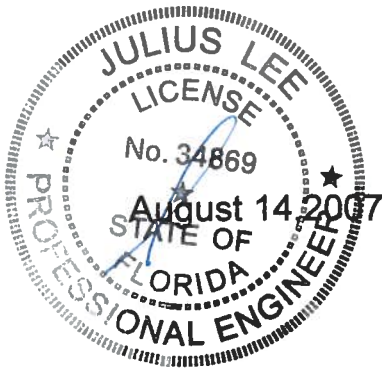
Builders FirstSource

Project Information for: **L249613**

Address:	22279 Southwest State Road 47
...	Fort White, Florida 32038
County:	Columbia
Truss Count:	8
Design Program:	MiTek 20/20 6.3
Building Code:	FBC2004/TPI2002



Truss Design Load Information:

Gravity: **Wind:**

Roof (psf): 42.0 Wind Standard: ASCE 7-02 Wind Exposure: B

Floor (psf): N/A Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Architect of Record, responsible for structural engineering:

Thomas Gregory Florida Architect License No. AR00006171

Address: 2916 North Oak Street Valdosta, Georgia 31602

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1878494	PB1	8/14/07
2	J1878495	T01	8/14/07
3	J1878496	T02	8/14/07
4	J1878497	T03	8/14/07
5	J1878498	T04	8/14/07
6	J1878499	T05	8/14/07
7	J1878500	T06	8/14/07
8	J1878501	T06G	8/14/07

Builders FirstSource

Project Information for: L249613

August 14,2007

... ...
Address: 22279 Southwest State Road 47
... Fort White, Florida 32038
County: Columbia
Truss Count: 8
Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002

Truss Design Load Information:

Gravity: **Wind:**

Roof (psf): 42.0 Wind Standard: ASCE 7-02 Wind Exposure: B
Floor (psf): N/A Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Architect of Record, responsible for structural engineering:
Thomas Gregory Florida Architect License No. AR00006171
Address: 2916 North Oak Street Valdosta, Georgia 31602

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869
Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

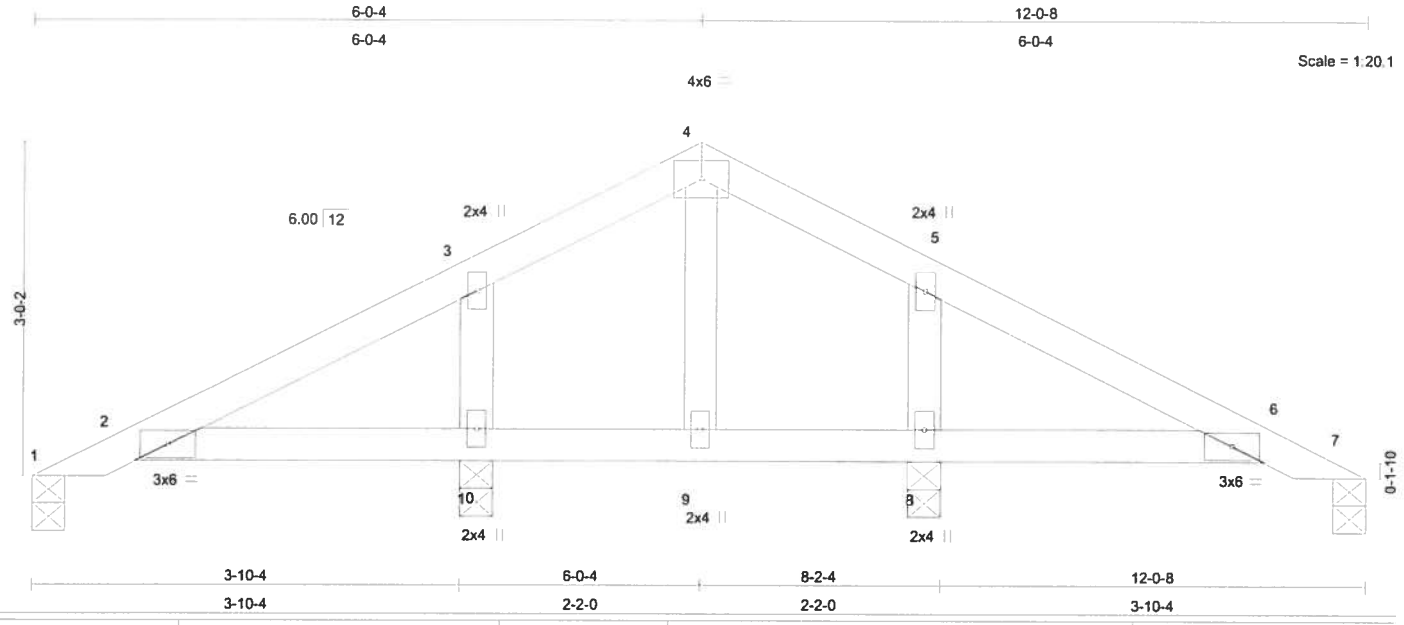
Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1878494	PB1	8/14/07
2	J1878495	T01	8/14/07
3	J1878496	T02	8/14/07
4	J1878497	T03	8/14/07
5	J1878498	T04	8/14/07
6	J1878499	T05	8/14/07
7	J1878500	T06	8/14/07
8	J1878501	T06G	8/14/07

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE BAPTIST CHURCH J1878494
L249613	PB1	PIGGYBACK	59	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Aug 13 17:41:56 2007 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	-0.01	2-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	-0.01	2-10	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 42 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=83/0-3-8, 7=83/0-3-8, 10=294/0-3-8, 8=294/0-3-8
Max Horz 1=38(load case 5)
Max Uplift 1=-9(load case 7), 7=-15(load case 7), 10=-95(load case 6), 8=-86(load case 7)
Max Grav 1=88(load case 10), 7=88(load case 11), 10=294(load case 1), 8=294(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-36/40, 2-3=-87/83, 3-4=-16/37, 4-5=-16/37, 5-6=-87/83, 6-7=-36/22
BOT CHORD 2-10=-32/137, 9-10=-32/137, 8-9=-32/137, 6-8=-32/137
WEBS 4-9=-57/43, 3-10=-201/204, 5-8=-201/204

JOINT STRESS INDEX
2 = 0.31, 3 = 0.11, 4 = 0.09, 5 = 0.11, 6 = 0.31, 8 = 0.11, 9 = 0.02 and 10 = 0.11

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2

John Lee
Truss Design Engineer
Florida PE No. 34627
1100 Crandall Bay Blvd
Boynton Beach, FL 33435

August 14,2007

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE BAPTIST CHURCH
					J1878494
L249613	PB1	PIGGYBACK	59	1	Job Reference (optional)

NOTES

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 9 lb uplift at joint 1, 15 lb uplift at joint 7, 95 lb uplift at joint 10 and 86 lb uplift at joint 8.
7) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1106 Coastal Bay Blvd
Boynton Beach, FL 33435

August 14,2007

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE BAPTIST CHURCH J1878495
L249613	T01	SPECIAL	20	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Aug 13 17:41:57 2007 Page 1

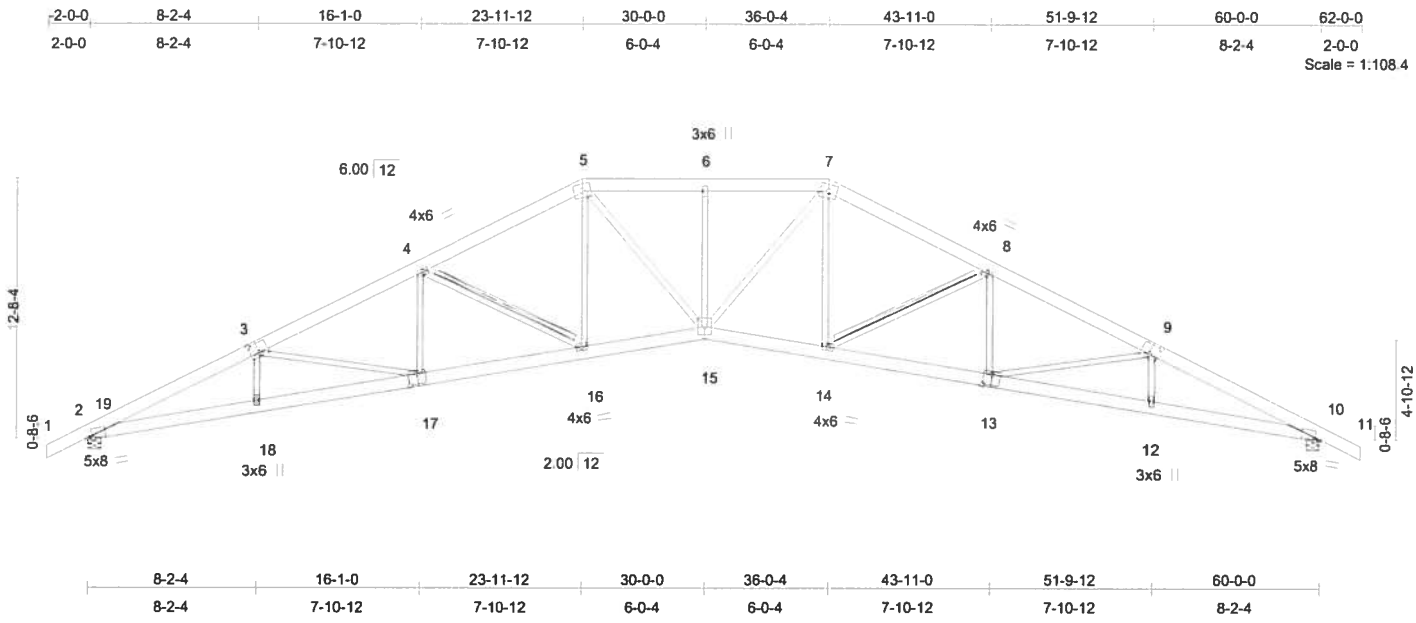


Plate Offsets (X,Y): [2:0-2-9,0-0-6], [3:0-5-0,0-6-0], [9:0-5-0,0-6-0], [10:0-2-9,0-0-6], [13:0-5-0,0-6-0], [17:0-5-0,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	0.35	15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.56	15	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.51	Horz(TL)	0.35	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 534 lb	

LUMBER
TOP CHORD 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-6 oc purlins, except
2-0-0 oc purlins (6-0-0 max.): 5-7.
BOT CHORD Rigid ceiling directly applied or 8-0-5 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-16, 8-14
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=2025/0-7-10, 10=2025/0-7-10
Max Horz 2=176(load case 6)
Max Uplift 2=-484(load case 6), 10=-484(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-19=0/39, 2-19=0/45, 2-3=-5098/2678, 3-4=-4561/2448, 4-5=-3816/2078, 5-6=-3885/2160, 6-7=-3885/2160, 7-8=-3816/2078, 8-9=-4561/2448, 9-10=-5098/2678, 10-11=0/45
BOT CHORD 2-18=-2195/4467, 17-18=-2200/4474, 16-17=-1824/4064, 15-16=-1280/3385, 14-15=-1280/3385, 13-14=-1824/4064, 12-13=-2200/4474, 10-12=-2195/4467
WEBS 3-18=0/210, 3-17=-448/377, 4-17=-97/361, 4-16=-774/608, 5-16=-339/568, 5-15=-324/957, 6-15=-231/129, 7-15=-324/957, 7-14=-339/568, 8-14=-774/608, 8-13=-97/361, 9-13=-448/377, 9-12=0/210

Julius Lee
Truss Design Engineer
Florida PE No. 34869
109 Coastal Bay Run
Boynton Beach, FL 33435

August 14,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE BAPTIST CHURCH J1878495
L249613	T01	SPECIAL	20	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.96, 3 = 0.34, 4 = 0.29, 5 = 0.46, 6 = 0.15, 7 = 0.46, 8 = 0.29, 9 = 0.34, 10 = 0.96, 12 = 0.15, 13 = 0.57, 14 = 0.28, 15 = 0.59, 16 = 0.28, 17 = 0.57 and 18 = 0.15

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) WARNING: The unusually long span and/or configuration of this truss requires that extreme care be used in its application. Use proper transportation, unloading and erection methods. Assure that all required web lateral bracing is communicated to the building contractor. Ensure that over-all building bracing is designed by a qualified engineer, architect, or building designer.
4) Provide adequate drainage to prevent water ponding.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All plates are 8x10 MT20 unless otherwise indicated.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Bearing at joint(s) 2, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 484 lb uplift at joint 2 and 484 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1102 Coastal Bay Pkwy
Boynton Beach, FL 33435

August 14,2007

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE BAPTIST CHURCH J1878496
L249613	T02	SPECIAL	9	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Aug 13 17:41:59 2007 Page 1

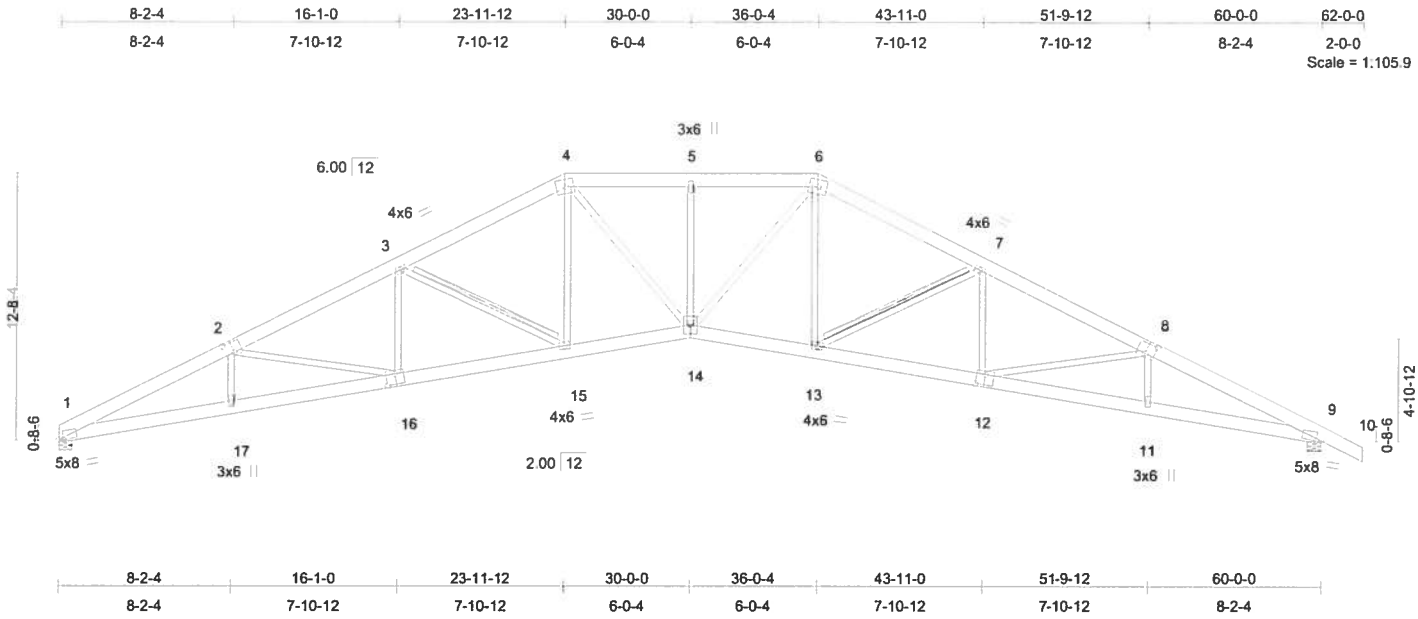


Plate Offsets (X,Y): [1:0-2-9,0-0-6], [2:0-5-0,0-6-0], [8:0-5-0,0-6-0], [9:0-2-9,0-0-6], [12:0-5-0,0-6-0], [16:0-5-0,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	0.35	14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.22	Vert(TL)	-0.57	14	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.36	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 527 lb	

LUMBER
TOP CHORD 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-1-14 oc purlins, except
2-0-0 oc purlins (5-11-14 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 7-10-9 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-15, 7-13
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 1=1897/0-7-10, 9=2027/0-7-10
Max Horz 1=-188(load case 7)
Max Uplift 1=-384(load case 6), 9=-484(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5153/2765, 2-3=-4582/2481, 3-4=-3828/2096, 4-5=-3895/2175, 5-6=-3895/2175, 6-7=-3824/2090, 7-8=-4568/2460, 8-9=-5105/2689, 9-10=0/45
BOT CHORD 1-17=-2283/4523, 16-17=-2285/4528, 15-16=-1852/4083, 14-15=-1298/3396, 13-14=-1292/3392, 12-13=-1835/4071, 11-12=-2210/4481, 9-11=-2205/4473
WEBS 2-17=0/213, 2-16=-484/434, 3-16=-113/364, 3-15=-782/620, 4-15=-348/574, 4-14=-322/957, 5-14=-230/129, 6-14=-331/962, 6-13=-339/568, 7-13=-774/607, 7-12=-97/361, 8-12=-448/376, 8-11=0/210

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1100 Crandall Bay Blvd
Boynton Beach, FL 33435

August 14,2007

Continued on page 2

JOINT STRESS INDEX

1 = 0.97, 2 = 0.35, 3 = 0.29, 4 = 0.46, 5 = 0.15, 6 = 0.46, 7 = 0.29, 8 = 0.35, 9 = 0.97, 11 = 0.15, 12 = 0.58, 13 = 0.28, 14 = 0.60, 15 = 0.28, 16 = 0.58 and 17 = 0.15

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) WARNING: The unusually long span and/or configuration of this truss requires that extreme care be used in its application. Use proper transportation, unloading and erection methods. Assure that all required web lateral bracing is communicated to the building contractor. Ensure that over-all building bracing is designed by a qualified engineer, architect, or building designer.
4) Provide adequate drainage to prevent water ponding.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All plates are 8x10 MT20 unless otherwise indicated.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 384 lb uplift at joint 1 and 484 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34869
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

August 14,2007

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE BAPTIST CHURCH J1878497
L249613	T03	GABLE	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Aug 13 17:42:05 2007 Page 1

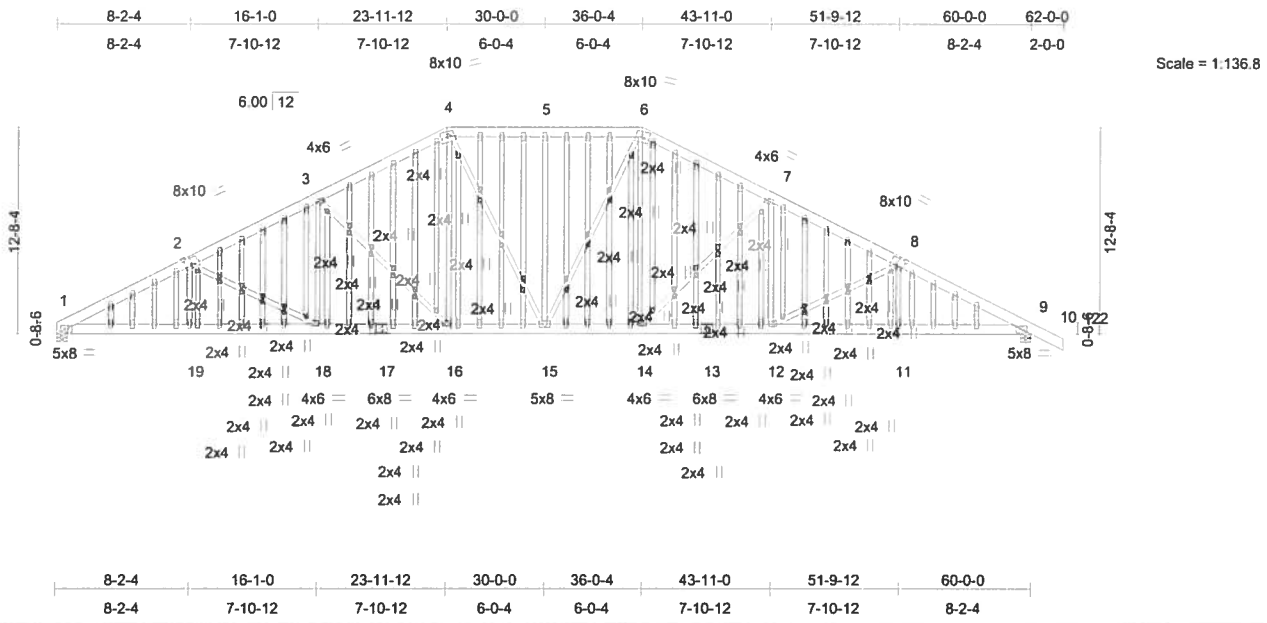


Plate Offsets (X,Y): [2:0-5-0,0-6-0], [8:0-5-0,0-6-0], [102:0-1-12,0-1-0], [105:0-1-12,0-1-0], [108:0-1-12,0-1-0], [111:0-1-12,0-1-0], [49:0-1-12,0-1-0], [52:0-1-12,0-1-0], [55:0-1-12,0-1-0], [58:0-1-12,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	0.18	16-18	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.28	16-18	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.59	Horz(TL)	0.12	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 1982 lb	

LUMBER

TOP CHORD	2 X 8 SYP No.1D
BOT CHORD	2 X 8 SYP No.1D
WEBS	2 X 4 SYP No.3 *Except* 4-15 2 X 4 SYP No.2, 6-15 2 X 4 SYP No.2
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-6.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=3499/0-7-10, 9=3633/0-7-10
Max Horz 1=-190(load case 7)
Max Uplift 1=-827(load case 6), 9=-928(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-6899/3836, 2-3=-6151/3513, 3-4=-5193/3075, 4-5=-4691/2925, 5-6=-4691/2925, 6-7=-5190/3071, 7-8=-6141/3498, 8-9=-6866/3783, 9-122=0/48, 10-122=0/41
BOT CHORD	1-19=-3215/5999, 18-19=-3215/5999, 17-18=-2750/5387, 16-17=-2750/5387, 15-16=-2154/4527, 14-15=-2150/4524, 13-14=-2738/5380, 12-13=-2738/5380, 11-12=-3161/5965, 9-11=-3161/5965
WEBS	2-19=-40/327, 2-18=-707/526, 3-18=-338/664, 3-16=-1248/858, 4-16=-711/1213, 4-15=-198/602, 5-15=-419/228, 6-15=-200/605, 6-14=-702/1207, 7-14=-1242/848, 7-12=-321/654, 8-12=-677/478, 8-11=-16/323

Julius Lee
Truss Design Engineer
Florida PE No 34869
1100 Crandall Bay Rd
Boynton Beach, FL 33435

August 14,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE BAPTIST CHURCH
					J1878497
L249613	T03	GABLE	1	2	Job Reference (optional)

JOINT STRESS INDEX

1 = 0.63, 2 = 0.24, 3 = 0.29, 4 = 0.30, 5 = 0.15, 6 = 0.30, 7 = 0.29, 8 = 0.24, 9 = 0.63, 11 = 0.15, 12 = 0.24, 13 = 0.47, 14 = 0.27, 15 = 0.28, 16 = 0.27, 17 = 0.47, 18 = 0.24, 19 = 0.15, 20 = 0.33, 20 = 0.33, 21 = 0.15, 22 = 0.15, 23 = 0.33, 23 = 0.33, 24 = 0.15, 25 = 0.15, 26 = 0.33, 26 = 0.33, 27 = 0.15, 28 = 0.15, 29 = 0.15, 30 = 0.33, 31 = 0.33, 32 = 0.15, 33 = 0.33, 33 = 0.33, 34 = 0.15, 35 = 0.15, 36 = 0.33, 36 = 0.33, 37 = 0.15, 38 = 0.15, 39 = 0.33, 39 = 0.33, 40 = 0.15, 41 = 0.15, 42 = 0.33, 42 = 0.33, 43 = 0.15, 44 = 0.15, 45 = 0.15, 46 = 0.33, 47 = 0.33, 48 = 0.15, 49 = 0.39, 49 = 0.33, 50 = 0.15, 51 = 0.15, 52 = 0.39, 52 = 0.33, 53 = 0.15, 54 = 0.15, 55 = 0.39, 55 = 0.33, 56 = 0.15, 57 = 0.15, 58 = 0.39, 58 = 0.33, 59 = 0.15, 60 = 0.15, 61 = 0.15, 62 = 0.33, 63 = 0.15, 64 = 0.15, 65 = 0.15, 66 = 0.15, 67 = 0.15, 68 = 0.15, 69 = 0.15, 70 = 0.15, 71 = 0.15, 72 = 0.15, 73 = 0.33, 73 = 0.33, 74 = 0.15, 75 = 0.15, 76 = 0.33, 76 = 0.33, 77 = 0.15, 78 = 0.15, 79 = 0.33, 79 = 0.33, 80 = 0.15, 81 = 0.33, 82 = 0.33, 83 = 0.15, 84 = 0.15, 85 = 0.15, 86 = 0.33, 86 = 0.33, 87 = 0.15, 88 = 0.15, 89 = 0.33, 89 = 0.33, 90 = 0.15, 91 = 0.15, 92 = 0.33, 92 = 0.33, 93 = 0.15, 94 = 0.15, 95 = 0.33, 95 = 0.33, 96 = 0.15, 97 = 0.33, 98 = 0.33, 99 = 0.15, 100 = 0.15, 101 = 0.15, 102 = 0.39, 102 = 0.33, 103 = 0.15, 104 = 0.15, 105 = 0.39, 105 = 0.33, 106 = 0.15, 107 = 0.15, 108 = 0.39, 108 = 0.33, 109 = 0.15, 110 = 0.15, 111 = 0.39, 111 = 0.33, 112 = 0.15, 113 = 0.33, 114 = 0.15, 115 = 0.15, 116 = 0.15, 117 = 0.15, 118 = 0.15, 119 = 0.15, 120 = 0.15 and 121 = 0.15

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
   Top chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
   Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
   Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
5) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
6) WARNING: The unusually long span and/or configuration of this truss requires that extreme care be used in its application. Use proper transportation, unloading and erection methods. Assure that all required web lateral bracing is communicated to the building contractor. Ensure that over-all building bracing is designed by a qualified engineer, architect, or building designer.
7) Provide adequate drainage to prevent water ponding.
8) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) All plates are 3x6 MT20 unless otherwise indicated.
10) Gable studs spaced at 1-4-0 oc.
11) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 827 lb uplift at joint 1 and 928 lb uplift at joint 9.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 4-6=-92(F=-38), 10-122=-54, 14-16=-48(F=-38)
   Trapezoidal Loads (plf)
   Vert: 1=-64(F=-10)-to-4=-92(F=-38), 6=-92(F=-38)-to-122=-64(F=-10), 1=-20(F=-10)-to-16=-48(F=-38), 14=-48(F=-38)-to-9=-20(F=-10)

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August 14,2007

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE BAPTIST CHURCH J1878498
L249613	T04	HIP	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Feb 15 2006 MiTek Industries, Inc. Mon Aug 13 17:42:07 2007 Page 1

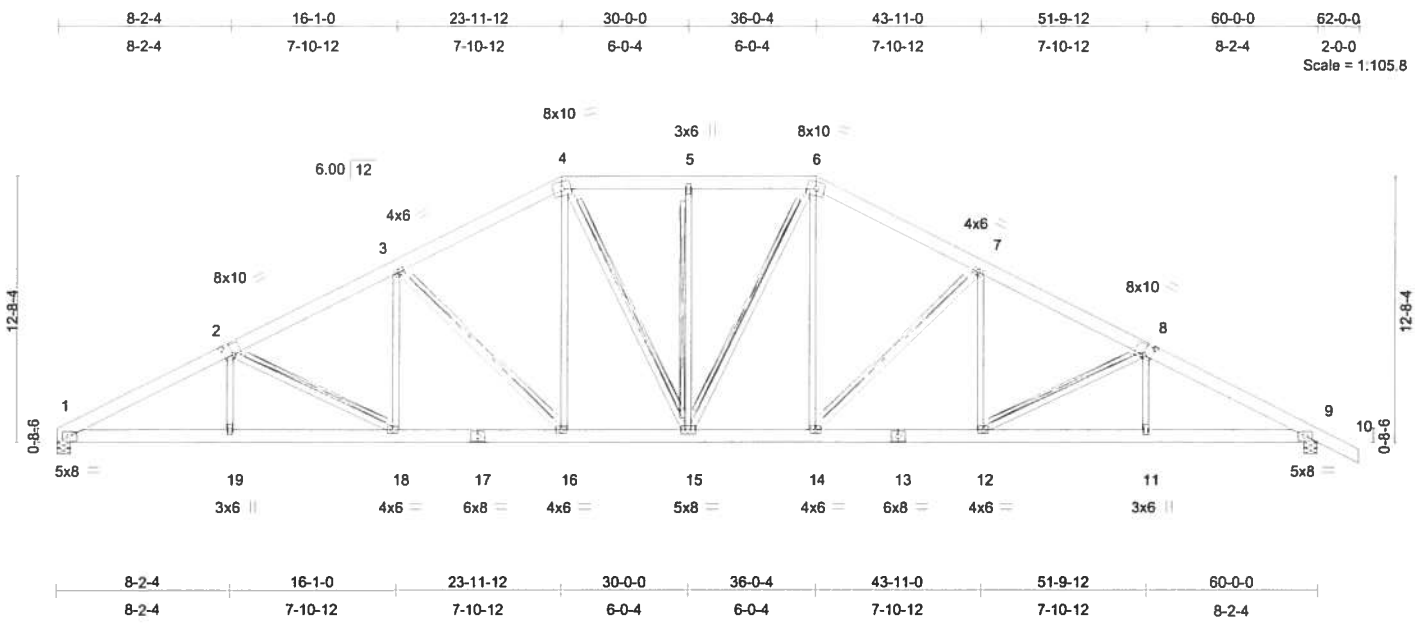

Plate Offsets (X,Y): [2:0-5-0,0-6-0], [8:0-5-0,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.18	Vert(LL)	0.18	16-18	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.28	16-18	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.85	Horz(TL)	0.12	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 575 lb	

LUMBER

TOP CHORD 2 X 8 SYP No.1D
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
4-15 2 X 4 SYP No.2, 6-15 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-11-6 oc purlins, except
2-0-0 oc purlins (6-0-0 max.): 4-6.
BOT CHORD Rigid ceiling directly applied or 7-6-6 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 2-18, 3-16, 4-15, 5-15 , 6-15, 7-14, 8-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 1=1897/0-7-10, 9=2027/0-7-10
Max Horz 1=-190(load case 7)
Max Uplift 1=-384(load case 6), 9=-484(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-3665/2019, 2-3=-3161/1837, 3-4=-2618/1635, 4-5=-2335/1581, 5-6=-2335/1581, 6-7=-2615/1631, 7-8=-3152/1822, 8-9=-3632/1966, 9-10=0/47
BOT CHORD 1-19=-1595/3159, 18-19=-1595/3159, 17-18=-1242/2744, 16-17=-1242/2744, 15-16=-862/2263, 14-15=-858/2260, 13-14=-1231/2737, 12-13=-1231/2737, 11-12=-1541/3125, 9-11=-1541/3125
WEBS 2-19=0/234, 2-18=-484/399, 3-18=-137/399, 3-16=-699/545, 4-16=-342/566, 4-15=-141/399, 5-15=-268/142, 6-15=-144/402, 6-14=-333/560, 7-14=-693/534, 7-12=-121/397, 8-12=-455/351, 8-11=0/231

Continued on page 2

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August 14,2007

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE BAPTIST CHURCH J1878498
L249613	T04	HIP	2	1	Job Reference (optional)

JOINT STRESS INDEX

1 = 0.68, 2 = 0.29, 3 = 0.29, 4 = 0.33, 5 = 0.15, 6 = 0.33, 7 = 0.29, 8 = 0.29, 9 = 0.68, 11 = 0.15, 12 = 0.24, 13 = 0.46, 14 = 0.25, 15 = 0.28, 16 = 0.25, 17 = 0.46, 18 = 0.24 and 19 = 0.15

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) WARNING: The unusually long span and/or configuration of this truss requires that extreme care be used in its application. Use proper transportation, unloading and erection methods. Assure that all required web lateral bracing is communicated to the building contractor. Ensure that over-all building bracing is designed by a qualified engineer, architect, or building designer.
4) Provide adequate drainage to prevent water ponding.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 384 lb uplift at joint 1 and 484 lb uplift at joint 9.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	CORNERSTONE BAPTIST CHURCH
					J1878499
L249613	T05	HIP	27	1	Job Reference (optional)

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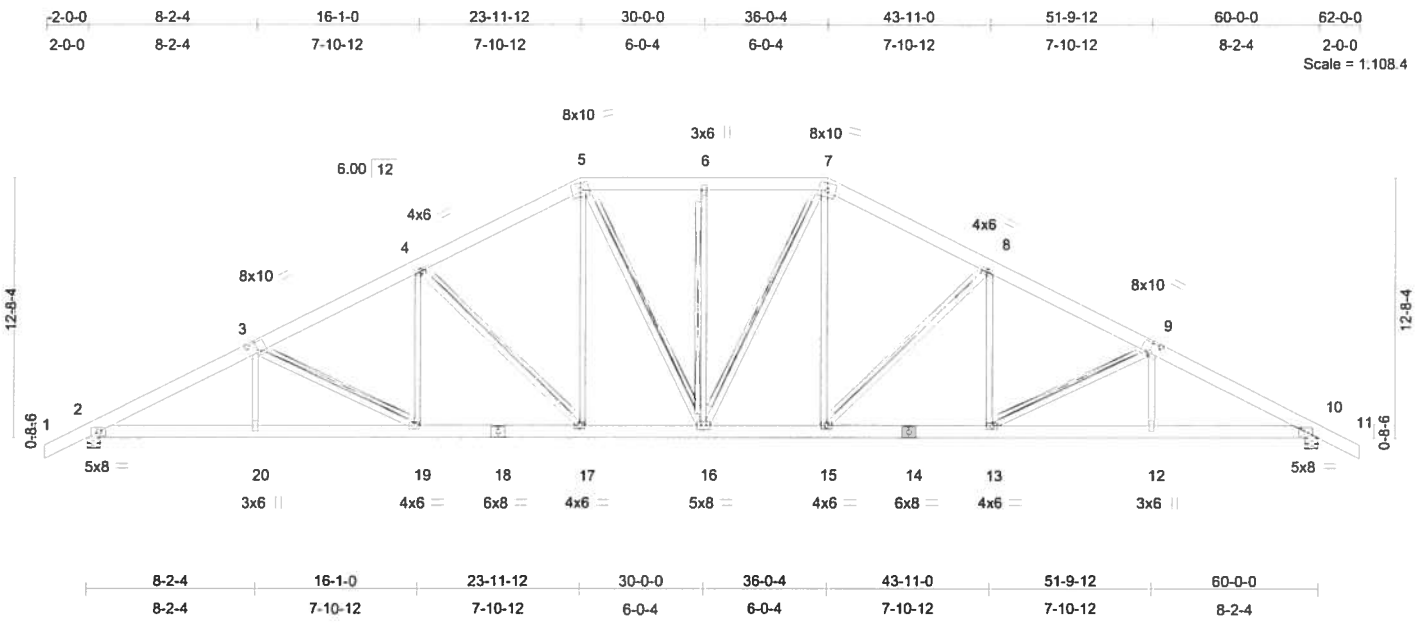

Plate Offsets (X,Y): [3:0-5-0,0-6-0], [9:0-5-0,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.18	Vert(LL)	0.18	15	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.28	17-19	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.82	Horz(TL)	0.12	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 582 lb	

LUMBER
TOP CHORD 2 X 8 SYP No.1D
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
5-16 2 X 4 SYP No.2, 7-16 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-14 oc purlins, except
2-0-0 oc purlins (6-0-0 max.): 5-7.
BOT CHORD Rigid ceiling directly applied or 7-9-2 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-19, 4-17, 5-16, 6-16 , 7-16, 8-15, 9-13
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=2025/0-7-10, 10=2025/0-7-10
Max Horz 2=-177(load case 7)
Max Uplift 2=-484(load case 6), 10=-484(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3627/1958, 3-4=-3146/1814, 4-5=-2610/1623, 5-6=-2329/1572, 6-7=-2329/1572, 7-8=-2610/1623, 8-9=-3146/1814, 9-10=-3627/1958, 10-11=0/47
BOT CHORD 2-20=-1534/3121, 19-20=-1534/3120, 18-19=-1223/2732, 17-18=-1223/2732, 16-17=-850/2256, 15-16=-850/2256, 14-15=-1223/2732, 13-14=-1223/2732, 12-13=-1534/3120, 10-12=-1534/3121
WEBS 3-20=0/231, 3-19=-455/351, 4-19=-121/397, 4-17=-693/535, 5-17=-333/560, 5-16=-143/399, 6-16=-268/143, 7-16=-143/399, 7-15=-333/560, 8-15=-693/535, 8-13=-121/397, 9-13=-455/351, 9-12=0/231

Continued on page 2

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August 14,2007

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE BAPTIST CHURCH J1878499
L249613	T05	HIP	27	1	Job Reference (optional)

JOINT STRESS INDEX

2 = 0.68, 3 = 0.27, 4 = 0.29, 5 = 0.33, 6 = 0.15, 7 = 0.33, 8 = 0.29, 9 = 0.27, 10 = 0.68, 12 = 0.15, 13 = 0.24, 14 = 0.46, 15 = 0.25, 16 = 0.28, 17 = 0.25, 18 = 0.46, 19 = 0.24 and 20 = 0.15

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) WARNING: The unusually long span and/or configuration of this truss requires that extreme care be used in its application. Use proper transportation, unloading and erection methods. Assure that all required web lateral bracing is communicated to the building contractor. Ensure that over-all building bracing is designed by a qualified engineer, architect, or building designer.
4) Provide adequate drainage to prevent water ponding.
5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 484 lb uplift at joint 2 and 484 lb uplift at joint 10.

LOAD CASE(S) Standard

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August 14,2007

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE BAPTIST CHURCH
					J1878500
L249613	T06	COMMON	18	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.300 s Apr 19 2006 MiTek Industries, Inc. Tue Aug 14 15:10:57 2007 Page 1

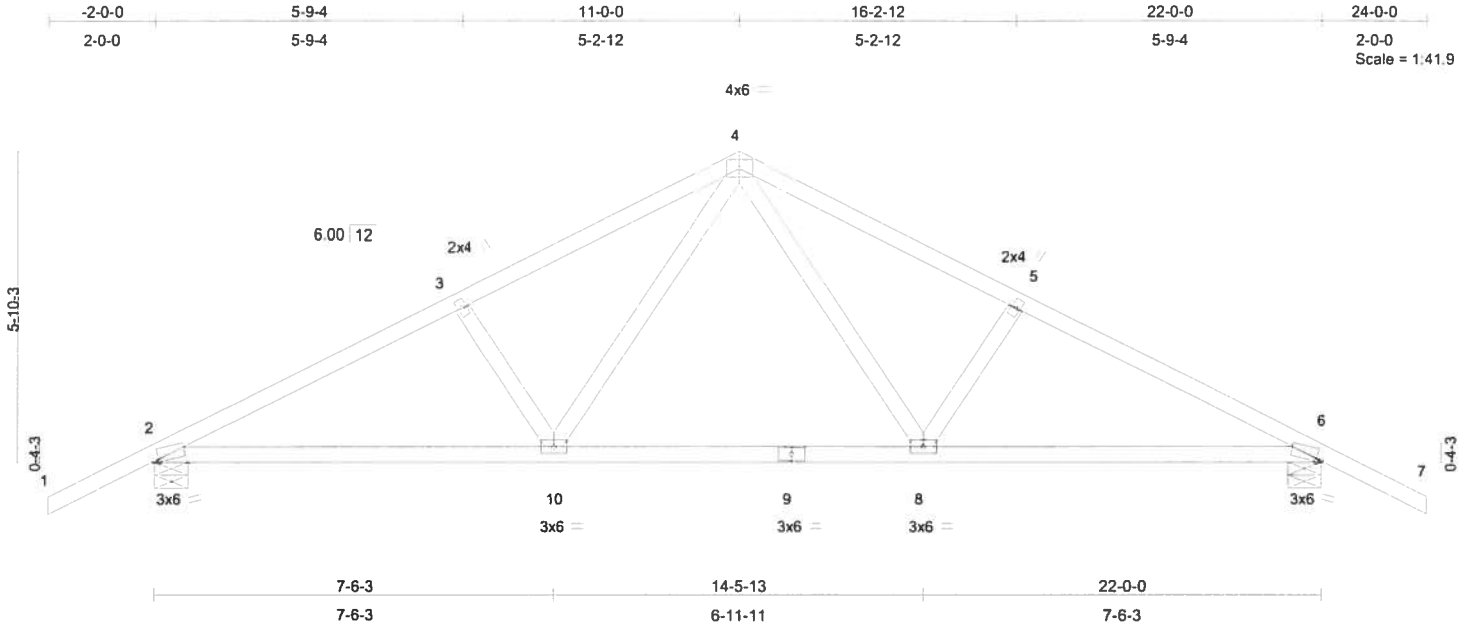

Plate Offsets (X,Y): [2:0-1-1,0-0-7], [6:0-1-1,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	0.17	2-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.12	2-10	>999	240		
BCLL	10.0 *	Rep Stress Incr	YES	WB	0.49	Horz(TL)	-0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 104 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-9-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-8-6 oc bracing.

REACTIONS (lb/size) 2=809/0-7-10, 6=809/0-7-10
Max Horz 2=101(load case 6)
Max Uplift 2=-520(load case 6), 6=-520(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/48, 2-3=-1135/1471, 3-4=-978/1471, 4-5=-978/1471, 5-6=-1135/1471, 6-7=0/48
BOT CHORD 2-10=-1140/940, 9-10=-674/653, 8-9=-674/653, 6-8=-1140/940
WEBS 3-10=-247/234, 4-10=-639/329, 4-8=-639/329, 5-8=-247/234

JOINT STRESS INDEX
2 = 0.81, 3 = 0.34, 4 = 0.60, 5 = 0.34, 6 = 0.81, 8 = 0.43, 9 = 0.25 and 10 = 0.43

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 520 lb uplift at joint 2 and 520 lb uplift at joint 6.

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August 14,2007

Continued on page 2

Builders FirstSource

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE BAPTIST CHURCH J1878500
L249613	T06	COMMON	18	1	Job Reference (optional)

LOAD CASE(S) Standard

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Ocean Beach, FL 32456

August 14,2007

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE BAPTIST CHURCH
L249613	T06G	GABLE	1	1	J1878501
					Job Reference (optional)

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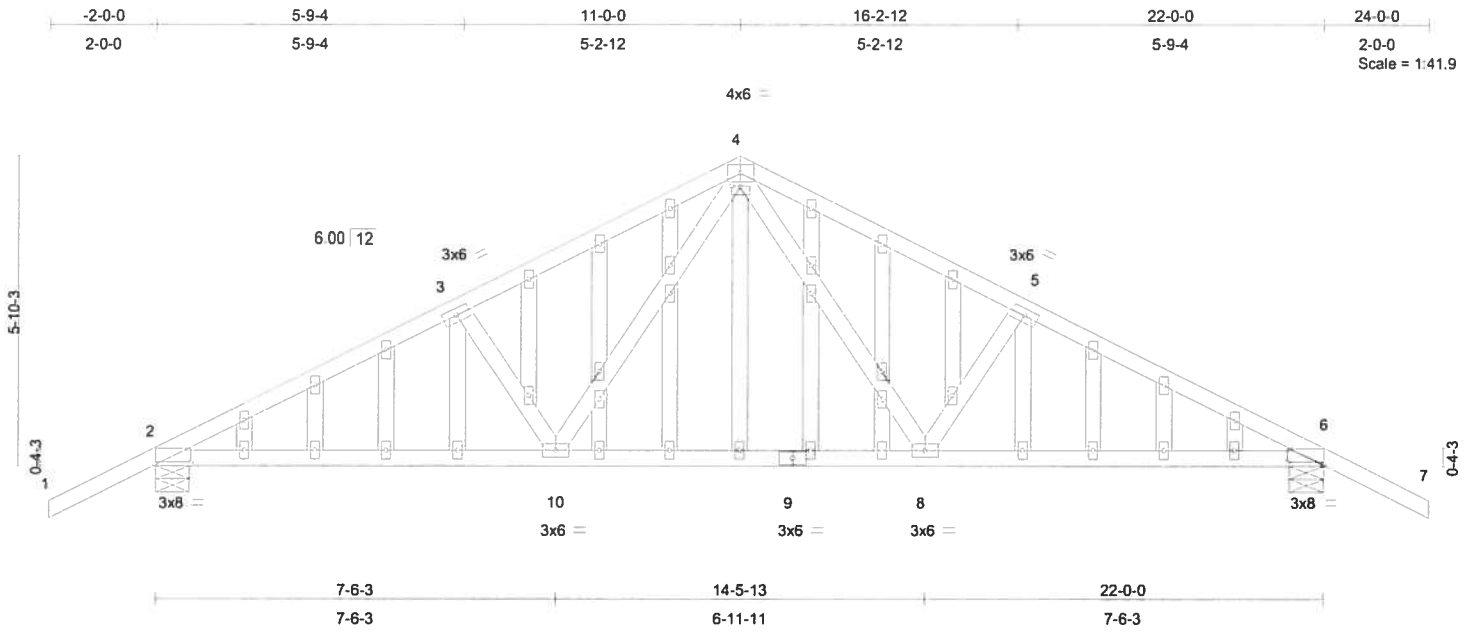

Plate Offsets (X,Y): [2:0-8-0,0-0-10], [4:0-2-0,0-0-4], [6:0-8-0,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	0.19	8-10	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	-0.15	8-10	>999	240		
BCLL	10.0 *	Rep Stress Incr	NO	WB	0.56	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 163 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-5-1 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 5-0-5 oc bracing.

REACTIONS (lb/size) 2=1316/0-7-10, 6=1316/0-7-10
Max Horz 2=-115(load case 7)
Max Uplift 2=-946(load case 6), 6=-946(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-12/83, 2-3=-1836/1850, 3-4=-1566/1787, 4-5=-1566/1787, 5-6=-1836/1850, 6-7=-12/83
BOT CHORD	2-10=-1466/1512, 9-10=-902/1052, 8-9=-902/1052, 6-8=-1466/1512
WEBS	3-10=-409/326, 4-10=-731/491, 4-8=-731/491, 5-8=-409/326

JOINT STRESS INDEX

2 = 0.72, 3 = 0.51, 4 = 0.83, 4 = 0.67, 5 = 0.51, 6 = 0.72, 8 = 0.43, 9 = 0.39, 10 = 0.43, 11 = 0.34, 12 = 0.34, 12 = 0.34, 13 = 0.34, 14 = 0.34, 15 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34, 29 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.34, 32 = 0.34, 33 = 0.34, 34 = 0.34, 35 = 0.34, 36 = 0.34, 37 = 0.34, 38 = 0.34, 39 = 0.34, 40 = 0.34 and 41 = 0.34

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

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August 14,2007

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE BAPTIST CHURCH J1878501
L249613	T06G	GABLE	1	1	Job Reference (optional)

NOTES

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable studs spaced at 1-4-0 oc.
7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 946 lb uplift at joint 2 and 946 lb uplift at joint 6.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
10) Gable truss supports 1' 6" max. rake gable overhang.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 2-6=-10, 1-4=-93(F=-39), 4-7=-93(F=-39)

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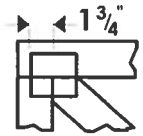
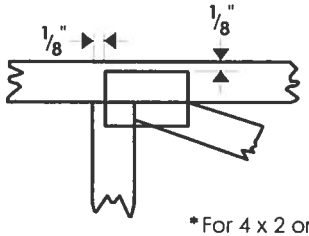
August 14,2007

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

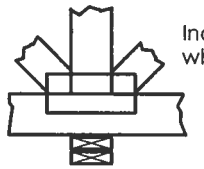
4 x 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

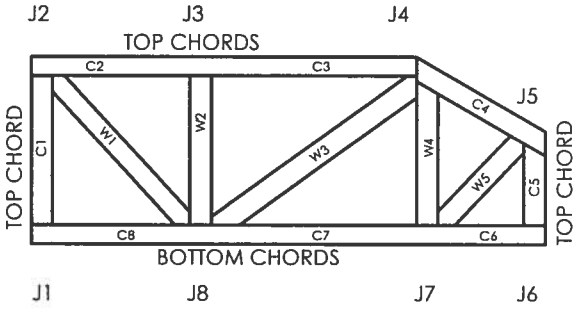
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE +		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 8"	10' 10"	11' 2"	12' 11"	13' 3"
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 8"	11' 8"
	SP DFL	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#2	3' 7"	6' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"
		#3	3' 6"	5' 0"	5' 0"	6' 8"	6' 8"	8' 3"	8' 8"	10' 4"	10' 4"	12' 11"	13' 7"
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 8"	10' 3"	10' 3"	12' 11"	13' 7"
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"
16" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 8"	12' 6"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 10"	6' 10"	9' 2"	9' 2"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 3"	7' 4"	7' 7"	8' 9"	8' 11"	10' 5"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	13' 8"	14' 0"	14' 0"
		STANDARD	4' 2"	6' 11"	6' 11"	7' 10"	7' 10"	10' 5"	10' 5"	12' 3"	12' 3"	14' 0"	14' 0"
	SP DFL	#1	4' 8"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 7"	7' 4"	7' 11"	8' 9"	9' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 2"	7' 2"	8' 9"	9' 2"	10' 5"	10' 11"	13' 5"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 1"	7' 1"	8' 9"	9' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

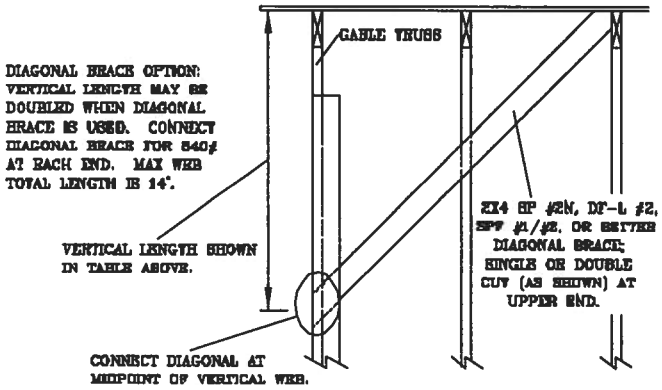
SOUTHERN PINE
#3
STUD
STANDARD

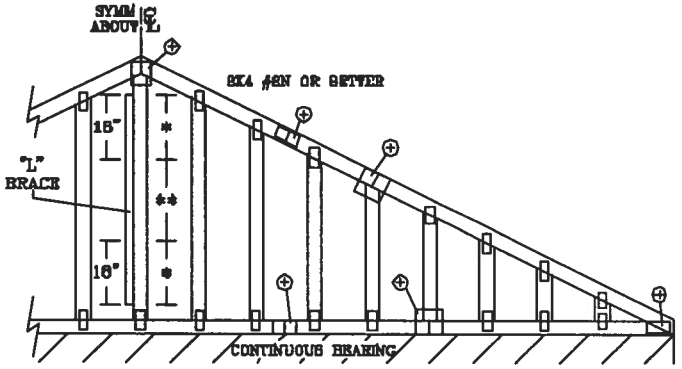
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 16" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 16" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF ASCE7-02-GAB13015

DATE 11/26/03

DRWG MITEK STD GABLE 15 E HT

-ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 2"	5' 6"	5' 8"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"
		#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STUD	3' 1"	4' 6"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"
		STANDARD	2' 11"	3' 9"	3' 9"	5' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"
	SP DFL	#1	3' 6"	5' 8"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#2	3' 5"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"
		#3	3' 3"	4' 6"	4' 8"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 8"
		STUD	3' 3"	4' 6"	4' 8"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 8"
		STANDARD	3' 0"	3' 10"	3' 10"	5' 1"	5' 1"	6' 11"	6' 11"	8' 0"	8' 0"	10' 10"	10' 10"
16" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 6"	4' 8"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 8"	8' 1"	8' 11"	8' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 8"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	9' 6"	11' 6"	11' 6"	14' 0"	14' 0"
		STUD	3' 9"	5' 6"	5' 8"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 6"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	13' 3"
12" O.C.	SPF HF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 5"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"
		#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"
		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"
	SP DFL	#1	4' 5"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#2	4' 4"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"
		#3	4' 2"	6' 5"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"
		STANDARD	4' 0"	5' 8"	5' 8"	7' 3"	7' 3"	8' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

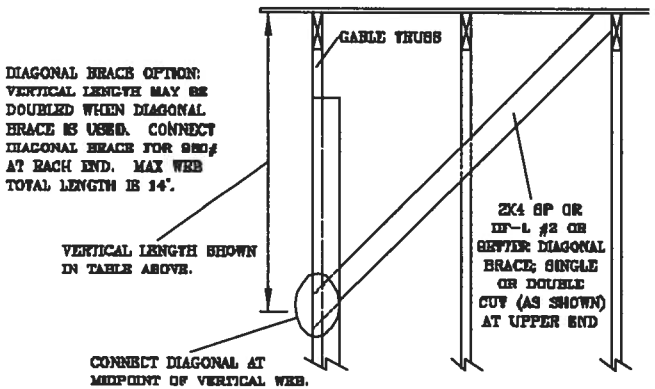
SOUTHERN PINE
#3
STUD
STANDARD

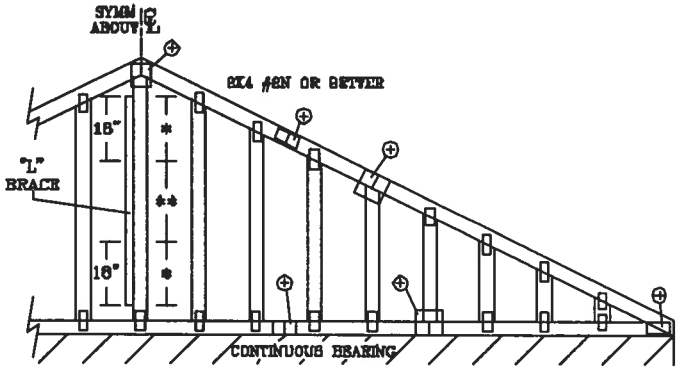
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 8" O.C. IN 16" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 16" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

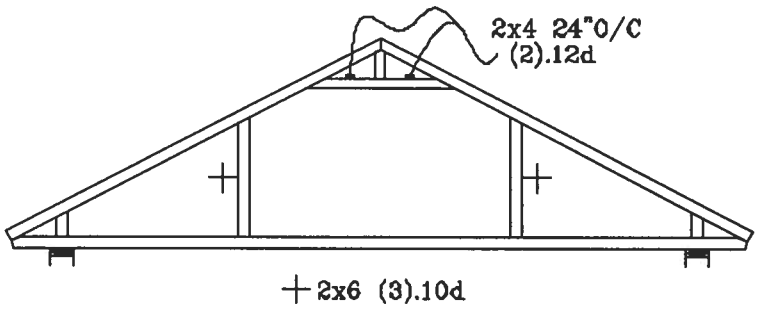
JULIUS LEE'S
CONS. ENGINEERS P.A.
1466 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161
No: 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

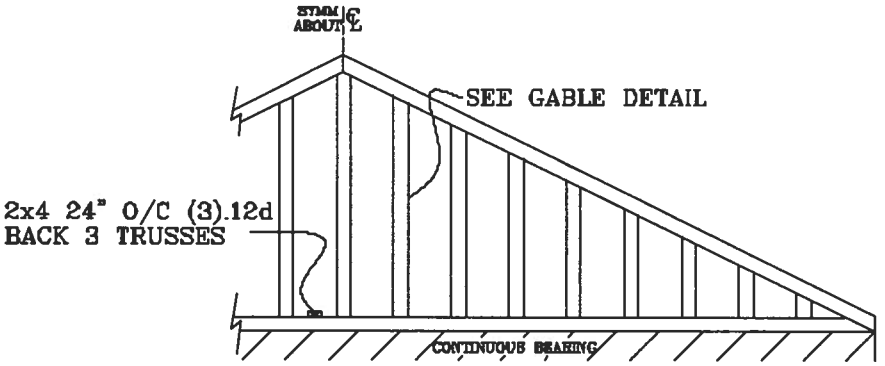
MAX. SPACING 24.0"

REF ASCE7-02-GAB13030
DATE 11/26/03
DWG MTEK STD GABLE 30' E W7
-ENG

TYPICAL ATTIC TRUSS BRACING

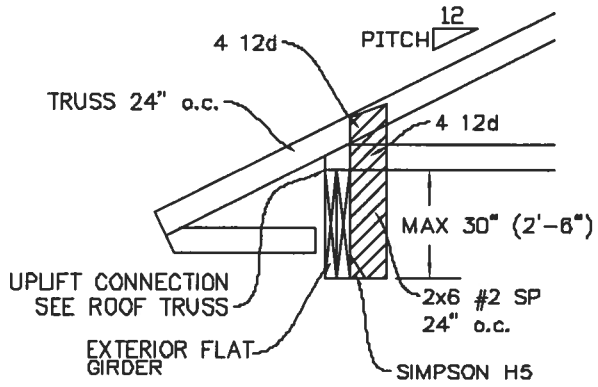


GABLE END TRUSS DETAIL

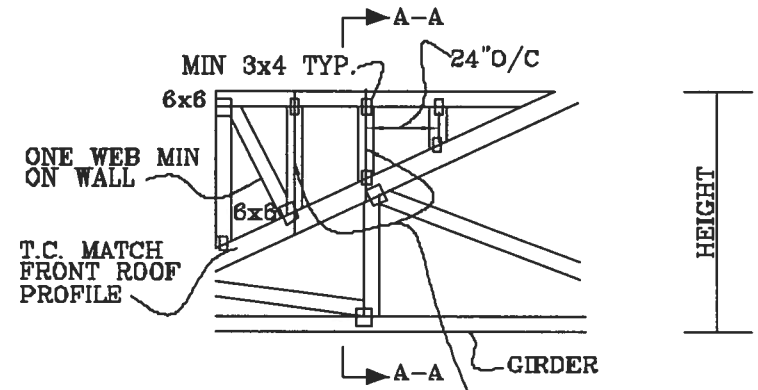


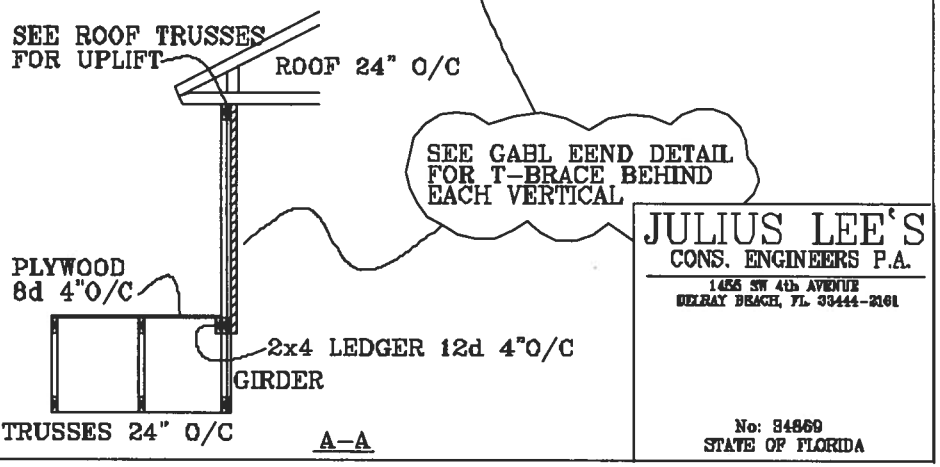
MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL





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DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-93, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

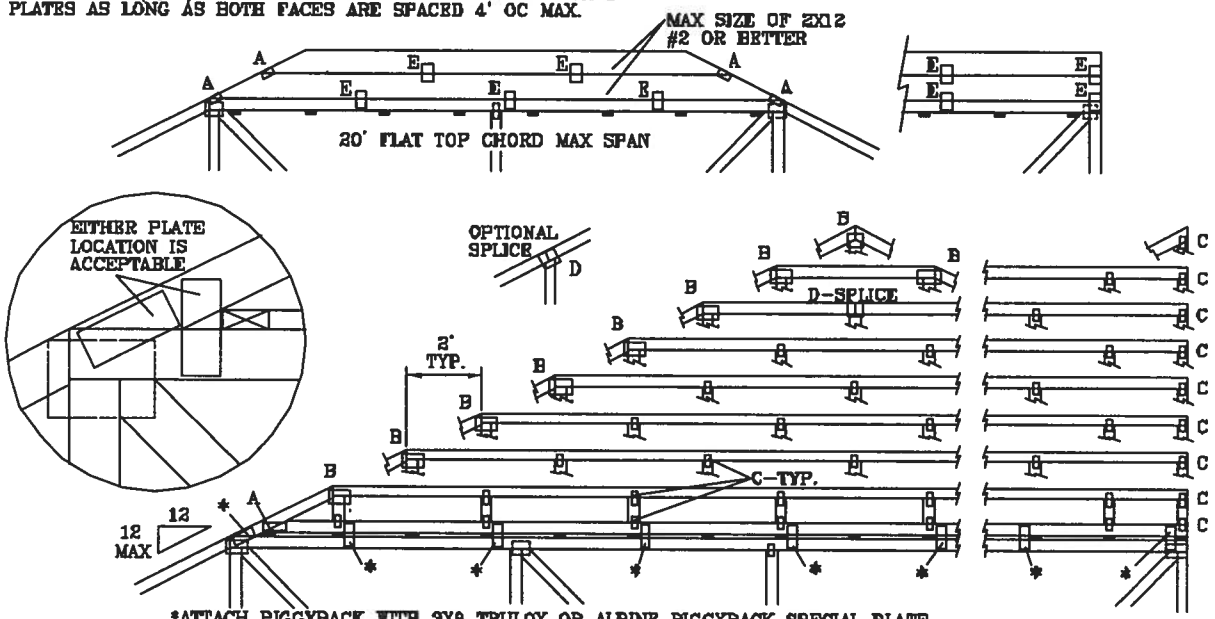
110 MPH WIND, 30' MEAN HGT, SEC
ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL=5 PSF, WIND BC DL=5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X8 OR 3X8 TRULOX AT 4' OC, ROTATED VERTICALLY			

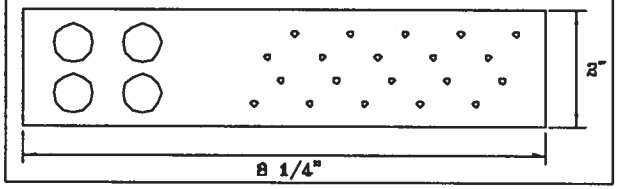
ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.



WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" OC.

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (1-03 BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161
No: 34869
STATE OF FLORIDA

MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF PIGGYBACK
DATE 11/26/03
DRWG MITEK STD PIGGY
-ENG JL

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR SBC 110 MPH, ASCE 7-93 110 MPH WIND OR (3) 16d FOR ASCE 7-98 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

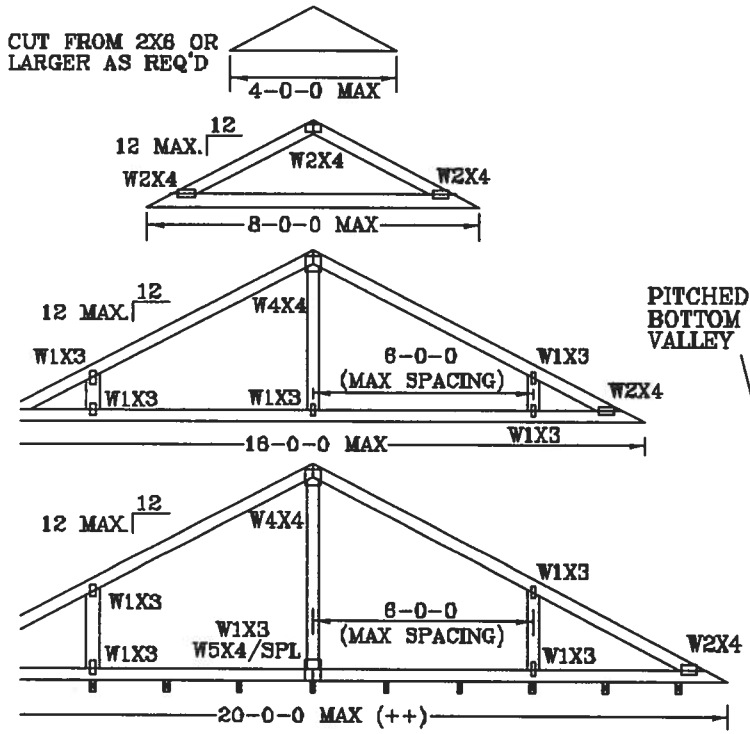
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

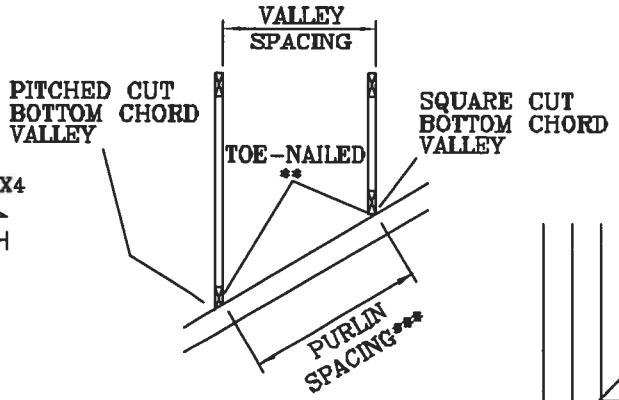
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

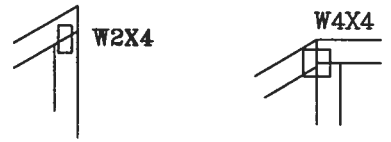
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

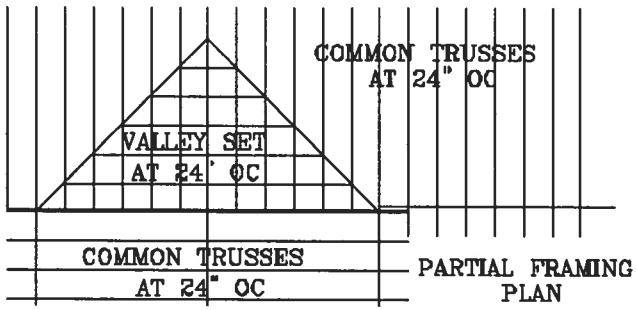
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.







THIS DRAWING REPLACES DRAWING A105

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF VALLEY DETAIL
TC DL	7	15	PSF	DATE 11/26/03
BC DL	5	5	PSF	DRWG VALTRUSS1103
BC LL	0	0	PSF	-ENG JL
TOT. LD.	32	40	PSF	
DUR.FAC.	1.25	1.25		
SPACING		24"		

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-1997 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

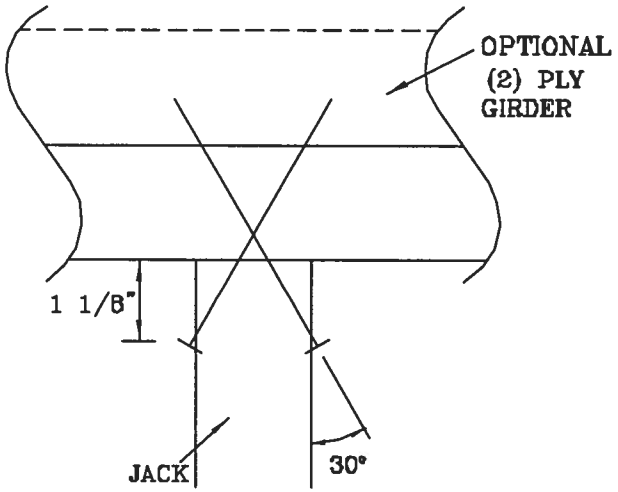
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

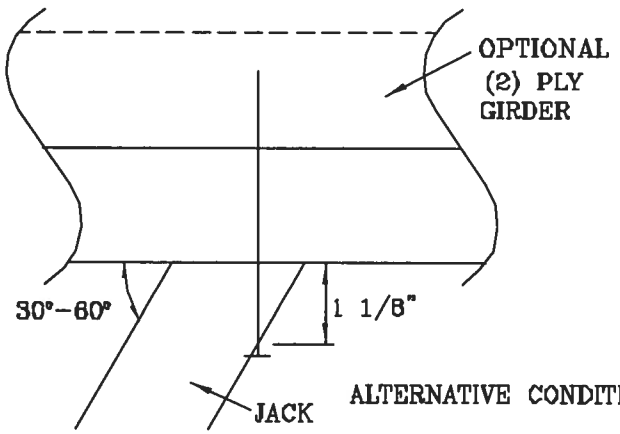
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM LATERAL RESISTANCE OF 16d (0.162"X3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES	1 PLY	2 PLIES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.





ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI. 53719) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DELRAY BEACH, FL 33444-2161
No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

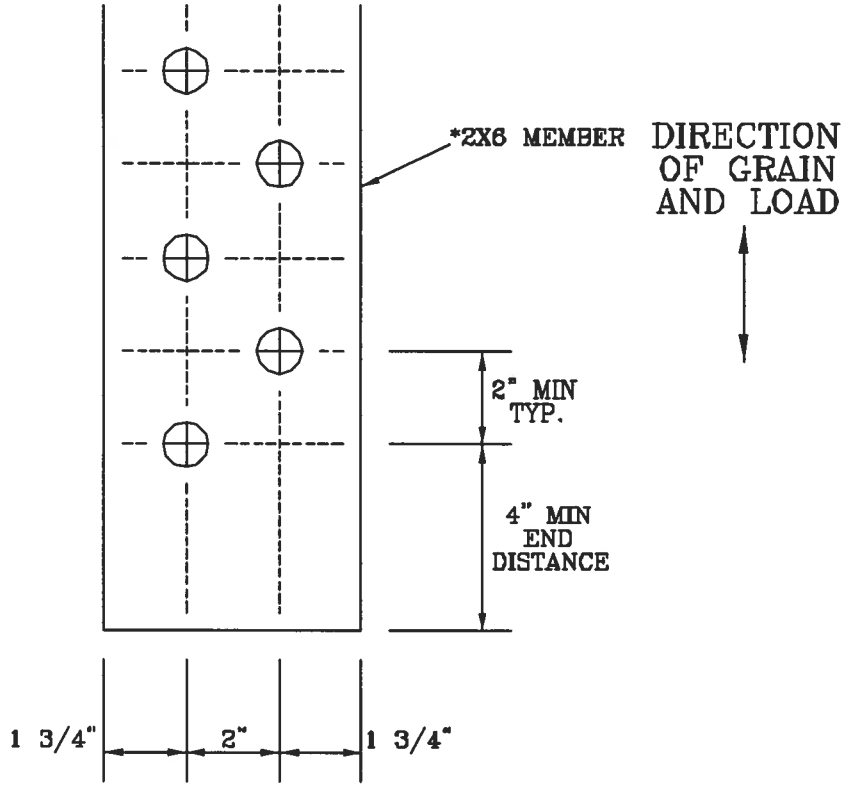
1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

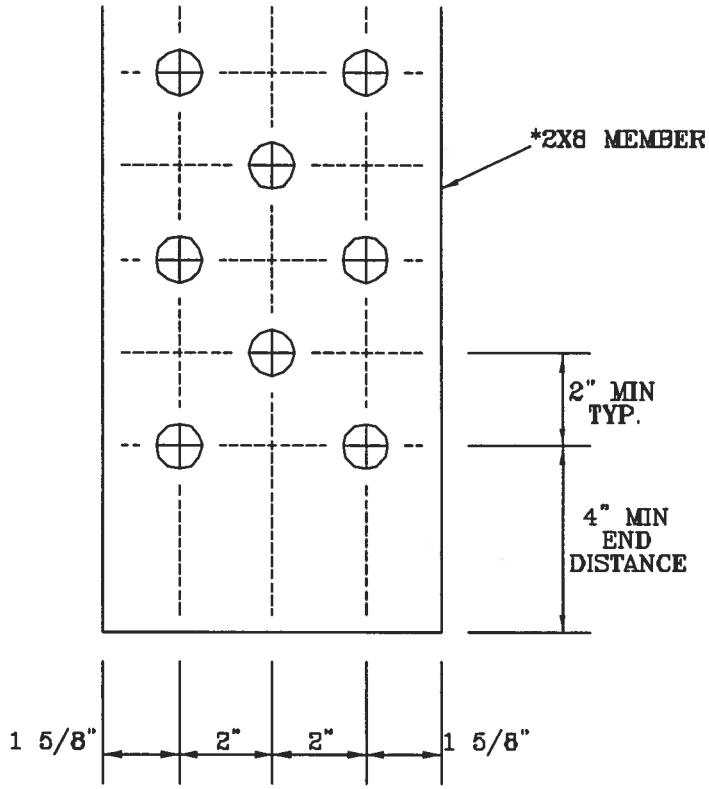
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL



2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016

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No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/09
BC DL	PSF	DRWG	CNBOLTSP1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TRULOX CONNECTION DETAIL

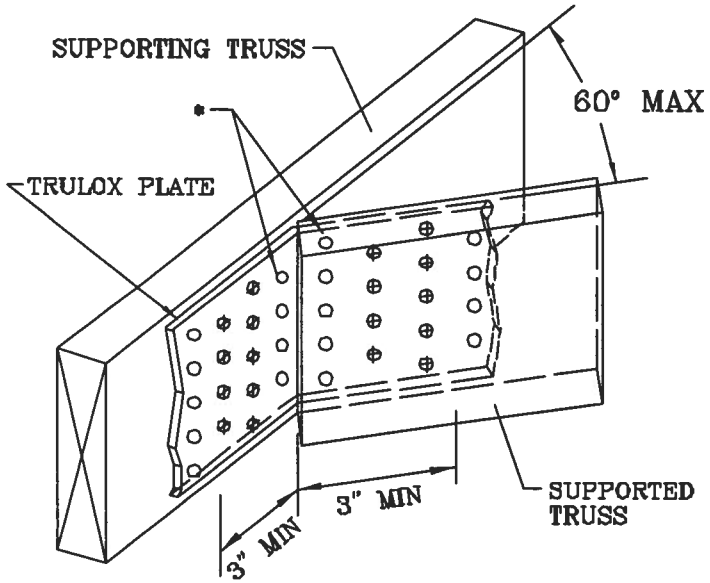
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (⊕).

* NAILS MAY BE OMITTED FROM THESE ROWS.

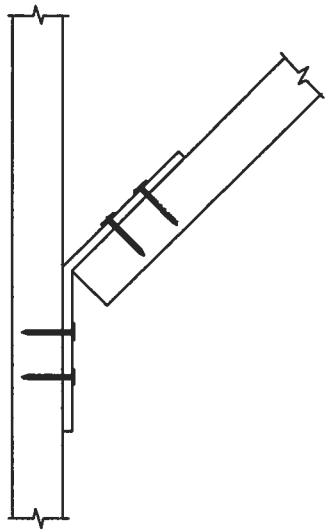
THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

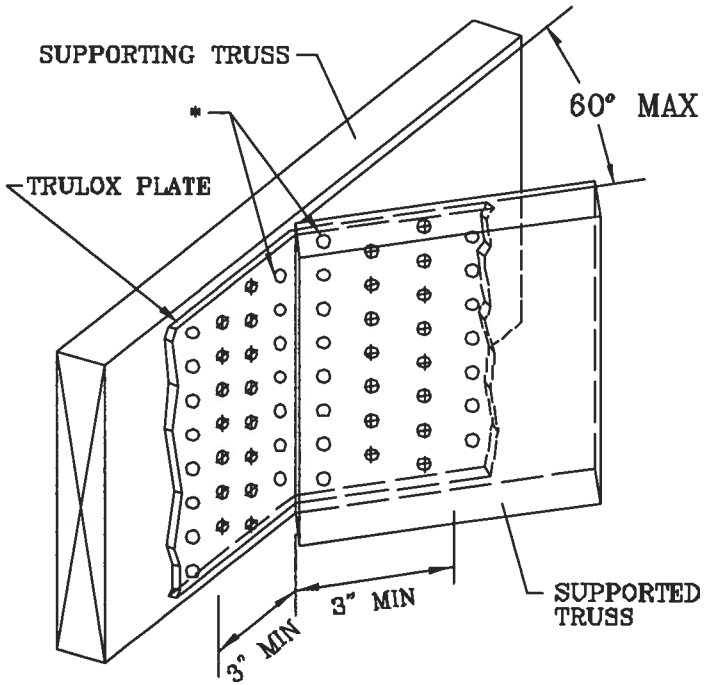
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE





MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

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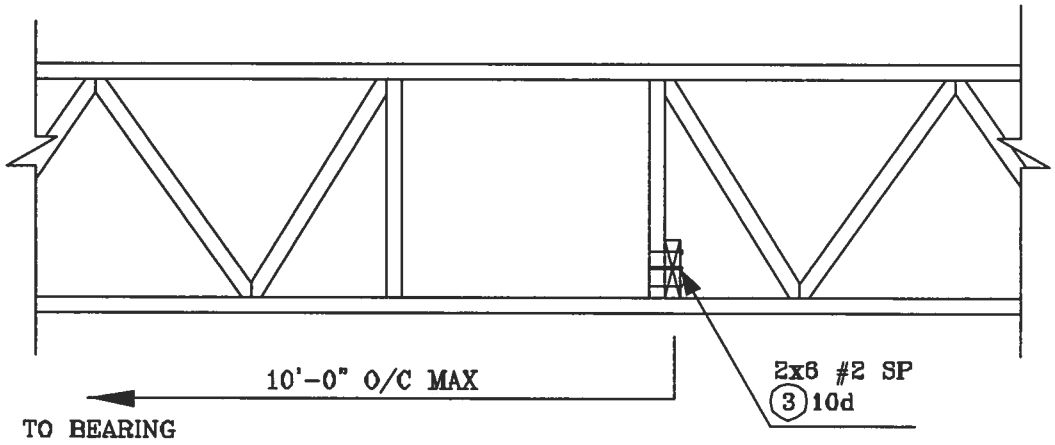
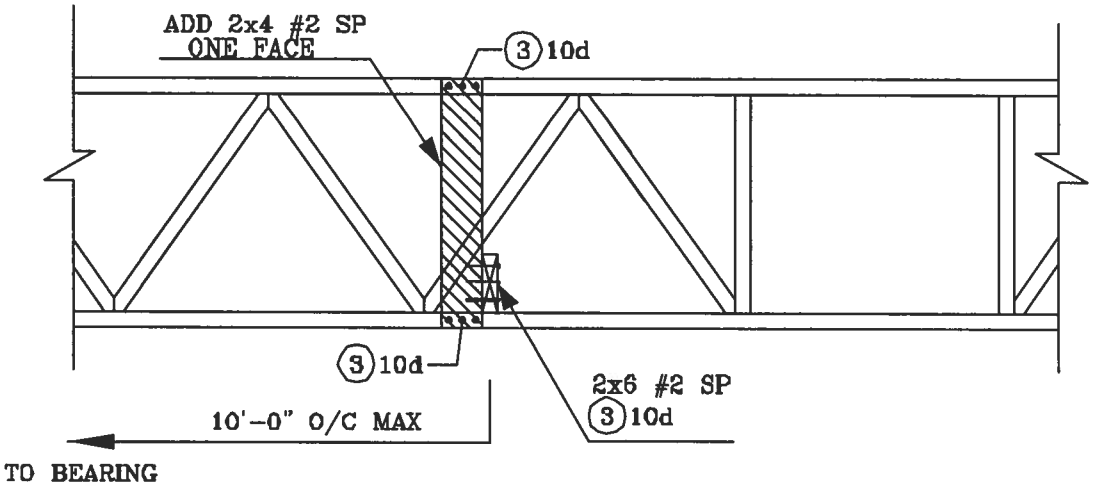
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

STRONG BACK DETAIL
SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR
STRONG BACK WITH VERTICAL
NOT LINING UP



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