This Permit Expires	One Year From the Date of Issue 000024834
APPLICANT ROBERT "BOB" MCINTOSH	PHONE 386.754.8678
ADDRESS 289 NW CORINTH DRIVE OWNER JAY & CATHLEEN HART	PHONE 386.752.9812 FL 32055
OWNER JAY & CATHLEEN HART ADDRESS 675 SW MORNING STAR GLEN	FORT WHITE FL 32038
CONTRACTOR MICHAEL DELAHOZ	PHONE 386.754.8678
	AR GLN,TL @ CHURCH AND IT'S THE 2ND BRICK
HOME ON L.	R GEN, IL @ CHURCH AND IT'S THE 2ND BRICK
TYPE DEVELOPMENT SCREEN POOL ENCL.	ESTIMATED COST OF CONSTRUCTION 9840.00
HEATED FLOOR AREA TO	TAL AREA HEIGHT 14.00 STORIES
FOUNDATION WALLS	ROOF PITCH FLOOR
LAND USE & ZONING A-3	MAX. HEIGHT 35
Minimum Set Back Requirments: STREET-FRONT	30.00 REAR 25.00 SIDE 25.00
NO. EX.D.U. 1 FLOOD ZONE XPP	DEVELOPMENT PERMIT NO.
PARCEL ID 34-5S-16-03752-109 SUB	DIVISION SHANNA MEADOWS
LOT 9 BLOCK PHASE	UNIT TOTAL ACRES
	JEK JTH N J & Zoning checked by Approved for Issuance New Resident
	Check # or Cash 3106
Temporary Power Foundatio	ZONING DEPARTMENT ONLY (footer/Slab) Monolithic
date/app. by	date/app. by date/app. by
Under slab rough-in plumbing	Slab Sheathing/Nailing
date/app. by	date/app. by date/app. by
Framing Rough-in plu	ambing above slab and below wood floor
Flectrical rough-in	date/app. by
date/app. by	Duct Peri. beam (Lintel) date/app. by
Permanent power C.O. Final	Culvert
date/app. by M/H tie downs, blocking, electricity and plumbing	date/app. by Pool
	date/app. by date/app. by
Reconnection Pump pole date/app. by	Utility Pole date/app. by
M/H Pole Travel Trailer	Re-roof
date/app. by	date/app. by date/app. by
BUILDING PERMIT FEE \$ CERTIFICAT	TION FEE \$ 0.00 SURCHARGE FEE \$ 0.00
MISC. FEES \$ 0.00 ZONING CERT. FEE \$	50.00 FIRE FEE \$ 0.00 WASTE FEE \$
LECOD DE VELOTIVIENT L'ESTA A L'INCODIZATION	E\$ CULVERT FEE\$ TOTAL FFF 100 00
INSPECTORS OFFICE	CLERKS OFFICE CULVERT FEE \$ TOTAL FEE100.00

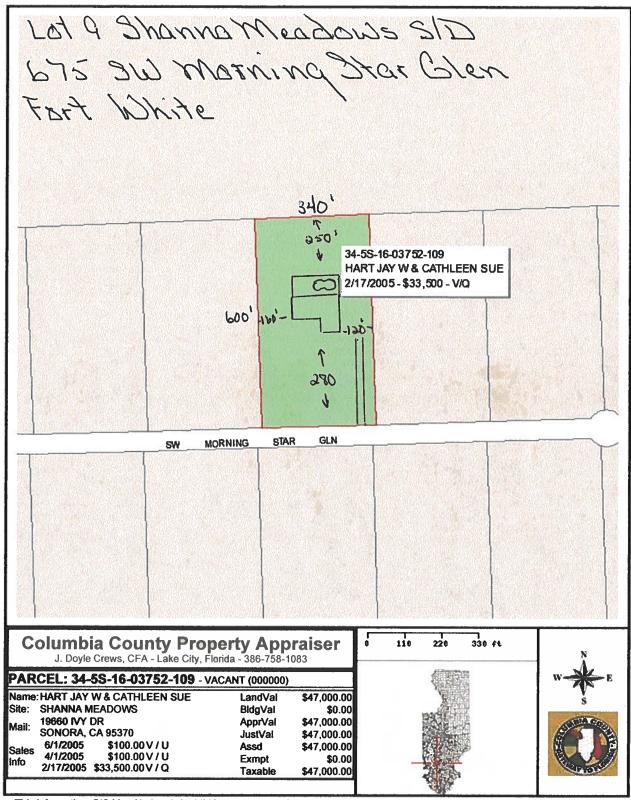
NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

For Office Use Only Application # 0607-59	Date Received 7-24% By LH Permit # 24834
Application Approved by - Zoning Official	Date 100 Diana Providence
Flood Zone Development Permit MA	Zoning A-3 Land Use Plan Map Category A-3
Comments Accessory Use	
Applicants Name West West which (F) Fo	10/ Enclosure Phone 386-754-8678
Address 289 NW Corinth Dr Lax	0 (, +1 +1 3) V = 280 - 124 - 80 18
Owners Name Hart, Jay & Cathlee	
911 Address 675 8W Morning Star C	Phone 152-9812
Contractors Name \ \\\ \\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	23 25 25 25 25 25 25 25 25 25 25 25 25 25
Address 289 NW Carinth Dr. Lake	Phone 386-754-8678
Fee Simple Owner Name & Address 1/1/14	2114.41 20022
Bonding Co. Name & Address V	
Architect/Engineer Name & Address Lawrence	F Bonnett DE 2d Honnet
Mortgage Lenders Name & Address N/N	17 mother divise
•	- <u>Clay Elec.</u> - <u>Suwannee Valley Elec.</u> - <u>Progressive Energy</u>
Property ID Number 34 - 58 - 16 - 03752 - 109	Estimated Cost of Construction Rogressive Energy
Subdivision Name Shanna Meadows	Estimated Cost of Construction 4 8 4 0 , 80
Driving Directions 2 47 11 4 14 4 14	Lot Block Unit Phase
Of Charch Sound print	turn lest on Morning Star Glen
at Chrisch, Second brick	House on Lot
Type of Construction Screen tool Embrante	Mumbar of Public Page 1
Total Acreage S Lot Size 34000 you need a	- Culvert Permit or Culvert Walver or Have an Existing Drive
residuice of siluctule from Fronenv lines . From	al XII i etala i i o i o o o o o o o o o
Total Building Height	
Monthes of Stories	Negled Floor Area Roof Pitch
	mit and that all work be performed to meet the standards of
OWNERS AFFIDAVIT: I hereby certify that all the foregoing compliance with all applicable laws and regulating constr	ucuon and zoning
WARNING TO OWNER! YOUR EAST LIRE TO DECORD A MA	
ENDER OR ATTORNEY BEFORE RECORDING YOUR NO	OTICE OF COMMENCMENT MAY RESULT IN YOU PAYING OU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR TICE OF COMMENCEMENT.
But I / C Ch	V 1/11 1/1/1/1/1/1/1/1/1/1/1/1/1/1/1/1/1
Owner Builder or Agent (including Contractor)	Contractor Signature
TATE OF FLORIDA	Contractors License Number SCC TXT 155C
COUNTY OF COLUMBIA	Competency Card Number
worn to (or affirmed) and subscribed before me	Commission # DD274404
nis day of 20	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
ersonally known or Produced Identification	Notary Signature
,	# 2 ve um 3106



This information, GIS Map Updated: 6/19/2006, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for advalorem assessment purposes.

NOTICE OF COMMENCEMENT FORM COLUMBIA COUNTY, FLORIDA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 34-59-16-03752-109

good survey pho	the property and street address or 911 address)
675 SW Morning	Star Can Fort White, F132038
2. General description of improvement:	seen Enclosure
3. Owner Name & Address Toy & Cal	Meen Hort
	her than owner): 1 1 12
5. Contractor Name Wike Delatos	Phone Number 386-754-8678
•	Phone Number
AddressAmount of Bond	
7. Lender Name \(\frac{1}{N}\) \(\frac{1}{N}\)	Phone Number
	ted by the Owner upon whom notices or other documents may be
NameN	Phone Number
9. In addition to himself/herself the owner de	signates FI PoolEnclosures of
289 NW Corinth Dr. Lake City to rece	ive a copy of the Lienor's Notice as provided in Section 713.13 (1) -
	ement (the expiration date is 1 (one) year from the date of recording,
(Unless a different date is specified)	
NOTICE AS PER CHAPTER 713, Florida Statute	
The owner must sign the notice of commencer	nent and no one else may be permitted to sign in his/her stead.
	Sworn to (or affirmed) and subscribed before day of, 2001
May (Has)	NOTARY STAMPINE Nadean G.S. McIntos

Inst:2006017496 Date:07/24/2006 Time:13:16
______DC,P.DeWitt Cason,Columbia County B:1090 P:1601

Expires November 14, 2008
Serated Tray Fain - Insurance, Inc. 200-345-7019

Signature of Notary

This Instrument was prepared by: Tania M. Vazquez P.O. Box 160 Fort White, FL 32038

Inst:2005013440 Date:06/07/2005 Time:13:55

Doc Stamp-Deed: 0.70

DC,P.DeWitt Cason,Columbia County B:1048 P:858

QUITCLAIM DEED

THIS QUITCLAIM DEED, Executed this 1st day of June 2005 by first party (Grantor), Kingdom Properties, Inc. as to 100% (one hundred percent interest) whose post office address is P.O. Box 160, Fort White, FL 32038 to second party (Grantees), Jay W Hart & Cathleen Sue Hart as to 100% (one hundred percent interest) whose address is 19660 Ivy Drive, Sonora, CA 95370.

WITNESSETH, That the said first party, for good consideration, maintenance, up kept and for the sum of \$10.00 paid by the said second party, the receipt whereof is hereby acknowledged, does hereby remise, release and quitclaim unto the said second party forever, all the right, title, interest and claim which the said first party has in and to the following described parcel of land, and improvements and appurtenances thereto in the County of Columbia, State of Florida to wit:

Lot 9, Shanna Meadows, according to the map or plat thereof recorded in Plat Book 7, Page 76-77, of the Public Records of Columbia County, Florida.

Parcel Number: 03752-109

Grantor(s) warrants that at the time of this conveyance, the subject property is not Grantor's (s) homestead within the meaning set forth in the state of Florida, nor is it contiguous to or a part of homestead property.

IN WITNESS WHEREOF, The said first party has signed and sealed these presents the day and year first above written.

Witness Name: Hogo Escalante

Witness Name: Marleen Escalante

STATE OF Florida COUNTY OF Dade

The foregoing instrument was acknowledged before me this the ______ day of _______ copy ______, who personally known to me or has produced _______ as identification and who did/did not take an oath.



Notary Signature)

Witness Name: Marken Escalante.

STATE OF Florida Country OF Dade

The foregoing instrument was acknowledged before me this the day of day of who personally known to me or has produced as identification and who did/did not take an oath.

Tania M. Vazquez Notary Signature.

Tania M. Vazquez Commission #DD308194 Expires: Apr 07, 2008 Bonded Thru Atlantic Bonding Co., Inc.

Notary Name (typed, printed or stamped)

Seal

My Commission Expires:

ISING FELORIDA, COUNTY OF COLUMBIA HEREAV ERTIFY, that the above and foregoing

a fue copy of the original filed in this office.

DEWINE BASON, CLERK OF COURTS

Deputy Clark

UNTOALONIN

June

January 01, 2006

LAWRENCE E. BENNETT, P.E. P.O. BOX 214368 SOUTH DAYTONA, FL 32121 386-767-4774

TO ALL BUILDING DEPARTMENTS

Re: Master File Engineering
"ALUMINUM STRUCTURES DESIGN MANUAL"
2004 edition & 2006 edition

Dear Building Official/Plans Examiner,

This is to certify that the following contractor/company is hereby authorized to use my 2004 ed "ALUMINUM STRUCTURES DESIGN MANUAL" during the year 2006. When we publish and distribute the 2006 ed of the "ALUMINUM STRUCTURES DESIGN MANUAL", they will be authorized to use that manual for the remainder of 2006.

Our authorization is based on a January to January basis requardless of the edition of the manual. This authorization also applies to contractor master file drawings, " ONE PERMIT ONLY" drawings or any "site specific" drawings that I may furnish the contractor.

Mike Delahoz AAF Mid Fl Florida Pool Enclosures Inc P.O. Box 521136 Longwood, FL 32752

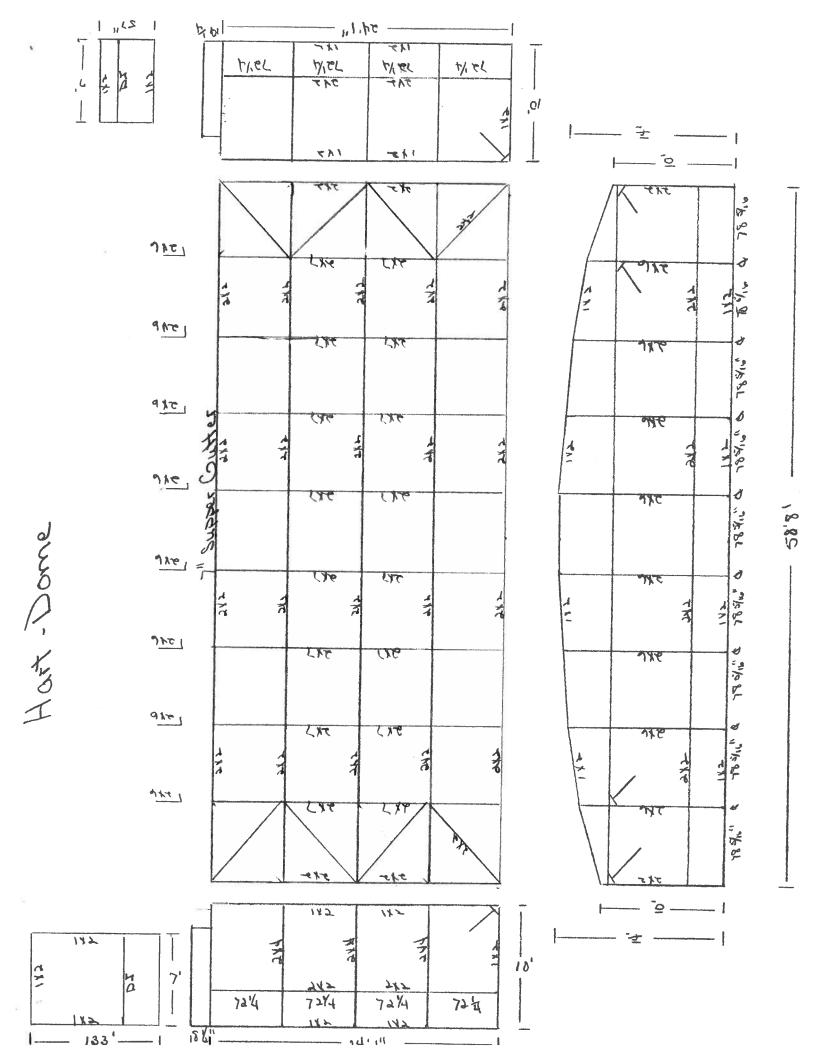
They are hereby added to my 2006 MASTERFILE LIST

Should you have any questions please contact me at your convenience.

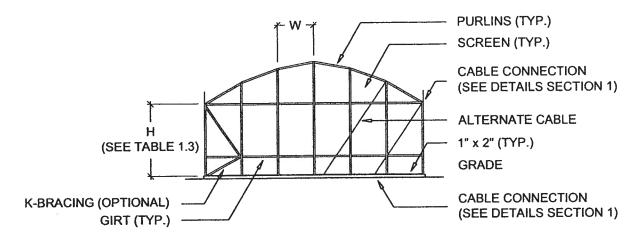
Sincerely,

Lawrence E. Bennett, P.E. #16644

Office copy

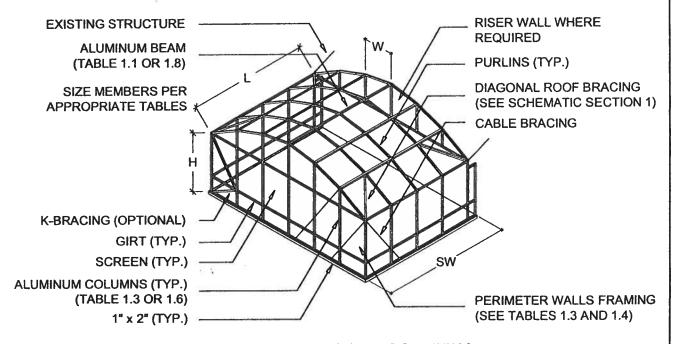


SECTION 1



TYPICAL DOME ROOF - ELEVATION

SCALE: N.T.S.



TYPICAL DOME ROOF - ISOMETRIC

SCALE: N.T.S.

CONNECTION DETAILS AND NOTES ARE FOUND IN THE SUBSEQUENT PAGES.

Lawrence E. Bennett, P.E. FL # 16644

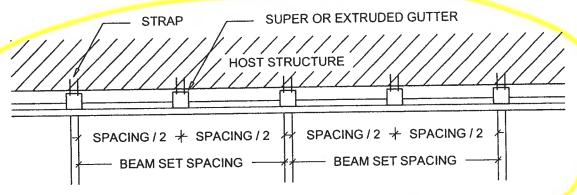
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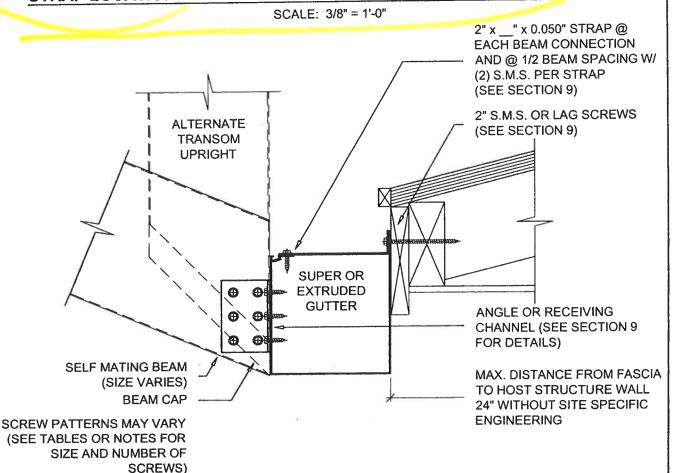
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PAGE

SECTION 1



STRAP LOCATION FOR SUPER OR EXTRUDED GUTTER REINFORCEMENT



SELF MATING BEAM CONNECTION TO SUPER OR EXTRUDED GUTTER

SCALE: 3" = 1'-0"

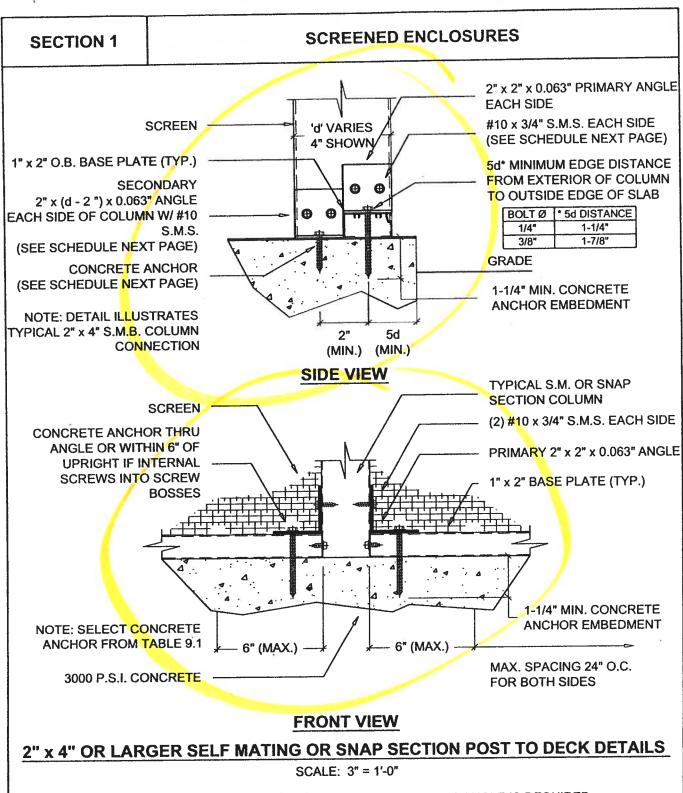
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NOTE: FOR SIDE WALLS OF 2" x 4" OR SMALLER ONLY ONE ANGLE IS REQUIRED.

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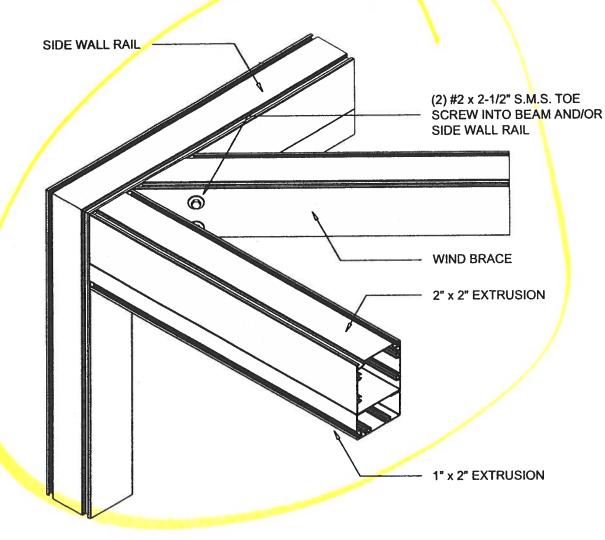
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SECTION 1

SCREENED ENCLOSURES



WIND BRACE CONNECTION DETAIL

SCALE: 3" = 1'-0"

NOTES

1. Wind bracing shall be provided at each side wall panel when enclosure projects more than (4) panels from host structure.

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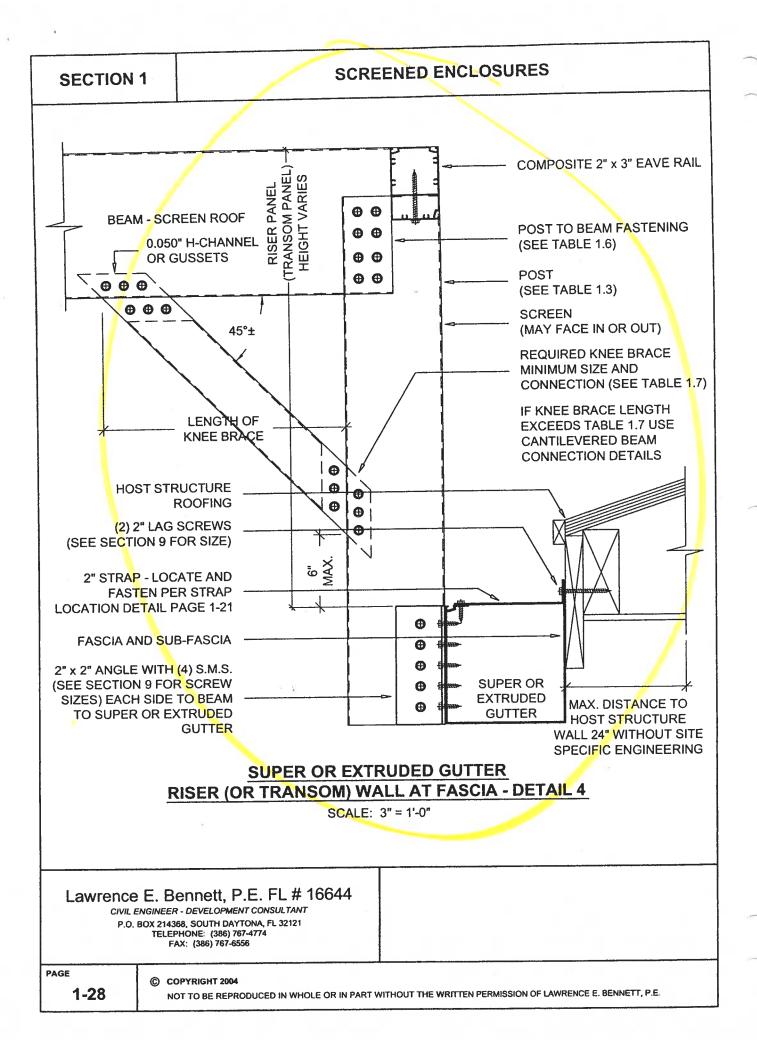
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PAGE

1-36

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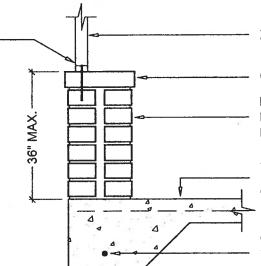
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SECTION 1

1/4" x 6" RAWL TAPPER THROUGH 1" x 2" AND **ROWLOCK INTO FIRST COURSE OF BRICKS**

ALTERNATE CONNECTION OF SCREENED ENCLOSURE FOR BRICK OR OTHER NON-STRUCTURAL KNEE WALL 1" WIDE x 0.063" THICK STRAP @ EACH POST FROM POST TO FOOTING W/ (2) #10 x 3/4" S.M.S. STRAP TO POST AND (1) 1/4" x 1-3/4" CONCRETE ANCHOR TO SLAB OR **FOOTING**



ALUMINUM FRAME SCREEN WALL

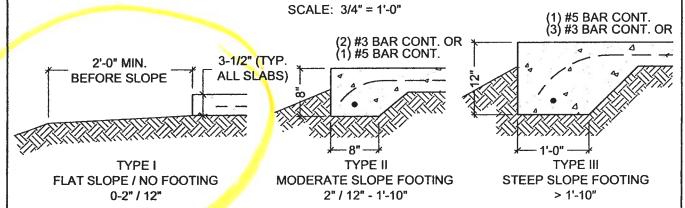
CAP BRICK

BRICK KNEEWALL TYPE 'S' MORTAR REQUIRED FOR LOAD BEARING BRICK WALL

4" (NOMINAL) PATIO **CONCRETE SLAB (SEE NOTES CONCERNING FIBER MESH)**

(3) #3Ø BARS OR (1) #5Ø BAR W/ 2-1/2" COVER (TYP.)

BRICK KNEEWALL AND FOUNDATION FOR SCREEN WALLS



Notes for all foundation types:

- 1. No footing required except when addressing erosion until the slab width in the direction of the primary exceeds 32 ft., then a Type II footing is required under the load bearing wall only unless the side wall exceeds 16 ft. in height or the enclosure is in a "C" exposure catagory in which case a Type II footing is required for all walls.
- 2. The foundations shown are based on a minimum soil bearing pressure of 1,500 PSF. Bearing capacity of soil shall be verified, using a pocket penetrometer, field soil test, or by a soil testing lab, to be above 1,500 PSF prior to placing the slab.
- 3. The slab / foundation shall be cleared of debris and roots and compacted prior to placement of concrete.
- 4. Monolithic slabs and footings shall be minimum 2,500 psi concrete with 6 x 6 10 x 10 welded wire mesh or crack control fiber mesh: Fibermesh ® Mesh, InForce™ e3™ (Formerly Fibermesh MD) per maufacturer's specification may be used in lieu of wire mesh.
- 5. If local building codes require a minimum footing, use Type II footing or footing sections required by local code. Local code governs.

SLAB-FOOTING DETAILS

SCALE: 3/4" = 1'-0"

Lawrence E. Bennett, P.E. FL # 16644

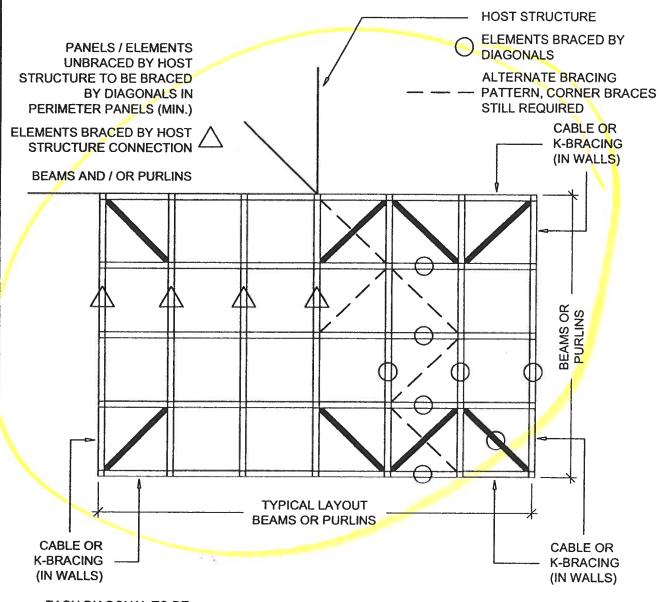
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1-55

SECTION 1



EACH DIAGONAL TO BE FASTENED EACH END W/ (2) EACH #10 S.M.S. (MIN.)

2 x 2 (MIN) ROOF DIAGONAL, MEET WALL AT WALL BRACING AT CORNERS (TYP.)

(POOL ENCLOSURE SCREEN ROOF MAY BE FLAT, GABLE, MANSARD, DOME, OR HIP)

POOL ENCLOSURE DIAGONAL BRACING - SCHEMATIC PLAN VIEW

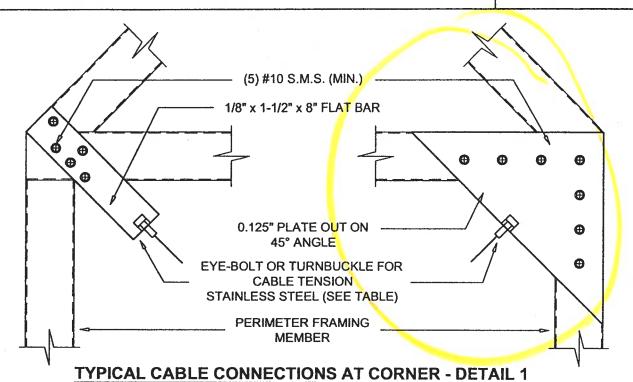
SCALE: 3/8" = 1'-0"

Lawrence E. Bennett, P.E. FL # 16644

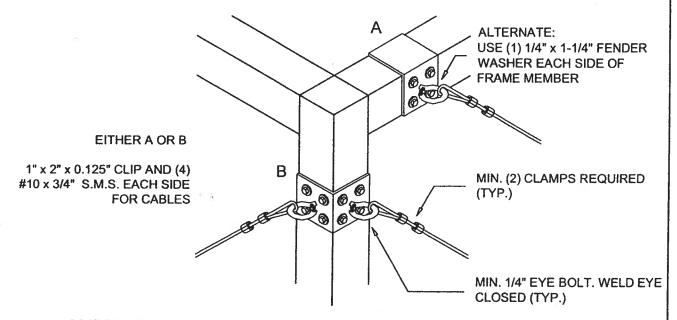
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SECTION 1



SCALE: 3" = 1'-0"



ALTERNATE TOP CORNER OF CABLE CONNECTION - DETAIL 1A

SCALE: 3" = 1'-0"

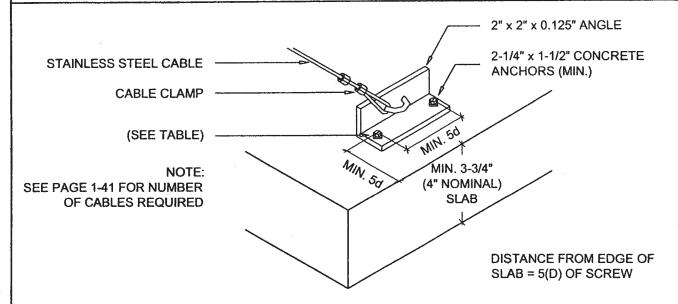
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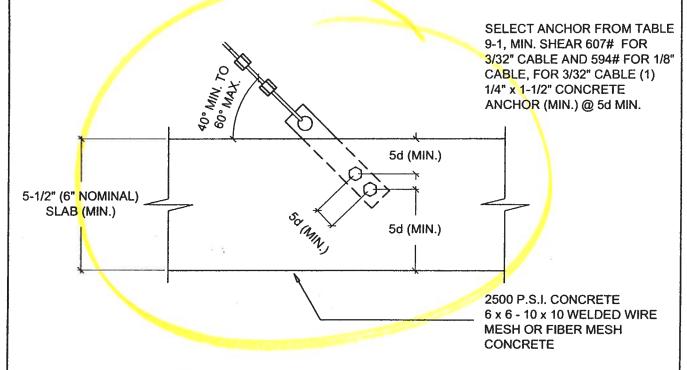
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SECTION 1



ALTERNATE CABLE CONNECTION AT SLAB DETAIL - DETAIL 2B

SCALE: 3" = 1'-0"



ALTERNATE CABLE CONNECTIONS AT FOUNDATION - DETAIL 2C

SCALE: 3" = 1'-0"

Lawrence E. Bennett, P.E. FL # 16644

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Table 1.6 Minimum Upright Sizes and Number of Screws for Connection of Roof Beams to Wall Uprights or Beam Splicing

Beam	Upright	Minimum Purlin, Girt	Deck	Notes	Minimu	ım Number ol	Screws*	Beam Stitching
Size	Size	& Knee Brace Size**	1 1		#8 x ½"	#10 x ½"	#12 x ½"	Screw @ 24" O.C.
2" x 3"	2" x 3"	2" x 2" x 0.044"	2	Full Lap	6	4	4	-
2" x 4"	2" x 3"	2" x 2" x 0.044"	2	Full Lap	8	6	4	#8
2" x 4"	2" x 4"	2" x 2" x 0.044"	2	Full Lap	8	6	4	#10
2" x 5"	2" x 3"	2" x 2" x 0.044"	2	Full Lap	8	6	4	#8
2" x 6"	2" x 3"	2" x 2" x 0.044"	4	Full Lap	10	8	6	#10
2" x 6"	2" x 4"	2" x 2" x 0.044"	4	Partial Lap	10	8	6	#10
2" x 7"	2" x 4"	2" x 2" x 0.044"	4	Partial Lap	14	12	10	#12
2" x 8"	2" x 5"	2" x 3" x 0.044"	6	Partial Lap	16	14	12	#14
2" x 9"	2" x 6"	2" x 3" x 0.045"	6	Partial Lap	18	16	14	#14
2" x 9"**	2" x 7"	2" x 4" x 0.050"	8	Partial Lap	20	18	16	#14
2" x 10"	2" x 8"	2" x 4" x 0.050"	10	Partial Lap	20	18	16	#14

Screw Size	Minimum Distance and S	Spacing of Screws	Gusset Plate Thickness							
	Edge To Center	Center To Center	Beam Size	Thickness						
#8	5/16"	5/8"	2" x 7" x 0.055" x 0.120"	1/16" = 0.063"						
#10	3/8"	3/4"	2" x 8" x 0.072" x 0.224"	1/8" = 0.125"						
#12	1/2"	1"	2" x 9" x 0.072" x 0.224"	1/8" = 0.125"						
#14 or 1/4"	3/4"	1-1/2"	2" x 9" x 0.082" x 0306"	1/8" = 0.125"						
5/16"	7/8"	1-3/4"	2" x 10" x 0.092" x 0.369"	1/4" = 0.25"						
3/8"	1"	2"								

^{*} Refers to each side of the connection of the beam and upright and each side of splice connection.

Note:

- 1. Connection of 2" x 6" to 2" x 3" shall use a full lap cut or 1/16" gusset plate.
- 2. All gusset plates shall be a minimum of 5052 H-32 Alloy or have a minimum yield strength of 23 ksi.
- 3. For beam splice connections the number of screws shown is the total for each splice with 1/2 the screws on each side of the cut.
- 4. The number of screws is based on the maximum allowable moment of the beam.
- 5. The number of deck anchors is based on RAWL R Tapper allowable load data for 2,500 psi concrete and / or equal anchors may be used. The number shown is the total use 1/2 per side.
- 6. Hollow splice connections can be made provided the connection is approved by the engineer.
- 7. If a larger than minimum upright is used the number of screws is the same for each splice with 1/2 the screws on each side of the cut.
- 8. All beam to upright connections for 2" x 7" beams or larger shall have an internal or external gusset plates. Gusset plates are required for mansard or gable splice connections.
- 9. For gusset plate connections 2" x 9" beams or larger use 3/4" long screws.
- 10. The side wall upright shall have a minimum beam size as shown above, ie., a 2" x 4" upright shall have a 2" x 3" beam.
- 11. Connect beam to upright w/ H-bar, gusset plate, or angle clips for each splice with 1/2 the screws on each side of the cut.
- 12. For girt size use upright size (i.e. 2" x 6"). Read the 2" x 6" beam row for min. girt of 2" x 2" x 0.044".

Table 1.7 Minimum Size Screen Enclosure Knee Braces and Anchoring Required
Aluminum 6063 T-6

Brace Length	Extrusion	Anchoring System
0' - 2'-0"	2" x 2" x 0.044"	2" H-Channel With (3) #10 x 1/2" EACH SIDE
To 3'-0"	2" x 3" x 0.045"	2" H-Channel With (3) #10 x 1/2" EACH SIDE
To 4'-6"	2" x 4" x 0.044" x 0.12"	2" H-Channel With (4) 3/4" long screws (size to be determined by beam size, see table 9.6)

(See Table 1.6 For Number And Size Of Screws)

Note:

- 1. For required knee braces greater than 4'-6" contact engineer for specifications and details.
- 2. Cantilever beam detail shown on page 1-32 shall be used for host structure attachment when knee brace length exceeds 4'-6".

Lawrence E. Bennett, P.E. FL # 16644

CIVIL ENGINEER - DEVELOPMENT CONSULTANT
P.O. BOX 214368, SOUTH DAYTONA, FL 32121
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^{** 0.082&}quot; wall thickness, 0.310" flange thickness

Table 1.4 Allowable Post / Girt / Chair Rail Spans, Header Spans & Upright Heights for Secondary Screen Wall Frame Members
Aluminum Alloy 6063 T-6

For 3 second wind gust at velocity of 120 MPH or an applied load of 14 # / sq. ft.*

A. Sections As Horizontals Fastened To Posts With Clips

		Tributary Load Width 'W'														
Hollow Sections	3'-6'	3'-6'		3'-6' 4'-0" 4'-6" 5'-0" 5'-6"					•	6'-0"	•	6'-8"				
		Allowable Heights 'H' / bending 'b' or deflection 'd'														
2" x 2" x 0.044"	6'-10"	ď	6'-6"	ь	6'-1"	b	5'-9"	ь	5'-6"	b	5'-3"	b	5'-0"	ь		
2" x 2" x 0.055"	7'-3"	d	6'-11"	d	6'-8"	b	6'-4"	ь	6'-0"	ь	5'-9"	Ь	5'-6"	ь		
3" x 2" x 0.045"	7'-9"	ď	7'-5"	d	7'-1"	d	6'-10"	d	6'-7"	ь	6'-4"	ь	5'-11"	Ъ		
2" x 3" x 0.045"	9'-4"	ь	8'-9"	ь	8'-3"	b	7'-10"	ь	7'-5"	ь	7'-2"	b	6'-9"	b		
2" x 4" x 0.050"	10'-3"	ь	9'-7"	ь	9'-0"	b	8'-7"	ь	8'-2"	ь	7'-10"	ь	7'-5"	b		

Snap Sections		Α	llowable Hei	ghts 'H' /	bendir	g 'b' or	deflection '	ď'		
2" x 2" x 0.044"	7'-6"	d 7'-2"	d 6'-11"	d 6'-8	3" b	6'-4"	b 6'-1"	ь	5'-9"	b

B. Sections As Horizontals Fastened To Posts Through Side Into Screw Bosses

	Tributary Load Width 'W'														
Hollow Sections	3'-6'	4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8'	**		
	Allowable Heights 'H' / bending 'b' or deflection 'd'														
2" x 2" x 0.044"	7'-9"	b	7'-3"	b	6'-10"	ь	6'-6"	b	6'-2"	b	5'-11"	b	5'-7"	ь	
2" x 2" x 0.055"	8'-5"	b	7'-11"	b	7'-5"	ь	7'-1"	b	6'-9"	b	6'-5"	b	6'-1"	b	
3" x 2" x 0.045"	9'-3"	b	8'-8"	Ь	8'-2"	ь	7'-9"	ь	7'-5"	b	7'-1"	ь	6'-8"	ь	
2" x 3" x 0.045"	10'-5"	ь	9'-9"	b	9'-2"	b	8'-9"	b	8'-4"	b	7'-11"	ь	7'-7"	b	
2" x 4" x 0.050"	11'-6"	b	10'-9"	b	10'-1"	b	9'-7"	b	9'-2"	b	8'-9"	ь	8'-4"	b	
Snap Sections			All	lowa	able Hei	ghts	'H' / be	ndir	ıg 'b' or	defl	ection 'c	1			
2" x 2" x 0.044"	9'-2"	ь	8'-7"	b	8'-1"	ь	7'-8"	b	7'-4"	b	7'-0"	Ы	6'-8"	b	

^{*} For allowable heights at wind velocities other than 120 MPH, see conversion table 1A on the specifications for tables page at the beginning of this section and example below.

Notes:

- Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. Using screen panel width 'W' select girt lengths.
- 3. Site specific engineering required for pool enclosures over 20' in mean roof height.
- 4. Span is to be measured from center of beam and upright connection to fascia or wall connection.
- 5. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height can be considered as residential guardrails provided they are attached with min. (3) #10 x 1-1/2" S.M.S. into the screw bosses and do not exceed 8'-0" in span.
- 6. Girt spacing shall not exceed 6'-8".
- 7. Spans may be interpolated.

IF HEIGHTS FOR 'C' EXPOSURE CATAGORY AND/OR WINDZONES OTHER THAN 120 MPH ARE REQUIRED, SEE EXAMPLE ON SPECIFICATION PAGE FOR TABLES AT THE BEGINNING OF THIS SECTION.

Lawrence E. Bennett, P.E. FL # 16644

CIVIL ENGINEER - DEVELOPMENT CONSULTANT
P.O. BOX 214368, SOUTH DAYTONA, FL 32121
TELEPHONE: (386) 767-4774
FAX: (386) 767-6556

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Table 1.3 Allowable Post / Upright Heights for Primary Screen Wall Frame Members
Aluminum Alloy 6063 T-6

For 3 second wind gust at velocity of 120 MPH or an applied load of 14 # / sq. ft.*

Hollow Sections		Tributary Load Width 'W' = Upright Spacing													
	3'-0"	•	4'-0"	•	5'-0'	•	6'-0"	1	7'-0'		8'-0'	•	9'-0"		
	Allowable Height 'H' / bending 'b' or deflection 'd'														
2" x 2" x 0.044"	8'-4"	b	7'-3"	b	6'-6"	b	5'-11"	b	5'-6"	b	5'-1"	Ь	4'-10"	b	
2" x 2" x 0.055"	9'-1"	b	7'-11"	b	7'-1"	b	6'-5"	b	5'-11"	ь	5'-7"	b	5'-3"	b	
2" x 3" x 0.045"	11'-3"	b	9'-9"	b	8'-9"	b	7'-11"	b	7'-5"	b	6'-11"	b	6'-6"	b	
2" x 4" x 0.050"	12'-5"	b	10'-9"	b	9'-7"	b	8'-9"	b	8'-1"	b	7'-7"	b	7'-2"	ь	

		Tributary Load Width 'W' = Upright Spacing													
Self Mating Sections	3'-0"		4'-0"	4'-0"		5'-0"		6'-0"			8'-0"	9'-0		,	
			Al	ow	able Hei	ght	'H' / ben	din	g 'b' or c	lefle	ction 'd'				
2" x 4" x 0.044 x 0.100"	16'-11"	b	14'-8"	b	13'-1"	b	11'-11"	b	11'-1"	ь	10'-4"	b	9'-9"	b	
2" x 5" x 0.050" x 0.100"	20'-11"	b	18'-1"	b	16'-2"	b	14'-9"	b	13'-8"	b	12'-10"	b	12'-1"	ь	
2" x 6" x 0.050" x 0.120"	24'-2"	b	20'-11"	b	18'-9"	b	17'-1"	b	15'-10"	ь	14'-10"	b	13'-11"	ь	
2" x 7" x 0.055" x 0.120"	27'-3"	b	23'-7"	b	21'-1"	b	19'-3"	b	17'-10"	b	16'-8"	b	15'-9"	b	
2" x 7" x 0.055" w/ insert	36'-3"	b	31'-4"	ь	28'-1"	b	25'-7"	ь	23'-9"	b	22'-2"	b	20'-11"	ь	
2" x 8" x 0.072" x 0.224"	35'-2"	b	30'-6"	b	27'-3"	b	24'-10"	ь	23'-0"	b	21'-6"	ь	20'-4"	b	
2" x 9" x 0.072" x 0.224"	38'-2"	b	33'-0"	b	29'-6"	b	26'-11"	ь	24'-11"	b	23'-4"	ь	22'-0"	b	
2" x 9" x 0.082" x 0.310"	41'-10"	b	36'-3"	b	32'-5"	b	29'-7"	ь	27'-5"	b	25'-8"	ь	24'-2"	b	
2" x 10" x 0.092" x 0.369"	50'-4"	b	43'-7"	b	38'-11"	b	35'-7"	b	32'-11"	b	30'-10"	b	29'-1"	b	

Snap Sections	Tributary Load Width 'W'= Upright Spacing													
	3'-0"	3'-0"			5'-0"		6'-0"		7'-0"		8'-0"	9'-0		
	Allowable Height 'H' / bending 'b' or deflection 'd'													
2" x 2" x 0.044"	9'-11"	b	8'-7"	b	7'-8"	b	7'-0"	b	6'-6"	b	6'-1"	b	5'-9"	b
2" x 3" x 0.045"	12'-9"	b	11'-0"	ь	9'-10"	b	9'-0"	b	8'-4"	b	7'-10"	b	7'-4"	b
2" x 4" x 0.045"	15'-7"	b	13'-6"	b	12'-1"	b	11'-0"	b	10'-2"	b	9'-7"	b	8'-11"	b
2" x 6" x 0.062"	26'-5"	b	22'-10"	b	20'-5"	b	18'-8"	b	17'-3"	b	16'-2"	b	15'-3"	b
2" x 7" x 0.062"	29'-5"	ь	25'-5"	b	22'-9"	ь	20'-9"	b	19'-3"	b	17'-11"	b	16'-11"	b

^{*} For allowable heights at wind velocities other than 120 MPH, see conversion table 1A on the specification page for tables at the beginning of this section and example below.

Note:

- 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. Using screen panel width 'W' select upright length 'H'.
- 3. Above heights do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
- 4. Site specific engineering required for pool enclosures over 20' in mean roof height.
- 5. Height is to be measured from center of beam and upright connection to fascia or wall connection.
- 6. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height can be considered as residential guardrails provided they are attached with min. (3) #10 x 1-1/2" S.M.S. into the screw bosses and do not exceed 8'-0" in span.
- 7. Heights may be interpolated.

CHECK TABLE 1.6 FOR MINIMUM UPRIGHT SIZE FOR BEAMS.

IF SPANS FOR 'C' EXPOSURE CATAGORY AND/OR WINDZONES OTHER THAN 120 MPH ARE REQUIRED, SEE EXAMPLE ON SPECIFICATION PAGE FOR TABLES AT THE BEGINNING OF THIS SECTION.

Lawrence E. Bennett, P.E. FL # 16644

CIVIL ENGINEER - DEVELOPMENT CONSULTANT
P.O. BOX 214368, SOUTH DAYTONA, FL 32121
TELEPHONE: (386) 767-4774
FAX: (386) 767-6556

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Table 1.1

Allowable Spans for Primary Screen Roof Frame Members Aluminum Alloy 6063 T-6

For Areas with Wind Loads up to 150 M.P.H. and Latitudes Below 30°-30'-00" North (Jacksonville, FL)

Hollow Sections	Tributary Load Width 'W' = Beam Spacing														
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"		
	Allowable Span 'L' / bending 'b' or deflection 'd'														
2" x 2" x 0.044"	9'-10"	ь	8'-7"	ь	7'-8"	b	6'-11"	b	6'-6"	ь	6'-1"	Ы	5'-8"		
2" x 2" x 0.055"	10'-9"	ь	9'-4"	ь	8'-4"	b	7'-7"	b	7'-1"	b	6'-7"	Ы	6'-3"	_ 1	
2" x 3" x 0.045"	13'-4"	ь	11'-7"	b	10'-4"	b	9'-5"	b	8'-9"	b	8'-2"	ь	7'-8"	_ 1	
2" x 4" x 0.050"	14'-8"	Ь	12'-8"	b	11'-4"	b	10'-4"	b	9'-7"	b	8'-11"	b	8'-5"	ا	

Self Mating Sections	Tributary Load Width 'W' = Beam Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	,
	Allowable Span 'L' / bending 'b' or deflection 'd'													
2" x 4" x 0.044 x 0.100"	19'-11"	Ь	17'-4"	ь	15'-6"	b	14'-2"	b	13'-1"	b	12'-3"	b	11'-6"	b
2" x 5" x 0.050" x 0.100"	24'-9"	Ь	21'-5"	ь	19'-2"	b	17'-6"	b	16'-2"	b	15'-2"	b	14'-3"	b
2" x 6" x 0.050" x 0.120"	28'-7"	b	24'-9"	b	22'-2"	b	20'-3"	b	18'-9"	b	17'-6"	b	16'-6"	b
2" x 7" x 0.055" x 0.120"	32'-3"	b	27'-11°	b	24'-11"	b	22'-9"	b	21'-1"	b	19'-9"	b	18'-7"	b
2" x 7" x 0.055" w/ insert	42'-10"	b	37'-1"	b	33'-2"	b	30'-4"	b	28'-1"	Ь	26'-3"	b	24'-9"	b
2" x 8" x 0.072" x 0.224"	41'-7"	b	36'-1"	b	32'-3"	b	29'-5"	b	27'-3"	b	25'-6"	b	24'-0"	b
2" x 9" x 0.072" x 0.224"	45'-1"	b	39'-1"	b	34'-11"	b	31'-11"	b	29'-6"	b	27'-8"	b	26'-1"	b
2" x 9" x 0.082" x 0.310"	49'-6"	<u>-</u>	42'-11"	b	38'-4"	b	35'-0"	b	32'-5"	b	30'-4"	b	28'-7"	b
2" x 10" x 0.092" x 0.369"	59'-6"	b	51'-7"	b	46'-1"	b	42'-1"	b	38'-11"	ь	36'-5"	b	34'-4"	b

Snap Sections	Tributary Load Width 'W' = Beam Spacing													
	3'-0"	3'-0"			5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Span 'L' / bending 'b' or deflection 'd'													
2" x 2" x 0.044"	11'-9"	b	10'-2"	ь	9'-1"	b	8'-4"	b	7'-8"	b	7'-2"	b	6'-9"	b
2" x 3" x 0.045"	15'-1"	Ь	13'-1"	b	11'-8"	b	10'-8"	b	9'-10"	b	9'-3"	b	8'-8"	b
2" x 4" x 0.045"	18'-5"	ь	15'-11"	ь	14'-3"	Ь	13'-0"	b	12'-1"	b	11'-3"	b	10'-8"	b
2" x 6" x 0.062"	31'-3"	b	27'-1"	ь	24'-2"	b	22'-1"	b	20'-5"	b	19'-2"	b	18'-0"	b
2" x 7" x 0.062"	34'-9"	b	30'-1"	b	26'-11"	b	24'-7"	b	22'-9"	b	21'-3"	b	20'-1"	b

Note:

- 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. The structures designed using this section shall be limited to a maximum combined span and upright height of 55' and a maximum upright height of 20'. Structures larger than these limits shall have site specific engineering.
- 3. Spans are based on a minimum of 10# / Sq. Ft. for up to a 150 M.P.H. wind load.
- 4. Span is measured from center of beam and upright connection to fascia or wall connection.
- 5. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
- 6. Purlin spacing shall not exceed 6'- 8". For beam spans greater than 40'-0" the beam at the center purlin and one purlin for each 14'-0" on each side of the center purlin shall include lateral bracing as shown in detail (48'-0") span with purlins at 6'- 8" o.c. center purlin and (2) purlins each side of center purlin need lateral bracing.

7. Spans may be interpolated.

Example: Max. 'L' for 2" x 4" x 0.050" hollow section with "W' = 5'-0" = 11'-4"

Lawrence E. Bennett, P.E. FL # 16644

CIVIL ENGINEER - DEVELOPMENT CONSULTANT P.O. BOX 214368, SOUTH DAYTONA, FL 32121 TELEPHONE: (386) 767-4774 FAX: (386) 767-6556

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1-56

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