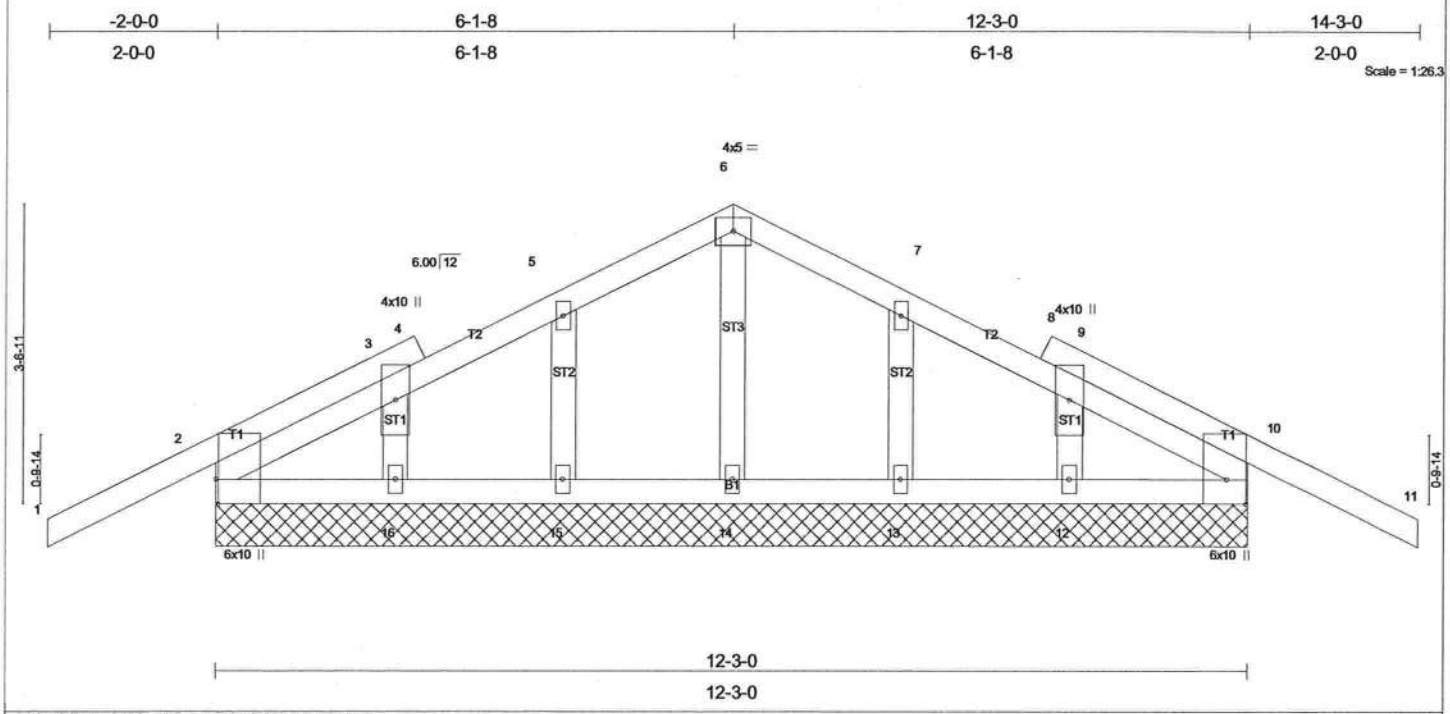

Plate Offsets (X,Y): [2:0-3-8,Edge], [10:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.04	11	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.07	11	n/r	90		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.05	Horz(TL)	0.00	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 67 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=463/12-3-0, 10=463/12-3-0, 14=229/12-3-0, 15=266/12-3-0, 16=144/12-3-0, 13=266/12-3-0, 12=144/12-3-0
Max Horz 2=-65(load case 7)
Max Uplift2=-241(load case 6), 10=-257(load case 7), 14=-23(load case 6), 15=-131(load case 6), 16=-56(load case 7), 13=-131(load case 7), 12=-59(load case 6)
Max Grav 2=463(load case 1), 10=463(load case 1), 14=229(load case 1), 15=270(load case 10), 16=144(load case 1), 13=270(load case 11), 12=144(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-15/54, 2-3=-119/49, 3-4=-21/52, 4-5=-11/62, 5-6=-63/131, 6-7=-63/131, 7-8=-11/52, 8-9=-21/40, 9-10=-119/49, 10-11=-15/54
BOT CHORD 2-16=0/90, 15-16=0/90, 14-15=0/90, 13-14=0/90, 12-13=0/90, 10-12=0/90
WEBS 6-14=-213/71, 5-15=-242/209, 3-16=-154/126, 7-13=-242/209, 9-12=-154/126

NOTES (10)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 241 lb uplift at joint 2, 257 lb uplift at joint 10, 23 lb uplift at joint 14, 131 lb uplift at joint 15, 56 lb uplift at joint 16, 131 lb uplift at joint 13 and 59 lb uplift at joint 12.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-114(F=-60), 6-11=-114(F=-60), 2-10=-10

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T05	POLYNESIAN	12	1	Job Reference (optional)

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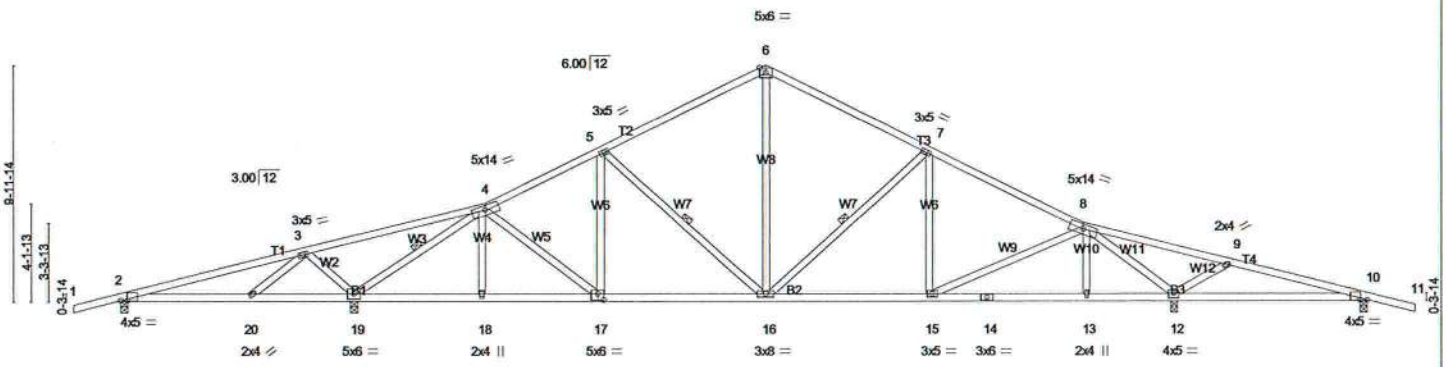
2-0-0	7-10-11	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	46-4-9	52-4-0	54-4-0
2-0-0	7-10-11	7-5-3	4-9-6	6-10-11	6-10-11	6-5-6	6-0-8	5-11-7	2-0-0

Scale = 1:93.6
Camber = 1/16 in



5-7-1	9-9-12	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	44-2-4	52-4-0
5-7-1	4-2-11	5-6-3	4-9-6	6-10-11	6-10-11	6-5-6	3-10-3	8-1-12

Plate Offsets (X,Y): [2:0-3-4,0-0-2], [10:0-3-4,0-0-2], [17:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	0.15	10-12	>636	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	-0.16	16-17	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.05	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 283 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-19, 5-16, 7-16

REACTIONS (lb/size) 2=276/0-3-8, 19=1562/0-3-8, 12=1519/0-3-8, 10=205/0-3-8
Max Horz 2=-132(load case 7)
Max Uplift2=-267(load case 4), 19=-521(load case 6), 12=-449(load case 7), 10=-246(load case 5)
Max Grav 2=302(load case 10), 19=1562(load case 1), 12=1519(load case 1), 10=237(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-6/204, 3-4=-444/692, 4-5=-1237/741, 5-6=-1026/702, 6-7=-1029/705, 7-8=-1273/748, 8-9=-382/768, 9-10=-115/364, 10-11=0/25
BOT CHORD 2-20=-145/0, 19-20=-249/191, 18-19=-330/963, 17-18=-332/961, 16-17=-361/1054, 15-16=-387/1071, 14-15=-297/815, 13-14=-297/815, 12-13=-292/816, 10-12=-311/176
WEBS 3-20=-304/237, 3-19=-566/585, 4-19=-1940/1151, 4-18=0/136, 4-17=-37/124, 5-17=-17/113, 5-16=-340/248, 6-16=-311/475, 7-16=-364/282, 7-15=-46/123, 8-15=-100/282, 8-13=0/115, 8-12=-1934/1028, 9-12=-463/400

NOTES (5)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 267 lb uplift at joint 2, 521 lb uplift at joint 19, 449 lb uplift at joint 12 and 246 lb uplift at joint 10.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.	J1970496
L279547	T05G	GABLE	1	1	Job Reference (optional)	

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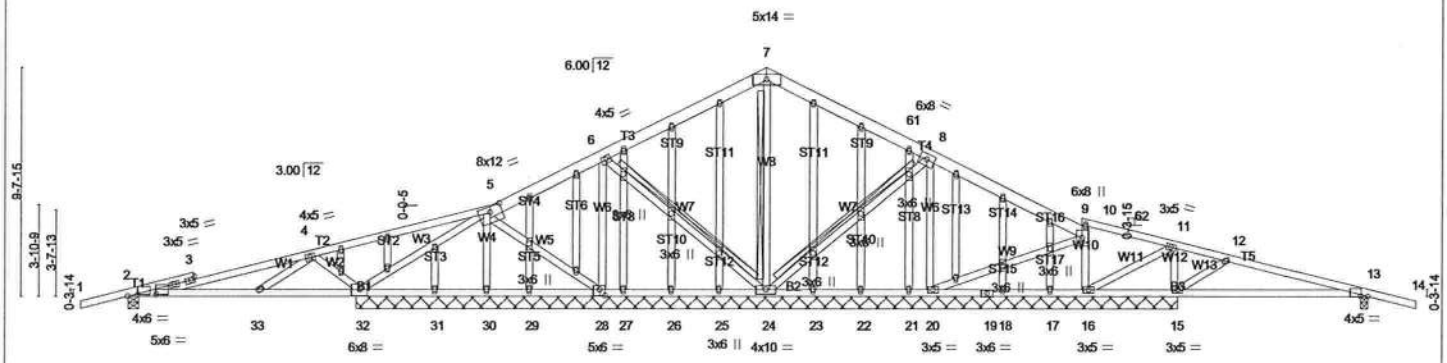
2-0-0	7-10-11	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	46-4-9	52-4-0	54-4-0
2-0-0	7-10-11	7-5-3	4-9-6	6-10-11	6-10-11	6-5-6	6-0-8	5-11-7	2-0-0

Scale = 1:93.6
Camber = 1/16 in



5-7-1	9-9-12	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	44-2-4	52-4-0
5-7-1	4-2-11	5-6-3	4-9-6	6-10-11	6-10-11	6-5-6	3-10-3	8-1-12

Plate Offsets (X,Y): [2:0-5-4,0-0-12], [2:1-2-8,0-0-12], [5:0-6-0,0-2-14], [13:0-3-7,0-0-2], [28:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	0.17	13-15	>573	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.15	13-15	>662	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.58	Horz(TL)	0.01	15	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 401 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D *Except* T2 2 X 4 SYP No.1D, T5 2 X 4 SYP No.2, T1 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 6-24, 7-24, 8-24
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=690/0-5-8, 32=1197/34-8-0, 30=656/34-8-0, 28=482/34-8-0, 24=990/34-8-0, 20=775/34-8-0, 16=605/34-8-0, 15=567/34-8-0, 13=342/0-3-8, 25=20/34-8-0, 26=22/34-8-0, 27=9/34-8-0, 29=25/34-8-0, 31=17/34-8-0, 23=20/34-8-0, 22=22/34-8-0, 21=7/34-8-0, 18=27/34-8-0, 17=24/34-8-0
Max Horz 2=-170(load case 7)
Max Uplift 2=-641(load case 4), 32=-1020(load case 4), 30=-485(load case 6), 28=-373(load case 6), 24=-703(load case 6), 20=-602(load case 7), 16=-452(load case 7), 15=-417(load case 5), 13=-328(load case 5), 17=-30(load case 6)
Max Grav 2=690(load case 10), 32=1197(load case 10), 30=656(load case 1), 28=493(load case 10), 24=990(load case 1), 20=790(load case 11), 16=605(load case 11), 15=567(load case 11), 13=342(load case 11), 25=59(load case 2), 26=65(load case 2), 27=27(load case 2), 29=78(load case 2), 31=72(load case 2), 23=59(load case 2), 22=65(load case 2), 21=21(load case 2), 18=81(load case 2), 17=64(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-30/53, 2-3=-237/356, 3-4=-203/323, 4-5=-985/992, 5-6=-124/191, 6-7=-144/232, 7-61=0/196, 8-61=-182/273, 8-9=-281/301, 10-62=-25/44, 11-62=-66/57, 11-12=-36/118, 12-13=-191/148, 13-14=0/25
BOT CHORD 2-33=-217/197, 32-33=-42/141, 31-32=-205/356, 30-31=-205/356, 29-30=-178/331, 28-29=-178/331, 27-28=-18/207, 26-27=-18/207, 25-26=-18/207, 24-25=-18/207, 23-24=0/142, 22-23=0/142, 21-22=0/142, 20-21=0/142, 19-20=0/119, 18-19=0/119, 17-18=0/119, 16-17=0/119, 15-16=-100/162, 13-15=-65/147
WEBS 4-33=-361/291, 4-32=-1171/1306, 5-32=-787/879, 5-30=-637/603, 5-28=-142/210, 6-28=-594/547, 6-24=-37/145, 7-24=-811/661, 8-24=-218/331, 8-20=-806/802, 9-20=-58/121, 9-16=-673/699, 9-10=-192/193, 11-16=-37/142, 12-15=-291/267, 11-15=-333/239

NOTES (9)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 641 lb uplift at joint 2, 1020 lb uplift at joint 32, 485 lb uplift at joint 30, 373 lb uplift at joint 28, 703 lb uplift at joint 24, 602 lb uplift at joint 20, 452 lb uplift at joint 16, 417 lb uplift at joint 15, 328 lb uplift at joint 13 and 30 lb uplift at joint 17.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-7=-114(F=-60), 7-61=-114(F=-60), 9-61=-141(F=-87), 10-62=-141(F=-87), 14-62=-54, 2-13=-10

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T06	POLYNESIAN	1	1	Job Reference (optional)

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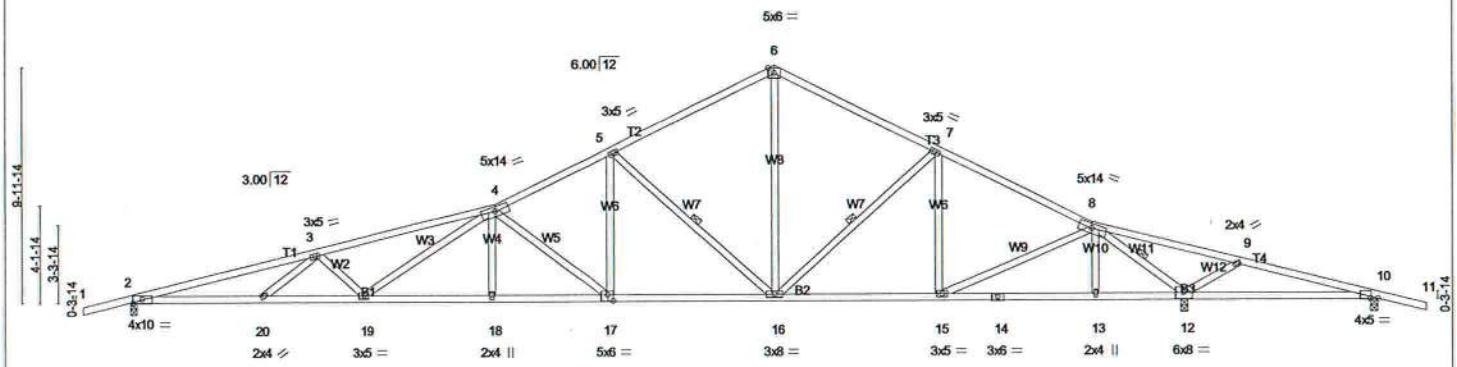
2-0-0	7-10-11	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	46-4-9	52-4-0	54-4-0
2-0-0	7-10-11	7-5-3	4-9-6	6-10-11	6-10-11	6-5-6	6-0-8	5-11-7	2-0-0

Scale = 1:93.6
Camber = 3/8 in



5-7-1	9-9-12	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	44-2-4	52-4-0
5-7-1	4-2-11	5-6-3	4-9-6	6-10-11	6-10-11	6-5-6	3-10-3	8-1-12

Plate Offsets (X,Y): [10:0-3-7,0-0-2], [17:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	0.54	18-19	>972	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.84	18-19	>630	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.97	Horz(TL)	0.18	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 283 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-1-7 oc bracing.
WEBS 1 Row at midpt 5-16, 7-16, 8-12

REACTIONS (lb/size) 2=1439/0-3-8, 12=2224/0-3-8, 10=-101/0-3-8
Max Horz 2=-132(load case 7)
Max Uplift2=-409(load case 6), 12=-544(load case 7), 10=-240(load case 10)
Max Grav 2=1439(load case 1), 12=2224(load case 1), 10=85(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-4433/2410, 3-4=-4040/2310, 4-5=-2533/1515, 5-6=-1534/1008, 6-7=-1537/1012, 7-8=-1568/934, 8-9=-1056/1970, 9-10=-777/1544, 10-11=0/25
BOT CHORD 2-20=-2237/4245, 19-20=-2295/4210, 18-19=-1733/3315, 17-18=-1734/3313, 16-17=-1043/2196, 15-16=-553/1334, 14-15=-138/544, 13-14=-138/544, 12-13=-134/545, 10-12=-1451/815
WEBS 3-20=0/197, 3-19=-481/346, 4-19=-381/686, 4-18=0/145, 4-17=-1425/883, 5-17=-525/966, 5-16=-1204/798, 6-16=-583/908, 7-16=-236/179, 7-15=-313/227, 8-15=-458/912, 8-13=0/108, 8-12=-3029/1659, 9-12=-496/418

NOTES (5)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 409 lb uplift at joint 2, 544 lb uplift at joint 12 and 240 lb uplift at joint 10.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T07	POLYNESIAN	2	1	Job Reference (optional)

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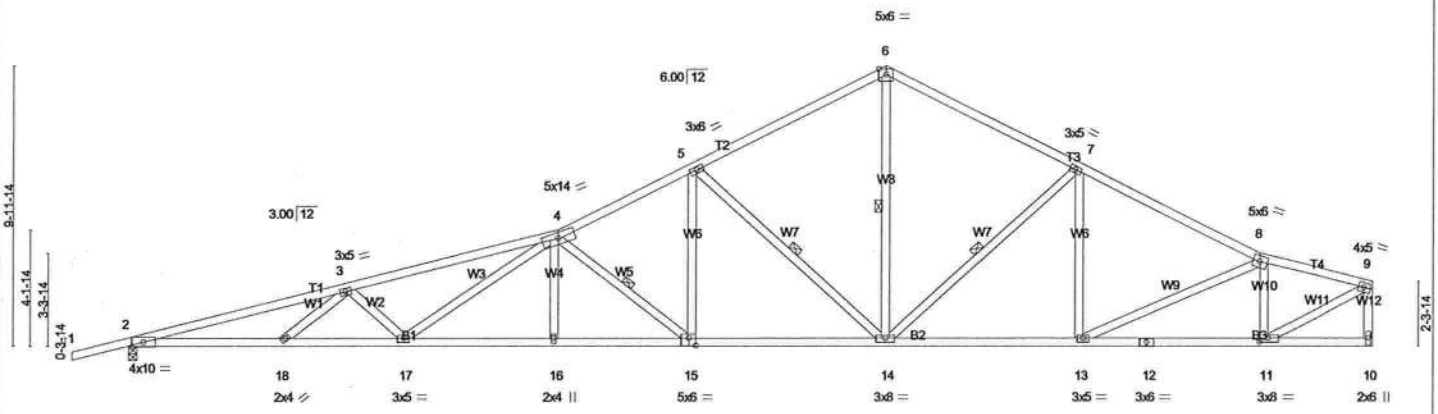
-2-0-0	7-10-11	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	44-4-0
2-0-0	7-10-11	7-5-3	4-9-6	6-10-11	6-10-11	6-5-6	3-11-15

Scale = 1:79.0
Camber = 7/16 in



5-7-1	9-9-12	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	44-4-0
5-7-1	4-2-11	5-6-3	4-9-6	6-10-11	6-10-11	6-5-6	3-11-15

Plate Offsets (X,Y): [11:0-3-8,0-1-8], [15:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.80	Vert(LL)	0.61	16-17	>863	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-0.92	16-17	>575	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.59	Horz(TL)	0.21	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 253 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 4-15, 5-14, 6-14, 7-14

REACTIONS (lb/size) 2=1528/0-3-8, 10=1407/Mechanical
Max Horz 2=180(load case 6)
Max Uplift2=-429(load case 6), 10=-282(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-4779/2607, 3-4=-4395/2514, 4-5=-2851/1698, 5-6=-1809/1167, 6-7=-1813/1171, 7-8=-2102/1240, 8-9=-1706/984, 9-10=-1379/800
BOT CHORD 2-18=-2572/4579, 17-18=-2633/4549, 16-17=-2077/3664, 15-16=-2078/3662, 14-15=-1350/2480, 13-14=-972/1815, 12-13=-960/1678, 11-12=-960/1678, 10-11=-21/41
WEBS 3-18=0/193, 3-17=-476/341, 4-17=-378/682, 4-16=0/145, 4-15=-1509/931, 5-15=-555/1019, 5-14=-1254/826, 6-14=-726/1156, 7-14=-432/317, 7-13=-10/161, 8-13=-27/188, 8-11=-852/554, 9-11=-1044/1826

NOTES (6-7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 429 lb uplift at joint 2 and 282 lb uplift at joint 10.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
7) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T08	POLYNESIAN	4	1	Job Reference (optional)

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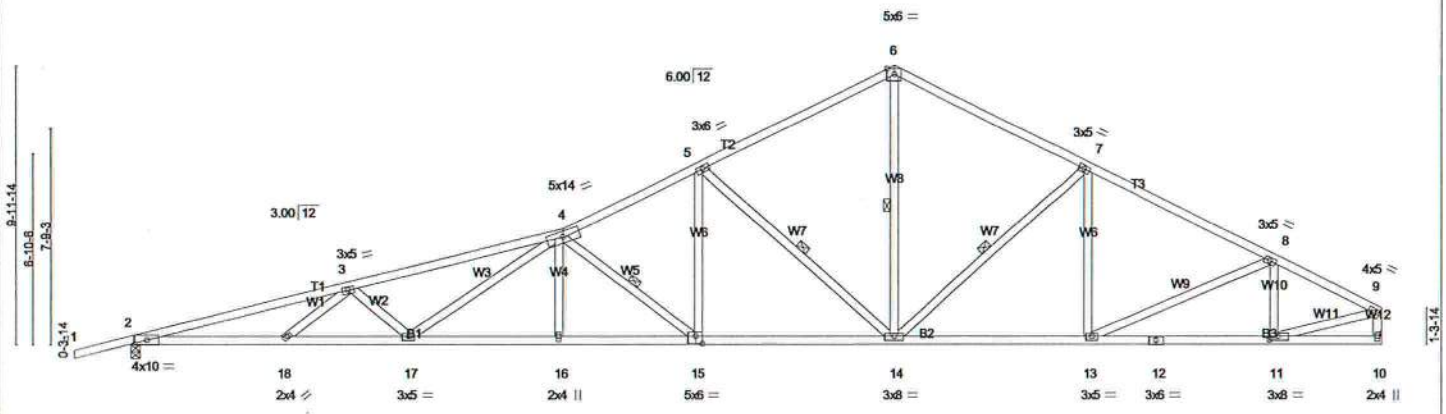
-2-0-0	7-10-11	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	44-4-0
2-0-0	7-10-11	7-5-3	4-9-6	6-10-11	6-10-11	6-5-6	3-11-15

Scale = 1:79.0
Camber = 7/16 in



5-7-1	9-9-12	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	44-4-0
5-7-1	4-2-11	5-6-3	4-9-6	6-10-11	6-10-11	6-5-6	3-11-15

Plate Offsets (X,Y): [11:0-3-8,0-1-8], [15:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.80	Vert(LL)	0.61	16-17	>869	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-0.92	16-17	>576	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.21	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 252 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 4-15, 5-14, 6-14, 7-14

REACTIONS (lb/size) 2=1528/0-3-8, 10=1407/Mechanical
Max Horz 2=160(load case 6)
Max Uplift2=-430(load case 6), 10=-281(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-4779/2615, 3-4=-4395/2521, 4-5=-2852/1705, 5-6=-1809/1172, 6-7=-1814/1176, 7-8=-2105/1250, 8-9=-1900/1088, 9-10=-1376/796
BOT CHORD 2-18=-2529/4579, 17-18=-2590/4549, 16-17=-2034/3664, 15-16=-2035/3662, 14-15=-1306/2480, 13-14=-933/1816, 12-13=-931/1668, 11-12=-931/1668, 10-11=-55/91
WEBS 3-18=0/193, 3-17=-476/341, 4-17=-378/682, 4-16=0/145, 4-15=-1509/932, 5-15=-556/1019, 5-14=-1254/828, 6-14=-732/1157, 7-14=-434/325, 7-13=-10/161, 8-13=-23/196, 8-11=-429/305, 9-11=-914/1643

NOTES (6-7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 430 lb uplift at joint 2 and 281 lb uplift at joint 10.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
7) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T08G	GABLE	1	1	Job Reference (optional)

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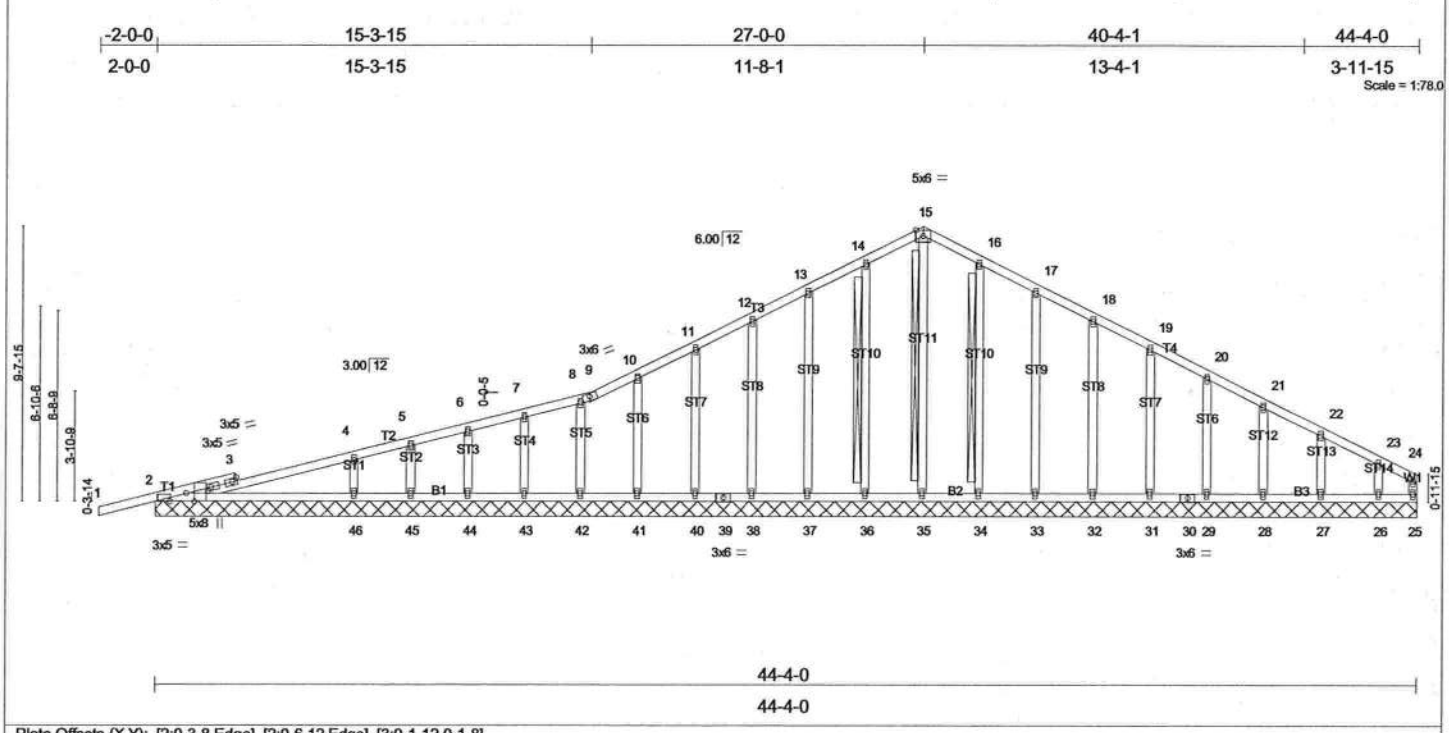


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-6-12,Edge], [3:0-1-12,0-1-8]

LOADING (psf)		SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0		Plates Increase 1.25	TC 0.35	Vert(LL) 0.00 1 n/r 120	MT20 244/190
TCDL 7.0		Lumber Increase 1.25	BC 0.22	Vert(TL) 0.03 1 n/r 90	
BCLL 10.0	*	Rep Stress Incr YES	WB 0.10	Horz(TL) 0.01 25 n/a n/a	
BCDL 5.0		Code FBC2004/TPI2002	(Matrix)		Weight: 275 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 15-35, 14-36, 16-34
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=303/44-4-0, 25=-24/44-4-0, 35=161/44-4-0, 36=131/44-4-0, 37=127/44-4-0, 38=128/44-4-0, 40=129/44-4-0, 41=125/44-4-0, 42=123/44-4-0, 43=118/44-4-0, 44=174/44-4-0, 45=-56/44-4-0, 46=455/44-4-0, 34=131/44-4-0, 33=127/44-4-0, 32=128/44-4-0, 31=128/44-4-0, 29=128/44-4-0, 28=128/44-4-0, 27=127/44-4-0, 26=146/44-4-0
Max Horz 2=186(load case 6)
Max Uplift2=-211(load case 4), 25=-43(load case 10), 36=-73(load case 6), 37=-92(load case 6), 38=-85(load case 6), 40=-87(load case 6), 41=-83(load case 6), 42=-63(load case 6), 43=-67(load case 4), 44=-87(load case 4), 45=-56(load case 1), 46=-213(load case 4), 34=-69(load case 7), 33=-93(load case 7), 32=-85(load case 7), 31=-87(load case 7), 29=-86(load case 7), 28=-90(load case 7), 27=-74(load case 7), 26=-189(load case 7)
Max Grav 2=303(load case 1), 25=151(load case 7), 35=168(load case 7), 36=135(load case 10), 37=127(load case 1), 38=128(load case 10), 40=129(load case 10), 41=125(load case 1), 42=123(load case 1), 43=118(load case 10), 44=174(load case 1), 45=14(load case 7), 46=455(load case 1), 34=133(load case 11), 33=127(load case 11), 32=128(load case 1), 31=128(load case 1), 29=128(load case 11), 28=128(load case 1), 27=130(load case 11), 26=146(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-207/38, 3-4=-199/90, 4-5=-138/33, 5-6=-132/62, 6-7=-104/56, 7-8=-84/71, 8-9=-71/87, 9-10=-68/100, 10-11=-41/138, 11-12=-9/173, 12-13=0/209, 13-14=0/265, 14-15=0/312, 15-16=-16/312, 16-17=-14/265, 17-18=-14/205, 18-19=-14/148, 19-20=-14/91, 20-21=-14/66, 21-22=-46/66, 22-23=-94/67, 23-24=-160/67, 24-25=-94/30
BOT CHORD 2-46=-41/132, 45-46=-41/132, 44-45=-41/132, 43-44=-41/132, 42-43=-41/132, 41-42=-41/132, 40-41=-41/132, 39-40=-41/132, 38-39=-41/132, 37-38=-41/132, 36-37=-41/132, 35-36=-41/132, 34-35=-41/132, 33-34=-41/132, 32-33=-41/132, 31-32=-41/132, 30-31=-41/132, 29-30=-41/132, 28-29=-41/132, 27-28=-41/132, 26-27=-41/132, 25-26=-41/132
WEBS 15-35=-166/0, 14-36=-115/88, 13-37=-107/118, 12-38=-108/109, 11-40=-109/111, 10-41=-105/111, 8-42=-102/101, 7-43=-101/85, 6-44=-140/112, 5-45=-6/28, 4-46=-356/254, 16-34=-113/88, 17-33=-107/118, 18-32=-108/109, 19-31=-108/111, 20-29=-108/110, 21-28=-108/111, 22-27=-111/107, 23-26=-110/148

NOTES (9)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 211 lb uplift at joint 2, 43 lb uplift at joint 25, 73 lb uplift at joint 36 , 92 lb uplift at joint 37, 85 lb uplift at joint 38, 87 lb uplift at joint 40, 83 lb uplift at joint 41, 63 lb uplift at joint 42, 67 lb uplift at joint 43, 87 lb uplift at joint 44, 56 lb uplift at joint 45, 213 lb uplift at joint 46, 69 lb uplift at joint 34, 93 lb uplift at joint 33, 85 lb uplift at joint 32, 87 lb uplift at joint 31, 86 lb uplift at joint 29, 90 lb uplift at joint 28, 74 lb uplift at joint 27 and 189 lb uplift at joint 26.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

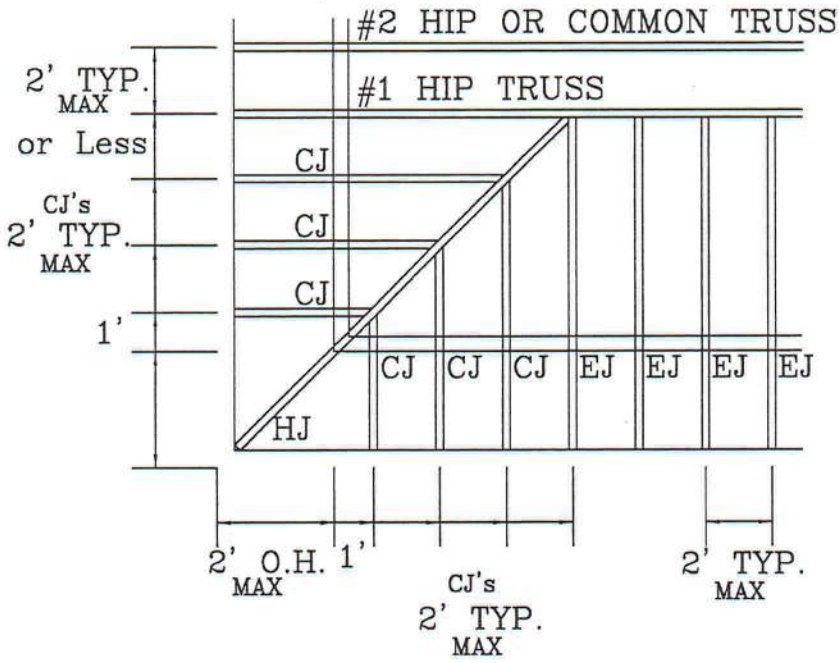
LOAD CASE(S) Standard

STEPDOWN CORNER SET

TOP CHORD 2X4 SO. PINE #2 or Better
BOT CHORD 2X4 SO. PINE #2 or Better
WEBS 2X4 SO. PINE #3 or Better

120 MPH MAX

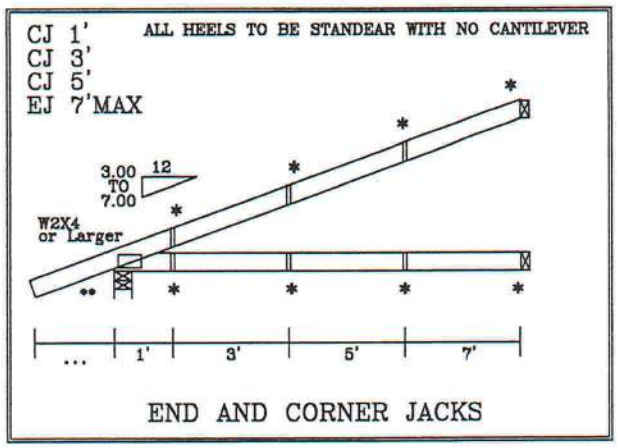
Setback 7' or Less



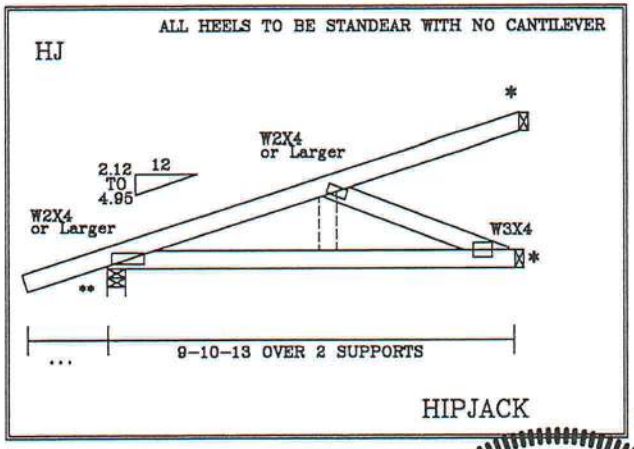
PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED. TILE
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 15.0 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "B" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)



END AND CORNER JACKS



HIPJACK

* (3) 16d TOENAILS

** SEE EOR FOR TIE DOWN

UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED
BC LIVE LOAD IS NON CONCURRENT 10*

CORNER SET SETBACK
7'0" MAX

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S/K) ASTM A653 GRADE 40/60 (W,K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (D) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

JULIUS LEE
CONS. ENGINEERS, P.A.
1485 SW 4th AVENUE
DELRAY BEACH, FL 33444-2181

STATE OF FLORIDA
No. 34869
PROFESSIONAL ENGINEER

SHINGLE			
TC	LL	20	MAX PSF
TC	DL	7	MAX PSF
BC	LL	10*	MAX PSF
BC	DL	5	MAX PSF
TILE			
TC	LL	20	MAX PSF
TC	DL	20	MAX PSF
BC	LL	10*	MAX PSF
BC	DL	5	MAX PSF
DUR. FAC.		1.25	
SPACING		2'	MAX

REF	7'MAX STBK CS
DATE	Jun./27/2008
DRWG	
-ENG	

REVIEWED
By julius lee at 10:52 am, Jun 27, 2008

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 8"	11' 8"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	6' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	6' 0"	6' 8"	6' 8"	8' 3"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	8' 7"	6' 7"	8' 3"	8' 8"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 8"	12' 6"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	6' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 5"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 6"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 6"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	6' 11"	6' 11"	7' 10"	7' 10"	10' 6"	10' 6"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 9"	8' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 6"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 9"	9' 2"	10' 6"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

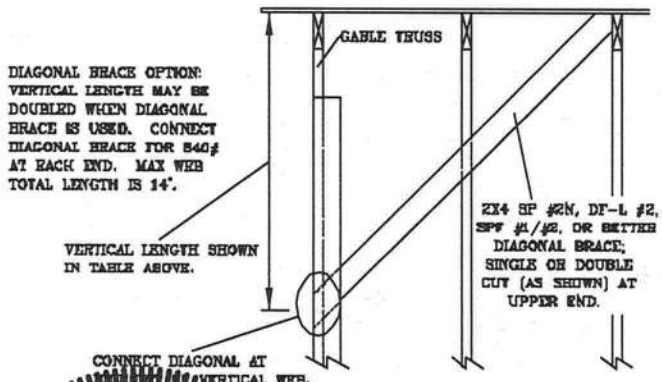
SOUTHERN PINE
#3
STUD
STANDARD

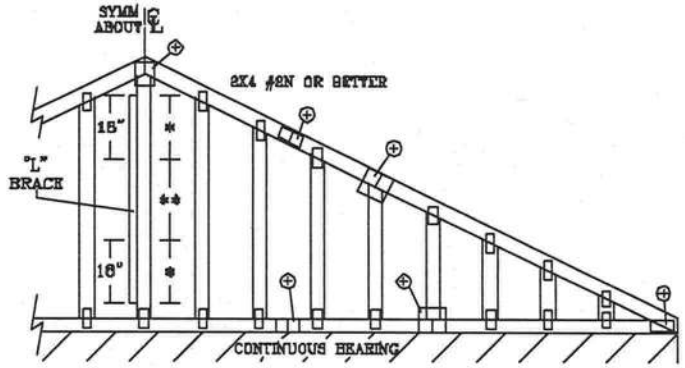
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB13015
DATE	11/26/03
DRWG	MITEK STD GABLE 15 E HT
	-ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	5' 8"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	6' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 6"	5' 6"	5' 11"	6' 5"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 6"
		STUD	3' 3"	4' 6"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 6"
		STANDARD	3' 0"	3' 10"	3' 10"	5' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	5' 4"	5' 10"	7' 5"	8' 1"	8' 11"	8' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	5' 4"	6' 10"	7' 3"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	6' 7"	7' 4"	7' 4"	8' 11"	9' 6"	11' 5"	11' 6"	14' 0"	14' 0"
		STUD	3' 8"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	5' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	5' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 5"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 5"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

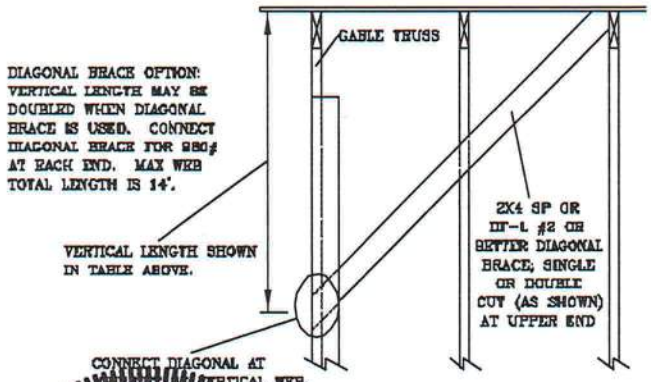
SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

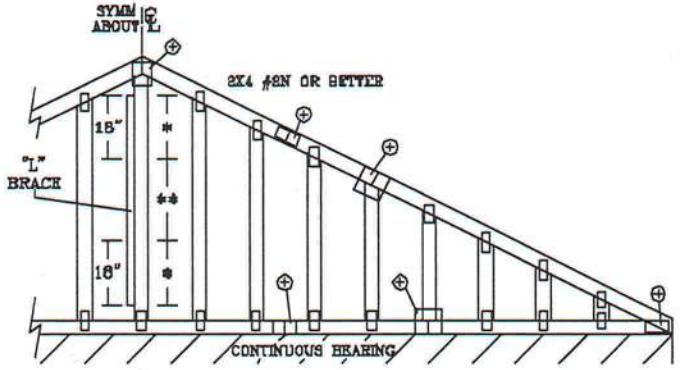
DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE	DOUGLAS FIR-LARCH
#1	#1
#2	#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BRAKING (5 PSF TO DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

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REVIEWED
By julius lee at 12:00 pm, Jun 11, 2008

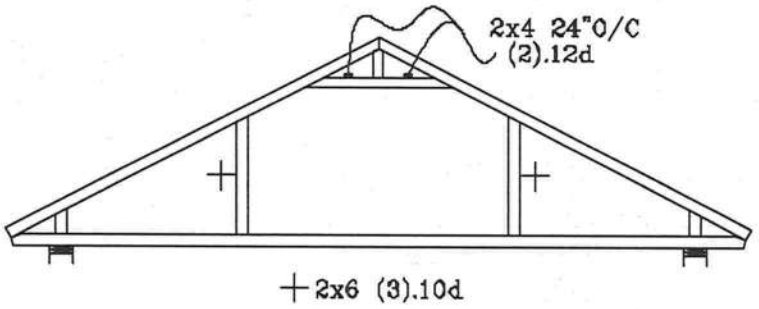
JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

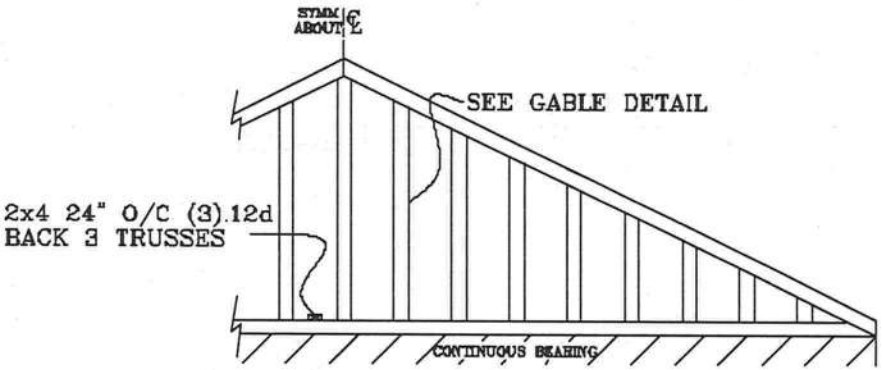
MAX. SPACING 24.0"

REF	ASCE7-02-GAB13030
DATE	11/26/03
DWG	MITEK STD GABLE 30' E W7
-ENG	

TYPICAL ATTIC TRUSS BRACING

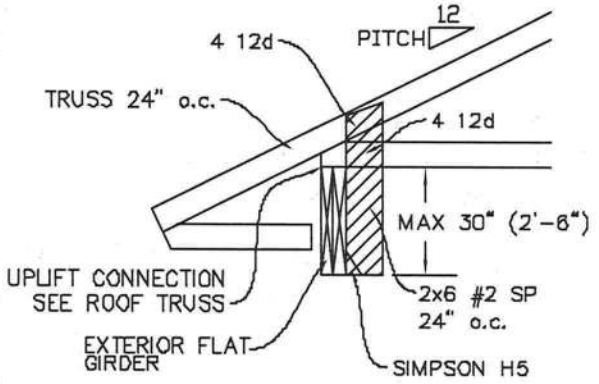


GABLE END TRUSS DETAIL



MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

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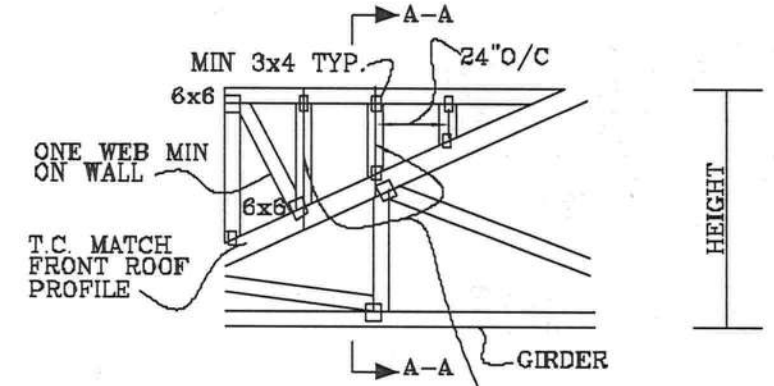
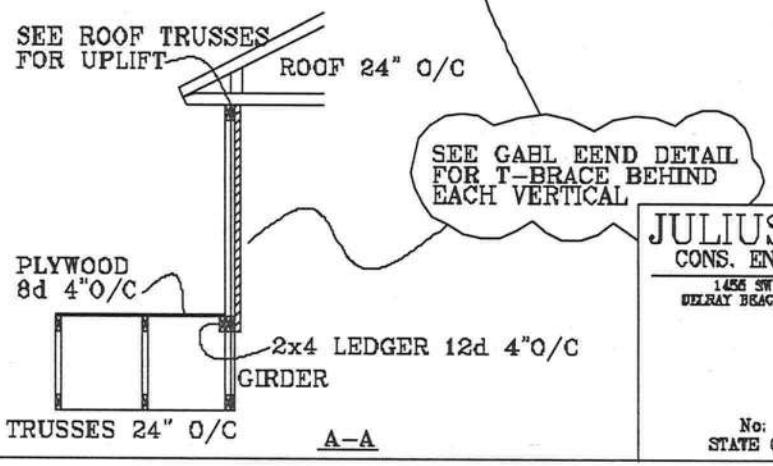
NO. 34869
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL





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DELRAY BEACH, FL 33444-2161

No: 34869
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PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

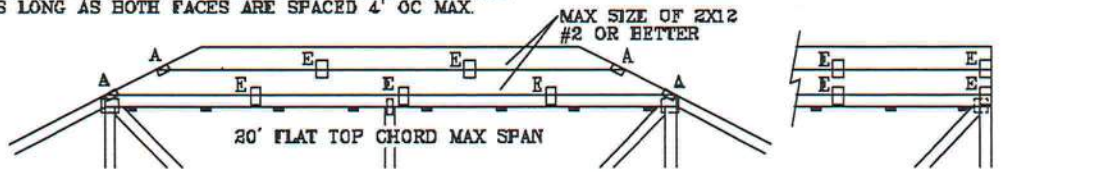
110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

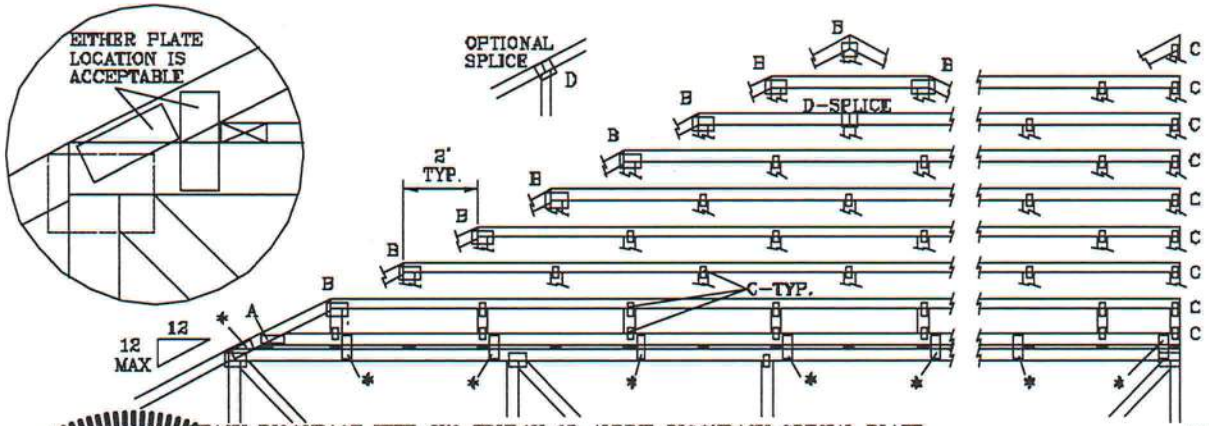
130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X8	6X8	6X8	5X8
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	6X6	6X5	5X6
E	4X8 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.



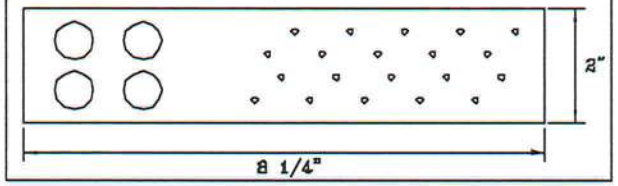


ATTACH PIGGYBACK WITH 3X6 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

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REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S CONS. ENGINEERS P.A.
1466 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161
No: 34869
STATE OF FLORIDA

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG MITEK STD PIGGY
-ENG JL

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

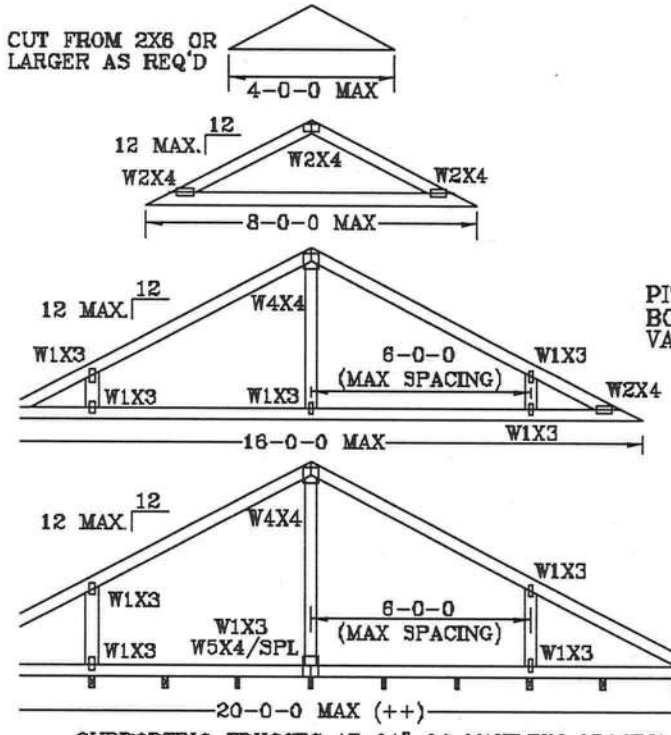
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

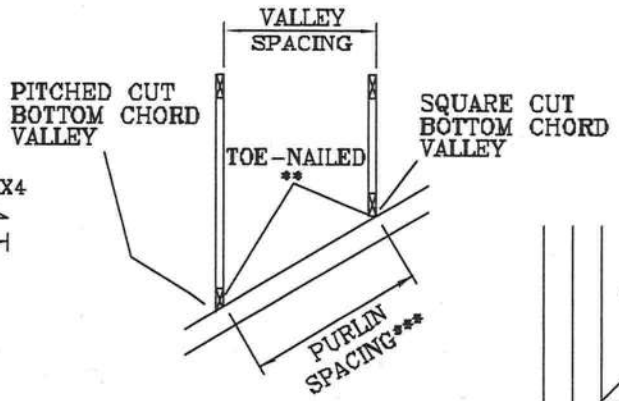
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

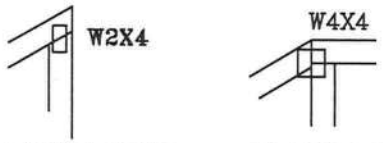
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

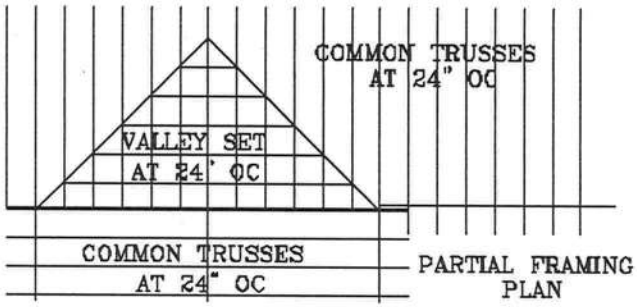
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.







THIS DRAWING REPLACES DRAWING A105

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

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REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF VALLEY DETAIL
TC DL	7	15	PSF	DATE 11/26/03
BC DL	5	5	PSF	DRWG VALTRUSS1103
BC LL	0	0	PSF	-ENG JL
TOT. LD.	32	40	PSF	
DUR.FAC. 1.25		1.25		
SPACING		24"		

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

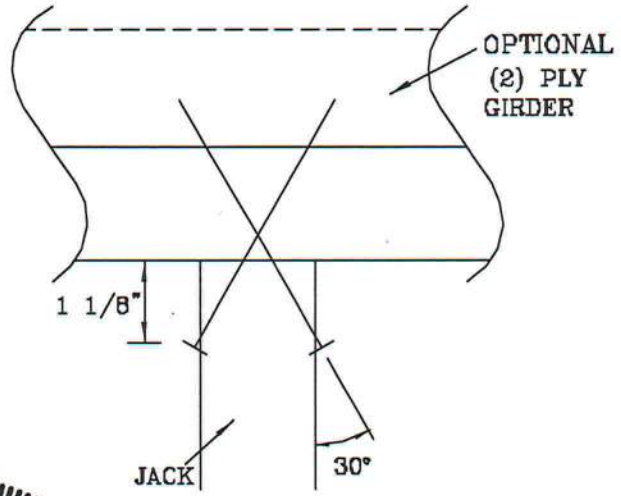
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

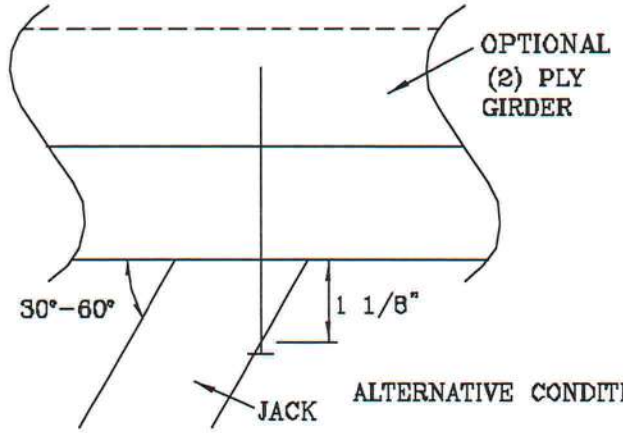
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

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1450 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

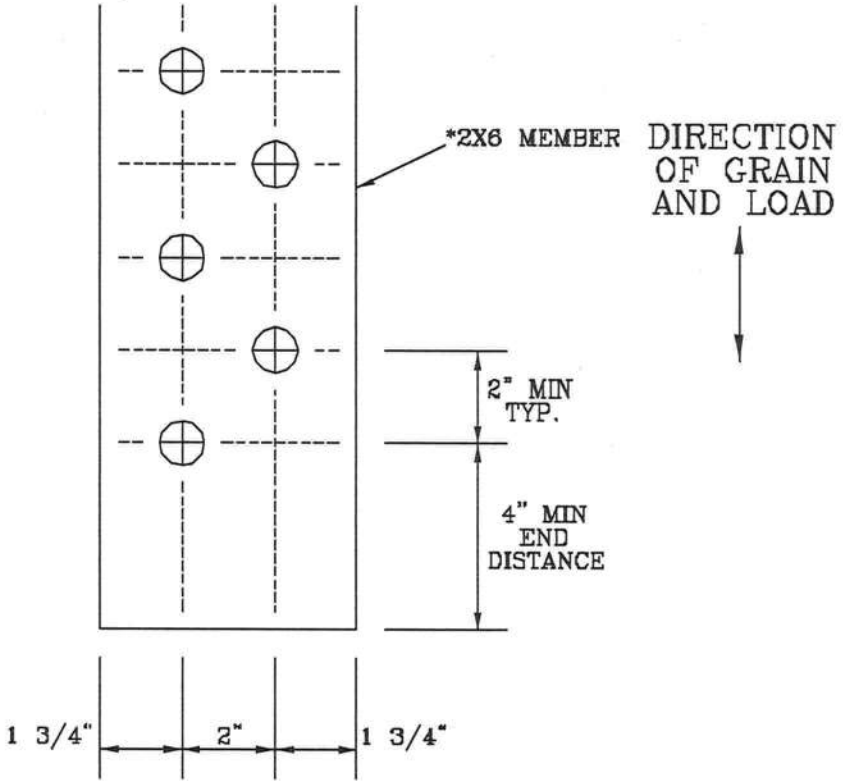
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

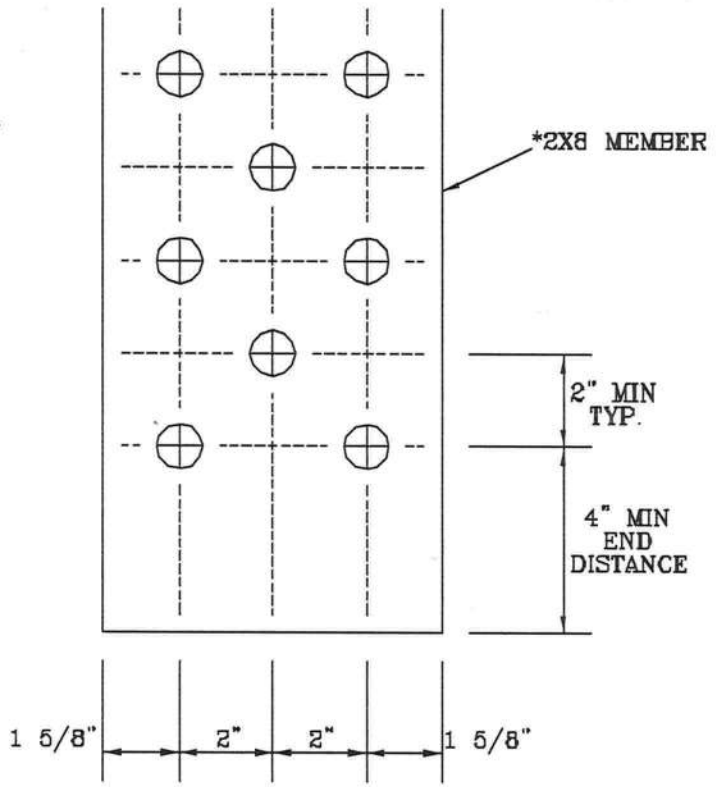
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628,016

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

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CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSP1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TRULOX CONNECTION DETAIL

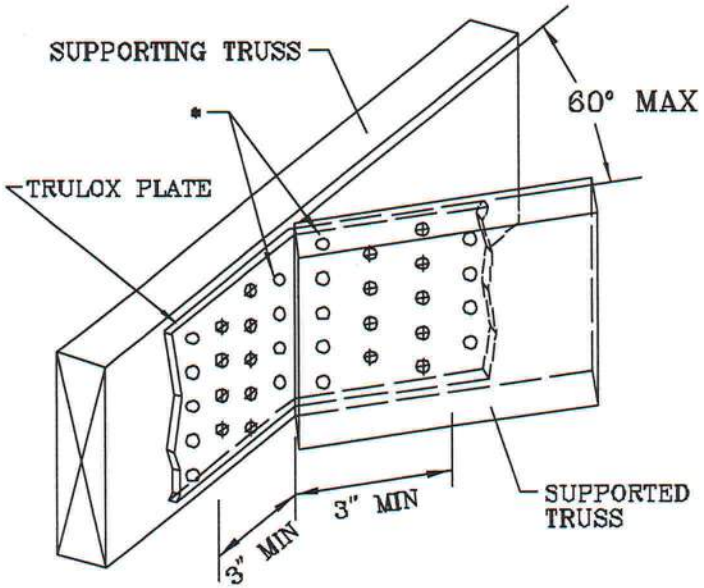
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

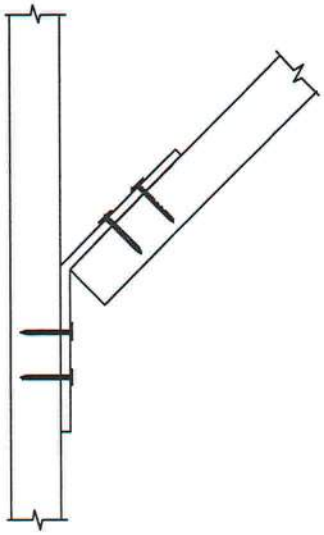
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

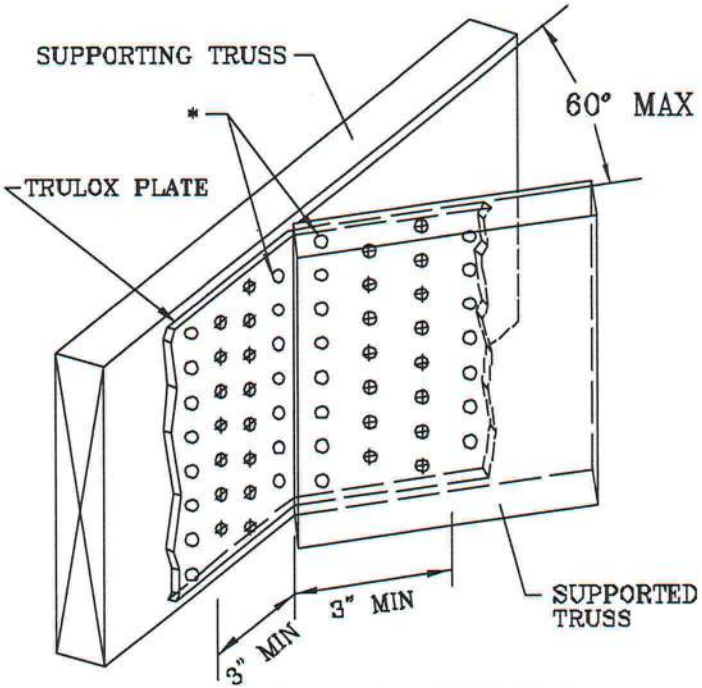
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE





MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

REVIEWED
By julius lee at 11:58 am, Jun 11, 2008

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524

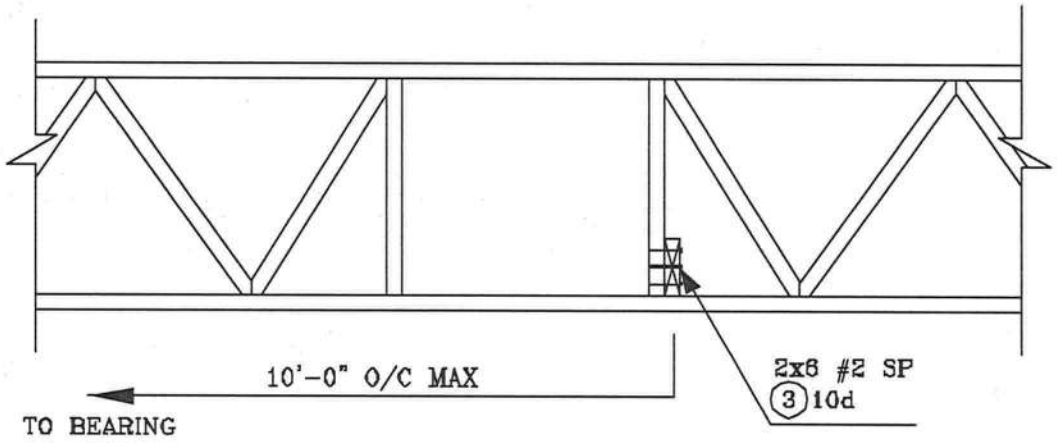
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR, SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERVISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

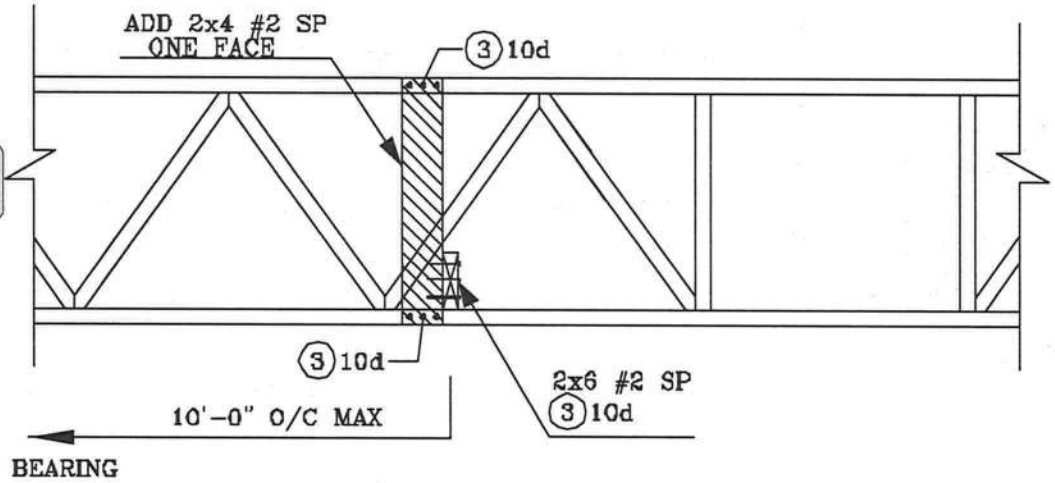
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

STRONG BACK DETAIL
SYSTEM–42 OR FLAT TRUSS



ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP



REVIEWED
By julius lee at 11:58 am, Jun 11, 2008

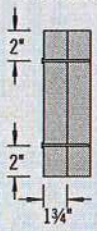
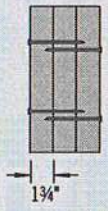
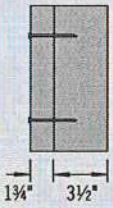
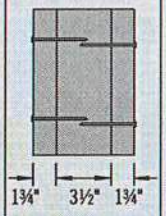
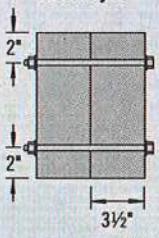
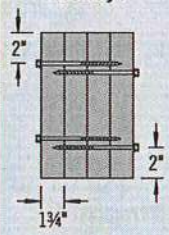
JULIUS LEE
LICENSE
NO. 34869
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A 	Assembly B 	Assembly C 	Assembly D 	Assembly E 	Assembly F 
			3½" 2-ply	5¼" 3-ply	5¼" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail[1]	2	12"	370	***280***	280	***245***		
	3	12"	555	***415***	415	***370***		
½" A307 Through Bolts[2][4]	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS ¼" x 3½"[4]	2	24"	680	***510***	510	***455***		
		19.2"	850	***640***	640	***565***		
		16"	1,020	***765***	765	***680***		
SDS ¼" x 6"[3][4]	2	24"				***455***	***465***	***455***
		19.2"				***565***	***580***	***565***
		16"				***680***	***695***	***680***
USP WS35 [4]	2	24"	480	***360***	360	***320***		
		19.2"	600	***450***	450	***400***		
		16"	715	***540***	540	***480***		
USP WS6 [3][4]	2	24"				***350***	***525***	***350***
		19.2"				***440***	***660***	***440***
		16"				***525***	***790***	***525***
3⅛" TrussLok[4]	2	24"	635	***475***	475	***425***		
		19.2"	795	***595***	595	***530***		
		16"	955	***715***	715	***635***		
5" TrussLok[4]	2	24"		***500***	500	***445***	***480***	***445***
		19.2"		***625***	625	***555***	***600***	***555***
		16"		***750***	750	***665***	***725***	***665***
6¾" TrussLok[4]	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 9⁄16" maximum.

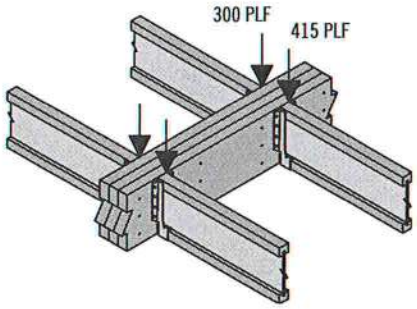
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- ***Bold Italic*** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by ½ the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example

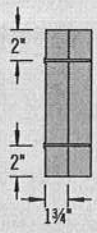
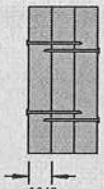
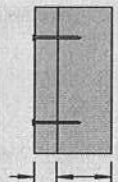
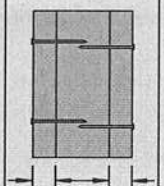
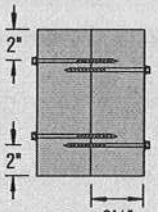
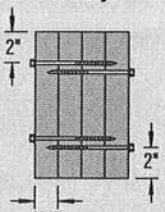


First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1¾" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:
Two rows of ½" bolts or SDS ¼" x 3½" screws at 19.2" on-center.

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
							
		3½" 2-ply	5¼" 3-ply	5¼" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	*835*	835	*740*		
	12	2,225	*1,670*	1,670	*1,485*		
	18	3,335	*2,505*	2,505	*2,225*		
	24	4,450	*3,335*	3,335	*2,965*		
SDS Screws ¼" x 3½" or WS35 ¼" x 6" or WS6(1)	4	1,915	1,435(4)	1,435	*1,275*	*1,860(2)*	*1,405(2)*
	6	2,870	2,150 (4)	2,150	*1,915*	*2,785(2)*	*2,110(2)*
	8	3,825	2,870 (4)	2,870	*2,550*	*3,715(2)*	*2,810(2)*
3⅜" or 5" TrussLok™	4	2,545	1,910 (4)	1,910	*1,695*	*1,925(3)*	*1,775(3)*
	6	3,815	2,860 (4)	2,860	*2,545*	*2,890(3)*	*2,665(3)*
	8	5,090	3,815 (4)	3,815	*3,390*	*3,855(3)*	*3,550(3)*

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

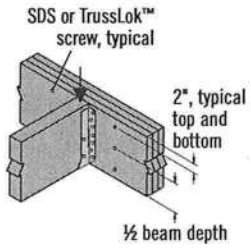
(2) 6" long screws required.

(3) 5" long screws required.

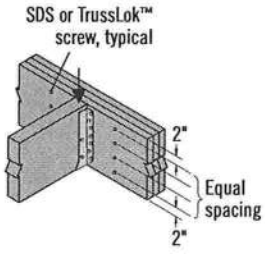
(4) 3½" and 3⅜" long screws must be installed on both sides.

Connections

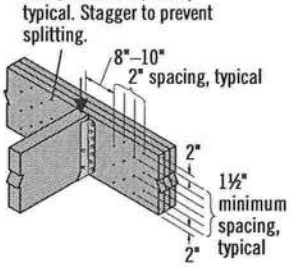
4 or 6 or Screw Connection



8 Screw Connection

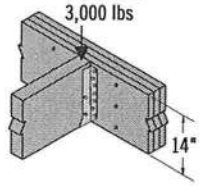


Nail Connection



There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1¾" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1¾" assembly, eight 3⅜" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

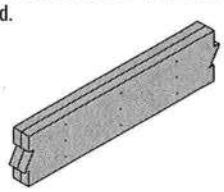
MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1¾" Wide Pieces

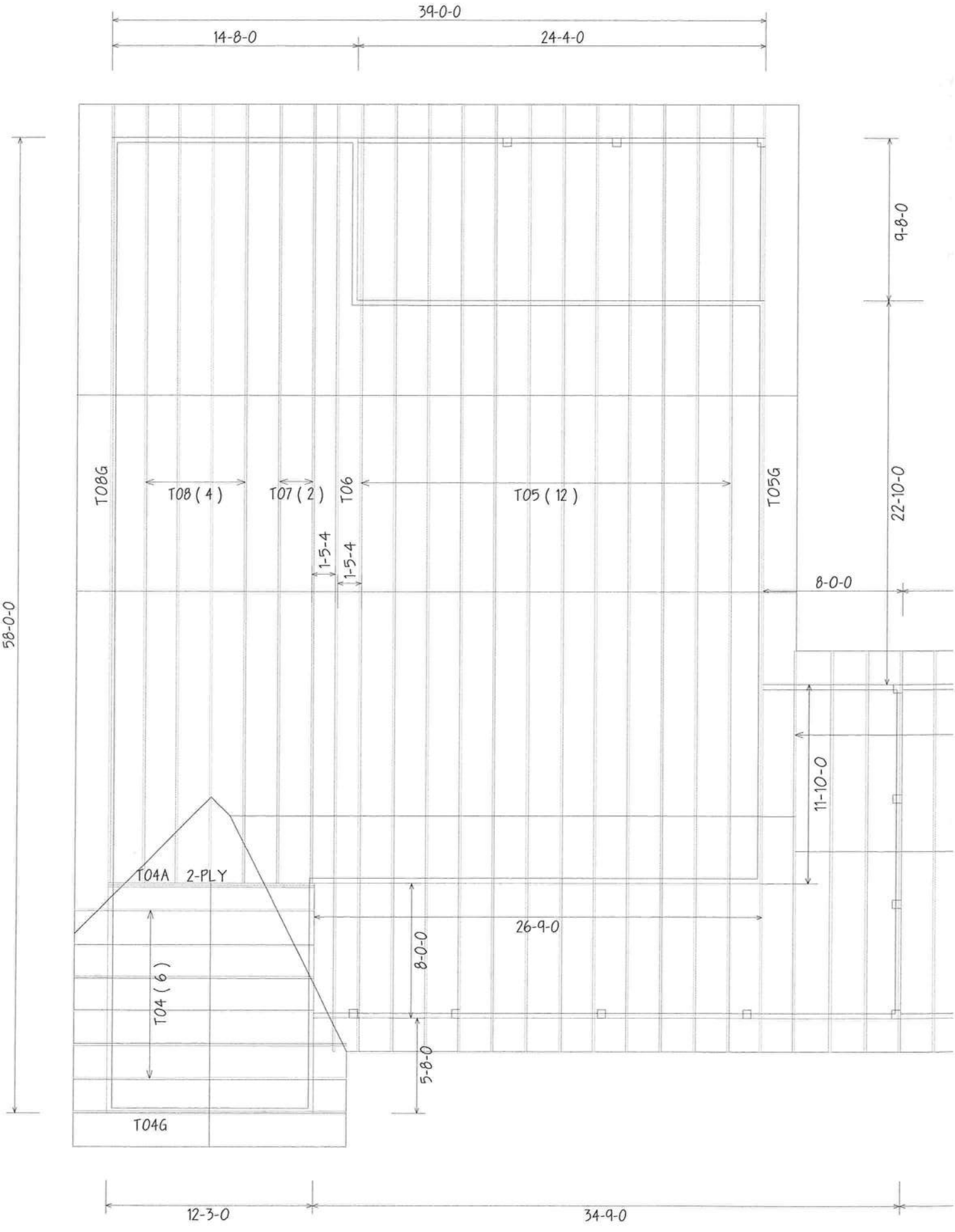
- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3⅜" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3½" Wide Pieces

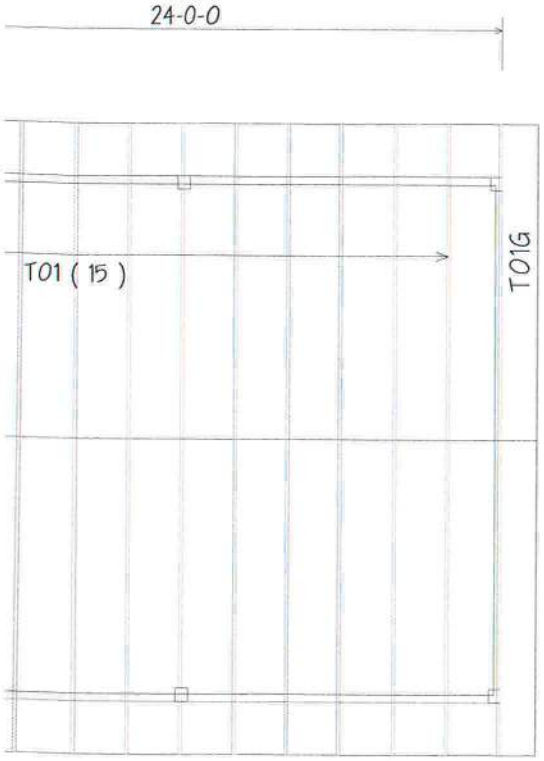
- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of ½" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"


39-0-0
14-8-0
24-4-0
58-0-0
9-8-0
22-10-0
T08G
T08 (4)
T07 (2)
T06
T05 (12)
T05G
1-5-4
1-5-4
8-0-0
11-10-0
T04A 2-PLY
26-9-0
8-0-0
T04 (6)
5-8-0
T04G
12-3-0
34-9-0

6/12 PITCH HOUSE - 2'0" O/H
3/12 PITCH PORCHES/CARPORT



ALL FLAT CEILINGS

BEARING HEIGHT SCHEDULE

8'-0"

HANGER SCHEDULE
7 - HTU26

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V10S FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HTU26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date _____

Approved by: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 386-755-6894 FAX: 386-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: NORTON HOMES		
LEGAL ADDRESS: DAMON RES.		
MODEL: CUSTOM		REVISION: SCALE: NTS
DATE: 7-9-08	DRAWN BY: K.L.H.	JOB #: L279547

Builders FirstSource

HLAKMF

0807-48

Project Information:

Builder: Norton Home Imp. Co. Inc.
Model: custom
Builders FirstSource Job #: L279547
Street: 249 SW LOGSTON CT.
City: Fort White
County: Columbia
Building Code: FBC2004/TPI2002
Computer Program Used: MiTek 6.3

Builders FirstSource
2525 E. Duval St.
Lake City, FL 32055

Truss Design Information:

Gravity Loads
Roof: 32 psf Total
Floor: 55 psf Total

Wind
Wind Standard: ASCE 7-02
Wind Speed: 110 mph
Mean Roof Ht: 14 ft
Exposure: B

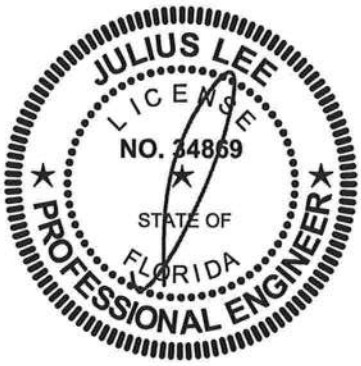
Note: Refer to individual truss design drawings for special loading conditions, design criteria, truss geometry, lumber, and plate information.

Design Professional Information:

Design Professional Of Record:
Delegated Truss Engineer: Julius Lee

License # :
License # : 34869

This truss specification package consists of this index sheet and 11 truss design drawings. This signed and sealed index sheet indicates acceptance of my professional engineering responsibility solely for listed truss design drawings. The suitability and use of each truss component for any particular building is the responsibility of the building designer per ANSI/TPI 1-1995, Section 2.



REVIEWED
By julius lee at 3:47 pm, Jul 10, 2008

JULIUS LEE'S CONSULT. ENGR INC
1455 SW 4TH AVE , DELRAY BEACH
FLORIDA. 33444

Truss #	Truss Label	Drawing #	Seal Date
1	T01	L279547001	7/10/2008
2	T01G	L279547002	7/10/2008
3	T04	L279547003	7/10/2008
4	T04A	L279547004	7/10/2008
5	T04G	L279547005	7/10/2008
6	T05	L279547006	7/10/2008
7	T05G	L279547007	7/10/2008
8	T06	L279547008	7/10/2008
9	T07	L279547009	7/10/2008
10	T08	L279547010	7/10/2008
11	T08G	L279547011	7/10/2008

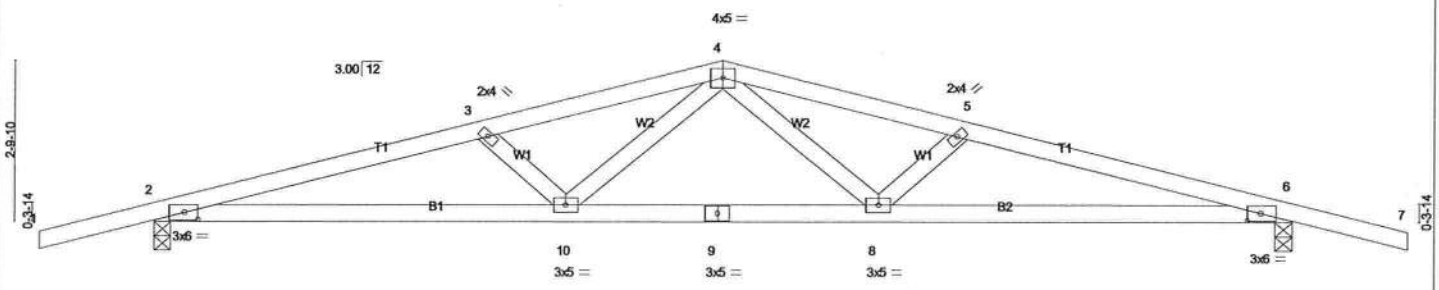
Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T01	COMMON	15	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jul 10 09:27:07 2008 Page 1

-2-0-0 | 5-9-14 | 9-11-0 | 14-0-2 | 19-10-0 | 21-10-0

2-0-0 | 5-9-14 | 4-1-2 | 4-1-2 | 5-9-14 | 2-0-0

Scale = 1:38.6
Camber = 1/8 in



7-2-6 | 12-7-10 | 19-10-0

7-2-6 | 5-5-3 | 7-2-6

Plate Offsets (X,Y): [2:0-2-12,0-1-8], [6:0-2-12,0-1-8]

LOADING (psf)		SPACING 2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase 1.25	TC 0.45	Vert(LL)	0.33	2-10	>704	360	MT20	244/190
TCDL	7.0	Lumber Increase 1.25	BC 0.35	Vert(TL)	-0.22	8-10	>999	240		
BCLL	10.0 *	Rep Stress Incr YES	WB 0.19	Horz(TL)	-0.07	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002	(Matrix)						Weight: 82 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-11-7 oc bracing.

REACTIONS (lb/size) 2=741/0-3-8, 6=741/0-3-8
Max Horz 2=-47(load case 7)
Max Uplift2=-488(load case 4), 6=-488(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-1762/2439, 3-4=-1541/2324, 4-5=-1541/2324, 5-6=-1762/2439, 6-7=0/25
BOT CHORD 2-10=-2280/1667, 9-10=-1596/1191, 8-9=-1596/1191, 6-8=-2280/1667
WEBS 3-10=-296/234, 4-10=-700/395, 4-8=-700/395, 5-8=-296/234

NOTES (5)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 488 lb uplift at joint 2 and 488 lb uplift at joint 6.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T01G	GABLE	1	1	Job Reference (optional)

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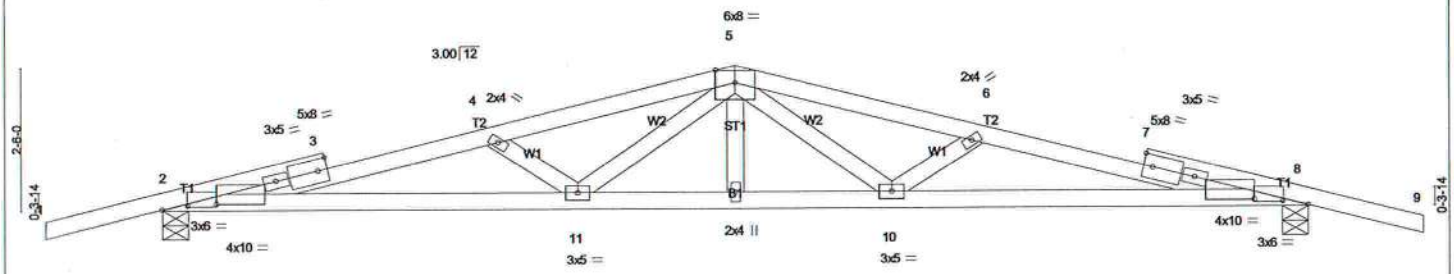
-2-0-0	5-9-14	9-11-0	14-0-2	19-10-0	21-10-0
2-0-0	5-9-14	4-1-2	4-1-2	5-9-14	2-0-0

Scale = 1:38.6
Camber = 3/16 in



7-2-6	12-7-10	19-10-0
7-2-6	5-5-3	7-2-6

Plate Offsets (X,Y): [2:0-5-4,0-0-12], [2:0-11-4,0-1-4], [8:0-5-4,0-0-12], [8:0-11-4,0-1-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.95	Vert(LL)	0.61	10-11	>379	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.72	Vert(TL)	-0.44	10-11	>526	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.29	Horz(TL)	-0.12	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 89 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except* T1 2 X 4 SYP No.2, T1 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-10-0 oc bracing.

REACTIONS (lb/size) 2=1038/0-5-8, 8=1038/0-5-8
Max Horz 2=-56(load case 5)
Max Uplift2=-909(load case 6), 8=-909(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-13/37, 2-3=-2975/4186, 3-4=-2942/4175, 4-5=-2492/3694, 5-6=-2492/3694, 6-7=-2942/4175, 7-8=-2975/4186, 8-9=-13/37
BOT CHORD 2-11=-4025/2876, 10-11=-2521/1830, 8-10=-4025/2876
WEBS 4-11=-624/726, 5-11=-1185/700, 5-10=-1185/700, 6-10=-624/726

NOTES (8)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 909 lb uplift at joint 2 and 909 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-79(F=-25), 5-9=-79(F=-25), 2-8=-10

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T04	COMMON	6	1	Job Reference (optional)

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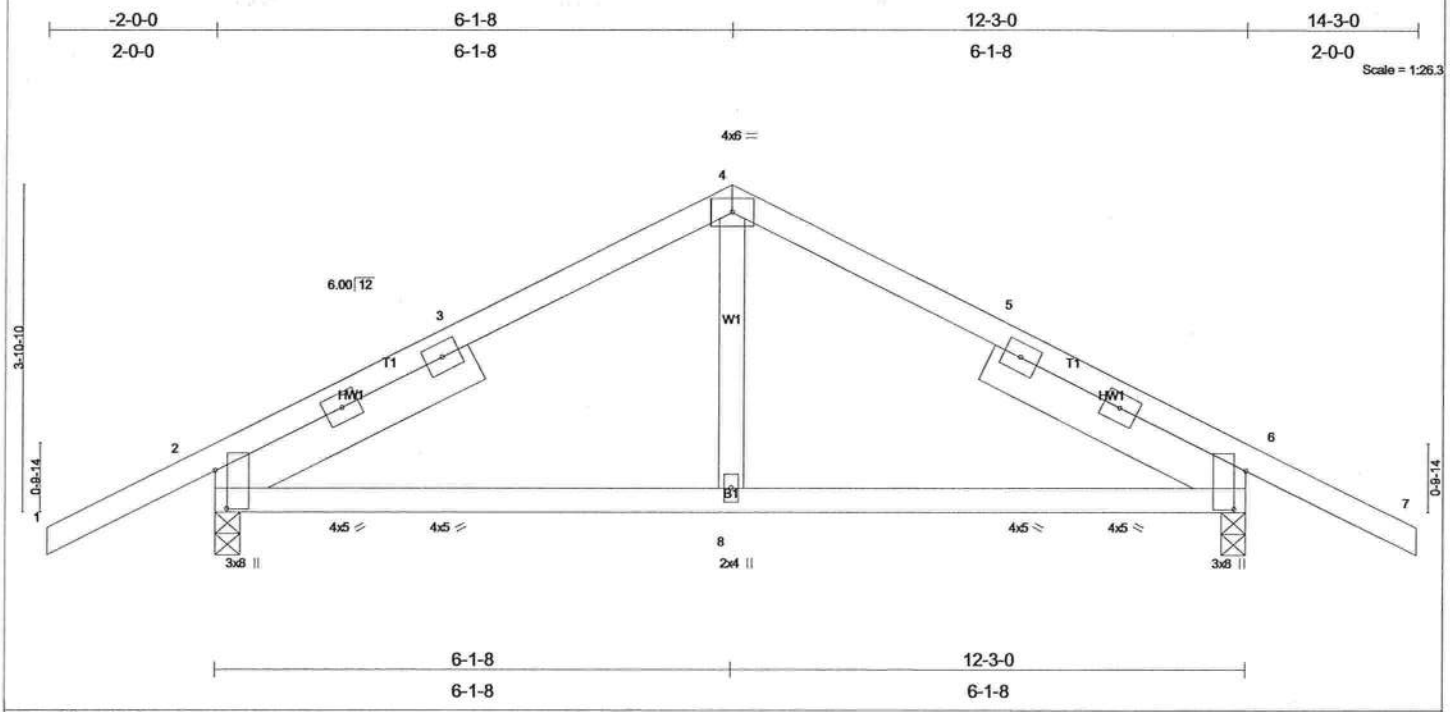

Plate Offsets (X,Y): [2:0-5-7,0-1-11], [6:0-5-7,0-1-11]

LOADING (psf)			SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0		Plates Increase	1.25	TC	0.22	Vert(LL)	-0.02	2-8	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.03	2-8	>999	240		
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0		Code FBC2004/TPI2002		(Matrix)							Weight: 67 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 3-5-3, Right 2 X 6 SYP No.1D 3-5-3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=500/0-3-8, 6=500/0-3-8
Max Horz 2=60(load case 6)
Max Uplift2=-175(load case 6), 6=-175(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/26, 2-3=-496/260, 3-4=-392/282, 4-5=-392/282, 5-6=-496/260, 6-7=0/26
BOT CHORD 2-8=-80/351, 6-8=-80/351
WEBS 4-8=0/188

NOTES (5)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 175 lb uplift at joint 2 and 175 lb uplift at joint 6.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T04A	COMMON	1	2	Job Reference (optional)

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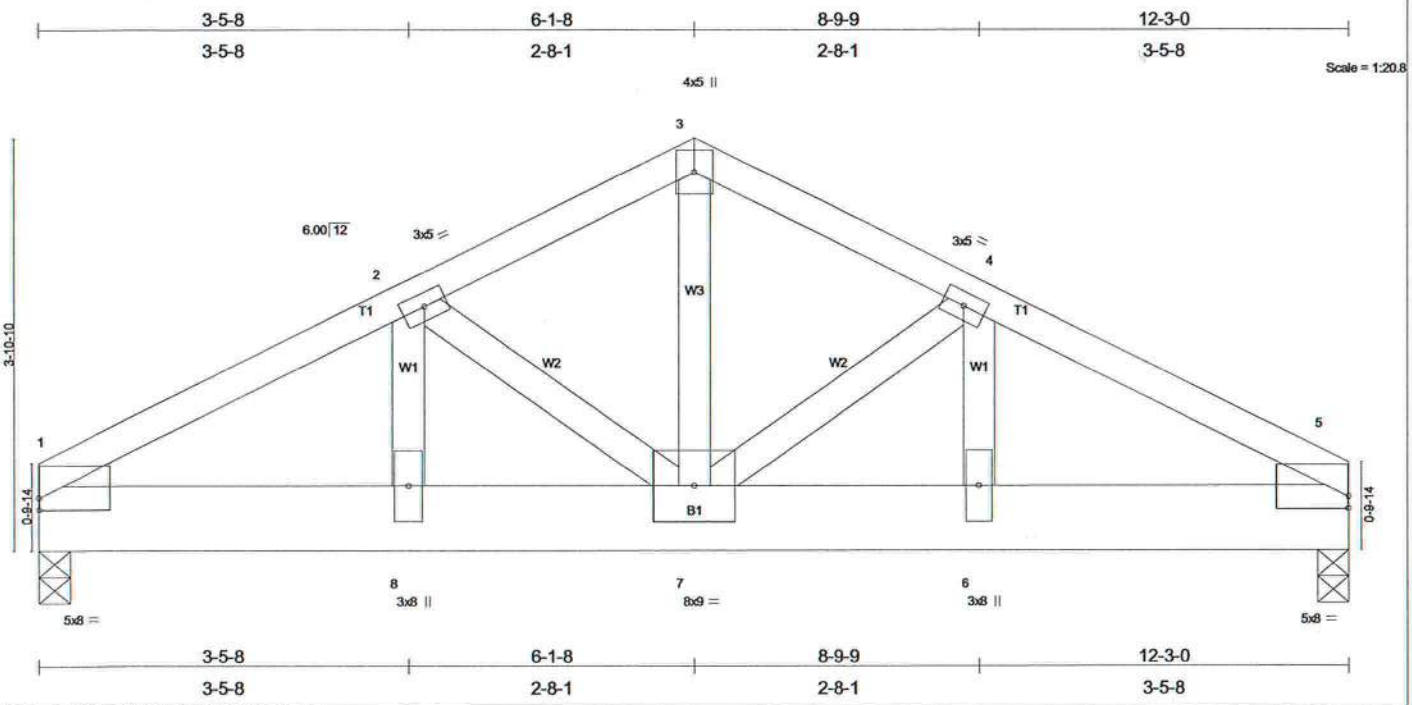

Plate Offsets (X,Y): [1:0-0-1,0-1-6], [5:0-0-1,0-1-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.05	7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.09	7	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.61	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 153 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-5-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=4433/0-3-8, 5=4433/0-3-8
Max Horz 1=44(load case 4)
Max Uplift1=-1202(load case 5), 5=-1202(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5924/1599, 2-3=-4513/1245, 3-4=-4513/1245, 4-5=-5924/1600
BOT CHORD 1-8=-1354/4980, 7-8=-1354/4980, 6-7=-1314/4980, 5-6=-1314/4980
WEBS 2-8=-465/1752, 2-7=-1229/375, 3-7=-1044/3802, 4-7=-1229/376, 4-6=-467/1752

NOTES (8)
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1202 lb uplift at joint 1 and 1202 lb uplift at joint 5.
7) Girder carries tie-in span(s): 44-4-0 from 0-0-0 to 12-3-0
8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-54, 1-5=-687(F=-677)

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T04G	GABLE	1	1	Job Reference (optional)

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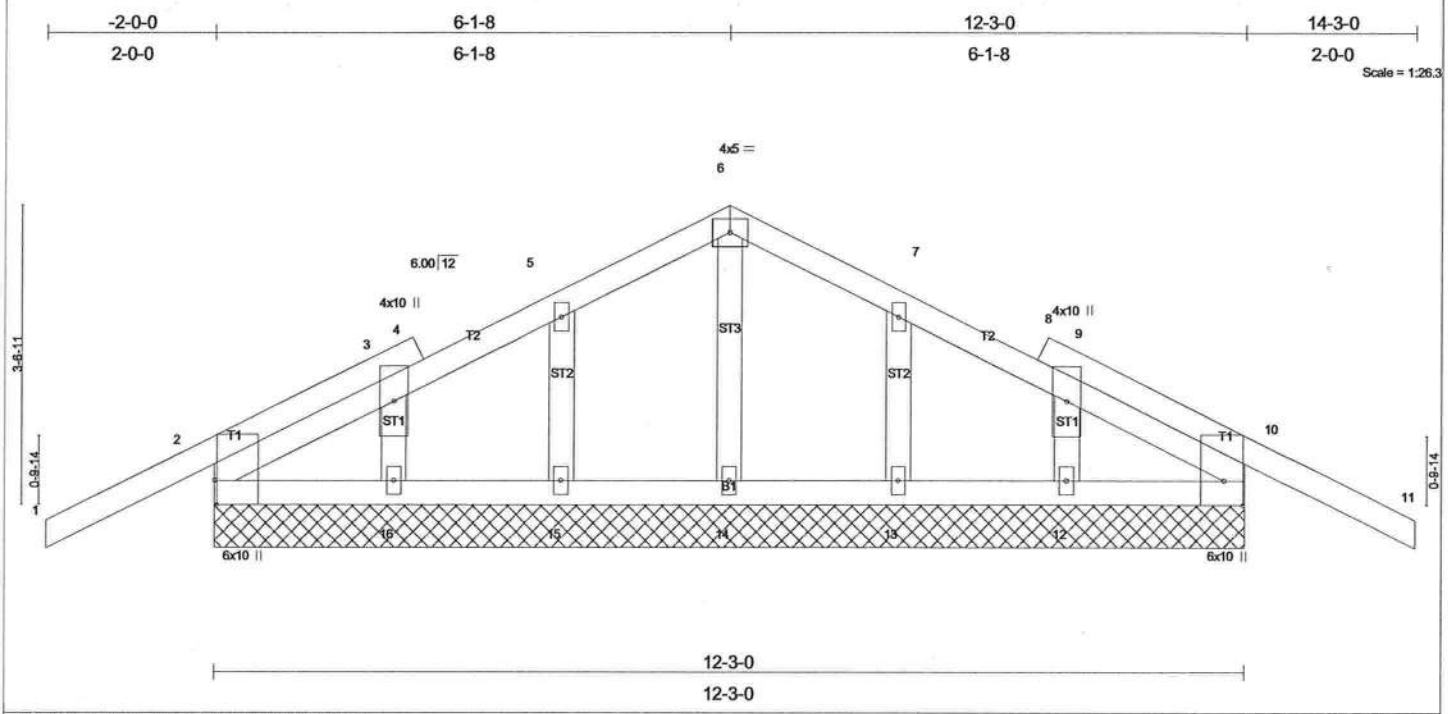

Plate Offsets (X,Y): [2:0-3-8,Edge], [10:0-3-8,Edge]

LOADING (psf)		SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0		Plates Increase 1.25	TC 0.49	Vert(LL) -0.04 11 n/r 120	MT20 244/190
TCDL 7.0		Lumber Increase 1.25	BC 0.08	Vert(TL) -0.07 11 n/r 90	
BCLL 10.0	*	Rep Stress Incr NO	WB 0.05	Horz(TL) 0.00 10 n/a n/a	
BCDL 5.0		Code FBC2004/TPI2002	(Matrix)		Weight: 67 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=463/12-3-0, 10=463/12-3-0, 14=229/12-3-0, 15=266/12-3-0, 16=144/12-3-0, 13=266/12-3-0, 12=144/12-3-0
Max Horz 2=-65(load case 7)
Max Uplift2=-241(load case 6), 10=-257(load case 7), 14=-23(load case 6), 15=-131(load case 6), 16=-56(load case 7), 13=-131(load case 7), 12=-59(load case 6)
Max Grav 2=463(load case 1), 10=463(load case 1), 14=229(load case 1), 15=270(load case 10), 16=144(load case 1), 13=270(load case 11), 12=144(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-15/54, 2-3=-119/49, 3-4=-21/52, 4-5=-11/62, 5-6=-63/131, 6-7=-63/131, 7-8=-11/52, 8-9=-21/40, 9-10=-119/49, 10-11=-15/54
BOT CHORD 2-16=0/90, 15-16=0/90, 14-15=0/90, 13-14=0/90, 12-13=0/90, 10-12=0/90
WEBS 6-14=-213/71, 5-15=-242/209, 3-16=-154/126, 7-13=-242/209, 9-12=-154/126

NOTES (10)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 241 lb uplift at joint 2, 257 lb uplift at joint 10, 23 lb uplift at joint 14, 131 lb uplift at joint 15, 56 lb uplift at joint 16, 131 lb uplift at joint 13 and 59 lb uplift at joint 12.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-114(F=-60), 6-11=-114(F=-60), 2-10=-10

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T05	POLYNESIAN	12	1	Job Reference (optional)

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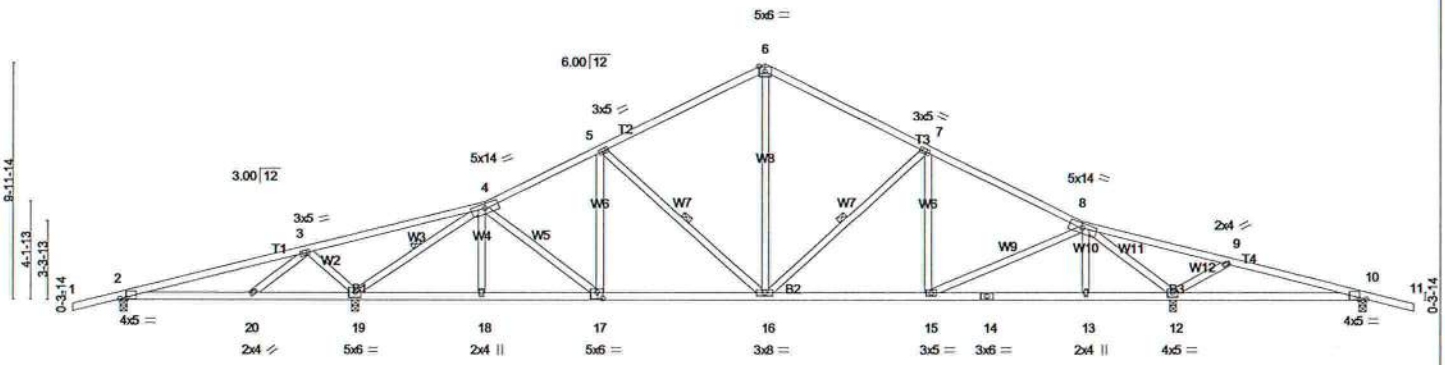
2-0-0	7-10-11	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	46-4-9	52-4-0	54-4-0
2-0-0	7-10-11	7-5-3	4-9-6	6-10-11	6-10-11	6-5-6	6-0-8	5-11-7	2-0-0

Scale = 1:93.6
Camber = 1/16 in



5-7-1	9-9-12	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	44-2-4	52-4-0
5-7-1	4-2-11	5-6-3	4-9-6	6-10-11	6-10-11	6-5-6	3-10-3	8-1-12

Plate Offsets (X,Y): [2:0-3-4,0-0-2], [10:0-3-4,0-0-2], [17:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	0.15	10-12	>636	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	-0.16	16-17	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.05	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 283 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-19, 5-16, 7-16

REACTIONS (lb/size) 2=276/0-3-8, 19=1562/0-3-8, 12=1519/0-3-8, 10=205/0-3-8
Max Horz 2=-132(load case 7)
Max Uplift2=-267(load case 4), 19=-521(load case 6), 12=-449(load case 7), 10=-246(load case 5)
Max Grav 2=302(load case 10), 19=1562(load case 1), 12=1519(load case 1), 10=237(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-6/204, 3-4=-444/692, 4-5=-1237/741, 5-6=-1026/702, 6-7=-1029/705, 7-8=-1273/748, 8-9=-382/768, 9-10=-115/364, 10-11=0/25
BOT CHORD 2-20=-145/0, 19-20=-249/191, 18-19=-330/963, 17-18=-332/961, 16-17=-361/1054, 15-16=-387/1071, 14-15=-297/815, 13-14=-297/815, 12-13=-292/816, 10-12=-311/176
WEBS 3-20=-304/237, 3-19=-566/585, 4-19=-1940/1151, 4-18=0/136, 4-17=-37/124, 5-17=-17/113, 5-16=-340/248, 6-16=-311/475, 7-16=-364/282, 7-15=-46/123, 8-15=-100/282, 8-13=0/115, 8-12=-1934/1028, 9-12=-463/400

NOTES (5)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 267 lb uplift at joint 2, 521 lb uplift at joint 19, 449 lb uplift at joint 12 and 246 lb uplift at joint 10.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.	J1970496
L279547	T05G	GABLE	1	1	Job Reference (optional)	

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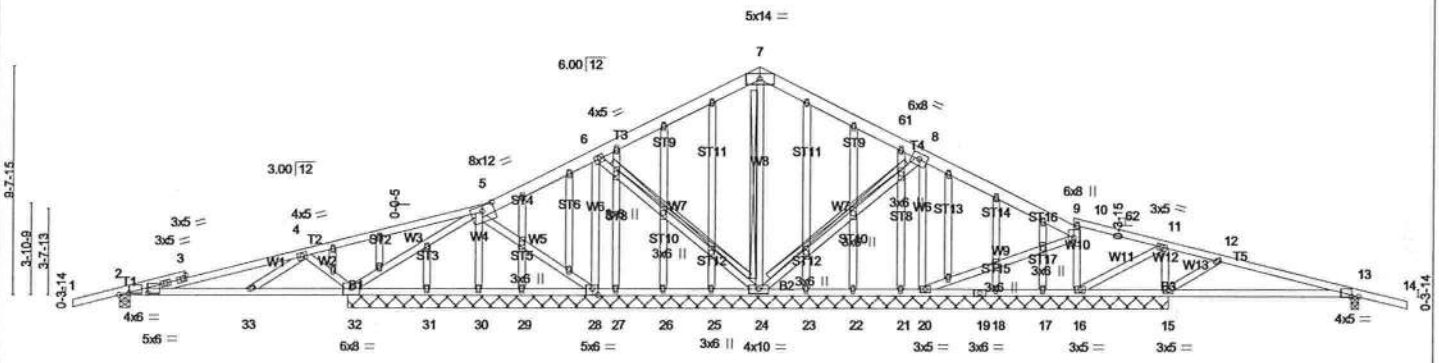
2-0-0	7-10-11	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	46-4-9	52-4-0	54-4-0
2-0-0	7-10-11	7-5-3	4-9-6	6-10-11	6-10-11	6-5-6	6-0-8	5-11-7	2-0-0

Scale = 1:93.6
Camber = 1/16 in



5-7-1	9-9-12	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	44-2-4	52-4-0
5-7-1	4-2-11	5-6-3	4-9-6	6-10-11	6-10-11	6-5-6	3-10-3	8-1-12

Plate Offsets (X,Y): [2:0-5-4,0-0-12], [2:1-2-8,0-0-12], [5:0-6-0,0-2-14], [13:0-3-7,0-0-2], [28:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	0.17	13-15	>573	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.15	13-15	>662	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.58	Horz(TL)	0.01	15	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 401 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D *Except*
T2 2 X 4 SYP No.1D, T5 2 X 4 SYP No.2, T1 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 6-24, 7-24, 8-24
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=690/0-5-8, 32=1197/34-8-0, 30=656/34-8-0, 28=482/34-8-0, 24=990/34-8-0, 20=775/34-8-0, 16=605/34-8-0, 15=567/34-8-0, 13=342/0-3-8, 25=20/34-8-0, 26=22/34-8-0, 27=9/34-8-0, 29=25/34-8-0, 31=17/34-8-0, 23=20/34-8-0, 22=22/34-8-0, 21=7/34-8-0, 18=27/34-8-0, 17=24/34-8-0
Max Horz 2=-170(load case 7)
Max Uplift 2=-641(load case 4), 32=-1020(load case 4), 30=-485(load case 6), 28=-373(load case 6), 24=-703(load case 6), 20=-602(load case 7), 16=-452(load case 7), 15=-417(load case 5), 13=-328(load case 5), 17=-30(load case 6)
Max Grav 2=690(load case 10), 32=1197(load case 10), 30=656(load case 1), 28=493(load case 10), 24=990(load case 1), 20=790(load case 11), 16=605(load case 11), 15=567(load case 11), 13=342(load case 11), 25=59(load case 2), 26=65(load case 2), 27=27(load case 2), 29=78(load case 2), 31=72(load case 2), 23=59(load case 2), 22=65(load case 2), 21=21(load case 2), 18=81(load case 2), 17=64(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-30/53, 2-3=-237/356, 3-4=-203/323, 4-5=-985/992, 5-6=-124/191, 6-7=-144/232, 7-61=0/196, 8-61=-182/273, 8-9=-281/301, 10-62=-25/44, 11-62=-66/57, 11-12=-36/118, 12-13=-191/148, 13-14=0/25
BOT CHORD 2-33=-217/197, 32-33=-42/141, 31-32=-205/356, 30-31=-205/356, 29-30=-178/331, 28-29=-178/331, 27-28=-18/207, 26-27=-18/207, 25-26=-18/207, 24-25=-18/207, 23-24=0/142, 22-23=0/142, 21-22=0/142, 20-21=0/142, 19-20=0/119, 18-19=0/119, 17-18=0/119, 16-17=0/119, 15-16=-100/162, 13-15=-65/147
WEBS 4-33=-361/291, 4-32=-1171/1306, 5-32=-787/879, 5-30=-637/603, 5-28=-142/210, 6-28=-594/547, 6-24=-37/145, 7-24=-811/661, 8-24=-218/331, 8-20=-806/802, 9-20=-58/121, 9-16=-673/699, 9-10=-192/193, 11-16=-37/142, 12-15=-291/267, 11-15=-333/239

NOTES (9)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 641 lb uplift at joint 2, 1020 lb uplift at joint 32, 485 lb uplift at joint 30, 373 lb uplift at joint 28, 703 lb uplift at joint 24, 602 lb uplift at joint 20, 452 lb uplift at joint 16, 417 lb uplift at joint 15, 328 lb uplift at joint 13 and 30 lb uplift at joint 17.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-7=-114(F=-60), 7-61=-114(F=-60), 9-61=-141(F=-87), 10-62=-141(F=-87), 14-62=-54, 2-13=-10

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T06	POLYNESIAN	1	1	Job Reference (optional)

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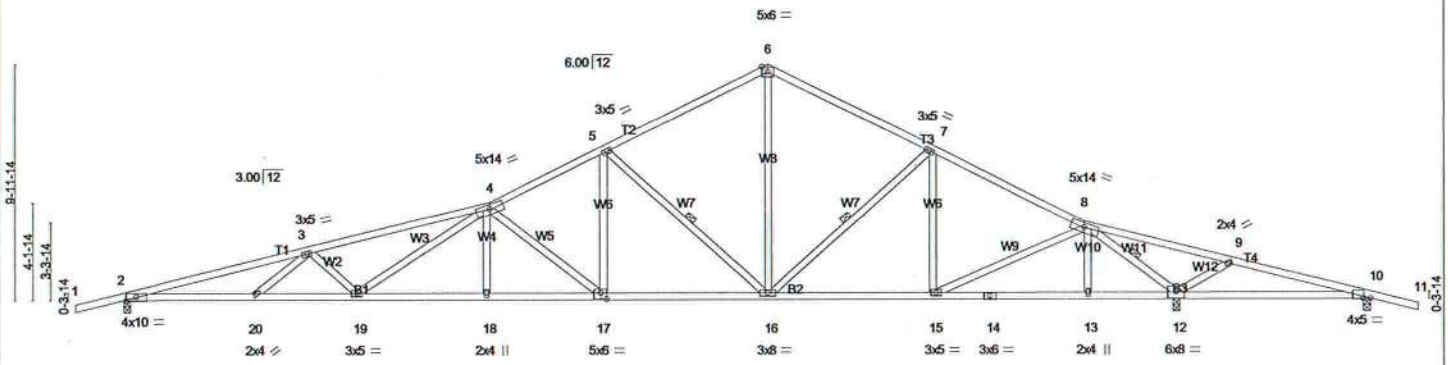
-2-0-0	7-10-11	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	46-4-9	52-4-0	54-4-0
2-0-0	7-10-11	7-5-3	4-9-6	6-10-11	6-10-11	6-5-6	6-0-8	5-11-7	2-0-0

Scale = 1:93.6
Camber = 3/8 in



5-7-1	9-9-12	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	44-2-4	52-4-0
5-7-1	4-2-11	5-6-3	4-9-6	6-10-11	6-10-11	6-5-6	3-10-3	8-1-12

Plate Offsets (X,Y): [10:0-3-7,0-0-2], [17:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	0.54	18-19	>972	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.84	18-19	>630	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.97	Horz(TL)	0.18	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 283 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-1-7 oc bracing.
WEBS 1 Row at midpt 5-16, 7-16, 8-12

REACTIONS (lb/size) 2=1439/0-3-8, 12=2224/0-3-8, 10=-101/0-3-8
Max Horz 2=-132(load case 7)
Max Uplift2=-409(load case 6), 12=-544(load case 7), 10=-240(load case 10)
Max Grav 2=1439(load case 1), 12=2224(load case 1), 10=85(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-4433/2410, 3-4=-4040/2310, 4-5=-2533/1515, 5-6=-1534/1008, 6-7=-1537/1012, 7-8=-1568/934, 8-9=-1056/1970, 9-10=-777/1544, 10-11=0/25
BOT CHORD 2-20=-2237/4245, 19-20=-2295/4210, 18-19=-1733/3315, 17-18=-1734/3313, 16-17=-1043/2196, 15-16=-553/1334, 14-15=-138/544, 13-14=-138/544, 12-13=-134/545, 10-12=-1451/815
WEBS 3-20=0/197, 3-19=-481/346, 4-19=-381/686, 4-18=0/145, 4-17=-1425/883, 5-17=-525/966, 5-16=-1204/798, 6-16=-583/908, 7-16=-236/179, 7-15=-313/227, 8-15=-458/912, 8-13=0/108, 8-12=-3029/1659, 9-12=-496/418

NOTES (5)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 409 lb uplift at joint 2, 544 lb uplift at joint 12 and 240 lb uplift at joint 10.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T07	POLYNESIAN	2	1	Job Reference (optional)

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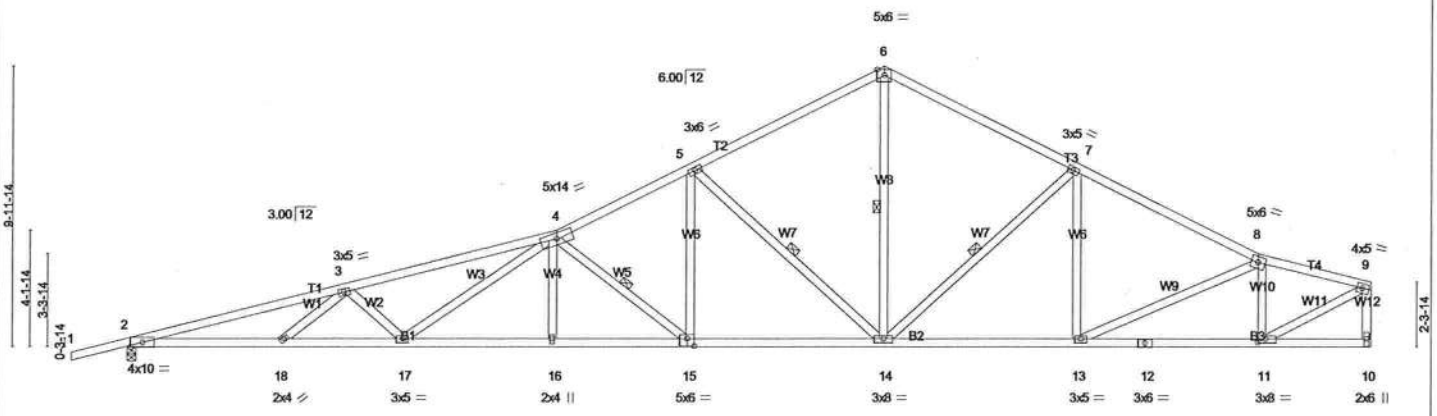
-2-0-0	7-10-11	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	44-4-0
2-0-0	7-10-11	7-5-3	4-9-6	6-10-11	6-10-11	6-5-6	3-11-15

Scale = 1:79.0
Camber = 7/16 in



5-7-1	9-9-12	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	44-4-0
5-7-1	4-2-11	5-6-3	4-9-6	6-10-11	6-10-11	6-5-6	3-11-15

Plate Offsets (X,Y): [11:0-3-8,0-1-8], [15:0-3-0,0-3-0]

LOADING (psf)		SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0		Plates Increase 1.25	TC 0.80	Vert(LL) 0.61 16-17 >863 360	MT20 244/190
TCDL 7.0		Lumber Increase 1.25	BC 0.94	Vert(TL) -0.92 16-17 >575 240	
BCLL 10.0	*	Rep Stress Incr YES	WB 0.59	Horz(TL) 0.21 10 n/a n/a	
BCDL 5.0		Code FBC2004/TPI2002	(Matrix)		Weight: 253 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 4-15, 5-14, 6-14, 7-14

REACTIONS (lb/size) 2=1528/0-3-8, 10=1407/Mechanical
Max Horz 2=180(load case 6)
Max Uplift2=-429(load case 6), 10=-282(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-4779/2607, 3-4=-4395/2514, 4-5=-2851/1698, 5-6=-1809/1167, 6-7=-1813/1171, 7-8=-2102/1240, 8-9=-1706/984, 9-10=-1379/800
BOT CHORD 2-18=-2572/4579, 17-18=-2633/4549, 16-17=-2077/3664, 15-16=-2078/3662, 14-15=-1350/2480, 13-14=-972/1815, 12-13=-960/1678, 11-12=-960/1678, 10-11=-21/41
WEBS 3-18=0/193, 3-17=-476/341, 4-17=-378/682, 4-16=0/145, 4-15=-1509/931, 5-15=-555/1019, 5-14=-1254/826, 6-14=-726/1156, 7-14=-432/317, 7-13=-10/161, 8-13=-27/188, 8-11=-852/554, 9-11=-1044/1826

NOTES (6-7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 429 lb uplift at joint 2 and 282 lb uplift at joint 10.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
7) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T08	POLYNESIAN	4	1	Job Reference (optional)

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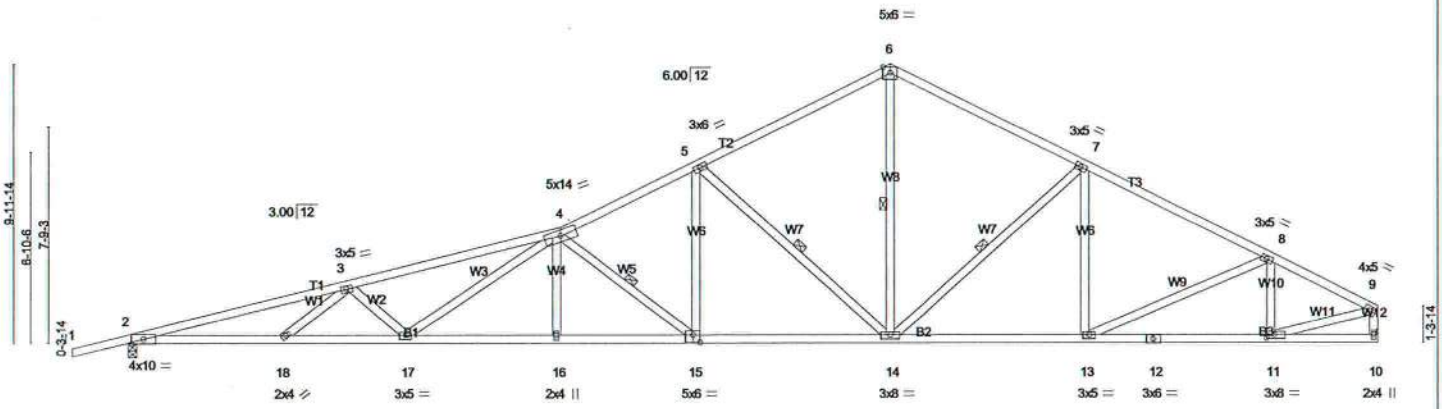
-2-0-0	7-10-11	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	44-4-0
2-0-0	7-10-11	7-5-3	4-9-6	6-10-11	6-10-11	6-5-6	3-11-15

Scale = 1:79.0
Camber = 7/16 in



5-7-1	9-9-12	15-3-15	20-1-5	27-0-0	33-10-11	40-4-1	44-4-0
5-7-1	4-2-11	5-6-3	4-9-6	6-10-11	6-10-11	6-5-6	3-11-15

Plate Offsets (X,Y): [11:0-3-8,0-1-8], [15:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.80	Vert(LL)	0.61	16-17	>869	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-0.92	16-17	>576	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.21	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 252 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 4-15, 5-14, 6-14, 7-14

REACTIONS (lb/size) 2=1528/0-3-8, 10=1407/Mechanical
Max Horz 2=160(load case 6)
Max Uplift2=-430(load case 6), 10=-281(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-4779/2615, 3-4=-4395/2521, 4-5=-2852/1705, 5-6=-1809/1172, 6-7=-1814/1176, 7-8=-2105/1250, 8-9=-1900/1088, 9-10=-1376/796
BOT CHORD 2-18=-2529/4579, 17-18=-2590/4549, 16-17=-2034/3664, 15-16=-2035/3662, 14-15=-1306/2480, 13-14=-933/1816, 12-13=-931/1668, 11-12=-931/1668, 10-11=-55/91
WEBS 3-18=0/193, 3-17=-476/341, 4-17=-378/682, 4-16=0/145, 4-15=-1509/932, 5-15=-556/1019, 5-14=-1254/828, 6-14=-732/1157, 7-14=-434/325, 7-13=-10/161, 8-13=-23/196, 8-11=-429/305, 9-11=-914/1643

NOTES (6-7)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 430 lb uplift at joint 2 and 281 lb uplift at joint 10.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
7) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	NORTON - DAMON RES.
L279547	T08G	GABLE	1	1	Job Reference (optional)

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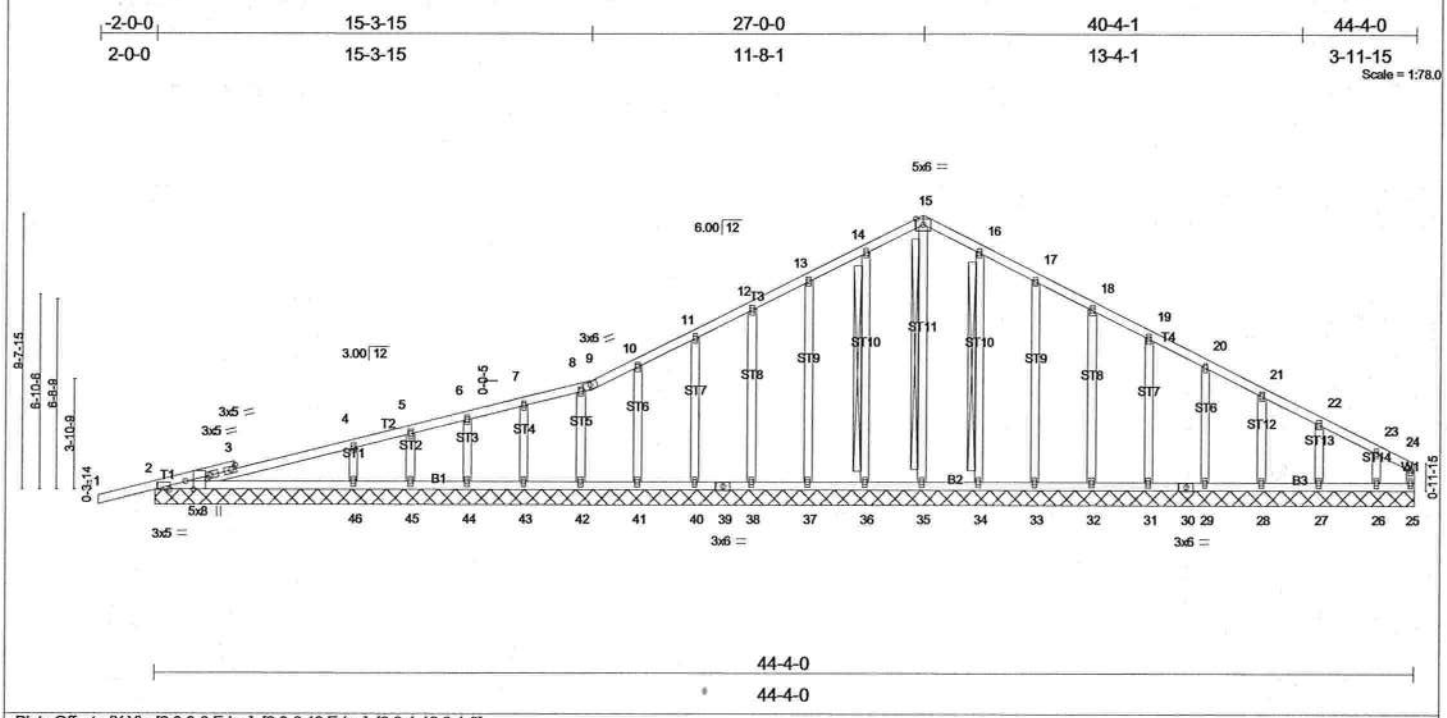


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-6-12,Edge], [3:0-1-12,0-1-8]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.22	Vert(TL)	0.03	1	n/r	90		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.10	Horz(TL)	0.01	25	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 275 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 15-35, 14-36, 16-34
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. ,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=303/44-4-0, 25=-24/44-4-0, 35=161/44-4-0, 36=131/44-4-0, 37=127/44-4-0, 38=128/44-4-0, 40=129/44-4-0, 41=125/44-4-0, 42=123/44-4-0, 43=118/44-4-0, 44=174/44-4-0, 45=-56/44-4-0, 46=455/44-4-0, 34=131/44-4-0, 33=127/44-4-0, 32=128/44-4-0, 31=128/44-4-0, 29=128/44-4-0, 28=128/44-4-0, 27=127/44-4-0, 26=146/44-4-0
Max Horz 2=186(load case 6)
Max Uplift 2=-211(load case 4), 25=-43(load case 10), 36=-73(load case 6), 37=-92(load case 6), 38=-85(load case 6), 40=-87(load case 6), 41=-83(load case 6), 42=-63(load case 6), 43=-67(load case 4), 44=-87(load case 4), 45=-56(load case 1), 46=-213(load case 4), 34=-69(load case 7), 33=-93(load case 7), 32=-85(load case 7), 31=-87(load case 7), 29=-86(load case 7), 28=-90(load case 7), 27=-74(load case 7), 26=-189(load case 7)
Max Grav 2=303(load case 1), 25=151(load case 7), 35=168(load case 7), 36=135(load case 10), 37=127(load case 1), 38=128(load case 10), 40=129(load case 10), 41=125(load case 1), 42=123(load case 1), 43=118(load case 10), 44=174(load case 1), 45=14(load case 7), 46=455(load case 1), 34=133(load case 11), 33=127(load case 11), 32=128(load case 1), 31=128(load case 1), 29=128(load case 11), 28=128(load case 1), 27=130(load case 11), 26=146(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/25, 2-3=-207/38, 3-4=-199/90, 4-5=-138/33, 5-6=-132/62, 6-7=-104/56, 7-8=-84/71, 8-9=-71/87, 9-10=-68/100, 10-11=-41/138, 11-12=-9/173, 12-13=0/209, 13-14=0/265, 14-15=0/312, 15-16=-16/312, 16-17=-14/265, 17-18=-14/205, 18-19=-14/148, 19-20=-14/91, 20-21=-14/66, 21-22=-46/66, 22-23=-94/67, 23-24=-160/67, 24-25=-94/30
BOT CHORD 2-46=-41/132, 45-46=-41/132, 44-45=-41/132, 43-44=-41/132, 42-43=-41/132, 41-42=-41/132, 40-41=-41/132, 39-40=-41/132, 38-39=-41/132, 37-38=-41/132, 36-37=-41/132, 35-36=-41/132, 34-35=-41/132, 33-34=-41/132, 32-33=-41/132, 31-32=-41/132, 30-31=-41/132, 29-30=-41/132, 28-29=-41/132, 27-28=-41/132, 26-27=-41/132, 25-26=-41/132
WEBS 15-35=-166/0, 14-36=-115/88, 13-37=-107/118, 12-38=-108/109, 11-40=-109/111, 10-41=-105/111, 8-42=-102/101, 7-43=-101/85, 6-44=-140/112, 5-45=-6/28, 4-46=-356/254, 16-34=-113/88, 17-33=-107/118, 18-32=-108/109, 19-31=-108/111, 20-29=-108/110, 21-28=-108/111, 22-27=-111/107, 23-26=-110/148

NOTES (9)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 211 lb uplift at joint 2, 43 lb uplift at joint 25, 73 lb uplift at joint 36 , 92 lb uplift at joint 37, 85 lb uplift at joint 38, 87 lb uplift at joint 40, 83 lb uplift at joint 41, 63 lb uplift at joint 42, 67 lb uplift at joint 43, 87 lb uplift at joint 44, 56 lb uplift at joint 45, 213 lb uplift at joint 46, 69 lb uplift at joint 34, 93 lb uplift at joint 33, 85 lb uplift at joint 32, 87 lb uplift at joint 31, 86 lb uplift at joint 29, 90 lb uplift at joint 28, 74 lb uplift at joint 27 and 189 lb uplift at joint 26.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

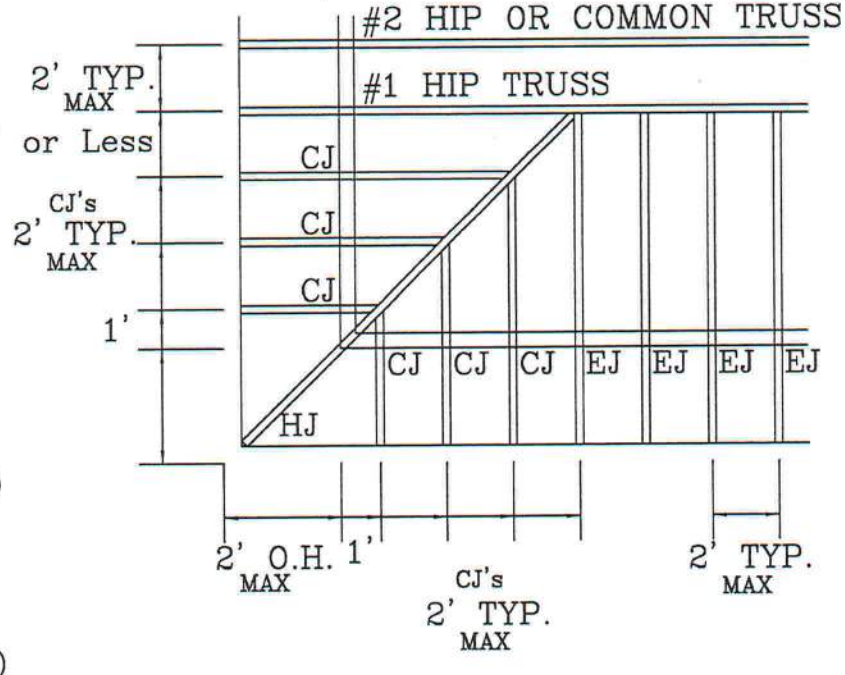
LOAD CASE(S) Standard

STEPDOWN CORNER SET

TOP CHORD 2X4 SO. PINE #2 or Better
BOT CHORD 2X4 SO. PINE #2 or Better
WEBS 2X4 SO. PINE #3 or Better

120 MPH MAX

Setback 7' or Less



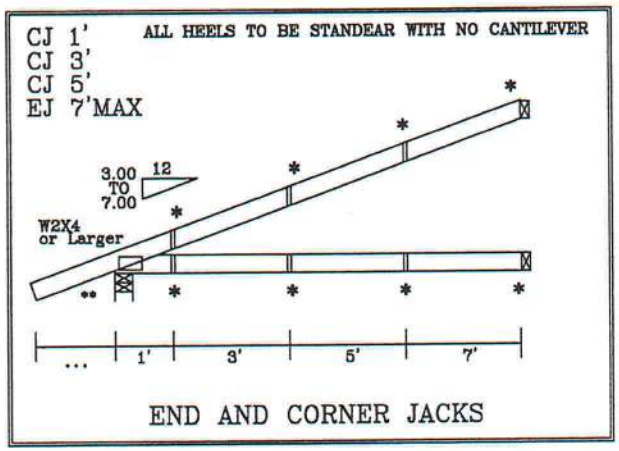
PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT=28 FT. ENCLOSED. (ASCE 7-02)

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED. TILE
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 15.0 PSF TOTAL DEAD LOAD. WIND SPEED=120 "C" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

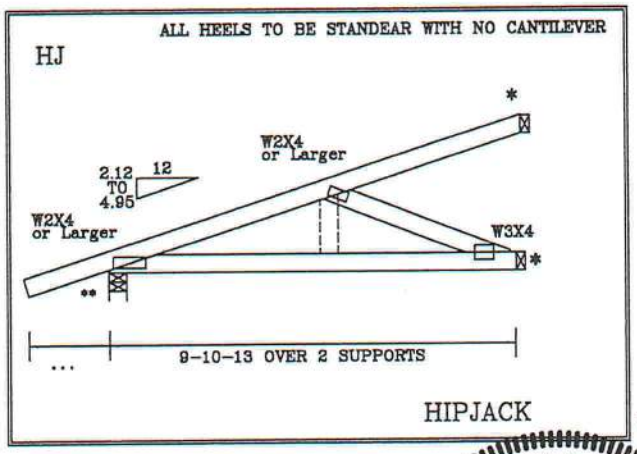
PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.
UPLIFT: 400# or Less
BRG LOC: *
UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND SPEED=120 "B" MPH. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)

* (3) 16d TOENAILS

** SEE EOR FOR TIE DOWN



END AND CORNER JACKS



HIPJACK

UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED
BC LIVE LOAD IS NON CONCURRENT 10*

SHINGLE	TC LL	20	MAX PSF
	TC DL	7	MAX PSF
	BC LL	10*	MAX PSF
	BC DL	5	MAX PSF
TILE	TC LL	20	MAX PSF
	TC DL	20	MAX PSF
	BC LL	10*	MAX PSF
	BC DL	5	MAX PSF
DUR. FAC.		1.25	
SPACING		2' MAX	

CORNER SET SETBACK
7'0" MAX

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/S,K) ASTM A653 GRADE 40/60 (W,K/H,S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

JULIUS LEE P.E.
CONS. ENGINEERS, P.A.
1485 SW 4TH AVENUE
DELRAY BEACH, FL 33444-2101
NO. 54869
STATE OF FLORIDA
PROFESSIONAL ENGINEER

REF	7'MAX STBK CS
DATE	Jun./27/2008
DRWG	
-ENG	

REVIEWED
By julius lee at 10:52 am, Jun 27, 2008

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 6"	11' 6"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	6' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	5' 0"	6' 8"	6' 8"	8' 3"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 8"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 8"	12' 6"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 6"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 6"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 6"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	6' 11"	6' 11"	7' 10"	7' 10"	10' 6"	10' 6"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 6"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

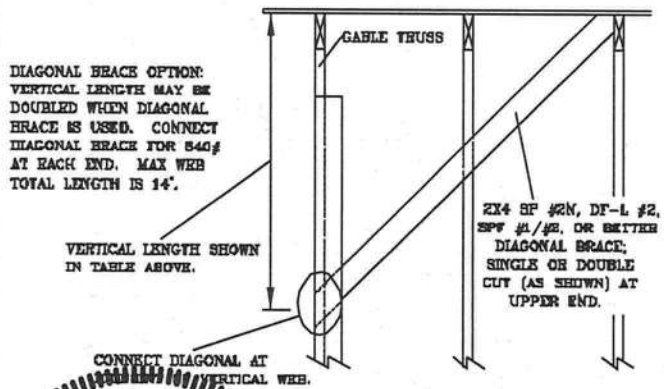
SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

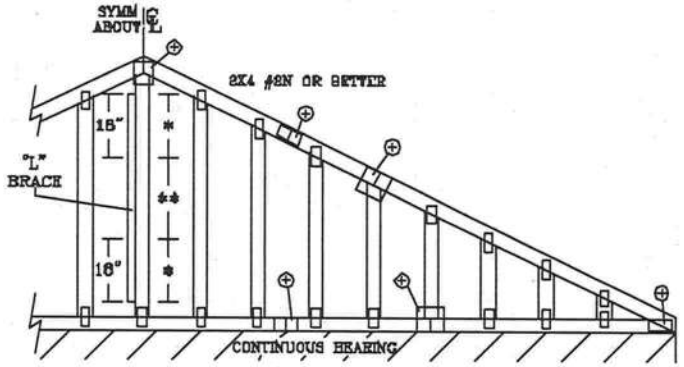
DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE	DOUGLAS FIR-LARCH
#1	#1
#2	#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR, SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB13015
DATE	11/26/03
DRWG	MITEK STD GABLE 15 E HT
	-ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	6' 5"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	6' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 6"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 5"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 8"
		STUD	3' 3"	4' 6"	4' 6"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 8"
		STANDARD	3' 0"	3' 10"	3' 10"	5' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 6"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 6"	4' 6"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 6"	6' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	6' 7"	7' 4"	7' 4"	8' 11"	9' 6"	11' 5"	11' 6"	14' 0"	14' 0"
		STUD	3' 8"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 6"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 6"	8' 3"	6' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 5"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 5"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

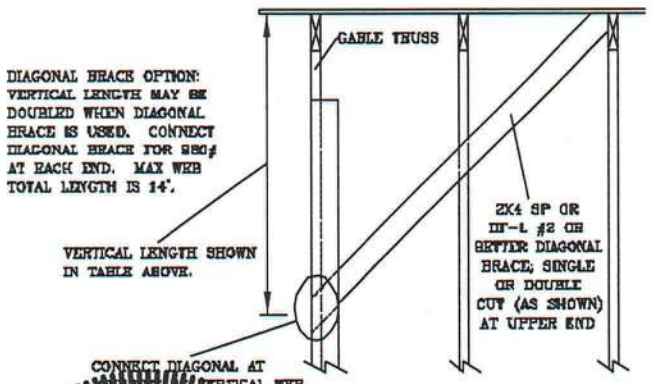
ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

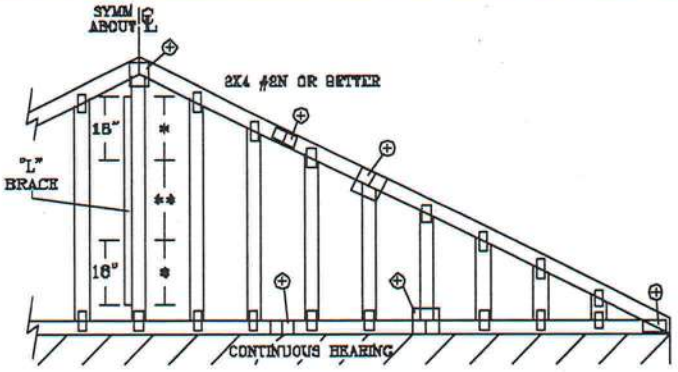
GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.



DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 980# AT EACH END. MAX WEB TOTAL LENGTH IS 14'.



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By julius lee at 12:00 pm, Jun 11, 2008

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

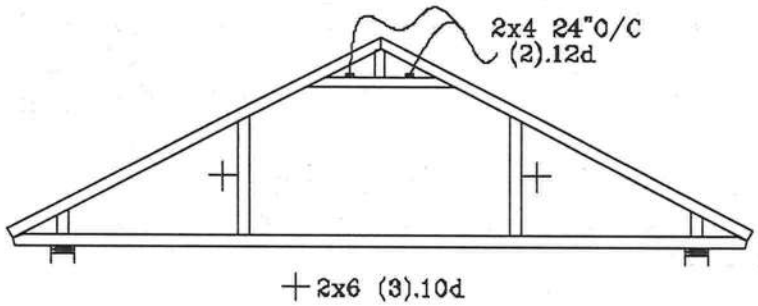
JULIUS LEE'S CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

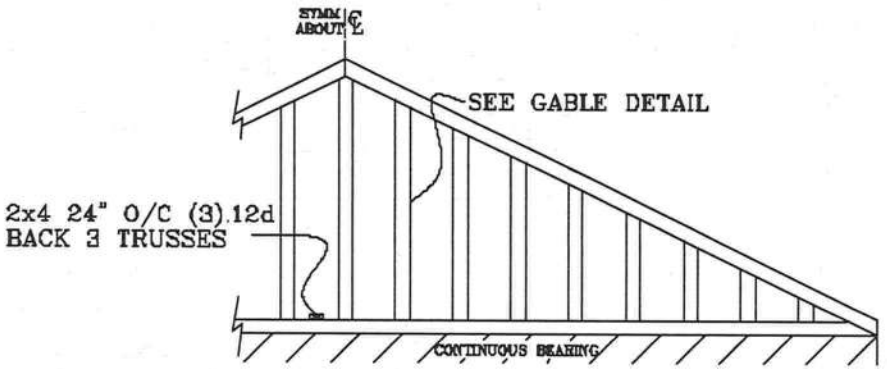
MAX. SPACING 24.0"

REF ASCE7-02-GAB13030
DATE 11/26/03
DWG MITEK STD GABLE 50' E N7
-ENG

TYPICAL ATTIC TRUSS BRACING

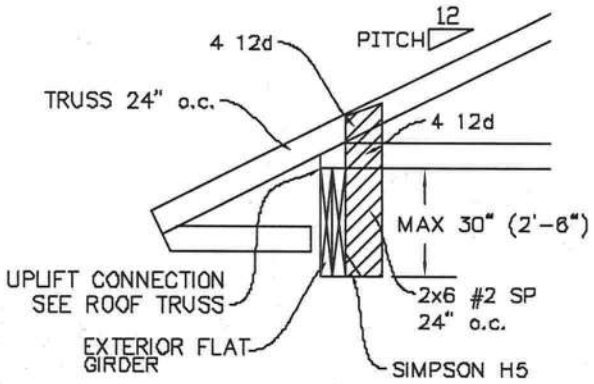


GABLE END TRUSS DETAIL

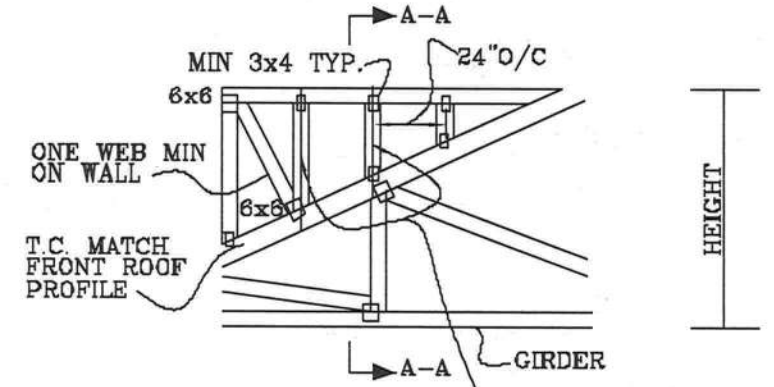


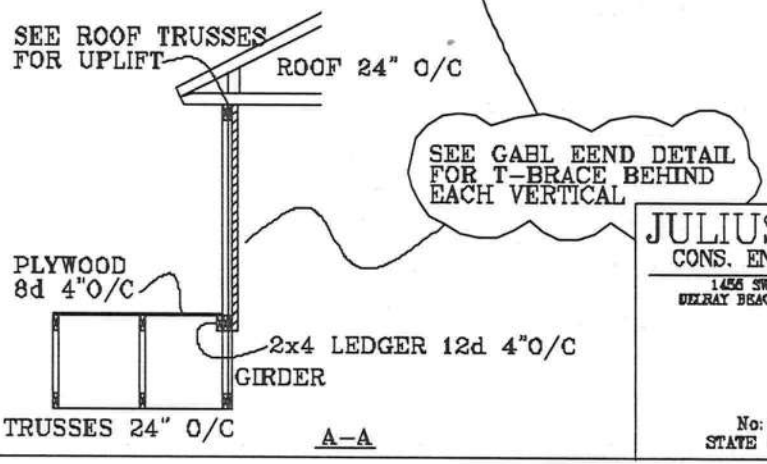
MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL





REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE
LICENSE
NO. 34869
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

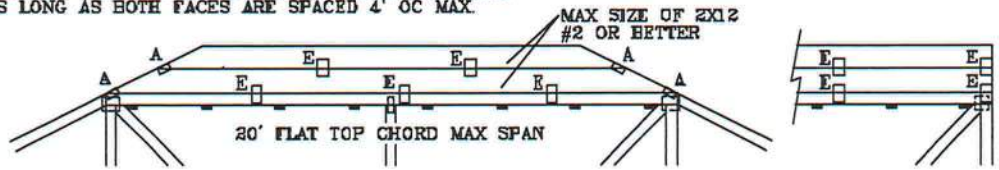
110 MPH WIND, 30' MEAN HGT, FBC
ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL=5 PSF, WIND BC DL=5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

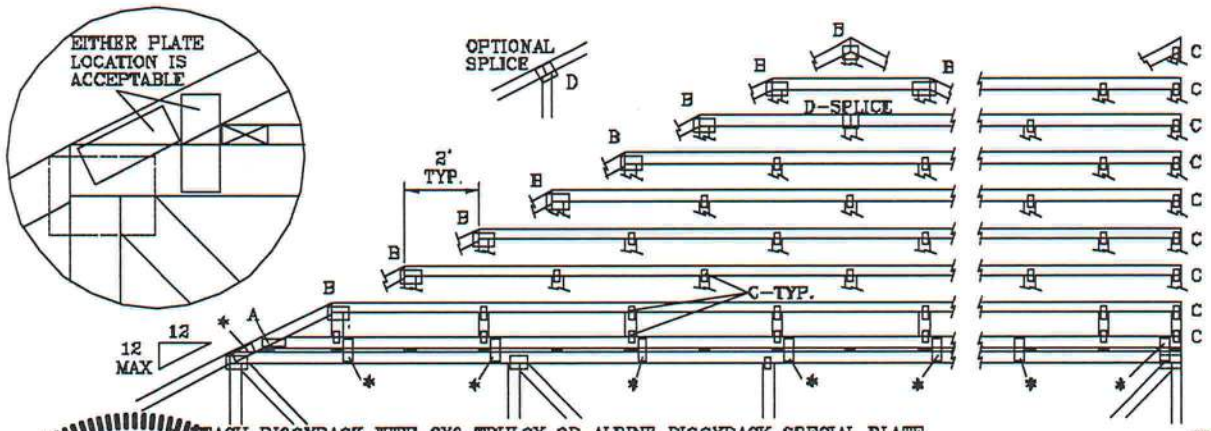
FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X8	5X8	5X8	5X8
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X8 OR 3X8 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (6) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.



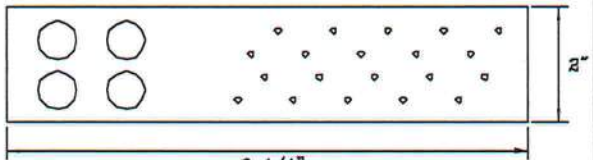
WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.



ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1466 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.

SPACING 24.0"

REF PIGGYBACK
DATE 09/12/07
DRWG MITEK STD PIGGY
-ENG JL

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

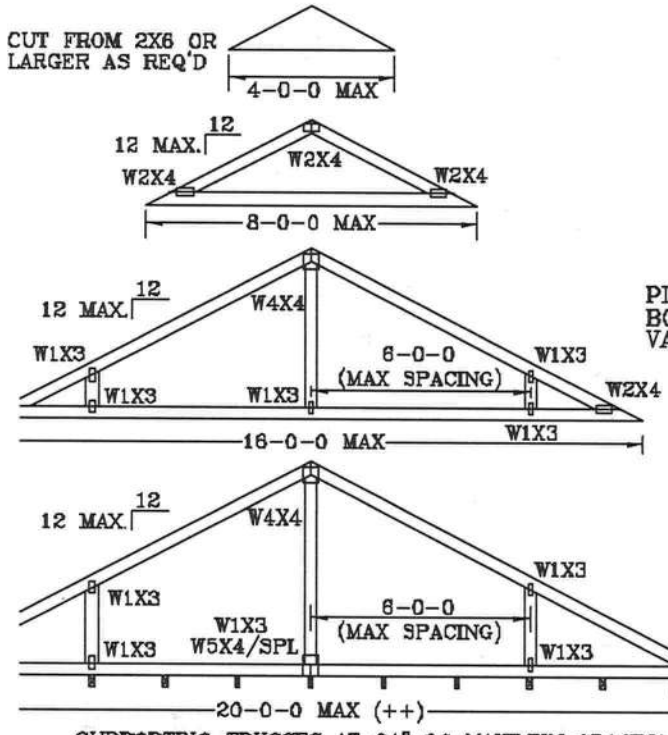
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

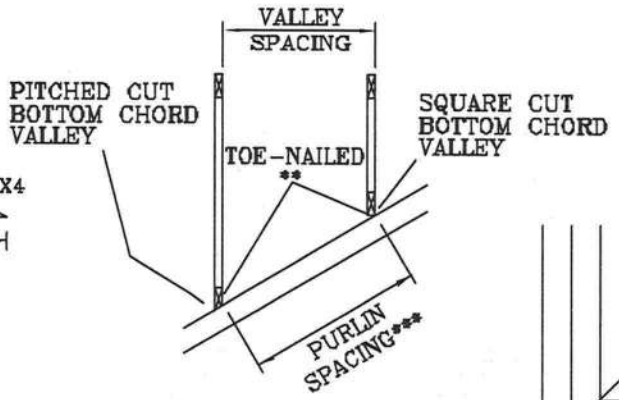
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

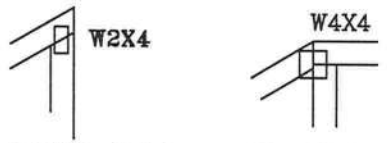
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

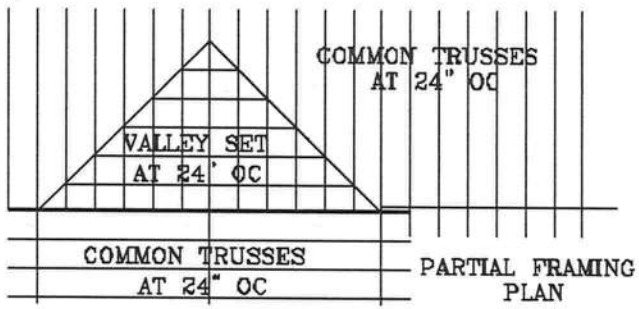
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.







THIS DRAWING REPLACES DRAWING A105

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVIEWED
By julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF VALLEY DETAIL
TC DL	7	15	PSF	DATE 11/26/03
BC DL	5	5	PSF	DRWG VALTRUSS1103
BC LL	0	0	PSF	-ENG JL
TOT. LD.	32	40	PSF	
DUR.FAC. 1.25		1.25		
SPACING		24"		

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

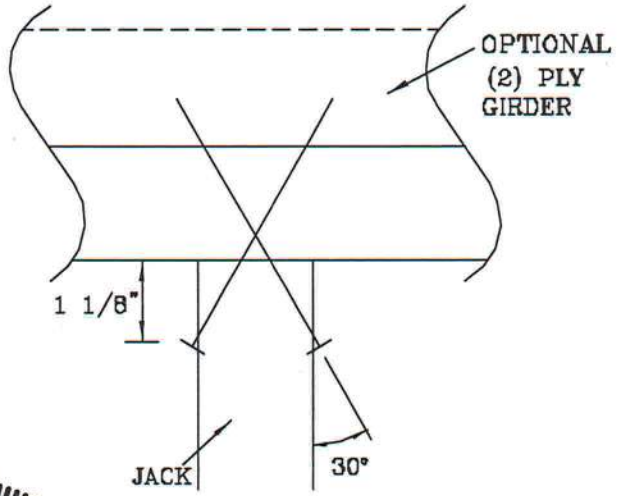
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

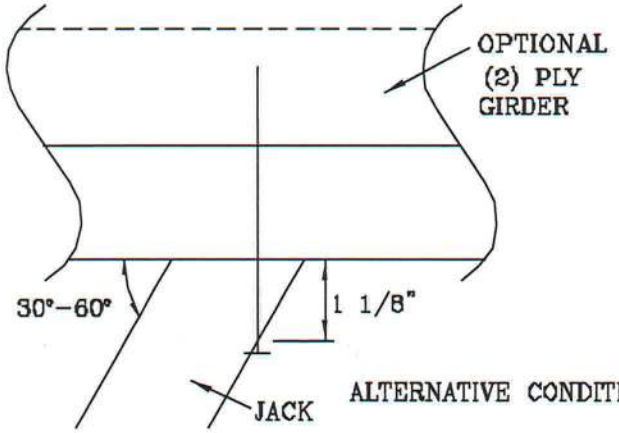
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

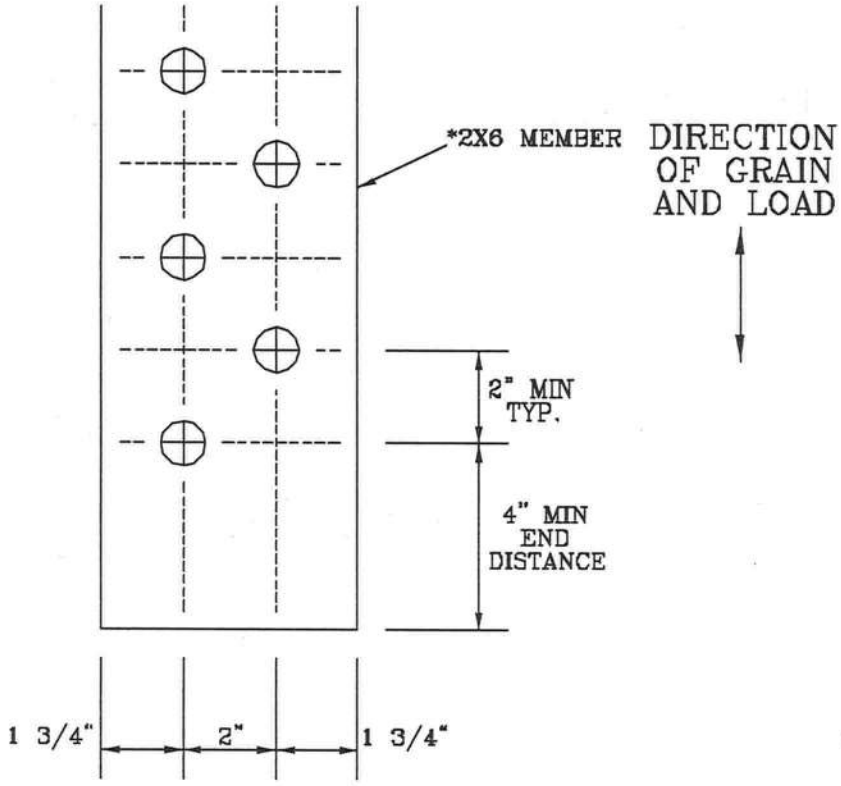
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

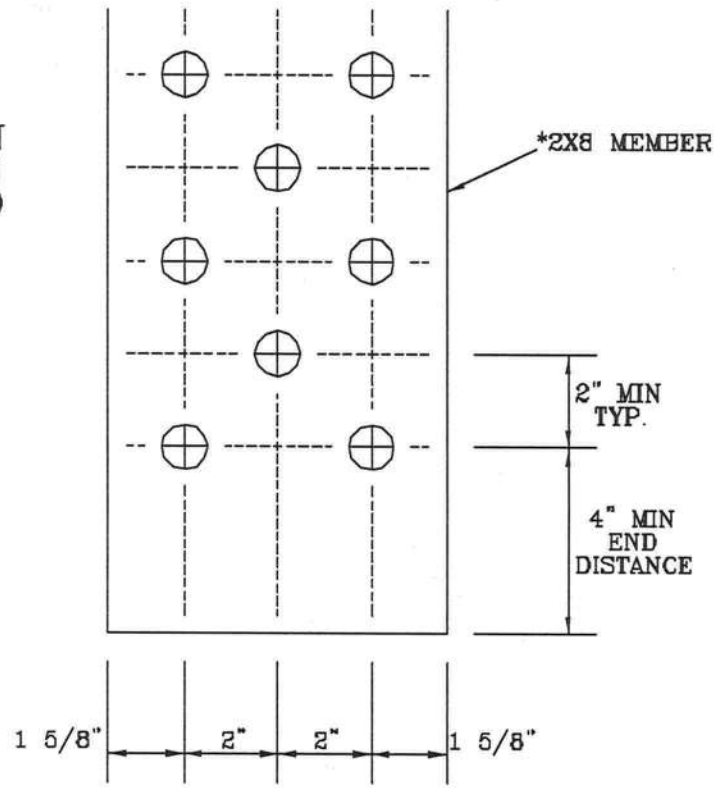
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016

JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

"WARNING" TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF BOLT SPACING
TC DL	PSF	DATE 11/26/03
BC DL	PSF	DRWG CNBOLTSP1103
BC LL	PSF	-ENG JL
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

TRULOX CONNECTION DETAIL

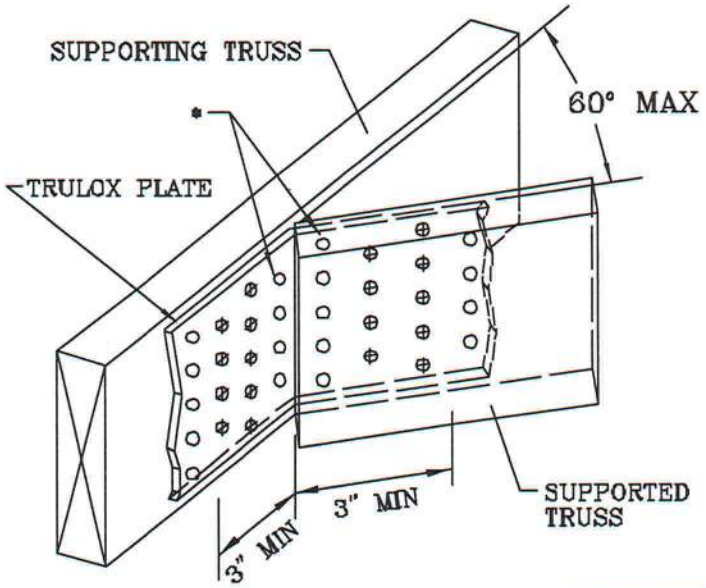
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

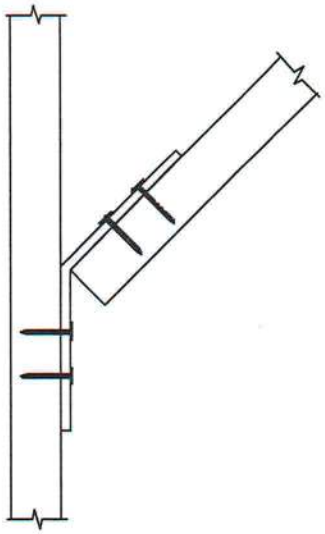
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

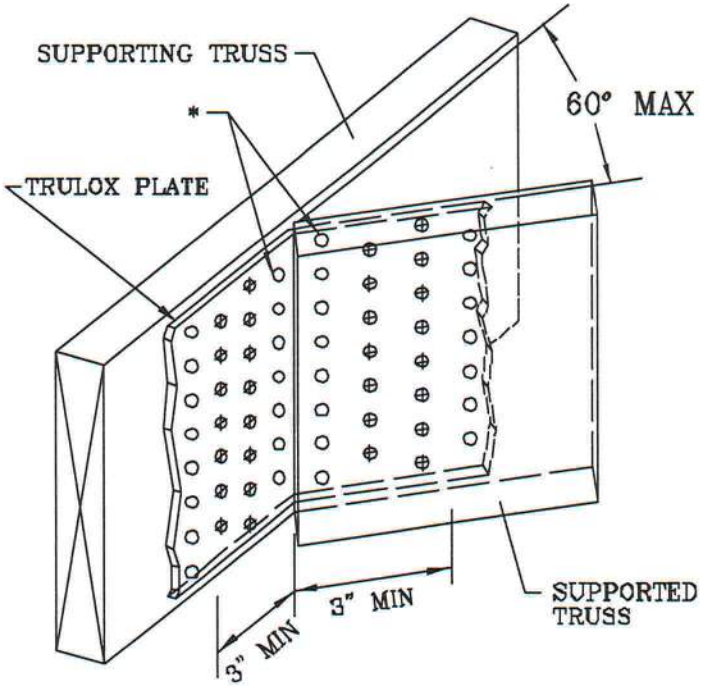
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE





MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

REVIEWED
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JULIUS LEE LICENSE NO. 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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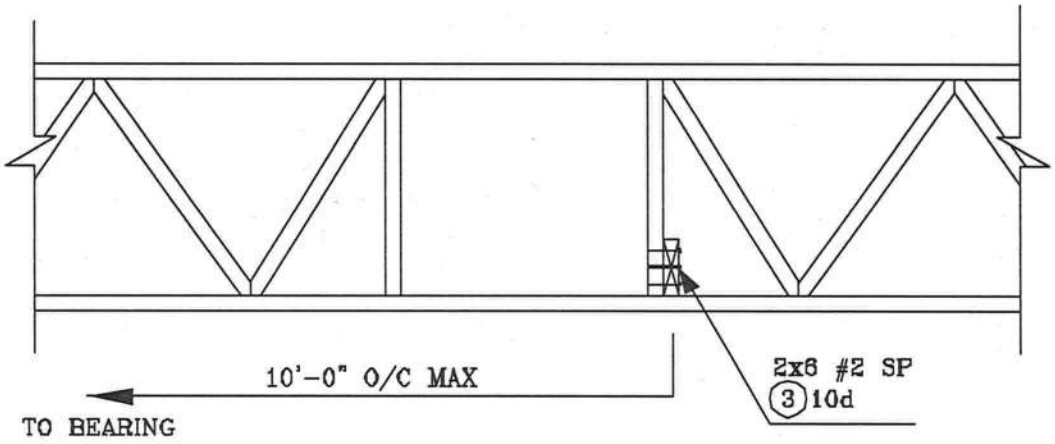
DELRAY BEACH, FL 33444-2161

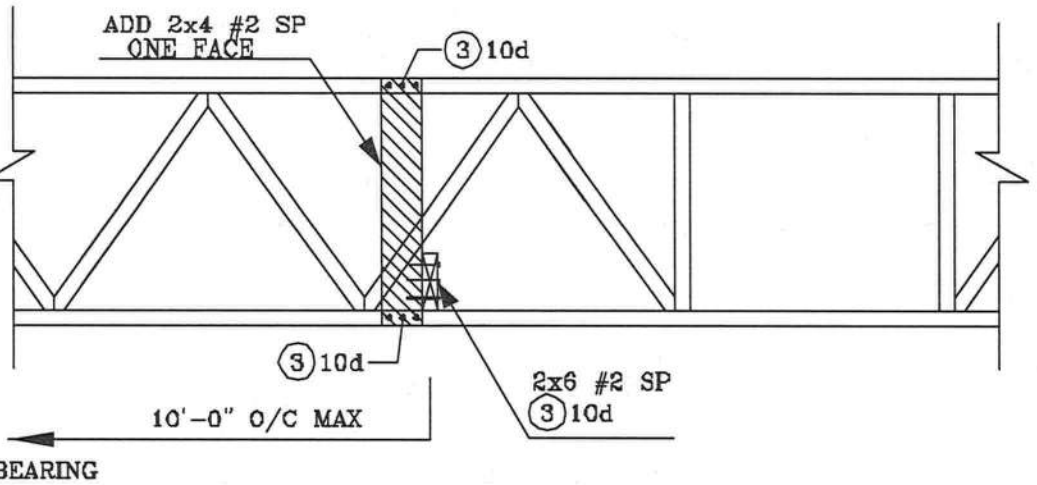
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

STRONG BACK DETAIL
SYSTEM–42 OR FLAT TRUSS



ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP



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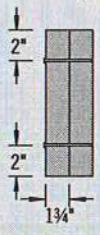
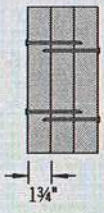
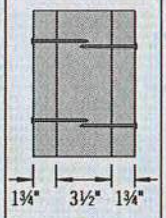
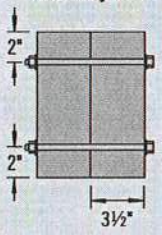
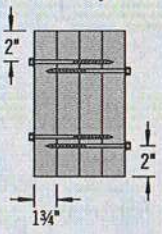
1456 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A 	Assembly B 	Assembly C 	Assembly D 	Assembly E 	Assembly F 
			3½" 2-ply	5¼" 3-ply	5¼" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail[1]	2	12"	370	***280***	280	***245***		
	3	12"	555	***415***	415	***370***		
½" A307 Through Bolts[2][4]	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS ¼" x 3½"[4]	2	24"	680	***510***	510	***455***		
		19.2"	850	***640***	640	***565***		
		16"	1,020	***765***	765	***680***		
SDS ¼" x 6"[3][4]	2	24"				***455***	***465***	***455***
		19.2"				***565***	***580***	***565***
		16"				***680***	***695***	***680***
USP WS35 [4]	2	24"	480	***360***	360	***320***		
		19.2"	600	***450***	450	***400***		
		16"	715	***540***	540	***480***		
USP WS6 [3][4]	2	24"				***350***	***525***	***350***
		19.2"				***440***	***660***	***440***
		16"				***525***	***790***	***525***
3⅛" TrussLok[4]	2	24"	635	***475***	475	***425***		
		19.2"	795	***595***	595	***530***		
		16"	955	***715***	715	***635***		
5" TrussLok[4]	2	24"		***500***	500	***445***	***480***	***445***
		19.2"		***625***	625	***555***	***600***	***555***
		16"		***750***	750	***665***	***725***	***665***
6¾" TrussLok[4]	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 9⁄16" maximum.

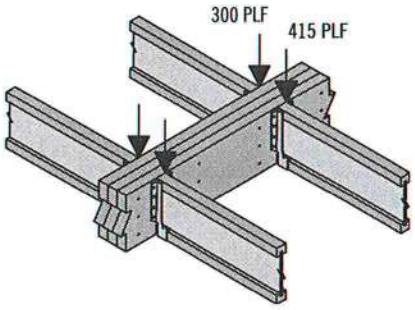
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- ***Bold Italic*** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by ½ the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example

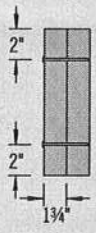
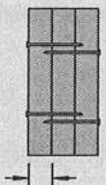
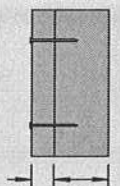
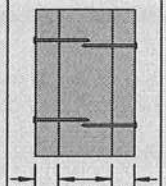
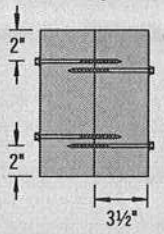
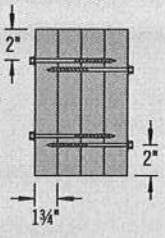


First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1¾" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:
Two rows of ½" bolts or SDS ¼" x 3½" screws at 19.2" on-center.

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern: Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
							
		3½" 2-ply	5¼" 3-ply	5¼" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	*835*	835	*740*		
	12	2,225	*1,670*	1,670	*1,485*		
	18	3,335	*2,505*	2,505	*2,225*		
	24	4,450	*3,335*	3,335	*2,965*		
SDS Screws ¼" x 3½" or WS35 ¼" x 6" or WS6(1)	4	1,915	1,435(4)	1,435	*1,275*	*1,860(2)*	*1,405(2)*
	6	2,870	2,150 (4)	2,150	*1,915*	*2,785(2)*	*2,110(2)*
	8	3,825	2,870 (4)	2,870	*2,550*	*3,715(2)*	*2,810(2)*
3⅜" or 5" TrussLok™	4	2,545	1,910 (4)	1,910	*1,695*	*1,925(3)*	*1,775(3)*
	6	3,815	2,860 (4)	2,860	*2,545*	*2,890(3)*	*2,665(3)*
	8	5,090	3,815 (4)	3,815	*3,390*	*3,855(3)*	*3,550(3)*

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

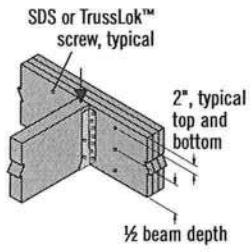
(2) 6" long screws required.

(3) 5" long screws required.

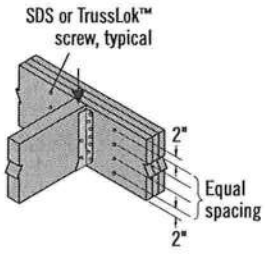
(4) 3½" and 3⅜" long screws must be installed on both sides.

Connections

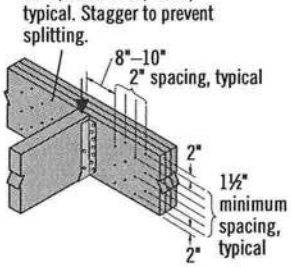
4 or 6 or Screw Connection



8 Screw Connection

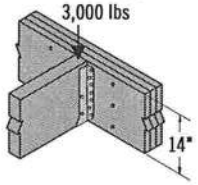


Nail Connection



There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1¾" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1¾" assembly, eight 3⅜" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

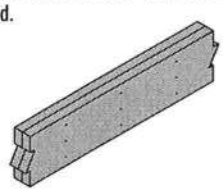
MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1¾" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3⅜" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3½" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by ½ of the required connector spacing.
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of ½" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"