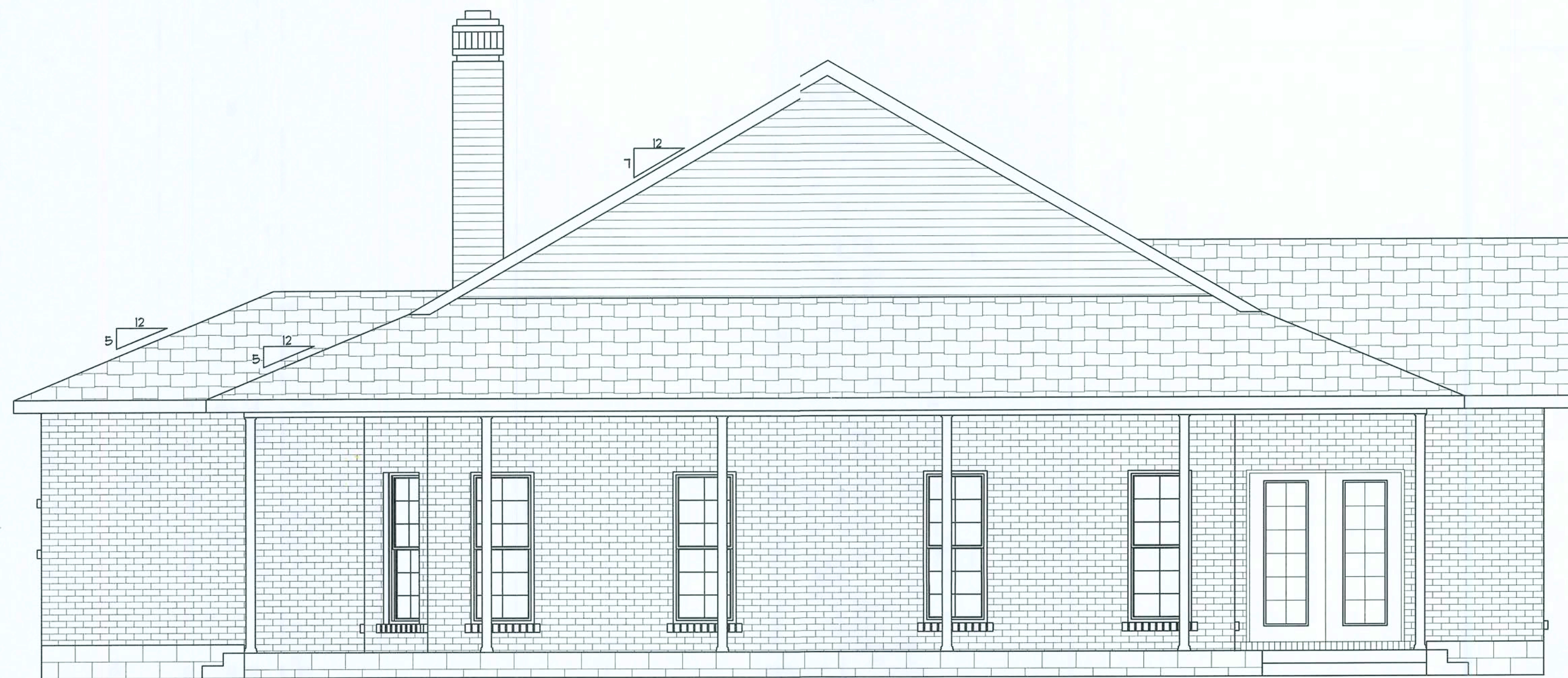
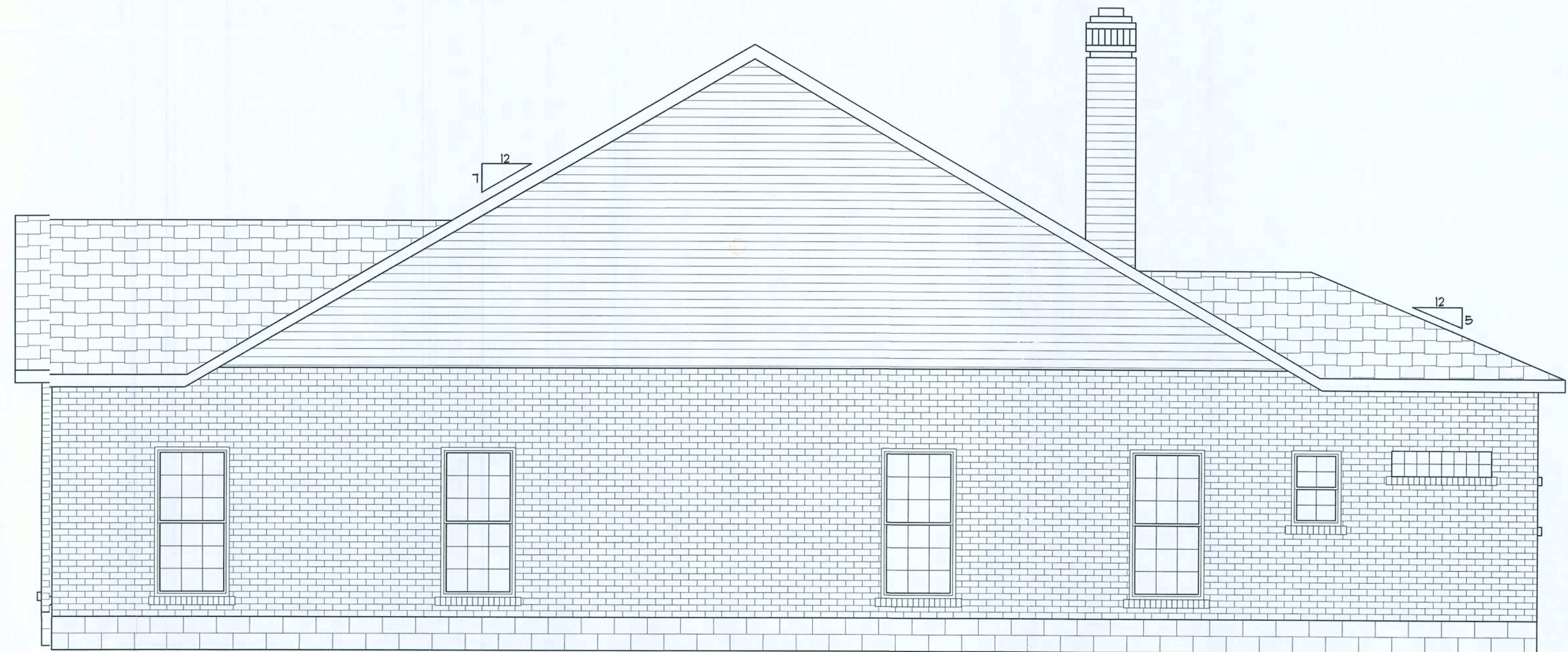




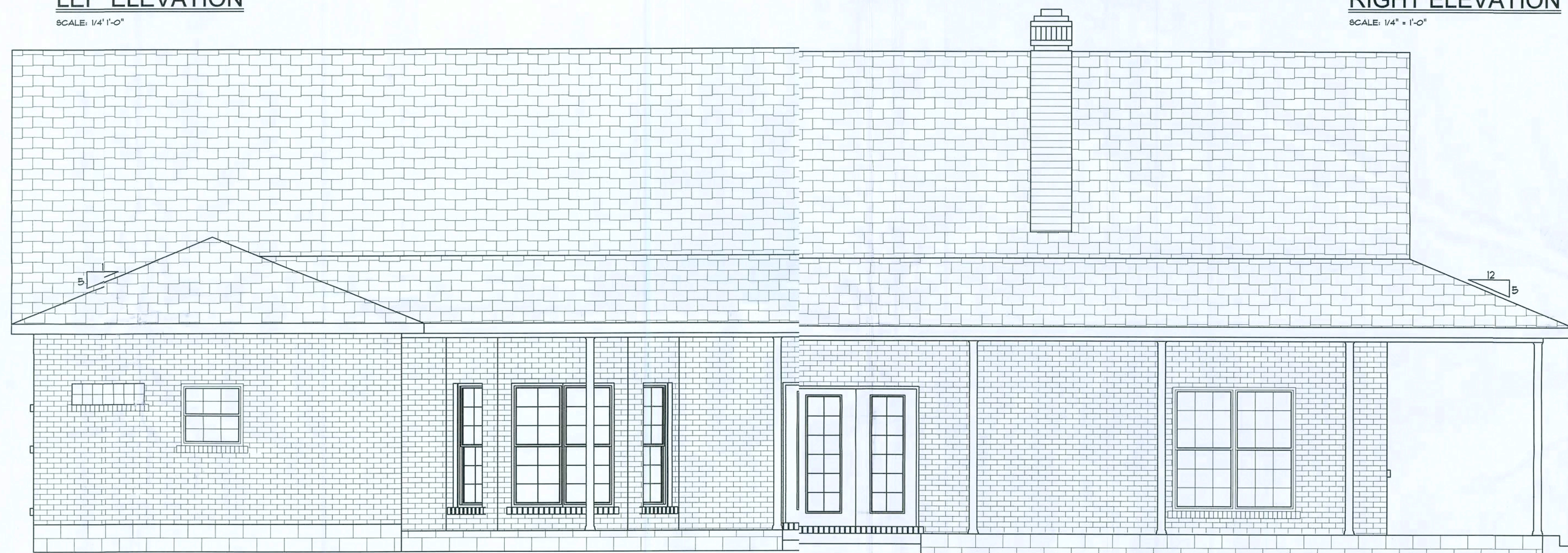
LEF ELEVATION

SCALE: 1/4" = 1'-0"



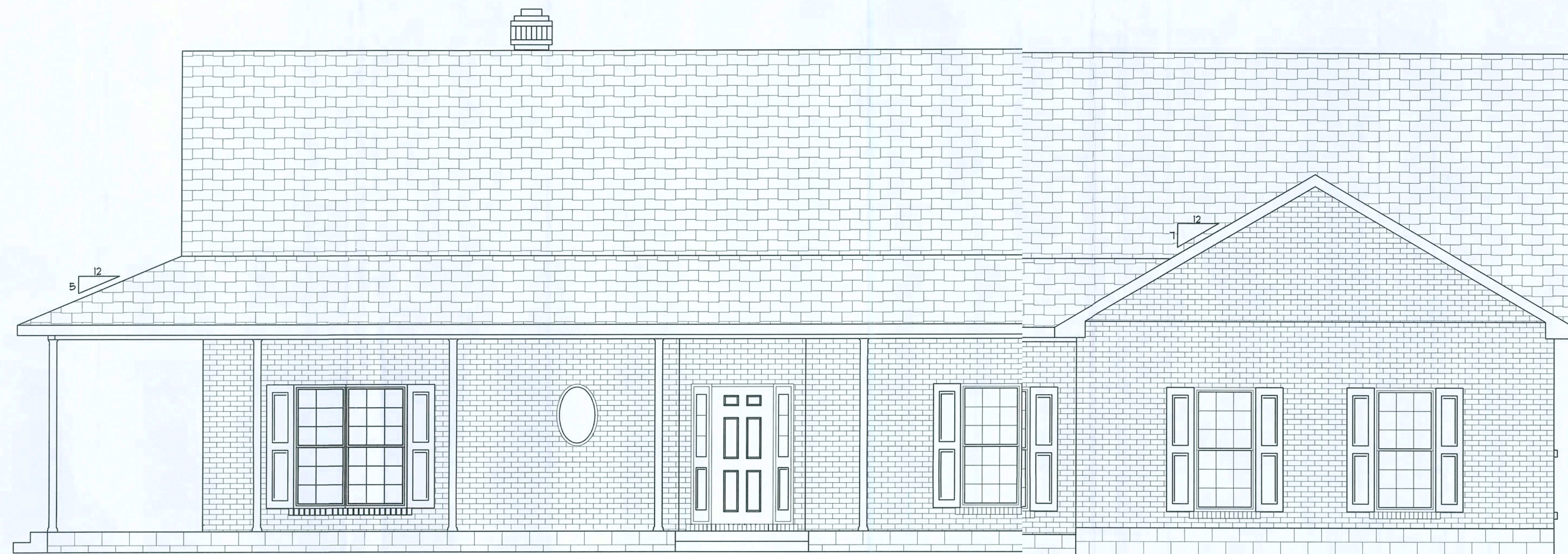
RIGHT ELEVATION

SCALE: 1/4" = 1'-0"



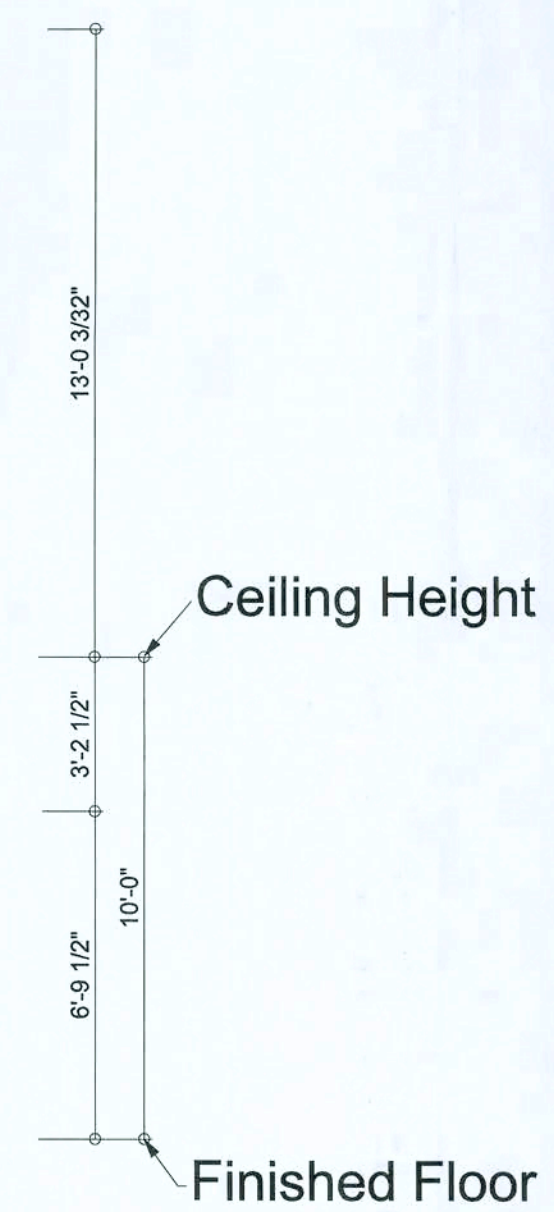
REAR ELEVATION

SCALE: 1/4" = 1'-0"



FRONT ELEVATION

SCALE: 1/4" = 1'-0"



AREA SUMMARY

LIVING AREA	2593.2 SF
PORCHES	1086.2 SF
TOTAL AREA	3679.4 SF

ELEVATIONS

1725

Building
Dept.

NEW SINGLE FAMILY DWELLING FOR:

Mike Kuznik

Tustenuggee Woods, Lot 10

Jason Parker
Architectural design

DESIGNER: Jason Parker
PHONE: (386) 867-2660

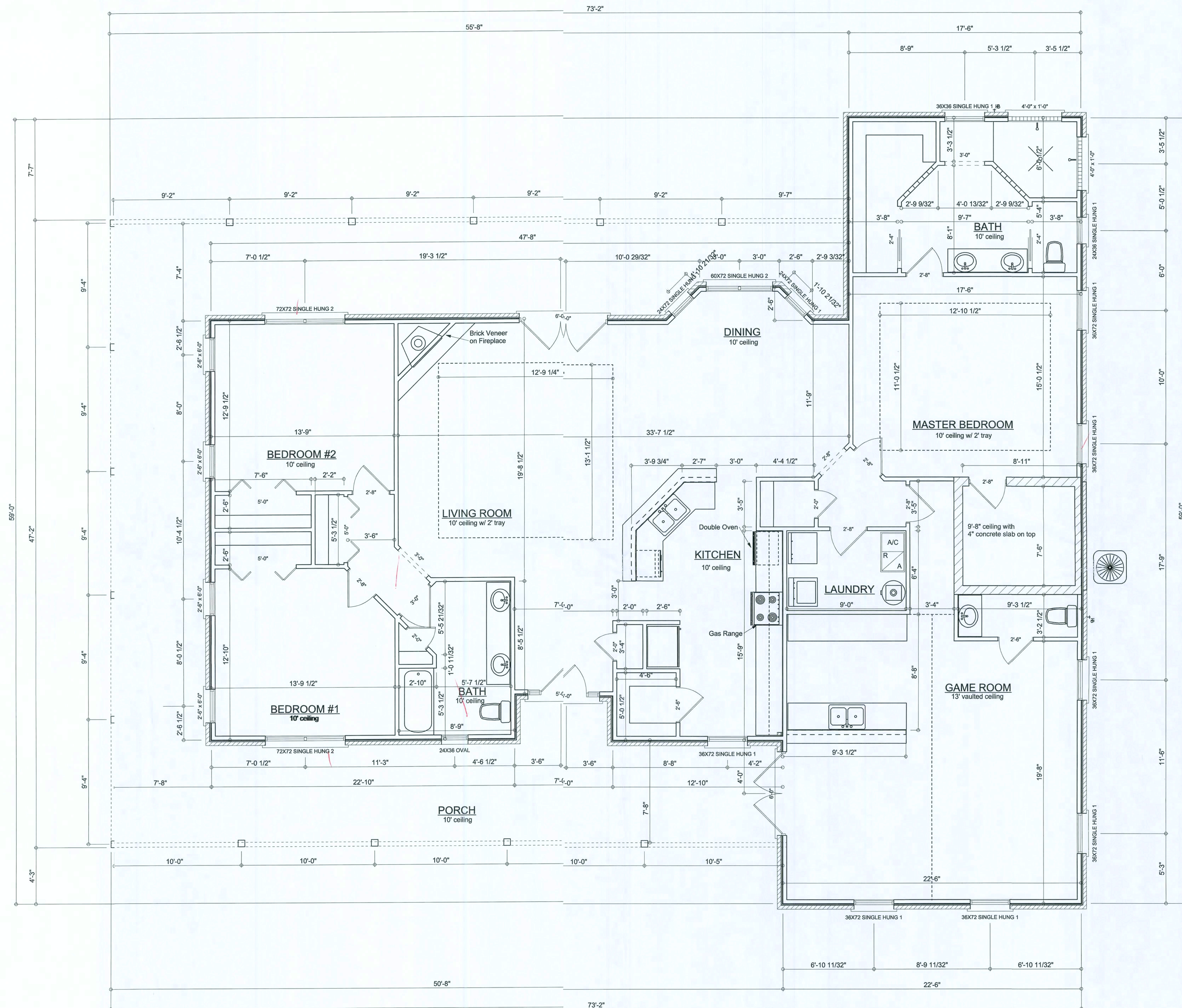
DATE:

CHECKED BY:

SHEET NUMBER

A-1

OF 4 SHEETS



MAIN FLOOR PLAN
SCALE: 1/4" = 1'-0"

AREA SUMMARY	
LIVING AREA	2593.2 SF
PORCHES	1086.2 SF
TOTAL AREA	3679.4 SF

MAIN FLOOR PLAN

NEW SINGLE FAMILY DWELLING FOR:
Mike Kuznik
Tustenuggee Woods, Lot 10

Jason Parker
Architectural design
DESIGNER: Jason Parker
PHONE: (386) 867-2660

DATE:

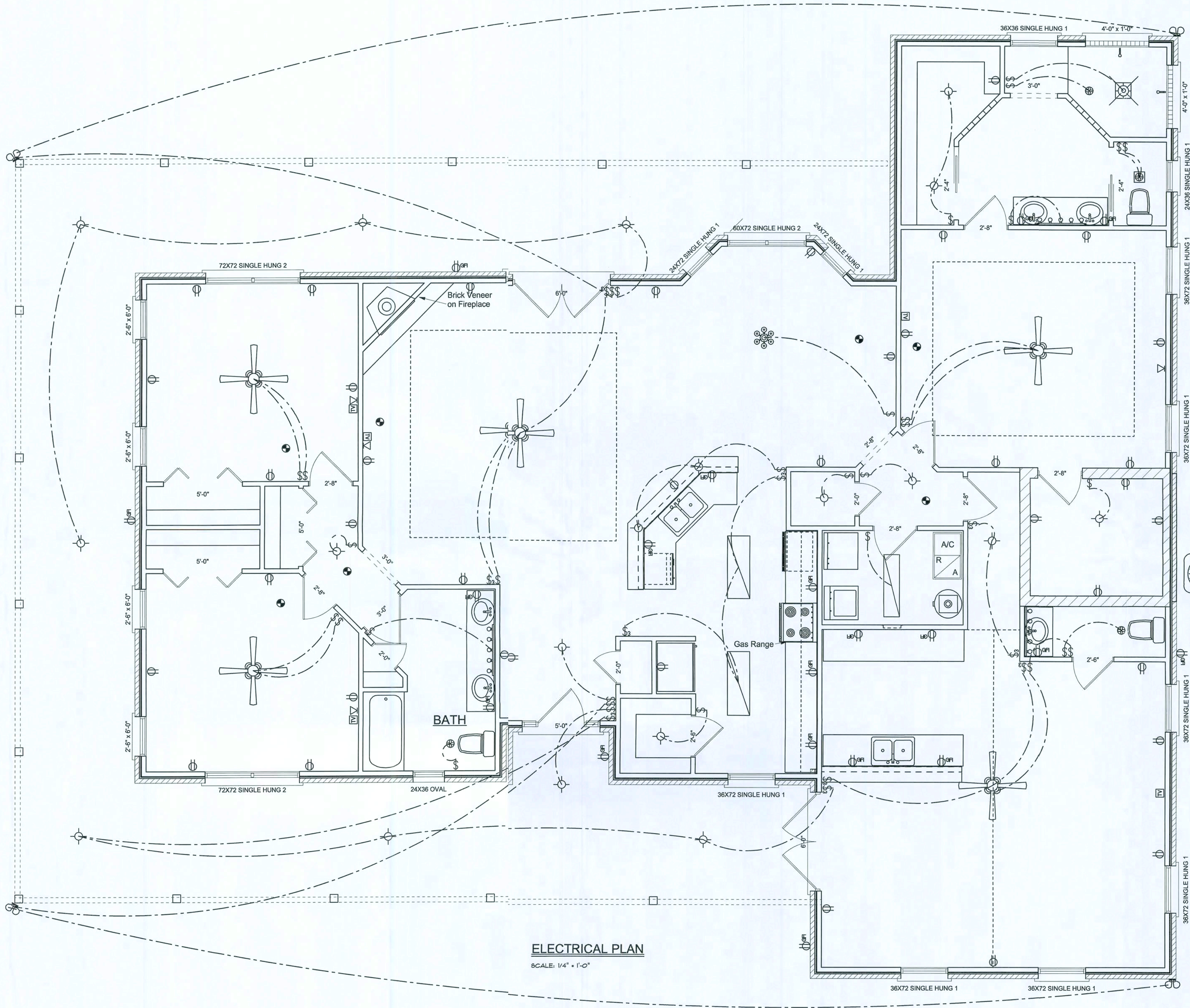
CHECKED BY:

SHEET NUMBER

A-2

OF 4 SHEETS

ELECTRICAL	COUNT	SYMBOL
ceiling fan spotlights 2	5	
ceiling lamp large	2	
ceiling lamp small	1	
Can Light	1	
chandelier	1	
double spotlight	4	
vanity bar light	3	
electrical panel	1	
Fan - Light Combo	1	
cable tv outlet	5	
fan	3	
light	19	
outlet	37	
outlet gfi	19	
smoke detector	7	
switch	31	
switch 3 way	10	
telephone	4	



- ELECTRICAL PLAN NOTES**
- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPARATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
- E -10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL.

AREA SUMMARY	
LIVING AREA	2593.2 SF
PORCHES	1086.2 SF
TOTAL AREA	3679.4 SF

ELECTRICAL PLAN

NEW SINGLE FAMILY DWELLING FOR:
Mike Kuznik
Tustenuggee Woods, Lot 10

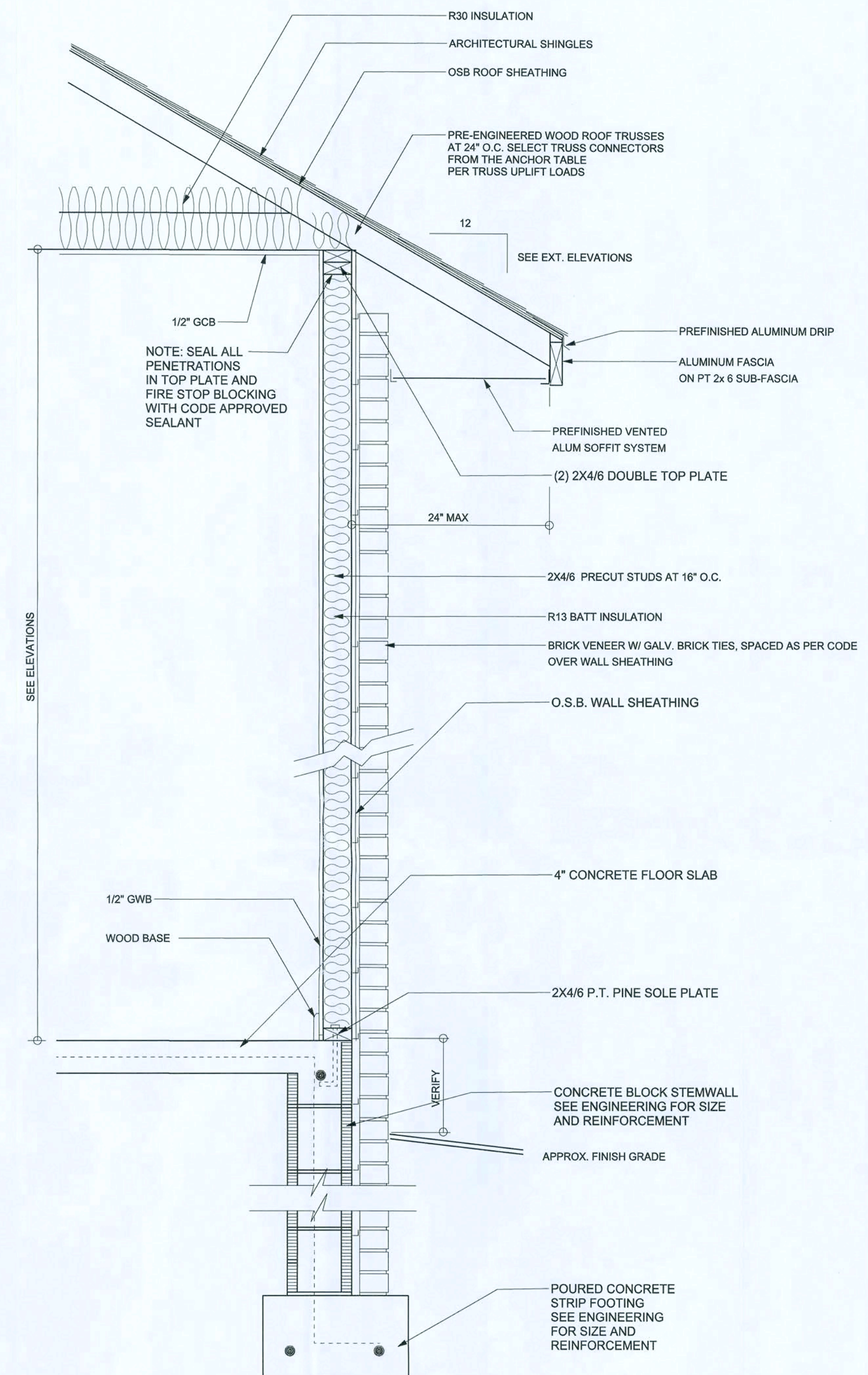
Jason Parker
P.L.L.C.
DESIGNER: Jason Parker
PHONE: (386) 867-2680

DATE:
CHECKED BY:

SHEETNUMBER
A-3
OF 4 SHEETS



ROOF PLAN
SCALE: 1/4" = 1'-0"



TYPICAL DESIGN WALL SECTION
NON - STRUCTURAL DATA
SCALE: 1" = 1'-0"

AREA SUMMARY	
LIVING AREA	2593.2 SF
PORCHES	1086.2 SF
TOTAL AREA	3679.4 SF

ROOF PLAN AND
TYPICAL WALL SECTION

NEW SINGLE FAMILY DWELLING FOR:
Mike Kuznik
Tustenuggee Woods, Lot 10

Jason Parker
Architectural design
DESIGNER: Jason Parker
PHONE: (386) 867-2660

DATE:

CHECKED BY:

SHEET NUMBER

A-4

OF 4 SHEETS

REVISIONS



ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H8	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LG12	14-16d	14-16d	
HEAVY GIRDER TIEDOWNS*					TO FOUNDATION
< 3965	< 3330	MG1		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE BILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE BILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SP44			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SP16			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*					TO FOUNDATION
< 1350	< 1305	LTT119		8-16d	1/2" AB
< 2310	< 2310	LTT131	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	H02A		2-5/8" BOLTS	5/8" AB
< 4175	< 3695	H1116	18-16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18-16d		2-5/8" AB

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, TRUSS CONNECTIONS, UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED AND SEALED BY THE TRUSS MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REACTION LOADS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 3000$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1 x 4 W1 A, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, $F_y = 60$ KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, $F_b = 2.4$ ksi, $E = 1800$ ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS. **ROOF SHEATHING:** ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 6 COMMON NAILS (13D), 6" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS. GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE BEAM OR 12" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 3/16" WITH 5/8" BOLTS TO BE 3" x 3" x 3/16" WITH 3/4" BOLTS TO BE 3" x 3" x 3/16". WITH 7/8" BOLTS TO BE 3" x 3" x 3/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, TRUSS CONNECTIONS, UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL MOVEMENT. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls $F_m = 1500$ psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 60, $F_y = 60$ ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft ² or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft ² or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50' HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = B
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = <30 FT
- INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft ²)	
1	19.9 -21.8	18.1 -18.1
2	19.9 -25.5	18.1 -21.8
2 On g		40.6 -40.6
3	19.9 -25.5	18.1 -21.8
3 On g		42.4 -42.4
4	21.8 -23.6	18.5 -20.4
5	21.8 -29.1	18.5 -22.6
Doors & Windows Worst Case (Zone 5, 10 ft ²)		21.8 -29.1
8x7 Garage Door		19.5 -22.9
16x7 Garage Door		18.5 -21.0

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)
30 PSF (SLEEPING ROOMS)
30 PSF (ATTICS WITH STORAGE)
10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF 20 PSF (FLAT OR <4:12)
16 PSF (4:12 TO <12:12)
12 PSF (12:12 AND GREATER)
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY 1000PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)

High Springs Construction

Mke Kuznik Residence

ADDRESS:
Lot 10 Tstunegue Woods S/D
Columbia County, Florida

Mark Disoway P.E.
P.O. Box 888
Lake City, Florida 32056
Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
November 28, 2007

DRAWN BY: David Disoway

CHECKED BY:

FINALS DATE:
28 / Nov / 07

JOE NUMBER:
711021

DRAWING NUMBER

S-1

OF 3 SHEETS

7/16" OSB ROOF SHEATHING UNLOCKED
NAILED TO ROOF FRAMING 8d COMMON NAILS
6" O.C. EDGES, 12" O.C. FIELD, 4" O.C. GABLES

PRE-ENGINEERED WOOD ROOF TRUSSES
AT 24" O.C. SELECT TRUSS CONNECTORS
FROM THE ANCHOR TABLE
PER TRUSS UPLIFT LOADS

(2) 2x4x6 SPF#2 DOUBLE TOP PLATE

SPH4/6 @ 48" O.C.

24" MAX

NOTE: SEAL ALL PENETRATIONS IN TOP PLATE AND FIRE STOP BLOCKING WITH CODE APPROVED SEALANT

NOTE: IF TRUSS BEARING LOAD EXCEEDS 425 PSF USE SYP #2 TOP PLATES; IF IT EXCEEDS 565 PSF AND ADDITIONAL BEARING BLOCKS OR USE SIMPSON TRUSS BEARING ENHANCER

2x4x6 SPF#2 PRECUT STUDS AT 16" O.C. SEE STUD TABLE

7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD UNLESS OTHERWISE NOTED ON STRUCTURAL PLAN

4" CONCRETE FLOOR SLAB REINFORCED WITH 10x6-14/14 WELDED WIRE MESH PLACED ON CHAIRS AT 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6dAIL POLY VAPOR BARRIER WITH 6" LAPS SEALED WITH POLY TAPE OVER TERMI-TREATED AND COMPACTED FILL

2x4x6 P.T. PINE SOLE PLATE ANCHORED WITH 1/2"x10" ANCHOR BOLTS WITH 2X2X140" STEEL WASHER 48" O.C. & 8" FROM CORNERS

SPH4/6 @ 48" O.C.

6" MIN.

FINISH GRADE

ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER FBCR 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

TERIOR BEARING WALL

SCALE: 1/2" = 1'-0"

NOTE: IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEAD OF THE SPH4/6 @ 48" O.C. ARE NOT REQUIRED

(6) 131 x 3 1/4" GUN NAILS TOE NAILED THRU SILL INTO KING STUD

SPH4/6 ALL OPENINGS (U.N.O.)

(6) 131 x 3 1/4" GUN NAILS TOE NAILED THRU HEADER INTO KING STUD

SPH4/6 @ 48" O.C. (U.N.O.)

(4) 131 x 3 1/4" GUN NAILS TOE NAILED THRU SILL INTO JACK STUD U.N.O.

NOTE: TYPICAL STRAPPING (U.N.O.) (SEE STRUCTURAL PLAN)

SPH4/6 ALL OPENINGS (U.N.O.)

(1) 2X6 SPF #2 SILL UP TO 11'-0" U.N.O.

(1) 2X4 SPF #2 SILL UP TO 7'-3" U.N.O. (FOR: 110 MPH, 10'-0" WALL HEIGHT U.N.O.)

TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

PRE-ENGINEERED ROOF TRUSS

DOUBLE 2x4 SPF TOP PLATE NAILED TOGETHER W/2-16d NAILS AT 16" O.C. 4" MIN. LAP w/ (12) - 16d OR 4" LAP w/ CS20 w/ (4) - 16d & (14) - 10d

INTERIOR CEILING AS SPECIFIED ON FLOOR PLAN

CONTINUOUS FRAME TO TOP PLATE AT BOTTOM CHORD OF TRUSS

ALL STUDS TO BE 2x4 SPF NAILED TO TOP AND BOTTOM PLATES WITH 2-16d NAILS

SYP #2 HEADER

(4) #10X2 1/2" FULL THREAD WOOD SCREWS (MIN. 2" THREAD PENETRATION MIN. 3/8" EDGE SPACING PER NDS

(4) #12X3/4" SS TEK SCREWS

1/8"x3"x3"x1 1/2" ALUMINUM BRACKET w/ FLANGE WINGS

3"x3"x.092" ALUMINUM COLUMN, 6053-T6

3"x3"x.092" ALUMINUM COLUMN, 6053-T6

1/8"x3"x3"x1 1/2" ALUMINUM BRACKET 6053-T6

1/2" WASHER, 410SS

(4) #12X3/4" SS TEK SCREWS

1/2" THD ROD, G90 OR HDG w/ SIMPSON AT EPOXY

4 1/4" MIN. EMBEDMENT IN CMU STEM WALL

(2) 2X12 SYP #2 U.N.O. SEE STRUCTURAL PLAN

(2) SIMPSON LSTA21 w/ (8) -16d TO HEADER AND (8) -16d TO POST

6X6 / 4X4 SYP #2 POST

SIMPSON ABU POST BASE w/ (12) - 16d & 5/8" x 10" ANCHOR BOLT

SEE FOOTING DETAILS

1 3/4" MIN. EDGE SPACING

4 1/4" MIN. EMBEDMENT IN CMU STEM WALL

(2) 1/4"x 2 1/2" TAPCON, 410SS OR G95 w/ SS WASHERS 2 1/4" MIN. EMBEDMENT (PER MIAMI DADE NOA 03-0114.03)

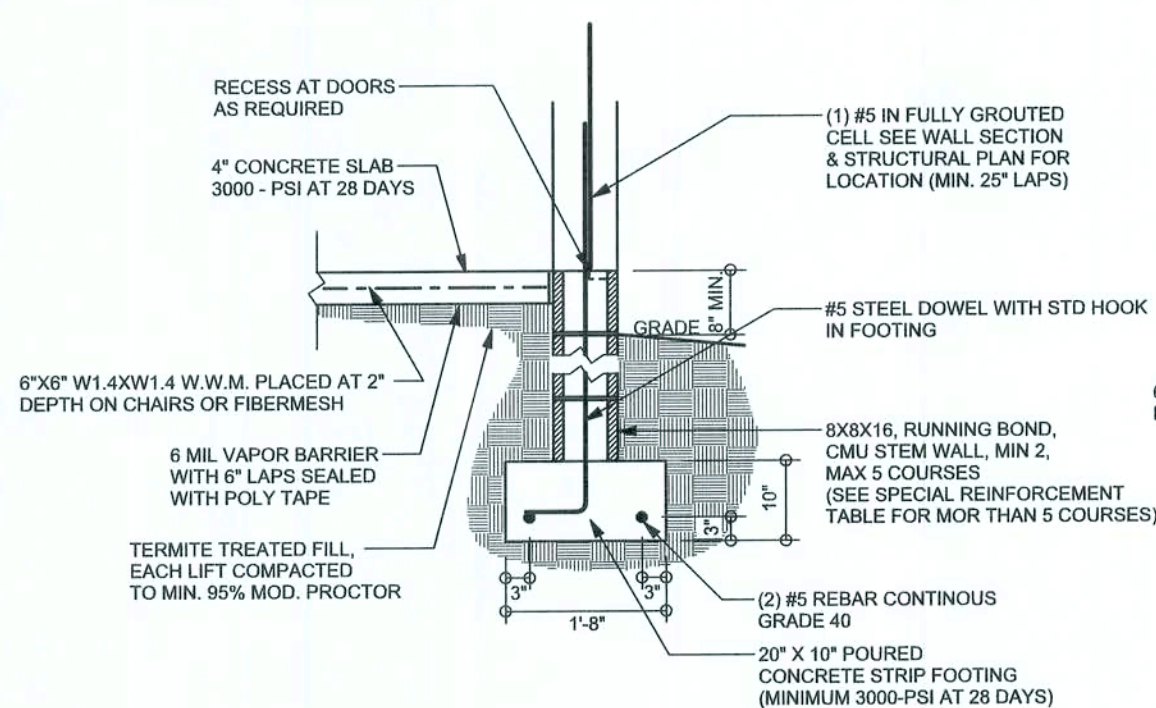
(1) #5 CONTINUOUS IN STEM WALL INTERSECTION w/ SLAB

4" CONCRETE SLAB MONOLITHIC w/ STEM WALL

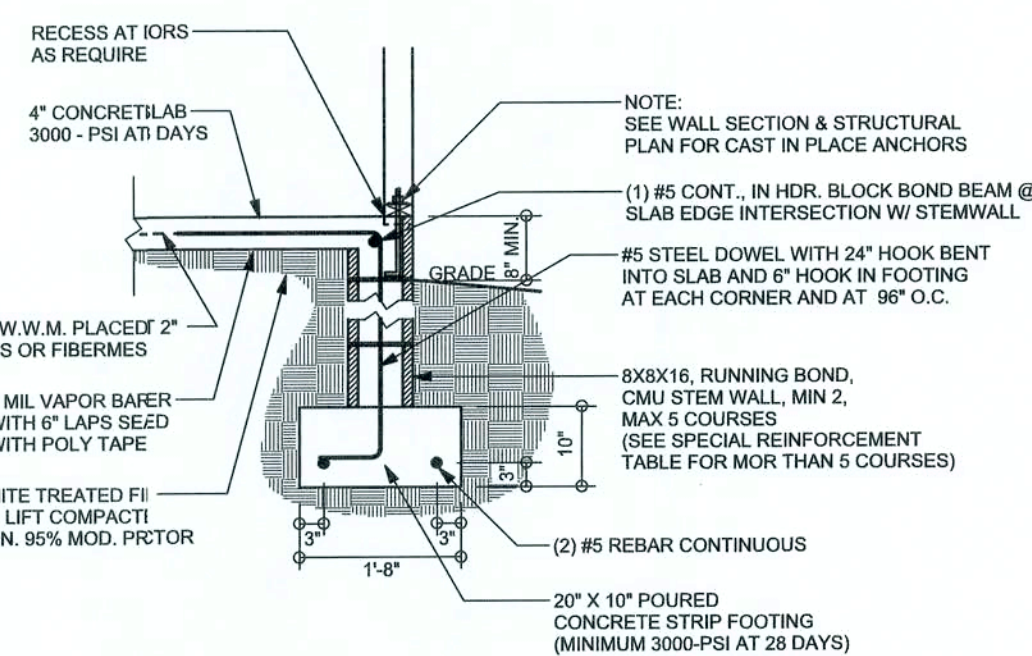
BEAM MID-WALL CONNECTION DETAIL

REVISIONS	

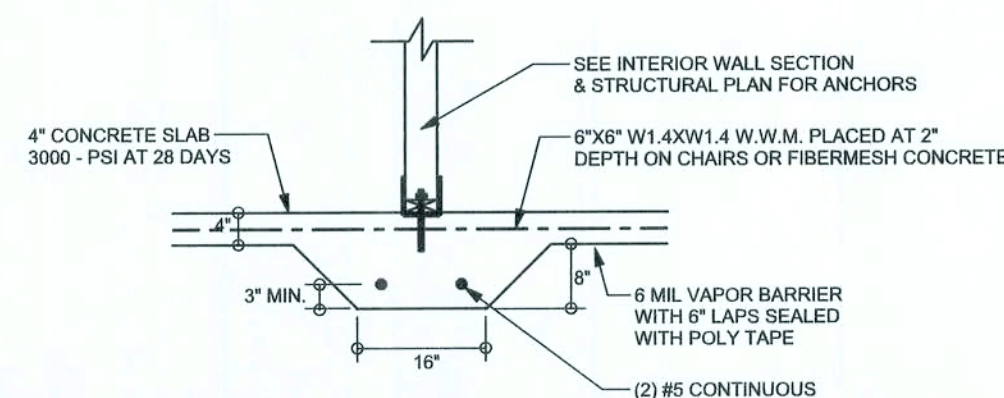
SOTPLAN
ARCHITECTURAL DESIGN SOFTWARE



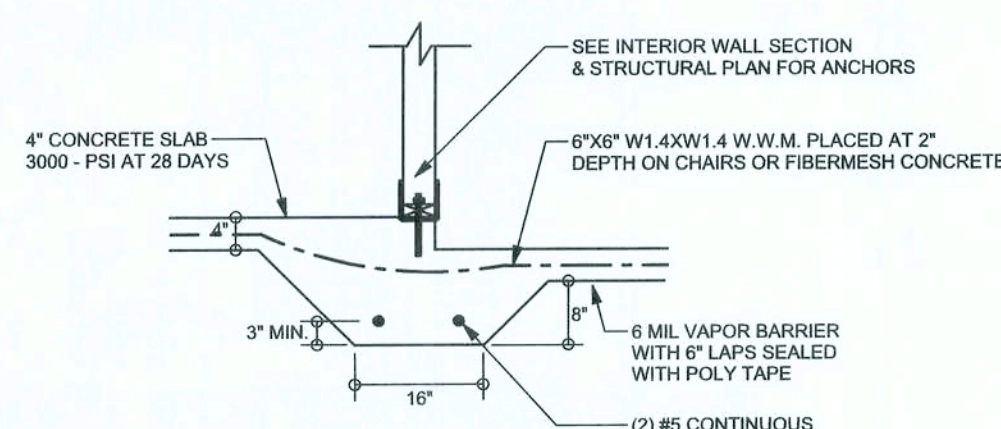
F17 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



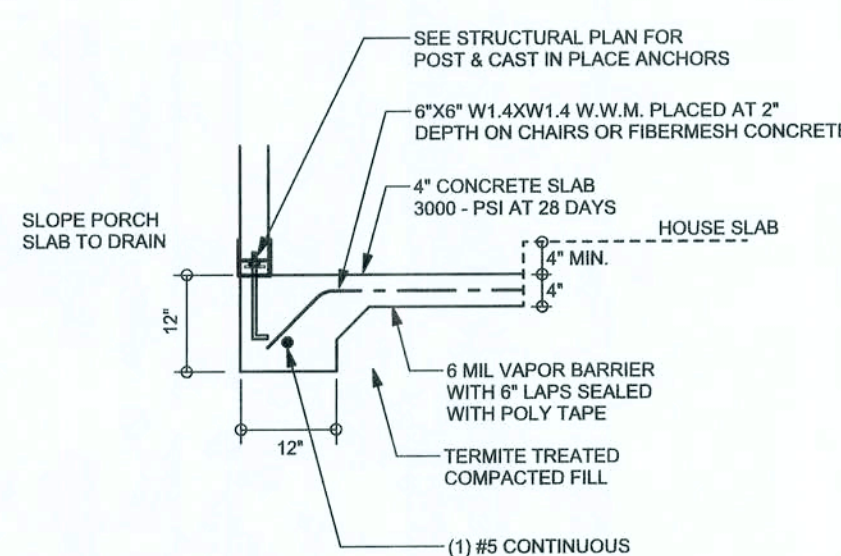
F18 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



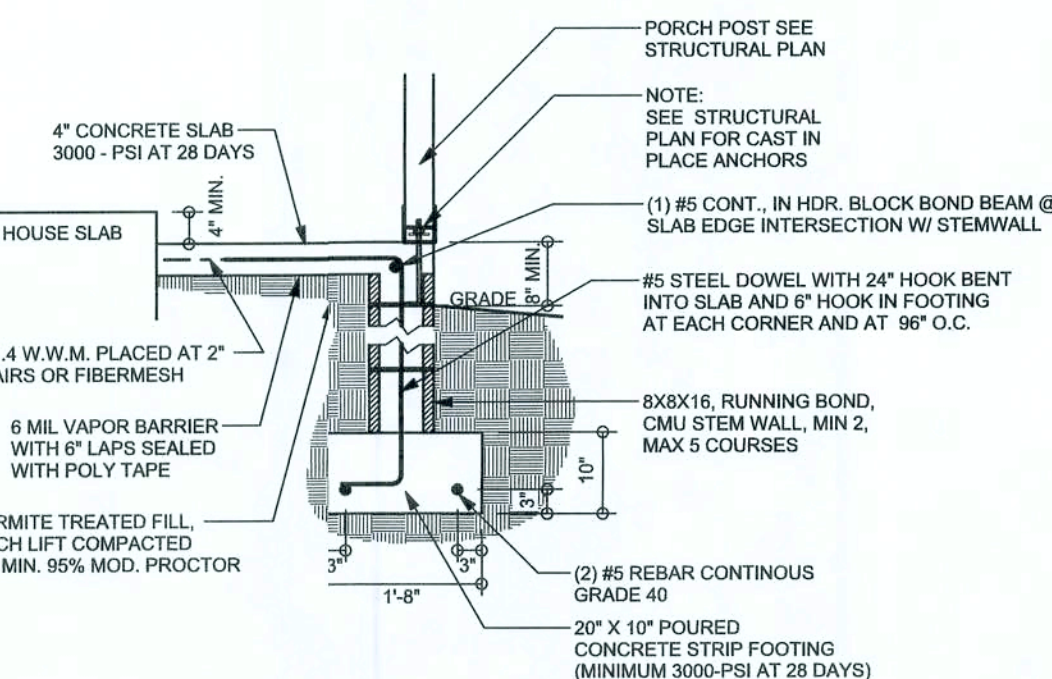
F2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F5 PORCH FOOTING
SCALE: 1/2" = 1'-0"



F12 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

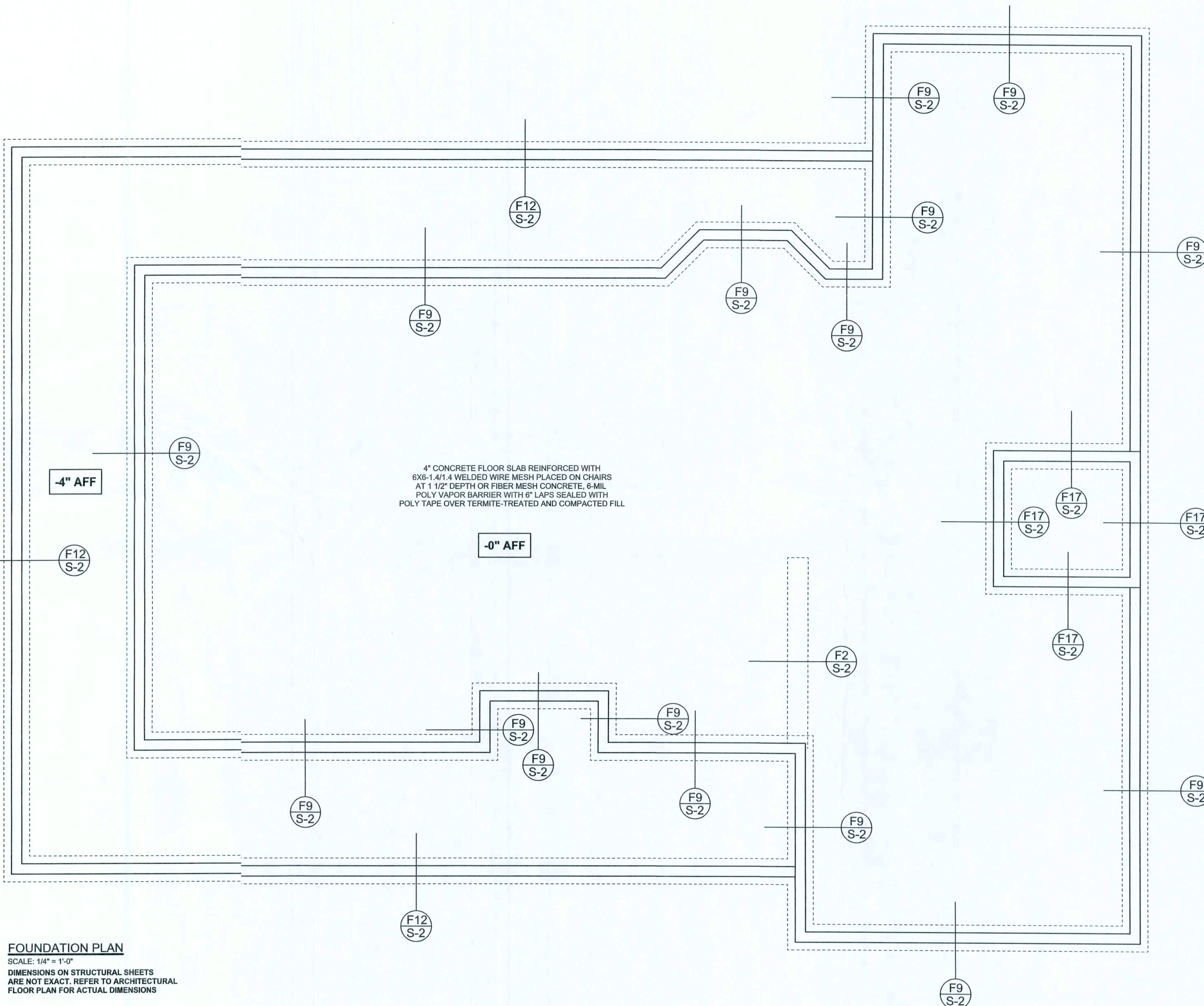
TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The verticality is to be placed toward the tension side of the CMU wall (away from the soil press, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 18" O.C. vertically or a horizontal bond beam with 16S continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT IN 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#	#7	#8	#5	#7	#8
3.3	3.0	9	96	96	96	96	96
4.0	3.7	9	96	96	96	96	96
4.7	4.3	8	96	96	96	96	96
5.3	5.0	5	96	96	96	96	96
6.0	5.7	4	80	96	80	96	96
6.7	6.3	3	56	80	56	96	96
7.3	7.0	2	40	56	40	80	96
8.0	7.7	1	32	48	32	64	80
8.7	8.3	1	24	32	24	48	64
9.3	9.0	1	16	24	16	40	48

FOUNDATION PLAN

SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS



WINDLOAD ENGINEER: Mark Disoway,
P.E. No. 53915, P.G. 868, Lake City, FL
30506, 386-754-4118

DIMENSIONS: Stated dimension supercede scaled dimensions. Refer all questions to Mark Disoway, I.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MRK DISOWAY
P.E. 53915

Mark Disoway
28 Nov 2007
SEAL

**High Springs
Construction**

**Mke Kuznik
Residence**

ADDRESS:
Lot 10 Tutumgee Woods S/D
Columbia County, Florida

Mark Disoway P.E.
P.O. Box 868
Lake City, Florida 32056
Phone: (886) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
November 28, 2007

DRAWN BY: David Disoway
CHECKED BY:

FINALS DATE:
28 / Nov / 07

JOB NUMBER:
711021

DRAWING NUMBER

S-2

OF 3 SHEETS

