

# **TESTING • INSPECTION • ENGINEERING**

305 N. OAKLAND AVE. ● P.O. BOX 490 ● NAPPANEE. IN 46550 ● P: 574.773.7975 ● F: 574.773.2732 ● ICC-NTA.ORG

July 13, 2021

Franklin Structures, LLC 10655 Hey 43, South Russellville, AL 35653

RE: MFT10886-5078-64-3-47-R1 NTA JOB NUMBER: FH062421-5

Dear Ms. Christie Jackson,

The referenced manufactured building has been reviewed and approved. ICC NTA LLC certifies this plan is in compliance with 2020 Florida Codes – 7<sup>th</sup> Edition as referenced in the approved drawings. This approval covers the factory build structure only. Any alterations to the factory-built structure, on site, voids the approval. This plan is subject to the following limitations:

- 1. This plan is NOT approved for High Velocity Hurricane Zone (i.e. Broward and Dade Counties).
- 2. Signed and sealed plans are on file with ICC NTA, LLC
- 3. The Chapter 633 Plan Review and Inspection shall be conducted by the local fire safety inspector.
- 4. Items installed on site are subject to review and approval by the local authority having jurisdiction. Please reference the list of site installed items on the approved plans.
- 5. This review included products for compliance with 553.8425 or FAC Chapter 61G20-3.

If you have any additional questions or comments regarding this matter, please contact me at your convenience at (574) 773-7975.

Respectfully,

# Michael Faller

Michael Faller SMP-056 Account Manager ICC-NTA LLC



A MEMBER OF THE ICC FAMILY OF SOLUTIONS

# Notes:

- 1. These plans comply with the 7th Edition (2020) Florida Building Code: Residential, 7th Edition (2020) Florida Building Code: Energy Conservation and 2017 National Electrical Code.
- 2. These plans comply with Rule 61G20-3.006 for product approval.
- 3. The raised seal set (or electronic sealed set) of plans are on file in the third agency's office as directed by DBPR.
- 4. This building is subject to review and approval of the fire inspector on site with compliance with Chapter 633 Fire Safety Code.
- 5. The manufacturer's data sheet and the state (DBPR) insignia are permanently mounted to or about the electrical panel.
- 6. This building has been designed for erection or installation on a site built permanent foundation and is not designed to be moved once so erected or installed.

ANY WINDBORNE DEBRIS PROTECTION TO BE PROVIDED ON—SITE BY OTHERS AND SUBJECT TO LOCAL CODES.
WOOD STRUCTRURAL PANELS TO BE PROVIDED FOR ALL GLAZED OPENINGS PER R301.2.1.2

This building has not been designed or approved for placement in high VELOCITY HURRICANE ZONES (HVHZ), (I.E. DADE AND BROWARD COUNTIES)

INDEX TO DRAWINGS DESCRIPTION:	SHEET NUMBER
INDEX TO DRAWINGS AND NOTES	1 of 8
ELEVATIONS	2 of 8
FLOOR PLAN	3 of 8
ELECTRICAL	4 of 8
WATER SCHEMATIC	5 of 8
OFF-FRAME DRAIN SCHEMATIC	6 of 8
TYPICAL OFF-FRAME CROSS SECTION	7 of 8
TYPICAL ON-FRAME CROSS SECTION	8 of 8

### INDEX TO ADDENDUMS

WHOLE HOUSE STRUCTURAL CALCULATIONS	27	PAGES
PORCH DETAIL	4	PAGE
RESCHECK		
HVAC CALCULATIONS (WRIGHTSOFT)	6	PAGES
FLORIDA PRODUCT APPROVAL SPEC SHEET	1	PAGE

# Notes:

- 9. Vult wind speed = EXPOSURE-C AT Vult=167MPH & Vasd = 130MPH
- 10. RISK CATEGORY=II
- 11. Building Mean Roof Height 18'
- 12. Roof live load = 20 psf
- 13. Floor live load = 40 psf
- 14. Seismic Zone = A, B or C
- 15. Building Category = Type 5B, Unprotected, Wood Construction.
- 16. Use Group = Single Family Dwelling
- 17. Roof Interior (zone 1) = +34.7/-63.6psf
- 18. Roof Exterior (zone 2) = +34.7/-70 psf
- 19. Roof Corner (zone 3) = +34.7/-108.6 psf
- 20. Wall Interior (zone 4) = +37.9/-41.1 psf
- 21. Wall Exterior (zone 5) = +37.9/-50.7 psf
- 22. OVERHANG EXTERIOR LOAD=(OH Z1) -89.3 psf.
- 23. OVERHANG EXTERIOR LOAD=(OH Z2) -95.7 psf.
- 24. Overhang Corner Load = (OH-Z3) -134.3 psf
- 25. Site address per FRC R319.1
- 26. Internal pressure coefficient =  $\pm$  0.18 psf

COMPONENTS & CLADDING PRESSURES ARE SHOWN AS ALLOWABLE STRENGTH PRESSURES BASED ON ULTIMATE LOADS

# SITE INSTALLED ITEMS

NOTE: THAT THIS LIST DOES NOT NECESSARILY LIMIT THE ITEMS OF WORK AND MATERIALS THAT MAY BE REQUIRED FOR A COMPLETE INSTALLATION.
ALL SITE RELATED ITEMS ARE SUBJECT TO LOCAL BUILDING OFFICIAL REVIEW AND APPROVAL REQUIRING TO BE IN COMPLIANCE WITH THE FLORIDA BUILDING CODE

DATE 7/13/2021

- 1) THE COMPLETE FOUNDATION SUPPORT AND TIE DOWN SYSTEM
- 2) RAMPS, STAIRS, AND GENERAL ACCESS TO THE BUILDING
- 3) PORTABLE FIRE EXTINGUISHER(S)
- 4) BUILDING DRAINS, CLEAN OUTS AND HOOKUP TO PLUMBING SYSTEM
- 5) ELECTRICAL SERVICE HOOKUP, INCLUDING THE FEEDERS, TO THE BUILDING
- 6) THE MAIN ELECTRICAL PANEL AND SUB-FEEDERS
- 7) CONNECTION OF ELECTRICAL CIRCUITS CROSSING OVER MODULE MATE LINES (MULTI-UNITS ONLY)
- 8) STRUCTURAL AND AESTHETIC INTERCONNECTIONS BETWEEN MODULES (MULTI-UNITS ONLY)
- 9) ANY SITE FLASHING OR SHINGLES INSTALLED AT SITE REFER TO ARMA
- PUBLICATION " RESIDENTIAL ASPHALT ROOFING MANUAL", IN GUIDE LINES WITH FBC CODE

  10) ALL FOUNDATION WORK WILL BE COMPLETED ON SITE. IS THE RESPONSIBILITY OF THE LOCAL CONTRACTOR AND IS SUBJECT TO LOCAL JURISDICTION.
- 11) MANDATORY BLOWER DOOR TEST MUST BE COMPLETED PER FLORIDA ENERGY CODE
- 12) MAIN DISCONNECT WILL BE INSTALLED ON-SITE AND SUBJECT TO LOCAL CODES

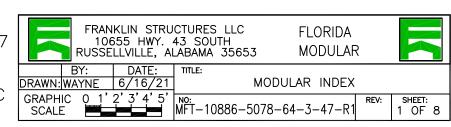
Florida Plan Number:

Model MFT10886-5078-64-3-47-R1

# PHYSICAL ADDRESS

STOCK

1800 STATE ROAD 207 ST.AUGUSTINE FL, ST. JOHNS COUNTY ONLY FOR EXPOSURE C





CERT. NO SMP-056

PLAN NUMBER MFT 10886-5078-64-347R1

APPROVED BY Michael Faller

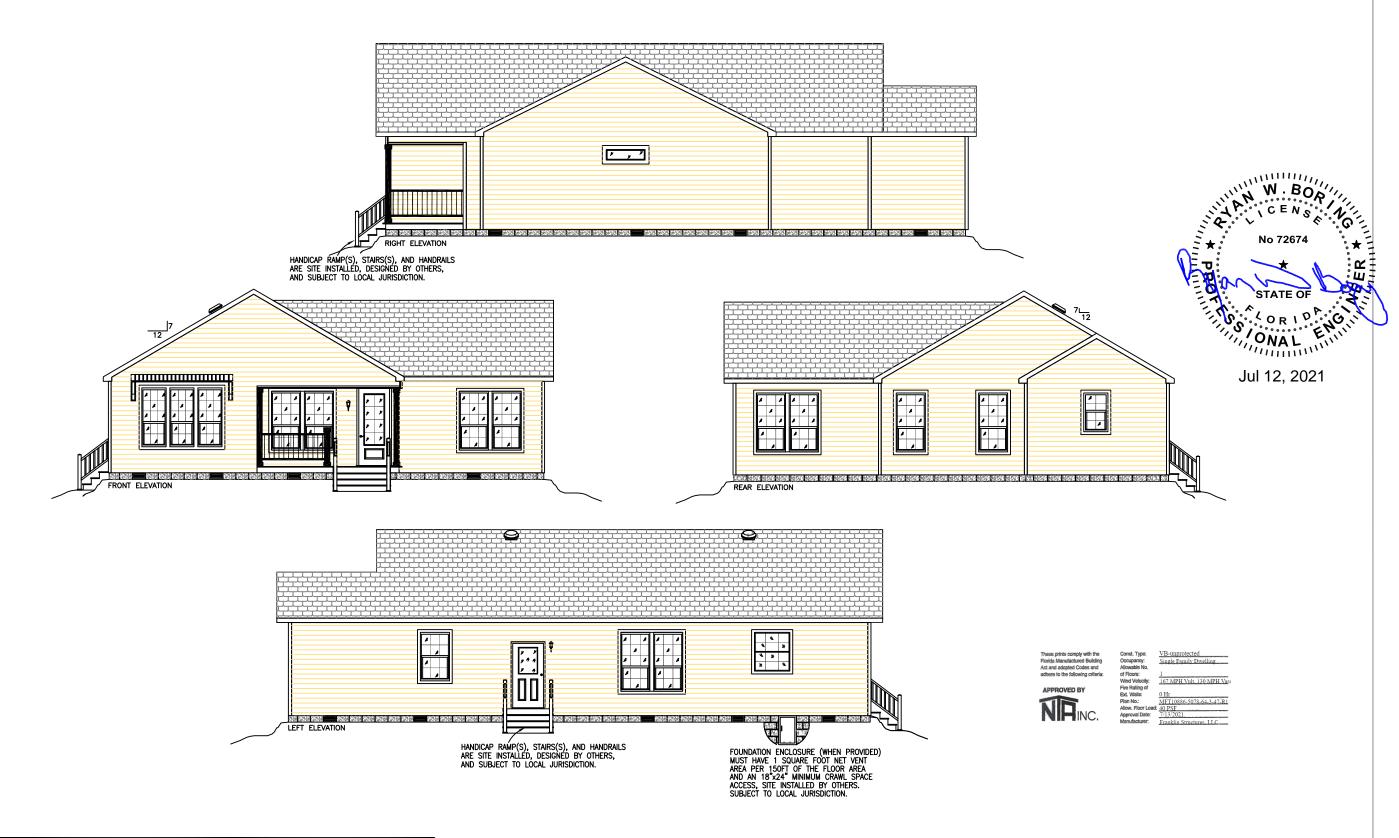
No 72674

No 72674

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Jul 12, 2021



- 1. EXTERIOR COVERING IS VINYL LAP SIDING WITH VINYL APPURTENANCES STANDARD. OTHER EXTERIOR COVERINGS SHALL CONFORM TO INTERNATIONAL BUILDING CODE.

  EXTERIOR COVERINGS FOR FRONT AND REAR ELEVATIONS ARE SUPPLIED BY FRANKLIN

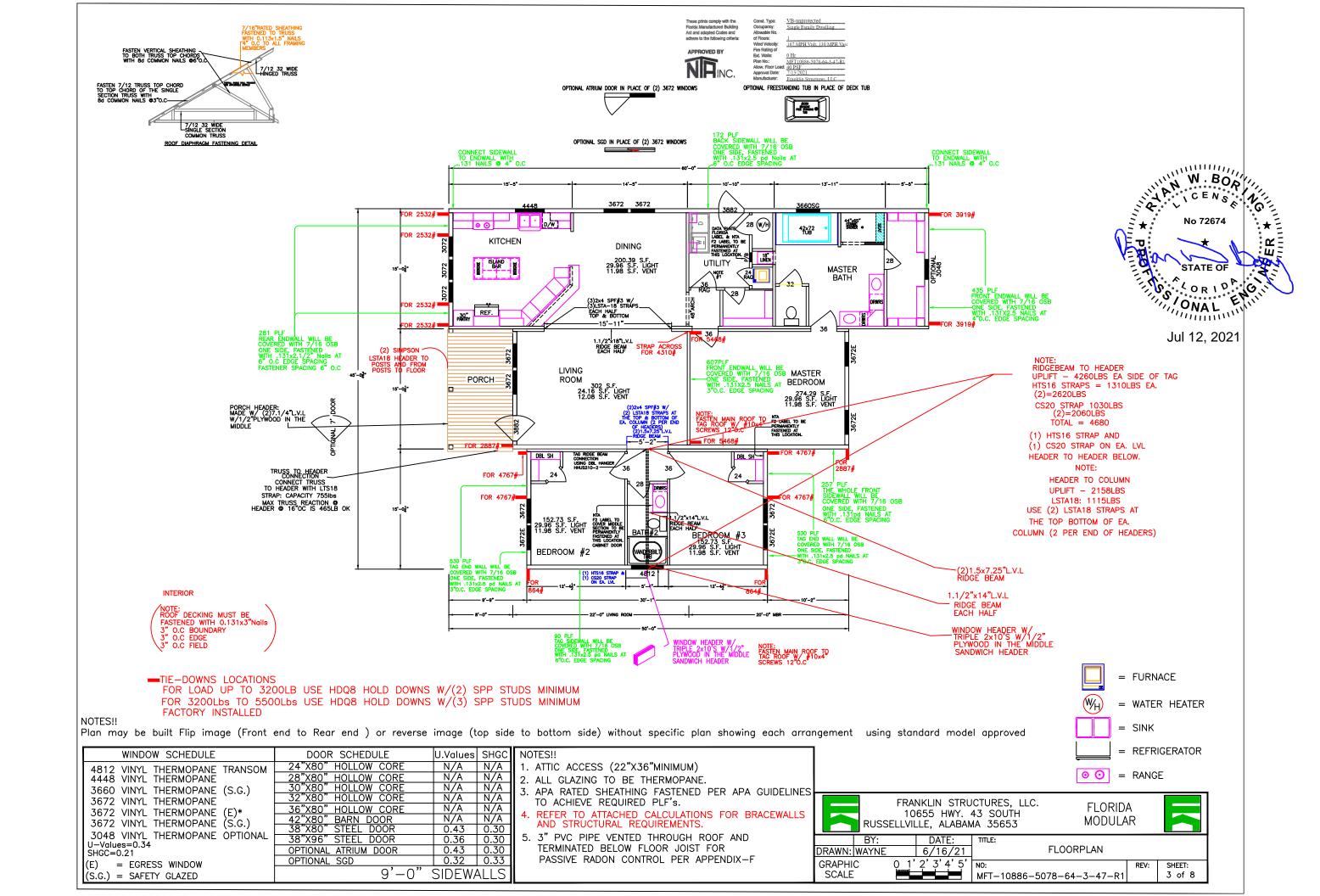
  HOMES AND INSTALLED ON—SITE BY LOCAL CONTRACTOR.

  ROOF COVERING IS 240# FIBERGLASS SHINGLES. SHINGLES FOR RIDGE ARE SUPPLIED BY

  FRANKLIN HOMES, INSTALLED ON—SITE BY LOCAL CONTRACTOR.
- WINDOWS ARE VINYL CLAD THERMOPANE.
- 4. MINIMUM ATTIC VENTILATION VIA CONTINUOUS VENTILATED SOFFIT & WHIRLY BIRD VENTS IS 8.60 SQ.FT PER 2578 SQ.FT. OF HOME DIVIDED BY 300 SQ.FT. OF CONTINUOUS VENTILATION
- 5. CRAWL SPACE VENTILATION SHALL CONFORM TO REQUIREMENTS OF 1/150 { BY OTHERS} 6. FOUNDATIONS INSTALLED BY LOCAL CONTRACTOR PER LOCAL CODE REQUIREMENTS. 7. SHUTTERS SHOWN ARE NON-STRUCTURAL (AVAILABLE AS OPTION).

NOTES!!
Plan may be built Flip image (Front end to Rear end ) or reverse image (top side to bottom side) without specific plan showing each arrangement using standard model approved

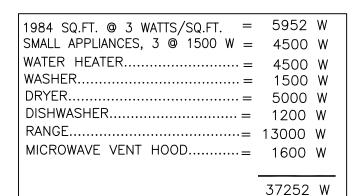
REVISION:	BY:	DATE:	FRANKLIN STRUCTURES, LLC.
			10655 HWY. 43 SOUTH FLORIDA
			RUSSELLVILLE, ALABAMA 35653 MODULAR
			BY: DATE: TITLE:
			DRAWN: WAYNE 6/16/21 ELEVATIONS
			GRAPHIC 0 1'2'3'4'5' NO:   SHEET:   SCALE   REV:   SHEET:   2 of 8
			SCALE MF1-10886-50/8-64-3-4/-R1 2 of 8



CIR#	DESCRIPTION	BREAKER PO	DLES	WIRE
1***	GENERAL LIGHTING	15 AMP/AFI	1	14-2 W/G
2**	GENERAL LIGHTING	15 AMP/AFI	1	14-2 W/G 14-2 W/G
3**	GENERAL LIGHTING	20 AMP/AFI	1	12-2 W/G
4**	GENERAL LIGHTING	15 AMP/AFI	1	14-2 W/G
5	WATER HEATER	NOTE 1		,
6	WASHER	20 AMP/AFI/GFI	1	12-2 W/G
7	PORTABLE APPLIANCE	20 AMP'/AFI'/GFI	1	12-2 W/G
8	WATER HEATER WASHER PORTABLE APPLIANCE PORTABLE APPLIANCE FURNACE	20 AMP/AFI/GFI 20 AMP/AFI/GFI	1	12-2 W/G
9	FURNACE	NOŤE 2		•
10	DRYER	30 AMP		10-3 W/G
11	RANGE	40 AMP	2	8-3 W/G
11A	GAS RANGE	40 AMP 15 AMP/AFI/GFI 15 AMP/AFI	1	14-2 W/G
12**	GENERAL LIGHTING	15 AMP/ÁFI	1	14-2 W/G
13**	GENERAL LIGHTING	15 AMP/AFI	1	14-2 W/G
14	COOKTOP	40 AMP	2	8-3 W/G

CIR#	DESCRIPTION	BREAKER	POLES	WIRE
14Ä	GAS COOKTOP IGNITER	15 AMP/AFI	1	14-2 W/G
15	OVEN/MICROWAVE	30 AMP	2	10-3 W/G
16	DISHWASHER	NOTE 3/AFI/GFI		
17	OPT. WHIRLPOOL	20 AMP/AFI/GFI	1	14-2 W/G
18	OPT. EURO RANGE HOOD	20 AMP/AFI	1	12-2 W/G
19	MICROWAVE VENT HOOD	20 AMP/AFI/GF	1	12-2 W/G
20	BATH RECEPTACLES	20 AMP/GFI	1	12-2 W/G
	LAUNDRY CIRCUIT	20 AMP/AFI/GFI	1	12-2 W/G
	SMOKE DETECTORS	15 AMP/AFI		14-2 W/G
40**		20 AMP/AFI	1	12-2 W/G
41**		15 AMP/AFI	1	14-2 W/G
42**	GENERAL LIGHTING	15 AMP/AFI	1	14-2 W/G
43**	GENERAL LIGHTING	15 AMP/AFI	1	14-2 W/G
51**	ELECTRIC FIRE PLACE	15 AMP/AFI/GFI	1	14-2 W/G

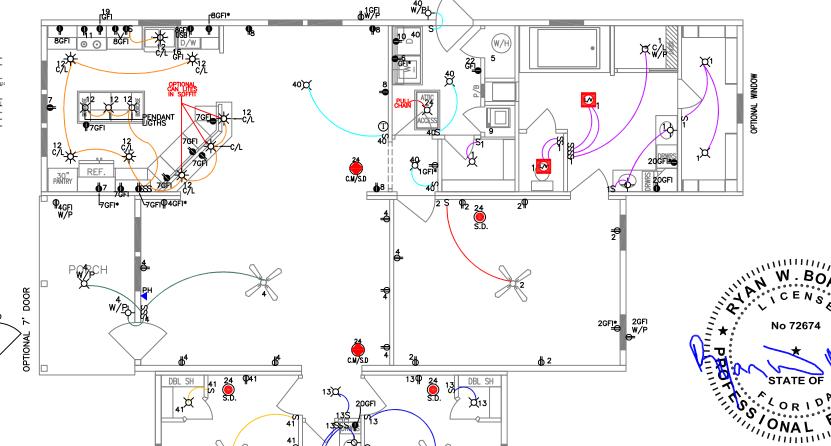
SYMBOLS:  S - SWITCH  → - 15 AMP RECEPTACLE  → - 20 AMP RECEPTACLE  → - 30 AMP RECEPTACLE  → - 40 AMP RECEPTACLE  LIGHT FIXTURE  LAVLIGHT&PORCH LIGHT  S.D.	→ LIGHT\VENT FAN  → C/L=CAN LIGHT  → TELEPHONE  +TV - TELEVISION  ① - THERMOSTAT  GFI* - MASTER GFI  P/B - PANEL BOX  W/P - WATERPROOF  C.M/S.D	- FLOOD LIGHT - 2' FLUORESCENT - 4' FLUORESCENT - OPT.CEILING FAN W/LIGHT COMBO SMOKE DETECTOR AND CARBON MONOXIDE DETECTOR ALARM



= 10000 WFIRST 10000 W @ 100% REMAINDER @ 40% (27252)(.4)= 10901 W FURNACE (HVAC) = 23000 W43901 W

CALCULATED LOAD FOR SERVICE SIZE 43901 WATTS / 240 VOLTS = 183 AMPERES 200 AMP SERVICE STANDARD





OPTIONAL ATRIUM DOOR IN PLACE OF (2) 3672 WINDOWS

OPTIONAL SGD IN PLACE OF (2) 3672 WINDOWS

ALL CEILING BOXES MUST ALL RECEPTACLES MUST BE LISTED AS ABLE TO **BUILT FOR THE** SUPPORT 50lbs. 2017 NEC!!

BE LISTED AS TAMPER **RESISTANT** 

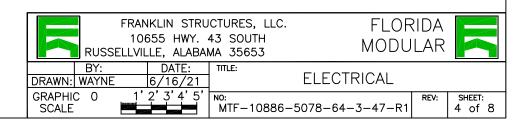
- \* ALL CIRCUITS MAY NOT BE USED, SEE APPROVAL DRAWINGS FOR SPECIFIC CIRCUITS
- \*\* SELECTION IS BASED ON APPLIANCE LOAD AND MANUFACTURER'S INSTALLATION INSTRUCTIONS.
- GENERAL LIGHTING CIRCUITS MAY BE WIRED WITH 12-2 W/G AND 20 AMP BREAKERS

\*1.a)20 AMP; 2 POLE; 12-2 w/G or b)25 AMP; 2 POLE; 10-2 w/G or c)30 AMP; 2 POLE; 10-2 w/G

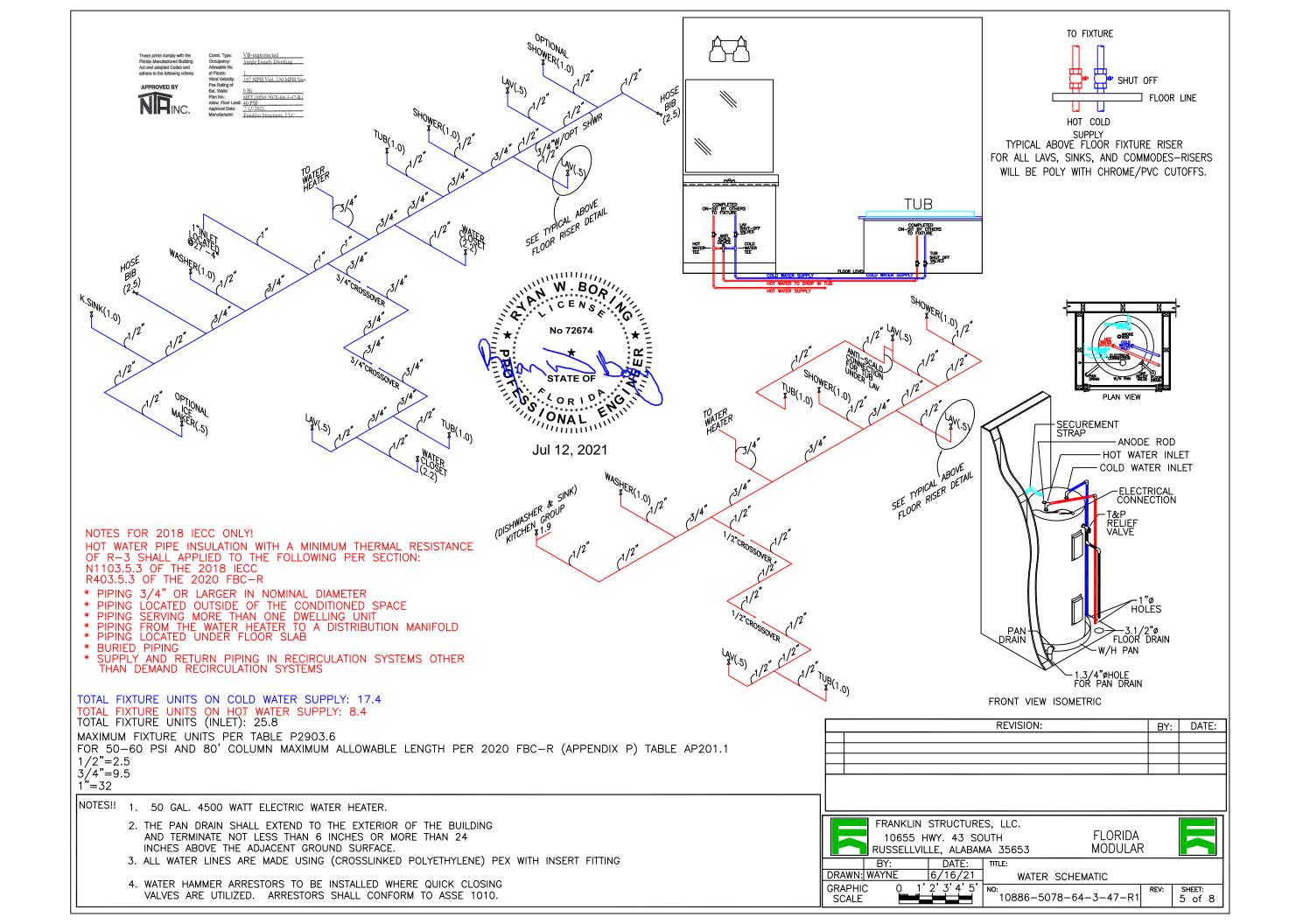
2.a)15KW: 60 AMP; 2 POLE; 4-4-6 and 30 AMP; 2 POLE; 10-2 W/G b)20KW; 60 AMP; 2 POLE; (2) 4-4-6 c)23KW; 60 AMP; 2 POLE; 6-6-8 and 50 AMP; 2 POLE; 6-6-8

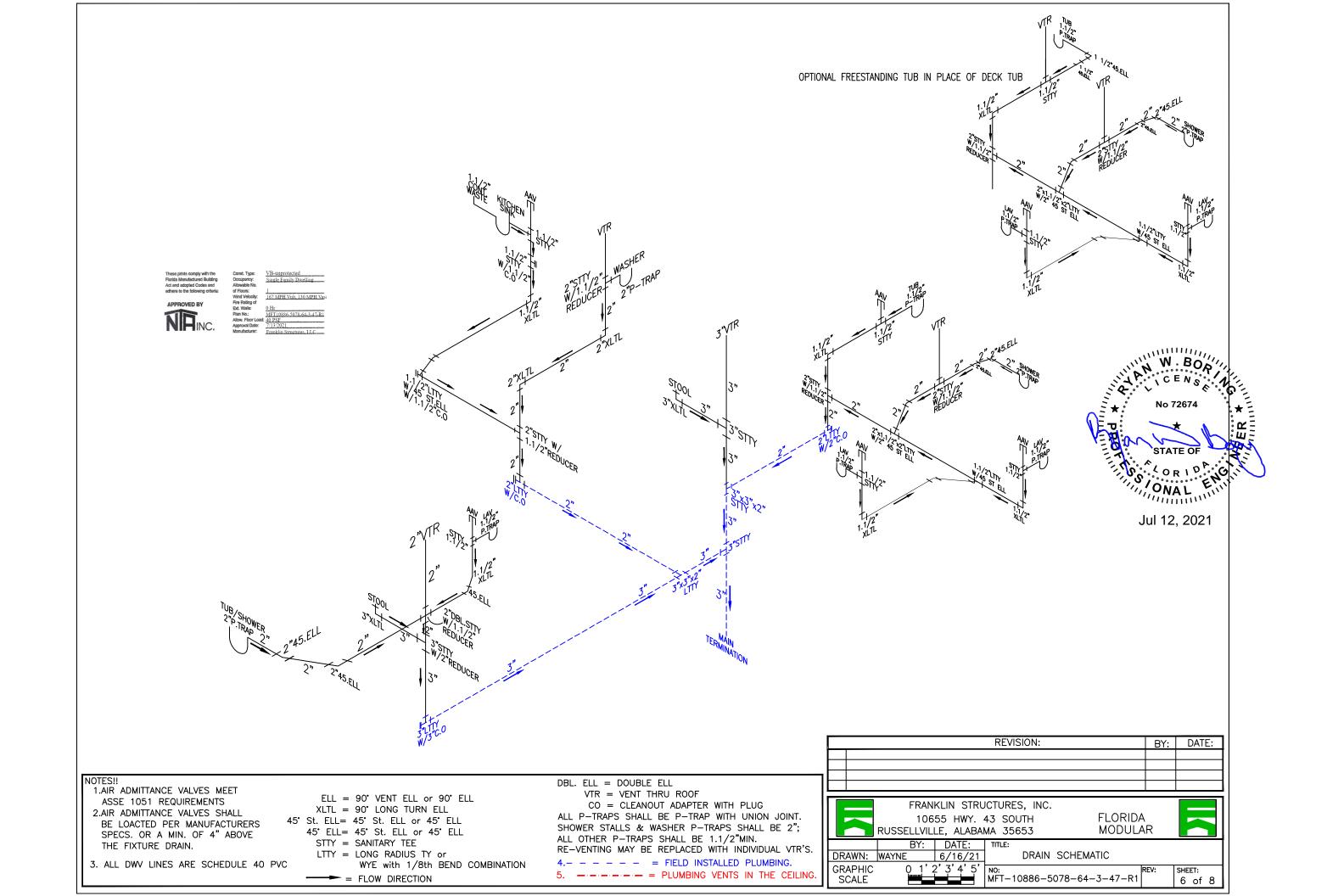
\*3.a)15 AMP; 1 POLE; 14-2 w/G or b)20 AMP; 1 POLE; 12-2 w/G \*4.a)15 AMP; GFI.; 1 POLE; 14-2 w/G or b)20 AMP; GFI.; 1 POLE; 12-2 w/G

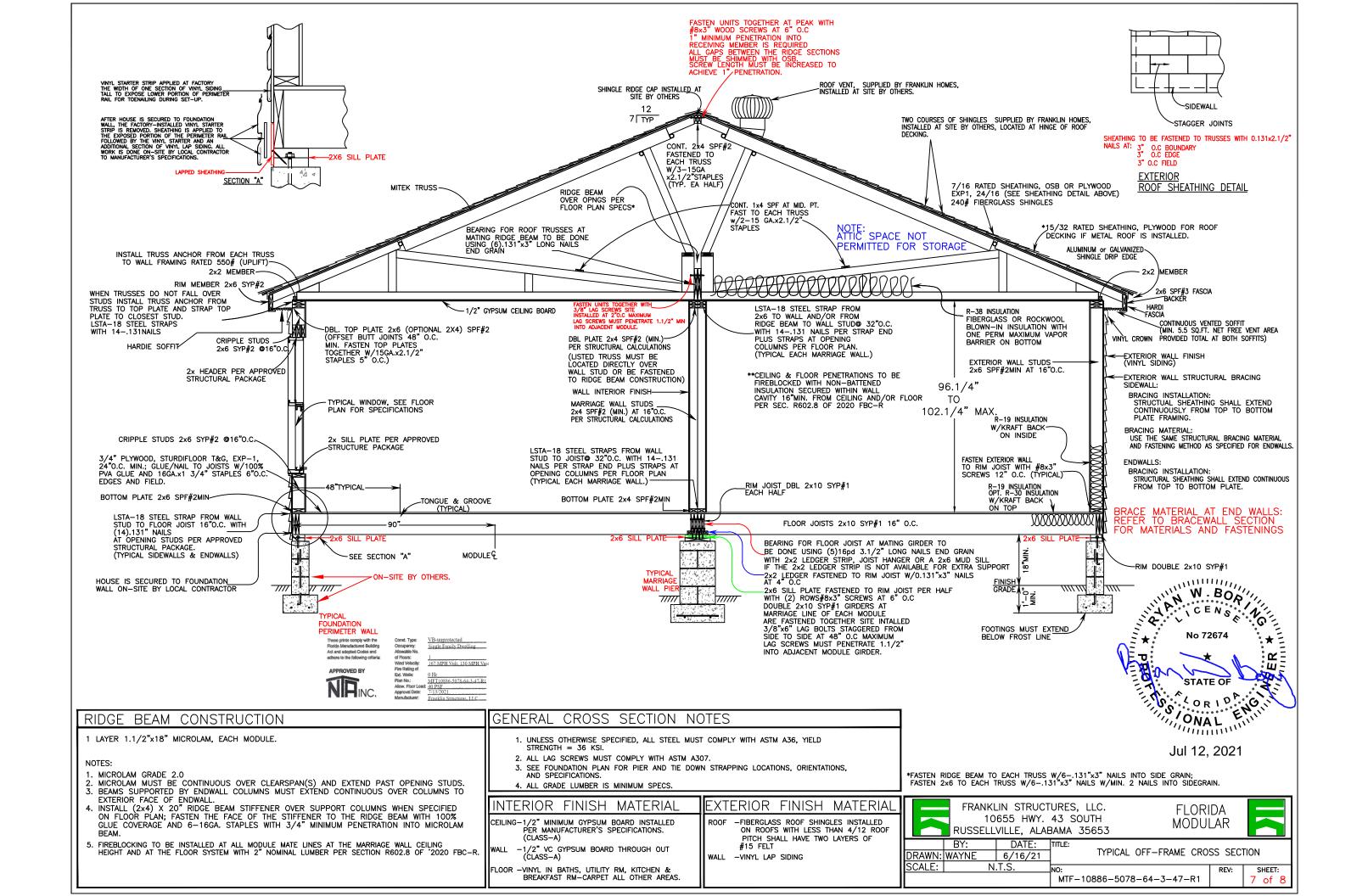
- 5. CIRCUIT NUMBERS SHOWN HERE ARE USED FOR IDENTIFICATION OF CIRCUITS SHOWN ON ELECTRICAL DIAGRAMS SUBMITTED FOR APPROVALS. CIRCUIT IDENTIFICATION IN THE DISTRIBUTION PANEL BOXES WILL BE ACCOMPLISHED BY DESCRIBING EACH CIRCUIT (EG. WATER HEATER, LIGHTING, ETC.). IT IS PREFERRED THAT CIRCUIT NUMBERS ON DISTRIBUTION PANEL MATCH THOSE SHOWN ON THIS CHART, BUT IT IS NOT A REQUIREMENT.
- 6. SERVICE ENTRANCE WIRE SIZE IS (3)-#2/0 WITH (1)-#4 COPPER GROUND.
- 7. ALL FAMILY ROOMS, DINING ROOMS, LIVING ROOMS, PARLORS, LIBRARIES, DENS, BEDROOMS, SUNROOMS, RECREATION ROOMS, CLOSET, HALLWAYS, KITCHEN, LAUNDRY OR SIMILAR AREAS SHALL BE PROTECTED BY A LISTED ARC-FAULT CIRCUIT INTERRUPTER DEVICE OF THE COMBINATION TYPE.

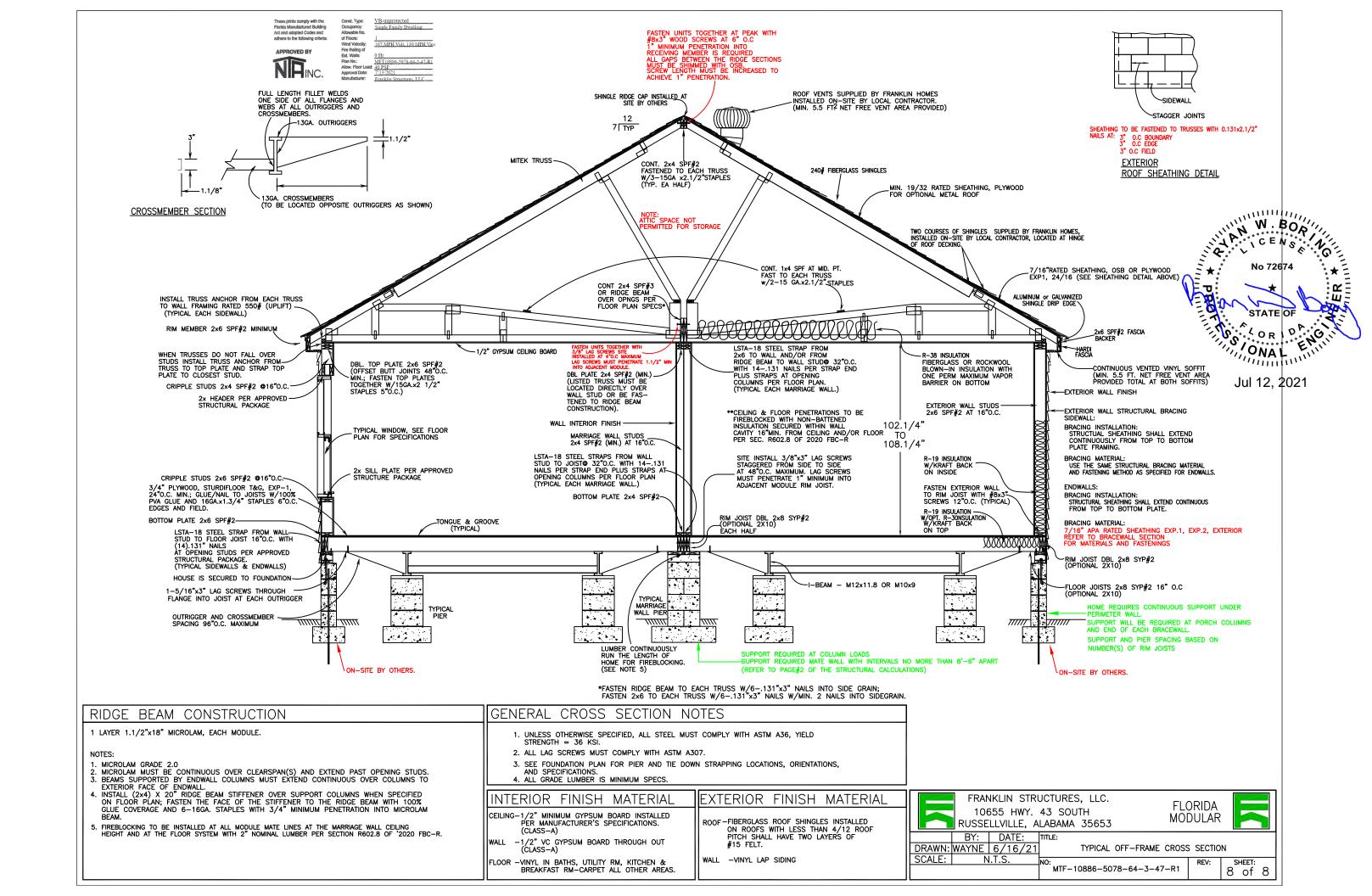


Jul 12, 2021









# **Franklin Structures, LLC**

MTF-5078-64-3-47

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:

APPROVED BY

Width: 30'1/2" with 15'1/4" Tag

Length: 60'

Roof Live Load: 20psf Roof LL

Wind Speeds: 167mph Vult

Wind Exposure: C

Wall Height: 9'

Max Mean Roof Height: 18'



Jul 12, 2021

Page	Description
1-5	Design Criteria and Load Cases (C&C page 1)
6	Matewall Headers
7	Matewall Columns
8	Sidewall Headers
9-10	Sidewall Columns (King & Jack)
11-12	Uplift Straps (Sidewall & Matewall)
13	Sill Plates and Lateral Only Sill/Header Connection
	(Sill must also be installed at top of window/door)
14-19	Shearwalls and Diaphragms
20-21	Connections
22	Floor Joist
23-24	Matewall Girders
25-26	Porch Design
27-30	Tag Header, Columns and Straps
31-34	On-Frame Floor Joists (w/ and w/out setback)



NOTE: These calculations are applicable only to the structural elements and loading criteria specifically noted herein. These calculations shall not be construed in any way to specify, certify, or design any aspects of the structure not contained herein. Structural elements not contained herein are to be constructed in accordance with the prescriptive requirements of the adopted building code or designed by other registered design professionals, as applicable. Specified design criteria are based solely on information provided by the client and must be verified and approved by the local authority having jurisdiction. Ryan W. Boring, P.E. is not responsible for fabrication or erection. If it is suspected that the calculations listed in this index have been modified, substituted, or altered in any way, contact Ryan W. Boring, P.E. directly to obtain a file copy.

# ASCE 7-10 (and 16) Chapter 30 Part I:

	Wind Speed:	Wind Speed: 167 MPH				e: Gable	e (Gable or Hip)			
	Wind Exposure:		С		Roof Pitch	ո։	7 /12	g 6/12		
	Mean Roof Height	::	18 FT		Roof Angle	e:	30.3			
	Elevation:		0 FT		Max Width	ո։	30.04 ft	Ma	ain truss wi	dth
	Ke:		1.00							
	Kd:		0.85							
	Kzt:		1							
	kt:		0.88							
	gh:		53.53 psf							
	Building Type:	Е	nclosed .							
	Gcpi:		0.18							
	·		-0.18							
	Min net pressure:		16 psf							
			•							
Roof										
GCP	Area	Pos	Neg	Z	Pressure	Area	Pos	Ne	g	
Zone 1		Min	0.9	-1.8	Zone 1		Min	57.8	-106.0	
		100	0.9	-0.8			100	57.8	-52.5	
Zone 2		Min	0.9	-2	Zone 2		Min	57.8	-116.7	
		100	0.9	-1.2			100	57.8	-73.9	
Zone 3		Min	0.9	-3.2	Zone 3		Min	57.8	-180.9	
		100	0.9	-1.5			100	57.8	-89.9	
OH Z1		Min		-2.6	OH Z1		Min		-148.8	
		100		-1.6			100		-95.3	
OH Z2		Min		-2.8	OH Z2		Min		-159.5	
		100		-2			100		-116.7	
OH Z3		Min		-4	OH Z3		Min		-223.8	
		100		-2.3			100		-132.8	
										We are a factor
Walls										These p Florida N Act and
Gcp	Area	Pos	Neg	g	Pressure	Area	Pos	Ne	g	adhere t
Zone 4		10	1	-1.1	Zone 4		10	63.2	-68.5	APPR
		100	0.825	-0.93			100	53.8	-59.2	M
Zone 5		10	1	-1.4	Zone 5		10	63.2	-84.6	14

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:



100

53.8

-68.5

orest. Types:
coupancy:

Single Famil

Floors:
Ind Velocity:
e Retain of
t. Walls:
OHF
an No:
WFT10886-5
ow, Floor Load:
40 PSF
proval Date:
Franklin Stri

Franklin Stri

Franklin Stri

# Design Pressures

Pressure	Area	Pos	Neg	
Zone 1		Min	34.7	-63.6
		100	34.7	-31.5
Zone 2		Min	34.7	-70.0
		100	34.7	-44.3
Zone 3		Min	34.7	-108.6
		100	34.7	-54.0
OH Z1		Min		-89.3
		100		-57.2
OH Z2		Min		-95.7
		100		-70.0
OH Z3		Min		-134.3
		100		-79.7
Zone 4		10	37.9	-41.1
		100	32.3	-35.5
Zone 5		10	37.9	-50.7
		100	32.3	-41.1

100

0.825

-1.1

Note: Min area provides the highest loads, 10 sq. ft could have a lower load

# MWFRS

Transvers	se											
	1	2	3	4	1e	2e	3e	4e				
+GCpi	20.34	1.61	-32.65	-29.44	27.30	4.82	-38.01	-35.33				
-Gcpi	39.61	20.88	-13.38	-10.17	46.57	2.00	-18.74	-16.06				
Max	39.61	20.00	-32.65	-29.44	46.57	4.82	-38.01	-35.33				
Longitudi	inal											
	1	2	3	4	5	6	1e	2e	3e	4e 5e	9 6	е
+GCpi	-33.72	-46.57	-29.44	-33.72	11.78	-25.16	-35.33	-66.91	-38.01	-35.33	23.02	-32.65
-Gcpi	-14.45	-27.30	-10.17	-14.45	31.05	-5.89	-16.06	-47.64	-18.74	-16.06	42.29	-13.38
Max	-33.72	-46.57	-29.44	-33.72	31.05	-25.16	-35.33	-66.91	-38.01	-35.33	42.29	-32.65
	Vertical						Horz					
	End		Int		Overhang		End		Int			
	WW	LW	WW	LW	End	Int	Roof	Wall	Roof	Wall		
Trans	4.82	-38.01	20.00	-32.65	-28.3717	-31.5836	42.83	62.63	34.26	49.78		
Long	-66.91	-38.01	-46.57	-29.44	-28.3717	-	-28.91	55.67	-17.13	36.94		

Design Loading

	Vertical H						Horz			
	End		Int		Overhang		End		Int	
	ww	LW	WW	LW	End	Int	Roof	Wall	Roof	Wall
Trans	2.89	-22.80	12.00	-19.59	-17.02	-18.95	25.70	37.58	20.56	29.87
Long	-40.15	-22.80	-27.94	-17.67	-65.84	-53.64	25.70	33.40	20.56	22.16

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:



onst. Type: VB-ui coupancy: Single llowable No. Filoors: 1 Infind Velocity: 167 M re Rating of xt. Walls: 0 Infino. Filoor Load: 40 PSI poproval Date: 7/13/2

VB-unprotected Single Family Dwelling 1 167 MPH Vult, 130 MPH Vas 0 Hr MFT10886-5078-64-3-47-R1 40 PSF 7/13/2021

## Design Criteria:

Total Length:	60 ft		
Total Width:	30.04 ft	Top chord DL:	10 psf
Unit Width:	15.02 ft	Bottom chord DL:	10 psf
Pitch:	7 /12	Bottom chord LL:	0 psf
Roof Angle:	30.3 deg	Stories:	1
Wall height:	9 ft	Floor Live Load:	40 psf
Overhang:	12 in	Floor Dead Load:	10 psf
Blocking Height:	36 in	Wall Dead Load:	5 psf
Eave Height:	10 ft	Ceiling R value:	30
Min Mean Roof ht:	18 ft	Framing Rafters?: N	
Mean Roof Height:	19.26 ft	Truss Spacing:	24 in oc

Snow Loading:

Ground Snow Load: 0 psf Snow Thermal factor: 1.1 Snow exposure factor: 1 Snow importance Factor: 1 0 psf Flat Roof Snow, Pf: Sloped Roof Snow Ps: 0 psf Unbalanced Roof Load: 0.00 psf Minimum Roof Lr: 17 psf

Wind Loading:

	WW	LW	WWOH	
Transverse End:	2.89	-22.80		-17.02
Interior:	12.00	-19.59		-18.95
Long End:	-40.15	-22.80		-65.84
Interior:	-27.94	-17.67		-53.64

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:



Const. Type: Occupancy: Allowable No. of Floors: Wind Velocity: Fire Rating of Ext. Walls: Plan No.: Allow. Floor Loa Approval Date: Manufacturer: VB-unprotected Single Family Dwelling 1 167 MPH Vult, 130 MPH Vasc 0 Hr MFT10886-5078-64-3-47-R1 40 PSF 7/13/2021

Vult:

Vasd:

End zone, 2a: 6.0083333 ft

Exposure: C Internal Press:

167 mph

129 mph

0.18

### Snow Loading

Design Parameters:

Eave to ridge Distance, W: 16 ft Ground Snow Load, pg: 0 psf Exposure Factor, C<sub>e</sub> 1.0 Thermal Factor, C<sub>+</sub>. 1.1 Importance Factor, I: 1.0 Framing Type: Trusses/Other

Snow Density (g):

density: 14 pcf

Flat-Roof Snow Load (p<sub>f</sub>):

Pf: 0.0 psf

Rain on Snow Surcharge:

amax: 0.3204167 deg prss: 0.0 psf pf: 0.0 psf

Minimum Values for Low-Slope Roofs:

Mono: 15.0 deg 4.9 deg amin: 2.38 deg 15.0 deg amin:

pf, pg<=20: 0.0 psf 20.0 psf pf, pg>20: pfmin: 0.0 psf 0.0 psf pf:

Sloped Roof Snow Loads:

degrees: 30.3 Cs Ct=1: 0.99 Ct=1.1: 1.00 Ct=1.2: 1.00 0.0 Ps:

Unbalanced Snow Loads:

amax: 70.00 deg 4.87 deg amin: amin: 2.38 deg minimum: 2.38 deg S: 1.71 /1 W: 20.0 ft hd: 0.58 ft Pww: 0.0 Plw Ridge: 0.0 Plw Length: 0.0

Plw Eave:

Ice Dams Along Eves<sup>2</sup>:

0.0 psf ps:

0.0

Minimum Roof Live Load:

R1: 1.0 R2: 0.85 F: 7.0 Lr: 17.0 psf These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:



VB-unprotected Single Family Dwelling

### **Truss Reactions**

Floor Load only

Max down

Lateral

269

20.96

Gravity Matewall: 552 lbs Uplift: Matewall: 697 lbs 671 lbs Sidewall: 494 lbs Sidewall: Truss Spacing: 24 in oc. Snow Load: 0 psf

Wost case truss reactions

Load Cases fo	or Ranch		Roof			Roof and 1 Stor	у
l	Load Case	Sidewall	Matewall	Endwall	Sidewall	Matewall	Endwall
1	D	160	150	30	280	270	85
2	S	0	0	0	0	0	0
3	Su	0	0	0	0	0	0
4	Lr	145	128	34	145	128	34
5	L	0	0	0	300	300	40
6	Wp	5	22	6	5	22	6
7	Wn	-343	-439	-106	-343	-439	-106
8	.75(L+Lr)	109	96	26	334	321	56
9	.75(L+S)	0	0	0	225	225	30
10	.75(L+Su)	0	0	0	225	225	30
11	.75(L+S+Wp)	4	16	4	229	242	34
12	D+L	160	150	30	581	571	125
13	D+Lr	305	278	64	425	398	119
14	D+S	160	150	30	280	270	85
15	D+Su	160	150	30	280	270	85
16	D+.75(L+Lr)	269	246	56	614	591	141
17	D+.75(L+S)	160	150	30	506	496	115
18	D+.75(L+Su)	160	150	30	506	496	115
19	D+.75(L+S+Wp)	164	166	34	509	512	119
20	.6D+Wn	-247	-349	-88	-175	-276	-55
_							
	Dead Load:	160	150	30	280	270	85
	Dead LC:	D	D	D	D	D	D
	Live Load:	145	128	34	334	321	56
	Live LC:	Lr	Lr	Lr	.75(L+Lr)	.75(L+Lr)	.75(L+Lr)
	Total Load:	305	278	64	614	591	141
	Total LC:	D+Lr	D+Lr	D+Lr	D+.75(L+Lr)	D+.75(L+Lr)	D+.75(L+Lr)
	Uplift Load:	-247	-349	-88	-175	-276	-55
	Uplift LC:	.6D+Wn	.6D+Wn	.6D+Wn	.6D+Wn	.6D+Wn	.6D+Wn
	Design Load:	336	278	64	581	571	125
	Design LC:	D+Lr	D+Lr	D+Lr	D+L	D+L	D+L
	Design LDF:	1.25	1.25	1.25	1	1	1
_		·		·			

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:

LDF 0.9 1.15 1.15 1.25 1 1.6 1.6 1.25 1.15 1.15 1.6 1 1.25 1.15 1.15 1.25 1.15 1.15 1.6 1.6



1

VB-unprotected Single Family Dwelling

Live			300	300		1
Dead			75	75		0.9
160	150	30	280	270	85 plf	
165	172	36	285	292	91 plf	
165	172	36	285	292	91 plf	
27.94	5	27.94	27.94	5	27.94 psf	
269	246	56	614	591	141 plf	
160	150	30	506	496	115 plf	
160	150	30	506	496	115 plf	
164	166	34	509	512	119 plf	
	160 165 165 27.94 269 160 160	Dead  160 150 165 172 165 172 27.94 5  269 246 160 150 160 150	Dead  160 150 30 165 172 36 165 172 36 27.94 5 27.94  269 246 56 160 150 30 160 150 30	Dead 75  160 150 30 280 165 172 36 285 165 172 36 285 27.94 5 27.94 27.94  269 246 56 614 160 150 30 506 160 150 30 506	Dead     75     75       160     150     30     280     270       165     172     36     285     292       165     172     36     285     292       27.94     5     27.94     27.94     5       269     246     56     614     591       160     150     30     506     496       160     150     30     506     496	Dead         75         75           160         150         30         280         270         85 plf           165         172         36         285         292         91 plf           165         172         36         285         292         91 plf           27.94         5         27.94         27.94         5         27.94 psf           269         246         56         614         591         141 plf           160         150         30         506         496         115 plf           160         150         30         506         496         115 plf

56

20.96

614

20.96

591

3.75

141 plf

20.96 psf

246

3.75

Matewall Headers Supporting Roof

5 6

7

9

10

1

1

1

1.5

1.5

1.5

Location: Matewall Supporting: Roof

<u>V</u>	ertical I	Load												,	Wall height:	108 i	n
Dead	Load:	150	plf	D										Mii	n sill height:	18 i	n
Live	Load:	128	plf	Lr											LVL:	Microllam	
Total	Load:	278	plf	D+Lr										Ľ	VL MOE (E):	2000000	osi
Uplift	Load:	-349	plf	.6D+Wn											E min:	1016411	osi
															Fb:	2750	osi
															Fv:	285	osi
	Cr: 1	1.15	LL defl L	/ 240											Fcperp:	750	osi
	Cd: 1	1.25	TL defl L	/ 180										Volume ef	fect exp (e):	0.136	
															Cr (LVL):	1.04	
Vertical																	
	Qty.	В	D	Species	Grade Direction	Cfu	Cfb	Fb	Fv	Fcperp	Ε	Emin	Fb'	Fv'	Α	S	1
1	1	1.5	7.25	LVL	Edge	1	1.1	2750	285	750	2000000	1016411	3681	285	10.9	13.1	47.6
2	1	1.5	9.25	LVL	Edge	1	1.0	2750	285	750	2000000	1016411	3561	285	13.9	21.4	98.9
3	1	1.5	11.25	LVL	Edge	1	1.0	2750	285	750	2000000	1016411	3468	285	16.9	31.6	178.0
4	1	1.5	14	LVL	Edge	1	1.0	2750	285	750	2000000	1016411	3366	285	21.0	49.0	343.0

285

175

175

175

#N/A

#N/A

750

565

565

565

#N/A

#N/A

2000000 1016411

1400000 510000

1400000 510000

1400000 510000

#N/A

#N/A

#N/A

#N/A

3306

1156

1156

938

#N/A

#N/A

285

219

219

219

#N/A

#N/A

24.0

10.9

13.9

16.9

0.0

0.0

64.0

13.1

21.4

31.6

0.0

0.0

512.0

47.6

98.9

178.0

0.0

0.0

	Shear	Moment	LL def	TL def
1	193	129	102	134
2	246	162	130	171
3	299	195	158	208
4	373	239	196	259
5	426	270	224	296
6	151	72	90	119
7	193	92	115	152
8	235	101	140	185
9	#N/A	#N/A	#N/A	#N/A
10	#N/A	#N/A	#N/A	#N/A

16

7.25

9.25

11.25

LVL

SYP

SYP

SYP

#2

#2

#2

Edge

Edge

Edge

Edge

1

1

1

1

#N/A

#N/A

1.0

1.0

1.0

1.0

#N/A

#N/A

2750

925

925

750

#N/A

#N/A

		Reactio	ns (lbs)	Bearing
Member	Max Span	Gravity	Uplift	(in)
1 (1) 2x 7.25 LVL	101 in	1200	-1470	1.1
2 (1) 2x 9.25 LVL	129 in	1500	-1880	1.1
3 (1) 2x 11.25 LVL	157 in	1900	-2280	1.1
4 (1) 2x 14 LVL	196 in	2300	-2850	1.1
5 (1) 2x 16 LVL	224 in	2600	-3260	1.1
6 (1) 2x 8 SYP #2	72 in	900	-1050	1.5
7 (1) 2x 10 SYP #2	92 in	1100	-1340	1.5
8 (1) 2x 12 SYP #2	101 in	1200	-1470	1.5
9 () 0x 0	#N/A in	#N/A	#N/A	#N/A
10 () 0x 0	#N/A in	#N/A	#N/A	#N/A

Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:

APPROVED BY

Const. Type: Occupancy: Allowable No, of Floors: Wind Velocity: Fire Rating of Ext. Walls: Plan No.: Allow. Floor Load: Approval Date:

v D-unprotected
Single Family Dwelling
1
167 MPH Vult, 130 MPH Vasc
0 Hr
MFT10886-5078-64-3-47-R1
40 PSF
7/13/2021

Location: Matewall Supporting: Roof

Mate	wall	Columns	Supporting	Roof

Side Column																
Vertical L	.oad				Lateral Loa	d				Combined	Vert and La	it (max La	it)	Wa	II height:	108 in
NDS Load:	285	plf	D+Lr		Lateral only	/				Vertical:	150	plf	0	Top/Btm P	late (tp):	4.5 in
Total Load:	285	plf	D+Lr	9	Stud area:	27.0 f	t^2			Lateral:	5	psf	W		LVL:	Microllam
Uplift Load:	-326	plf	.6D+Wn		Lateral:	5 p	sf	W		Combined	Vert and La	t (max Ve	ert)	LVL	MOE (E):	2000000 psi
										Vertical:	285	plf	0		E min:	1016411 psi
										Lateral:	3.8	psf	.75W		Fb:	2750 psi
					def=.7	C&C				Combined	Uplift and L	.at			Fv:	285 psi
Cr:	1									Vertical:	-326	plf	.6D+Wn		Fcperp:	750 psi
Cd:	1.6									Lateral:	3.8	psf	W	V	ol eff (e):	0.136
Cd grav:	1.25			ateral de	flection L/ 1	120									Cr (LVL):	1.04
Vertical															. ,	
Spacing	В	D	Species	Grade	С	Ie/D	Cfb	CfC	Fb	Fc (grav)	Fc (comb)	Fcperp	Ε	Emin	FcE	Ft
1 16	1.5	3.5	SPF	#3	0.8	29.6	1.5	1.15	500	650	650	425	1200000	440000	414	250
									Ср	0.39	0.32					
Header	1.5	inches					Al	owable:		366	379	425	1200000	440000		600
# of Studs			1	2	3	4	5	6	7	8						
<u>Properties</u>																
Area in^2			5.3	10.5	15.8	21.0	26.3	31.5	36.8	42.0						
Sx in^3			3.1	6.1	9.2	12.3	15.3	18.4	21.4	24.5						
lx in^4			5.4	10.7	16.1	21.4	26.8	32.2	37.5	42.9						
Axial Loading																
Fc compression	1		81	162	243	323	404	485	566	647						
Fc Perp compre	ession		40	80	121	161	201	241	281	322						
Tension			116	232	348	464	580	696	812	928						
Fcperp																
Combined Load	ding															
Uplift/Lateral			116	232	348	464	580	696	812	928						
Vert/Lateral ma			137	295	453	612	778	936	1095	1254						
Vert/Lateral Ma	ax Vert		75	158	242	325	379	460	542	624						
Deflection Chec	<u>ck</u>	L/			2406	3208	4009	4811	5613	6415						
			OK	OK	OK	OK	OK	OK	OK	OK						
			40	80	121	161	201	241	281	322						
Max Span			(Trib				Max Side C									
1			) in			n =	5	ft -		in						
2			) in		141 i		11	ft -		in						
3		121			220 i		18	ft -		in						
4		161			299 i		24	ft -	11							
5		201			378 i		31	ft -		in						
6		241			457 i		38	ft -		in						
7		201	Lin		536 i	n –	44	£L.	7	:	1					1
8		322			615 i		51	ft - ft -		in in						

Notes: Center column is total span on both sides of column. Side column is total clear span All studs are to be braced in weak axis by gypsum or sheathing.

Center column must be in center 1/3 of span.

Studs must be as wide as header.

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:



Const. Type: Occupancy: Allowable No. of Floors: Wind Velocity: Fire Rating of Ext. Walls: Plan No.: Allow. Floor Load: Approval Date:

	VB-unprotected Single Family Dwelling
	1 167 MPH Vult, 130 MPH Va
	0 Hr MFT10886-5078-64-3-47-R1
d;	40 PSF 7/13/2021 Franklin Structures, LLC

Location: Sidewall Supporting: Roof

Sidewall Headers Supporting Roof

U-Headers

For Lateral Loading See Sill Plate, This calculation is only for vertical load and a sill plate must be used at the top of the opening

#N/A

#N/A

<u>verticar L</u>	<u>uau</u>	
Dead Load:	160 plf	D
Live Load:	145 plf	Lr
Total Load:	336 plf	D+Lr
Uplift Load:	-247 plf	.6D+Wn

Wall height: 108 in Min sill height: 18 in LVL: Microllam LVL MOE (E): 2000000 psi E min: 1016411 psi Fb: 2750 psi Fv: 285 psi

Cr: 1.15 LL defl L/ 240 Cd: 1.25 TL defl L/ 180

Fcperp:	750 ps
Volume effect exp (e):	0.136
Cr (LVL):	1.04

0.0

0.0

S

3.1

6.1

15.1

26.3

3.1

6.1

15.1

26.3

0.0

0.0

1

3.9

10.7

41.6

95.3

3.9

10.7

41.6

95.3

0.0

0.0

																	CI (LV
١	ertic/	al															
		Qty.	В	D	Species	Grade	Direction	Cfu	Cfb	Fb	Fv	Fcperp	Е	Emin	Fb'	Fv'	Α
	1	2	1.5	2.5	SPF	#2	Edge	1	1.5	875	135	425	1400000	510000	1641	169	7.5
	2	2	1.5	3.5	SPF	#2	Edge	1	1.5	875	135	425	1400000	510000	1641	169	10.5
	3	2	1.5	5.5	SPF	#2	Edge	1	1.3	875	135	425	1400000	510000	1422	169	16.5
	4	2	1.5	7.25	SPF	#2	Edge	1	1.2	875	135	425	1400000	510000	1313	169	21.8
	5	2	1.5	2.5	SYP	#2	Edge	1	1.0	1100	175	565	1400000	510000	1375	219	7.5
	6	2	1.5	3.5	SYP	#2	Edge	1	1.0	1100	175	565	1400000	510000	1375	219	10.5
	7	2	1.5	5.5	SYP	#2	Edge	1	1.0	1100	175	565	1400000	510000	1375	219	16.5
	8	2	1.5	7.25	SYP	#2	Edge	1	1.0	925	175	565	1400000	510000	1156	219	21.8

#N/A

	Shear	Moment	LL def	TL def
1	65	38	44	50
2	92	54	62	70
1 2 3 4	144	78	97	110
4	190	99	128	145
5	83	35	44	50
6	117	49	62	70
7	183	77	97	110
	241	93	128	145
9	#N/A	#N/A	#N/A	#N/A
10	#N/A	#N/A	#N/A	#N/A

9

Member 1 (2) 2x 3 SPF #2 2 (2) 2x 4 SPF #2 3 (2) 2x 6 SPF #2 4 (2) 2x 8 SPF #2 5 (2) 2x 3 SYP #2 6 (2) 2x 4 SYP #2 7 (2) 2x 6 SYP #2 8 (2) 2x 8 SYP #2 9 () 0x 0 10 () 0x 0

10

	Sileai	Monient	LL UEI	IL uei
1	65	38	44	50
2	92	54	62	70
3	144	78	97	110
4	190	99	128	145
5	83	35	44	50
6	117	49	62	70
7	183	77	97	110
8	241	93	128	145
9	#N/A	#N/A	#N/A	#N/A
10	#N/A	#N/A	#N/A	#N/A

	Reaction	ns (lbs)	Bearing
Max Span	Gravity	Uplift	(in)
38 in	600	-400	0.5
53 in	800	-550	0.5
78 in	1100	-810	0.5
99 in	1400	-1020	0.5
35 in	500	-370	0.4
49 in	700	-510	0.4
77 in	1100	-800	0.4
93 in	1400	-960	0.4
#N/A in	#N/A	#N/A	#N/A
#N/A in	#N/A	#N/A	#N/A

					# can be based off span or header
					# can be based on span of neader
.131	x3"		# n	ails	# .131x3" nails per header
Grav	69.1	lb	Grav	Uplift	
uplift	88.4	lb	4.34	2.26	5
			5.79	3.11	6
			7.96	4.58	8
			10.13	5.77	11
			3.62	2.09	4
			5.07	2.88	6
			7.96	4.52	8
			10.13	5.43	11



0 Hr MFT10886-5078-64-3-47-R1 40 PSF 7/13/2021

Location: Sidewall Supporting: Roof

Sidewall Studs (King)

**Lateral Load** Combined Vert and Lat (max Lat) Wall height: 108 in Vertical Load 0 Top/Btm Plate (tp): NDS Load: 336 plf D+Lr 4.5 in Lateral only Vertical: 165 plf Total Load: 336 plf D+Lr Stud area: 27.0 ft^2 Lateral: 38 psf W Header height: 80 in Uplift Load: -247 plf .6D+Wn Zone 5: 46 psf W Combined Vert and Lat (max Vert) LVL: Microllam LVL MOE (E): 2000000 psi Zone 4: 39 psf W Vertical: 269 plf 0 Lateral: 28.2 psf E min: 1016411 psi .75W def=.7 C&C Combined Uplift and Lat Fb: 2750 psi 285 psi Cr: 1 Vertical: -247 plf .6D+Wn Fv: Cd: 1.6 Lateral: 28.2 psf Fcperp: 750 psi 1.25 ateral deflection L/ 120 Vol eff (e): 0.136 Cd grav: Vertical Cr (LVL): 1.04 Trib В D le/D Cfb CfC Fb Fc (grav) Fc (comb) Fcperp FcE Ft Species Grade Emin 0.8 20.7 1.0 1.00 1000 1400 1400 565 1400000 510000 978 600 175.0 1 16 1.5 5 SYP #2 Ср 0.47 0.39 Allowable: 1600 829 868 565 1400000 510000 960 Single King Stud Lateral Only Opening: 16 in Vertical Only Combined Max Lat Combind Max Vert Fb: 1092 psi 60 psi CSI: Max CSI: 0.70 ОК Fc: CSI: 0.70 0.54

Check Single stud for a max given max opening

Then check jacks and kings seperately and take controlling number, say calc 1 king for wind and it spans X distance and it takes 3 jacks to span that give results for different combinations

			Kin	g Studs En	d Zone			Kir	ng Studs In	t Zone				
		1	2	3	4	5	1	2	3	4	5			
<u>Properties</u>														
Area in^2		7.5	15.0	22.5	30.0	37.5	7.5	15.0	22.5	30.0	37.5			
Sx in^3		6.3	12.5	18.8	25.0	31.3	6.3	12.5	18.8	25.0	31.3			
lx in^4		15.6	31.3	46.9	62.5	78.1	15.6	31.3	46.9	62.5	78.1			
Lateral Only (trib)		1	2	3	4	5	1	2	3	4	5			
Moment at Center		23	47	70	94	117	28	56	83	111	139			
Moment at Header		31	63	94	126	157	37	74	112	149	186			
Shear		53	106	159	212	265	63	126	189	252	315			
Combined Loading														
Max Lat		308	802	1281	1757	2233	308	802	1281	1757	2233			
Max Vert		217	515	808	1100	1391	217	515	808	1100	1391			
Deflection Check	L/	203	203	203	203	203	171	171	171	171	171			
		OK	OK	OK	OK	OK	OK	OK	OK	ОК	OK			
Max Span		23	47	70	94	117	28	56	83	111	139			
Max Span						End Zo	ne					Interior	Zone	
1				30	in =	2	ft -	6	in	39	in =	3	ft -	3 in
2				77	in =	6	ft -	5	in	95	in =	7	ft -	11 in
3				124	in =	10	ft -	4	in	150	in =	12	ft -	6 in
4				171	in =	14	ft -	3	in	206	in =	17	ft -	2 in
5				218	in =	18	ft -	2	in	262	in =	21	ft -	9 in

All studs are to be braced in weak axis by gypsum or sheathing.

Center column must be in center 1/3 of span.

Studs must be as wide as header.

NOTE: RIPPED LUMBER MUST BE REGRADED!

Florida Manufactured Building Act and adopted Codes and adhere to the following criteria



0 Hr MFT10886-5078-64-3-47-R k 40 PSF 7/13/2021

Location: Sidewall Supporting: Roof

Sidewall Studs (Jack)

<u>Vertical I</u> NDS Load: Total Load: Uplift Load:	336 p 336 p 336 p -247 p	lf	D+Lr D+Lr .6D+Wn											Top/Btm F	LVL: MOE (E):	108 in 4.5 in Microllam 2000000 psi 1016411 psi 2750 psi 285 psi
Cr:	1														Fcperp:	750 psi
Cd:	1.6													V	ol eff (e):	0.136
Cd grav:	1.25			ateral def	lection L/	120									Cr (LVL):	1.04
Vertical																
Spacing	В	D	Species	Grade	С	Ie/D	Cfb	CfC	Fb		Fc (comb)		E	Emin	FcE	Ft
1 16	1.5	5	SYP	#2	0.8	20.7	1.0	1.00	1000	1400	1400	565	1400000	510000	978	600
									Ср	0.47	0.39					
								Allowable:	1600	829	868	565	1400000	510000		960
Header	width:	1.5	in :													
		3 4.5	in in													
		4.5	111													
# of Studs Properties			1	2	3	4	5	6	7	8	9	10				
Area in^2			7.5	15.0	22.5	30.0	37.5	45.0	52.5	60.0	67.5	75.0				
Sx in^3			6.3	12.5	18.8	25.0	31.3	37.5	43.8	50.0	56.3	62.5				
lx in^4			15.6	31.3	46.9	62.5	78.1	93.8	109.4	125.0	140.6	156.3				
Axial Loading			222	445	667	200	4440	1001	4553	4770	2002	222.				
Fc compression			222	445	667	890	1112	1334	1557	1779	2002	2224				
Fc Perp compr			45	91	136	182	227	273	318	364	409	455				
Fc Perp compr			91	182	273	364	455	546	637	728	818	909				
Fc Perp compre	ession 4.5	ın	136	273	409	546	682	818	955	1091	1228	1364				
Tension			350	700	1049	1399	1749	2099	2449	2798	3148	3498				

Trib taken by King stud: 0 in Increase of span: 0

Max Span

1 2 3

			Triple Head	
14 ft -	7 in	266 in =	22 f	t - 2 in
29 ft -	3 in	533 in =	44 1	t - 4 in
43 ft -	11 in	800 in =	66 f	t - 8 in
	29 ft -	29 ft - 3 in	29 ft - 3 in 533 in =	29 ft - 3 in 533 in = 44 f

NOTE: RIPPED LUMBER MUST BE REGRADED!

Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:



Const. Type: Occupancy: Allowable No. of Floors: Wind Velocity: Fire Rating of Ext. Walls: Plan No.: Allow. Floor Load Approval Date:

VB-unprotected Single Family Dwelling

1 167 MPH Vult. 130 MPH Vas

0 Hr
MFT10886-5078-64-3-47-R1
ad: 40 PSF
7/13/2021
Franklin Structures LLC Uplift Straps: Matewall

Uplift: -349 plf

Stud Spacing: 16 in

**Strapping** 

Strap All: 921.3 lbs LSTA18 (.83 reduction for .131)

# fasteners: 14 .131x 2.5" (.83 REDUCTION)

Strap Spacing: 2.64 ft (MAX SPACING USE 32" OC)

At openings

Span (in)

# of straps	Side opening	center opening
1	46	62
2	110	126
3	174	190
4	236	252
5	300	316
6	364	380
7	428	444
8	490	506
		total span

If sheathing is being used for uplift NOT at openings:

Max OSB per E510A: 1500 plf

.131 Nail: 108 lb 15ga: 82 lb

Spacing from OSB to rail:

.131 Nail: 3.72 " oc 15ga: 2.82 " oc

# fasteners into Studs:

.131 Nail: 4.3 lb Use 3 15ga: 5.7 lb Use 4 These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria



t. Type: VB-unprotected Single Family Dw able No. ors: 1 167 MPH Vult. 13 telling of

0 Hr MFT10886-5078-64-3-47-R1 4 40 PSF 7/13/2021 Uplift Straps: Sidewall

Uplift: -247 plf Estimated

Stud Spacing: 16 in

**Strapping** 

Strap All: 921.3 lbs LSTA18 (.83 reduction for .131)

# fasteners: 14 .131x 2.5" (.83 REDUCTION)

Strap Spacing: 3.73 ft (MAX SPACING USE 32" OC)

At openings

Span (in)

# of straps	Side opening	center opening
1	72	88
2	162	178
3	252	268
4	342	358
5	430	446
6	520	536
7	610	626
8	700	716
		1.1.1

total span

If sheathing is being used for uplift NOT at openings:

Max OSB per E510A: 1500 plf

.131 Nail: 108 lb 15ga: 82 lb

Spacing from OSB to rail:

.131 Nail: 5.25 " oc 15ga: 3.98 " oc

# fasteners into Studs:

.131 Nail: 3.0 lb Use 2 15ga: 4.0 lb Use 3 These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria



st. Type: VB-unprotected
upanoy: Single Family Dwellin
vable No.
oors: 1
167 MPH Vult, 130 M

0 Hr MFT10886-5078-64-3-47-R1 40 PSF Sill Plates

8

3

4

5

6

Wall height: 108 in Lateral Load Lateral only Min sill height: 18 in

192 plf (C&C) LVL: Microllam Wind: def=.7 C&C LVL MOE (E): 2000000 psi E min: 1016411 psi

Fb: 2750 psi Fv: 285 psi Cr: 1.15 Fcperp: 750 psi Volume effect exp (e): Cd: 1.6 Lateral deflection L/ 120 0.136 Cr (LVL): 1.04

Vertical Qty. В D Species Grade Direction Cfu Cfb Fb Fν Ε Fb' Fv' Fcperp Emin Α S SPF 1400000 510000 7.5 1 1 1.5 5 #2 Edge 1 1.4 875 135 425 1960 216 6.3 15.6 2 SPF Edge 1.4 875 135 425 1400000 510000 1960 216 15.0 12.5 31.3 3 5 SPF #2 875 135 425 1400000 510000 2254 216 22.5 18.8 46.9 3 1.5 Edge 1 1.4 4 SYP #2 1000 175 1400000 510000 1600 280 7.5 6.3 15.6 1 1.5 5 Edge 1 1.0 565 5 2 1.5 SYP #2 Edge 1 1.0 1000 175 565 1400000 510000 1600 280 15.0 12.5 31.3 6 1.5 SYP Edge 1.0 1000 175 565 1400000 510000 1840 280 22.5 18.8 46.9 7

9 10 lu Fbe 1.0 133689 1 2 1 2 1.0

Cl Shear Moment Def 1.00 145 78 96 2.1 133689 1.00 280 111 120 1.0 2.1 133689 1.00 414 145 138 1.0 133689 1.00 185 71 96 2.1 133689 1.0 2.1 1.00 359 100 120 1.0 133689 1.00 534 131

Sill and header lateral connection:

Reactions (lbs) Member Max Span Gravity 1 (1) 2x 5 SPF #2 78 in 700 2 (2) 2x 5 SPF #2 110 in 900 3 (3) 2x 5 SPF #2 137 in 1100 4 (1) 2x 5 SYP #2 70 in 600 5 (2) 2x 5 SYP #2 99 in 800 6 (3) 2x 5 SYP #2 131 in 1100 8 9 10

Span	Load @	Nail	0.131	15ga	15gax2.5'
(ft)	Ends (lbs)	Zeg(lb)	Nails	Zeg(lb)	Staples
2	192.30	88	3	48	5
3	288.45		4		7
4	384.60		5		9
5	480.75		6		11
6	576.90		7		13
7	673.05		8		15
8	769.19		9		17
9	865.34		10		19
10	961.49		11		21
11	1057.64		12		23
12	1153.79		14		25
13	1249.94		15		27
14	1346.09		16		29



Franklin Structures, LLC

Model: 5078-64-3-47 Date: 8/6/2019

Wind Pressures for Low-rise buildings or buildings with h<60ft

ASCE 7-10 Chapter 30 Part I:

Wind Speed, Vult: 167 MPH Roof Style: Gable Wind Exposure: С Roof Pitch: 7 /12 Mean Roof Height: 18 FT Roof Angle: 30.3 Elevation: 0 FT Width 30.04 ft Ke: 1.00 6 ft 2a: Kd: 0.85 Wall Height: 9 ft Kzt: 1 Heel Ht: 6 in 0.88 Roof Ht: 8.76 ft kt: 53.53 psf Stud Spacing: 16 "oc qh: **Building Type:** Enclosed Overhang: 12 " Gcpi: 0.18 Int. Shearwall: YES

Main

Actual Width 42'6.75"

-0.18 Min net pressure: 16 psf

MWFRS

Transverse												
	1	2	3	4 1e	2e	3e	4e					
+GCpi	20.3	1.6	-32.7	-29.4	27.3	4.8	-38.0	-35.3				
-Gcpi	39.6	20.9	-13.4	-10.2	46.6	24.1	-18.7	-16.1				
Max	39.6	20.9	-32.7	-29.4	46.6	24.1	-38.0	-35.3				
Longitudinal												
	1	2	3	4	5	6 1e	2e	3e	4e	5e	6	ie
+GCpi	-33.7	-46.6	-29.4	-33.7	11.8	-25.2	-35.3	-66.9	-38.0	-35.3	23.0	-32.7
-Gcpi	-14.5	-27.3	-10.2	-14.5	31.0	-5.9	-16.1	-47.6	-18.7	-16.1	42.3	-13.4
Max	-33.7	-46.6	-29.4	-33.7	31.0	-25.2	-35.3	-66.9	-38.0	-35.3	42.3	-32.7
Vertical						Horz	!					
End		- 1	nt	Over	hang	End		Int				
WW	LV	٧ ١	WW LV	V End	Int	Roof	f Wal	l Roof	Wall			
Trans	24.1	-38.0	20.9	-32.7	-28.4	-31.6	42.8	62.6	34.3	49.8		
Long	-66.9	-38.0	-46.6	-29.4	-28.4 -		-28.9	55.7	-17.1	36.9		

Design Loading

Design Loa	uiiig												
	Vertical						Horz						
		End		Int		Overhang			End		Int		
	ww	LW	١	ww i	LW	End	Int	Roof	,	Wall	Roof	Wall	
Trans		14.5	-22.8	12.5	-19.6	-17	0 -19.0	) :	25.7	37.6	20	.6 29.9	
Long		-40.1	-22.8	-27.9	-17.7	-65	8 -53.6		25.7	33.4	20	.6 22.2	

Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:



Franklin Structures, LLC

Model: 5078-64-3-47 Date: 8/6/2019 Reduction due to tag:

Left wall: 3533.58 lb Interior wall: 1766.79 lb

Shearwalls:

Left Endwall:		Right Endwall:		Interior Shearwall:					
End Zone:	Yes	End Zone:	Yes	End Zone:	Yes				
Trib:	15 ft	Trib:	15 ft	Trib:	30 ft				
End Roof:	1351 lb	End Roof:	1351 lb	End Roof:	1351 lb				
End Wall:	1127 lb	End Wall:	1127 lb	End Wall:	1127 lb				
Int Roof:	1621 lb	Int Roof:	1621 lb	Int Roof:	4323 lb				
Int Wall:	1344 lb	Int Wall:	1344 lb	Int Wall:	3584 lb				
Total force:	1910 lb	Total force:	5443 lb	Total force	8619 lb				
Sheathing Thickness:	7/16 in	Sheathing Thickness:	7/16 in	Sheathing Thickness:	7/16 in				
Fastener: .1	131 nail	Fastener: .:	131 nail	Fastener: .	131 nail				
Wall Length:	15 ft	Wall Length:	15 ft	Wall Length:	14.19 ft				
FHS Length:	6.8 ft	FHS Length:	12.5 ft	FHS Length:	14.19 ft				
Wall Height:	9 ft	Wall Height:	9 ft	Wall Height:	9 ft				
Tallest Opening:	h/2	Tallest Opening:	h/3	Tallest Opening:	h				
r:	0.62	r:	0.94	r:	1.00				
Co:	0.79	Co:	1.00	Co:	1.00				
Perf or Segmented:	S	Perf or Segmented:	Р	Perf or Segmented:	S				
Blocked:	YES	Blocked:	YES	Blocked:	YES				
PLF required:	281.28	PLF required:	435.48	PLF required:	607.49				
Framing:	SPF	Framing:	SPF	Framing:	SPF				
Required Spacing:	6 " OC	Required Spacing:	4 " OC	Required Spacing:	3 " OC				
Tiedown:	2531.5 lb	Tiedown:	3919.3 lb	Tiedown:	5467.4 lb				
				Strap for:	4309.355 lb				
Top Sidewall:		Bottom Sidewall:							
End Zone:	yes	End Zone:	Yes						
Trib:	18.00 ft	Trib:	18.00 ft						
End Wall:	1522 lb	End Wall:	1522 lb						
Int Wall:	3834 lb	Int Wall:	3834 lb						
Total force:	5356 lb	Total force:	7544 lb						

Sheathing Thickness:

Wall Length:

FHS Length:

Wall Height:

Framing:

Tiedown:

Required Spacing:

Fastener: .131 nail

Tallest Opening: h
r: 0.88
Co: 0.80

Perf or Segmented: P
Blocked: YES
PLF required: 256.59

7/16 in

42 ft

9 ft

6 " OC

2886.6 lb

36.8 ft

net. Type: VB-unprotected
single Family Dwelling
watelin No.
167 MPH Vult. 130 MPH Vilt.
167 MPH Vult. 130 MPH Vilt.
167 MPH Vult. 130 MPH Vilt.
168 MPH Vult.
169 MPH Vult.
169 MPH Vult.
169 MPH Vult.
160 MPH Vul

Strap: 2200 S20

Sheathing Thickness:

Wall Length:

FHS Length:

Wall Height:

Tallest Opening:

Perf or Segmented:

Required Spacing:

Fastener: .131 nail

r: Co:

Blocked:

Framing:

Tiedown:

PLF required:

Sidewall interconnection: 4 "oc .131 Nails OR #8 Screw

2178.9 lb

7/16 in

60 ft

9 ft

43.8 ft

5h/6

0.76

0.71

YES

6 " OC

172.16

Capacity: 130 lbs per nail
Total Capacity 3510 lbs

			Tiedown	Perf/	# of S20	Corner
Summary:	Fastener	Edge Spacing	Force	Segment	or equal	Connection**
Left Endwall	.131 nail	6 "OC	2531.5 lb	S	2	
Right Endwall	.131 nail	4 "OC	3919.3 lb	Р	2	
Interior Shearwall	.131 nail	3 " OC	5467.4 lb	S	3	
Top Sidewall	.131 nail	6 "OC	2178.9 lb	Р	1	YES
Bottom Sidewall	.131 nail	6 "OC	2886.6 lb	Р	2	YES

<sup>\*\* 4 &</sup>quot;oc .131 nails from sidewall to endwall where both walls have tiedown at the corner, then the sidewall is transfered 2" oc fastener spacing requires double studs and staggered fasteners at panel seams.

Franklin Structures, LLC Model: 5078-64-3-47 Date: 8/6/2019

### Diaphragm:

Max Force: 8618.7 lbs
Load: 269.0 plf
Sheathing: 7/16 in
Fastener: .131 Nail
Framing: SPF
Unblocked Capacity: 294.4 plf

Blocked: 200.9 plf Blocking distance: 0.0 ft

0 ft blocked each end with .131 Nail or fasten with .131 nails (unblocked) 6" oc Boundary, 6" oc Edge

### Notes:

all 15ga staples minimum length of 1.5" all .131 nails minimum length of 2"

#### Single to double connection:

Max Force: 5443 lb
Length: 180 "
.131 nail: 108 lb
Spacing: 3.6 " OC

Connect single wide truss top chords to double wide top chord/gabe endwall w/ .131 nails @ 3" oc

### Tag to Main conection:

Connection force: 2188 lb Length: 30 ft #10x4" screw: 159 lb spacing: 12.0 "

Screw tag to main module with #10x4" screw across bottom chord of tag to rail of main unit

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria





VB-unprotected
Single Family Dwalling

1
167 MPH Vult. 130 MPH Vas
0 Hr
MFT 10886-5078-64-3-47-R1
ad 40 PSF
7/13/2021
Franklin Structures LLC

Franklin Structures, LLC Tag Unit

Model: 5078-64-3-47 Date: 8/6/2019

Wind Pressures for Low-rise buildings or buildings with h<60ft ASCE 7-10 Chapter 30 Part I:

Wind Speed, Vult	t:	167 N	<b>ЛРН</b>	Roof Style: G	able
Wind Exposure:		С		Roof Pitch:	6 /12
Mean Roof Height:		18 F	·T	Roof Angle:	26.6
Elevation:		0 F	T	Width	30.00 ft
Ke:		1.00		2a:	6 ft
Kd:		0.85		Wall Height:	9 ft
Kzt:		1		Heel Ht:	8 in
kt:		0.88		Roof Ht:	7.50 ft
qh:		53.53 p	osf	Stud Spacing:	16 "oc
Building Type:	Enclosed			Overhang:	12 "
Gcpi:		0.18		Int. Shearwall:	NO
		-0.18			
Min net pressure:		16 p	osf		

M	W	FI	RS
---	---	----	----

Transverse											
	1	2	3	4 1e	2e	3e	4e				
+GCpi	19.8	-14.9	-33.6	-30.5	29.3	-19.8	-40.9	-38.3			
-Gcpi	39.1	4.3	-14.3	-11.3	48.6	-0.6	-21.7	-19.0			
Max	39.1	-14.9	-33.6	-30.5	48.6	-19.8	-40.9	-38.3			
Longitudinal											
	1	2	3	4	5	6 1e	2e	3e	4e	5e	6e
+GCpi	-33.7	-46.6	-29.4	-33.7	11.8	-25.2	-35.3	-66.9	-38.0	-35.3	23.0 -32.7
-Gcpi	-14.5	-27.3	-10.2	-14.5	31.0	-5.9	-16.1	-47.6	-18.7	-16.1	42.3 -13.4
Max	-33.7	-46.6	-29.4	-33.7	31.0	-25.2	-35.3	-66.9	-38.0	-35.3	42.3 -32.7
Vertical						Hor	Z				
End			Int	Ov	erhang	End		Int			
WW	LW		ww l	.W End	d Int	Roo	f Wal	ll Roc	f Wa	II	
Trans	-19.8	-40.9	-14.9	-33.6	-53.0	-48.1	21.1	67.6	18.6	50.3	
Long	-66.9	-38.0	-46.6	-29.4	-53.0 -		-28.9	55.7	-17.1	36.9	
					Wal	I forces red	uced due to	wind hittir	ng main uni	t	

Design Loading

Design Loa	aing													
	Vertical								Horz					
		End	Int			Overhang			End			Int		
	ww	LW		WW	LW	End		Int	Roof	Wa	II F	Roof	Wall	
Trans		-11.9	-24.6	-9.0	-20.1		-31.8	-28.9		12.7	40.6	11.2	30.2	
Long		-40.1	-22.8	-27.9	-17.7		-65.8	-53.6		12.7	33.4	11.2	22.2	

Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:



nst. Type:

VB-unprotected
Single Family Dwelling
Welling
167 MPH Vuit. 130 MPH Vai
167 MPH Vuit

Franklin Structures, LLC Model: 5078-64-3-47 Date: 8/6/2019

Pressure reduction for house blocking wind:
Pressure to tag: -22.96 psf End

-18.33 psf Int

Shearwalls:

Left Endwall:		Right Endwall:	Transferred to house
End Zone:	Yes	End Zone:	Yes
Trib:	7.5 ft	Trib:	7.5 ft
End Roof:	570 lb	End Roof:	570 lb
End Wall:	1257 lb	End Wall:	1257 lb
Int Roof:	126 lb	Int Roof:	126 lb
Int Wall:	234 lb	Int Wall:	234 lb
Total force:	2188 lb	Total force:	2188 lb
Sheathing Thickness:	7/16 in		
Fastener: 15ga staple			
Wall Length:	30.08 ft		
FHS Length:	26.00 ft		
Wall Height:	9 ft		
Tallest Opening:	h/2		
r:	0.93		
Co:	0.94		
Perf or Segmented:	Р		
Blocked:	YES		

89.85 SPF

863.5 lb

6 " OC

Bottom Sidewall:

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria

APPROVED BY

Top Sidewall:

PLF required:

Required Spacing: Tiedown:

Framing:

End Zone:	yes	End Zone:	Yes
Trib:	15 ft	Trib:	15 ft
End Wall:	919 lb	End Wall:	919 lb
Int Wall:	1718 lb	Int Wall:	1718 lb
Total force:	3305 lb	Total force:	3305 lb
Sheathing Thickness:	7/16 in	Sheathing Thickness:	7/16 in
Fastener: .131 n	ail	Fastener: .:	131 nail
Wall Length:	6.24 ft	Wall Length:	6.24 ft
FHS Length:	6.24 ft	FHS Length:	6.24 ft
Wall Height:	9 ft	Wall Height:	9 ft
Tallest Opening:	5h/6	Tallest Opening:	h
r:	1.00	r:	1.00
Co:	1.00	Co:	1.00
Perf or Segmented:	Р	Perf or Segmented:	Р
Blocked:	YES	Blocked:	YES
PLF required:	529.63	PLF required:	529.63
Framing:	SPF	Framing:	SPF
Required Spacing:	3 " OC	Required Spacing:	3 " OC
Tiedown:	4766.7 lb	Tiedown:	4766.7 lb

Strap: 2490 CS14 (lbs)
Sidewall interconnection: 6 "oc .131 Nails
Capacity: 130 lbs per nail
Total Capacity 2340 lbs

			Tiedown	Perf/	# CS14	Corner
Summary:	Fastener	Edge Spacing	Force	Segment	or equal	Connection**
Left Endwall	15ga staple	6 "OC	863.5 lb	Р	1	
Right Endwall	See connection below					
Top Sidewall	.131 nail	3 "OC	4766.7 lb	Р	2	NO
Bottom Sidewall	.131 nail	3 "OC	4766.7 lb	Р	2	NO

<sup>\*\* 6 &</sup>quot;oc .131 nails from sidewall to endwall where both walls have tiedown at the corner, then the sidewall is transfered 2" oc fastener spacing requires double studs and staggered fasteners at panel seams.

Franklin Structures, LLC Model: 5078-64-3-47 Date: 8/6/2019

## Diaphragm:

Max Force: 3304.7 lbs
Load: 103.3 plf
Sheathing: 7/16 in

Fastener: 15ga staple

Framing: SPF
Unblocked Capacity: 172.2 plf
Blocked @ 6" edge: 188.6 plf
Blocking distance: 0.0 ft

## 0 ft blocked each end with 15ga staple

Notes:

all 15ga staples minimum length of 1.5" all .131 nails minimum length of 2"

### Connection to main unit:

Force: 2188 lb
Length: 30 ft
#10x4" screw 159 lb
Z: Spacing: 12 "oc

## 12 in oc #10x4 in screws from tag ceiling to main ceiling

Tag added for endwall vertical projection:

Pressure difference: 11.9 psf
Area per wall: 56.25 ft^2
Force: 667.92 lbs

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:



Const. Type: V
Occupancy: S
Allowable No.
of Filors: 1
Wind Velocity: 1
Fire Rating of
Ext. Walls: Plan No.: 0
Allow. Filor Load: 4
Approval Date: 7

VB-unprotected
Single Family Dwelling

1
167 MPH Vult, 130 MPH Vasx
0 Hr
MFT10886-5078-64-3-47-R1
4 40 PSF
7/113/2021

# **Connections**

# Truss to exterior wall uplift:

Uplift Force: 494 lb H2.5A: 535 lb MTS18: 1030 lb

### Truss to exterior wall Lateral:

End: 50.7 psf
Int: 41.1 psf
Height: 9 ft
Spacing: 24 in oc

Load:

End: 456.7 lb Int: 370.0 lb 1 nail: 114.84 lb

.131 nail: 114.84 l .131x3"Nails End: 4.0

.131x3" Nails Int: 3.2

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:



onst. Type: VB-unproted Single Family lowable No.
Floors: 1
Ind Velocity: 167 MPH Vuice Rating of

167 MPH Vult, 130 MPH Va 0 Hr MFT10886-5078-64-3-47-R ond: 40 PSF er 7/13/2021 Franklin Structures, LLC

# Truss king post to Header:

Uplift: 697 lb Gravity: 552 lb .131 nail EG: 88.44 lb .131 nail EG LL: 69.09 lb

# .131x3": 8.0 Nails

Or hanger rated for 552lb grav and 652lb uplift

# Stud to Plate:

End: 50.75 psf
Int: 41.11 psf
Height: 9 ft
Spacing: 16 in oc

Load:

Nails Int:

End: 304.5 lb Int: 246.7 lb .131 nail: 88.44 lb Nails End: 3.4

2.8

End Zone: Use (4) .131x3" nails to connect studs to plates. Int Zone: Use (3) .131x3" nails to connect studs to plates.

# Plate to floor and plate interconnection (top plate):

End: 50.7 psf
Int: 41.1 psf
Height: 9 ft
Load: 228.37 plf

.131 Nailx3": 108 lb

Spacing of .131 nail: 5.7 " OC int and end zones

15gax2.5" staple: 72 lb

Spacing of 15ga: 3.78 " OC int and end zones

# **Sheathing Suction Connections (wall and roof)**

End Int Wall: -50.75 -41.11 psf Roof: -108.6 -63.6 psf .131x2.5: 66 66 lbs 15gax2.5" staple: 56.8 56.8 lbs Wall Member spacing: " oc 16 16 Nail: 11.7 12.0 " oc " oc Staple: 10.1 12.0 Roof " ос Member spacing: 24 24 " oc Nail: 3.6 6.2 " oc Staple: 3.1 5.4

Note End zone is 3ft from the end of the house and from eave/ridge on roof This spacing is a minimum for edge AND field fastening.

# **Truss Kneewall Connection:**

Tension: 687 lb Compare to truss print,
Shear: 338 lb Must be higher then truss.

Tension:

CS 22: 845 lb (8-.131 x2.5" nails each side)

Shear:

Rail to king post toed
.131x3" nail: 93.31 lb
#: 3.6 Use 4

Rail to rail:

.131x3" nail: 103.125 lbs

#: 3.3 Use 4 per bay

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria APPROVED BY



Matewall Girders Supporting Floor Load Only

76 plf

304 plf

380 plf

0 plf

Cr: 1.15 LL defl L/ 360 Cd: 1 TL defl L/ 240

D

D+L

.75(L+Lr)

Vertical Load

Dead Load:

Live Load:

Total Load:

Uplift Load:

Location: Matewall Supporting: Roof & 1 Floor

Wall height: 108 in Min sill height: 18 in LVL: Microllam LVL MOE (E): 2000000 psi

E min: 1016411 psi Fb: 2750 psi Fv: 285 psi Fcperp: 750 psi

		205 ps.
	Fcperp:	750 psi
Volume eff	fect exp (e):	0.136
	Cr (LVL):	1.04
Ev'	Δ	ς

Verti	cal																	
	Qty	. В	D	Species	Grade	Direction	Cfu	Cfb	Fb	Fv	Fcperp	Е	Emin	Fb'	Fv'	Α	S	1
1	1	1.5	9.25	SYP	#1	Edge	1	1.0	1050	175	565	1600000	580000	1050	175	13.9	21.4	98.9
2	1.5	1.5	9.25	SYP	#1	Edge	1	1.0	1050	175	565	1600000	580000	1050	175	20.8	32.1	148.4
3	2	1.5	9.25	SYP	#1	Edge	1	1.0	1050	175	565	1600000	580000	1050	175	27.8	42.8	197.9
4							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0
5							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0
6							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0
7							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0
8							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0
9							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0
10							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0

	Shear	Moment	LL def	TL def
1	121	75	110	121
2	172	92	126	139
3	223	107	139	153
4	#N/A	#N/A	#N/A	#N/A
5	#N/A	#N/A	#N/A	#N/A
6	#N/A	#N/A	#N/A	#N/A
7	#N/A	#N/A	#N/A	#N/A
8	#N/A	#N/A	#N/A	#N/A
9	#N/A	#N/A	#N/A	#N/A
10	#N/A	#N/A	#N/A	#N/A

		Reaction	ns (lbs)	Bearing
Member	Max Span	Gravity	Uplift	(in)
1 (1) 2x 10 SYP #1	75 in	1200	0	1.5
2 (1.5) 2x 10 SYP #1	92 in	1500	0	1.0
3 (2) 2x 10 SYP #1	106 in	1700	0	8.0
4 () 0x 0	#N/A in	#N/A	#N/A	#N/A
5 () 0x 0	#N/A in	#N/A	#N/A	#N/A
6 () 0x 0	#N/A in	#N/A	#N/A	#N/A
7 () 0x 0	#N/A in	#N/A	#N/A	#N/A
8 () 0x 0	#N/A in	#N/A	#N/A	#N/A
9 () 0x 0	#N/A in	#N/A	#N/A	#N/A
10 () 0x 0	#N/A in	#N/A	#N/A	#N/A

Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:

NHINC.

Const. Type: Occupancy: Allowable No. of Floors: Wind Velocity: Fire Rating of Ext. Walls: Plan No.: Allow. Floor Load: Approval Date:

VB-unprotected
Single Family Dwelling

1
167 MPH Vult, 130 MPH Vasx
0 Hr
MFT10886-5078-64-3-47-R1
40 PSF
7/13/2021

Matewall Girders Supporting Roof & 1 Floor

270 plf

321 plf

571 plf

-276 plf

D

D+L

.75(L+Lr)

Vertical Load

Dead Load:

Live Load:

Total Load:

Uplift Load:

Location: Matewall Supporting: Roof & 1 Floor

Wall height: 108 in Min sill height: 18 in 18 in LVL MOE (E): 2000000 psi

E min: 1016411 psi Fb: 2750 psi 285 psi Fv:

	Cr:	1.15	LL defl L,	/ 360												Fcperp:	ا 750	psi
	Cd:	1	TL defl L	/ 240											Volume eff	ect exp (e):	0.136	
																Cr (LVL):	1.04	
Vertica	al																	
	Qty.	В	D	Species	Grade	Direction	Cfu	Cfb	Fb	Fv	Fcperp	E	Emin	Fb'	Fv'	Α	S	1
1	1	1.5	9.25	SYP	#1	Edge	1	1.0	1050	175	565	1600000	580000	1050	175	13.9	21.4	98.9
2	1.5	1.5	9.25	SYP	#1	Edge	1	1.0	1050	175	565	1600000	580000	1050	175	20.8	32.1	148.4
3	2	1.5	9.25	SYP	#1	Edge	1	1.0	1050	175	565	1600000	580000	1050	175	27.8	42.8	197.9
4							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0
5							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0
6							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0
7							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0
8							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0
9							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0
10							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0

	Shear	Moment	LL def	TL def
1	87	61	108	110
2	121	75	124	126
3	155	87	136	139
4	#N/A	#N/A	#N/A	#N/A
5	#N/A	#N/A	#N/A	#N/A
6	#N/A	#N/A	#N/A	#N/A
7	#N/A	#N/A	#N/A	#N/A
8	#N/A	#N/A	#N/A	#N/A
9	#N/A	#N/A	#N/A	#N/A
10	#N/A	#N/A	#N/A	#N/A

		Reaction	ns (lbs)	Bearing
Member	Max Span	Gravity	Uplift	(in)
1 (1) 2x 10 SYP #1	61 in	1500	-710	1.8
2 (1.5) 2x 10 SYP #1	75 in	1800	-870	1.2
3 (2) 2x 10 SYP #1	86 in	2100	-1000	0.9
4 () 0x 0	#N/A in	#N/A	#N/A	#N/A
5 () 0x 0	#N/A in	#N/A	#N/A	#N/A
6 () 0x 0	#N/A in	#N/A	#N/A	#N/A
7 () 0x 0	#N/A in	#N/A	#N/A	#N/A
8 () 0x 0	#N/A in	#N/A	#N/A	#N/A
9 () 0x 0	#N/A in	#N/A	#N/A	#N/A
10 () 0x 0	#N/A in	#N/A	#N/A	#N/A

VB-unprotected Single Family Dwelling

### Floor Joist Calculation

Vertical Load

13 plf Dead Load: D 53 plf Live Load: L Total Load: 67 plf D+Lr

Uplift Load: 0 plf

> Cr: 1.15 LL defl L/ 360 Cd: 1 TL defl L/ 240

Vertical

. В Qty. D Species Grade Direction Cfu Cfb Fb Fv Fcperp E Emin Fb' 565 1600000 580000 1050 Fv' Α S -1 1.5 9.25 175 13.9 21.4 98.9 1 1 SYP #1 Edge 1 1.0 1050 175 3

9 10

	Shear	Moment	LL def	TL def
1	603	180	197	217
2				
3				
4				
5				
1 3 4 5 6 7 8				
7				
8				
9				
10				

		Reaction	ns (lbs)	Bearing
Member	Max Span	Gravity	Uplift	(in)
1 (1) 2x 10 SYP #1	180 in	500	0	0.6
2				
3				
4				
5				
6				
7				
8				
9				
10				

These prints comply with the Florida Manufactured Building Act and adopted Codes and

#### Porch Headers matewall and sidewall

150 plf

128 plf

334 plf

-341 plf

D

Lr

LL defl L/ 240

TL defl L/ 180

D+Lr

.6D+Wn

Vertical Load

Cr: 1.15

Cd: 1.25

Dead Load:

Live Load:

Total Load:

Uplift Load:

Location: Matewall Supporting: Roof

108 in	Wall height:
18 in	Min sill height:
Microllam	LVL:
2000000 psi	LVL MOE (E):
1016411 psi	E min:

Fb: 2750 psi Fv: 285 psi

Fcperp:	750 ps
Volume effect exp (e):	0.136
Cr (LVL):	1.04

																CI (LVL).	1.04	
Vertical	al																	
	Qty.	В	D	Species	Grade	Direction	Cfu	Cfb	Fb	Fv	Fcperp	Ε	Emin	Fb'	Fv'	Α	S	1
1	2	1.5	7.25	SYP	#1	Edge	1	1.0	1250	175	565	1600000	580000	1563	219	21.8	26.3	95.3
2	2	1.5	7.25	SPF	#2	Edge	1	1.2	875	135	425	1400000	510000	1313	169	21.8	26.3	95.3
3	1	1.5	11.25	SPF	#2	Edge	1	1.0	875	135	425	1400000	510000	1094	169	16.9	31.6	178.0
4	1	1.5	9.25	LVL		Edge	1	1.0	2750	285	750	2000000	1016411	3561	285	13.9	21.4	98.9
5	2	1.5	7.25	LVL		Edge	1	1.1	2750	285	750	2000000	1016411	3681	285	21.8	26.3	95.3
6							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0
7							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0
8							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0
9							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0
10							#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	#N/A	0.0	0.0	0.0

Since deflection doesn't govern the LDF between uplift and gravity (1.6 vs 1.25)

allow a maximum uplift per truss of (334\*1.6/1.25\*1.33ft=572lb)

	Shear	Moment	LL def	TL def
1	243	109	166	157
2	191	100	159	150
3	159	100	196	185
4	208	148	181	171
5	312	167	179	169
6	#N/A	#N/A	#N/A	#N/A
7	#N/A	#N/A	#N/A	#N/A
8	#N/A	#N/A	#N/A	#N/A
9	#N/A	#N/A	#N/A	#N/A
10	#N/A	#N/A	#N/A	#N/A

Connect tru Max truss re					
Header to c	olumn co	rner:			
ΓC-44 Brad	ket: Capa	acity 198	Olbs		
	for boad	100 001	s 1860LB	OK	

Connections:

Header to standard column: (2) Ista18 straps:1105 BC4:605 uplift Max reaction @ 8': 1860lb

Column to Floor: BC-40 bracket: 510lbs (2) Ista18 straps:1105 Total:2720lbs

Max reation for 8' span: 1860lb OK

			Reactio	Bearing	
Member	Max Spa	ın	Gravity	Uplift	(in)
1 (2) 2x 8 SYP #1	108 in		1600	-1540	1.0
2 (2) 2x 8 SPF #2	99 in		1400	-1410	1.3
3 (1) 2x 12 SPF #2	99 in		1400	-1410	2.6
4 (1) 2x 9.25 LVL	148 in		2100	-2110	1.5
5 (2) 2x 7.25 LVL	166 in		2400	-2360	0.8
6 () 0x 0	#N/A in		#N/A	#N/A	#N/A
7 () 0x 0	#N/A in		#N/A	#N/A	#N/A
8 () 0x 0	#N/A in		#N/A	#N/A	#N/A
9 () 0x 0	#N/A in		#N/A	#N/A	#N/A
10 () 0x 0	#N/A in		#N/A	#N/A	#N/A



0 Hr MFT10886-5078-64-3-47-R1 40 PSF 7/13/2021

Location: Matewall Supporting: Roof

<u>Vertical L</u>	.oad				Lateral Loa	<u>ıd</u>				Combined	Vert and La	it (max La	at)	Wa	all height:	108 in
NDS Load:	336	plf	D+Lr		Lateral onl	У				Vertical:	150	plf	0	Top/Btm F	Plate (tp):	4.5 in
Total Load:	336	plf	D+Lr	9	Stud area:	27.0	ft^2			Lateral:	5	psf	W		LVL:	Microllam
Uplift Load:	-349	plf	.6D+Wn		Lateral:	5	psf	W		Combined	Vert and La	t (max V	ert)	LVL	MOE (E):	2000000 psi
·							•			Vertical:	336		0			1016411 psi
										Lateral:	3.8	•	.75W		Fb:	2750 psi
					def=.7	C&C					Uplift and I	•			Fv:	285 psi
Cr:	1									Vertical:	-349		.6D+Wn		Fcperp:	
Cd:	1.6									Lateral:	3.8	•	W	V	ol eff (e):	
Cd grav:	1.25			atoral dof	lection L/	120				Lateran	5.0	p5.	••	•	Cr (LVL):	1.04
Vertical	1.25			atterar att	icction L/	120									Ci (LVL).	1.04
Spacing	В	D	Species	Grade	С	le/D	Cfb	CfC	Fb	Ec (gray)	Fc (comb)	Fcperp	Е	Emin	FcE	Ft
1 96	3.5	3.5	SYP	#3	0.8	29.6	1.0	1.00	650	850	850	565	1300000	470000	442	400
1 90	3.5	5.5	317	#3	0.6	29.0	1.0			0.37	0.30	303	1300000	470000	442	400
	4.5								Cp			5.05	1200000	470000		640
Header	1.5	inches						Allowable:	1040	395	407	565	1300000	470000		640
# of Studs			1													
<u>Properties</u>																
Area in^2			12.3													
Sx in^3			7.1													

# of	Studs	1			
Prop	<u>perties</u>				
Area	in^2		12.3		
Sx	in^3		7.1		
lx	in^4		12.5		
Axia	l Loading				
Fc co		173			
Fc P	erp compression		106		
Tens	sion		270		
Fcpe	erp				
Com	bined Loading				
Upli	ft/Lateral		270		
Vert	/Lateral max Lat		234		
Vert	/Lateral Max Vert		126		
<u>Defl</u>	ection Check	L/	338 OK 106		

Max Span	Max Trib	М	Max Side Opening			
1	106 in	112 in =	9 ft -	4 in		
2						
3						
4						
5						
6						
7						
8						

Notes: Center column is total span on both sides of column. Side column is total clear span All studs are to be braced in weak axis by gypsum or sheathing.

Center column must be in center 1/3 of span.

Studs must be as wide as header.

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria



Const. Type: Occupancy: Allowable No. of Floors: Wind Velocity: Fire Rating of Ext. Walls: Plan No.: Allow. Floor Load Approval Date:

pe:	VB-unprotected
y:	Single Family Dwelling
No.	
	1
city:	167 MPH Vult, 130 MPH Vasc
g of	
ė	0 Hr
	MFT10886-5078-64-3-47-R1
or Load:	40 PSF
Date:	7/13/2021
irer:	Franklin Structures, LLC



## COMPANY

**PROJECT** Ryan W. Boring Franklin point load beam

Aug. 26, 2019 21:06

# **Design Check Calculation Sheet**

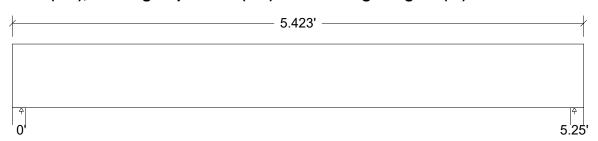
WoodWorks Sizer 11.1

## Loads:

IInfactored:

Load	Type	Distribution	Pat-	Location	[ft]	Magnitud	е	Unit
			tern	Start	End	Start	End	
Point Load	Roof live	Point		2.62		4140		lbs
Self-weight	Dead	Full UDL				6.3		plf

# Maximum Reactions (lbs), Bearing Capacities (lbs) and Bearing Lengths (in):



Dead Roof Live Factored:	16 2141		16 1999
Total Bearing:	2158		2015
Capacity			
Beam	3375		3375
Support	5951		5951
Des ratio			
Beam	0.64	These prints comply with the Const. Type: VB-unprotected Florida Manufactured Bullding Occupancy: Single Family Dwelling	0.60
Support	0.36	Act and adopted Codes and Allowable No. adhere to the following criteria: of Floors;	0.34
Load comb	#2	Wind Velocity:  ADDROVIED BY FIR Reling of FIR Reling of	#2
Length	1.50	Ext. Walls: 0 Hr Plan No. WFT10886-5078-64-3-47-R1	1.50
Min req'd	0.96	Allow. Floor Load: 40 PSF Approval Date: 7/13/2021	0.90
Cb	1.00	Manufacturer: Franklin Structures. LLC.	1.00
Cb min	1.00		1.00
Cb support	1.00		1.00
Fc sup	1150		1150

LVL n-ply, 2.0E, 3100Fb, 1-1/2"x7-1/4", 2-ply (3"x7-1/4") Supports: All - Lumber n-ply Column, S-P-F No.1/No.2 Total length: 5.25'; Clear span: 5.249'; volume = 0.8 cu.ft. Lateral support: top= at supports, bottom= at supports;

# Analysis vs. Allowable Stress and Deflection using NDS 2015:

Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	fv = 149	Fv' = 285	psi	fv/Fv' = 0.52
Bending(+)	fb = 2488	Fb' = 2535	psi	fb/Fb' = 0.98
Live Defl'n	0.11 = L/557	0.26 = L/240	in	0.43
Total Defl'n	0.11 = L/553	0.35 = L/180	in	0.33

WoodWorks® Sizer

## SOFTWARE FOR WOOD DESIGN

## point load beam

## WoodWorks® Sizer 11.1

# Page 2

### Additional Data: FACTORS: F/E(psi)CD LC# CMCt CLCV Cfu CrCfrt Ci Cn Fv' 1.00 1.00 285 1.00 1.00 2 Fb'+ 1.00 3100 1.00 0.818 1.00 1.00 1.00 2 750 1.00 1.00 Fcp' \_ \_ 2 2.0 million 1.00 Е' 1.00 2 Eminy' 1.04 million 1.00 1.00 CRITICAL LOAD COMBINATIONS: Shear : LC #2 = D+Lr, V max = 2158, V design = 2154 lbs Bending(+): LC #2 = D+Lr, M = 5449 lbs-ft (live) Deflection: LC #2 = D+Lr LC #2 = D+Lr(total) D=dead L=live S=snow W=wind I=impact Lr=roof live Lc=concentrated E=earthquake All LC's are listed in the Analysis output Load combinations: ASCE 7-10 / IBC 2015 CALCULATIONS: Deflection: EI = 95.3e06 lb-in2/ply "Live" deflection = Deflection from all non-dead loads (live, wind, snow...) Total Deflection = 1.50(Dead Load Deflection) + Live Load Deflection. Lateral stability(+): Lu = 5.25' Le = 10.38' RB = 20.0; b = single ply width

## **Design Notes:**

- 1. WoodWorks analysis and design are in accordance with the ICC International Building Code (IBC 2015), the National Design Specification (NDS 2015), and NDS Design Supplement.
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. SCL-BEAMS (Structural Composite Lumber): the attached SCL selection is for preliminary design only. For final member design contact your local SCL manufacturer.
- 4. Size factors vary from one manufacturer to another for SCL materials. They can be changed in the database editor.
- 5. BUILT-UP SCL-BEAMS: contact manufacturer for connection details when loads are not applied equally to all plys.
- 6. FIRE RATING: Joists, wall studs, and multi-ply members are not rated for fire endurance.

Florida Manufactured Building Act and adopted Codes and adhere to the following criteria: APPROVED BY



nst. Type: VB-unprotected Single Family Dwelling water foots: 1 Growth Meeting Water foots: 0 Hr METIOSS6-5078-64-3-47- We Floor Load: 40 PSE 713-2021



## **COMPANY**

PROJECT Ryan W. Boring Franklin Column1

Aug. 26, 2019 21:10

Calc is for jack studs only, king studs per other calculations.

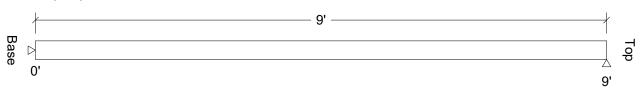
# **Design Check Calculation Sheet**

WoodWorks Sizer 11.1

## Loads:

Load	Туре	Distribution	Location [ft]	Magnitude	Unit
			Start End	Start End	
Point Load	Roof live	Axial	(Ecc. = 0.58")	2158	lbs
Self-weight	Dead	Axial		24	lbs

# Lateral Reactions (lbs):



Unfactored: Dead		
Roof Live	12	-12
Factored:		
R->L		-12
Load comb		#2
L->R	12	
Load comb	#2	#1

# **Point Load Jack Studs**

2x4 ran since studs are ripped to 5", all ripped lumber must

Support: Lumber-soft Sill plate, S. Pine No.2; Bearing length = column width; continuous lower support be regraded Total length: 9.0'; Clear span: 8.75'; volume = 0.7 cu.ft.

Pinned base; Load face = width(b); Built-up fastener: nails; Ke x Lb: 1.0 x 0.0 = 0.0 [ft]; Ke x Ld: 1.0 x 9.0 = 9.0 [ft]; Repetitive factor: applied where permitted (refer to online help);

# Analysis vs. Allowable Stress and Deflection using NDS 2015 :

Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	fv = 2	Fv' = 175	psi	fv/Fv' = 0.01
Bending(+)	fb = 206	Fb' = 1100	psi	fb/Fb' = 0.19
Axial	fc = 208	Fc' = 408	psi	fc/Fc' = 0.51
Combined (ax:	al + eccentric mo	oment)		Eq.15.4-3 = 0.65
Axial Bearing	fc = 208	Fc* = 1450	psi	fc/Fc* = 0.14
Support Bearing	fcp = 208	Fcp = 636	psi	fcp/Fcp = 0.33
Live Defl'n	0.06 = < L/999	0.90 = L/120	in	0.07
Total Defl'n	0.06 = < L/999	0.90 = L/120	in	0.07

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria



## SOFTWARE FOR WOOD DESIGN

# Column1 WoodWorks® Sizer 11.1 Page 2

### Additional Data: FACTORS: F/E(psi)CD LC# CMCt CL/CP CF Cfu CrCfrt Ci Fv' 175 1.00 1.00 1.00 1.00 1.00 2 Fb'+ 1.000 1.00 1.00 750 1.00 1.00 1.00 1.467 1.00 1.00 2 1250 1.00 1.00 1.00 0.282 1.160 1.00 1.00 2 Fc' \_ \_ Ε' 1.00 1.00 2 1.4 million 1.00 1.00 Emin' 0.51 million 1.00 1.00 1.00 1.00 2 Fc\* 1250 1.00 1.00 1.00 1.160 1.00 1.00 2 Fcp sup 565 1.00 1.00 1.00 1.00 2 CRITICAL LOAD COMBINATIONS: Shear : LC #2 = D+Lr, V max = 12, V design = 12 lbs Bending(+): LC #2 = D+Lr, M =105 lbs-ft Deflection: LC #2 = D+Lr (live) LC #2 = D+Lr(total) : LC #2 = D+Lr, P = 2182 lbs Kf = 1.00Axial Eq.15.4-3 : LC #2 = D+Lr Fb' = 1100FcE= 440 Pxe/S=fc(6xe/d)=206: LC #2 = D+Lr; R = 2182 lbs, Cap = 6674, Lb = 3.00", Cb = 1.13 D=dead L=live S=snow W=wind I=impact Lr=roof live Lc=concentrated E=earthquake All LC's are listed in the Analysis output Load combinations: ASCE 7-10 / IBC 2015 CALCULATIONS: Deflection: EI = 7.50e06 lb-in2/ply "Live" deflection = Deflection from all non-dead loads (live, wind, snow...) Total Deflection = 1.50(Dead Load Deflection) + Live Load Deflection.

# **Design Notes:**

- 1. WoodWorks analysis and design are in accordance with the ICC International Building Code (IBC 2015), the National Design Specification (NDS 2015), and NDS Design Supplement.
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. BUILT-UP COLUMNS: nailed or bolted built-up columns shall conform to the provisions of NDS Clause 15.3.
- 4. FIRE RATING: Joists, wall studs, and multi-ply members are not rated for fire endurance.
- 5. Axial load eccentricity applied in direction of load face only. It is the designers responsibility to check for effect of eccentricity in the other direction.

## ridgebeam to header

uplift- 4260lbs each side of tag HTS16 strap=1310lbs each (2)=2620lbs CS20 strap 1030lbs (2)= 2060lbs total =4680

use (1) HTS16 strap AND (1) CS20 strap on each LVL header to header below

header to column uplift-2158lbs Ista18: 1115lbs

Use (2) Ista18 straps at the top and bottom of each column (2 per end of headers)



On-frame floor id	oist
-------------------	------

Home Properties:	Joist Properties:			
Unit Width:	180 in	Size:	2x8	
Overhang:	12 in	Species/Gr	SYP #1	
Roof Live Load:	20 psf	Fb:	1250 psi	
Roof Dead Load:	20 psf	Cr:	1.15	
Wall Dead Load:	5 psf	Cf:	1	
Wall height:	9 ft	Fb':	1437.5 psi	
Floor Live Load:	40 psf	Cd:	1.25	
Floor Dead Load:	10 psf	E:	1600000 psi	
		Fv:	175 psi	
		Spacing:	16 in	
		Area:	10.875 in^2	
Chassis Properties:		I:	47.63 in^4	

S:

ОК

I-beam spacing: 95.5 in I-beam height: 10 in Outrigger thick: 13 ga

Outrigger thick: 0.0897 in Outrigger height: 9.5 in Outrigger width: 1.5 in 21780 psi Outrigger Str: Outrigger Fy: 33,000 psi 13200 psi Outrigger Fv: Outrigger Spacing: 96 in Outrigger setback: 0 in X-member thick: 13 ga X-member thick: 0.0897 in X-member width: 1.5 in X-member shape: С 2 " X-member depth:

PV= 513.3 lb

Cantilever Moment:

 Ext wall thickness:
 3.5

 B':
 40.5 in

 Floor Loads:
 5.56 lb/in

 M floor:
 379.69 lb-ft

 M Joist:
 1967.67 lb-ft

 M residual:
 1587.98 lb-ft

 PV1:
 470.5 lb

 Mid span moment:

M: -1439.9 lb-ft M allowable: 1967.7 lb-ft

M: 0.73 OK

## Determine Max PVL due to deflection:

PVL: 227 lb
WL: 4.4 lb/in
L/120: 0.23 in
Max PVL @ L/180 150.67 lb
Defl @ mid span: 0.10
L\360: 0.27
Live Load to frame: 76.00 lb

 PV2 due to moment:
 42.8 lb

 PV2 due to deflection:
 76.00 lb

 # joists:
 6

 MO:
 18467 lb-in

 V:
 456 lb

 Actual PV1:
 437 lb

crossmember properties

13.14 in^3

A1:	0.135	in^2
A2:	0.135	in^2
A3:	0.163	in^2
C1:	0.75	in
C2:	0.75	in
C3C:	1.455	in
AC1:	0.101	in^3

AC2: 0.101 in^3 AC3: 0.238 in^3 0.432 in^2 Sum A: Sum AC: 0.439 in^3 Cprime: 1.016 in 0.0252 in^4 11: 12: 0.0252 in^4 13: 0.0001 in^4 0.416 in y'1: y'2: 0.416 in 0.439 in y'3: Ay21: 0.023 in^4 0.023 in^4 Ay22: Ay23: 0.031 in^4 Sum I: 0.0506 in^4 0.078 in^4 Sum Ay2: 0.129 in^4 ly-y: r: 0.55 in

> These prints comply with the Florida Manufactured Building Act and adopted Codes and others to the following criteria:



Const. Type: Occupancy: Allowable No. of Floors: Wind Velocity: Fire Rating of Ext. Walls: Plan No.: Allow. Floor Load Approval Date: Manufacturer:

VB-unprotected
Single Family Dwelling

1
167 MPH Vult. 130 MPH Va
MFT10886-5078-64-3-47-R
d: 40 PSF
7/13/2021

## On-frame floor joist

Outrigger design:

I: 12.48 in^4
Sy: 2.63
M allow: 57226 lb-in
M: 18467 lb-in OK
Area: 0.852 in^2
Vallow: 11248.38 lbs

V: 456 lb **OK** 

1/8" weld:

 Applied M:
 1944 lb

 Weld Strength:
 18 ksi

 Throat thick:
 0.125 for 1/8" weld

 Load:
 1590.75 lb/in

 L for moment:
 1.22 in

 L for shear:
 0.29 in

 Length needed:
 1.30 in

Crossmember design:

k: 0.5 fixed kl/r: 87.5
Fa: 15.3 ksi
Allowable Comp: 6616 lbs
PH= 1944 lbs
Weld, L: 1.22 in

Lateral design:

# of Lags: 7

Lag stregnth: 310 lb (9mmx76mm Fastec or equal)

 LDF:
 1.25

 Lag stregnth:
 387.5 lb

 Total:
 2712.5 lb

 mu:
 0.2

 Friction:
 192.2 lb/joist

 Friction total:
 1153 lb

 Total lateral:
 3865 lb

 Letters forces:
 1043.0 lb

Lateral force: 1943.9 lb **OK** 

Short outrigger increased moment:

Moment:

 Short out M:
 0.00 in-lb

 M:
 22268.44 in-lb

 Total M:
 22268.44 in-lb

 M Allowable:
 23612.06 in-lb

 M/Mallow:
 **0.94 ok** 

 Note: all exterior doors must have piers under each side.

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:



Const. Type: Occupancy: Allowable No. of Floors: Wind Velocity: Fire Rating of Ext. Walls: Plan No.: Allow. Floor Load: Approval Date: Manufacturer:

VB-unprotected
Single Family Dwelling

1
167 MPH Vult, 130 MPH Vasc
0 Hr
MFT10886-5078-64-3-47-R1
44 0 PSF
7/13/2021

On-frame floor id	oist
-------------------	------

Home Properties:	Joist Properties:			
Unit Width:	180 in	Size:	2x8	
Overhang:	12 in	Species/Gr	SYP #1	
Roof Live Load:	20 psf	Fb:	1250 psi	
Roof Dead Load:	20 psf	Cr:	1.15	
Wall Dead Load:	5 psf	Cf:	1	
Wall height:	9 ft	Fb':	1437.5 psi	
Floor Live Load:	40 psf	Cd:	1.25	
Floor Dead Load:	10 psf	E:	1600000 psi	
		Fv:	175 psi	
		Spacing:	16 in	
		Area:	10.875 in^2	
Chassis Properties:		I:	47.63 in^4	

I-beam spacing: 95.5 in I-beam height: 10 in Outrigger thick: 13 ga

Outrigger thick: 0.0897 in Outrigger height: 9.5 in Outrigger width: 1.5 in 21780 psi Outrigger Str: Outrigger Fy: 33,000 psi Outrigger Fv: 13200 psi Outrigger Spacing: 96 in Outrigger setback: 8 in X-member thick: 13 ga X-member thick: 0.0897 in X-member width: 1.5 in X-member shape: С 2 " X-member depth:

PV= 513.3 lb

Cantilever Moment:

 Ext wall thickness:
 3.5

 B':
 40.5 in

 Floor Loads:
 5.56 lb/in

 M floor:
 379.69 lb-ft

 M Joist:
 1967.67 lb-ft

 M residual:
 1587.98 lb-ft

 PV1:
 470.5 lb

 Mid span moment:

M: -1439.9 lb-ft M allowable: 1967.7 lb-ft

M: 0.73 OK

## Determine Max PVL due to deflection:

PVL: 227 lb
WL: 4.4 lb/in
L/120: 0.23 in
Max PVL @ L/180 150.67 lb
Defl @ mid span: 0.10
L\360: 0.27
Live Load to frame: 76.00 lb

ОК

PV2 due to moment: 42.8 lb
PV2 due to deflection: 76.00 lb
# joists: 6
MO: 18467 lb-in
V: 456 lb
Actual PV1: 437 lb

crossmember properties

13.14 in^3

A1:		0.135	in^2
A2:		0.135	in^2
A3:		0.163	in^2
C1:		0.75	in
C2:		0.75	in
C3C:		1.455	in
AC1:		0.101	in^3

AC2:	0.101	in^3
AC3:	0.238	in^3
Sum A:	0.432	in^2
Sum AC:	0.439	in^3
Cprime:	1.016	in
I1:	0.0252	in^4
12:	0.0252	in^4
13:	0.0001	in^4
y'1:	0.416	in
y'2:	0.416	in
y'3:	0.439	in
Ay21:	0.023	in^4
Ay22:	0.023	in^4
Ay23:	0.031	in^4
Sum I:	0.0506	in^4
Sum Ay2:	0.078	in^4
ly-y:	0.129	in^4
r:	0.55	in

These prints comply with the Florida Manufactured Buildin Act and adopted Codes and



Const. Type: Occupancy: Allowable No. of Floors: Wind Velocity: Fire Rating of Ext. Walls: Plan No.: Allow. Floor Load Approval Date: Manufacturer;

VB-unprotected
Single Family Dwelling

1
167 MPH Vult. 130 MPH Va
0 Hr
MFT10886-5078-64-3-47-R1
rd: 40 PSF
7/13/2021

## On-frame floor joist

Outrigger design:

I: 12.48 in^4
Sy: 2.63
M allow: 57226 lb-in
M: 18467 lb-in OK
Area: 0.852 in^2
Vallow: 11248.38 lbs

V: 456 lb **OK** 

1/8" weld:

 Applied M:
 1944 lb

 Weld Strength:
 18 ksi

 Throat thick:
 0.125 for 1/8" weld

 Load:
 1590.75 lb/in

 L for moment:
 1.22 in

 L for shear:
 0.29 in

 Length needed:
 1.30 in

Crossmember design:

k: 0.5 fixed kl/r: 87.5
Fa: 15.3 ksi
Allowable Comp: 6616 lbs
PH= 1944 lbs
Weld, L: 1.22 in

Lateral design:

# of Lags: 7

Lag stregnth: 310 lb (9mmx76mm Fastec or equal)

 LDF:
 1.25

 Lag stregnth:
 387.5 lb

 Total:
 2712.5 lb

 mu:
 0.2

 Friction:
 192.2 lb/joist

 Friction total:
 1153 lb

 Total lateral:
 3865 lb

Lateral force: 1943.9 lb **OK** 

Short outrigger increased moment:

Moment:

 Short out M:
 3647.78 in-lb

 M:
 22268.44 in-lb

 Total M:
 25916.22 in-lb

 M Allowable:
 23612.06 in-lb

M/Mallow: 1.10 MUST DOUBLE JOIST OVER OUTRIGGER

Note: all exterior doors must have piers under each side.

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:



Const. Type: Occupancy: Allowable No. of Floors: Wind Velocity: Fire Rating of Ext. Walls: Plan No.: Allow. Floor Load: Approval Date:

VB-unprotected
Single Family Dwelling

1
167 MPH Vult, 130 MPH Vasc

0 Hr
MFT10886-5078-64-3-47-R1

240 PSF
7/113/2021



## COMPANY

PROJECT
Ryan W. Boring
Franklin
point load beam

ok w/in 5%

Sep. 5, 2019 16:11

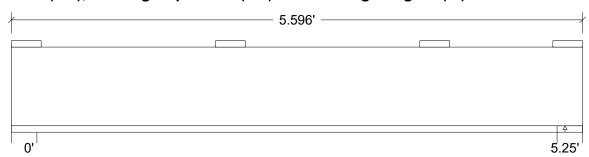
# **Design Check Calculation Sheet**

WoodWorks Sizer 11.1

## Loads:

Load	Type	Distribution	Pat-	Location	[ft]	Magnitud	е	Unit
			tern	Start	End	Start	End	
Point Load	Roof live	Point		2.80		4140		lbs
Self-weight	Dead	Full UDL				4.0		plf

# Maximum Reactions (lbs), Bearing Capacities (lbs) and Bearing Lengths (in):



		.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
Unfactored: Dead Roof Live Factored:	10 2070	A. V. ENSK. VO.	10 2070
Total Bearing:	2080	No 72674 ***	2080
Capacity Beam Support Des ratio Beam Support Load comb Length Min req'd Cb Cb min Cb support Fc sup	3375 5951 0.62 0.35 #2 3.00 1.85 1.00 1.00	These prints comply with the Plendis Manufactured Building Act and adopted Codes and adhere to the following criteria:  APPROVED BY  APPROVED BY  APPROVED BY  INC.  Const. Type: Const. Ty	3375 5951 0.62 0.35 #2 3.00 1.85 1.00 1.00 1.00

# LVL n-ply, 2.0E, 3100Fb, 1-1/2"x9-1/4", 1-ply

Supports: All - Lumber n-ply Column, S-P-F No.1/No.2 Total length: 5.25'; Clear span: 5.248'; volume = 0.5 cu.ft. Lateral support: top= 24 bottom= full; [in]

# This section FAILS the design check

WARNING: This section violates the following design criteria: Bending

# Analysis vs. Allowable Stress and Deflection using NDS 2015 :

						1
Criterion		Analysis Value	Design Value	Unit	Analysis/Design	
	Shear	fv = 225	Fv' = 285	psi	fv/Fv' = 0.7	7
	Bending(+)	fb = 3056	Fb' = 2961	psi	fb/Fb' = 1.03	
	Live Defl'n	0.11 = L/577	0.26 = L/240	in	0.42	
	Total Defl'n	0.11 = L/575	0.35 = L/180	in	0.31	

## SOFTWARE FOR WOOD DESIGN

## point load beam

## WoodWorks® Sizer 11.1

# Page 2

# Additional Data:

```
FACTORS:
            F/E(psi)CD
                                    Ct
                                           \mathtt{CL}
                                                                                              LC#
                             CM
                                                    CV
                                                           Cfu
                                                                   Cr
                                                                         Cfrt
                                                                                 Ci
                                                                                        Cn
 Fv'
                                                                                       1.00
            285
                     1.00
                                   1.00
                                                                         1.00
                                                                                               2
 Fb'+
                     1.00
                                          0.955
                                                    1.00
           3100
                                   1.00
                                                                  1.00
                                                                         1.00
                                                                                               2
            750
                                   1.00
                                                                         1.00
 Fcp'
             2.0 million
                                   1.00
                                                                                               2
 Е'
                                                                         1.00
 Eminy'
           1.04 million
                                   1.00
                                                                         1.00
                                                                                               2
```

## CRITICAL LOAD COMBINATIONS:

```
Shear : LC \#2 = D+Lr, V max = 2077, V design = 2077 lbs
```

Bending(+): LC #2 = D+Lr, M = 5448 lbs-ft

Deflection: LC #2 = D+Lr (live) LC #2 = D+Lr (total)

D=dead L=live S=snow W=wind I=impact Lr=roof live Lc=concentrated E=earthquake

All LC's are listed in the Analysis output Load combinations: ASCE 7-10 / IBC 2015

### CALCULATIONS:

Deflection: EI = 198e06 lb-in2

"Live" deflection = Deflection from all non-dead loads (live, wind, snow...)

Total Deflection = 1.50(Dead Load Deflection) + Live Load Deflection.

Lateral stability(+): Lu = 2.00' Le = 4.13' RB = 14.3

## **Design Notes:**

- 1. WoodWorks analysis and design are in accordance with the ICC International Building Code (IBC 2015), the National Design Specification (NDS 2015), and NDS Design Supplement.
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. SCL-BEAMS (Structural Composite Lumber): the attached SCL selection is for preliminary design only. For final member design contact your local SCL manufacturer.
- 4. Size factors vary from one manufacturer to another for SCL materials. They can be changed in the database editor.
- 5. BUILT-UP SCL-BEAMS: contact manufacturer for connection details when loads are not applied equally to all plys.
- 6. FIRE RATING: Joists, wall studs, and multi-ply members are not rated for fire endurance.



Jul 12, 2021

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:





## COMPANY

PROJECT
Ryan W. Boring
Franklin
point load beam

Sep. 5, 2019 16:02

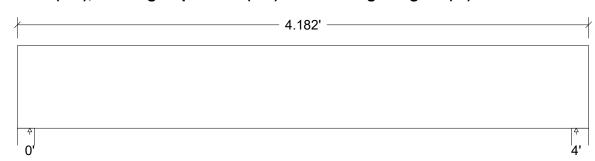
# **Design Check Calculation Sheet**

WoodWorks Sizer 11.1

## Loads:

Load	Type	Distribution	Pat-	Location	[ft]	Magnitud	le	Unit
			tern	Start	End	Start	End	
Point Load	Roof live	Point		2.09		4140		lbs
Self-weight	Dead	Full UDL				8.5		plf

# Maximum Reactions (lbs), Bearing Capacities (lbs) and Bearing Lengths (in):



Unfactored:			
Dead	17		17
Roof Live	2070		2070
Factored:	2070		2070
Total	2087		2087
Bearing:	2007		2007
Capacity			
Beam	3814	$m_{H_{II}}$	3814
	8539	W. Boby	8539
Support Des ratio	6539	CEN	0539
	0.55		0.55
Beam		No 72674	
Support	0.24	_ = ★: No /20/4 ** * = *	0.24
Load comb	#2	()=-:	#2
Length	1.50	E	1.50
Min req'd	0.82	A SHOW A SHOWER	0.82
Cb	1.00	STATE OF	1.00
Cb min	1.00		1.00
Cb support	1.00	OR TOWN	1.00
Fc sup	1150	"MONAL "IN"	1150
•		· inimite.	

Jul 12, 2021 Lumber n-ply, S. Pine, No. 1, 2x8, 3-ply (4-1/2"x7-1/4")

Supports: All - Lumber n-ply Column, S-P-F No.1/No.2 Total length: 4.0'; Clear span: 3.999'; volume = 0.9 cu.ft.

Lateral support: top= at supports, bottom= at supports; Repetitive factor: applied where permitted (refer to online help);

# Analysis vs. Allowable Stress and Deflection using NDS 2015:

Criterion	Analysis Value	Design Value	Unit	Analysis/Design
Shear	fv = 96	Fv' = 175	psi	fv/Fv' = 0.55
Bending(+)	fb = 1265	Fb' = 1333	psi	fb/Fb' = 0.95
Live Defl'n	0.04 = < L/999	0.20 = L/240	in	0.21
Total Defl'n	0.04 = < L/999	0.27 = L/180	in	0.16

Florida Manufactured Buildin Act and adopted Codes and adhere to the following criteri

## SOFTWARE FOR WOOD DESIGN

## point load beam

## WoodWorks® Sizer 11.1

# Page 2

```
Additional Data:
FACTORS:
                                           \mathtt{CL}
                                                                                             LC#
            F/E(psi)CD
                             CM
                                    Ct
                                                   CF
                                                          Cfu
                                                                  Cr
                                                                        Cfrt
                                                                                Ci
                                                                                       Cn
 Fv'
                                                                                      1.00
            175
                     1.00
                            1.00
                                   1.00
                                                                        1.00
                                                                               1.00
                                                                                              2
 Fb'+
                                                                 1.15
           1000
                     1.00
                            1.00
                                   1.00
                                          0.927
                                                  1.250
                                                          1.00
                                                                        1.00
                                                                                              2
                                                                               1.00
            565
                            1.00
                                   1.00
                                                                               1.00
 Fcp'
                                                    _
                                                                        1.00
                                                                                              2
 Е'
            1.6 million
                            1.00
                                   1.00
                                                                        1.00
                                                                               1.00
 Emin'
           0.58 million
                            1.00
                                   1.00
                                                                        1.00
                                                                               1.00
                                                                                              2
```

## CRITICAL LOAD COMBINATIONS:

```
Shear : LC \#2 = D+Lr, V max = 2082, V design = 2082 lbs
```

Bending(+): LC #2 = D+Lr, M = 4157 lbs-ft

Deflection: LC #2 = D+Lr (live) LC #2 = D+Lr (total)

D=dead L=live S=snow W=wind I=impact Lr=roof live Lc=concentrated E=earthquake

All LC's are listed in the Analysis output Load combinations: ASCE 7-10 / IBC 2015

### CALCULATIONS:

Deflection: EI = 76.2e06 lb-in2/ply

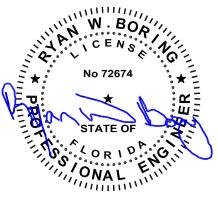
"Live" deflection = Deflection from all non-dead loads (live, wind, snow...)

Total Deflection = 1.50(Dead Load Deflection) + Live Load Deflection.

Lateral stability(+): Lu = 4.00' Le = 8.25' RB = 17.8; b = single ply width

## **Design Notes:**

- 1. WoodWorks analysis and design are in accordance with the ICC International Building Code (IBC 2015), the National Design Specification (NDS 2015), and NDS Design Supplement.
- 2. Please verify that the default deflection limits are appropriate for your application.
- 3. Sawn lumber bending members shall be laterally supported according to the provisions of NDS Clause 4.4.1.
- 4. BUILT-UP BEAMS: it is assumed that each ply is a single continuous member (that is, no butt joints are present) and that each ply is equally top-loaded. Where beams are side-loaded, special fastening details may be required.
- 5. FIRE RATING: Joists, wall studs, and multi-ply members are not rated for fire endurance.



Jul 12, 2021

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:



| Const. Type: | VB-unprotected | Cocupancy | Cocupanc

FLORIDA BUILDING CODE, ENERGY CONSERVATION						
Residential Building	Residential Building Thermal Envelope Approach					
R-Value C	R-Value Computation Method					
FORM R402—2020	Florida Climate Zone					
	BUILDER: Michael E Powell					
PROJECT NAMEAND ADDRESS: Coby Fincher 355 SE Kerce Glen LuLu Fl	PERMITTING OFFICE:					
10886-5078-64-3-47	JURISDICTION NUMBER:					
OWNER: STOCK Fincher	PERMIT NUMBER:					
PERMIT TYPE: Residential	NUMBER OF UNITS: 3					
WORST CASE?	CONDITIONED FLOOR AREA: 2136					

Scope: Compliance with Section R402.1.2 of the Florida Building Code, Energy Conservation, shall be demonstrated by the use of Form R402 for single- and multiple-family residences of three stories or less in height, additions to existing residential buildings, alterations, renovations and building systems in existing buildings, as applicable. To comply, a building must meet or exceed all of the energy efficiency requirements and applicable mandatory requirements summarized on this form. If a building does not comply with this method, or by the UA Alternative method, it may still comply under Section R405 or R406 of the Florida Building Code, Energy Conservation.

- 1. 1.Fill in all the applicable spaces of the "INSTALLED" row in the INSULATION AND FENESTRATION REQUIREMENTS BY COMPONENT table with the information requested. All "INSTALLED" values must be equal to or more efficient than the required levels. "AVG" indicates an area weighted average is allowed; "LOWEST" indicates the lowest R-value to be installed must be entered.
- 2. 2.Complete the tables for air infiltration and installed equipment.
- 3. 3.Read the MANDATORY REQUIREMENTS table and check each box to indicate your intent to comply with all applicable items.
- 4. 4.Read, sign and date the "Prepared By" certification statement at the bottom of this form. The owner or owner's agent must also sign and date the form.

# apps

# INSULATION AND FENESTRATION REQUIREMENTS BY COMPONENT<sup>1</sup>

REQUIREMENTS	FENESTRATIONU- FACTOR <sup>2, 3, 4</sup>	SKYLIGHT <sup>2</sup> U- FACTOR	GLAZEDFENESTRATIONSHGC <sup>2</sup> .	CEILINGR- VALUE	WOODFRAMEWALL R- VALUE	MASS WALLR- VALUE <sup>5,6</sup>	FLOORR- VALUE	BASEMENTWALL R- VALUE	SLAB <sup>7</sup> R- VALUE &DEPTH	CRAWLSPACEWALL R- VALUE
CLIMATE ZONE 1	<u>NR</u>	0.75	0.25	<u>30</u>	<u>13</u>	<u>3/4</u>	<u>13</u>	<u>O</u>	<u>0</u>	<u>0</u>
CLIMATE ZONE 2	0.40	<u>0.65</u>	0.25	<u>38</u>	<u>13</u>	<u>4/6</u>	<u>13</u>	О	<u>0</u>	<u>0</u>
<u>VALUE</u>	<u>AVG</u>	<u>AVG</u>	<u>AVG</u>	LOWEST	<u>LOWEST</u>	<u>LOWEST</u>	LOWEST	<u>LOWEST</u>	LOWEST	<u>LOWEST</u>
INSTALLED:	.34		.21 for GRID/ .24 NO GRID	38	19		19			

## R-Value Calculation Method - [PASS / FAIL]

For SI: 1 foot = 304.8 mm; NR = No requirement.

- 1. (1)R-values are minimums. U-factors and SHGC are maximums. When insulation is installed in a cavity which is less than the label or design thickness of the insulation, the installed R-value of the insulation shall not be less than the R-value specified in the table.
- 2. (2)The fenestration *U*-factor column excludes skylights. The SHGC column applies to all glazed fenestration. Exception: Skylights may be excluded from glazed fenestration SHGC requirements in Climate Zones 1 through 3 where the SHGC for such skylights does not exceed 0.30.
- 3. (3)For impact rated fenestration complying with Section R301.2.1.2 of the Florida Building Code, Residential or Section 1609.1.2 of the Florida Building Code, Building
- 4. (4)One side-hinged opaque door assembly up to 24 square feet is exempted from this U-factor requirement based on Section R402.3.4.
- 5. (5)R-values are for insulation material only as applied in accordance with manufacturer's installation instructions.
- 6. (6)The second R-value applies when more than half the insulation is on the interior of the mass wall.
- 7. (7)R-5 shall be added to the required slab edge R-values for heated slabs. Insulation depth shall be the depth of the footing or 2 feet, whichever is less in Climate Zones 1 through 3 for heated slabs.

# apps

infiltration:

Blower door test is required on the building envelope to verify leakage ≤ 7 ACH50; test report must be provided to code official before CO is issued. Florida Building Code, Energy Conservation Section R402.4.1.2 testing exception may apply for additions, alterations, or renovations.

apps



nst. Type: VB-unprotected single Family Dwelling webbe No. 16-100rs: 1 167 MPH Vult. 130 MPH Visits: 0 Hr WT 10886-5078-64-3-47-R 40 PSF. 2 1-32 (2) 1-32 (2) 1 1-32

FORM R402—continued

EQUIPMENT REQUIREMENTS AND INSTALLED VALUES

Fill in the "INSTALLED EFFICIENCY LEVEL" column with the information requested. For multiple systems of the same type, indicate the minimum efficient system. All "INSTALLED" values must be equal to or more efficient than the required level. If a listed "SYSTEM TYPE" is not to be installed, write in "N/A" for not applicable.

SYSTEM TYPE	MINIMUM EFFICIENCY LEVEL REQUIRED	INSTALLED EFFICIENCY LEVEL
Air distribution system <sup>1</sup>	Not allowed in attic	Location: ON SITE
Air handling unit	<u>Factory Sealed</u>	Factory Sealed? Y/N
Duct R-value	= R-8 (Ducts in unconditioned attics, Diameter ≥ 3 in.)	R-Value (In unc. attic) =
	= R-6 (Ducts in unconditioned non attics, Diam. ≥ 3 in.)	R-Value (In unc. non attics) =
	= R-6 (Ducts in unconditioned attics, Diameter < 3 in.)	R-Value (Small ducts in attic) =
	= R-4.2 (Ducts in unconditioned not attics, Diam. < 3 in.)	R-Value (Small ducts in unc) =
	All ducts are in conditioned space (No minimum)	All in conditioned space ? N
Air leakage/Duct test	Air handler installed: Total leakage = 4 cfm/100 s.f.	ON SITE
	Air handler not installed: Total leakage = 3 cfm/100 s.f.	Total leakage = cfm/100 s.f.
		Air handler installed? Y
Duct testing	Test not required if all ducts and AHU are within the building thermal envelope andfor additions or alterations where ducts extended from existing heating and cooling system through unconditioned space are < 40 linear ft.	Test report required? N
Air conditioning systems:	Minimum federal standard required by NAECA2:	ON SITE
Central system ≤ 65,000 Btu/h	SEER 14.0	SEER (Min)=
PTAC	EER [from Table C403.2.3(3)]	EER (Min)=
Other:	See Tables C403.2.3(1)–(11)	Type = Effic. (min) =
Heating systems:	Minimum federal standard required by NAECA2:	
Heat pump ≤ 65,000 Btu/h	<u>HSPF ≥ 8.2</u>	HSPF (Min) =
Gas furnace, non-weatherized	I <u>HSPF ≥ 80%</u>	AFUE (Min) =
Oil furnace, non-weatherized	<u>HSPF ≥ 83%</u>	AFUE (Min) =
Other:		Type = Effic. (min) =
Water heating system (storag type):	Minimum federal standard required by NAECA <sup>2</sup> :	<u>Capacity =</u>
Electric <sup>3, 6</sup>	<u>UEF 40 gal. 0.923; 50 gal.: 0.921; 60 gal.: 2.051</u>	UEF (Min) =
Gas fired <sup>4,6</sup>	<u>UEF 40 gal. 0.580; 50 gal.: 0.563; 60 gal.: 0.766</u>	UEF (Min) =
Other (describe) <sup>5, 6</sup> :		Type = Effic. (min) =

- (1)Ducts & AHU installed "substantially leak free" per Section R403.3.2. Test required by either individuals as defined in Section 553.993(5) or (7), Florida Statutes, or individuals licensed as set forth in Section 489.105(3)(f), (g), or (i), Florida Statutes. The total leakage test is not required for ducts and air handlers located entirely within the building thermal envelope, and for additions where ducts from an existing heating and cooling system extended to the addition through unconditioned space are less than 40 linear ft.
   (2)Minimum efficiencies are those set by the National Appliance Energy Conservation Act of 1987 for typical residential equipment and are subject to NAECA rules and regulations. For other types of equipment, see Tables C403.2.3 (1-11) of the Commercial Provisions of the Florida Building Code, Energy Conservation.
   (3)For electric storage volumes ≤ 55 gallons, minimum UEF = 0.934 or 0.0011 \* volume). For electric storage volumes > 55 gallons, minimum UEF = 0.9372 (0.0021 \* 1.0011 \* 1.0011 \* 1.0011 \* 1.0011
- 4. (4)For natural gas storage volumes ≤ 55 gallons, minimum UEF = 0.692 (0.00013 \* volume). For natural gas storage volumes > 55 gallons, minimum UEF = 0.8072 (0.0003 \* volume).
   5. (5)For electric tankless, min. UEF = 0.92. For natural gas tankless, min. UEF = 0.81.
- 6. (6)Referenced UEFs shown are for medium draw pattern value provided by manufacturer.

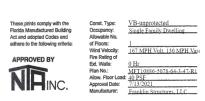
MANDATORY REQUIREMENTS						
<u>Component</u>	Component Section Summary of Requirements		Check			
<u>Air leakage</u>	<u>R402.4</u>	To be caulked, gasketed, weatherstripped or otherwise sealed perTable R402.4.1.1. Recessed lighting IC-rated as having ≤ 2.0 cfmtested to ASTM E283.Windows and doors: 0.3 cfm/sq.ft (swinging doors: 0.5 cfm/sf) whentested to NFRC 400 or AAMA/WDMA/CSA 101/I.S. 2/A440.Fireplaces: Tight-fitting fludampers & outdoor combustion air				
Programmable thermostat	<u>R403.1.2</u>	A programmable thermostat is required for the primary heating orcooling system.	X			





Air distribution system	R403.3.2R403.3.4	Ducts shall be tested as per Section R403.3.2 by either individualsas defined in Section 553.993(5) or (7), Florida Statutes, orindividuals licensed as set forth in Section 489.105(3) (f), (g) or (i), Florida Statutes. Air handling units are not allowed in attics.	ON SITE
Water heaters	<u>R403.5</u>	Comply with efficiencies in Table C404.2. Hot water pipes insulated to ≥ R-3 to kitchen outlets, other cases. Circulating systems to havean automatic or accessible manual OFF switch. Heat trap required for vertical pipe risers.	X
Cooling/heating equipment	<u>R403.7</u>	Sizing calculation performed & attached. Special occasion coolingor heating capacity requires separate system or variable capacitysystem.	X
Swimming pools & spas	R403.10	Spas and heated pools must have vapor-retardant covers or aliquid cover or other means proven to reduce heat loss except if70% of heat from site-recovered energy. Off/timer switch required.Gas heaters minimum thermal efficiency is 82%. Heat pump poolheaters minimum COP is 4.0.	NA
Lighting equipment	<u>R404.1</u>	Not less than 90% of the lamps in permanently installed luminairesshall have an efficacy of at least 45 lumens-per-watt or shall utilizelamps with an efficacy of not less than 65 lumens-per-watt.	ON SITE
	Fracie Terryn compliance with the Florida Building Code,	Review of plans and specifications covered by this form indicatecompliance with the Florida Bui Code, Energy Conservation. Beforeconstruction is complete, this building will be inspected for compliance inaccordance with Section 553.908, F.S.CODE OFFICIAL: Date:	<u>ilding</u>







# **Load Short Form Entire House**

Job: 5078-64-3-47UFL-FL

Date: 8/6/19

AMS of Indiana, Inc.



# **Project Information**

For: FRANKLIN HOMES, 5078-64-3-47UFL-FL

# Design Information

		Desigi	i iiiioiiiiatioii		sthatstere.
	Htg	Clg		Infiltration	
Outside db (°F)	27	96	Method		Simplified
Inside db (°F)	70	75	Construction quality		Average
Design TD (°F)	43	21	Fireplaces		1 (Average)
Daily range	-	L	·		, , ,
Inside humidity (%)	30	50			
Moisture difference (gr/lb)	16	78			

## **HEATING EQUIPMENT**

## **COOLING EQUIPMENT**

Make	Generic			Make	Generic		
Trade				Trade			
Model	AFUE 100			Cond	SEER 13.0		
AHRI ref				Coil			
				AHRI ref			
Efficiency		100 AFUE		Efficiency	11.6	EER, 13 SEE	3
Heating inpu	ut	8.5	kW	Sensible co	ooling	2592	3 Btuh
Heating out	out	29101	Btuh	Latent cool	ing	1111	0 Btuh
Temperature	e rise	24	°F	Total coolin	ıg	3703	3 Btuh
Actual air flo	OW	1112	cfm	Actual air f	low	111	2 cfm
Air flow fact	tor	0.041	cfm/Btuh	Air flow fac	ctor	0.04	5 cfm/Btuh
Static press	sure	0.30	in H2O	Static pres	sure	0.3	0 in H2O
Space thern	nostat			Load sensi	ble heat ratio	0.7	5

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
UTILITY	108	1610	1019	66	46
PTY	33	0	0	0	0
M BA	187	2275	1341	93	60
TOILET	23	103	91	4	4
C1	79	2348	1472	96	66
M BED	296	3469	3957	141	178
B3	184	3241	3370	132	152
B2	191	3303	3020	134	136
BA	56	661	387	27	17
ALCOVE	22	0	0	0	0
ALCOV	21	0	0	0	0
KIT\DIN\LIV	784	10304	10023	l 419 <sup>l</sup>	452

Calculations approved by ACCA to meet all requirements of Manual J 8th Ed.



Entire House Other equip loads Equip. @ 1.01 RSM Latent cooling	1984	27315 1787	24680 885 25923 8415	1112	1112
TOTALS	1984	29101	34339	1112	1112

Calculations approved by ACCA to meet all requirements of Manual J 8th Ed.

Job: 5078-64-3-47UFL-FL

Date: 8/6/19

AMS of Indiana, Inc.

# **Project Information**

For: FRANKLIN HOMES, 5078-64-3-47UFL-FL

Notes:

# **Design Information**

Weather: St Augustine, FL, US

# **Winter Design Conditions**

# **Summer Design Conditions**

Outside db Inside db	27 °F 70 °F	Outside db Inside db	96 °F 75 °F
Design TD	43 ℉	Design TD Daily range	21 °F L
		Relative humidity Moisture difference	50 % 78 gr/lb

## **Heating Summary**

# **Sensible Cooling Equipment Load Sizing**

Structure Ducts Central vent (38 cfm) Outside air	20296 7019 1787	Btuh Btuh Btuh	Structure Ducts Central vent (38 cfm) Outside air	17926 Btuh 6754 Btuh 885 Btuh
Humidification	0	Btuh	Blower	0 Btuh
Piping	0	Btuh		
Piping Equipment load	29101	Btuh	Use manufacturer's data	n
Infiltration			Rate/swing multiplier Equipment sensible load	1.01 25923 Btuh

Method	Simplified
Construction quality Fireplaces	Average 1 (Average)

	Heating	Cooling
Area (ft²)	1984	1984
Volume (ft³)	17854	17854
Air changes/hour	0.45	0.20
Equiv. AVF (cfm)	133	60

# **Latent Cooling Equipment Load Sizing**

Structure Ducts Central vent (38 cfm) Outside air	4140 2292 1984	Btuh
Equipment latent load	8415	Btuh
Equipment Total Load (Sen+Lat) Req. total capacity at 0.70 SHR	34339 3.1	

Generic

# **Heating Equipment Summary**

Make Trade	Generic	
Model AHRI ref	AFUE 100	

100 AFUE	
8.5 kW	
29101 Btuh	
24 °F	
1112 cfm	
0.041 cfm/Btuh	
0.30 in H2O	
	8.5 kW 29101 Btuh 24 ℉ 1112 cfm 0.041 cfm/Btuh

# **Cooling Equipment Summary**

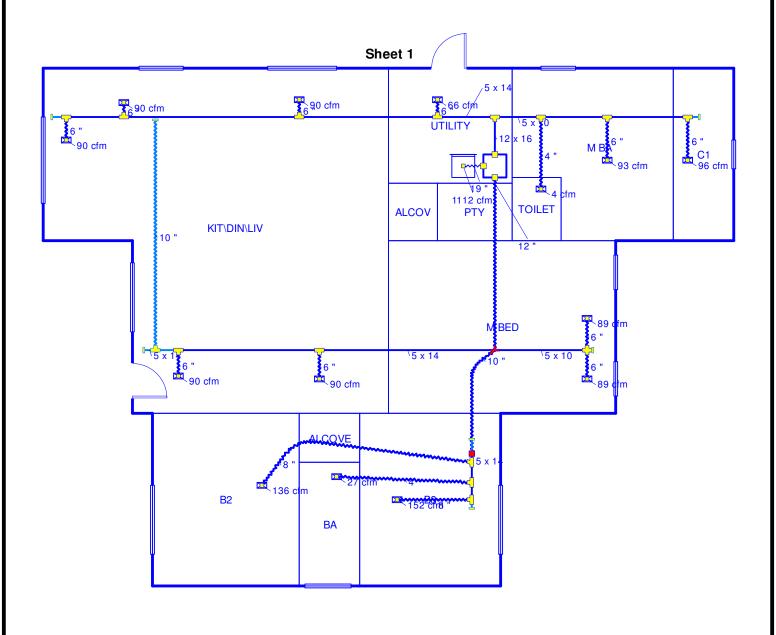
Cond Coil	SEER 13.0	)		
AHRI ref Efficiency		11.6 EER,		
Sensible co Latent coolii	ng Č		25923 11110	Btuh
Total cooling Actual air flo	ów		37033 1112	cfm
Air flow fact Static press	sure		0.30	cfm/Btuh in H2O
Load sensib	le heat ratio	)	0.75	

Calculations approved by ACCA to meet all requirements of Manual J 8th Ed.

Make

Trade





These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:

APPROVED BY NET

nst. Type:

VB-unprotected
cupanory:

Single Family Dwelling
reading of

Value:

O H:

MTELIOSS6-5078-64-3-47-R1

Job #: 5078-64-3-47UFL-FL
Performed by AMS of Indiana, Inc. for:
FRANKLIN HOMES

Scale: 1:100

Page 1

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Job: 5078-64-3-47UFL-FL

Date: 8/6/19

AMS of Indiana, Inc.

# **Project Information**

FRANKLIN HOMES, 5078-64-3-47UFL-FL For:

Cooling Heating 0.30 in H2O 0.30 in H2O External static pressure 0.06 in H2O 0.06 in H2O Pressure losses Available static pressure 0.24 in H2O 0.24 in H2O Supply / return available pressure 0.203 / 0.037 in H2O 0.203 / 0.037 in H2O Lowest friction rate 0.060 in/100ft 0.060 in/100ft Actual air flow 1112 cfm 1112 cfm Total effective length (TEL) 398 ft

# **Supply Branch Detail Table**

Name		esign Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	H x W (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
B2	С	3020	134	136	0.060	8.0	0x 0	VIFx	46.1	290.0	st10
B3	С	3370	132	152	0.067	8.0	0x 0	VIFx	35.5	265.0	st10
ВА	h	661	27	17	0.065	4.0	0x 0	VIFx	39.3	275.0	st10
C1	h	2348	96	66	0.093	6.0	0x 0	VIFx	23.8	195.0	st3
KIT\DIN\LIV	С	2005	84	90	0.084	6.0	0x 0	VIFx	36.8	205.0	st4
KIT\DIN\LIV-A	С	2005	84	90	0.086	6.0	0x 0	VIFx	21.8	215.0	st4
KIT\DIN\LIV-B	С	2005	84	90	0.081	6.0	0x 0	VIFx	44.8	205.0	st7
KIT\DIN\LIV-C	С	2005	84	90	0.082	6.0	0x 0	VIFx	32.8	215.0	st7
KIT\DIN\LIV-D	С	2005	84	90	0.085	6.0	0x 0	VIFx	42.5	195.0	st4
м ва	h	2275	93	60	0.091	6.0	0x 0	VIFx	16.8	205.0	st3
M BED	С	1979	71	89	0.092	6.0	0x 0	VIFx	25.8	195.0	st8
M BED-A	С	1979	71	89	0.092	6.0	0x 0	VIFx	25.5	195.0	st8
TOILET	С	103	4	4	0.089	4.0	0x 0	VIFx	13.5	215.0	st3
UTILITY	h	1610	66	46	0.086	6.0	0x 0	VIFx	9.8	225.0	st4



VB-unprotected



# **Supply Trunk Detail Table**

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	H x W (in)	Duct Material	Trunk
st10 st7 st3 st4 st2 st8 st9 st1	Peak AVF Peak AVF Peak AVF Peak AVF Peak AVF Peak AVF Peak AVF	293 168 192 317 602 141 293 510	305 181 131 317 664 178 305 448	0.060 0.081 0.089 0.084 0.060 0.092 0.060 0.084	628 372 554 653 846 513 560 382	9.8 10.0 7.6 10.7 12.0 10.0 10.0	14 x 5 14 x 5 10 x 5 14 x 5 0 x 0 10 x 5 0 x 0 16 x 12	ShtMetl ShtMetl ShtMetl ShtMetl VinIFIx ShtMetl VinIFIx ShtMetl	st9 st2 st1 st1 st2 st2

# **Return Branch Detail Table**

Name	Grille Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	H x W (in)		Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x 0	1112	1112	61.8	0.060	565	19.0	0x	0		VIFx	

Bold/italic values have been manually overridden



Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: MH80182R10FL-20-167 -

MiTek USA, Inc. 16023 Swingley Ridge Rd Chesterfield, MO 63017

314-434-1200

Site Information:

Customer Info: Franklin Structures, LLC Project Name: . Model: .

Lot/Block: .

Address: ., .

City: . State: .

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Subdivision: .

License #:

Address:

City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special **Loading Conditions):** 

Design Code: FBC2020/TPI2014 Design Program: MiTek 20/20 8.4

Wind Code: ASCE 7-16 Wind Speed: 167 mph Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 2 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

Truss Name Date No. 146079313 5/12/21 1 2 5/12/21 146079314



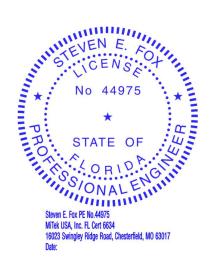
This item has been electronically signed and sealed by Fox, Steve, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Franklin Structures, LLC..

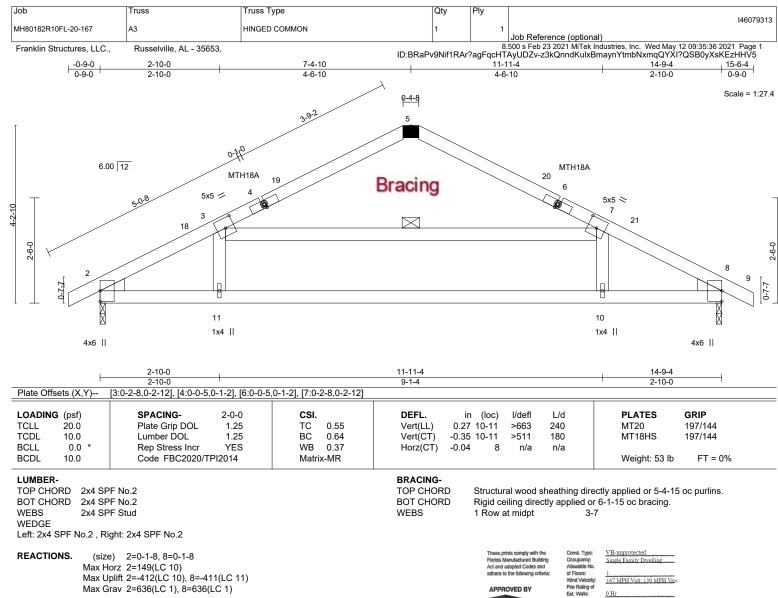
Truss Design Engineer's Name: Fox, Steve

My license renewal date for the state of Florida is February 28, 2023.

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



May 12,2021



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1050/1017, 3-5=-316/424, 5-7=-316/426, 7-8=-1050/1015

**BOT CHORD** 2-11=-811/921, 10-11=-801/927, 8-10=-811/921 **WEBS** 3-11=-11/331, 7-10=-11/330, 3-7=-793/769

- Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in)

REQUIRED FIELD JOINT CONNECTIONS 5=209/428/213/0

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=167mph (3-second gust) Vasd=129mph; TCDL=6.0psf; BCDL=6.0psf; h=24ft; Cat. II; Exp C; Encl. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-9-0 to 2-3-0, Interior(1) 2-3-0 to 4-5-1, Exterior(2R) 4-5-1 to 10-5-1, Interior(1) 10-5-1 to 12-6-4, Exterior(2E) 12-6-4 to 15-6-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) See HINGE PLATE DETAILS for plate placement.
- 6) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
- 7) All additional member connections shall be provided by others for forces as indicated.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 2, 8.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 412 lb uplift at joint 2 and 411 lb uplift at ioint 8.

This item has been electronically signed and sealed by Fox, Steve, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Steven E. Fox PE No.44975 MiTek USA, Inc. FL Cert 663 16023 Swingley Ridge Road, Chesterfield, MO 63017

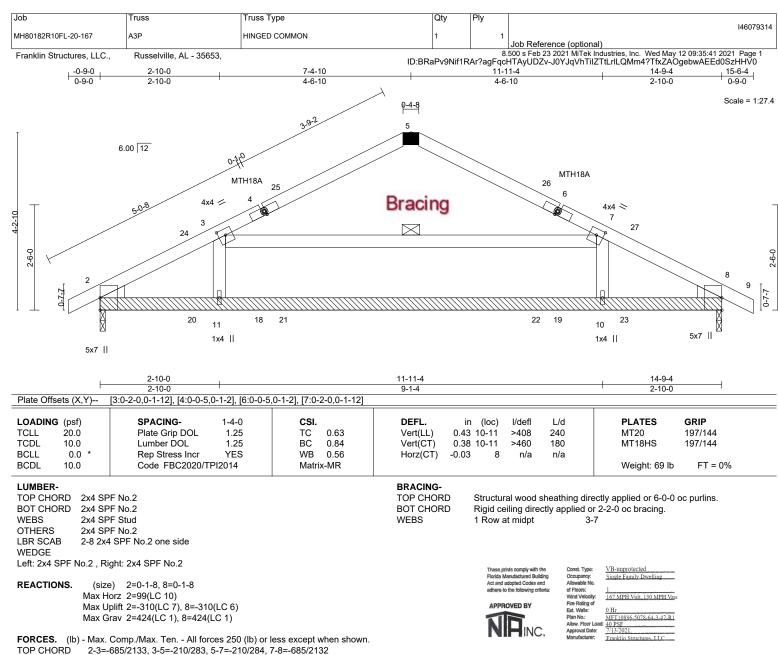
May 12,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

AMSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

MiTek 16023 Swingley Ridge Rd Chesterfield, MO 63017



**BOT CHORD** 

REQUIRED FIELD JOINT CONNECTIONS

2-11=-1816/607, 10-11=-1848/610, 8-10=-1816/607

**WEBS** 3-11=-580/209, 7-10=-577/208, 3-7=-467/1827

- Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in)

5=140/286/142/0

NOTES-

- 1) Attached 14-9-4 scab 2 to 8, front face(s) 2x4 SPF No.2 with 1 row(s) of 10d (0.131"x3") nails spaced 9" o.c.except : starting at 1-3-0 from end at joint 2, nail 1 row(s) at 4" o.c. for 4-2-1; starting at 9-5-1 from end at joint 2, nail 1 row(s) at 4" o.c. for 4-1-3.
- 2) Unbalanced roof live loads have been considered for this design.
- 3) Wind: ASCE 7-16; Vult=167mph (3-second gust) Vasd=129mph; TCDL=6.0psf; BCDL=6.0psf; h=24ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-9-0 to 2-3-0, Interior(1) 2-3-0 to 4-5-1, Exterior(2R) 4-5-1 to 10-5-1, Interior(1) 10-5-1 to 12-6-4, Exterior(2E) 12-6-4 to 15-6-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) See HINGE PLATE DETAILS for plate placement.
- 7) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
- 8) All additional member connections shall be provided by others for forces as indicated.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 2, 8.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 310 lb uplift at joint 2 and 310 lb uplift at joint 8.

This item has been electronically signed and sealed by Fox, Steve, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Steven E. Fox PE No.44975 MiTek USA, Inc. FL Cert 6634 16023 Swingley Ridge Road, Chesterfield, MO 63017

May 12,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chorembers only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see

\*\*AMSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information\*\* available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

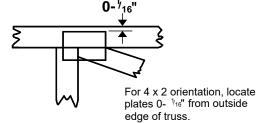


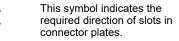
# **Symbols**

### PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.





<sup>\*</sup> Plate location details available in MiTek 20/20 software or upon request.

### PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

### LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated

## **BFARING**



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

ANSI/TPI1: National Design Specification for Metal

Plate Connected Wood Truss Construction.

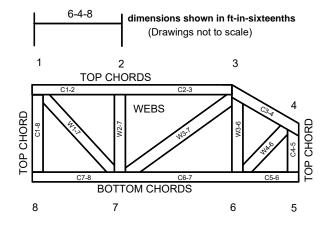
DSB-89:

BCSI:

Guide to Good Practice for Handling.

Connected Wood Trusses.

**Numbering System** 



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

Trusses are designed for wind loads in the plane of the

Lumber design values are in accordance with ANSI/TPI 1

section 6.3 These truss designs rely on lumber values

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

# PRODUCT CODE APPROVA These prints comply with the

truss unless otherwise shown.

established by others.

ICC-ES Reports:

ESR-1311, ESR-1352, ESR19 01 ER-3907, ESR-2362, ESR-139

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Const. Type: VB-unprotected Occupancy: Allowable No of Floors: Fire Rating of Plan No.: Allow. Floor Load: 40 PSF

# **General Safety Notes**

## Failure to Follow Could Cause Property Damage or Personal Injury

- 1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- 2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- 3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- 4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- 6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- 7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- 8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 1.11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- 12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing. or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- 16. Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- 18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- 19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- 20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.



Design Standard for Bracing.

Building Component Safety Information, Installing & Bracing of Metal Plate

MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020



Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: MH7632R11 -

MiTek USA, Inc. 16023 Swingley Ridge Rd Chesterfield, MO 63017

314-434-1200

Site Information:

Customer Info: Franklin Structures, LLC Project Name: . Model: . Subdivision: .

Lot/Block: .

Address: ., .

City: .

State: .

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

License #:

Address:

City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special **Loading Conditions):** 

Design Code: FBC2020/TPI2014 Design Program: MiTek 20/20 8.4

Wind Code: ASCE 7-16 Wind Speed: 168 mph Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 2 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

Truss Name Date No. 146576448 HP127-7 6/15/21 1 2 146576449 HP127-7P 6/15/21



This item has been electronically signed and sealed by Fox, Steve, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Franklin Structures, LLC..

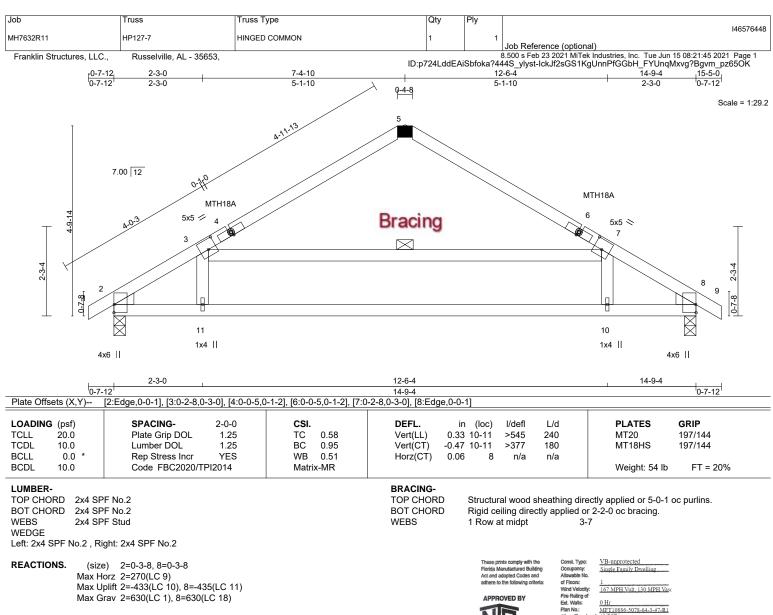
Truss Design Engineer's Name: Fox, Steve

My license renewal date for the state of Florida is February 28, 2023.

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



June 15,2021



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1176/606, 3-5=-358/303, 5-7=-358/300, 7-8=-1176/602 BOT CHORD 2-11=-493/1000, 10-11=-470/1007, 8-10=-470/1000

WEBS 3-11=-131/432, 7-10=-131/431, 3-7=-869/389

REQUIRED FIELD JOINT CONNECTIONS - Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in)

5=305/306/211/0

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=168mph (3-second gust) Vasd=130mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) See HINGE PLATE DETAILS for plate placement.
- 6) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
- 7) All additional member connections shall be provided by others for forces as indicated.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 433 lb uplift at joint 2 and 435 lb uplift at joint 8.

This item has been electronically signed and sealed by Fox, Steve, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Steven E. Fox PE No.44975 MiTek USA, Inc. FL Cert 6634 18023 Swingley Ridge Road, Chesterfield, MO 63017 Date:

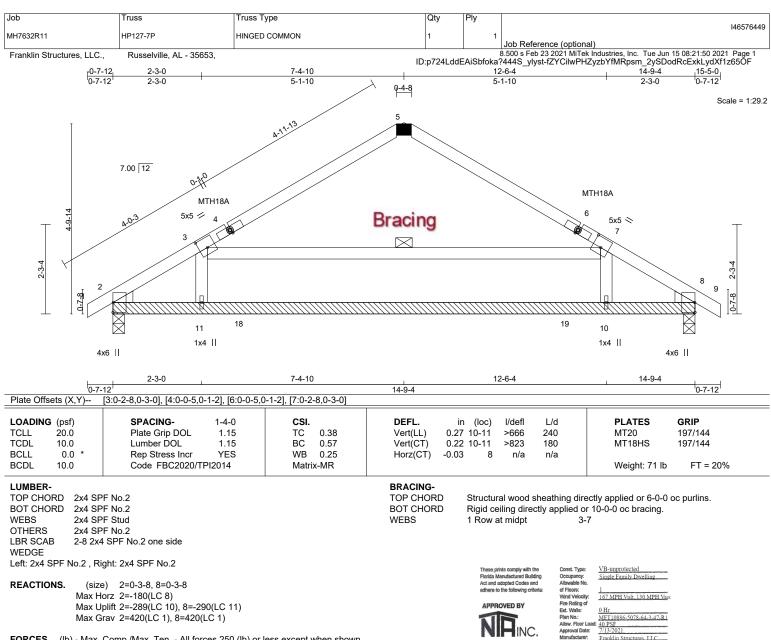
June 15,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

AMSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-650/505, 7-8=-650/501

**BOT CHORD** 2-11=-413/557, 10-11=-414/561, 8-10=-393/557 **WEBS** 3-11=-329/250, 7-10=-329/250, 3-7=-429/358

REQUIRED FIELD JOINT CONNECTIONS

- Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in)

5=203/204/141/0

## NOTES-

1) Attached 14-9-4 scab 2 to 8, front face(s) 2x4 SPF No.2 with 1 row(s) of 10d (0.131"x3") nails spaced 9" o.c.except : starting at 1-3-0 from end at joint 2, nail 1 row(s) at 3" o.c. for 3-0-0; starting at 10-6-4 from end at joint 2, nail 1 row(s) at 3" o.c. for 3-0-0.

2) Unbalanced roof live loads have been considered for this design.

- 3) Wind: ASCE 7-16; Vult=168mph (3-second gust) Vasd=130mph; TCDL=6.0psf; BCDL=6.0psf; h=30ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.33 plate grip DOL=1.33
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) See HINGE PLATE DETAILS for plate placement.
- 7) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
- 8) All additional member connections shall be provided by others for forces as indicated.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 289 lb uplift at joint 2 and 290 lb uplift at joint 8.

This item has been electronically signed and sealed by Fox, Steve, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Steven E. Fox PE No.44975 MiTek USA, Inc. FL Cert 663 16023 Swingley Ridge Road, Chesterfield, MO 63017

June 15,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

AMSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

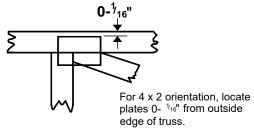


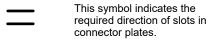
# **Symbols**

### PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.





<sup>\*</sup> Plate location details available in MiTek 20/20 software or upon request.

### **PLATE SIZE**

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

### LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated

## **BEARING**



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Min size shown is for crushing only.

### **Industry Standards:**

ANSI/TPI1: National Design Specification for Metal

Plate Connected Wood Truss Construction.

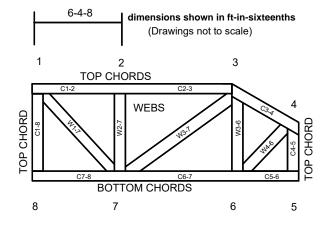
DSB-89: Design Standard for Bracing.

BCSI:

Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate

Connected Wood Trusses.

# **Numbering System**



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

## PRODUCT CODE APPRising Manufactured Building

ICC-ES Reports:

APPROVED BY

Allowable No, of Floors; Wind Velocity: Fire Rating of Ext. Walls: Plan No.: Allow. Floor Load

VB-unprotected

ESR-1311, ESR-1352, EST 17 | No. 12 | Plan No. | Allow. Flor Land | Al

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020



# **General Safety Notes**

# Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- 3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- 7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21.The design does not take into account any dynamic or other loads other than those expressly stated.



Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: MH7281R16-FL20 -

MiTek USA, Inc. 16023 Swingley Ridge Rd

Chesterfield, MO 63017 314-434-1200

Customer Info: Franklin Structures, LLC Project Name: . Model: .

Lot/Block: . Address: ., .

Site Information:

Subdivision: .

City: .

State: .

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

lame: License #:

Address:

City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2020/TPI2014 Design Program: MiTek 20/20 8.4

Wind Code: ASCE 7-16 Wind Speed: 167 mph Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 2 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No. Seal# Truss Name Date 1 I45076586 A1 3/5/21 2 I45076587 A1P 3/5/21

> These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:

APPROVED BY

| 2001.1. Type: | VB-umprotected | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2002.00 | 2



This item has been electronically signed and sealed by Galinski, John, PE using a Digital Signature.

Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Franklin Structures, LLC..

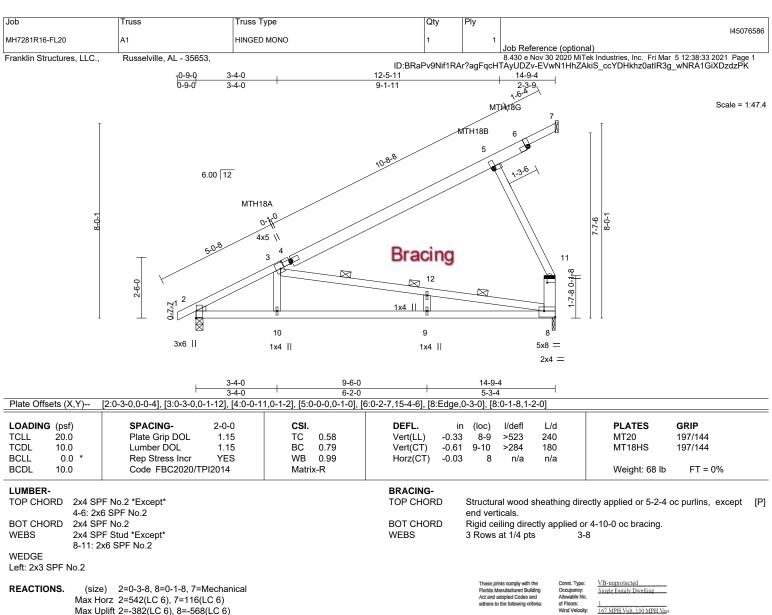
Truss Design Engineer's Name: Galinski, John

My license renewal date for the state of Florida is February 28, 2023.

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



March 5,2021



Max Grav 2=667(LC 1), 8=552(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1130/416, 3-5=-341/70, 8-11=-299/477 TOP CHORD **BOT CHORD** 2-10=-833/874, 9-10=-833/874, 8-9=-833/874

**WEBS** 3-10=0/384, 3-12=-740/618, 8-12=-745/609, 5-11=-332/529

# REQUIRED FIELD JOINT CONNECTIONS

- Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in)

11=332/529/229/0

## NOTES-

- 1) Wind: ASCE 7-16; Vult=167mph (3-second gust) Vasd=129mph; TCDL=6.0psf; BCDL=6.0psf; h=24ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) See HINGE PLATE DETAILS for plate placement.
- 5) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
- 6) All additional member connections shall be provided by others for forces as indicated.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 9) Refer to girder(s) for truss to truss connections
- 10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 8.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 382 lb uplift at joint 2 and 568 lb uplift at ioint 8.

This item has been electronically signed and sealed by Galinski, John, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

John L Galinski PE No.60642 MiTek USA, Inc. FL Cert 6634 16023 Swingley Ridge Road, Chesterfield, MO 63017

March 5,2021

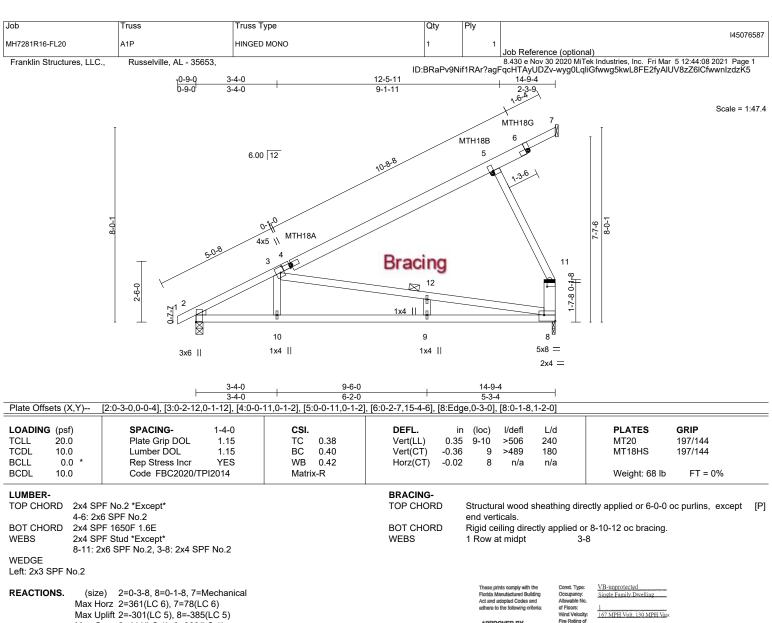


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chorembers only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see

\*\*AMSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information\*\* available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Max Grav 2=444(LC 1), 8=368(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-753/525, 8-11=-200/318

**BOT CHORD** 2-10=-642/583, 9-10=-642/583, 8-9=-642/583

**WEBS** 3-10=-179/254, 3-12=-493/562, 8-12=-496/568, 5-11=-221/352

REQUIRED FIELD JOINT CONNECTIONS 11=221/352/152/0

- Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in)

# NOTES-

- 1) Wind: ASCE 7-16; Vult=167mph (3-second gust) Vasd=129mph; TCDL=6.0psf; BCDL=6.0psf; h=24ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to
- the use of this truss component.
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) See HINGE PLATE DETAILS for plate placement.
- 5) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
- 6) All additional member connections shall be provided by others for forces as indicated.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 9) Refer to girder(s) for truss to truss connections
- 10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 8.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 301 lb uplift at joint 2 and 385 lb uplift at ioint 8.

This item has been electronically signed and sealed by Galinski, John, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

John L Galinski PE No.60642 MiTek USA, Inc. FL Cert 6634 16023 Swingley Ridge Road, Chesterfield, MO 63017

March 5,2021

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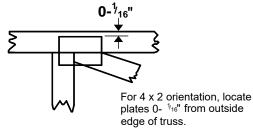


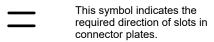
# **Symbols**

### PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.





<sup>\*</sup> Plate location details available in MiTek 20/20 software or upon request.

### **PLATE SIZE**

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

### LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated

## **BEARING**



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

### **Industry Standards:**

ANSI/TPI1: National Design Specification for Metal

Plate Connected Wood Truss Construction.

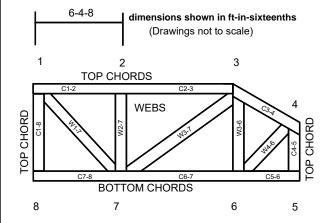
DSB-89: Design Standard for Bracing.

BCSI:

Building Component Safety Information, Guide to Good Practice for Handling,

Installing & Bracing of Metal Plate Connected Wood Trusses.

# **Numbering System**



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

## PRODUCT CODE APPROVAM and adopted Codes and

ICC-ES Reports:

These prints comply with the Florida Manufactured Building Old and adopted Codes and adhere to the following criteria:

Occupancy:
Allowable No.
of Floors:
Wind Velocity:
Fire Rating of
Ext. Walls:
Plan No.:
Allow. Floor Load:

Const. Type:

VB-unprotected
Single Family Dwelling

1
167 MPH Vult, 130 MPH V
0 Hr

ESR-1311, ESR-1352, ESF 115 Allow Floor Load ER-3907, ESR-2362, ESR-1311, ESR-3282/Manufacturer:

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020



# **General Safety Notes**

# Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- 3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- 7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21.The design does not take into account any dynamic or other loads other than those expressly stated.



Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: MH7303R29 -

Site Information:

MiTek USA, Inc. 16023 Swingley Ridge Rd Chesterfield, MO 63017

Customer Info: Franklin Structures, LLC Project Name: . Model: .

Lot/Block: . Subdivision: .

Address: ., .

City: .

314-434-1200

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

State: .

License #:

Address:

City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special **Loading Conditions):** 

Design Code: FBC2020/TPI2014 Design Program: MiTek 20/20 8.4

Wind Code: ASCE 7-16 Wind Speed: 168 mph Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 2 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

Truss Name Date No. Seal# 146576446 HM177-7 6/15/21 1 2 146576447 HM177-7P 6/15/21



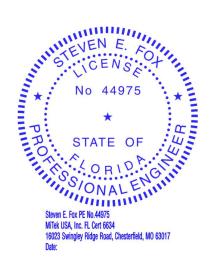
This item has been electronically signed and sealed by Fox, Steve, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Franklin Structures, LLC..

Truss Design Engineer's Name: Fox, Steve

My license renewal date for the state of Florida is February 28, 2023.

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



June 15,2021

Job Truss Truss Type Qty 146576446 MH7303R29 HM177-7 HINGED MONO Job Reference (optional) 8.500 s Feb 23 2021 MITek Industries, Inc. Tue Jun 15 08:23:07 2021 Page 1 ID:Z2VuxlyvW\_aZ8LgBRailsNzqsGA-uNiJs4si8Df1KN2ASUkqbHeZgru69xn42Qn6s3z65N2 Franklin Structures, LLC., Russelville, AL - 35653, -0-7-12 0-7-12 2-2-9 14-9-4 2-2-9 12-6-11 1-11-12 Scale = 1:53.7 MTH18G MTH18B 7.00 12 MTH18A Bracing 3 14

> 7-4-10 14-9-4

9

5x5 =

1x4 |

4x4

10

1x4 ||

Plate Off	Plate Offsets (X,Y)- [3:0-2-8,0-2-12], [4:0-0-11,0-1-2], [5:0-0-11,0-1-2], [6:0-2-5,15-6-6], [8:0-1-8,1-2-0], [8:0-3-0,0-3-0], [9:0-2-8,0-3-0]											
LOADIN	G (psf)	SPACING-	1-4-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.52	Vert(LL)	-0.36	`8 <b>-</b> 9	>480	240	MT20	197/144
TCDL	10.0	Lumber DOL	1.25	ВС	0.59	Vert(CT)	-0.47	8-9	>372	180	MT18HS	197/144
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.72	Horz(CT)	-0.02	8	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-MR						Weight: 80 lb	FT = 0%

**BRACING-**

TOP CHORD

**BOT CHORD** 

**JOINTS** 

LUMBER-

TOP CHORD 2x4 SPF No.2 \*Except\*

4-6: 2x6 SP No.2 2x4 SPF No.2

**BOT CHORD WEBS** 2x4 SPF No.2 \*Except\*

8-13: 2x6 SP No.2

WEDGE

NOTES-

Left: 2x4 SPF No.2

REACTIONS. (size) 7=Mechanical, 8=0-1-8, 2=0-1-8

Max Horz 7=93(LC 10), 2=419(LC 10) Max Uplift 8=-409(LC 10), 2=-230(LC 10) Max Grav 8=457(LC 17), 2=483(LC 17)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-732/140, 3-5=-284/57, 8-13=-313/370

**BOT CHORD** 2-11=-581/791, 10-11=-554/786, 9-10=-543/781, 8-9=-543/781

**WEBS** 3-11=-75/389, 12-14=-608/345, 8-14=-611/333, 3-12=-610/346, 5-13=-360/425

REQUIRED FIELD JOINT CONNECTIONS - Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in)

## 13=360/425/209/0

1) Wind: ASCE 7-16; Vult=168mph (3-second gust) Vasd=130mph; TCDL=6.0psf; BCDL=6.0psf; h=24ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

11

1x4 |

5x7 ||

- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) See HINGE PLATE DETAILS for plate placement. See HINGE PLATE DETAILS for plate placement.
- 5) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
- 6) All additional member connections shall be provided by others for forces as indicated.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 8, 2.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 409 lb uplift at joint 8 and 230 lb uplift at joint 2.

1 Brace at Jt(s): 12, 13, 14

8 5x7 =

2x4 =

Sheathed or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 6-1-3 oc bracing.

VB-unprotected

This item has been electronically signed and sealed by Fox, Steve, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Steven E. Fox PE No.44975 MiTek USA, Inc. FL Cert 6634 16023 Swingley Ridge Road, Chesterfield, MO 63017

June 15,2021

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ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job Truss Truss Type Qty 146576447 MH7303R29 HM177-7P HINGED MONO Job Reference (optional) 8.500 s Feb 23 2021 MiTek Industries, Inc. Tue Jun 15 08:23:27 2021 Page 1 ID:Z2VuxlyvW\_aZ8LgBRailsNzqsGA-JEvt3w5FRNACjSa0dh5WPVTxbvgSr?x0eYdAYvz65Mk Franklin Structures, LLC., Russelville, AL - 35653, -0-7-12 0-7-12 2-2-9 14-9-4 2-2-9 12-6-11 1-11-12 Scale = 1:53.7 MTH18G 6 MTH18B 7.00 12 MTH18A 5x5 / Bracino 204 13 3 19 4x4 1x4 | 11 10 9 8 5x7 = 1x4 || 1x4 | 5x5 = 5x7 || 3x4 = 2-2-9 5-3-1 7-4-10 14-9-4 Plate Offsets (X,Y)--[3:0-2-8,0-2-12], [4:0-0-11,0-1-2], [5:0-0-11,0-1-2], [6:0-2-5,15-6-6], [8:0-1-7,1-0-15], [8:0-3-0,0-3-0], [9:0-2-8,0-3-0]

DEFL.

Vert(LL)

Vert(CT)

Horz(CT)

**BRACING-**

**JOINTS** 

TOP CHORD

**BOT CHORD** 

LUMBER-

TCLL

TCDL

**BCLL** 

**BCDL** 

LOADING (psf)

20.0

10.0

10.0

0.0

TOP CHORD 2x4 SPF No.2 \*Except\*

4-6: 2x6 SP No.2 2x4 SPF No.2

BOT CHORD **WEBS** 2x4 SPF No.2 \*Except\* 8-13: 2x6 SP No.2 OTHERS 2x4 SPF No.2

LBR SCAB 8-12 2x4 SPF No.2 one side

WEDGE

Left: 2x4 SPF No.2

REACTIONS. (size) 7=Mechanical, 8=0-1-8, 2=0-1-8

Max Horz 7=93(LC 10), 2=419(LC 10) Max Uplift 8=-409(LC 10), 2=-272(LC 7) Max Grav 8=361(LC 1), 2=447(LC 1)

SPACING-

Plate Grip DOL

Rep Stress Incr

Code FBC2020/TPI2014

Lumber DOL

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-698/1589, 3-5=-284/57, 8-13=-313/370

BOT CHORD

2-11=-1678/572, 10-11=-1722/571, 9-10=-1743/571, 8-9=-1743/571

WFBS 3-11=-645/368, 12-14=-492/1538, 8-14=-512/1587, 3-12=-493/1540, 9-14=-415/166,

1-4-0

1 25

1.25

YES

CSI.

TC

BC

WB

Matrix-MR

0.65

0.81

0.70

5-13=-360/425

se prints comply with the Act and adopted Codes and

I/defl

>259

>302

n/a

1 Brace at Jt(s): 12, 13, 14

(loc)

8-9

8-9

8

0.67

0.58

-0.03

L/d

240

180

n/a

Sheathed or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 2-11-7 oc bracing.

APPROVED BY

of Floors: Wind Velo

**PLATES** 

MT18HS

Weight: 90 lb

MT20

GRIP

197/144

197/144

FT = 0%

VB-unprotected 167 MPH Vult. 130 MPH Vas MFT10886-5078-64-3-47-R1

# REQUIRED FIELD JOINT CONNECTIONS

- Maximum Compression (lb)/ Maximum Tension (lb)/ Maximum Shear (lb)/ Maximum Moment (lb-in)

13=360/425/209/0

## NOTES-

- 1) Attached 9-1-12 scab 8 to 12, front face(s) 2x4 SPF No.2 with 1 row(s) of 10d (0.131"x3") nails spaced 9" o.c.except: starting at 5-11-10 from end at joint 8, nail 1 row(s) at 7" o.c. for 3-1-11.
- 2) Wind: ASCE 7-16; Vult=168mph (3-second gust) Vasd=130mph; TCDL=6.0psf; BCDL=6.0psf; h=24ft; Cat. II; Exp C; Encl.. GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-7-12 to 2-4-4, Interior(1) 2-4-4 to 14-8-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) See HINGE PLATE DETAILS for plate placement. See HINGE PLATE DETAILS for plate placement.
- 6) Provisions must be made to prevent lateral movement of hinged member(s) during transportation.
- 7) All additional member connections shall be provided by others for forces as indicated.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 11) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 8, 2.

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Steven E. Fox PE No.44975 MiTek USA, Inc. FL Cert 663 16023 Swingley Ridge Road, Chesterfield, MO 63017

June 15,2021

### nued on page 2

## 🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	
MH7303R29	LIM477 7D	HINGED MONO			146576447
MH7303R29	HM1//-/P	HINGED MONO	1	'	Job Reference (optional)

Russelville, AL - 35653,

8.500 s Feb 23 2021 MiTek Industries, Inc. Tue Jun 15 08:23:27 2021 Page 2 ID:Z2VuxlyvW\_aZ8LgBRailsNzqsGA-JEvt3w5FRNACjSa0dh5WPVTxbvgSr?x0eYdAYvz65Mk

### NOTES-

12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 409 lb uplift at joint 8 and 272 lb uplift at joint 2.

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criteria:

VB-unprotected Single Family Dwelling 167 MPH Vult, 130 MPH Vase 0 Hr MFT10886-5078-64-3-47-R1 40 PSF 7/13/2021 Franklin Structures, LLC

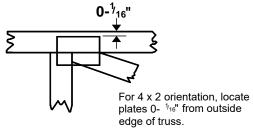


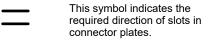
# **Symbols**

### PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths.
Apply plates to both sides of truss and fully embed teeth.





<sup>\*</sup> Plate location details available in MiTek 20/20 software or upon request.

### **PLATE SIZE**

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

### LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated

## **BEARING**



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

### **Industry Standards:**

ANSI/TPI1: National Design Specification for Metal

Plate Connected Wood Truss Construction.

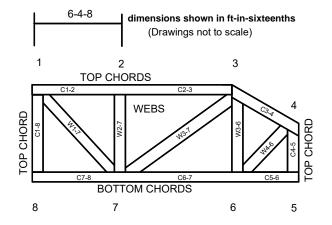
DSB-89: Design Standard for Bracing.

BCSI:

Building Component Safety Information, Guide to Good Practice for Handling,

Installing & Bracing of Metal Plate Connected Wood Trusses.

# **Numbering System**



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

### PRODUCT CODE APPROVALS

ICC-ES Reports:

These prints comply with the Florida Manufactured Building Act and adopted Codes and

ESR-1311, ESR-1352, ESR1988 of Flooring orderia: of Flooring Control of Flooring Contr

Occupancy:
Allowable No.
of Floors:
Wind Velocity:
Tips Rating of
Allowable No.
O. H.

0 Hr MFT10886-5078-64-3-47-F oad: 40 PSF o: 7/13/2021 Franklin Structures LLC

VB-unprotected

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 5/19/2020



# **General Safety Notes**

# Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- 7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- 8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- 13. Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21.The design does not take into account any dynamic or other loads other than those expressly stated.

These prints comply with the Florida Manufactured Building Act and adopted Codes and adhere to the following criterion



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Single Family Dwelling

1
167 MPH Vult. 130 MPH Vas
0 Hr
MFT10886-5078-64-3-47-R1
ab: 40 PSF

# PRODUCT APPROVAL SPECIFICATION SHEET

Manufacturer:	Franklin Structures, L	LC. Plan #: N	MFT10866-5078-64-3-47-R1		
and the product approval number manufactured building for which product supplier should you not l	er(s) on the building compon you are applying for a DBPI know the product approval r	rative Code 61G20-3.006 please prents listed below if they will be utilized insignia. We recommend you contumber for any of the applicable lister betained at www.floridabuilding.org.	red on the ntact your local		
Category	Manufacturer	Product Description	Approval #(s)		
EXTERIOR DOORS					
Swinging	Dunbarton	Achiever Steel Frame	FL-9231-R		
Sliding	Jeld-Wen	Sliding Glass Door	FL11109.2-R		
Swinging	Dunbarton	ATRUIM Frame	15362.1-R3		
	•	2.3,15362.9, 15362.12,			
Sliding	VIWINCO	Sliding Glass Door	FL15590.1-R		
WINDOWS					
Single Hung	KINRO	WINDOWS	FL993.2/993.5 R-1		
ROOFING PRODUCTS					
Asphalt Shingles	Tamko Shingles	Classic 3-Tab	FL18355-R		
Underlayments	Owens Corning	Oakridge	FL17420.1-R		
PANEL WALL	g	- Continuing			
Siding	ROYAL BUILDIN	NG PRODUCTS(D45DLSTD )	FL21069-R		
Soffits		ARDIESOFFIT	FL13265-R		
STRUCTURAL COMPONENTS		ANDIESOFFII	1 L 13203-11		
Wood Connector / Anchor	Simpson	HDU11-11473.8	FL10441R		
Wood Connector / Anchor	Simpson	HD3B	FL11496-R		
Wood Connector / Anchor	Simpson	HDQ8	FL10441-F		
Wood Connector / Anchor	Simpson	CMSTC16	FL13872- R4		
Wood Connector / Anchor	Simpson	STHD14	FL10441.12		
Wood Connector / Anchor	Simpson	LSTA18	FL10456.1		
Truss Plates	MiTek	MT18 & MT20	FL2197-R		
Engineered Lumber	Weyerhauser	Microllam LVL	FL6527.1- R1		
Engineered Lumber	VERSA-LAM	Microllam LVL	FL1644-R9		
of inspection of these product manufacturing plant: (1) Copy of the information listed on For requirements.	ts, the following information of the product approval from No. 9B-72.130(5). (2) may have to be removed if	approval at plan review. I underst n must be available to the inspect rom the Local or State Building Co Copy of the applicable manufactu approval cannot be demonstrate CHRISTIE JACKSON	tor at the ommission, or supply all ırers' installation		
Manufacturer's Authorized Age		Printed Name	Date		