DATE <u>07/30/2008</u>	Columbia County This Permit Must Be Prominently Post		struction	PERMIT 000027219
APPLICANT PATRICK		PHONE	904.296.1490	000027219
ADDRESS 6800	SOUTHPOINT PKWY, #300	JACKSONVILLE	904.250.1450	FL 32216
Washington to the contract of	A HOMES INC.,OF FLORIDA	PHONE	904.296.1490	
ADDRESS 299	SW TIMBER RIDGE DRIVE	LAKE CITY		FL 32024
CONTRACTOR THE	DDORE C. BROCK	PHONE	407.227.3504	
LOCATION OF PROPERT	Y 90-W TO SR.247-S,TL TO C-2	252-B,TR TO TIMBER RID	GE,TL	
	9TH LOT ON R.			
TYPE DEVELOPMENT	SFD/UTILITY	ESTIMATED COST OF CO	NSTRUCTION	153050.00
HEATED FLOOR AREA	_2236.00 TOTAL A	REA 3061.00	HEIGHT	STORIES 1
FOUNDATION CONC	WALLS FRAMED	ROOF PITCH 6'12	FLO	OOR CONC
LAND USE & ZONING	RSF-1	-	. HEIGHT 3:	-
		00X 0440 500000	202270300	Section Section
Minimum Set Back Requirm	1	00 REAR	15.00	SIDE 10.00
NO. EX.D.U. 0	FLOOD ZONE X	DEVELOPMENT PERM	MIT NO.	
PARCEL ID 10-4S-16-02	2856-109 SUBDIVIS	ION TIMBERLANDS		
LOT 9 BLOCK	PHASE UNIT	TOTA	LACRES 0.5	60
000001642) 1 1	
000001643	CBC1256382	- / The	1 Nelso	
	Culvert Waiver Contractor's License N	/	Applicant/Owner/O	
	08-402 BLK		/R	Y
			roved for Issuance	New Resident
COMMENTS: ELEVATIO	N CONFIRMATION LETTER REQUIRE	ED @ SLAB. MFE @ 100.0	0'.	
			nateur no se mas	21000
			Check # or Ca	sh 918689
	FOR BUILDING & ZON	ING DEPARTMENT	ONLY	(footer/Slab)
Temporary Power	Foundation		Monolithic	
	date/app. by	date/app. by		date/app. by
Under slab rough-in plumbin	g Slab date/app. by		Sheathing/N	lailing
Framing	5-04-05-05-05-05-05-05-05-05-05-05-05-05-05-	date/app. by above slab and below wood	floor	date/app. by
date/app.	by	above stab and below wood		date/app. by
Electrical rough-in	Heat & Air Duct	F	eri. beam (Lintel)	
	ate/app. by	date/app. by	om count (Emici)	date/app. by
Permanent powerdate/	app. by C.O. Final	1	Culvert	
M/H tie downs, blocking, elec		date/app. by	Pool	date/app. by
		pp. by		date/app. by
Reconnectiondat	e/app. by Pump pole	Utility Pole	date/app. by	_
M/H Pole	Travel Trailer		Re-roof	
date/app. by		date/app. by		date/app. by
BUILDING PERMIT FEE \$	770.00 CERTIFICATION F	FE \$ 15.30	SURCHARGE I	DEE \$ 15.20
MISC. FEES \$ 0.00	ZONING CERT. FEE \$ 50.0	0 FIRE FEE \$ 0.00	WASTE	FEE \$
FLOOD DEVELOPMENT FE	FLOOD ZONE FEE \$ 25.	00 CULVERT FEE \$ 2	5.00 TOTA	L FEE 900.60
INSPECTORS OFFICE	X/	CLERKS OFFICE	(1)	/
0	HE REOLIBEMENTS OF THIS DEPART. THE	Pour Montatourinata Contents burgates		

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only Application # 0806-55 Date Received 6/27 By 50 Permit # 27219-1643
Zoning Official Date 14.07.08 Flood Zone X FEMA Map # N/A Zoning RSF-2
Land Use Estar DevElevation W/A MFE/W off River W/A Plans Examiner WD Date 7/3/08
Comments Elevia Consiruation Regulater Regulard at Stab
NOC = EH Deed or PA Site Plan State Road Info Parent Parcel #
□ Dev Permit # □ In Floodway □ Letter of Authorization from Contractor
□ Unincorporated area □ Incorporated area □ Town of Fort White □ Town of Fort White Compliance letter
Septic Permit No. 08-402 Fax (904)-332-6367
THE PARTY OF THE P
Name Authorized Person Signing Permit Theodore C. Brock/ Phone (904)-296-1490
Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216
Owners Name Maronda Homes Inc. of Florida Phone (904)-296-1490
911 Address 299 SW Timber Ridge DRINE Lake City, FL, 32024
Contractors Name Theodore C. Brock Phone (407)-227-3504
Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216
Fee Simple Owner Name & AddressN/A
Bonding Co. Name & AddressN/A
and the state of t
Architect/Engineer Name & Address Tomas Ponce 4005 Maronda Way Sanford, FL 32771
Mortgage Lenders Name & Address Bank of America 250 Park Ave. S. #400 Winter Park, FL 32789
Circle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Progress Energy
Property ID Number 10-45-10-02850-109 Estimated Cost of Construction \$91,890.00
Subdivision Name Timber lands Lot Disch Lot Block L Unit Phase
Driving Directions Hwy 90, Left on 247 South; Right on 252B; Left on Timber Ridge, 9th Lot on
Right.
Number of Existing Dwellings on Property
Construction of Residential Single Family Dwelling Total Acreage Lot Size
Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height
Server warren of have an extensing briter
Actual Distance of Structure from Property Lines - Front 30.0 Side 27.0 Side 27.0 Rear 79.0
Actual Distance of Structure from Property Lines - Front 50.0 Side 27.0 Side 27.0 Rear 79.0 Number of Stories Heated Floor Area 2230 Total Floor Area 3001 Roof Pitch 12/6
Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or
installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.
I- 918695
Page 1 of 2 (Both Pages must be submitted together.)

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Theodore C. Brock Owners Signature

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's Signature (Permitee)

Theodore C. Brock

Contractor's License Number (BC 12510382 **Columbia County**

Competency Card Number

Affirmed under penalty of perjury to by the Contractor and subscribed before me this \mathcal{L} day of \mathcal{M}

Personally known_XXX or Produced Identification_

State of Florida Notary Signature (For the Contractor) Melissa L. McKague

SEAL:



Notary Public State of Florida Melissa L McKague My Commission DD493647 Expires 11/22/2009

Coloninia Contry Ballania Lauric Whitegrant

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be

done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written resobrabilities in Columbia County for obtaining this Building Permit. CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit. Contractor's License Number CBC 250382 Contractor's Signature (Permitee) Columbia County Theodore C. Brock Competency Card Number Affirmed under penalty of perjury to by the <u>Contractor</u> and subscribed before me this 24 day of <u>JUNC</u> Personally known XXX or Produced Identification SEAL: State of Florida Notary Signature (For the Contractor) Notary Public State of Florida Melissa L. McKague Melissa L McKague My Commission DD493647 Expires 11/22/2009

Page 2 of 2 (Both Pages must be submitted together.)

Revised 11-30-07



STATE OF FLORIDA DEPARTMENT OF HEALTH

08-402

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

	,				53	*		rei	mit Ap	piicatio	n Num	iber			
					PA	ART II - S	SITE PL	.AN							
Scale: Eac	h block re	nrecente	5 foot a	nd 1 inch					Ĭ.						
ПППП	LILLET	Pieseills	Jieet al	id i inch	= 50 feet.	77777	-اساداداد	harbadanlanda	- Secondario Secondari	nya-hapotanjanahina	COCPANIE -	Minimum days			
			HH			1111						++++	111	TIT	TIII
HHH						1111		$\Box \Box \Box \Box$	+++	+++	HH	$\Pi\Pi\Pi$	1111	11	土土土
				11111	++++	++++			Π	111	口口		111	士士	}
		+++++	++++	HHH	HHHH					计廿	111		+H	++	$\Pi\Pi$
					11:111				1111	} 	++++	HH	-1-1-1	777	口は
			1111		111111			++++	++++	++++	HH			山土	十十十
					++++	+++	+++		111					111	1-1-1-
			++++	++++		771	HH	HIT	1111	1111		<u> </u>		+++	+HH
+++++-	+++-		1111						1111	1111	1	+++-	$\rightarrow \rightarrow$		TXI
			11:11:						+++	HH	HH	\Box	HH	444	1111
					++++	-++-	HHH	HHH	444	$\Pi\Pi$	$\Pi\Pi$	111		世世	
			++++											+++	+++
1	HHH									1111	++++	++++	HH	44	777
					1				1111	+++	HH	HH	日日	##	111
			111	* T	141	14-	4-11	1	FIN		1111	 			1111
		HHH	111/		14	1/1		99	+4	1111					++++
HHHH									1111	1111	+++	+++	HHH	HH	111
								11111	+++	+HH	+H	$\Pi\Pi$	$\Pi\Pi$	HH	111
			1.1111	191		HDV .	11/	+HH	+	Π	111	111		口口	1111
			++++	1111		140	TVV		111			1111			1-1-1-1
$H \rightarrow H + H + H + H + H + H + H + H + H + $	++++		\Box									++++	++++	HH	+HH
HHH										1111	+HH	+++	$\Pi\Pi$	HH	111
			1:1-1-1-	+++++	++++		HHH	+++	$\Pi\Pi$	1111			口口	山廿	<u> </u>
			+++	+++++			$\Pi\Pi$	$\Pi\Pi$							1111
+++++												+++	HHH	HH	+HH
									-HH	++++	++++	+HT	$\Pi\Pi$	\Box	1111
			+1+++	+++++	++++	++++	HHH	++++		$\Pi\Pi$	111	111		口口	士士士
HHHH			$\Box\Box$							1111			+++	HH	+++
							HHH		HHH	+++	++++	++++	+++	HH	Π
			++++	+++++	-++++-	++++	HHH	+HF						世世	士士士
+++++	├ ┼┦┼┼		++++								111		++++	1	
											+++	+HH	+++	HH	HH
					++++	++++	HHHH	++++						\Box	
			++++	+HH	+HH			777					1111	出出	
Maion	# 13			COMMITTED AND AND AND AND AND AND AND AND AND AN	The State of the Land of the State of the St	the attribute when	configuration and except	mateur laurit de la constant de la c			- 1	1			لالللا
Notes:	1.	- 111	1 .		^				KEV	1500-	(W	18,	1		
	* See	Attac	Lod	SitE	DIAN				4	14	1	1/1	1	27.201.00	
	4	11/11/10	MEDI	-116	1000				~	Vill	and	11/11	-		
									1	7	. ,				
									/						
-				,	2	/									
Site Plan	submitte	d by:	1	Mar	1/1/	-	21		8			4	466UT	-	
		547.1	/ -	D		ignature			B ·	馬用	ø.		-0-10	Title	
Plan Appr	oved A	1		PPn		ot App	roved_	-LO			aC	HD)	0/19/	8
Ву	- WV		2 11	n 11 11 16	UBU							Cou	nty He	alth Г)enari
					31								,		- chait

LEGEND:

- FOUND 1/2"
 IDENTIFICATION
- o = FOUND 1/2" LB. 6894
- O = SET 1/2" RE L.B. 6894
- ø = FOUND 3/4"
- = FOUND 4" X NO IDENTIFIC
- D = SET 4" X 4" P.S.M. 5582
- X = SET NAIL &
- X = FOUND NAIL
- ⊠ = FOUND 6" X
 R/W MON.
- ☐ = CATV RISER
- ☑ = TELEPHONE
- Ø = WOOD POWE

THIS I

NOT VALID A FLORIDA TO THIS MA

TRUE AND MY SUPERVITHE MINIMUL BOARD OF STATUTES

DY:

DATE:



SCALE: 1" = 30"

DATE: 5/19/08

Y BE CH. THIS THE

IS OF

FECT Y SUCH FIELD WORK COM

PREPARED FOR:

LEGAL DESCRIPTION:

LOT NINE (9) OF "TIMBERLANDS" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGE 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

W٠

CERTIFIED TO:

1) MARONDA HOMES

6/25/8

REVISED

BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'



APPLOYED

Columbia CHD

SURVEYOR NOTES:

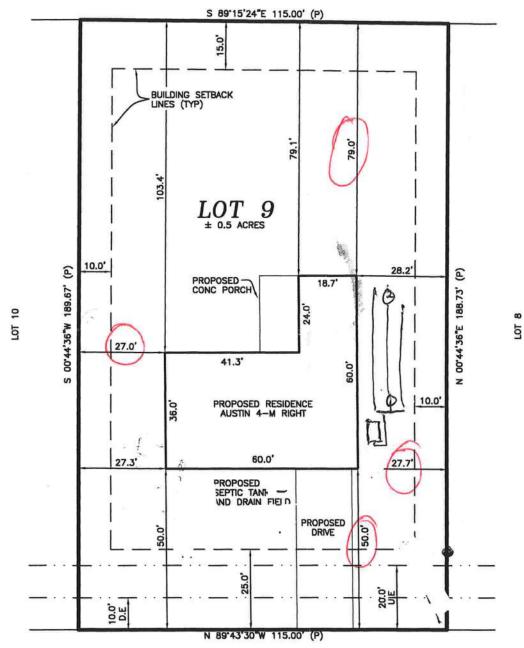
- TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
- THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

下 ミショドロの 向をださる



PROPOSED BUILDING LAYOU

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA



LOOD NOTE:

S.W. TIMBER RIDGE DRIVE

THE OPINION OF THIS SURVEYOR, ACCORDING TO THE TIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE P COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-88, IS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA TERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS ALED FROM SAID MAP. INFORMATION FROM THE FEDERAL IERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE TE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE FERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE RIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ITHE MOST CURRENT MAP.

TITLE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAD DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH SURVEYOR HAS NOT PERFORMED A SEARCH OF 1 PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIM TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFTHIS PARCEL THE PRESENCE OR ABSENCE OF AN CLAIMS ARE NOT CERTIFIED HEREON.

90 60 30 0 30 = 30

LEGEND:

m = FOUND 1/2" REBAR NO IDENTIFICATION

= FOUND 1/2" REBAR & CAP LB. 6894

O = SET 1/2" REBAR & CAP LB. 6894

ø = FOUND 3/4" IRON PIPE

■ = FOUND 4" X 4" CONC. MON. NO IDENTIFICATION

□ = SET 4" X 4" CONC. MON. P.S.M. 5582

X = SET NAIL & DISK P.S.M. 5582

X = FOUND NAIL & DISK

 ⊠ = FOUND 6" X 6" S.R.D. R/W MON.

□ = CATV RISER

TELEPHONE PEDESTAL

Ø = WOOD POWER POLE

ABBREVIATIONS:

A/C = AIR CONDITIONER

ASPH = ASPHALT
C = CALCULATED FROM MEASURED

CATV = CABLE TELEVISION

C/B = CONCRETE BLOCK

CLF = CHAIN LINK FENCE

CM = CONCRETE MONUMENT

CONC = CONCRETE ELEC = ELECTRIC

ELEV = ELEVATION

= FOUND FND

LICENSED SURVEYOR BUSINESS

(M) = FIELD ME MH = MANHOLE FIELD MEASURED

O.U. = OVERHEAD UTILITIES
P = PLAT

PB = PLAT BOOK
P.U.E. = PUBLIC UTILITIES EASEMENT

TRANS = TRANSFORMER
TYP = TYPICAL

WATER METER WATER VALVE

THIS IS NOT A BOUNDARY SURVEY CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES AND CHAPTER 11617-6, FLORIDA ADMINISTRATIVE CODE.

DATE: 5 20 08

BRINKMAN SURVEYING & MAPPING INC.

4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609

PHONE: (352) 374-7707

FAX: (352) 374-8757

SCALE: 1" = 30"

"THE BENCHMARK IN QUALITY SERVICE"

DRAWN BY: ZL

DATE: 5/19/08

CHECKED BY: J.B.

FIELD WORK COMPLETED ON ****

FIELDBOOK **, PAGE **

PREPARED FOR: MARONDA

DRAWING NUMBER 114-08

FI Th DE SCENE

W٩

/ BE H. THIS HE IS OF Y SUCH

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Project Name: AUSTIN 4 BDR GAINESVILLE Address: ZON SW TIMBER RIGHE DO City, State: LAKE CHY, PL 321015 Owner: ELECTRIC Climate Zone: North	Builder: MARONDA HOMES Permitting Office: COLUMDIA Permit Number: 2799 Jurisdiction Number: 221000
1. New construction or existing 2. Single family or multi-family 3. Number of units, if multi-family 4. Number of Bedrooms 5. Is this a worst case? 6. Conditioned floor area (ft²) 7. Glass type¹ and area: (Label reqd. by 13-104.4.5 if not default) a. U-factor:	12. Cooling systems a. Central Unit Cap: 40.5 kBtu/hr SEER: 13.00 b. N/A c. N/A 13. Heating systems a. Electric Heat Pump Cap: 40.5 kBtu/hr HSPF: 8.10 b. N/A c. N/A 14. Hot water systems a. Electric Resistance Cap: 50.0 gallons EF: 0.90 b. N/A c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) 15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)
Glass/Floor Area: 0.08 Total as-built po	
I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code. PREPARED BY: DATE: WMUML CAMPBIL 05 21 08 I hereby certify that this building, as designed, is in compliance with the Florida Energy Code. OWNER/AGENT: MCIWA MAGNE	Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL:

DATE:

DATE: Melissa My Caque

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: ,,,	PERMIT #:

	BASE			The second secon		AS-	BUI	LT	ASTRUM			u u
GLASS TYPES .18 X Condition Floor Ar	ned X BS	SPM = F	Points	Type/SC		erhang	Llest	A V	CDI	4 V	COL	- Delate
1 1001 A	Ca			Type/SC	Ornt	Len	ngı	Area X	3PI	VI X	50F	= Points
.18 2236	.0	18.59	7482.0	1.Single, Clear	W	1.0	6.0	16.0	43.8		0.97	680.0
				2.Single, Clear	N	1.0	6.0	16.0	21.7		0.97	338.0
				3.Single, Clear	W	1.0	3.0	5.0	43.8		0.85	186.0
			21	4.Single, Clear	W	1.0	7.0	30.0	43.8		0.98	1294.0
				5.Single, Clear 6.Single, Clear	W	1.0 1.0	8.0 6.0	40.0	43.8		0.99	1734.0
				7.Single, Clear	E	1.0	6.0	20.0	47.9		0.97	927.0 927.0
				8.Single, Clear	E	1.0	6.0	30.0	47.8		0.97	1394.0
				o.omgie, olear	_	1.0	0.0	30.0	47.0	2	0.57	1354.0
				As-Built Total:				177.0		22		7480.0
WALL TYPES	Area X	BSPM	= Points	Туре		R-	Value	Area	Х	SPN	1 =	Points
Adjacent	320.0	0.70	224.0	1. Concrete, Int Insul, Exte	rior	Married World	4.1	1199.0		1.13		1360.9
Exterior	1199.0	1.70	2038.3	2. Frame, Steel, Adjacent	1101	1,401	13.0	320.0		0.90		288.0
			2000.0	E. Frame, Otool, Adjacont			10.0	320.0		0.50		200.0
Base Total:	1519.0		2262.3	As-Built Total:	-			1519.0				1648.9
DOOR TYPES	Area X	BSPM	= Points	Туре				Area	Х	SPN	1 =	Points
Adjacent	18.0	2.40	43.2	1.Adjacent Wood				18.0		2.40		43.2
Exterior	20.0	6.10	122.0	2.Exterior Insulated				20.0		4.10		82.0
												7327 270527050
Base Total:	38.0		165.2	As-Built Total:				38.0				125.2
CEILING TYPES	S Area X	BSPM	= Points	Туре		R-Valu	ie A	Area X S	SPM	X SC	= M	Points
Under Attic	2236.0	1.73	3868.3	1. Under Attic		*	19.0	2450.0 2	.34 X	(1.00		5733.0
Base Total:	2236.0		3868.3	As-Built Total:				2450.0	Ħ			5733.0
FLOOR TYPES	Area X	BSPM	= Points	Туре		R-	Value	Area	Х	SPM	1 =	Points
Slab	210 O(p)	37.0	9102.0	1 Slob On Grada Edge Inc	ulation	STATE STATE OF	0.0	210.0/5	III ESTABLISH	44.00		-9022.8
Raised	219.0(p) 0.0	-37.0 0.00	-8103.0 0.0	1. Slab-On-Grade Edge Ins	sulation		0.0	219.0(p	1	41.20		-9022.8
Base Total:			-8103.0	As-Built Total:				219.0				-9022.8
INFILTRATION	Area X	BSPM	= Points					Area	Χ	SPM	l =	Points
	2236.0	10.21	22829.6			0		2236.0		10.21		22829.6

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

	A STATE OF THE PARTY OF THE PAR
ADDRESS:,,,	PERMIT #:

*	BASE		AS-BUILT								
Summer Ba	se Points:	28504.3	Summer As-Built Points: 28793.8								
Total Summer Points	X System Multiplier	= Cooling Points	Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)								
28504.3	0.3250	9263.9	(sys 1: Central Unit 40500btuh ,SEER/EFF(13.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS) 28794								

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BASE		AS-	BUI	LT				
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area	O Type/SC Orn	verhang at Len	Hgt	Area X	WPI	их	WOF	= Points
.18 2236.0 20.17 8118.0	1.Single, Clear 2.Single, Clear 3.Single, Clear	N 1.0 / 1.0	6.0 6.0 3.0	16.0 16.0 5.0	28.84 33.22 28.84	2 4	1.01 1.00 1.04	465.0 531.0 150.0
es es so	4.Single, Clear W 5.Single, Clear W 6.Single, Clear E 7.Single, Clear E	/ 1.0 = 1.0 = 1.0	7.0 8.0 6.0 6.0	30.0 40.0 20.0 20.0	28.84 28.84 26.4 26.4	1 1 1	1.00 1.00 1.02 1.02	869.0 1157.0 536.0 536.0
	8.Single, Clear E	1.0	6.0	30.0 177.0	26.4	1	1.02	804.0 5048.0
WALL TYPES Area X BWPM = Points	Туре	R-\	/alue	Area	X١	NPM	=	Points
Adjacent 320.0 3.60 1152.0 Exterior 1199.0 3.70 4436.3	Concrete, Int Insul, Exterior Frame, Steel, Adjacent	1	4.1 13.0	1199.0 320.0		6.42 4.90		7697.6 1568.0
Base Total: 1519.0 5588.3	As-Built Total:			1519.0				9265.6
DOOR TYPES Area X BWPM = Points	Туре			Area	ΧV	NPM	=	Points
Adjacent 18.0 11.50 207.0 Exterior 20.0 12.30 246.0	1.Adjacent Wood 2.Exterior Insulated	_	-	18.0 20.0		1.50 8.40		207.0 168.0
Base Total: 38.0 453.0	As-Built Total:		- 1/2	38.0				375.0
CEILING TYPES Area X BWPM = Points	Туре	R-Value	Are	ea X W	PM X	WC	M =	Points
Under Attic 2236.0 2.05 4583.8	1. Under Attic	1	9.0	2450.0 2	2.70 X	1.00		6615.0
Base Total: 2236.0 4583.8	As-Built Total:	-		2450.0			aye, majara	6615.0
FLOOR TYPES Area X BWPM = Points	Туре	- R-\	/alue	Area	Χ١	VPM	=	Points
Slab 219.0(p) 8.9 1949.1 Raised 0.0 0.00 0.0	1. Slab-On-Grade Edge Insulation		0.0 2	19.0(p	1	8.80		4117.2
Base Total: 1949.1	As-Built Total:			219.0				4117.2
INFILTRATION Area X BWPM = Points				Area	×ν	VPM	=	Points
2236.0 -0.59 -1319.2				2236.0)	-0.59		-1319.2

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

	BASE		AS-BUILT								
Winter Base	Points:	19373.0	Winter As-Built Points: 24101.5								
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit = Heating Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)								
19373.0	0.5540	10732.6	(sys 1: Electric Heat Pump 40500 btuh ,EFF(8.1) Ducts:Unc(S),Con(R),Int(AH),R6.0 24101.5 1.000 (1.060 x 1.169 x 0.88) 0.421 0.950 10552.7 24101.5 1.00 1.095 0.421 0.950 10552.7								

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	15.4	PERMIT #:
		i Lixivii i #.

BASE				AS-BUILT								
WATER HEA Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	Х	Tank X Ratio	Multiplier	X Credit Multiplie	
4		2635.00		10540.0	50.0	0.90	4 .		1.00	2693.56	1.00	10774.2
					As-Built To	otal:						10774.2

				CODE	C	OMPLI	ANCE	S	TATUS	3			
		BAS	SE		Secumental					AS	-BUILT		
Cooling Points	+	Heating Points	+	Hot Water Points	Section and Section	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
9264		10733		10540	Recalgell	30537	7623	12	10553	Ţ	10774		28950

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS:,,,	PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	1
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	V
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	V
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	V
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	V
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	/
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	/

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	1
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	V
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	V
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	V

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.8

The higher the score, the more efficient the home.

ELECTRIC, , , ,

1.	New construction or existing	New	_	12. Cooling systems	
2.	Single family or multi-family	Single family		a. Central Unit	Cap: 40.5 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 13.00
4.	Number of Bedrooms	4		b. N/A	W 820
5.	Is this a worst case?	Yes			
6.	Conditioned floor area (ft²)	2236 ft²	Trade Control	c. N/A	
7.	Glass type 1 and area: (Label reqd. by 13-	104.4.5 if not default)			_
a.		Description Area		13. Heating systems	- F
b.	(or Single or Double DEFAULT) 7a(Sr SHGC:	igle Default) 177.0 ft²	-	a. Electric Heat Pump	Cap: 40.5 kBtu/hr HSPF: 8.10
	(or Clear or Tint DEFAULT) 7b.	(Clear) 177.0 ft ²	-	b. N/A	_
8.	Floor types	2			_
	Slab-On-Grade Edge Insulation	R=0.0, 219.0(p) ft		c. N/A	·
	N/A		_		
C.	N/A			14. Hot water systems	
9.	Wall types			a. Electric Resistance	Cap: 50.0 gallons _
	Concrete, Int Insul, Exterior	R=4.1, 1199.0 ft ²	_		EF: 0.90
b.	Frame, Steel, Adjacent	R=13.0, 320.0 ft ²	_	b. N/A	<u></u>
c.	N/A		_	*	
d.	N/A		_	c. Conservation credits	-
e.	N/A	1	_	(HR-Heat recovery, Solar	
10.	Ceiling types			DHP-Dedicated heat pump)	
a.	Under Attic	R=19.0, 2450.0 ft ²		15. HVAC credits	PT,
b.	N/A	1	_	(CF-Ceiling fan, CV-Cross ventilation,	31
c.	N/A	- 1	_	HF-Whole house fan,	
11.	Ducts			PT-Programmable Thermostat,	
a.	Sup: Unc. Ret: Con. AH(Sealed):Interior	Sup. R=6.0, 200.0 ft	_	MZ-C-Multizone cooling,	
b.	N/A		_	MZ-H-Multizone heating)	
		į.		8	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: WI A MA IN OU

Date: 05 27 08

Address of New Home: 299 SW TIMBER RINGE Dity/FL Zip: LAN CITY, FOL

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

NOTICE OF COMMENCEMENT
Tax Parcel Identification Number 0 45-10-02850-109 County Clerk's Office Stamp or Seal
THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.
1. Description of property (legal description): 299 SW TIMBER RUME DY. a) Street (job) Address:
2. General description of improvements: COISTINICTION OF A SINITE FARMING AWAITING
a) Name and address: Maronda Howes Inc. A. P. LOOD Southpoint Pkwy #300 Jax Ft 30216 b) Name and address of fee simple titleholder (if other than owner) c) Interest in property
4. Contractor Information Mayonda House Inc a Followith Scatternant Plans & 200 100
4. Contractor Information a) Name and address: MAYONA HOME INC & FL USOD SOUTHPOINT PLOWY \$300. Mx FL 300 US b) Telephone No. (904) 294-1490 Fax No. (Opt.) (904) 332-103-15
n) Name and address:
b) Amount of Bond: c) Telephone No.: Fax No. (Opt.)
a) Name and address: b) Phone No
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served: a) Name and address: SWTNEYN THE HOLDING CO LUC 3943 BAYMEMOWS RO MX FL 3221 b) Telephone No.: (102) 739-2205 Fax No. (Opt.)
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(I)(b).
Piorida Statutes;
a) Name and address: b) Telephone No.: Fax No. (Opt.)
9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified):
WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.
STATE OF FLORIDA COUNTY OF COLUMBIA 10.
Signature of Owner or Owner's Authorized Office/Director/Purtner/Managor TNEODOYE C- BYDOAC
Print Name
The foregoing instrument was acknowledged before me, a Plorida Notary, this
fact) for MUYONIA HOMES INC OF FLOY WA (name of party on behalf of whom instrument was executed).
Personally Known OR Produced Identification Type Notary Public State of Florida
Notary Signature MVWW MCCommission DD493647 Expires 11/22/2009
1. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.
NV / I

Signature of Natural Poson Signing (in line #10 above.)

Columbia County Building Department Culvert Permit

Culvert Permit No.

000001643

DATE 07/3	0/2008 PARCEL ID # 10-4S-1	6-02856-109		
APPLICANT	PATRICK WILSON	PHONE	904.296.1490	
ADDRESS 6	SOUTHPOINT PKWY. #300	JACKSONVILLE	FL	32216
OWNER MA	ARONDA HOMES INC. OF FLORIDA	PHONE	904.296.1490	
ADDRESS 29	99 SW TIMBER RIDGE DRIVE	LAKE CITY	FL	32024
CONTRACTO	R THEODORE C. BROCK	PHONE	407.227.3504	
LOCATION O	F PROPERTY 90-W TO SR.247-S,TL TO C-252-B,T	TR TO TIMBER RI	DGE,TL	
9TH LOT ON R.				
SIGNATURE X	INSTALLATION REQUIREMENTS Culvert size will be 18 inches in diameter with a driving surface. Both ends will be mitered 4 foot thick reinforced concrete slab. INSTALLATION NOTE: Turnouts will be required a majority of the current and existing drivers b) the driveway to be served will be paved or Turnouts shall be concrete or paved a mining concrete or paved driveway, whichever is great current and existing paved or concreted turnouts and existing paved or concreted turnouts.	red as follows: way turnouts are formed with con mum of 12 feet or reater. The width	e paved, or; crete. wide or the width	h a 4 inch
	Culvert installation shall conform to the approx	ved site plan sta	ndards.	
	Department of Transportation Permit installation	on approved star	ndards.	
	Other			

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00





Maronda Systems

Maronda Systems

4005 Maronda Way

Sanford FL 32771

(407) 321-0064

AUSTIN M4

Fax (407) 321-3913

Tomas Ponce, P.E. Engineer/Architect of Record:

299 SW TIMBER

RIDGE DR

367 Medallion PL.

Chuluota, Fl. 32766

FL PE # 50068,

Design Criteria: TPI PLAN JOB #

9TM00901

9-1

Design: Matrix Analysis MiTek software **ADDRESS** LOT

DIV/SUB

JAX-9TM

MODEL **AUSM4 RIGHT**

This structure was designed in accordance with, and meets the requirements of TPI standards and the FLORIDA 2004 BUILDING CODE for 125 M.P.H. Wind Zone.

Truss loading is in accordance with ASCE 7-02. These trusses are designed for an enclosed building.

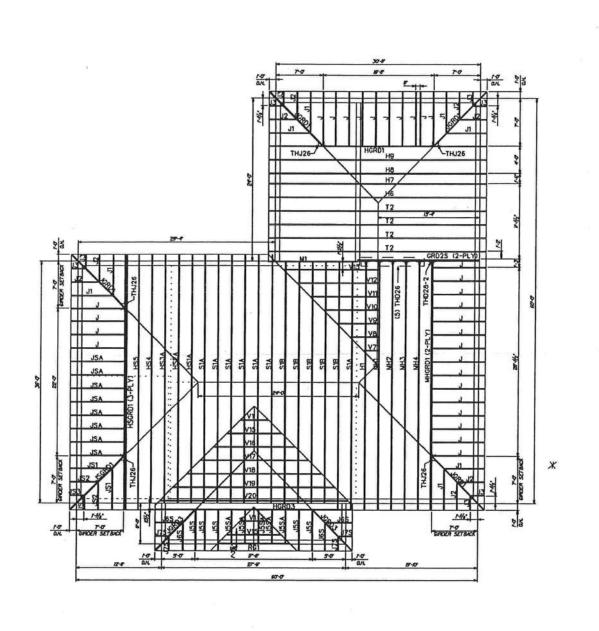
The Truss Engineering package for the above referenced site was generated by the Truss Designer/Architect/MiTek/Trenco.

I. Tomas Ponce, P.E.

the Architect/Engineer of Record for the above referenced lot

Have reviewed the package and confirmed that it matches the physical and structural

Truss ID	Run Date	Drawing Reviewed	Truss ID	Run Date	Drawing Reviewed	No. of Eng. 56 Dwgs:
Layout	10/10/07		MHGRD1	11/21/07		Roof Loads-
V	07/27/05		RG1	11/21/07		TC Live: 16.0
HIP	11/02/06		S1A	11/21/07		TC Dead: 7.0
GRD2S	01/03/08		S1B	11/21/07		BC Live: 10.0
H1	11/21/07		T2	11/21/07	1	BC Dead: 10.0
H6	11/21/07		V1	11/21/07		Total 43.0
H7	11/21/07		V10	11/21/07		
H8	11/21/07		V11	11/21/07		DurFac- Lbr: 1.25
H9	11/21/07	7	V12	11/21/07	11/1/11/11	DurFac- Plt: 1.25
HGRD1	11/21/07		V13	11/21/07		O.C. Spacing: 24.0"
HGRD3	11/21/07		V15	11/21/07		
HS1A	11/21/07		V16	11/21/07		_
HS2A	11/21/07		V17	11/21/07		
HS3A	11/21/07		V18	11/21/07		
HS4	11/21/07		V19	11/21/07		
HS5	11/21/07		V20	11/21/07		
HSGRD1	11/21/07		V7	11/21/07		
J	11/21/07		V8	11/21/07		
J1	11/21/07		V9	11/21/07		
J2	11/21/07					
J3	11/21/07					1
J5S	11/21/07					
J5SA	11/21/07					
J6S	11/21/07					- 1 L
J7S	11/21/07					
JGRD1	11/21/07					
JGRD3	11/21/07					
JS1	11/21/07		INV#	DESC	QNTY	
JS2	11/21/07		18331	THD26	5	
JS3	11/21/07		18330	THD28		
JSA	11/21/07		18357	SKHH26R		
JSGRD1	11/21/07		18357	SKHH26L		1//
M1	11/21/07		18363	JUS26		///
MH1	11/21/07		18370	THJ26	5	
MH2	11/21/07		18336	THD28-2	1	
мнз	11/21/07					MAY 1 6 20
MH4	11/21/07		SEAT PLA	TES		DATE:



HARDWARE LEGEND

- 1 HUS26
- 2 HUS28
- 3 JUS26
- 4 MP6F
- 5 MPA1 & MPA1F
- 6 SKH26 L/R
- 7 SKHH26 L/R
- 8 SUS26
- 9 SUS28
- 10 THD26
- 11 THD28
- 12 THD28-2
- 13 THDH28-3
- 14 THD48
- 15 THJ26**
- 16 LTW12



HARDWARE MANUFACTURED BY USP

- * HARDWARE MANUFACTURED BY SIMPSON
- * * HARDWARE MANUFACTURED BY CLEVELAND

LOADING-FRC2004/TPI2002

DRAWN BY: EJ

SCALE: 1/8" • 1'-0" DATE: 10/10/2007

GARAGE: RIGHT

Maronda Homes

(407) 321-0064 4005 MARONDA WAY SANFORD, FLORIDA

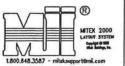
AUSTIN 4 "M" - FL

Writek_IffXous4-m-125-6x1 .HT Dec. 04, 2007

TC LVE	16,00	SNOW LOAD	0.00
TC DEAD	7.00	LUMBER DOL	1.25
BC LVE	10,00	PLATE DOL	125
BC DEAD	10.00	WIND	125
TOTAL	43,00	SPACING	2-0

DESIGNER: CP

CHECKER: MIKE



GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detalled Information.

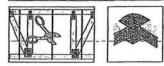
Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members for more information. All other permanent bracing design is the responsibility of the Building Designer.

Los trusses no están marcados de ningún modo que Identifique la frecuencia o localización de restricción lateral y arriostre diagonal temporales. Use las recomendaciones

e manejo, Instalación, restricción y arriostre temporal de los trusses. Vea el folleto <u>BCSI Guía de Buena Práctica</u> para el Manelo, Instalación, Restricción y Arriostre de los Trusses de Madera Conectados con Placas de Metal para información más detallada.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen BCSI-B3 - Restricción/Arriostre Permanente de Cuerdas y Miembros Secundarios para más Información. O resto de los diseños de arriostres perman responsabilidad del Diseñador del Edificio.





The consequences of Improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal Infury or death.

El resultado de un manejo, levantamiento, instalación, restricción y arrisotre incorrecto puede ser la caída de la estructura o aún peor, heridos o

Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

Empaques y placas de metal tienen bordes afilados. Use guantes y lentes protectores cuando corte los empaques.

NOTAS GENERALES HOISTING RECOMMENDATIONS FOR TRUSS BUNDLES RECOMENDACIONES PARA LEVANTAR PAGUETES DE TRUSSES.

⚠ Warning! Don't overload the crane. iAdvertencial iNo sobrecarga la grúa!

Never use banding alone to lift a bundle. Do not lift a group of Individually banded bundles. Nunca use sólo los empaques para levantar un paquete No levante un grupo de empaques Individuales.

A single lift point may be used for bundles with trusses up to 45'.

Two lift points may be used for bundles with trusses up to 60'. Usé at least 3 lift points for bundles with trusses greater than 60'.

Puede usar un solo lugar de levantar para paquetes de trusses hasta 45 pies. Puede usar dos puntos de levantar para paquetes más de 60 pies.

Use por lo menos tres puntos de levantar para paquetes más de 60 ples.



⚠ Warning! Do not over load supporting structure with truss bundle. lAdvertendal No sobrecargue la estructura

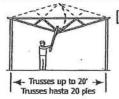
apoyada con el paquete de trusses. Place truss bundles in stable position.

Puse paquetes de trusses en una posición

INSTALLATION OF SINGLE TRUSSES BY HAND INSTALACIÓN POR LA MANO DE TRUSSES INDIVIDUALES

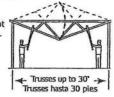
Trusses 20' or less, support at peak. Levante

del pico los trusses de 20 pies o menos.



Trusses 30' or less, support at quarter points.

Levante de los cuartos de tramo los trusses de 30 ples a menas.



HANDLING — MANEJO

Avoid lateral bending. — Evite la flexión lateral.



The contractor is responsible for properly

at the fobsite.

receiving, unloading and storing the busses

El contratista tiene la responsabilidad de

redbir, descargar y almacenar adecuada-

If trusses are to be stored horizontally, place blocking of sufficient height beneath the stack of trusses at 8' to 10' on center.

For trusses stored for more than one week.

cover bundles to prevent moisture gain but

Refer to BCSI Guide to Good Practice for

of Metal Plate Connected Wood Trusses

handling and jobsite storage of trusses.

Si los trusses estarán guardados horizon-

talmente, ponga bloqueando de altura suficiente detrás de la pila de los trusses.

Para trusses guardados por más de una

semana, cubra los paquates para prevenir

aumento de humedad pero permita venti-

Handling, Installing, Restraining & Bracing

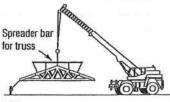
for more detailed information pertaining to

allow for ventilation.

mente los trusses en la obra.

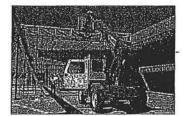
Use special care in windy weather or near power lines and airports.

Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



✓ Use proper rigging and hoisting eguloment.

Use equipp apropiado para levantar e improvisar.



Do not store

No almacene verticalmente los trucces queltos



Do not store on

No almacene en tierra desigual.



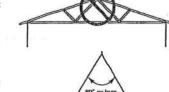
HOISTING OF SINGLE TRUSSES — LEVANTAMIENTO DE TRUSSES INDIVIDUALI

Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the base of featured in the house of featured in the h is installed and the truss is fastened to the bearing points.

Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalado y el truss está asegurado en los soportes.

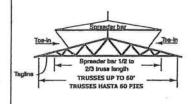
Marning! Using a single pick-point at the peak can damage the truss.

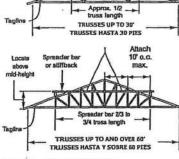
iAdvertencial El uso de un solo lugar para levantar en el pico puede hacer daño al truss.



HOISTING RECOMMENDATIONS FOR SINGLE

RECOMENDACIONES PARA LEVANTAR TRUSSES INDIVIDUALES





TEMPORARY RESTRAINT & BRACING RESTRICCIÓN Y ARRIOSTRE TEMPORAL

Refer to BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing for more information.

> Vea el resumen BCSI B2 - Restricción/ Arriostre Temporal y Instalación de los Trusses para más información.

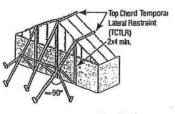
Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

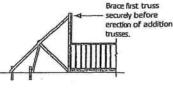
Coloque los arriostres de tierra para el primer truss directamente en linea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la table en la próxima



Do not walk on unbraced trusses.

sueltos.





Vea el folleto BCSI Guia de Buena Práctica para el Manejo, Instalación, Restricción y Arriostres de los Trusses de Madera Conectados con Placas de Metal para información más detallada sobre el manejo y almacenado de los trusses en área de trabajo

TEPS TO SETTING TRUSSES AS MEDIDAS DE LA INSTALLACIÓN DE LOS TRUSSES

- 1) Install ground bracing. 2) Set first truss and attach securely to ground bracing. 3) Set next 4 trusses with short member temporary lateral restraint (see below). 4) Install top chord diagonal bracing (see below). 5) Install web member plane diagonal bracing to stabilize the first five trusses (see below). 6) Install bottom chord temporary lateral restraint and diagonal bracing (see below). 7) Repeat process on groups of four trusses until all trusses are set.
 - 1) Instale los arriostres de tierra. 2) Instale el primero truss y ate seguramente al arriostre de tierra. 3) Instale los próximos cuatro trusses con restricción lateral temporal de miembro corto (vea abajo). 4) Instale el arriostre diagonal de la cuerda superior (vea abajo). 5) Instale arriostre diagonal para los planos de los miembros secundarios para estable los primeros cinco trusses (vea abajo). 6) Instale la restricción lateral temporal y arriostre diagonal para la cuerda inferior (vea abajo). 7) Repita éste procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.
- Refer to BCSI-B2 Summary Sheet Truss Installation & Temporary Restraint/Bracing for more information.

Vea el resúmen <u>BCSI-B2 - Instalación de Trusses y Arriostre Temporal</u> para mayor información.

ESTRAINT/BRACING FOR ALL PLANES OF TRUSSES

- RESTRICCIÓN/ARRIOSTRE EN TODOS PLANOS DE TRUSSES.
- This restraint & bracing method is for all brusses except 3x2 and 4x2 parallel chord trusses.

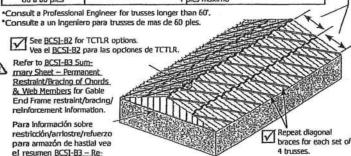
Este método de restricción y arriostre es para !odo trusses excepto trusses de cuerdas paralelas 3x2 y 4x2.

) TOP CHORD — CUERDA SUPERIOR

stricción/Arriostre

Permanente de Cuerdas y

Truss Span	Top Chord Temporary Lateral Restraint (TCTLR) Spacing
Longitud de Tramo	Espaciamiento del Arriostre Temporal de la Cuerda Superior
Up to 30°	10' o.c. max.
Hasta 30 pies	10 pies máximo
30' to 45'	8' o.c. max.
30 a 45 pies	8 pies máximo
45' to 60'	6' a.c. max.
45 a 60 ples	6 ples máximo
60° to 80°*	4° o.c. max.
60 a 80 pies*	4 pies máximo



Ground bracing not shown for clarity.

Repita los arrisotres

arupo de 4 trusses.

truss spaces (20' max.)

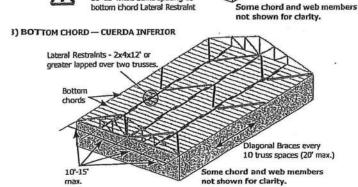
diagonales para cada

Miembros Secundarios.

1) WEB MEMBER PLANE — PLANO DE LOS MIEMBROS SECUNDARIOS

10'-15' max. Same spading as

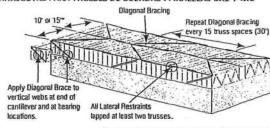
ATERAL RESTRAINT
DIAGONAL BRACING.
RE VERY IMPORTANT
LA RESTRICCIÓN
ATERAL Y EL
ARRIOSTRE
DIAGONAL
ON MUY
MPORTANTES!
Diagonal Braces every 10



RESTRAINT & BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES
LA RESTRICCIÓN Y EL ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3X2 Y 4X2

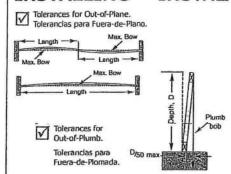
Refer to BCSI-B7
Summary Sheet
- Temporary & Permanent Restraint
Bracing for Parallel
Chord Trusses for
more information.
Vea el resumen

vea el resumen BCSI-B7 — Restricción y Arriostre Temporal y Permanente de Trusses de Cuerdas Paralelas para más información.



*Top chord Temporary Lateral Restraint spacing shall be 10 o.c. max. for 3x2 chords and 15 o.c. for 4x2 chords.

INSTALLING — INSTALACION



	elgo)
D/50	D (ft.)
1/4"	1'
1/2*	2.
3/4"	3,
1-	4"
1-1/4"	5'
1-1/2"	6*
1-3/4"	7.
2*	≥8'

Material

Gypsum Board

Plywood or OSB

Asphalt Shingles

Clay Tile

1265 CO. 10	票付替的
Max. Bow	Truss Length
3/4"	12.5'
7/8*	14.6
1"	16.7
1-1/8"	18.8
1-1/4*	20.8
-1-3/8*	22.9
1-1/2	25.0
1-3/4"	29.2
2-	≥33.3'

Heloht

16

2 hmdles

8

3-4 fles high

CONSTRUCTION LOADING -- CARGA DE CONSTRUCCION

⚠ Do not proceed with construction until all lateral restraint and bracing is securely and properly in place.

No proceda con la construcción hasta que todas las restricciones laterales y los arriostres estén colocados en forma aproplada y segura.

O Do not exceed maximum stack heights. Refer to <u>BCSI-B4</u>
Summary Sheet - Construction Loading for more information,

No exceda las máximas alturas recomendadas. Vea el resúmen BCSI-B4 Carga de Construcción para mayor Información.



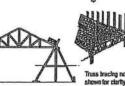
- Do not overload small groups or single brusses.
 No sobrecarque pequeños grupos o trusses individuales.
- Never stack materials near a peak.

 Nunca amontone material cerca del pico.
- Place loads over as many trusses as possible.

 Coloque las cargas sobre tantos trusses como sea posible
- Position loads over load bearing walls.

 Coloque las cargas sobre las paredes soportantes.





ALTERATIONS — ALTERACIONES

- Refer to BCSI-BS Summary Sheet Truss Damage, Jobsite Modifications & Installation Errors.

 Vea el resúmen BCSI-BS Daños de trusses, Modificaciones en la Obra y Errores de Instalación,
- O Do not cut, alter, or drill any structural member of a truss unless specifically permitted by the Truss Design Drawing.

 No corte, altere o perfore ningún miembro estructural de los

No corte, altere o perfore ningún miembro estructural de los trusses, a menos que esté especificamente pernitido en el dibujo del diseño del truss.



⚠ Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin una autorización previa del Fabricante de Trusses, pueden reducir o eliminar la garantía del Fabricante de Trusses.

NOTE: The Thuss Manufacturer and Truss Designer rely on the presumption that the Contractor and crone operator (if applicable) are professionals with the capability to undertake the work they have agreed to do on any given project. If the Contractor believes it needs assistance in some aspect of the construction project, it should seek assistance from a competant party. The methods and procedures outlined in this document are intended to ensure that the overall construction techniques employed will put the busses into place SAFEIX. These recommendations for handling, installing, restraining and bracing trusses are based upon the collective experience of leading personnel involved with truss design, manufacture and installation, but must, due to the nature of responsibilities or being manufacture and installation, but must, due to the nature of responsibilities handled building Designer's design specification for handling, installing, restraining and bracing trusses and it does not produce the use of other equivalent methods for restraining/bracing and providing stability for the waits, orburns, floors, roots and all the intervaluation studying building components as determined by the Contractor. Thus, WTCA and TPI expressly disclain any responsibility for demages raising from the use, application, or reflance on the recommendations and information contraded herein.



6300 Enterprise Lane - Madison, WI 53719



B1WARN11x17 20061115



MARONDA SYSTEMS

4005 Maronda Way

Sanford, FL 32771

(407) 321-0064

Fax (407) 321-3913

Date: November 1, 2006

To: Building Department

From: Maronda Systems

Tomas Ponce

Professional Engineer State of Florida #0050068

Subject: Valley Trusses

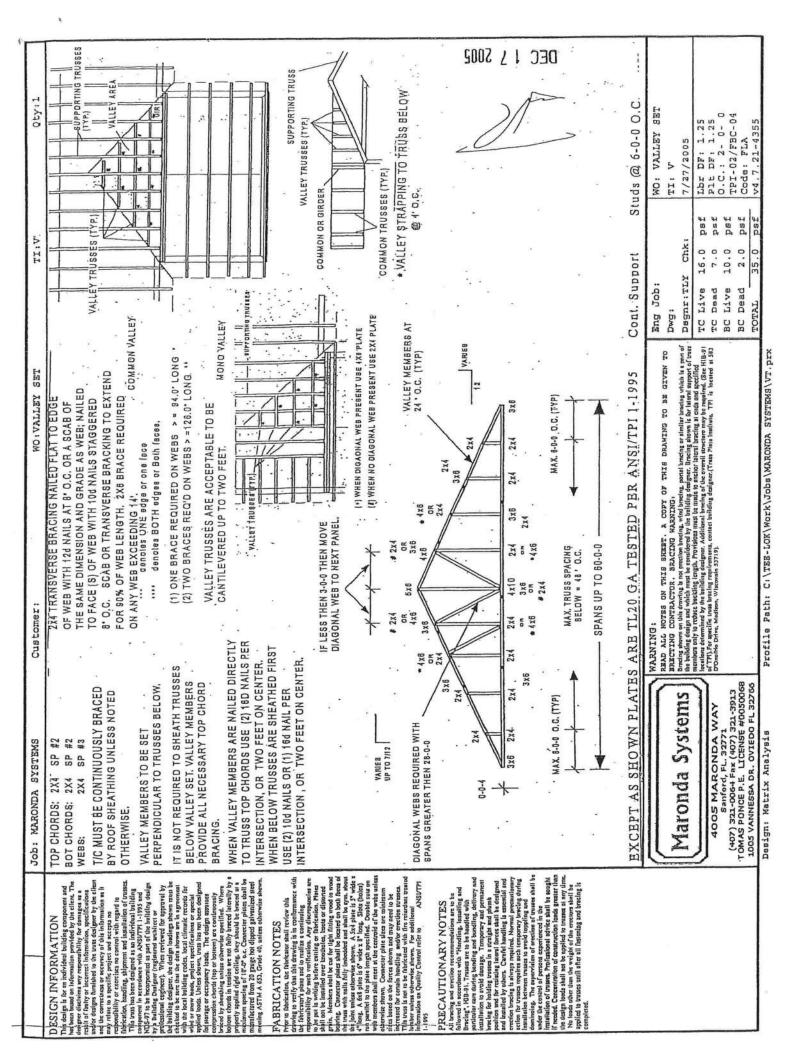
All valley trusses labeled V-1 through 100 are covered under the general valley sheet provided in the truss package signed and sealed by the engineer of record. The connections are noted on the structural info sheet of the plans. All criteria of the valley trusses are noted on the general sheet.

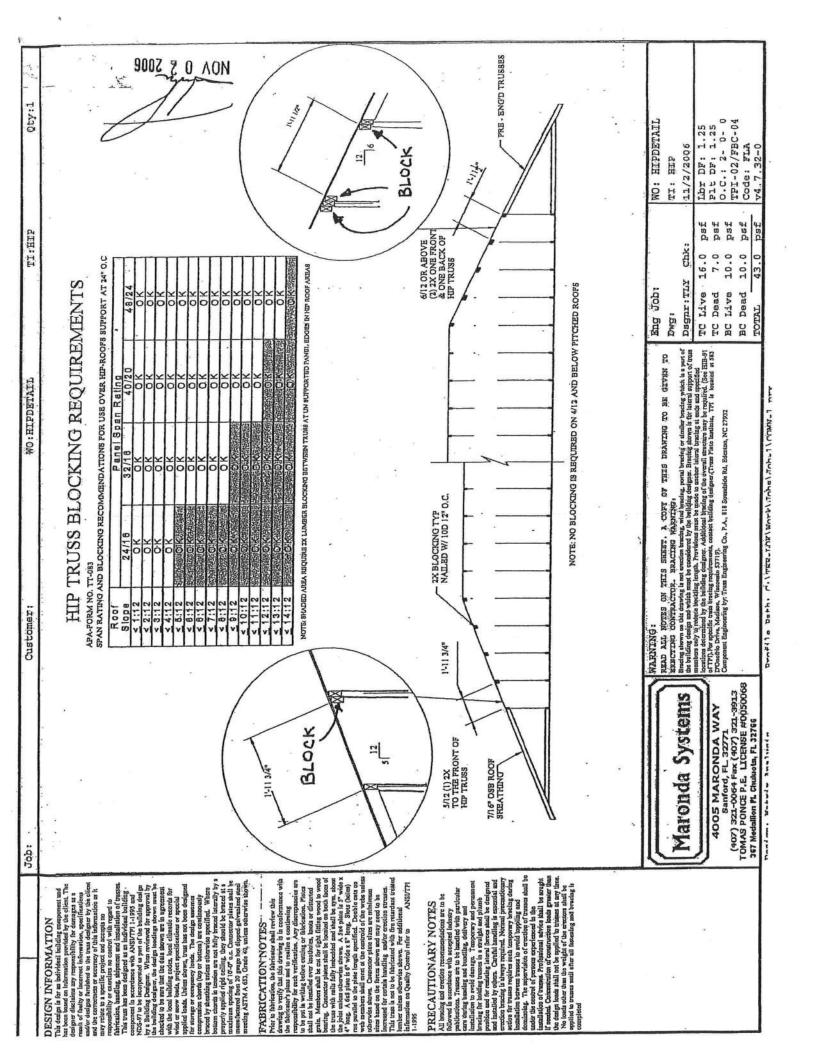
If you have any questions please feel free to call at 407-321-0064.

Sincerely,

Tomas Ponce, P.E.

Date: 11/1/06





AUSTIN_FL_125 Job Truss Truss Type E4595120 GRD2S AUSTIN Job Reference (optional) 9 2007 MiTek Industries, Inc. Thu Jan 03 16:55:13 2008 Page 1 Homes Inc., Sanford, Florida 4-0-0 4-6-15 3-D-D 3-1-8 3-7-9 4-0-0 Scale = 1:51.4 8x10 > 6.00 12 5×8 = 3x4 = 3x4 = 3 15 10 14 4x10 = 12 11 13 3x6 || 8x10 = 8x10 = 10x10 = 3x12 || 10-9-1 15-4-0 18-4-0 4-8-15 3-0-0 3-7-9 4-0-0 3-1-8 Plate Offsets (X.Y): [1:0-3-9.0-2-0], [7:0-2-3.Edge], [10:0-3-8.0-5-8], [11:0-5-0.0-6-0] GRIP **PLATES** DEFL in (loc) I/defl Ud SPACING LOADING (psf) -0.15 10-11 244/190 >989 240 TC BC Vert(LL) 16.0 Plates Increase 1.25 0.41 TCLL -0.28 10-11 >779 180 Vert(TL) TCDL 0.92 7.0 Lumber Increase 1.25 WB 0.99 Horz(TL) 0.05 n/a n/a Rep Stress Incr NO BCLL 10.0 Weight: 302 lb Code FBC2004/TP12002 (Matrix) BCDL 10.0 BRACING Structural wood sheathing directly applied or 3-9-5 oc purlins, except TOP CHORD TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 8 SYP No.2 BOT CHORD Rigid ceiling directly applied or 8-11-14 oc bracing. 2 X 4 SYP No.2 WEBS 1 Row at midpt REACTIONS (lb/size) 1=5327/0-8-0, 8=8121/0-8-0 Max Horz 1=276(LC 5) Max Uplift1=2359(LC 5), 8=3272(LC 5) FORCES (lb) - Maximum Compression/Maximum Tension 1-2=10711/4798, 2-3=-11019/5006, 3-4=-10999/5014, 4-5=-7773/3268, 5-6=-3111/1256, 6-7=-3096/1280, TOP CHORD 7-8=-7232/2967 1-12-4494/9484, 11-12-4494/9484, 11-13-4647/9838, 10-13-4647/9838, 10-14-3018/6934, 14-15-3018/6934, **BOT CHORD** 9-15=-3018/6934, 9-16=-18/43, 8-16=-18/43 2-12=381/241, 2-11=-291/506, 4-11=-2226/4064, 4-10=-4003/2246, 5-10=-2926/6707, 5-9=-6389/2884, WEBS 6-9=-1026/2606, 7-9=-2625/6389 NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 8 - 2 rows at 0-3-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated. 3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.

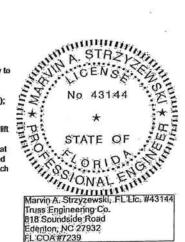
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2359 lb uplift at joint 1 and 3272 lb uplift

7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3454 lb down and 2152 lb up at 7-1-8, 1708 lb down and 813 lb up at 9-0-12, 1706 lb down and 631 lb up at 11-0-12, 1706 lb down and 645 lb up at 13-0-12, and 1706 lb down and 656 lb up at 15-0-12, and 1632 lb down and 601 lb up at 17-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

Continued on page 2



January 4,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 BEFORE USB. Design void for use only with Mifek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. For any designer, Bracing shown is for taleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer. For general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding foliopidation, quality control, storage, delivery, erection and bracing, consult. ANSI/TP11 Quality Citierla, DS8-89 and BCS11 Building Component Safely Intermetten available from Truss Plate Institute. S83 D'Onofrio Drive, Madison, Wt 53719.



Job	Truss	Truss Type	Qty	Ply	AUSTIN_FL_125	E4595120
AUSTIN	GRD2S	COMMON	1		Job Reference (optional)	
				7.02	20 s Nov 9 2007 MiTek Industries, Inc. Thu J	an 03 16:55:13 2008 Page 2

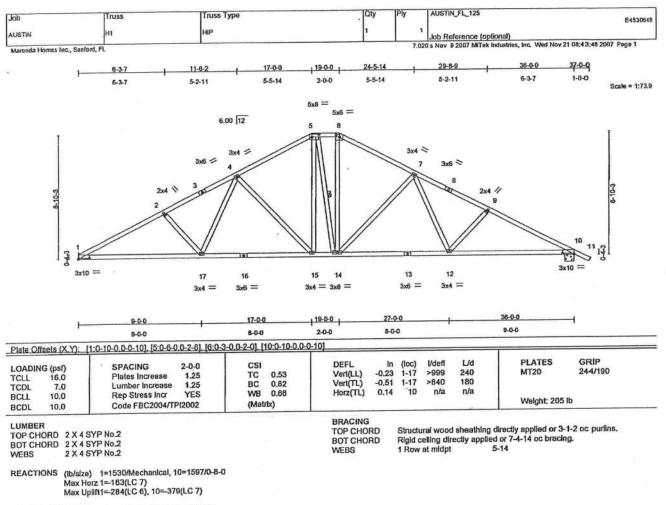
Maronda Homes Inc., Sanford, Florida

LOAD CASE(S) Standard 1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-6=-46, 6-7=-46, 1-8=-40 Concentrated Loads (lb) Vert: 11=-3454(F) 9=-1706(F) 13=-1706(F) 14=-1706(F) 15=-1706(F) 16=-1632(F)

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 BEFORE USE.

Design valid for use only with Mirek connections. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not inus designer. Bracing shown is for toleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer. For general guidance regarding reactor, Additional parameter in proceedings to the temporary bracing designer. For general guidance regarding fabrication, quality control, storage, delivery, eraction and bracing, consult. ANS/IPI1 Quality Criteria, DSB-89 and &CS11 Building Component Salety Information available from Trus Plate Institute. S83 D'Onofrio Drive, Madson, Wi 53719.





FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-2831/921, 2-3=-2646/854, 3-4=-2547/872, 4-5=-1850/713, 5-6=-1616/693, 6-7=-1852/714, 7-8=-2481/835, TOP CHORD

8-9-2580/825, 9-10-2744/872, 10-11-0/21 1-17-886/2491, 16-17-468/2050, 15-16-468/2060, 14-15-242/1612, 13-14-453/2030, 12-13-453/2030,

BOT CHORD

10-12=-630/2389

2-17=271/263, 4-17=105/640, 4-15=659/329, 5-15=174/634, 5-14=143/172, 6-14=169/618, 7-14=614/307, WEBS

7-12=65/574, 9-12=-209/226

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interfor(1) zone; Lumber DOL=1.60 plate grlp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 284 lb uplift at joint 1 and 379 lb uplift at loint 10.

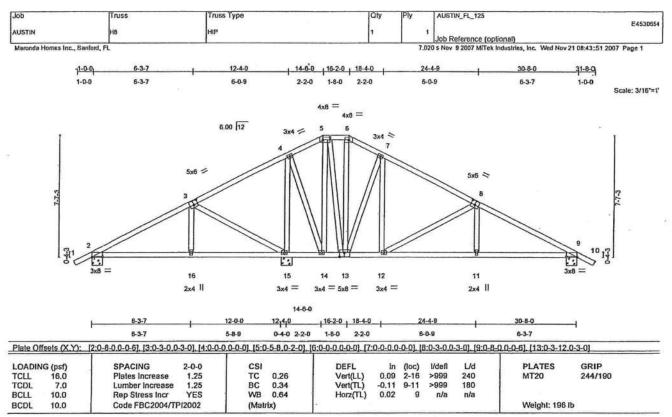
LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 BEFORE USE. Design void for use only with Mile's connector. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer and inside designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer. For general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding officers, adulting the property of th



Edenton, NC 27932



LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.2 BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 5-11-5 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 14-15,13-14.

REACTIONS (lb/size) 2=407/0-8-0, 15=1579/0-8-0, 9=723/0-8-0 Max Horz 2=135(LC 7)

Max Uplift2=310(LC 6), 15=535(LC 6), 9=224(LC 7) Max Grav2=436(LC 10), 15=1579(LC 1), 9=740(LC 11)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-392/406, 3-4=-137/401, 4-5=-13/105, 5-6=-101/127, 6-7=-157/140, 7-8=-436/128, 8-9=-1036/199, 9-10=0/21

BOT CHORD

2-16=-228/297, 15-16=-220/290, 14-15=-308/389, 13-14=-79/336, 12-13=0/331, 11-12=-67/857, 9-11=-65/864

3-16=317/272, 3-15=618/692, 4-15=1084/432, 4-14=167/764, 5-14=653/256, 5-13=-257/617, 6-13=-39/28,

7-13=-644/281, 7-12=-62/493, 8-12=-600/257, 8-11=0/274

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 310 lb uplift at joint 2, 535 lb uplift at joint 15 and 224 lb uplift at joint 9.

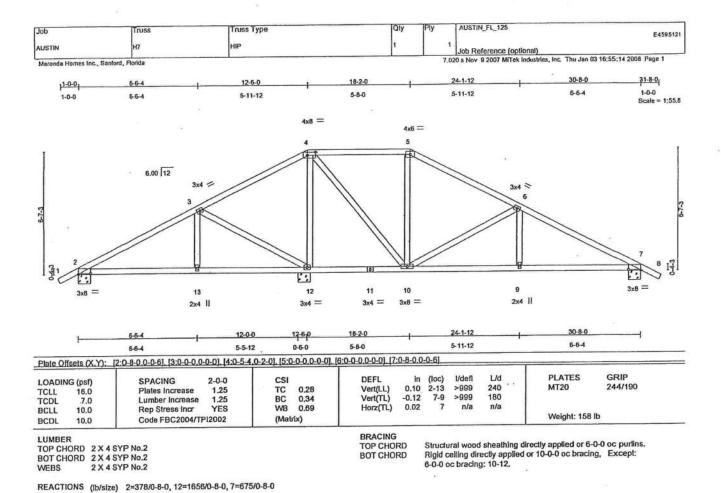
LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE FACE MIL-7473 BEFORE USE.

Design voild for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer- not inus designer, forcing shows for folleral support of individual web membes only. Additional temporary brocing to insure slothly during construction is the responsibility of the erector. Additional permonent brocing of the overal shucture is the responsibility of the building designer, for general guidance regarding foblaction, quality control, storage, delivery, erection and brocing, consul — MSI/IPI1 Quality Criteria, DSB-89 and 8CS11 Building Component Safety Information available from Truss Plate Institute, SS3 D'Onotrio Drive, Modison, WI S3719.





Max Horz 2=119(LC 6) Max Uplift2=323(LC 6), 12=508(LC 6), 7=-211(LC 7) Max Grav 2=416(LC 10), 12=1656(LC 1), 7=701(LC 11)

TOP CHORD

FORCES (lb) - Maximum Compression/Maximum Tension

BOT CHORD

1-2=0/21, 2-3=-334/414, 3-4=-147/487, 4-5=-235/131, 5-6=-330/93, 6-7=-937/184, 7-8=0/21 2-13=-234/244, 12-13=-234/244, 11-12=-409/413, 10-11=-409/413, 9-10=-40/776, 7-9=-40/776 3-13=-337/277, 3-12=-633/721, 4-12=-1129/516, 4-10=-301/897, 5-10=-166/198, 8-10=-616/265, 6-9=0/280

NOTES

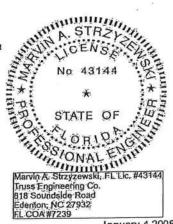
1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 323 lb uplift at joint 2, 508 lb uplift at joint 12 and 211 lb uplift at joint 7.

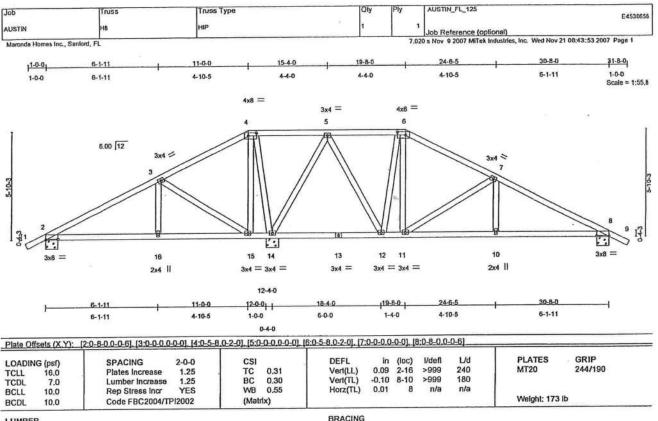
LOAD CASE(S) Standard



January 4,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. AWARNING - Verify design parameters and READ NOTES ON THIS AND INCLOSED at 15th Control of the c





LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.2

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

(lb/size) 2=325/0-8-0, 14=1710/0-8-0, 8=674/0-8-0 Max Horz 2=-108(LC 7) REACTIONS

Max Uplift2=321(LC 6), 14=471(LC 6), 8=209(LC 7) Max Grav2=365(LC 10), 14=1710(LC 1), 8=697(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-247/387, 3-4=-60/471, 4-5=-141/530, 5-6=-264/127, 6-7=-430/144, 7-8=-945/205, 8-9=0/21 2-16=-215/169, 15-16=-215/169, 14-15=-374/292, 13-14=-102/268, 12-13=-102/268, 11-12=0/342, 10-11=-45/784,

TOP CHORD **BOT CHORD**

8-10=-45/784

3-16=-296/263, 3-15=-560/609, 4-15=-497/294, 4-14=-700/715, 5-14=-886/381, 5-12=-173/655, 6-12=-377/220,

6-11=94/317, 7-11=531/215, 7-10=0/257

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

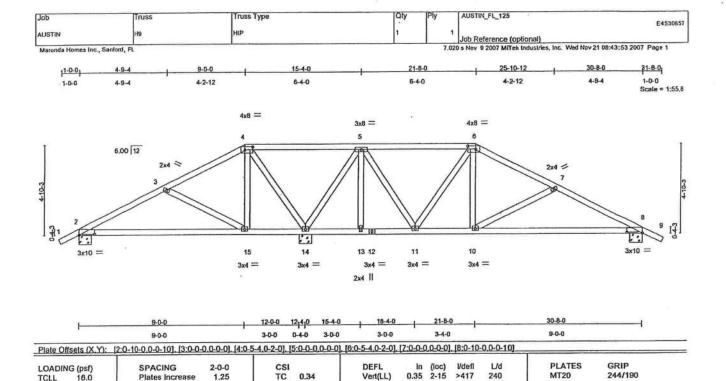
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 321 lb uplift at joint 2, 471 lb uplift at joint 14 and 209 lb uplift at Joint 8.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEX REFERENCE PAGE MII-7473 BEFORE USE. A MANUMO - verty design parameters and READ NOTES OF THIS AND INCIDENT OF THE ABOUT OF THE ABOUT





LUMBER

TCLL

TCDL

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.2

16.0

7.0

10.0

BRACING

Vert(LL)

Vert(TL)

Horz(TL)

0.24 2-15 >605

0.02

8 n/a

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Weight: 162 lb

Rigid celling directly applied or 6-0-0 oc bracing.

180

n/a

REACTIONS (lb/size) 2=364/0-8-0, 14=1651/0-8-0, 8=694/0-8-0

Plates Increase

Lumber Increase

Code FBC2004/TPI2002

Rep Stress Incr

1.25

1.25

YES

Max Horz 2=93(LC 6) Max Uplift2=331(LC 6), 14=-499(LC 5), 8=-202(LC 7)

Max Grav 2=394(LC 10), 14=1651(LC 1), 8=712(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD **BOT CHORD** 1-2=0/21, 2-3=-254/370, 3-4=-14/222, 4-5=-202/600, 5-6=-314/127, 6-7=-691/191, 7-8=-912/281, 8-9=0/21 2-15=234/213, 14-15=-96/72, 13-14=-11/218, 12-13=-11/218, 11-12=-11/218, 10-11=0/591, 8-10=-122/789 3-15=-274/289, 4-15=-516/445, 4-14=-894/778, 5-14=-1070/452, 5-13=0/162, 5-11=-146/504, 6-11=-493/160,

0.34

TC BC 0.54

WB 0.48

(Matrix)

6-10=0/435, 7-10=-239/186

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

- Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 331 lb uplift at joint 2, 499 lb uplift at joint 14 and 202 lb uplift at joint 8.

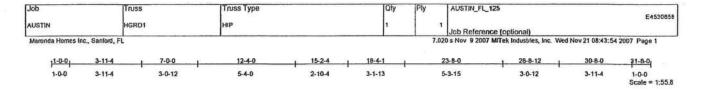
LOAD CASE(S) Standard

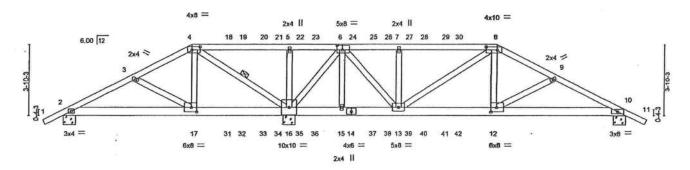
This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-7473 BEFORE USE.

Design volid for use only with Miter connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. For linus designer, Bracing show for followed support of individual web members only. Additional temporary bracing to insure stocially during constructions it the responsibility of the building designer. For general guidance regarding tobaccillan, quality control, strage, delivery, erection and bracing, consult. AMSI/IPI1 Quality Citleria, DS8-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.







		1-0-0	16	TV	1921	10.11	_	20.0.0		00.00	
		7-0-0	5-4	L 0	2-10-4	3-1-13		5-3-15	n n	7-0-0	
Plate Of		[2:0-0-0,0-0-0], [3:0-0-0,0	-0-0], [4:0-5-4,	0-2-0], [5:0-	-0-0,0-0-0], [6:0	-3-8,0-3-0], [7	:0-0-0,0-0-0], [8:0-7-0,0	-2-0], [9:0-0-	-0,0-0-0], [10:0-0-0,0-0	0-0], [12:0-3-8
_		.0-3-0]. [17:0-3-8.0-3-0]		_						T	
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	In (loc)	V defl	L/d	PLATES	GRIP
TCLL	16.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.06 12-13	>999	240	MT20	244/190
COL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.13 12-13	>999	180	100000000	
BCLL	10.0	Rep Stress Incr	NO	WB	0.64	Horz(TL)	-0.03 2	n/a	n/a		
BCDL	10.0	Code FBC2004/TF	12002	(Matri	ix)					Weight: 186 lb	

BRACING

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except*

4-6 2 X 4 SYP No.1D, 6-8 2 X 4 SYP No.1D

BOT CHORD 2 X 6 SYP No.2

2 X 4 SYP No.2

REACTIONS (lb/size) 2=546/0-8-0, 16=3961/0-8-0, 10=1390/0-8-0

(ID/SIZE) Z=0400-0-1, Max Horz 10=80(LC 7) Max Uplift2=444(LC 5), 16=-2380(LC 8), 10=-672(LC 8) Max Grav 2=603(LC 9), 16=3961(LC 1), 10=1398(LC 10)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/24, 2-3=-744/504, 3-4=-629/514, 4-18=-704/1391, 18-19=-704/1392, 19-20=-704/1392, 20-21=-705/1392, 5-21=-705/1392, 5-22=-704/1392, 22-23=-704/1392, 6-23=-704/1392, 6-24=-1407/811, 24-25=-1407/811, 25-26=-1407/811, 7-26=-1407/811, 7-27=-1406/810, 27-28=-1406/811, 28-29=-1406/811, 29-30=-1407/811, TOP CHORD

8-30=-1407/811, 8-9=-2343/1236, 9-10=-2420/1181, 10-11=0/24

2-17=396/629, 17-31=-409/595, 31-32=-409/595, 32-33=-409/595, 33-34=-409/595, 16-34=-409/595, 16-35=0/141, 35-36=0/141, 15-36=0/141, 14-15=0/141, 14-37=0/141, 37-38=0/141, 13-38=0/141, 13-39=-1082/2136, 39-40=-1062/2136, 40-41=-1062/2136, 41-42=-1062/2136, 12-42=-1062/2136, 10-12=-1008/2093 **BOT CHORD**

3-17=-112/89, 4-17=-494/1018, 4-16=-2242/1426, 5-16=-584/653, 6-16=-2222/1186, 6-15=0/105, 6-13=-1135/2025,

7-13=563/653, 8-13=884/449, 8-12=267/1015, 9-12=102/87

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2pst; BCDL=6.0psf; Calegory II; Exp B; enclosed; MWFRS (low-rise); porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.

Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 444 lb uplift at joint 2, 2380 lb uplift at joint 16 and 672 lb uplift at joint 10.

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007

Structural wood sheathing directly applied or 3-8-14 oc purlins.

Rigid ceiling directly applied or 7-7-10 oc bracing.

Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REPERENCE PAGE MII-7473 BEPORE USE. Design vold for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not introduced building component is for lateral support of individual web members only. Additional temporary brocing to insure stability during accessfunction is the responsibility of the execut. Additional permanent brocing of the overal structure is the responsibility of the building designer. For general guidance regarding fobrication, quality control. Storage, delivery, erection and brocing, consul. ANSI/TPH Quality Criteria, DSB-89 and BCSII Building Component Solely Information available from Truss Piote Institute, SSS D'Onotifo Drive, Modeson, WI SSSTPS.



Job	Truss	Truss Type	Qty	Ply	AUSTIN_FL_125	4530658
AUSTIN	HGRD1	HIP	1	1	Job Reference (optional)	4330030

Maronda Homes Inc., Sanford, FL

7.020 s Nov 9 2007 MiTek Industries, Inc. Wed Nov 21 08:43:54 2007 Page 2

6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 116 lb down and 194 lb up at 23-8-0, 116 lb down and 194 lb up at 21-7-4, 116 lb down and 194 lb up at 20-11-4, 116 lb down and 194 lb up at 18-11-4, 116 lb down and 194 lb up at 16-11-4, 116 lb down and 194 lb up at 12-11-4, 116 lb down and 194 lb up at 12-11-4, 116 lb down and 194 lb up at 12-11-4, 116 lb down and 194 lb up at 7-0-0, 116 lb down and 194 lb up at 7-0-0, 116 lb down and 194 lb up at 7-0-0, 116 lb down and 194 lb up at 7-0-0, 116 lb down and 194 lb up at 9-0-12, 116 lb down and 194 lb up at 9-8-12, 116 lb down and 194 lb up at 11-8-12, 116 lb down and 194 lb up at 13-8-12, 116 lb down and 194 lb up at 15-8-12, 116 lb down and 194 lb up at 17-8-12, 116 lb down and 194 lb up at 19-8-12, and 116 lb down and 194 lb up at 21-8-12, and 116 lb down and 194 lb up at 23-8-0 on top chord, and 644 lb down and 345 lb up at 23-8-0, 92 lb down at 21-7-4, 92 lb down at 20-11-4, 92 lb down at 18-11-4, 92 lb down at 16-11-4, 92 l lb up at 23-8-0, 92 lb down at 9-0-12, 92 lb down at 9-8-12, 92 lb down at 11-8-12, 92 lb down at 13-8-12, 92 lb down at 15-8-12, 92 lb down at 17-8-12, and 92 lb down at 19-8-12, and 92 lb down at 21-8-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others. 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

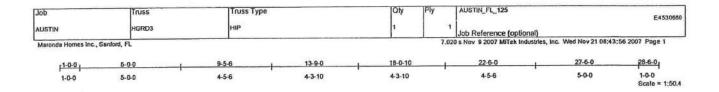
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

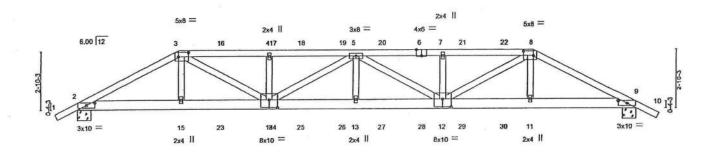
Uniform Loads (plf)

Vert: 1-4=-46, 4-8=-46, 8-11=-46, 2-10=-40

Concentrated Loads (lb)
Vert: 4=.116(F) 8=.116(F) 17=.644(F) 6=.116(F) 15=.92(F) 12=.644(F) 18=.116(F) 20=.116(F) 22=.116(F) 25=.116(F) 27=.116(F) 29=.116(F) 30=.116(F) 31=.92(F) 33=-92(F) 35=-92(F) 37=-92(F) 39=-92(F) 41=-92(F) 42=-92(F)







9		5-0-0	9-5-6		13-9-0	18	-0-10			22-6-0	27-6-0	
		5-0-0	4-5-6		4-3-10	4-	3-10			4-5-6	5-0-0	,
Plate Offset	s (X.Y): [2:0-5-0.0-1-7]. [3:0-6-0.0	2-8]. [6:0-3-0	.Edge]. [8:0	-6-0.0-2-8]. [9:0-5-0.0-1-7]. [1	2:0-5-0.	0-4-8].	[14:0-5-0	0.0-4-8]		
LOADING (psf) 16.0	SPACING Plates Increase	2-0-0 1.25	CSI	0.56	DEFL Vert(LL)	in 0.34	(loc)	Vdefl >944	L/d 240	PLATES MT20	GRIP 244/190
CDL	7.0	Lumber Increase Rep Stress Incr	1.25 NO	BC WB	0.74	Vert(TL) Horz(TL)	-0.60 0.11	13	>535 n/a	180 n/a		
	0.0	Code FBC2004/TF		(Mati		V4000 TAX 10 TO \$1.			10,000	BARCASK	Weight: 153 lb)

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.1D

WEBS

2 X 4 SYP No.2

BRACING

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 2-5-13 oc purlins.

Rigid celling directly applied or 4-10-7 oc bracing.

REACTIONS (lb/size) 2=2087/0-8-0, 9=2108/0-8-0

Max Horz 2=65(LC 7) Max Uplift2=1071(LC 7), 9=1091(LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/25, 2-3=-3979/2011, 3-16=-5219/2699, 16-17=-5218/2699, 4-17=-5218/2699, 4-18=-5218/2699, TOP CHORD

18-19=5218/2699, 5-19=-5218/2699, 5-20=-5240/2718, 6-20=-5240/2718, 6-7=-5240/2718, 7-21=-5240/2717,

21-22-5240/2717, B-22-5241/2718, B-9-4027/2058, B-10=0/25 2-15=-1749/3479, 15-23=-1746/3504, 23-24=-1746/3504, 14-24=-1746/3504, 14-25=-2952/5861, 25-26=-2952/5861, **BOT CHORD**

13-26-2952/5861, 13-27-2952/5861, 27-28-2952/5861, 12-28-2952/5861, 12-29-1750/3547, 29-30-1750/3547,

11-30=1750/3547, 9-11=-1754/3522

3-15=0/415, 3-14=-1029/1992, 4-14=-354/388, 5-14=-745/396, 5-13=0/350, 5-12=-720/373, 7-12=-352/383,

8-12=-1009/1967, 8-11=0/430

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1071 lb uplift at joint 2 and 1091 lb uplift

6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 236 lb down and 306 lb up at 5-0-0, 69 lb down and 132 lb up at 7-0-12, 77 lb down and 145 lb up at 9-0-12, 77 lb down and 145 lb up at 11-0-12, 77 lb down and 145 lb up at 13-0-12, 77 lb down and 145 lb up at 13-0-12, 69 lb down and 132 lb up at 15-0-12, 77 lb down and 145 lb up at 17-0-12, 69 lb down and 132 lb up at 19-0-12, and 69 lb down and 132 lb up at 21-0-12, and 236 lb down and 306 lb up at 22-6-0 on top chord, and 162 lb down at 5-0-0, 57 lb down at 7-0-12, 57 lb down at 9-0-12, 57 lb down at 13-0-12, 57 lb down at 15-0-12, 57 lb down at 17-0-12, 57 lb down at 19-0-12, and 57 lb down at 21-0-12, and 162 lb down at 22-5-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Continued on page 2

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL 7473 BEFORE USE. Lancing of the use only with Miller connectors. This design is based only upon porometers show, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer only this designer control is to a control of the co



Truss	Truss Type	Qty	Ply	AUSTIN_FL_125	D-EAST-MANUAL PARTY
HGRD3	HIP	1	,	Inh Reference (policost)	E4530660

Maronda Homes Inc., Sanford, FL

7.020 s Nov 9 2007 MiTek Industries, Inc. Wed Nov 21 08:43:56 2007 Page 2

LOAD CASE(S) Standard

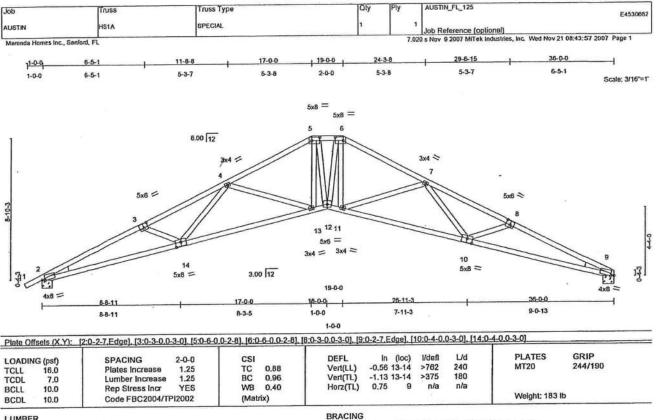
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (pif) Vert: 1-3=-46, 3-8=-46, 8-10=-46, 2-9=-40

Concentrated Loads (lb)

Vert: 3-196(F) 6-77(F) 8-196(F) 15-162(F) 11-162(F) 16-69(F) 17-77(F) 18-77(F) 19-77(F) 20-77(F) 21-69(F) 22-69(F) 23-57(F) 24-57(F) 25-57(F) 26-57(F) 27-57(F) 28-57(F) 29-57(F) 30-57(F)





TOP CHORD

BOT CHORD

Structural wood sheathing directly applied. Rigid ceiling directly applied or 2-2-0 oc bracing.

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 *Except*

2-14 2 X 4 SYP No.1D, 9-10 2 X 4 SYP No.1D

WEBS 2 X 4 SYP No 2

WEDGE

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

REACTIONS (lb/size) 9=1518/0-8-0, 2=1585/0-8-0 Max Horz 2=159(LC 6)

Max Uplift9=280(LC 7), 2=362(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/18, 2-3=-4842/1457, 3-4=-4669/1334, 4-5=-3450/989, 5-6=-3172/985, 6-7=-3443/991, 7-8=-4648/1350, TOP CHORD

BOT CHORD WEBS

7-11=771/415, 7-10=-96/661, 8-10=-140/272

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Bearing at joint(s) 9, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 9 and 362 lb uplift at

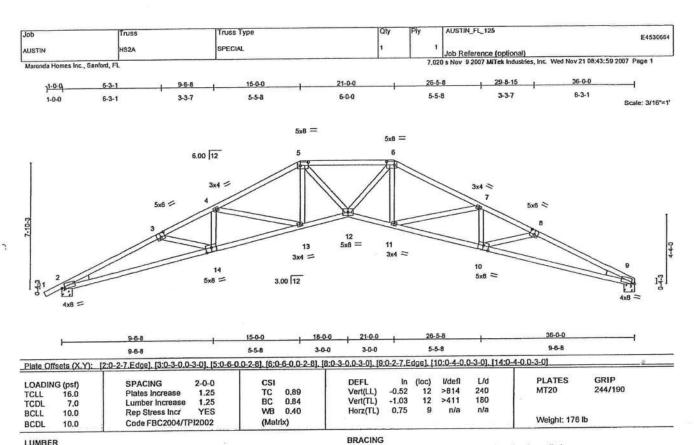
loint 2.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEX REFERENCE PAGE MII-7473 BEFORE USE.





TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.1D

WEBS 2 X 4 SYP No.2

WEDGE

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

REACTIONS (lb/size) 9=1518/0-8-0, 2=1585/0-8-0

Max Horz 2=143(LC 6)

Max Uplift9=-267(LC 7), 2=-348(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/18, 2-3=-4769/1492, 3-4=-4607/1362, 4-5=-3753/1147, 5-6=-4143/1242, 6-7=-3755/1151, 7-8=-4619/1391,

2-14=-1264/4321, 13-14=-1068/4230, 12-13=-758/3438, 11-12=-762/3440, 10-11=-1091/4240, 9-10=-1311/4342 3-14=-91/197, 4-14=-0/386, 4-13=-802/303, 5-13=-104/513, 5-12=-226/1190, 6-12=-220/1188, 6-11=-108/515, BOT CHORD WEBS

7-11=-811/323, 7-10=-18/395, 8-10=-101/219

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

- 3) Provide adequate drainage to prevent water ponding.
 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) Bearing at joint(s) 9, 2 considers parallel to grain value using ANSUTPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 267 lb uplift at joint 9 and 348 lb uplift at joint 2.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

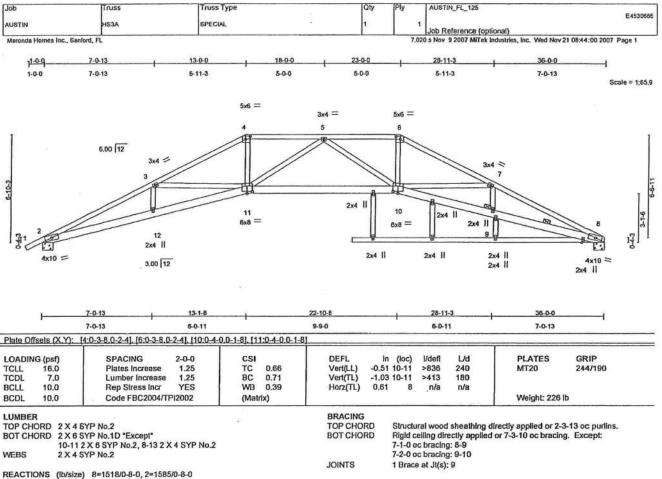
Structural wood sheathing directly applied.

Rigid ceiling directly applied or 5-8-9 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE.



Edenton, NC 27932



Max Horz 2=132(LC 6)

Max Uplift8=-251(LC 7), 2=-333(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/22, 2-3=-5019/1526, 3-4=-4233/1280, 4-5=-3790/1214, 5-6=-3793/1221, 6-7=-4237/1288, 7-8=-5042/1580 **BOT CHORD**

2-12=-1299/4530, 11-12=-1300/4548, 10-11=-1051/3935, 9-10=-1349/4569, 8-9=-1353/4553
3-12=0/206, 3-11=-666/357, 4-11=-341/1603, 5-11=-327/240, 5-10=-325/237, 6-10=-346/1605, 7-10=-683/399,

WEBS

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

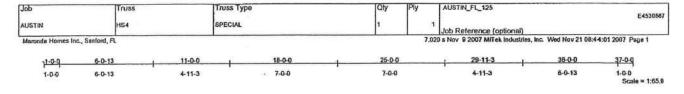
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Bearing at joint(s) 8, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 8 and 333 lb uplift at loint 2.

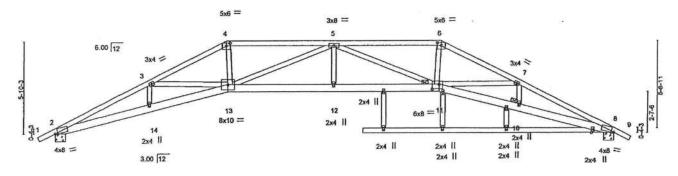
LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with Miles connectors. This design is based only upon parameters and with Miles connectors. This design is based only upon parameters show, and is for an individual building compone Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer. Brocing is for lateral support of individual web members only. Additional temporary brocing to insure shifty during construction is the responsibility erector. Additional permanent brocing of the everall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and brocing, consult. AMSI/FIT (quality Catleria, DSB-B9 and BCSIT Building Composately Information available from Trust Plate Intillule, 333 D'Onotifo Drive, Maddon, WI 53719.







9	B-	0-13	-1-8	1	8-0-0	2	4-10-8		-	29-11-3	36-0-0		
1	6-	0-13 5-	0-11	6	-10-8		-10-8			5-D-11	6-0-13		
Plate Offs	ets (X.Y): [2:0-1-15.0-0-9], [4:0-3-0,	0-2-0]. [6:0-3	-0.0-2-0]. [B:	0-1-15.0-0-9]. [11:0-5-4.0-3-8]							
LOADING	(psf)	SPACING	2-0-0	CSI		DEFL		(loc)	Vdefi	L∕d	PLATES	GRIP	
TCLL	16.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.50	12	>850	240	MT20	244/190	
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.99 1	2-13	>428	180			
BCLL	10.0	Rep Stress Incr	YES	WB	0.74	Horz(TL)	0.61	8	n/a	n/a			
BCDL	10.0	Code FBC2004/TF	212002	(Mati	ix)						Weight: 229 lb	E	120.000

BRACING

TOP CHORD

BOT CHORD JOINTS

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.1D *Except* 8-15 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.2

REACTIONS (Ib/size) 2=1584/0-8-0, 8=1584/0-8-0 Max Horz 2=108(LC 6) Max Uplift2=-315(LC 6), 8=-315(LC 7)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=0/22, 2-3=-4994/1481, 3-4=-4488/1388, 4-5=-4049/1292, 5-6=-4049/1292, 6-7=-4488/1368, 7-8=-4994/1481,

TOP CHORD

BOT CHORD

2-14=-1208/4494, 13-14=-1214/4515, 12-13=-1248/4774, 11-12=-1248/4774, 10-11=-1214/4515, 8-10=-1208/4494 3-14=0/167, 3-13=-391/249, 5-12=0/282, 5-11=-930/298, 7-11=-391/261, 7-10=0/167, 5-13=-930/300, 6-11=-364/1703, WEBS

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 1) Whild: ASCE 7-02; 125mph (3-second gust): h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for
- MWFRS for reactions specified.

 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Bearing at Joint(s) 2, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 315 lb uplift at joint 2 and 315 lb uplift at joint 8.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

Structural wood sheathing directly applied or 2-8-0 oc purlins. Rigid ceiling directly applied or 7-8-2 oc bracing. 1 Brace at JI(s): 11, 10

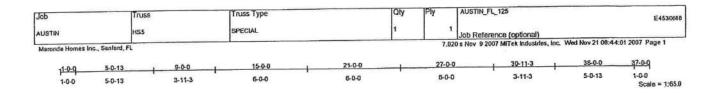
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL 7473 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE FAGE MET-1973 BEFORE UBB.

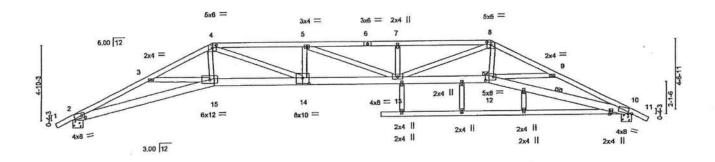
Design void for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design parameters and proper incorparation of component is responsibility of building designer- not it us designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer. For openeral guidance regarding executor, additional permanent bracing of the overal structure is the responsibility of the building designer. For openeral guidance regarding tobication, quality conirol, storage, delivery, etection and bracing, consult.

ANS/IPF1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Modison, WI 53719.







	9-1-8		15-0-0		21-0-0	20.10	-				
	9-1-8	٠,	5-10-8	N. S.	6-0-0	5-10-	8		9-1-8		
LOADING (psf)	2:0-2-4.0-2-0], [4:0-3-0.0 SPACING	2-0-0	0-2-0]. [10: CSI TC	0-2-11.0-0-9 0.57	DEFL Vert(LL)	In (loc) -0.64 13-14	4-8] Vdefl >665	L/d 240	PLATES MT20	GRIP 244/190	_
TCLL 16.0 TCDL 7.0 BCLL 10.0 BCDL 10.0	Plates Increase Lumber Increase Rep Stress Incr Code FBC2004/T	1.25 1.25 YES PI2002	BC WB (Matr	0.69 0.34	Vert(TL) Horz(TL)	-1.26 13-14 0.62 10	>335 n/a	180 n/a	Weight: 228	lb	

BRACING

JOINTS

TOP CHORD

BOT CHORD

26,10.8

7-3-0 oc bracing: 10-12

1 Brace at Jt(s): 12

38-0-0

Structural wood sheathing directly applied or 2-4-3 oc purlins. Rigid ceiling directly applied or 6-7-11 oc bracing. Except:

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 6 SYP No.4D *Except*

10-16 2 X 4 SYP No.2

2 X 4 SYP No.2 WEBS

REACTIONS (lb/size) 2=1584/0-8-0, 10=1584/0-8-0 Max Horz 2=-93(LC 7) Max Uplift2=-295(LC 6), 10=-295(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/22, 2-3=4835/1610, 3-4=4753/1444, 4-5=-5526/1846, 5-6=-5514/1843, 6-7=-5514/1843, 7-8=-5514/1843, TOP CHORD

8-9=4762/1448, 9-10=-4839/1611, 10-11=0/22

0-3-47621443, 3-10-4531611, 10-11-622 2-15=-1336/4371, 14-15=-1088/4298, 13-14=-1595/5541, 12-13=-1091/4308, 10-12=-1337/4375 3-15=-37/270, 4-14=-532/1417, 5-14=-307/255, 5-13=-115/71, 7-13=-276/257, 8-13=-523/1393, 9-12=-44/277, WEBS

8-12=-230/1390, 4-15=-223/1372

NOTES

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for

MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Bearing at joint(s) 2, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

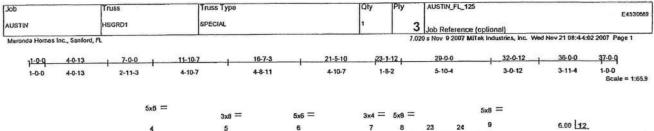
capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 295 lb uplift at joint 2 and 295 lb uplift at loint 10.

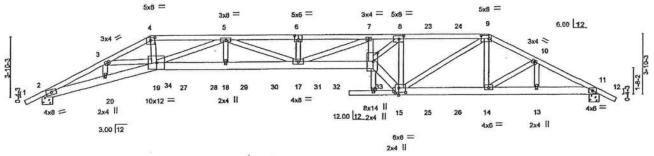
LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USB. WARNING - Verity design parameters and ISAAD NOTES ON THIS AND INCLUDED BUTTER HAS EXERCE PAGE BUTTER SECTION ON A PARAMETER OF THE AND INCLUDED BUTTER HAS EXERCE PAGE BUTTER SECTION OF THE PAGE AND THE CONTROL OF THE PAGE AND THE CONTROL OF THE PAGE AND THE CONTROL OF THE PAGE AND THE PAGE







100	43-1	7-4-8	11-10-7	16-	7-3	21-5-10	23-1-12	29-0	-0	32-0-12	38-0-0	
Į.	4-3-1	3-1-7	4-5-15	4-8	-11	4-10-7	1-8-2	5-10	4	3-0-12	3-11-4	
Plate Offsets	(X.Y): 14:0	3-0,0-2-0], [6:0-3-0,0	3-0]. [8:0-3-8	.0-2-8]. [9:0	6-0.0-2-8].	[15:0-1-12,0-2-12]. [15:0-5-4.0-3	-8], [16:0	6-12.0-3-8	ļ		
LOADING (p		SPACING	2-0-0 1.25	CSI TC	0.46	DEFL Vert(LL)	in (loc) 0.75 16-17	Vdefl >563	L/d 240	PLATES MT20	GRIP 244/190	
CDL	6.0 7.0	Plates Increase Lumber Increase	1.25	ВС	0.88	Vert(TL)	-1.26 16-17	>338	180		213/105	
	0.0	Rep Stress Incr Code FBC2004/TF	NO 212002	WB (Matr	0.79 ix)	Horz(TL)	-0.47 2	n/a	n/a	Weight: 6	653 lb	

BRACING

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.1D

BOT CHORD 2 X 6 SYP No.2 "Except" 16-19 2 X 6 SYP No.1D, 15-21 2 X 4 SYP No.2

WEB\$ 2 X 4 SYP No.2

REACTIONS (lb/size) 11=3534/0-8-0, 2=3592/0-8-0 Max Horz 11=-80(LC 6)

Max Uplift11=-1665(LC 8), 2=-1691(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

9-10=-6957/3391, 10-11=-6833/3194, 11-12=0/25, 4-5=-12589/6331, 5-6=-18762/9386, 6-7=-18762/9386, TOP CHORD

7-8=-18144/9016, 8-23=-9223/4588, 23-24=-9223/4588, 9-24=-9223/4588, 1-2=0/22, 2-3=-11630/5630,

3-4=-12800/6409

15-25=3021/6267, 25-26=3021/6264, 14-26=3020/6262, 13-14=2812/5981, 11-13=-2812/5981, 15-16=-5882/11965, BOT CHORD

19-27=8362/16846, 27-28=8362/16846, 18-28=8362/16846, 18-29=8362/16846, 29-30=8362/16846, 17-30=8362/16846, 17-31=8962/18164, 31-32=8964/18168, 32-33=8965/18171, 16-33=8967/18173,

2-20=-4992/10394, 20-34=-5040/10490, 19-34=-5156/10675 10-13=-118/155, 10-14=-311/381, 9-14=-408/1039, 9-15=-1790/3464, 8-15=-9174/4681, 8-16=-6409/12898, 7-16=-324/209, 7-17=-452/686, 6-17=-208/154, 5-17=-1025/2102, 5-18=-369/769, 5-19=-4643/2383, 4-19=-2772/5570,

3-19=-901/1444, 3-20=-282/241

NOTES

WEBS

1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2 X 6 - 2 rows at 0-6-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to
ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.

5) Provide adequate drainage to prevent water ponding.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity on November 21,2007. This is not of bearing surface of bearing surface.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1665 lb uplift at joint 11 and 1691 lb uplift document. Official sealed at joint 2.

Continued on page 2

This document was originally issued by Lassiter, Frank drawings are available

Structural wood sheathing directly applied or 5-6-9 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify deelign parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-1473 BEFORE USE.

Design vold for use only with Milek connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not huss designer. Bracing shown is for taleral support of individual web members only. Additional temporary bracing to insus subsity during construction is the responsibility of the building designer, for general guidance regarding fobiological quality control, storage, delivery, execution and bracing, consult — ABSI/IPI Quality Criteria, DSS-89 and BCSI1 Building Component Safety Information available from Trus Plate Institute, 583 D'Onotrio Drive, Madison, WI S3719.



HECON SECUL	Job	Truss	Truss Type		Oty	Ply	AUSTIN_FL_125	4530669
AUSTIN 3 Job Reference (optional)	AUSTIN	HSGRD1	SPECIAL	¢.	1	3	i (5	433000

Meronda Hornes Inc., Senford, FL

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 19 lb down and 72 lb up at 29-0-0, 19 lb down and 72 lb up at 27-0-12, and 19 lb down and 72 lb up at 25-0-12, and 19 lb down and 72 lb up at 25-0-12 on top chord, and 738 lb down and 433 lb up at 29-0-0, 190 lb down and 93 lb up at 27-0-12, 190 lb down and 93 lb up at 25-0-12, 190 lb down and 93 lb up at 23-1-12, 258 lb down and 185 lb up at 21-0-12, 258 lb down and 185 lb up at 17-0-12, 258 lb down and 185 lb up at 11-0-12, and 258 lb down and 185 lb up at 11-0-12, 9-0-12, and 768 lb down and 519 lb up at 7-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

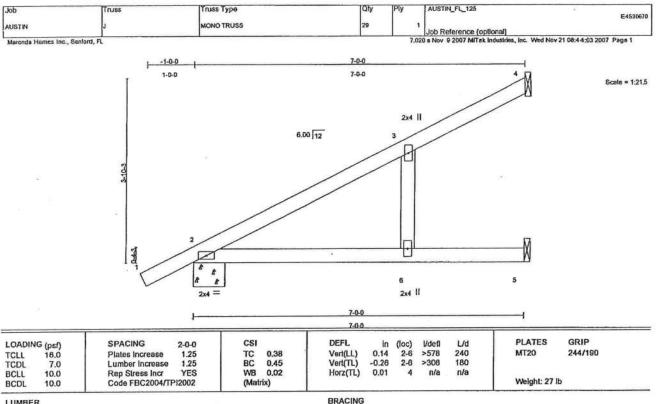
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (pil)
Vert: 9-12=-46, 4-9=-46, 1-4=-46, 11-15=-40, 15-16=-40, 16-19=-40, 2-19=-40

Concentrated Loads (lb)

Vert: 9=-19(F) 15=-190(F) 14=-738(F) 8=-19(F) 23=-19(F) 24=-19(F) 25=-190(F) 26=-190(F) 27=-258(F) 28=-258(F) 29=-258(F) 30=-258(F) 31=-258(F) 32=-258(F) 32=-258(F) 34=-758(F) 34=-758(F)





TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.2 WERS

REACTIONS (lb/size) 4=136/Mechanical, 2=355/0-8-0, 5=141/Mechanical Max Horz 2=178(LC 6)

Max Uplift4=-64(LC 6), 2=-120(LC 6), 5=-34(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-3=-126/10, 3-4=-46/56 BOT CHORD 2-6=0/0, 5-8=0/0

WEBS

3-6=14/132

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 64 lb uplift at joint 4, 120 lb uplift at joint 2 and 34 lb uplift at loint 5.
- 5) Attach with (2) 16d Common Toe-Nalls (0.162"x3.5") at Joints 4 and 5.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

Structural wood sheathing directly applied or 6-0-0 oc purlins, Rigid ceiling directly applied or 10-0-0 oc bracing.

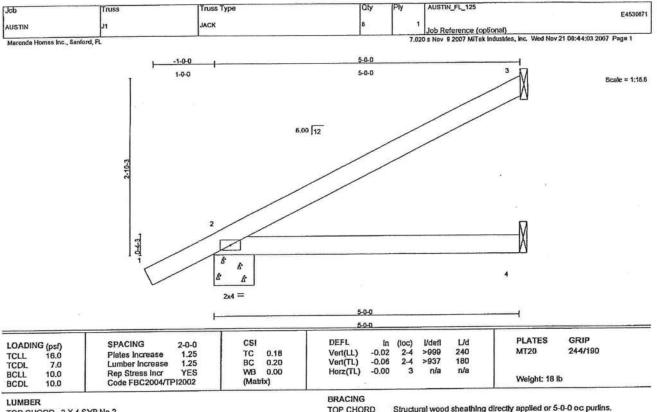
WARNING - Verify design parameters and XEAD NOTES ON THIS AND INCLUDED MITEX REFERENCE PAGE MILITATS BEFORD USE.

Design valid for use only with MITEX connectors. This design is based only upon parameters shown, and is for an individual building component.

Appicability of design parameters and proper incorporation of component it responsibility of building designer—not first designer, Bracing shown is for lateral support of individual web members only. Additional permanents bracing of the overall structure is the responsibility of the substitution in the responsibility of the valid incorporation of the overall structure is the responsibility of the valid incorporation of the overall structure is the responsibility of the valid incorporation of the overall structure is the responsibility of the valid incorporation of the overall structure. The responsibility of the valid incorporation of the overall structure is the responsibility of the valid incorporation of the overall structure.

ANSI/IPTI Quality Criteria, DSS-89 and BCS11 Building Component Safety Information available from Trus Piole Institute, SSS D'Onofrio Drive, Modison, WI SS719. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE.





TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BOT CHORD

Rigid celling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=96/Mechanical, 2=272/0-8-0, 4=92/Mechanical Max Horz 2=136(LC 6)

Max Uplift3=91(LC 6), 2=-114(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-3=-72/36

BOT CHORD

NOTES

- (b) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and G-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at joint 3 and 114 lb uplift at
- 5) Attach with (2) 16d Common Toe-Nalls (0.182"x3.5") at joints 3 and 4.

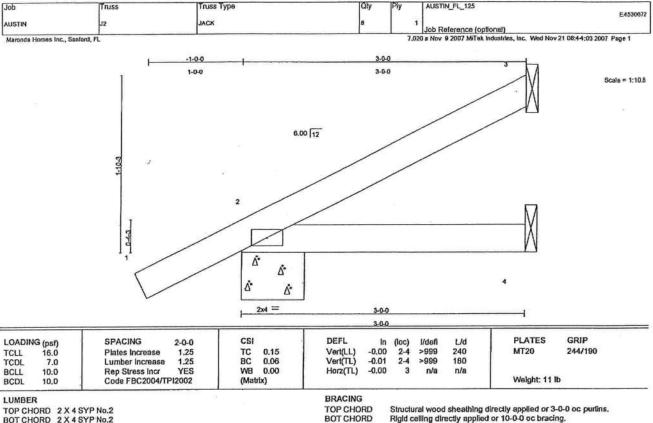
LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCS PAGE MIL-1473 BEFORE USE.

Design valid for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not inus designer. Broching shown is for tallerally support of individual web members only. Additional emprovery backing to Insura subskipt during construction is the responsibility of the erector. Additional parameters that the overall structure is the responsibility of the building designer. For general guidance regarding for the control parameters are designed to the control structure of the overall structure is the responsibility of the building designer. For general guidance regarding for the following designer. Additional parameters are designed to the control structure in the responsibility of the building designer. For general guidance regarding the following designer. For general guidance regarding the following designer. For general guidance regarding the subsking designer. For general guidance regarding the following designer.





REACTIONS (lb/size) 3=43/Mechanical, 2=194/0-8-0, 4=52/Mechanical Max Horz 2=95(LC 6) Max Uplift3=37(LC 6), 2=-113(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-3=-39/15

BOT CHORD

NOTES

- (5)
 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 3) Refer to girder(s) for truss to Iruss connections.

- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 3 and 113 lb uplift at
- 5) Attach with (2) 16d Common Toe-Nalls (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard

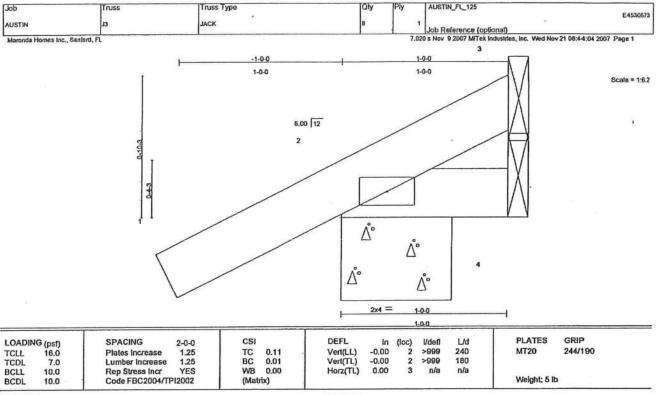
This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REPERENCE PAGE MIL-14473 BEFORE USE.

Design valid for use only with Milek connector. This design is based only upon parameters shown, and is for an included building component.

Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not invisides the responsibility of building designer - not invisides the responsibility of the certain support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding tobrication, quality control, strategy, delivery, erection and bracing, consult. AMSI/IPTI Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, SS3 D'Onotrio Drive, Madison, WI SS719.





LUMBER

TOP CHORD 2X4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 1-0-0 oc purlins. Rigid celling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/slze) 2=124/0-8-0, 4=18/Mechanical, 3=-11/Mechanical Max Horz 2=54(LC 6) Max Uplift2=116(LC 6), 3=-11(LC 1) Max Grav2=124(LC 1), 4=18(LC 1), 3=27(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension 1-2=0/21, 2-3=-27/26 2-4=0/0 TOP CHORD

BOT CHORD

NOTES

- NOTES (5)

 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=8.0psf; Calegory II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for
- MWFRS for reactions specified.

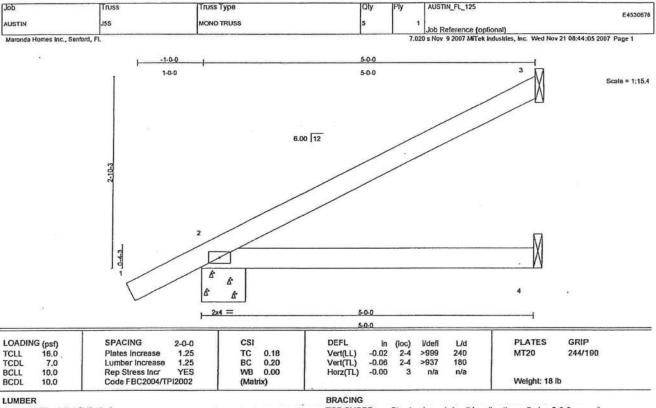
 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 116 lb uplift at joint 2 and 11 lb uplift at
- 5) Attach with (2) 16d Common Toe-Nails (0.162*x3.5") at joints 3 and 4.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ HOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. MARTING - Verify design parameters and READ NOTES ON THIS AND INCLUDED BITER RESIDENCE TAGE BUTCH BOARD BOARD DESIgn valid for use only with Milital connection. This design is based only upon porometers show and is for on individual bublishing component. Applicability of design parameters and proper incorporation of component is responsibility of building designer—not trus designer. Bracing shown to facilitate the upport of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the stability during construction is the responsibility of the stability of the stab





TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. TOP CHORD **BOT CHORD**

REACTIONS (lb/size) 3=96/Mechanical, 2=272/0-8-0, 4=92/Mechanical Max Horz 2=136(LC 6)

Max Uplift3=91(LC 6), 2=114(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-72/36

2-4=0/0 BOT CHORD

NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections,
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at joint 3 and 114 lb uplift at joint 2.
5) Attach with (2) 16d Common Toe-Nalls (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REPERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Millex connectors. This design is based only upon prometers shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for taleral support of individual web members only. Additional temporary broting to insure shifting the prostruction is the responsibility of the standard permanent bracing of the overal structure is the responsibility of the studieng designer. For general guidance regarding closkcation, quality control, storage, delivery, exection and bracing, consult. ANSI/IPII Quality Criteria, DSB-89 and BCSII Building Component Safety Information. ovaliable from Truss Plate Institute, 583 D'Onotrio Drive, Modern, WI 53719.



Job	Truss . Tru	iss Type	Qty	Ply	AUSTIN_FL_1	125	E4530677
AUSTIN	J5SA MC	NO TRUSS	5		Job Reference	(IIII)	E4030077
Maronda Homes Inc., 9	Sanford, FL			7.0			ov 21 08:44:05 2007 Page 1
	 		5-0-0			2 🖂	
	I		5-0-0				Scale = 1:15.4
						/13	
		6,00	12				
			/	/ ,			
	3		/ /				
¥1	2-10-3						
	j						
	1 /						
	1,6					M	
	1					M	
	A*						
	. & &					3	
	2x4 =		5-0-0				
,			5-0-0				
LOADING (psf)	SPACING . 2-0-0	CSI		n (loc)	l/defl L/d		GRIP
TCLL 16.0 TCDL 7.0	Plates Increase 1.25 Lumber Increase 1.25	TC 0.22 BC 0.20	Vert(LL) -0.0 Vert(TL) -0.0		>999 240 >937 180		244/190
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.0		n/a n/a	i lenament in	
BCDL 10.0	Code FBC2004/TPI2002	(Matrix)	0.40.40.2000.000			Weight: 16	3 lb
LUMBER	The second second		BRACING				
TOP CHORD 2 X BOT CHORD 2 X			TOP CHORD BOT CHORD	Structu Rigid co	ral wood sheat eiling directly a	thing directly applied or applied or 10-0-0 oc brad	5-0-0 oc purlins. cing.

REACTIONS (lb/size) 1=198/0-8-0, 2=106/Mechanical, 3=92/Mechanical Max Horz 1=104(LC 6)

Max Uplift1=20(LC 6), 2=104(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=81/41 BOT CHORD 1-3=0/0

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

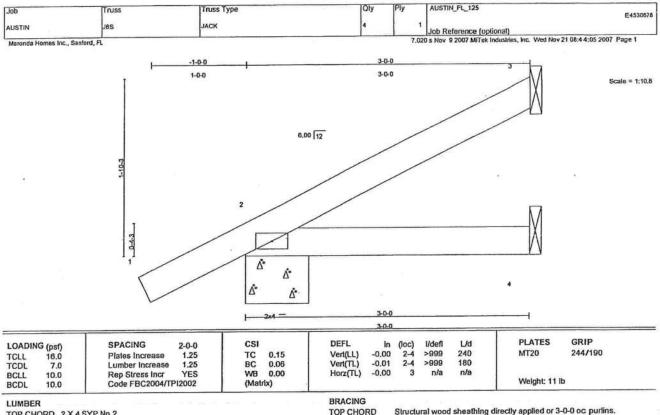
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 1 and 104 lb uplift at joint 2. 5) Attach with (2) 16d Common Toe-Nails (0.162*x3.5*) at joints 2 and 3.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design vold for use only with Milek connectors. This design is based only upon parameter shown, and is for on individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not fruss designer. Bracing shown is for fallers support of individual web members only. Additional temporary bracing to insue shiftly during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, for general guidance regarding to bracially under control structure is the responsibility of the building designer, for general guidance regarding to bracially under control structure, and a NASI/PTI Quality Criteria, DSS-89 and BCSII Building Component Safely Information available from Truss Piole Institute, SSS D'Onotifo Drive, Madison, WI 53719.





BOT CHORD

Rigid celling directly applied or 10-0-0 oc bracing.

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 3=43/Mechanical, 2=194/0-8-0, 4=52/Mechanical Max Horz 2=95(LC 6)

Max Uplift3=-37(LC 6), 2=-113(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-39/15

BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) Refer to girder(s) for truss to truss connections.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at Joint 3 and 113 lb uplift at ioint 2
- 5) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at Joints 3 and 4.

LOAD CASE(S) Standard

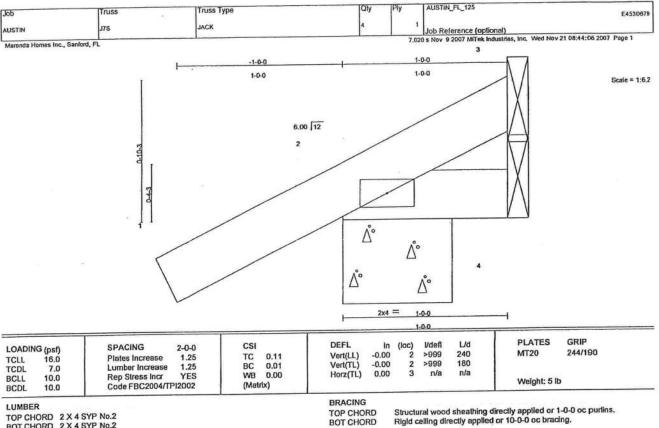
This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REPERENCE PAGE MIL-7473 BEFORE USE.

Design void for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not inus designer. Bracing shows is for taleral support of individual web members only. Additional temporary bracing to insure stability during construction is lite responsibility of the building designer. For general guidance regarding flobrication, quality coning, storage, delivery, execution and bracing, consult.

ANSI/IPI Quality Cities, DSS-89 and BCSI Building Component Safety Information available from Truss Plate Institute, SS3 D'Onotio Drive, Modison, WI 53719.





BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 2=124/0-8-0, 4=18/Mechanical, 3=-11/Mechanical

Max Horz 2=54(LC 6)

Max Uplift2=-116(LC 6), 3=-11(LC 1)

Max Grav2=124(LC 1), 4=18(LC 1), 3=27(LC 6)

FORCES (ib) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-27/26 2-4=0/0 TOP CHORD

BOT CHORD

- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 116 lb uplift at joint 2 and 11 lb uplift at
- joint 3.
 5) Attach with (2) 16d Common Toe-Nails (0.162*x3.5") at joints 3 and 4.

LOAD CASE(S) Standard

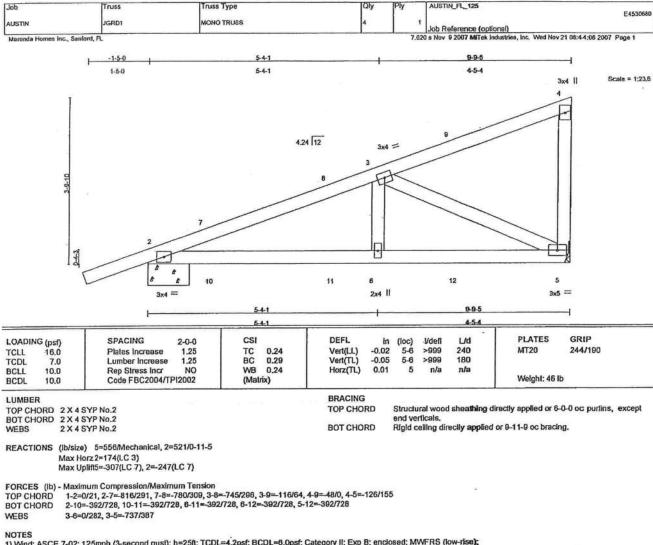
This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARMING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-1473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design parameters and proper incorporation of component is responsibility of building designer - not fins designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer, for general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, for general guidance regarding flobrication, quality control, storage, delivery, erection and bracing, consult. AMSUTPTI Quality Criteria, DSS-89 and BCS11 Building Component Safety Information. available from Trus Plate Institute, SS3 D'Onotifo Drive, Modison, WI S3719.





- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.80 plate grip DOL=1.60.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 307 lb uplift at joint 5 and 247 lb uplift at joint 2.
- Joint 2.

 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 43 lb up at 4-4-0, 43 lb up at 4-4-0, 59 lb down and 122 lb up at 7-1-15, 59 lb down and 122 lb up at 7-1-15, and 48 lb down at 1-6-1, and 52 lb down at 22 lb up at 1-6-1, 22 lb up at 1-6-1, 12 lb down at 4-4-0, 12 lb down at 4-4-0, and 52 lb down at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1,25, Plate Increase=1,25 Uniform Loads (plf)

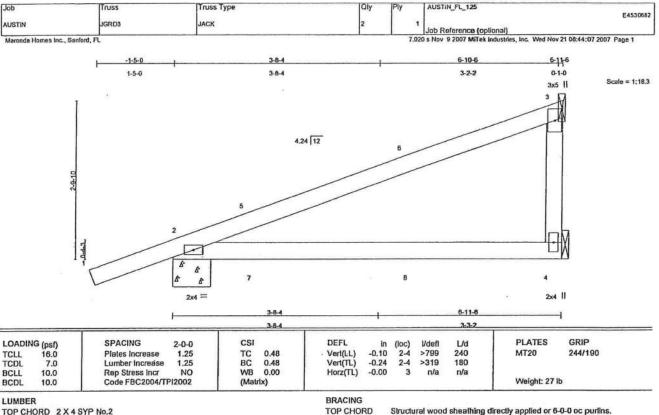
Vert: 1-4=-46, 2-5=-40

Verl: 8=8(F=4, B=4) 9=.118(F=.59, B=.59) 10=43(F=22, B=22) 11=-24(F=.12, B=-12) 12=-104(F=.52, B=.52)

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEH REFERENCE PAGE MII-7473 BEFORE USE. Dasign void for use only with Milek connector. This design is bosed only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not inust designer. Bracing show is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the series. Additional permanent bracing of the overall shucture is the responsibility of the building designer. For general guidance reporting in labeling designer, for general guidance reporting individual youthy control, storage, defivery, erection and bracing, consult. AMSI/TPI Quality Citleria, DSS-89 and SCS11 Suilding Component Salely Intermettion available from Truss Plate Institute, 583 D'Onotrio Drive, Modison, WI 53719.





BOT CHORD

Rigid celling directly applied or 10-0-0 oc bracing.

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.2

REACTIONS (lb/size) 2=358/0-8-0, 4=142/Mechanical, 3=149/Mechanical

Max Horz 2=132(LC 3)

Max Uplift2=-170(LC 7), 3=-164(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-5=-79/0, 5-6=-34/0, 3-6=-66/42 2-7=0/0, 7-8=0/0, 4-8=0/0

BOT CHORD

WERS 3-4=0/0

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise);
- Lumber DOL=1.60 plate grip DOL=1.60.

 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections,
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 170 lb uplift at joint 2 and 164 lb uplift at joint 3.

 5) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 9 lb down and 57 lb up at 4-4-0, 9 lb down and 57 lb up at 4-4-0, and 47 lb down at 1-6-1, and 47 lb down at 4-4-0, and 47 lb down at 4-4-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

8) Attach with (2) 16d Common Toe-Nalls (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-48, 2-4=-40

Concentrated Loads (lb) Vert: 6=-19(F=-9, B=-9) 7=41(F=21, B=21) 8=-33(F=-17, B=-17)

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE FAGE MIL-7473 BEFORE USE.

Design volid for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not inus designer. Bracing shown to raise to support of individual web members only. Additional temporary bracing to Insure slobility during construction is the responsibility of the erector. Additional permanent bracing of the overal structure is the responsibility of the building designer. For general guidance regarding fobication, quality control, storage, delivery, erection and bracing, consult — MSI/IPII Quality Cifferia, DSB-89 and BCS11 Building Component Safety Information available from Trus Plate Institute. SS3 D'Onotrio Drive, Modison, WI SS719.



Truss Truss Type QN AUSTIN FL 125 E4530683 AUSTIN IBI SPECIAL Job Reference (optional)
7.020 s Nov 9 2007 MiTek industries, Inc. Wed Nov 21 08:44:07 2007 Page 1 5-0-0 -1-0-0 1-0-0 5-0-0 Scale = 1:16,5 6.00 12 3.00 12 3x4 = SPACING DEFL **PLATES** GRIP LOADING (psf) 2-0-0 In (loc) I/defl L/d 18.0 Plates Increase 1.25 Vert(LL) -0.02 >999 240 MT20 244/190 TCLL 180 TCDL 7.0 Lumber Increase 1.25 BC 0.20 Vert(TL) -0.08 2-4 >909 WB 0.00 -0.00 BCLL 10.0 Rep Stress Incr YES Horz(TL) n/a n/a Code FBC2004/TP12002 (Matrix) Weight: 18 lb BCDL 10.0 BRACING LUMBER Structural wood sheathing directly applied or 5-0-0 oc purlins. TOP CHORD

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 3=96/Mechanical, 2=272/0-8-0, 4=92/Mechanical Max Horz 2=134(LC 6) Max Uplift3=93(LC 6), 2=-111(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/18, 2-3=-75/37

BOT CHORD 2-4=-18/18

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 93 lb uplift at joint 3 and 111 lb uplift at
- 6) Attach with (2) 16d Common Toe-Nalls (0.162"x3.5") at joints 3 and 4.

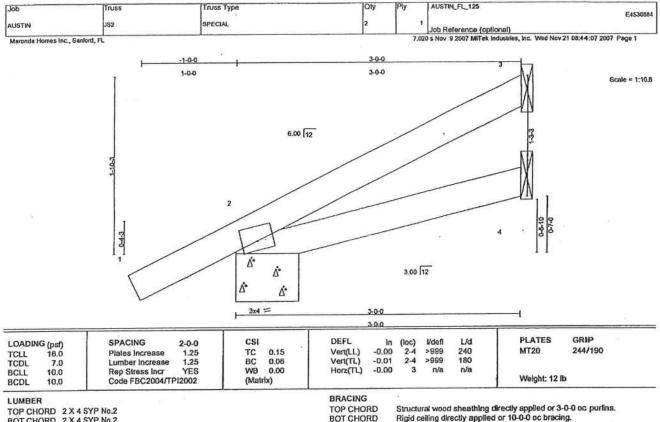
LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. AWARDING - Vertify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE FAGE MIT-7473 HEFORS USE. Design volid for use only with Mile is connection. This design is based only upon prometers show, and is for an inclividual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not have designer. Bracing shows to releval support of inclividual web members only. Additional temporary bracing to Inusz eithibity during construction is the responsibility of the erector. Additional permanent bracing of the overall shocking, consult. ANSI/TRIT Quality Criteria, DSB-89 and BCS11 Building Component fabrication, quality control stronge, delivery, erection and bracing, consult. ANSI/TRIT Quality Criteria, DSB-89 and BCS11 Building Component salely intermedian available from Trust Plate Institute, SS3 D'Chordino Drive, Modison, Wi SS3119.





BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 3=43/Mechanical, 2=194/0-8-0, 4=52/Mechanical

Max Horz 2=93(LC 6)

Max Uplift3=40(LC 6), 2=-110(LC 6)

FORCES (ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/18, 2-3=-40/16

BOT CHORD 2-4=-10/10

NOTES

- NOTES (6)

 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interlor(1) zone; Lumber DOL=1.60 plate grip DOL=1.80. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

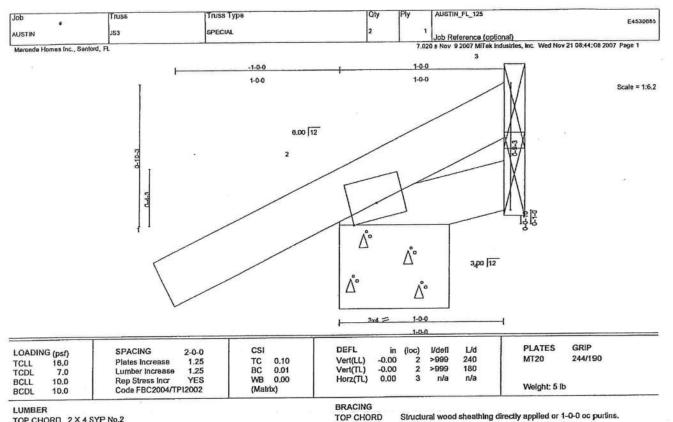
- 4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 40 lb uplift at joint 3 and 110 lb uplift at
- 6) Attach with (2) 18d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 BEFORE USE. AM WARNING - Verify design parameters and READ NOTES ON THIS BID INCLUDED BUT AS REPLECTED AND INCLUDED BUT AS REPLECTED BUT AS REPLECTED AND INCLUDED BUT AS REPLECTED BUT AS REPLECTE





BOT CHORD

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 2=124/0-8-0, 4=18/Mechanical, 3=-11/Mechanical Max Horz 2=51(LC 6) Max Uplift2=-108(LC 6), 3=-11(LC 1)

Max Grav 2=124(LC 1), 4=18(LC 1), 3=18(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/15, 2-3=-32/24

BOT CHORD 2-4=-4/4

NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL≃1.60 plate grip DOL≈1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

4) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 108 lb uplift at joint 2 and 11 lb uplift at loint 3.

6) Attach with (2) 16d Common Toe-Nails (0.162"x3.5") at joints 3 and 4.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REPERENCE PAGE MII-7473 BEFORE USE. WARNING - Verify design parameters and JUSAD NOTES ON THIS AND INCLUDED BITTER REFERENCE PAGE ME17479 BEFORE USE.

Design volid for use only with Mifek connections. This design is bosed only upon parameters shown, and is for an individual building component.

Applicability of design parameters and proper incorporation of component is responsibility of building designer, not fixes designer. Broking is for followed to individual web members only. Additional temporary broking to insure stability during construction is the responsibility of the building designer, for general guidance regarding erector. Additional permanent broking the overall structure is the responsibility of the building designer, for general guidance regarding fobication, quality control, storage, delivery, erection and broking, consult.

ARSYPTI Quality Chierta, DSS-89 and SCS11 Building Component Solely Information available from Trus Plate Institute, 583 D'Onokio Drive, Modson, WI 53719.



AUSTIN_FL_125 Truss Type Job Truss E4530688 SPECIAL AUSTIN Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industri es, Inc. Wed Nov 21 08:44:08 2007 Page 1 Maronda Homes Inc., Senford, FL -1-0-0 1-0-0 7-0-0 Scale: 1/2"=1" 3x5 | 6,00 12 2x4 = 3.00 12 3x4 = 7-0-0 6-11-5 **PLATES** GRIP LOADING (psf) SPACING CSI In (loc) 2-0-0 Vert(LL) 0.10 2-4 >754 240 MT20 244/190 TC 0.49 TCLL 16.0 Plates Increase 1,25 1.25 BC 0.45 Vert(TL) -0.262-4 >302 180 7.0 Lumber Increase TCDL Rep Stress Incr -0.00 n/a n/a YES WB 0.00 Horz(TL) BCLL Weight: 27 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0 BRACING LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. 2 X 4 SYP No.2 WEBS REACTIONS (lb/size) 2=338/0-8-0, 3=155/Mechanical, 4=143/Mechanical Max Horz 2=174(LC 6) Max Uplift2=122(LC 8), 3=-191(LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/18, 2-5=-120/0, 5-6=-93/0, 3-6=-101/60

2-7=-21/0, 7-8=-0/0, 4-8=0/28 **BOT CHORD**

WEBS

- NOTES (8)

 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and G-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Refer to girder(s) for truss to truss connections.

4) Bearing at Joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface,

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 122 lb uplift at joint 2 and 191 lb uplift at

6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 9 lb down and 57 lb up at 4-4-0, 9 lb down and 57 lb up at 4-4-0, and 47 lb down at 1-6-1, and 47 lb down at 1-6-1 on top chord, and 21 lb up at 1-6-1, 21 lb up at 1-6-1, and 17 lb down at 4-4-0, and 17 lb down at 4-4-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B). 8) Attach with (2) 16d Common Toe-Neils (0.162*x3.5*) at Joints 3 and 4.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=46, 2-4=-40

Concentrated Loads (ib)

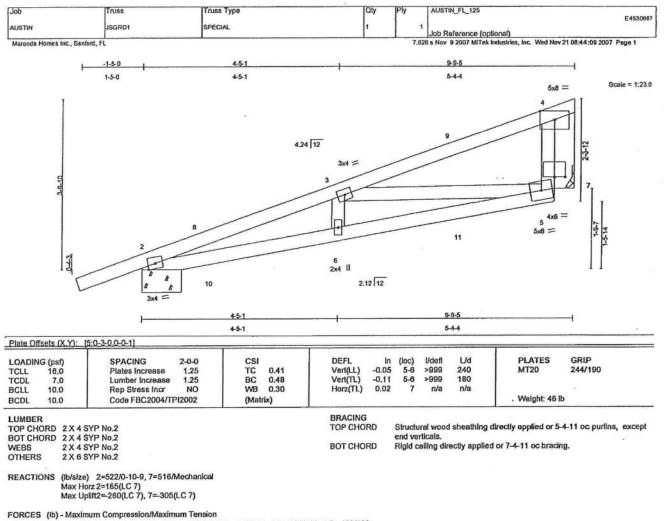
Vert: 6=-19(F=-9, B=-9) 7=41(F=21, B=21) 8=-33(F=-17, B=-17)

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 REFORE USE. Design valid for use only with Milité connectors. This design is based only upon porameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not it us designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer. For general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, for general guidance regarding fobiocation, qualify control, storage, delivery, erection and bracing, consult. ARSI/TPII Quality Criteria, DSB-89 and BCSII Building Component Safety Information, available from Trus Plate Institute, SS3 D'Onorito Drive, Modeon, VII S3719.



on. NC 27932



TOP CHORD

1-2=0/17, 2-8=-1374/611, 3-8=-1337/637, 3-9=-445/261, 4-9=-364/188, 4-5=-132/439

BOT CHORD 2-10=-696/1253, 6-10=-688/1266, 6-11=-711/1261, 5-11=-701/1300

WEBS 3-8=0/199, 3-5=-854/425, 4-7=-557/355, 5-7=-217/218

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Bearing at Joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 260 lb uplift at joint 2 and 305 lb uplift at Joint 7.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 47 lb up at 4-4-0, 47 lb up at 4.4.0, 59 lb down and 126 lb up at 7-1-15, 59 lb down and 126 lb up at 7-1-15, and 44 lb down at 1-6-1, and 44 lb down at 1-6-1, and 44 lb down at 1-6-1 on top chord, and 22 lb up at 1-6-1, 22 lb up at 1-6-1, 12 lb down at 4-4-0, 12 lb down at 4-4-0, and 52 lb down at 7-1-15, and 52 lb down at 7-1-15 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

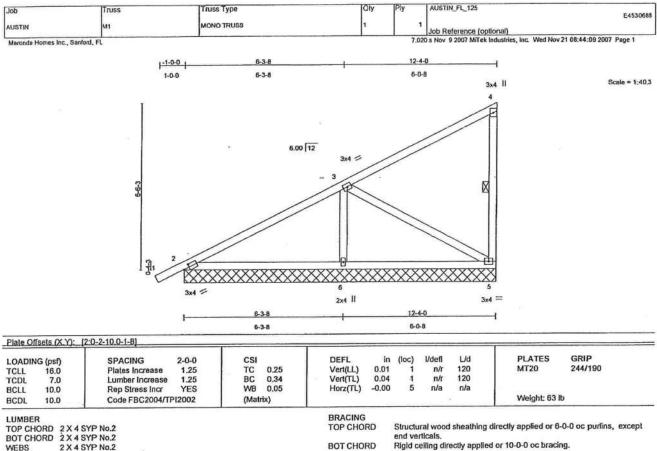
Vert: 1-4=-46, 2-5=-40 Concentrated Loads (lb)

Vert: 3=8(F=4, B=4) 6=-24(F=-12, B=-12) 9=-118(F=-59, B=-59) 10=43(F=22, B=22) 11=-104(F=-52, B=-52)

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTER REFERENCE PAGE MII-7473 REFORE USE. Design volid for use only with Milex connectors. This design is based only upon portaments and responsibility of design parameters and responsibility of design parameters and responsibility of design parameters only proper incorporation of component is responsibility of building designer - not itus designer. Broding shown is for tolered support of inchristiant with members only. Additional temperanty broding to insure stopy during construction is the responsibility of the building designer. For general guidance responsibility of the building designer, for general guidance regarding labs/color, quality control, storage, deliver, exection and broding, consult. ASS/IPI1 Quality Criteria, DSS-89 and BCS11 Building Component of the property of the control of the con





WEBS

1 Row at midpt

2 X 4 SYP No.2

REACTIONS (lb/size) 5=215/12-4-0, 2=288/12-4-0, 6=594/12-4-0 Max Horz 2=286(LC 6)

Max Uplift5=117(LC 6), 2=61(LC 6), 6=140(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-151/28, 3-4=-93/28, 4-5=-105/126 2-6=-98/43, 5-6=-98/43 TOP CHORD

BOT CHORD

3-6=-303/255, 3-5=-25/99 WEBS

1) Whild ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

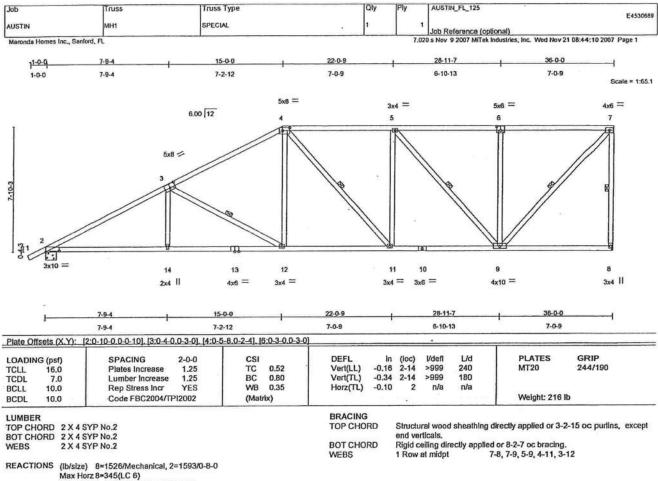
3) Gable requires continuous bottom chord bearing.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 117 lb uplift at joint 5, 61 lb uplift at joint 2 and 140 lb uplift at joint 6.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. AMARTING - Verify design parameters and READ INCLUSES ON THIS AND INCLUSED SITES REPRESENTED FOR SOME OF SOME





Max Uplift8=362(LC 5), 2=329(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

7-8=-1396/603, 4-5=-1748/668, 5-6=-1170/441, 6-7=-1170/441, 1-2=0/21, 2-3=-2771/812, 3-4=-2063/706

BOT CHORD WEBS

2-14=562/2390, 13-14=564/2383, 12-13=564/2383, 11-12=298/1787, 10-11=264/1748, 9-10=264/1748, 8-9=0/393 7-9=637/1689, 6-9=330/317, 5-9=856/337, 5-11=9/315, 4-11=103/121, 4-12=58/610, 3-12=691/304, 3-14=0/333

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 362 lb uplift at joint 8 and 329 lb uplift at joint 2.

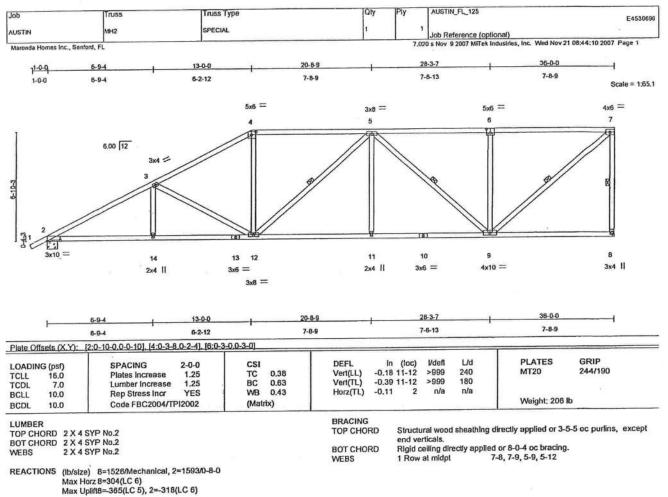
LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connector. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility to building designer - not trust designer, Bracing show is for toleral support of individual web members only. Additional temporary bracing to building designer - not trust designer, Bracing show is for toleral support of individual web members only. Additional temporary bracing to Insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, for general guidance reporting tobication, quality carried, storage, delivery, erection and bracing, consult. ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute. SS3 D'Onofrio Drive, Modison, WISS719.



Edenton, NC 27932



FORCES (Ib) - Maximum Compression/Maximum Tension

Maximin Companies of Maximin Territoria (1987) 17-8—1381/801, 45—1957/757, 5-8—1451/541, 6-7—1451/541, 1-2—0/21, 2-3—2807/852, 3-4—2234/774 2-14—608/2424, 13-14—608/2424, 12-13—608/2424, 11-12—430/2084, 10-11—430/2084, 9-10—430/2084, 8-9=0/341 TOP CHORD

BOT CHORD WEBS

7-9=693/1858, 6-9=358/343, 5-9=837/327, 5-11=0/305, 5-12=-168/139, 4-12=54/649, 3-12=547/238, 3-14=0/273

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Refer to girder(s) for truss to truss connections.

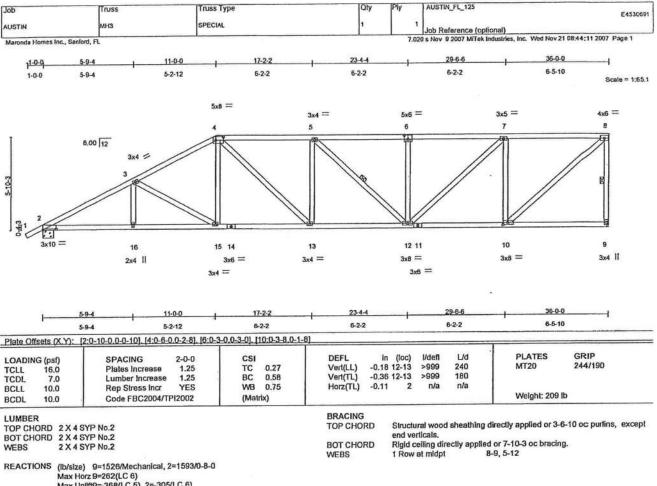
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 365 lb uplift at joint 8 and 318 lb uplift at loint 2.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design voild for use only with Milek connectors. This design is based only upon parameters shown, and is for on individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not inus designer. Broding shown is for tolerot support of individual web members only. Additional temporary broding to insure stability during construction is the responsibility of the building designer. Broding shown erector. Additional permanent individual web members only. Additional temporary broding to insure stability during construction is the responsibility of the building designer. For general guidance regarding erector. Additional permanent individual component is the responsibility of the building designer. For general guidance regarding forbidation, quality control, storage, defivery, erection and broding, consult. ANS/TP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Piate Institute, SS3 D'Onotifo Drive, Madson, WI S3719.





Max Uplift9=368(LC 5), 2=-305(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-2840/884, 3-4=-2382/835, 4-5=-2478/929, 5-6=-2269/842, 6-7=-2269/842, 7-8=-1479/546,

TOP CHORD

8-9=-1403/590

2-16=-647/2454, 15-16=-647/2454, 14-15=-490/2095, 13-14=-490/2095, 12-13=-618/2478, 11-12=-234/1479, BOT CHORD

10-11=-234/1479, 9-10=0/296

3-16=0/225, 3-15=-422/182, 4-15=-21/432, 4-13=-196/515, 5-13=-97/207, 5-12=-281/117, 6-12=-279/259,

7-12=398/1063, 7-10=1008/553, 8-10=-706/1915

WEBS NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 368 lb uplift at joint 9 and 305 lb uplift at

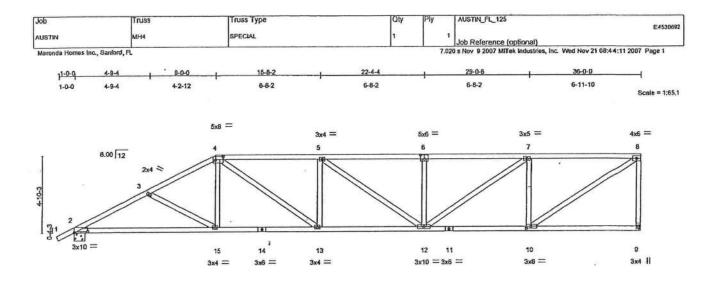
LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 BEFORE USE. AM WARRING - Pertily design parameters and KEAU ROLES ON THIS AND INCLUDED SHITER REFERENCE FACE SILE-1975 BEFORE DES.

Design void for use only with Miles connection. This design is based only upon parameters shown, and is for an Individual building component.
Applicability of design paramenters and proper incorporation of component is responsibility of building designer. For lives designer, Rocing sho
is for lateral support of Individual who members only. Additional temporary broting to issues stably during construction is the responsibility of the school by during construction is the responsibility of the subding designer. For general guidance regarding flobrication, quality control, strongs, desivery, erection and broting, consult. AMSUTPH Quality Cifleria, DSB-89 and BCS11 Building Component Safely Information available from Truss Picte Institute, 583 D'Onotrio Drive, Madison, WI 53719.





1	9-0-0		15-8-2		22-4-4		29-0)-6	36-0-0		
	9-0-0	501-1	6-8-2		6-8-2		6-8-	2	8-11-10	165	
Plate Offsets (X.)	(): [2:0-10-0,0-0-14], [4:0-6-0	0.0-2-8], [6:0-	3-0.0-3-0].[10:0-3-8.0-1-	8]						_
LOADING (psf) TCLL 16.0	SPACING Plates Increase	2-0-0 1.25	CSI TC	0.85	DEFL Vert(LL)	in (loc) -0.24 12-13	l/defl >999	L/d 240	PLATES MT20	GRIP 244/190	
TCDL 7.0 BCLL 10.0	Lumber Increase Rep Stress Incr	1.25 YES	BC WB	0.82 0.82	Vert(TL) Horz(TL)	-0.49 12-13 -0.12 2	>868 n/a	180 n/a			
BCDL 10.0	Code FBC2004/TI	P12002	(Mat	rix)					Weight: 195 lb)	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.2

BRACING

TOP CHORD

Structural wood sheathing directly applied or 3-5-3 oc purlins, except

end verticals.

BOT CHORD Rigid ceiling directly applied or 6-10-14 oc bracing.

REACTIONS (lb/size) 9=1526/Mechanical, 2=1593/0-8-0 Max Horz 9=221(LC 6)

Max Uplift9=370(LC 5), 2=-296(LC 5)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-2733/947, 3-4=-2573/880, 4-5=-2978/1107, 5-6=-2867/1052, 6-7=-2867/1052, 7-8=-1917/702, TOP CHORD

8-9=-1391/589

2-15=-704/2382, 14-15=-575/2276, 13-14=-575/2276, 12-13=-843/2978, 11-12=-437/1917, 10-11=-437/1917, **BOT CHORD**

9-10=0/242

3-15=133/171, 4-15=0/394, 4-13=324/849, 5-13=237/255, 5-12=134/67, 6-12=302/280, 7-12=424/1150,

7-10=966/545, 8-10=817/2230

NOTES

WEBS

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Calegory II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for
- MWFRS for reactions specified.

 2) Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 370 lb uplift at joint 9 and 296 lb uplift at Joint 2.

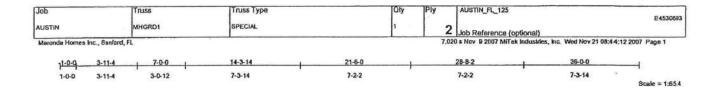
LOAD CASE(S) Standard

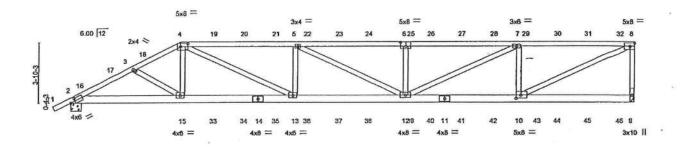
This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-1413 BEFORE USE.

Design voted for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not intus designant. Reaching shown is for toleral support of individual web members only. Additional temporary bracing to insure shiply during construction is the responsibility of the erector. Additional permanent brocking of the overall structure is the responsibility of the building designer. For general guidance regarding tobaccions, quality control, storage, deferver, exection and brocking, consult. AMSI/IPTI Quality Criteria, DSS-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, SSS D'Onotrio Drive, Madison, Wt 537 19.







	3-11-4	7-0-0	14-3-14		1	21-6-0		28-8-2	-	36-0-0	
	3-11-4	3-0-12	7-3-14			7-2-2		7-2-2		7-3-14	
Plate Offs	ets (X.Y): [2:	0-3-10.0-2-0]. [4:0-5-8.	0-2-4), [6:0-4-0,	0-3-0].[10	0:0-3-8,0-2-	8]					
LOADING	(psf)	SPACING	2-0-0	CSI		DEFL	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL	16.0	Plates Increase	1.25	TC	0.73	Vert(LL)	0.41 12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.70 12-13	>605	180	l .	
BCLL	10.0	Rep Stress Incr	NO	WB	0.73	Horz(TL)	-0.10 2	n/a	n/a		
BCDL	10.0	Code FBC2004/TF	P12002	(Matr	ix)					Weight: 428 I	b

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.2

2 X 4 SYP No.2

TOP CHORD

BRACING

Structural wood sheathing directly applied or 3-9-12 oc purlins, except

BOT CHORD Rigid ceiling directly applied or 8-0-0 oc bracing.

REACTIONS (lb/size) 9=3456/Mechanical, 2=3341/0-8-0 Max Horz 9=181(LC 5)

Max Uplift9=-1881(LC 7), 2=-1885(LC 8)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=0/25, 2-16=-8512/3586, 18-17=-6456/3503, 3-17=-8444/3424, 3-18=-6568/3513, 4-18=-6510/3523,

4-19=8802/4793, 19-20=8802/4793, 20-21=8801/4792, 5-21=8801/4792, 5-22=8720/4742, 22-23=8720/4742, 23-24=8720/4742, 24-25=8720/4742, 6-25=8720/4742, 6-26=8720/4742, 26-27=8720/4742, 27-28=6720/4742, 28-27=8720/4742, 28-27=8720/4742, 28-28=8720/

7-28=-8720/4742, 7-29=-5894/3288, 29-30=-5894/3288, 30-31=-5894/3288, 31-32=-5894/3288, 8-32=-5894/3288,

2-15=3023/5685, 15-33=-3059/5940, 33-34=-3059/5940, 14-34=-3059/5940, 14-35=-3059/5940, 13-35=-3059/5940, 13-36=-4611/8801, 38-37=-4611/8801, 37-38=-4611/8801, 38-39=-4611/8801, 12-39=-4611/8801, 12-40=-3172/5894,

11-40=-3172/5894, 11-41=-3172/5894, 41-42=-3172/5894, 10-42=-3172/5894, 10-43=-86/217, 43-44=-86/217,

44-45=-86/217, 45-46=-86/217, 9-46=-86/217 3-15=-285/250, 4-15=-189/957, 4-13=-1829/3186, 5-13=-724/874, 5-12=-90/74, 6-12=-733/859, 7-12=-1718/3140,

7-10=-2127/1602, 8-10=-3605/6306

WEBS

BOT CHORD

1) 2-ply truss to be connected together with 10d (0.131"x3") nalls as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-7-0 oc.

Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to
ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise);

Lumber DOL=1.60 plate grip DOL=1.60.

Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1881 ib uplift at joint 9 and 1885 ib uplift at joint 2.

Continued on page 2

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE FACE MU-7473 BEFORE USE.

Design volid for use only with Miles connection. This design is based only upon parameters shown, and is far an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not inus designer, Bracing shown is for tolered support of individual was members only. Additional temporary broting to insure slobility during construction is the responsibility of the enector. Additional permanent brocking of the overall structure is the responsibility of the building designer, for general guidance regarding fabrication, quality control, storage, delivery, erection, and ANSI/IPII Quality Criteria, DSS-89 and 8CSII Building Component Safety Information available from Iross Piole halitule, SSS D'Onofrio Drive, Modison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	AUSTIN_FL_125
300					E453089
AUSTIN	MHGRD1	SPECIAL	1		Job Reference (optional)

Maronda Homes Inc., Sanford, FL

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 116 lb down and 194 lb up at 35-0-12, 116 lb down and 194 lb up at 33-0-12, 116 lb down and 194 lb up at 31-0-12, 116 lb down and 194 lb up at 29-0-12, 116 lb down and 194 lb up at 23-0-12, 116 lb down and 194 lb up at 13-0-12, 116 up at 0-11-4, 194 ib up at 2-11-4, 194 ib up at 2-1

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=46, 4-8=46, 2-9=40

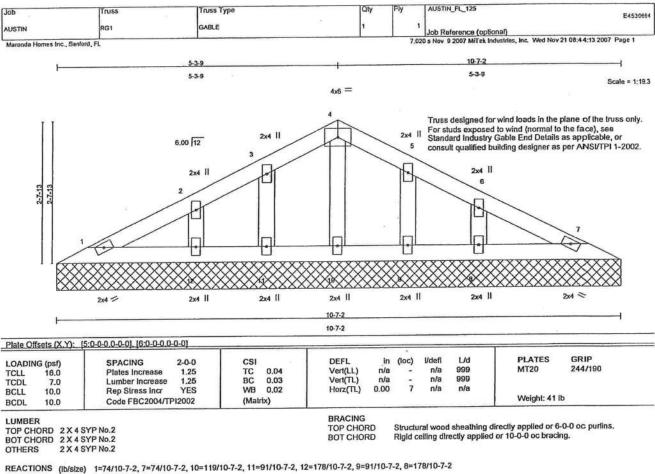
Concentrated Loads (lb)

Vert: 4=-116(B) 15=-644(B) 19=-116(B) 20=-116(B) 21=-116(B) 22=-116(B) 23=-116(B) 24=-116(B) 25=-116(B) 25=-116(B) 27=-116(B) 27=-116(B) 29=-116(B) 29=-116(B) 30=-116(B) 32=-116(B) 32=-116(B) 33=-92(B) 35=-92(B) 35=-92(B) 37=-92(B) 38=-92(B) 39=-92(B) 40=-92(B) 41=-92(B) 42=-92(B) 43=-92(B) 44=-92(B) 43=-92(B) 44=-92(B) 44=-45=-92(B) 46=-92(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 BEFORE USE.

Dasign valid for use only with Mitik connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for taleral support of individual web members only. Additional temporary backing to insure stability during construction is the responsibility of the building designer. For general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding tobrication, quality control, storage, delivery, erection and bracing, consult. ANS/IPIT Quality Criteria, USB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute. 583 D'Onofrio Drive, Madson, Wi 53719.





Max Horz 1=-44(LC 4)
Max Uplift1=-23(LC 7), 7=-21(LC 7), 11=-56(LC 6), 12=-112(LC 6), 9=-55(LC 7), 8=-113(LC 7)
Max Grav 1=74(LC 1), 7=74(LC 1), 10=119(LC 1), 11=94(LC 10), 12=178(LC 1), 9=94(LC 11), 8=178(LC 1)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=42/27, 2-3=25/65, 3-4=20/112, 4-5=20/112, 5-6=25/65, 6-7=24/19 1-12=0/51, 11-12=0/51, 10-11=0/51, 9-10=0/51, 8-9=0/51, 7-8=0/51

BOT CHORD

WEBS

4-10=-59/7, 3-11=-53/94, 2-12=-92/161, 5-9=-53/94, 6-8=-92/161

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Gable requires continuous bottom chord bearing.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 23 lb uplift at Joint 1, 21 lb uplift at Joint 7, 56 lb uplift at joint 11, 112 lb uplift at joint 12, 55 lb uplift at joint 9 and 113 lb uplift at joint 8.

LOAD CASE(S) Standard

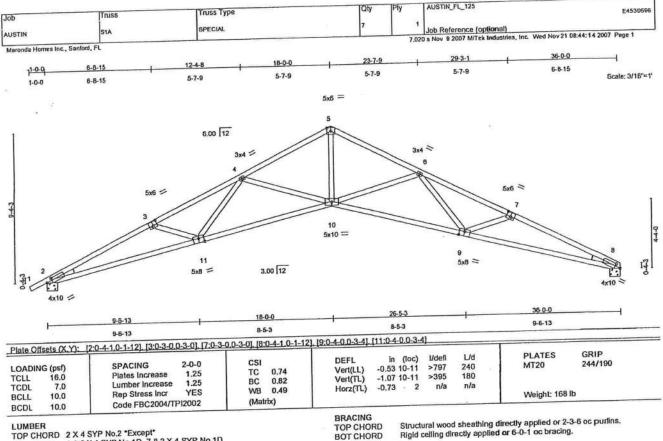
This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 BEFORE USS. WARRING - Verify design parameters and READ NOTES ON THIS AND INCLOSED MITTER REFERENCE PAGE BILLY FOR BELLY FOR BOST.

Design void for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design parameters and proper incorparation of component is responsibility of building designer. For including designer, Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer, for general guildingare reponsibility of the building designer, for general guilding Component Salety Intermetten available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.





TOP CHORD 2 X 4 SYP No.2 *Except*

1-3 2 X 4 SYP No.1D, 7-8 2 X 4 SYP No.1D

BOT GHORD 2 X 4 SYP No.1D 2 X 4 SYP No.2

WEBS

WEDGE

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

REACTIONS (lb/size) 8=1518/0-8-0, 2=1585/0-8-0

Max Horz 8=166(LC 6)

Max Uplift8=286(LC 7), 2=368(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/18, 2-3=-4813/1409, 3-4=-4589/1239, 4-5=-3320/885, 5-6=-3320/885, 6-7=-4602/1269, 7-8=-4831/1449 TOP CHORD

BOT CHORD

2-11=-1145/4368, 10-11=-811/3809, 9-10=-822/3814, 8-9=-1186/4387 3-11=-176/290, 4-11=-93/711, 4-10=-807/417, 5-10=-563/2647, 6-10=-812/428, 8-9=-118/722, 7-9=-183/306 WEBS

NOTES

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-fise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) This truss has been designed for a 10,0 psf bottom chord live load nonconcurrent with any other live loads.

4) Bearing at Joint(s) 8, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 8 and 368 lb uplift at joint 2.

LOAD CASE(S) Standard

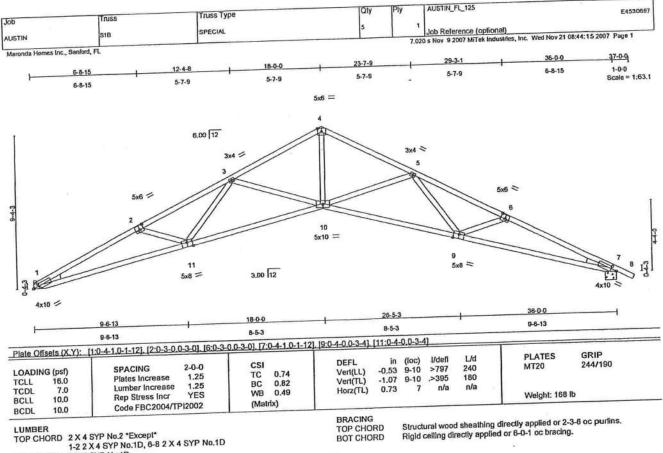
This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL 1473 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE FACE MIL-1473 BEPORE USE.

Design vold for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component in exponsibility of building designer. Foll russ designer, Brocking shows Applicability of design parameters and proper incorporation of component is responsibility of building designer. Foll russ designer, Brocking shows it for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the its for lateral support of individual web members only. Additional permanent bracking of the overall shucture is the responsibility of the building designer, for general guidance regarding fobrication, quality control, storage, designer, section and bracking, consult.

ARSI/TRI Quality Criteria, DSB-89 and BCSIT Building Component Safety Information available from Truss Plate Institute. SSB D'Onothio Drive, Madison, WI SST19.





BOT CHORD 2 X 4 SYP No.1D

2 X 4 SYP No.2 WEBS

WEDGE

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

REACTIONS (lb/size) 1=1518/Mechanical, 7=1585/0-8-0

Max Horz 1=166(LC 7)
Max Uplift1=286(LC 8), 7=368(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=4831/1449, 2-3=-4602/1269, 3-4=-3320/885, 4-5=-3320/885, 5-6=-4589/1239, 6-7=-4813/1409, 7-8=0/18 1-11=-1186/4387, 10-11=-822/3814, 9-10=-811/3809, 7-9=-1145/4368 BOT CHORD

2-11=183/306, 3-11=-118/722, 3-10=-812/428, 4-10=-563/2647, 5-10=-807/417, 5-9=-93/711, 6-9=-176/290 WEBS

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Bearing at Joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 1 and 368 lb uplift at Joint 7.

LOAD CASE(S) Standard

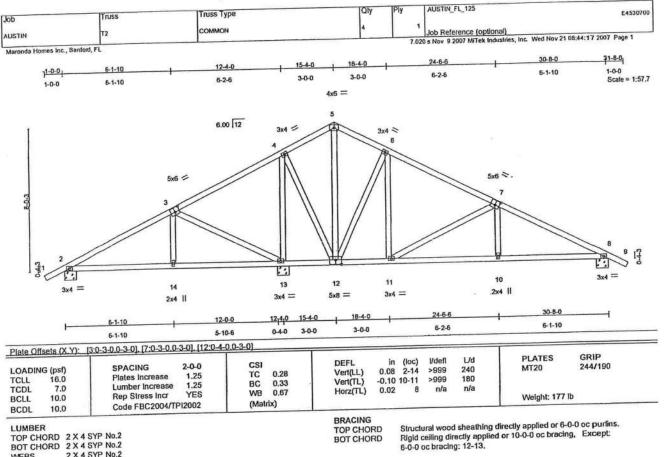
This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REPERENCE PAGE MII-7473 BEFORE USE.

Design volid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building of page 1. The page 1. T



Edenion, NC 27932



BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.2 WEBS

REACTIONS (lb/size) 2=364/0-8-0, 13=1651/0-8-0, 8=694/0-8-0

Max Horz 2=141(LC 6) Max Uplift2=-302(LC 6), 13=-556(LC 6), 8=-212(LC 7)

Max Grav2=408(LC 10), 13=1651(LC 1), 8=708(LC 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

BOT CHORD

мажиния сопртессионализации (ension 1-2=0/21, 2-3=-345/382, 3-4=-178/495, 4-5=-24/101, 5-6=-35/96, 6-7=-370/102, 7-8=-979/172, 8-9=0/21 2-14=-228/257, 13-14=-224/250, 12-13=-390/422, 11-12=0/271, 10-11=-48/808, 8-10=-46/815 3-14=-318/269, 3-13=-628/701, 4-13=-1143/476, 4-12=-194/808, 5-12=-99/28, 6-12=-649/296, 6-11=-69/487, 141=-69/259, 2-10-0/272 WEBS

7-11=605/258, 7-10=0/272

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 302 lb uplift at joint 2, 556 lb uplift at joint

13 and 212 lb uplift at joint 8.

LOAD CASE(S) Standard

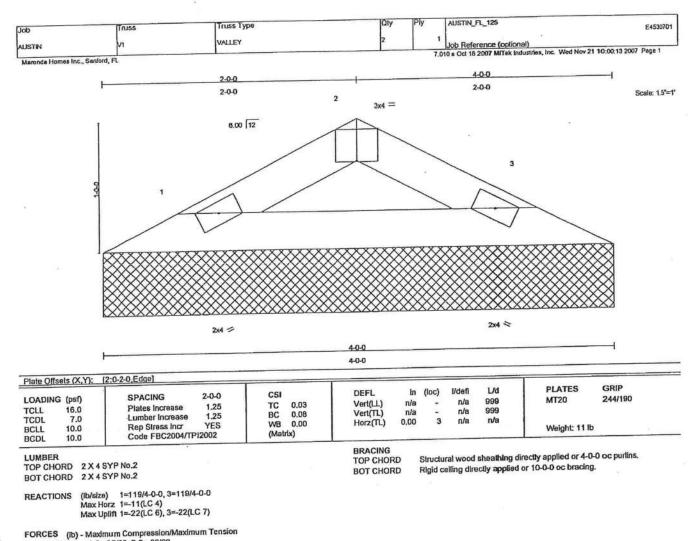
This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REPERENCE PAGE MIL-7473 BEFORE USE. MARRING - Verify design parameters and READ NOTES ON THIS AND INCLUDEM MITTER KERTAKENES FAVE MILITARY BECAGE USE.

Design valid for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper increparation of component is responsibility of building designer, not trust designer. Bracing sit for lateral support of Individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of its building designer. For general guidance regarding erector, Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding labifaction, quality control, strategy, defivery, erection and bracing, cornul.

ANSI/TRI Quality Citleria, DSB-89 and BCSI1 Building Compone Salely Information available from Truss Plate Institute, SS3 D'Onotifo Drive, Madison, WI 53719.





1-2=82/69, 2-3=82/69

TOP CHORD

1-3=-42/64 BOT CHORD

NOTES

1) Unbalanced roof live loads have been considered for this design.

1) Onbalanced ruot live loads have been considered for this design.
2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2pst; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 A) Gable requires continuous bottom chord bearing.

 WARNING: Top chord roof live load is below minimum required by ASCE 7. The building design professional for the overall structure to verify adequacy of top chord live load.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 1 and 22 lb uplift at joint 3.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 REFORE USE. MARNING - Verify design parameters and READ NOTES ON TIBE AND INCLUDE WITHER REFERENCE FACE WILLYYS REFUGE USE.

Design valid for use only with MTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer, not inus designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during constructions is the responsibility of the is for lateral support of individual web members only. Additional temporary bracing to insure stability during constructions is the responsibility of the information permanent bracing of the overall structure is the responsibility of the individual permanent bracing, and it is the responsibility of the individual permanent bracing, consult.

AMSUTETI Quality Criteria, DSR-89 and BCS11 suitiding Component Safety Information available from Truss Plate Institute, S83 D'Onotifo Drive, Maddon, WI S37 19.



Job	Truss	Truss Type	Qty	Ply	AUSTIN_FL_125	96	E4530702
AUSTIN	V10	GABLE	1	1	Job Reference (optional)		
Maronda Homes Inc., Sanford,	FL			7.02	20 s Nov 9 2007 MiTek Industri	ies, Inc. Wed Nov 2	1 08:44:18 2007 Page 1
			8-0-0		8,2-8		
			8-0-0		0-2-8		Scale = 1:25.4
	T	ss designed for wind loads in th	e plane of the Inject only		2x4		Otale = 1.25.4
	For	studs exposed to wind (normal	to the face), see		4		
	Sta	ndard Industry Gable End Detainsult qualified building designer	as per ANSI/TPI 1-2002.	. 2x4		I	
		6	.00 12	3 /			
	1						
	1		2x4 II	7			
	q		2//			sq.	
	4	/				3-9-5	
		0		1	15		
	'KXX	******	***********	XXX			
		2x4 ×	2x4	2x4	2x4		
			8-2-8	234	.,		
			8-2-8				
LOADING	SPACING 2-0-	CSI	DEFL in	(loc)	I/defi L/d	PLATES	GRIP
LOADING (psf) TCLL 16.0	Plates Increase 1.2	TC 0.11	Vert(LL) n/a		n/a 999	MT20	244/190
TCDL 7.0 BCLL 10.0	Lumber Increase 1.2 Rep Stress Incr YE		Vert(TL) n/a Horz(TL) 0.00		n/a 999 n/a n/a	7.	
BCDL 10.0	Code FBC2004/TPI2002		1.5574.57		2002	Weight: 36 lb	
LUMBER			BRACING				
TOP CHORD 2 X 4 SYF			TOP CHORD	Structure end ver	ral wood sheathing direc	tly applied or 6-0	0-0 oc purlins, except
BOT CHORD 2 X 4 SYF WEBS 2 X 4 SYF			BOT CHORD		elling directly applied or 1	10-0-0 oc bracin	g.
OTHERS 2 X 4 SYF							
REACTIONS (lb/size)	1=126/8-2-8, 5=72/8-2-8, 6=	129/8-2-8, 7=312/8-2-8					
Max Horz	1=154(LC 6) 5=-26(LC 6), 6=-48(LC 6), 7		2				
			20				
FORCES (lb) - Maximur TOP CHORD 1-2=-17	n Compression/Maximum To 0/44, 2-3=-71/10, 3-4=-27/1	ension 3, 4-5=-39/45					

NOTES

- Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3-6=-70/81, 2-7=-167/195

BOT CHORD 1-7=0/0, 6-7=0/0, 5-6=0/0

3) Gable requires continuous bottom chord bearing.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 5, 48 lb uplift at joint 6 and 114 lb uplift at joint 7.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. AWARING - Verify design parameters and READ NOTES ON THIS AND INCLIDED MITER REPERCENCE FASE aut-94-3 Beroite Use.

Design volid for use only with Miles connection. This design is based only upon perometers shown, and is for an individual building component.
Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not inus designer, Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insue should be distingtoned in the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of building designer, for general guidance regarding retailed only of the control structure is the responsibility of the design designer, for general guidance regarding fabrication, and procedure, consult. ANSI/IPI1 Quality Criteria, DSS-89 and 8CS11 Building Component Sately Information available from Trus Plate Institute, S83 D'Onolrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	aly	Ply	AUSTIN_FL_125	E4530703
	tamons	GABLE	1	1	L. D. F for F	
AUSTIN	VII	GABLE		7.02	Job Reference (optional) 20 s Nov 9 2007 MfTek Industri	ies, Inc. Wed Nov 21 08:44:18 2007 Page 1
Maronda Homes Inc., Sanfon	d, FL			1.02		ANTONY POTO LITERATURE STOCKET CONTROL TO THE
			10-0-0		10-218	
	-		10-0-0		0-2-8	Scale = 1:31.5
					2x4	Scale = 1.31,5
	1 5	russ designed for wind loads in the for studs exposed to wind (normal standard Industry Gable End Detail consult qualified building designer at 6.	to the race, set is as applicable, or as per ANSI/TPI 1-2002.		2x4	8.04
	<u>u.n.</u>	3x4 = 2x4	1l 2x4 1l		2x4 2x4	
		2x4	10-2-8			
	1		10-2-8		1	
LOADING (psf) TCLL 16.0 TCDL 7.0 BCLL 10.0 BCDL 10.0	Plates Increase Lumber Increase	0-0 CSI 1.25 TC 0.09 1.25 BC 0.10 YES WB 0.02 002 (Malrix)	DEFL in Vert(LL) n/a Vert(TL) n/a Horz(TL) 0.00		l/defl L/d n/a 999 n/a 999 n/a n/a	PLATES GRIP MT20 244/190 Weight: 49 lb
LUMBER TOP CHORD 2 X 4 S BOT CHORD 2 X 4 S WEBS 2 X 4 S	SYP No.2 SYP No.2 SYP No.2 SYP No.2		BRACING TOP CHORD BOT CHORD	end ve	ural wood sheathing directicals. celling directly applied or	ctly applied or 6-0-0 oc purlins, except
Max Ho Max Up	orz 1=198/I C 6)	2-8, 7=191/10-2-8, 8=114/10-2-8, 9), 8=-43(LC 6), 9=-114(LC 6) n Tension	=315/10-2-8		e e	

TOP CHORD 1-2=-214/40, 2-3=-126/8, 3-4=-82/21, 4-5=-24/14, 5-6=-35/41

BOT CHORD 1-9=-2/2, 8-9=-2/2, 7-8=-2/2, 6-7=-2/2 WEBS 4-7=-101/113, 3-8=-64/82, 2-9=-164/168

NUTES
1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

2) This muss has been designed for a 10.0 psr bottom chord live load nonconcurrent with any other live loads.
 3) Gable requires continuous bottom chord bearing.
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 6, 66 lb uplift at joint 7, 43 lb uplift at joint 8 and 114 lb uplift at joint 9.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 BEFORE USE.

Design vold for use only with MiTek connecton. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not laus designer. Bracing shown is for toleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building personal processing of the expensibility of the value designer. For general guidance regarding erector, Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding affects of the control permanent bracing of the overall structure. AMSI/TPI Quality Criteria, DSS-89 and BCS11 Building Component Safety Information available from Trus Piate Institute, SS3 D'Onofilo Drive, Madson, WI S3719.



Job	Truss	Truss Type	Qty	Ply AUSTIN_FL_12	25	E4530704
	V12	GABLE	1	Job Reference	e (ontional)	
AUSTIN				7.020 s Nov 9 2007 N	AiTek Industries, Inc. Wed Nov 2	21 08:44:18 2007 Page 1
Maronda Homes Inc., Sanfo	rd, FL			****		
	5		12-0-0		121218	
	ļ —		12-0-0		0-2-8	Scale = 1:37.7
					2x4	Scale = 1:37.7
					6	
	Fr. St. ox	uss designed for wind loads in the stude exposed to wind (normal andard Industry Gable End Deta nsult qualified building designer and the student of the stu	to the face), see its as applicable, or		2x4	
	The same		12-2-8			
÷			12-2-8			
LOADING (psf) TCLL 16.0 TCDL 7.0 BCLL 10.0 BCDL 10.0	Plates Increase Lumber Increase	0-0 CSI 2.25 TC 0.09 2.25 BC 0.10 2.25 WB 0.03 2.20 (Matrix)	DEFL in Vert(LL) n/a Vert(TL) n/a Horz(TL) 0.00	- n/a 99 - n/a 99	9 MT20	GRIP 244/190
TOP CHORD 2 X 4 BOT CHORD 2 X 4 WEBS 2 X 4 OTHERS 2 X 4	SYP No.2 SYP No.2 SYP No.2 e) 1=127/12-2-8, 7=69/12-2	-8, 8=173/12-2-8, 9=186/12-2-8,), 9=-69(LC 6), 10=-42(LC 6), 11		end verticals. Rigid ceiling directly	athing directly applied or 6 applied or 10-0-0 oc brac	

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=265/41, 2-3=-178/8, 3-4=-135/21, 4-5=-78/18, 5-6=-24/15, 6-7=-37/42

BOT CHORD 1-11=-2/2, 10-11=-2/2, 9-10=-2/2, 8-9=-2/2, 7-8=-2/2
WEBS 5-8=-93/104, 4-9=-99/109, 3-10=-64/80, 2-11=-164/165

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) Gable requires continuous bottom chord bearing.

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 7, 60 lb uplift at joint 8, 69 lb uplift at joint 9, 42 lb uplift at joint 10 and 115 lb uplift at joint 11.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REPERENCE PAGE MIL-7473 REPORE USE.

Design volid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an includual building component, Applicability of design parameters and proper incorporation of component is responsibility of building designer - not has designer. Bracing shown Applicability of design parameters and proper incorporation of component is responsibility of building designer - not have designer. Bracing shown in for toleral support of individual web members only. Additional temporary bracing is insure stability during construction is the responsibility of the building designer. For general guidance regarding erector, Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding erector, Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding erector, Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding erector, Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding erector, Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding designer and the support of the overall structure is the responsibility of the building designer and the support of the overall structure is the responsibility of the building designer. For general guidance and the support of the overall structure is the responsibility of the building designer. For general guidance and the support of the permanent perm



Job	Truss	Truss Type	Qty	Ply	AUSTIN_FL_125	E4530705
AUSTIN	VI3	GABLE	1	1	Job Reference (optional)	
Maronda Homes Inc., San	lord, FL			7.02	0 s Nov 9 2007 MiTek Industr	ies, Inc. Wed Nov 21 05:44:19 2007 Page 1
Martina Forms ma, our	7000.5.760		.122		14.2.9	
	!		14-0-0		14j3-8 0-2-8	
			14-0-0		2x4 II	Scale = 1:43.8
		Truss designed for wind lo	ads in the plane of the truss	s only.	7	
	1	For studs exposed to wind	(normal to the face), see		2x4 B	Ť.
		consult qualified building d	nd Details as applicable, or esigner as per ANSI/TPI 1-	2002	6	
		Solitani quanti articologi	Action and the second s	2x4 II		
			6.00 12 2x4	5		
	1		. /			
			2x4 II B		11 11	
	g		3	- 11		9-9
	7	2x4 II	B			16
		2 /		- 11		
	1	8	11 11			
				11		1
	1.			- 11		1
		0	- IBI - IBI	<u> </u>		1
		3x4 = 13	12 11	10	9 8	
		2x4	2x4 2x4	2x4	2x4 2x4	
	 		14-2-8			
			14-2-8			
LOADING (psf)	SPACING	2-0-0 CSI	· DEFL	In (loc)	I/defi L/d	PLATES GRIP
TCLL 16.0	Plates Increase	1.25 TC 0.09	515555ATTTM	n/a -	n/a 999 n/a 999	MT20 244/190
TCDL 7.0	Lumber Increase	1.25 BC 0.10 YES WB 0.05	C V. SECONDONIA 192	n/a - 0.00 8	n/a 999 n/a n/a	
BCLL 10.0 BCDL 10.0	Rep Stress Incr Code FBC2004/TPI		TIOLETTE)			Weight: 80 lb
			BRACING			
LUMBER TOP CHORD 2 X 4	CVD No 2		TOP CHORD	Structur	al wood sheathing direc	tly applied or 6-0-0 oc purlins, except
BOT CHORD 2X4				end vert		
WEBS 2X4	SYP No.2	(35)	BOT CHORD	Rigid ce	iling directly applied or t midet 7-8	10-0-0 oc bracing.
OTHERS 2 X 4	SYP No.2		WEBS	I NOW E	it intopt 1-0	
REACTIONS (lb/siz	e) 1=127/14-2-8, 8=68/14	-2-8, 9=178/14-2-8, 10=168/14	1-2-8, 11=187/14-2-8, 12=1	14/14-2-8, 1	3=315/14-2-8	
May I	lorz 1=279/J C 6)					
Max l	Jpliff8=-28(LC 6), 9=-61(LC	6), 10=-63(LC 6), 11=-67(LC	6), 12=42(LC 6), 13=115(I	LC 0)		
FORCES (lb) - Max	mum Compression/Maximu	m Tension				
TOP CHORD 1-2:	-315/41, 2-3=-229/9, 3-4=-	186/22, 4-5=-131/19, 5-6=-78/	19, 6-7=-24/15, 7-8=-36/41	1.		
BOT CHORD 1-13	3=-1/1, 12-13=-1/1, 11-12=- -94/104 5-10=-91/101 4-1	1/1, 10-11=-1/1, 9-10=-1/1, 8-1 1=-99/107, 3-12=-64/79, 2-13	=-1/1 =-164/163			

WEBS

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 Gable requires continuous bottom chord bearing.

Capite requires committees bottom chain of the bearing.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 8, 61 lb uplift at joint 9, 63 lb uplift at joint 10, 67 lb uplift at joint 11, 42 lb uplift at joint 12 and 115 lb uplift at joint 13.

LOAD CASE(S) Standard

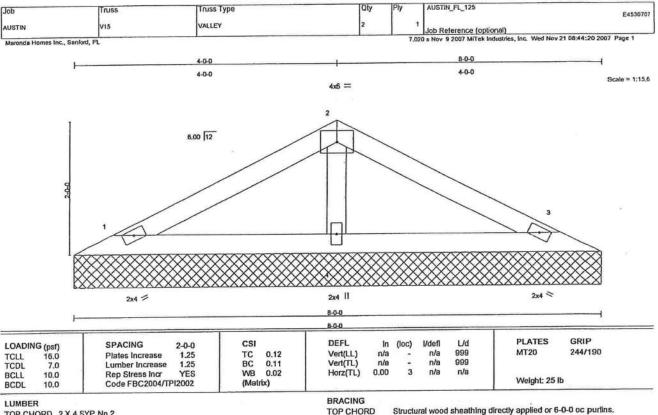
This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MILT473 BEFORE USE.

Design void for use only with Milek connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer. For general guidance regarding to the construction of the constitutions is the responsibility of the building designer. For general guidance regarding tobatication, quality control, storage, delivery, erection and bracing, careut.

AMSI/TRI Quality Criteria, DSS-89 and 8CSI) Building Component Safety Information available from Trust Plate Institute, SSS D'Onatrio Drive, Madson, VRI 53719.





TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.2 OTHERS

REACTIONS (lb/size) 1=136/8-0-0, 3=136/8-0-0, 4=310/8-0-0

Max Horz 1=28(LC 4)
Max Uplift1=44(LC 6), 3=49(LC 7), 4=21(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD BOT CHORD 1-2=-44/39, 2-3=-44/38 1-4=-2/18, 3-4=-2/18

2-4=-141/111 WEBS

NOTES

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-fise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Gable requires continuous bottom chord bearing.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 44 lb uplift at joint 1, 49 lb uplift at joint 3 and 21 lb uplift at joint 4.

LOAD CASE(S) Standard

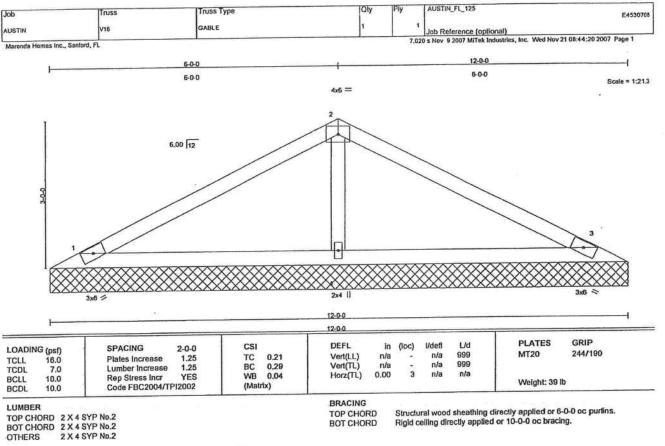
This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-1473 BEFORE USE.

Design valid for use only with Milek connector. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not loss designer. Fracting shown is for toleral support of individual web members only. Additional temporary bracing to insure slobily during construction is the responsibility of the erector. Additional permanent bracing of the overal structure is the responsibility of the building designer, for general guidance regarding tobaccions, quality control, storage, delivery, erection and bracing, consult — MSI/IPI Quality Citeria, DS8-87 and BCS11 Building Component Safety Information available from Trus Plate Institute, SS3 D'Onatrio Drive, Modison, WI SS719.





REACTIONS (lb/size) 1=191/12-0-0, 3=191/12-0-0, 4=544/12-0-0

Max Horz (=-45(LC 4) Max Uplift1=-49(LC 6), 3=-57(LC 7), 4=-77(LC 6) Max Grav1=196(LC 10), 3=196(LC 11), 4=544(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-70/62, 2-3=-70/60

TOP CHORD BOT CHORD

1-4=-3/34, 3-4=-3/34

WEBS

2-4=-268/190

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Gable requires continuous bottom chord bearing.

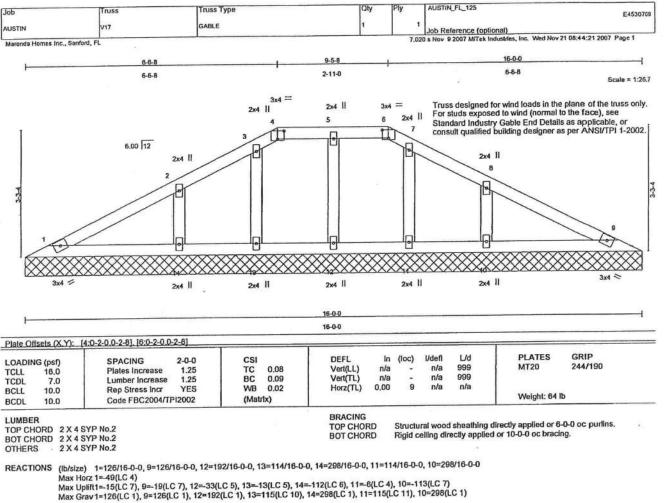
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at Joint 1, 57 lb uplift at Joint 3 and 77 lb uplift at joint 4.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USS. Design valid for use only with Milek connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer- not inus designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall shucture is the responsibility of the building designer, for general guidance regarding labelcolon, qualify control, storage, delivery, erection and bracing, consult. ANSI/TRI Quality Criteria, DSS-89 and 8CSI1 Building Component Salety Information. available from Truss Plate Institute, 583 D'Onofrio Drive, Madson, WI S3719.





FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-42/38, 2-3=-47/79, 3-4=-33/100, 4-5=-15/97, 5-6=-15/97, 6-7=-33/100, 7-8=-47/79, 8-9=-39/24 TOP CHORD 1-14=0/49, 13-14=0/49, 12-13=0/49, 11-12=0/49, 10-11=0/49, 9-10=0/49

BOT CHORD 5-12=99/64, 3-13=62/41, 2-14=-156/157, 7-11=-62/41, 8-10=-156/157

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Gable requires continuous bottom chord bearing.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 1, 19 lb uplift at joint 9, 33 lb uplift at joint 12, 13 lb uplift at joint 13, 112 lb uplift at joint 14, 6 lb uplift at joint 11 and 113 lb uplift at joint 10.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDE BUTER REPREADE AND ENLIPTED BY THE REPREADE FOR USE.

Design volid for use only with Milek connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of building designer - not inus designer. Bracing shown is for kateral support of individual web members only. Additional temporary bracing to inswess stability during construction is the responsibility of the properties. Additional permanent bracing of the overall shucture is the responsibility of the building designer, for general guidance regarding erector. Additional permanent bracing of the overall shucture is the responsibility of the building designer, for general guidance regarding erector. Additional permanent bracing of the overall shucture is the responsibility of the building designer, for general guidance regarding erector. Additional permanent bracing of the overall shucture is the responsibility of the building control storage, delivery, erection and bracing. ANSI/IPII Quality Criteria, DSS-89 and BCSII Building Component Salety Information available from Truss Plate Institute, SS3 D'Onotrio Drive, Modson, WI 53719.



nton, NC 27932

AUSTIN_FL_125 QN Truss Truss Type E4530710 AUSTIN V18 GABLE Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Wed Nov 21 08:44:22 2007 Page 1 nda Homes Inc., Sanford, FL 13-5-8 20-0-0 6-6-8 6-11-0 Truss designed for wind loads in the plane of the truss only For studs exposed to wind (normal to the face), see Scale = 1:33.3 Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002. 3x4 = 3x4 = 2x4 !! 2x4 !! 2×4 11 2x4 || 2x4 || 5 6 6.00 12 2x4 || 2x4 11 10 3x4 2x4 || 2x4 11 2x4 || 5x6 = 2x4 || 2x4 || 2x4 || 20-0-0 20-0-0 Plate Offsets (X.Y): [4:0-2-0.0-2-8], [8:0-2-0.0-2-8], [15:0-3-0.0-3-0] PLATES GRIP DEFL (loc) LOADING (psf) SPACING 2-0-0 244/190 Vert(LL) n/a n/a 999 MT20 1.25 TC 0.08 TCLL 16.0 7.0 Plates Increase 999 Lumber Increase 1.25 RC. 0.09 Vert(TL) n/a n/a Rep Stress Incr WB 0.02 BCLL 10.0 YES Horz(TL) Code FBC2004/TPI2002 Weight: 83 lb (Matrix) BCDL 10.0 BRACING LUMBER Structural wood sheathing directly applied or 6-0-0 oc purlins. TOP CHORD TOP CHORD 2 X 4 SYP No.2 Rigid ceiling directly applied or 10-0-0 oc bracing. **BOT CHORD** BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.2 OTHERS REACTIONS (lb/size) 1=125/20-0-0, 11=125/20-0-0, 15=167/20-0-0, 16=183/20-0-0, 17=117/20-0-0, 18=298/20-0-0, 14=183/20-0-0, 13=117/20-0-0, 12=298/20-0-0 Max Horz 1=49(LC 5) Max Uplift1=14(LC 7), 11=17(LC 7), 15=46(LC 4), 16=39(LC 5), 17=13(LC 5), 18=113(LC 6), 14=37(LC 5), 13=5(LC 4), 12=113(LC 7)
Max Grav 1=125(LC 1), 11=125(LC 1), 15=168(LC 10), 16=183(LC 1), 17=118(LC 10), 18=298(LC 1), 14=183(LC 1), 13=118(LC 11), 12=298(LC 1) FORCES (lb) - Maximum Compression/Maximum Tension 1-2=46/35, 2-3=-44/72, 3-4=-32/93, 4-5=-13/90, 5-6=-13/90, 6-7=-13/90, 7-8=-13/90, 8-9=-32/93, 9-10=-44/72,

TOP CHORD

10-11=-37/27

1-18=0/52, 17-18=0/52, 16-17=0/52, 15-16=0/52, 14-15=0/52, 13-14=0/52, 12-13=0/52, 11-12=0/52

BOT CHORD 6-15--91/91, 5-16--96/75, 3-17--63/41, 2-18--156/156, 7-14--96/75, 9-13--63/41, 10-12--156/156

WEBS NOTES

1) Unbalanced roof live loads have been considered for this design.

1) Unidamics from the loads have been consisted to the design.
2) Wind: ASCE 7-02; 125mph (3-second gust): h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and G-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Gable requires continuous bottom chord bearing.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 14 lb uplift at joint 1, 17 lb uplift at joint 11 , 46 lb uplift at joint 15, 39 lb uplift at joint 16, 13 lb uplift at joint 17, 113 lb uplift at joint 18, 37 lb uplift at joint 14, 5 lb uplift at joint 13 and 113 lb uplift at joint 12.

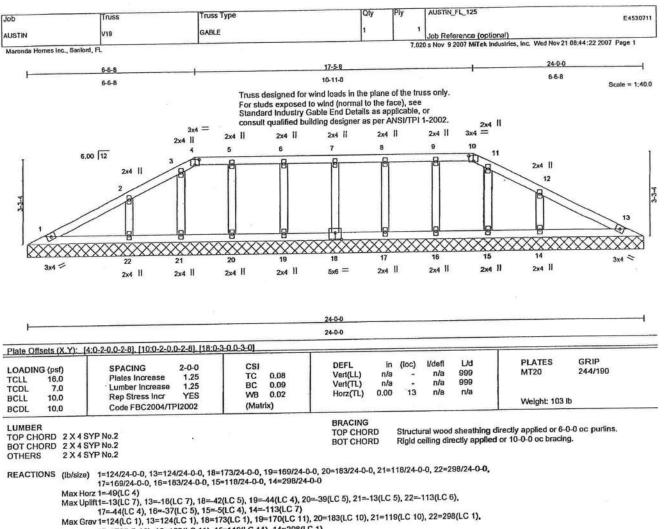
LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 BEFORE USE. Design valid for use only with Nifek connectors. This design is based only upon parameters than, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not inus designer, Broching shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the subject of the properties of the responsibility of the subject of the properties of the responsibility of the subject of the properties of the responsibility of the subject of the properties of the responsibility of the building control permanent bracking of the overall shuckure is the responsibility of the building designer, for general guidance regarding to labelication, quality control, storage, delivery, erection and bracking, consult.

ANSI/TPIT Quality Criteria, DSS-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, S83 D'Onotrio Drive, Madison, WI 53719.





17=170(LC 10), 16=183(LC 11), 15=119(LC 11), 14=298(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=48/34, 2-3=42/69, 3-4=-30/90, 4-5=-11/87, 5-6=-11/87, 6-7=-11/87, 7-8=-11/87, 8-9=-11/87, 9-10=-11/87, TOP CHORD

10-11=-30/90, 11-12=-42/69, 12-13=-35/29

1-22=0/53, 21-22=0/53, 20-21=0/53, 19-20=0/53, 18-19=0/53, 17-18=0/53, 16-17=0/53, 15-18=0/53, 14-15=0/53, **BOT CHORD**

13-14=0/53

7-18=92/85, 6-19=-92/89, 5-20=-96/76, 3-21=-64/42, 2-22=-155/156, 8-17=-92/89, 9-16=-96/76, 11-15=-64/42,

12-14=-155/156

NOTES

WEBS

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Gable requires continuous bottom chord bearing.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at Joint 1, 16 lb uplift at Joint 13 , 42 lb uplift at Joint 18, 44 lb uplift at joint 19, 39 lb uplift at joint 20, 13 lb uplift at joint 21, 113 lb uplift at joint 22, 44 lb uplift at joint 17

, 37 lb uplift at joint 16, 5 lb uplift at joint 15 and 113 lb uplift at joint 14.

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

▲ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an Individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not linus designer. Bracing show his for lateral support of individual web members only. Additional temporary bracing to Insure stability during construction is the responsibility of its related support of individual web members only. Additional temporary bracing to Insure stability during construction is the responsibility of its manufactured segment guidence regarding erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, for general guidence regarding foliocition, quality control, storage, delivery, erection and bracing, consult. ANSI/IPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safely Information available from Truss Plate Institute, 583 D'Onotifio Drive, Madison, WI S3719.



Job	Truss		Truss Type			Qly	Ply	AUST	IN_FL_125			E4530713
AUSTIN	V20		GABLE			1		Job R	eference (optional)		
Maronda Homes Inc.	, Sanford, FL						7.	020 s Nov	9 2007 MIT	ek Industries, Inc.	Wed Nov 21 08:44:	24 2007 Page 1
					21-5-8						28-0-0	
-	6-6-8 6-6-8				14-11-0					U. #. ∃	6-6-8	Scale = 1:46.9
1 1 3x4 =	6.00 12	3x4 =	5	For stude ex	ned for wind lo posed to wind dustry Gable E iffed building d 8	(normal nd Detail	o the fac s as appl s per AN	æ), see licable, o	r	3x4 = 12 13	14	15 25 3x4 \(\)

			28-0-0						
LOADING (psf) TCLL 16.0 TCDL 7.0	4:0-2-0.0-2-8]. [12:0-2-0.0-2-8]. [21:0-3 SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES	CSI TC 0.08 BC 0.09 WB 0.02	DEFL Vert(LL) Vert(TL) Horz(TL)	In n/a n/a 0.00	(loc) - - 15	I/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
BCLL 10.0 BCDL 10.0	Code FBC2004/TPI2002	(Malrix)	9.00					Weight: 123	lb

28-0-0

LUMBER

TOP CHORD 2 X 4 SYP No.2

OTHERS 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=123/28-0-0, 15=123/28-0-0, 21=172/28-0-0, 22=173/28-0-0, 23=169/28-0-0, 24=183/28-0-0, 25=119/28-0-0, 26=298/28-0-0, 20=173/28-0-0, 19=169/28-0-0, 18=183/28-0-0, 17=119/28-0-0, 16=298/28-0-0

May Horz 1= 49(LC 4)

Max Uplift1=-12(LC 7), 15=-16(LC 7), 21=-42(LC 4), 22=-42(LC 5), 23=-44(LC 4), 24=-40(LC 5), 25=-14(LC 5), 26=-112(LC 6), 20=-42(LC 5), 19=-44(LC 4), 18=-38(LC 5), 17=-6(LC 4), 16=-113(LC 7)

Max Grav1=123(LC 1), 15=123(LC 1), 21=172(LC 11), 22=173(LC 1), 23=170(LC 11), 24=183(LC 10), 25=119(LC 10),

26=298(LC 1), 20=173(LC 1), 19=170(LC 10), 18=183(LC 11), 17=119(LC 11), 16=298(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

Maximum Compression/Maximum rension
1-2=-50/33, 2-3=-40/66, 3-4=-28/88, 4-5=-10/85, 5-6=-10/85, 6-7=-10/85, 7-8=-10/85, 8-9=-10/85, 9-10=-10/85, 10-11=-10/85, 11-12=-10/85, 12-13=-28/88, 13-14=-40/66, 14-15=-33/30
1-26=0/54, 25-26=0/54, 24-25=0/54, 23-24=0/54, 22-23=0/54, 21-22=0/54, 20-21=0/54, 19-20=0/54, 18-19=0/54, 18-19=0/54, 18-19=0/54, 25-26=0/54, 24-25=0/54, 23-24=0/54, 22-23=0/54, 21-22=0/54, 20-21=0/54, 19-20=0/54, 18-19=0/5 TOP CHORD

BOT CHORD

17-18=0/54, 16-17=0/54, 15-16=0/54 8-21=92/86, 7-22=92/86, 6-23=-92/88, 5-24=-97/77, 3-25=-64/43, 2-26=-155/156, 9-20=-92/86, 10-19=-92/88,

11-18=-97/77, 13-17=-64/43, 14-16=-155/156

NOTES

WEBS

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Calegory II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) All plates are 2x4 MT20 unless otherwise indicated.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 12 ib uplift at joint 1, 16 lb uplift at joint 15

.42 lb uplift at joint 21, 42 lb uplift at joint 22, 44 lb uplift at joint 23, 40 lb uplift at joint 24, 14 lb uplift at joint 25, 112 lb uplift at joint 26 This document was originally

.42 lb uplift at joint 20, 44 lb uplift at joint 19, 38 lb uplift at joint 18, 6 lb uplift at joint 17 and 113 lb uplift at joint 16.

issued by Lassiter, Frank

LOAD CASE(S) Standard

on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milet connections. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design poramenters and proper incorporation of component is responsibility of building designer, not inust designer. Bracing shown
is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the
incorporation of the parameters of the overall structure is the responsibility of the building construction is the responsibility of the
area of the parameters of the control of the overall structure is the responsibility of the building construction of the control of the coveral structure is the responsibility of the building control control of the coveral structure is the responsibility of the building control of the coveral structure is the responsibility of the building designer, for general guidance regarding
area of the property of the coveral structure is the responsibility of the building designer, for general guidance regarding
area of the property of the coveral structure is the responsibility of the building designer, for general guidance regarding
area of the property of the coveral structure is the responsibility of the building designer, for general guidance regarding
area of the property of the coveral structure is the responsibility of the building designer, for general guidance regarding
area of the property of the coveral structure is the responsibility of the building designer, for an individual building control is a structure in the property of the prope



AUSTIN_FL_125 Truss Type E4530718 Truss VALLEY Job Reference (optional)
7.010 s Oct 16 2007 MiTek Industries, Inc. Wed Nov 21 10:01:45 2007 Page 1 AUSTIN 2-2-8 2-0-0 0-2-8 2-0-0 2 Scale: 1.5'=1' 2x4 11 6.00 12 2x4 % 2-2-8 2-2-8 GRIP PLATES I/defl in (loc) DEFL 244/190 SPACING 2-0-0 MT20 LOADING (psf) n/a n/a 999 Vert(LL) TC BC 0.01 1.25 Plates Increase Lumber Increase n/a 999 TCLL Vert(TL) n/a 0.01 1.25 TCDL 7.0 n/a n/a WB 0.00 Horz(TL) 0.00 YES Rep Stress Incr Weight: 7 lb 10.0 BCLL Code FBC2004/TPI2002 (Matrix) BCDL 10.0 BRACING Structural wood sheathing directly applied or 2-2-8 oc purlins, LUMBER TOP CHORD TOP CHORD 2 X 4 SYP No.2 except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing. 2 X 4 SYP No.2 BOT CHORD **BOT CHORD** 2 X 4 SYP No.2 WEBS REACTIONS (Ib/size) 1=48/2-2-8, 3=48/2-2-8 Mex Horz 1=30(LC 6) Max Uplift 1=-2(LC 6), 3=-21(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-24/11, 2-3=-26/36 TOP CHORD

1-3=0/0 BOT CHORD

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) WARNING: Top chord roof live load is below minimum required by ASCE 7. The building design professional for the overall structure to verify adequacy of top chord live load.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2 lb uplift at joint 1 and 21 lb uplift at

LOAD CASE(S) Standard

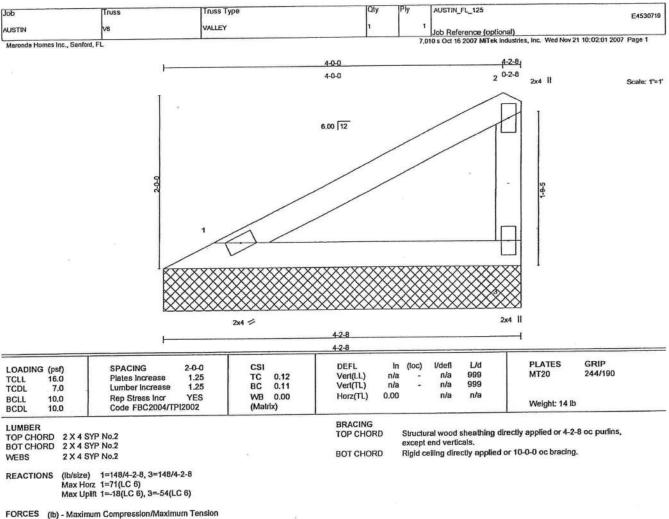
This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTER REFERENCE FACE MIL-7473 REPORE USE.

Design volid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design parameters and proper incorparation of component is responsibility of building designer - not lass designer. Brocking shows to follow support of helividual web members only. Additional temporary brocking to insure stability during construction is the responsibility of the building designer, for general guidance regarding erector. Additional permanent brocking of the overal shadure is the responsibility of the building designer, for general guidance regarding flobrication, qualify control, strange, delivery, erection and brocking, consul. ANSI/TP11 Quality Citleria, DS8-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, SS3 D'Onofilo Drive, Modison, WI SS7 19.





1-2=-55/28, 2-3=-79/96

TOP CHORD BOT CHORD 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 3) Gable requires continuous bottom chord bearing.
 4) WARNING: Top chord roof live load is below minimum required by ASCE 7. The building design professional for the overall structure to verify adequacy of top chord live load.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 1 and 54 lb uplift at

LOAD CASE(S) Standard

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper faceoperation of component is responsibility of building designer - not insus designer. Broding shown is for folleral support of individual web members only. Additional temporary broding to insure stability during construction is the responsibility of the exponsibility of the support of individual web members only. Additional temporary broding to insure stability during construction is the responsibility of the support of individual web members only. Additional temporary broding to insure stability during construction is the responsibility of the building designer, for general guidance regarding execution, qualify control, storage, delivery, eraction and broding, consult.

ANS/IFIT Quality Criteria, DSB-89 and BCSIT Building Component Safety Intermation available from Truss Plate Institute, 583 D'Onatrio Drive, Madison, Wi 53719.



AUSTIN_FL_125 Qty Truss Type Truss E4530720 VALLEY AUSTIN Job Reference (optional) 7.020 s Nov 9 2007 MiTek Industries, Inc. Wed Nov 21 08:44:26 2007 Page 1 Maronda Homes Inc., Sanford, FL 6-2-8 6-0-0 D-2-B 6-0-0 Scale = 1:20.0 2x4 11 3 6.00 12 2x4 || 2x4 || 2x4 | 2x4 = 6-2-8 6-2-B **PLATES** GRIP DEFL CSI (loc) I/defl L/d SPACING LOADING (psf) 2-0-0 244/190 MT20 n/a n/a 999 TC 0.11 Vert(LL) Plates Increase 1.25 16.0 TCLL 999 n/a 1.25 BC 0.08 Vert(TL) n/a TCDL 7.0 Lumber Increase n/a 0.00 n/a Rep Stress Incr YES WB 0.03 Horz(TL) BCIL 10.0 Weight: 24 lb Code FBC2004/TP12002 (Matrix) 10.0 BCDL BRACING LUMBER Structural wood sheathing directly applied or 6-0-0 oc purlins, except TOP CHORD TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 Rigid ceiling directly applied or 10-0-0 oc bracing. **BOT CHORD** 2 X 4 SYP No.2 WEBS OTHERS 2 X 4 SYP No.2 REACTIONS (lb/size) 1=44/6-2-8, 4=118/6-2-8, 5=317/6-2-8 Max Horz 1=113(LC 6)
Max Uplift4=70(LC 8), 5=-167(LC 8) Max Grav 1=88(LC 6), 4=118(LC 1), 5=317(LC 1) FORCES (lb) - Maximum Compression/Maximum Tension

WEBS

TOP CHORD BOT CHORD

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified,

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

1-8=125/0, 2-6=125/34, 2-7=43/15, 3-7=44/20, 3-4=59/84 1-8=0/0, 5-8=0/0, 5-9=0/0, 4-9=0/0

3) Gable requires continuous bottom chord bearing.

2-5=-173/208

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 70 lb uplift at joint 4 and 167 lb uplift at

5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 9 lb down and 57 lb up at 4-4-0, 9 lb down and 57 lb up at 4-4-0, and 47 lb down at 1-6-1, and 47 lb down at 1-6-1 on top chord, and 21 lb up at 1-6-1, 21 lb up at 1-6-1, and 17 lb down at 4-4-0, and 17 lb down at 4-4-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-3=-46, 1-4=-40

Concentrated Loads (tb)
Vert: 7=-19(F=-9, B=-9) 8=41(F=21, B=21) 9=-33(F=-17, B=-17)

This document was originally issued by Lassiter, Frank on November 21,2007. This is not considered a sealed document. Official sealed drawings are available upon request from the manufacturer indicated November 21,2007 above.

WARNING - Verify design parameters and READ HOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with Mirek connectors. This design is based only upon parameters shown, and is for an individual butding component. Applicability of design parameters and proper incorporation of component is responsibility of building designer not inus designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the backing designer, for general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the backing designer, for general guidance regarding of obtaining quality criteria, storage, delivery, erection and bracing, consult. AMSI/IP11 Quality Criteria, DSS-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Modeon, WI 53719.

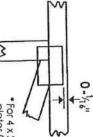


Symbols

PLATE LOCATION AND ORIENTATION



Apply plates to both sides of truss and fully embed teeth. Center plate on joint unless x, y Dimensions are in ft-in-sixteenths. offsets are indicated.



 For 4 x 2 orientation, locate edge of truss. plates 0- 118" from outside

This symbol indicates the

connector plates. required direction of slots in

Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

width measured perpendicular to slots. Second dimension is the length parallel to slots. The first dimension is the plate

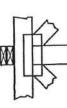
4 × 4

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



(supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Indicates location where bearings

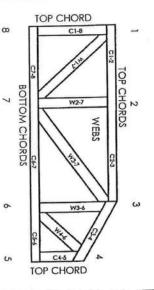
Industry Standards:

ANSI/TPI1: Building Component Safety Information. Guide to Good Practice for Handling, Plate Connected Wood Truss Construction.
Design Standard for Bracing. National Design Specification for Metal Installing & Bracing of Metal Plate Connected Wood Trusses.

DSB-89:

Numbering System

6-4-8 dimensions shown in fi-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B 9730, 95-43, 96-31, 9667A NER-487, NER-561 95110, 84-32, 96-67, ER-3907, 9432A

© 2006 MiTek® All Rights Reserved



MITek Engineering Reference Sheet: MII-7473

Damage or Personal Injury Failure to Follow Could Cause Property General Safety Notes

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSII.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Elminator bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- designer, erection supervisor, properly owner and all other interested parties. Provide copies of this truss design to the building
- Cut members to bear tightly against each other

S

- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be sultably protected from the environment in accord with ANS/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others
- 16. Do not cut or after fruss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or freated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Project Name: AUSTIN 4 BDR GAINESVILLE Address: 200 SW TIMBER RUGE DV. City, State: LAKE CHY, PL 32-1015 Owner: ELECTRIC Climate Zone: North	Builder: MARONDA HOMES Permitting Office: COLUMDIA Permit Number: Jurisdiction Number:
1. New construction or existing 2. Single family or multi-family 3. Number of units, if multi-family 4. Number of Bedrooms 5. Is this a worst case? 6. Conditioned floor area (ft²) 7. Glass type¹ and area: (Label reqd. by 13-104.4.5 if not default) a. U-factor:	12. Cooling systems a. Central Unit b. N/A c. N/A 13. Heating systems a. Electric Heat Pump b. N/A c. N/A 14. Hot water systems a. Electric Resistance b. N/A c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) 15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)
Glass/Floor Area: 0.08 Total as-built po	DACC

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY:

DATE: WALLINE CAMPBELL

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: MCIWA MYCANIE

DATE: Melissa My Cagne

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL:

DATE:

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

1	BASE				7.	AS-	BUI	ILT			-Verse	
GLASS TYPES .18 X Condition Floor Ar		SPM = I	Points	Type/SC		erhang Len		Area X	SPI	их	SOF	= Points
.18 2236.	0	18.59	7482.0	1.Single, Clear	W	1.0	6.0	16.0	43.8	34	0.97	680.0
				2.Single, Clear	N	1.0	6.0	16.0	21.7		0.97	338.0
				3.Single, Clear	W	1.0	3.0	5.0	43.8		0.85	186.0
				4.Single, Clear	W	1.0	7.0	30.0	43.8		0.98	1294.0
	0 6			5.Single, Clear	W	1.0	8.0	40.0	43.8	34	0.99	1734.0
				6.Single, Clear	Ε	1.0	6.0	20.0	47.9	92	0.97	927.0
1				7.Single, Clear	Ε	1.0	6.0	20.0	47.9	92	0.97	927.0
				8.Single, Clear	E	1.0	6.0	30.0	47.9	92	0.97	1394.0
				As-Built Total:				177.0				7480.0
WALL TYPES	Area X	BSPM	= Points	Туре		R-	Value	Area	Х	SPM	=	Points
Adjacent	320.0	0.70	224.0	1. Concrete, Int Insul, Exteri	or		4.1	1199.0		1.13		1360.9
Exterior	1199.0	1.70	2038.3	2. Frame, Steel, Adjacent			13.0	320.0		0.90		288.0
Base Total:	1519.0		2262.3	As-Built Total:				1519.0				1648.9
DOOR TYPES	Area X	BSPM	= Points	Туре				Area	Х	SPM	=	Points
Adjacent	18.0	2.40	43.2	1.Adjacent Wood				18.0		2.40		43.2
Exterior	20.0	6.10	122.0	2.Exterior Insulated				20.0		4.10		82.0
Base Total:	38.0		165.2	As-Built Total:				38.0				125.2
CEILING TYPES	Area X	BSPM	= Points	Туре	-	R-Valu	10 /	Area X S	DM	V 90	NA -	
Under Attic				iii		-					IVI —	Points
Orider Attic	2236.0	1.73	3868.3	1. Under Attic		,	19.0	2450.0 2	.34 X	1.00		5733.0
Base Total:	2236.0		3868.3	As-Built Total:				2450.0	100			5733.0
FLOOR TYPES	Area X	BSPM	= Points	Туре		R-	Value	Area	Х	SPM	=	Points
	19.0(p)	-37.0	-8103.0	1. Slab-On-Grade Edge Insu	lation		0.0 2	219.0(p	-4	41.20		-9022.8
Raised	0.0	0.00	0.0						68			
Base Total:			-8103.0	As-Built Total:				219.0				-9022.8
INFILTRATION	Area X	BSPM	= Points	SOLUTION OF THE PROPERTY OF TH				Area	X	SPM	=	Points
	2236.0	10.21	22829.6			-		2236.0		10.21		22829.6

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS:,,,	PERMIT #:

	BASE		AS-BUILT								
Summer Ba	se Points: 2	28504.3	Summer As-Built Points: 28793.8								
Total Summer Points	X System : Multiplier	= Cooling Points	Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)								
28504.3	0.3250	9263.9	(sys 1: Central Unit 40500btuh ,SEER/EFF(13.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS) 28794 1.00 (1.08 x 1.147 x 0.86) 0.260 0.950 7623.4 28793.8 1.00 1.072 0.260 0.950 7623.4								

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

BAS	E			,	AS-	BUI	LT				
GLASS TYPES .18 X Conditioned X Floor Area	BWPM =	Points	Type/SC		erhang Len	Hgt	Area X	WP	мх	WOF	= Points
.18 2236.0	20.17	8118.0	1.Single, Clear	W	1.0	6.0	16.0	28.8	4	1.01	465.0
			2.Single, Clear	N	1.0	6.0	16.0	33.2	2	1.00	531.0
			3.Single, Clear	W	1.0	3.0	5.0	28.8	4	1.04	150.0
			4.Single, Clear	W	1.0	7.0	30.0	28.8	4	1.00	869.0
			5.Single, Clear	W	1.0	8.0	40.0	28.8	4	1.00	1157.0
1			6.Single, Clear	E	1.0	6.0	20.0	26.4		1.02	536.0
			7.Single, Clear	E	1.0	6.0	20.0	26.4		1.02	536.0
			8.Single, Clear	E	1.0	6.0	30.0	26.4	1	1.02	804.0
			As-Built Total:				177.0				5048.0
WALL TYPES Area	X BWPM	= Points	Туре		R-	Value	Area	Х	WPN	1 =	Points
Adjacent 320.0	3.60	1152.0	1. Concrete, Int Insul, Exterior			4.1	1199.0		6.42		7697.6
Exterior 1199.0	3.70	4436.3	2. Frame, Steel, Adjacent		10	13.0	320.0		4.90		1568.0
Base Total: 1519.0)	5588.3	As-Built Total:				1519.0				9265.6
DOOR TYPES Area	X BWPM	= Points	Туре				Area	Х	WPM	=	Points
Adjacent 18.0	11.50	207.0	1.Adjacent Wood				18.0		11.50		207.0
Exterior 20.0	12.30	246.0	2.Exterior Insulated				20.0		8.40		168.0
20 20 4		e " = x	Prose de								
Base Total: 38.0)	453.0	As-Built Total:			-	38.0				375.0
CEILING TYPES Area	X BWPM	= Points	Туре	R	-Value	Ar	ea X W	PM 2	K WC	:M =	Points
Under Attic 2236.0	2.05	4583.8	1. Under Attic			19.0	2450.0 2	2.70 X	1.00		6615.0
Base Total: 2236.0		4583.8	As-Built Total:				2450.0				6615.0
FLOOR TYPES Area	X BWPM	= Points	Туре		R-V	/alue	Area	Х	WPM	=	Points
Slab 219.0(p) Raised 0.0	8.9 0.00	1949.1 0.0	1. Slab-On-Grade Edge Insula	ation		0.0	219.0(p		18.80		4117.2
Base Total:		1949.1	As-Built Total:				219.0				4117.2
INFILTRATION Area	X BWPM	= Points		//X			Area	X	WPM	=	Points
2236.0	-0.59	-1319.2	No. of the last of				2236.0)	-0.59		-1319.2

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:	
----------------	-----------	--

ii.	BASE		AS-BUILT								
Winter Base	Points:	19373.0	Winter As-Built Points: 24101.5								
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit = Heating Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)								
19373.0	0.5540	10732.6	(sys 1: Electric Heat Pump 40500 btuh ,EFF(8.1) Ducts:Unc(S),Con(R),Int(AH),R6.0 24101.5 1.000 (1.060 x 1.169 x 0.88) 0.421 0.950 10552.7 24101.5 1.00 1.095 0.421 0.950 10552.7								

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: , , , PERMIT #:

	BASE	AS-BUILT										
WATER HEA Number of Bedrooms	TING	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	х	Tank X Ratio	Multiplier	X Credit Multiplier	
4		2635.00		10540.0	50.0	0.90	4		1.00	2693.56	1.00	10774.2
					As-Built To	otal:						10774.2

		CODE	E C	OMPLI	ANCE	S	TATUS	3			
	BAS	SE						AS	-BUILT		
Cooling Points	+ Heating Points	+ Hot Wate Points	r 🖣	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
9264	10733	10540	Balancian	30537	7623		10553	150	10774		28950

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS:	
ADDRESS: , , ,	PERMIT #:
	S

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	1
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	/
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	√ ·
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	V
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	1
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	/

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	1
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	1
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	V
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	V
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	V

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.8

The higher the score, the more efficient the home.

ELECTRIC, ...

1.	New construction or existing	New	12	Cooling systems	
2.	Single family or multi-family	Single family		a. Central Unit	Cap: 40.5 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 13.00
4.	Number of Bedrooms	4		b. N/A	
5.	Is this a worst case?	Yes			-
6.	Conditioned floor area (ft2)	2236 ft²		c. N/A	-
7.	Glass type 1 and area: (Label reqd. by	13-104.4.5 if not default)			laborate de la companya de la compan
a.	U-factor:	Description Area	13	3. Heating systems	
b	(or Single or Double DEFAULT) 7a SHGC:	a(Sngle Default) 177.0 ft ²	<u> </u>	a. Electric Heat Pump	Cap: 40.5 kBtu/hr _ HSPF: 8.10 _
8.	(or Clear or Tint DEFAULT) 71 Floor types	b. (Clear) 177.0 ft ²	_	b. N/A	w.
b.	Slab-On-Grade Edge Insulation N/A	R=0.0, 219.0(p) ft		c. N/A	· -
с. 9.	N/A		14	4. Hot water systems	
a. b.	Wall types Concrete, Int Insul, Exterior Frame, Steel, Adjacent N/A	R=4.1, 1199.0 ft ² R=13.0, 320.0 ft ²		a. Electric Resistance b. N/A	Cap: 50.0 gallons EF: 0.90
	N/A		_	c. Conservation credits	-
e.	N/A		_	(HR-Heat recovery, Solar	_
10.	Ceiling types		_	DHP-Dedicated heat pump)	
	Under Attic	R=19.0, 2450.0 ft ²	15	5. HVAC credits	PT,
b.	N/A			(CF-Ceiling fan, CV-Cross ventilation,	11, _
c.	N/A			HF-Whole house fan,	
11.	Ducts			PT-Programmable Thermostat,	
a.	Sup: Unc. Ret: Con. AH(Sealed):Inte	erior Sup. R=6.0, 200.0 ft	22.00	MZ-C-Multizone cooling,	
	N/A		_	MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

based on installed Code compliant feature Builder Signature: White Market Signature: White Signature

Date: 05 27 108

mber Ridge Dity/FL Zip: Lake City, For

Address of New Home

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building

Construction, contact the Department of Community Affairs at 850/487-1824.



COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection
This Certificate of Occupancy is issued to the below named permit holder for the building

and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 10-4S-16-02856-109

Fire: 70.62

Building permit No. 000027219

Use Classification SFD/UTILITY

Permit Holder THEODORE C. BROCK

Waste: 184.25

Owner of Building MARONDA HOMES INC., OF FLORIDA

Total: 254.87

Location: 299 SW TIMBERRIDGE DRIVE, LAKE CITY, FL

Date: 11/14/2008

N. Kusa

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)

GEO-TECH, INC.

Engineering Consultants in Geotechnical • Environmental • Construction Materials Testing

27219

unsatisfactory test results.

FIEL	D DEN	SITY V	VORKS					
CLIENT MATON DA HUA	MES						Julyo	8
PROJECT NAME TIMBER LAHES	1	13111	city	/	PERMI	CT NO	-	
PROJECT NAME TIMBER LAHES EARTH CONTRACTOR LOT H 9	JUB	H	itm	0901	TESTE	D BY	5146	-
COMPACTION REQUIREMENT (%)	9890	□ Sta	andard Fodified P	Proctor		PATRICI	C_FIELD	CONTACT
TOTAL ON-SITE TIME							FICE	
□ Limerock □ Subgrade □ Pipe Backfill □	Building	Pad 🗷	Building	Footing	g 🗆 Oth	ner		
	LAB PR	OCTOR	TEST	PROBE	%	WET DENSITY	DRY DENSITY	%
TEST LOCATION	DENS.	OMC	DEPTH	DEPTH	MOIST.	(PCF)	(PCF)	COMP.
CTR, OS POP	104.8	10.1	Fh	124	6.8	110.6		
CTR. OF N. FH	1)			7.2	107.7	100.5	95.8
Ch. of S. F. th	7	r	1	4	6.9	107.7	100.7	96.0
	1 1	4.						*
								- 1
REMARKS						minii requ ** Rete dens obtai	sity failed to mum project irement est indicates sity requirer ined. nt is aware	et s minimum ment was

LOT NINE (9) OF "IMBERLANDS" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGE 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA. LEGAL

CERTIFIED TO:

1) MARONDA HOMES

BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'



)POSED •

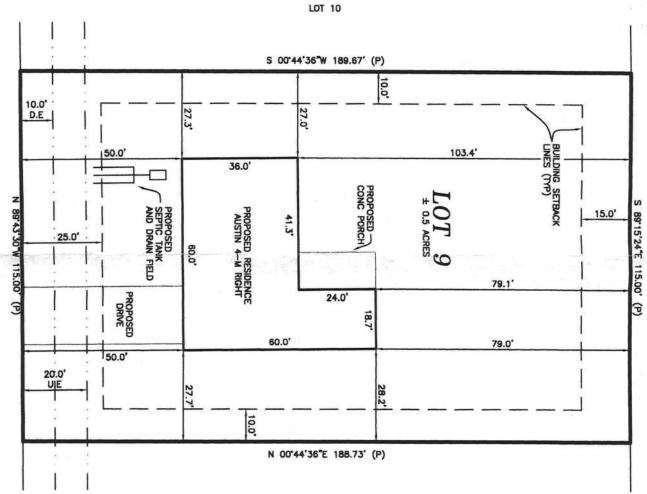
LAYOUT

IN SECTION 10, 16 EAST, COLUMBIA COUNTY, FLORIDA TOWNSHIP 4 SOUTH, RANGE

1, 30

30

90



LOT 8

FLOOD NOTE:

BUILDING SETBACK LINES DEPICTED HEREON ARE SUBJECT SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE, PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.

THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).

AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

3 2)

IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.

=

SURVEYOR NOTES:

TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.

ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.

TIMBER RIDGE DRIVE

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENUMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED

TILE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

o = FOUND 1/2" REBAR & CAP LB. 6894 ● = FOUND 1/2" REBAR NO IDENTIFICATION

ABBREVATIONS:

A/C = AIR CONDITIONER

C = CALCULATED FROM MEASURED

CAT = CANDIE TELEVISION

C/B = CONCRETE BLOCK

CLF = CHAIN LINK FENCE

CM = CONCRETE MONUMENT

O = SET 1/2" REBAR & CAP LB. 6894

= CONCRETE
= ELECTRIC
= ELEVATION
= FOUND
= FENCE

- Ø = FOUND 3/4" IRON PIPE
 ■ = FOUND 4" X 4" CONC. MON.
 NO IDENTIFICATION SET 4" X 4" CONC. MON. P.S.M. 5582
- X = FOUND NAIL & DISK

 E = FOUND 6" X 6" S.R.D.

 R/W MON. X = SET NAIL & DISK P.S.M. 5582 O.U.
 - B = LICENSED SURVEYOR BUSINESS
 H) = FIELD MEASURED
 H = MANHOLE
 L = OVERHEAD UTILTIES
 P = PLAT PUBLIC UTILITIES EASEMENT
- TRANSFORMER
 TYPICAL
 WATER METER
 WATER VALVE

Z = TELEPHONE PEDESTAL W = WOOD POWER POLE

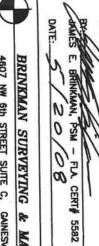
B = CATV RISER

THIS IS NOT A BOUNDARY SURVEY

CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF
A FLORIDA LICENSED SURVEYOR AND MAPPER, ADDITIONS OR DELETIONS
TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLURIDA BOARD OF JAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES AND CHAPTER 1919.77—6, FLORIDA ADMINISTRATIVE CODE.



BRINKMAN SURVEYING & MAPPING INC.

4607 NW 6th STREET SUITE C. GAINESVILLE, FL 32609

CHECKED BY: J.B.	SERVICE	COALL	2	THE BENCHMARK IN QUALITY SERVICE	DATE: 5/19/08
DRAWN BY: ZL	SERVICE.	2			SCALE: 1" = 30'
L-8757	FAX: (352) 374-8757	FAX		PHONE: (352) 374-7707	PHONE:

FIELD WORK COMPLETED ON **** FIELDBOOK **, PAGE DATE: 5/19/08

PREPARED FOR: MARONDA

DRAWING NUMBER 114-08

LEGAL DESCRIPTION:

LOT NINE (9) OF "TIMBERLANDS, PHASE 1" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGES 26—27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

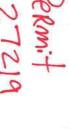


1) MARONDA HOMES

CERTIFIED TO:

BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'



BO ARY 7 YEY

 \equiv SECTION 16 EAST, COLUMBIA COUNTY, FLORIDA <u>1</u>0, TOWNSHIP 4 SOUTH, RANGE

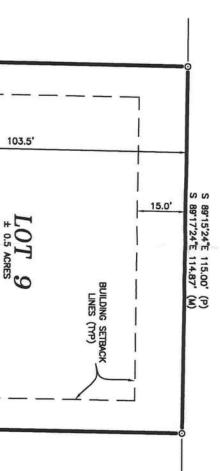
0

90

II

30

m



LOT 10 S 00'44'36"W 189.67' (P) S 00'46'27"W 189.57' (M) 10.0 27.0 29.4 LOT 9 ± 0.5 ACRES MFF = 100.00 24.0 30.7 28.2 N 00'44'36"E 188.73' (P)

X = FOUND

NAIL & DISK

NAIL & DISK P.S.M. 5757

CONC = CONCRETE
ELEC = ELECTRIC
ELEV = ELEVATION
FIND = FOUND
FNC = FENCE
LB = LICENSED SI
(M) = FIELD MEASI
MFF = MINIMUM FIN
MH = WANHOLI FIN
MH = WANHOLI FIN
MH = PLAT
PB = PLAT
PB = PLAT
PB = PLAT
TRANSFORME
TYP = WATER METE
WW = WATER METE

LB = LICENSED SURVEYOR BUSINESS

M) = FIELD MEASURED
FF = MINIMUM FINISHED FLOOR ELEVATION
(H = MANHOLE
U. = OVERHEAD UTILITIES
P = PLAT

=

SET 4" X 4" CONC. MON. P.S.M. 5582 FOUND 4" X 4" CONC. MON. P.S.M 5757

II

e = FOUND

3/4" IRON PIPE

O = SET 1/2" REBAR & CAP LB. 6894

⊙ = FOUND 1/2" REBAR & CAP LB. 7593

A/C = AIR CONDITIONER
ASPH = ASPHALT
C = CALCULATED FROM MEASURED
CATV = CABLE TELEVISION
C/B = CONCRETE BLOCK
CLF = CHAIN LINK FENCE
CM = CONCRETE MONUMENT

FOUND 1/2" REBAR NO IDENTIFICATION

LEGEND:

ABBREVIATIONS:

× II

FOUND 6" X 6" S.R.D. R/W MON.

Z = TELEPHONE PEDESTAL B = WATER VALVE

TRANSFORMER
TYPICAL
WATER METER
WATER VALVE

PUBLIC UTILITIES EASEMENT

FIRE HYDRANT

ELEC TRANSFORMER

FLOOD NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

THE NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-8B, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

SURVEYOR NOTES:

- TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.

N 00°16'30"E 29.92' (M) N 00°16'30"E 30.00'(C)

ZZ

89'43'38'W 205.39' 89'43'25'W 205.40'

EO

CONC.

٠

٠

•

N 89'43'38 W 115.00' N 89'43'38 W 114.96' BEARING BASIS

E

10.0' D.E

50,0

50,0

25.0

20.0'| U.E

•

EDGE OF PAVEMENT -

S.W.

- THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE
- AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

TIMBER RIDGE 80' RIGHT-OF-WAY ±20' ASPHALT ROAD DRIVE

CHMARK NOTE:

N 00'44'49"E 188.70' (M)

LOT 8

ELEVATIONS SHOWN HEREON ARE BASED UPON A BENCHMARK SET IN A 8" PINE AT THE FRONT OF LOT 2, WITH AN ELEVATION OF 98.76'. THIS INFORMATION WAS PROVIDED TO THIS SURVEYOR BY BRITT SURVEYING (PLATTING SURVEYOR) DATUM UNKNOWN.

27.3

36.0

FORMBOARDS FOR CONC. FOUNDATION ELEV.= 104.38'

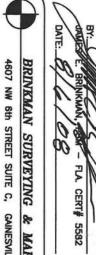
10.0

60.0

CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER, ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINING/M/TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER 87617—8, FLORIDA ADMINISTRATIVE CODE.



BRINKMAN SURVEYING & MAPPING INC.

SCALE: 1" = 30" PHONE: (352) 374-7707 GAINESVILLE, FL 32609 FAX: (352) 374-8757 DRAWN BY: ZL

PREPARED FIELD WORK COMPLETED ON 7/31/2008 DATE: 8/6/08 FOR: MARONDA "THE BENCHMARK IN QUALITY SERVICE" FIELDBOOK 98, PAGE 26 DRAWING NUMBER CHECKED BY: J.B.



HOMETEAM

PEST DEFENSE

TREATMENT WORKORDER

☐ Termite Baiting System w/Tubes-under-the slab Tubes-under-the slab and Treat

Bora-Care ☐ Treat Only DATE CALLED IN: DATE OF SCHEDULE: TIME CALLED IN: TIME SCHEDULE: SUBDIVISION: JOB NAME: JOB ADDRESS: BILLING NAME: BILLING PHONE: BILLING ADDRESS: CALLED IN BY: PHONE: LOT & MODEL NUMBER: DATE & TIME COMPLETED: 8/4/18 SOUARE FOOT: LINEAR FOOT: BLOCKVOIDS: TYPE OF FILL: SLAB TYPE: MONT APPROX. DEPTH OF FOOTING: Outside: Inside: ☐ Addition ☐ Spot Treat ☐ Pool Addition ☐ Driveway □ Other ☐ Final/Completion PESTICIDE USED: JWAYX PO TOTAL APPLIED: PERCENT (%) USED: _____/ ê STICKER POSTED: Dorn + 40 TOTAL FOR P.T. PRICE PER SQ. FT. = ADDITIONAL TAX: TOTAL AMOUNT X TECHNICIAN:

12/05

I hereby acknowledge the satisfactory completion of the above described work.

GT 23 / TCI

SCHOOL IMPACT FEE 00100003632900 FIRE PROTECTION IMPACT FEE 10200003632220 ROAD IMPACT FEE 10100003632400 CORRECTIONS IMPACT FEE 00100003632200 EMS IMPACT FEE 10300003632210 FEES: 046.00 93, 663, 67 CHECK NUMBER CODE

TOTAL FEES CHARGED