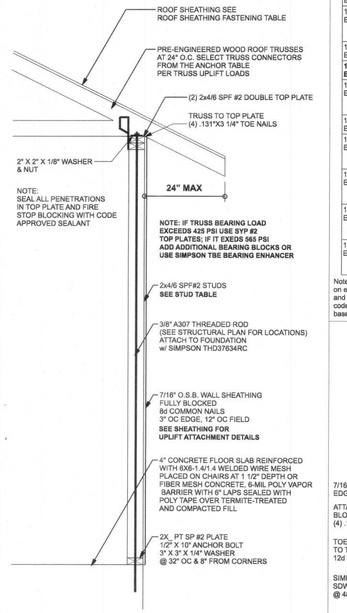


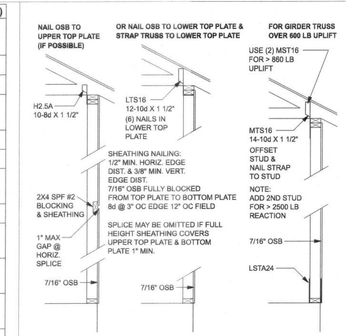
ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

ROOF SHEATHING FASTENING TABLE (RAER / TRUSS SG = 0.49)

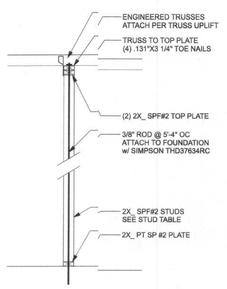
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nailscing aloranel edg	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RSRS-01 (2 3/8" × 0.113")	6" o	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RSRS-01 (2 3/8" × 0.113")	6" o	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RSRS-03 (2 1/2" × 0.131") or ASTM F1667 RSRS-04 (3" × 0.120")	6" o	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RSRS-01 (2 3/8" × 0.113")	6" o	6" oc
130 mph Exp. C	**15/32"**	**ASTM F1667 RSRS-01 (2 3/8" × 0.113")**	**6" o**	**6" oc**
130 mph Exp. D	19/32"	ASTM F1667 RSRS-03 (2 1/2" × 0.131") or ASTM F1667 RSRS-04 (3" × 0.120")	6" o	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RSRS-01 (2 3/8" × 0.113")	6" o	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RSRS-03 (2 1/2" × 0.131") or ASTM F1667 RSRS-04 (3" × 0.120")	6" o	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RSRS-03 (2 1/2" × 0.131") or ASTM F1667 RSRS-04 (3" × 0.120")	6" o	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RSRS-03 (2 1/2" × 0.131") or ASTM F1667 RSRS-04 (3" × 0.120")	6" o	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RSRS-03 (2 1/2" × 0.131") or ASTM F1667 RSRS-04 (3" × 0.120")	4" o	4" oc

Note: For sheathing located a minimum of 4 feet from the perimesdge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be thes on center along panel edges and 6 inches on center along intermediate supports in the panel1. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the athing may need to be increased based in the type of roofing material being used. See manufactu'florida product approval.



SHEATHING FOR UPLIFT ATTACHMENT DETAILS

ONE STORY WOOD FRAME



(TYP.) INTERIOR BEARING WALL

ONE STORY WOOD FRAME w/ RODS

CONNECTOR TABLE

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
815	485	SDWC15600	-	-
415	290	H3	4-8dx1 1/2"	4-8dx1 1/2"
575	495	H2.5A	5-8dx1 1/2"	5-8dx1 1/2"
1340	1015	H10A	9-10d1 1/2"	9-10d1 1/2"
720	620	LTS12-20	6-10d1 1/2"	6-10d1 1/2"
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"
Uplift SP	**Uplift SPF**	**Strap Ties**	**To One Member**	**To Other Member**
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	**Uplift SPF**	**Stud Plate Ties**	**To Stud**	**To Plate**
585	535	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	**Uplift SPF**	**Holdowns @ Stemwall**	**To Stud / Post**	**Anchor**
1825	1800	DTT2Z	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16dx2 1/2"	1/2"x12" Titen HD
Uplift SP	**Uplift SPF**	**Holdowns @ Mono**	**To Stud / Post**	**Anchor**
1825	1800	DTT2Z	8-SDS 1/4"x1 1/2"	1/2"x8" Titen HD
4235	3640	HTT4	18-16dx2 1/2"	1/2"x12" Titen HD
Uplift SP	**Uplift SPF**	**Post Bases @ Stemwall**	**To Post**	**Anchor**
2200		ABU44	12-16d	5/8"x12" Drill & Epoxy
2300		ABU66	12-16d	5/8"x12" Drill & Epoxy
Uplift SP	**Uplift SPF**	**Post Bases @ Mono**	**To Post**	**Anchor**
2200		ABU44	12-16d	5/8"x7" Drill & Epoxy
2300		ABU66	12-16d	5/8"x7" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH), STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE

		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X8 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, Fc = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W MM) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 40 * DB (20" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

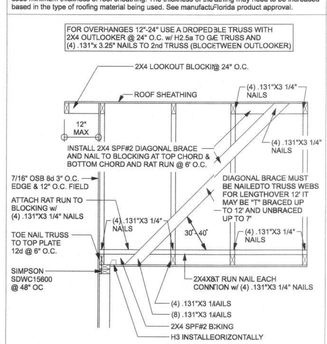
ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

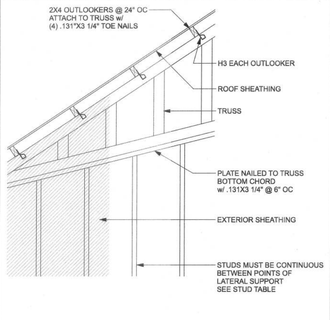
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.
FOR GABLE HEIGHT UP TO 25'-0" 130 MPH, EIC, ENCLOSED

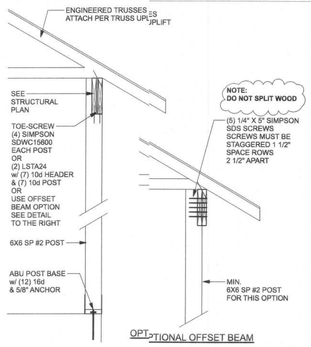
(TYP.) GABLE BRACING ETAIL

WOOD FRAME



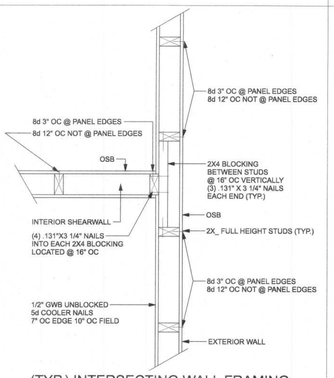
(TYP.) GABLE WALL w/ VAULTED CEILING

WOOD FRAME



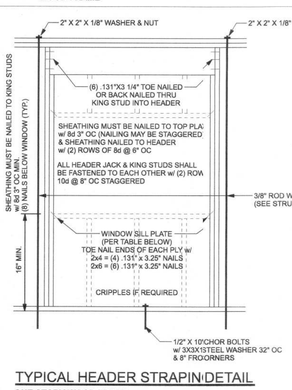
(TYP.) PORCH POST

ONE STORY WOOD



(TYP.) INTERSECTING WALL FRAMING

WOOD FRAME

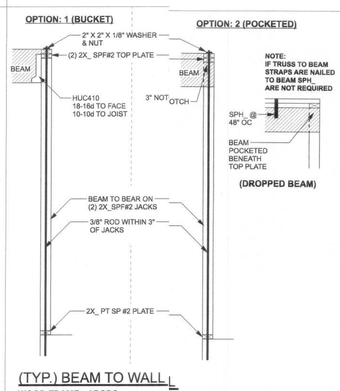


SILL PLATE SPANS FOR 10'-0" WALL HEIGHT

DESIGN WIND SPEED	MAX. SPANS FOR SPF #2 (1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6	BASED ON WFCM TABLE A-3.23B
130 MPH EXP. C	5'-2"	7'-9"	7'-7"	11'-3"	FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)

TYPICAL HEADER STRAPIN DETAIL

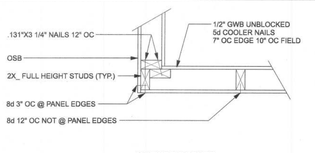
ONE STORY WOOD FRAME



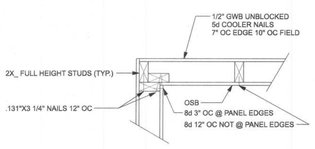
(TYP.) BEAM TO WALL

WOOD FRAME w/ RODS

ALLOWABLE UPLIFT:
1770 LB



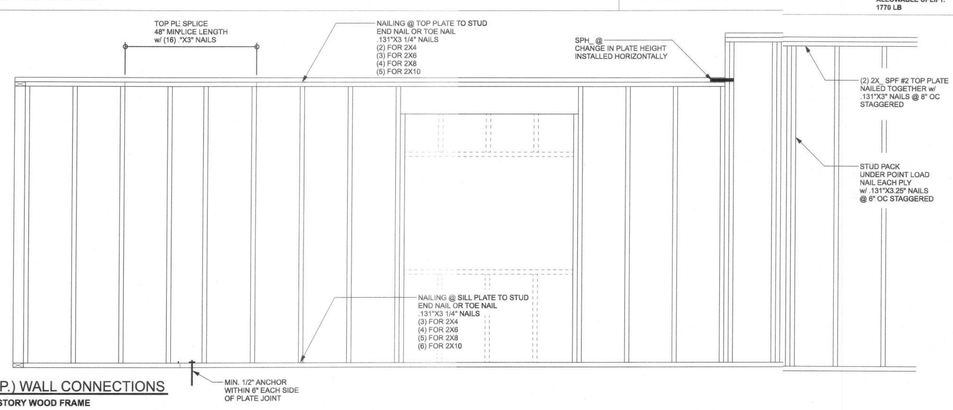
OUTSIDE CORNER



INSIDE CORNER

(TYP.) CORNER FRAMING

WOOD FRAME



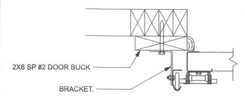
(TYP.) WALL CONNECTIONS

ONE STORY WOOD FRAME

2X6 SP #2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"X4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131X3 1/4" GN PER TABLE BELOW:

DOOR WIDTH	3/8"X4" LAG	16d STAGGER	(2) ROWS OF .131"X3 1/4" NAILS
8' - 10'	24" OC	5" OC	5" OC
11' - 15'	18" OC	4" OC	4" OC
16' - 18'	16" OC	3" OC	3" OC



(TYP.) GARAGE DOOR BUCK INSTALLATION

WOOD FRAME

DESIGN CRITERIA & LOADS:

BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)
CODE FOR DESIGN LOADS	ASCE 7-16
WINDLOADS	
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADING DESIGN PRESSURES 130 MPH (EXP C)

EFFECTIVE WIND AREA (FT2)	ZONE 4 INTERIOR		ZONE 5 END 4' FROM ALL OUTSIDE CORNER	
0 - 20	+25.6(Vasd)	-27.8(Vasd)	+25.6(Vasd)	-34.2(Vasd)
0 - 20	+42.6(Vult)	-48.2(Vult)	+42.6(Vult)	-57(Vult)

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)

9x7 GARAGE DOOR	+22.6(Vasd)	-25.5(Vasd)
16x7 GARAGE DOOR	+21.7(Vasd)	-24.1(Vasd)

Gibraltar Contracting, LLC

Steve & Kendra Condo Res.

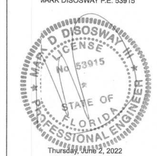
PROJECT ADDRESS:
339 SW Marynik Drive
High Springs, FL 32643

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examinel this plan, and that the applicable portions of the plan, relating to wind engineering comply ith the 7th Edition Florda Building Code Residential (2020) to the bet of my knowledge.

LIMITATION: This design is valid for one building,at specified location.

AARK DISOSWAY P.E. 53915



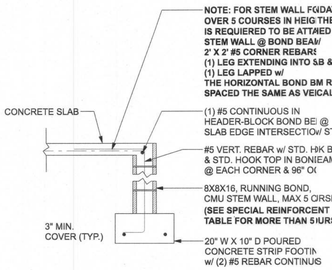
Thursday, June 2, 2022

Mark Disoway P.E.
16 SW Midtown Place
Suite 103
Lale City, Florida 32025
386.754.5419
disoswaydesign@gmail.com

JOB NUMBER:
220651

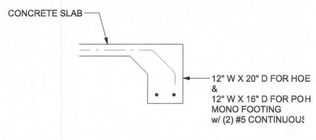
S-1

OF 3 SHEETS



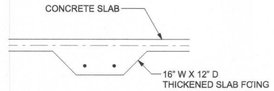
F1 S-2 OPTIONAL STEM WALL FOOTING

SCALE: 1/2" = 1'-0"



F1 S-2 MONOLITHIC FOOTING

SCALE: 1/2" = 1'-0"



F2 S-2 INTERIOR BEARING FOOTING

SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE:

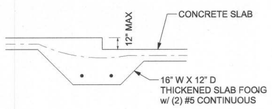
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 802). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

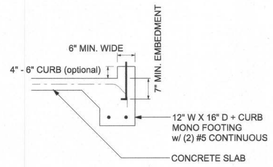
	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls f'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL



F3 S-2 INTERIOR BEARING STEP FOOTING

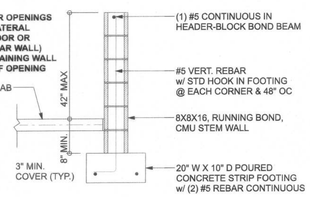
SCALE: 1/2" = 1'-0"



F4 S-2 MONOLITHIC CURB FOOTING

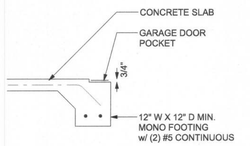
SCALE: 1/2" = 1'-0"

NOTE:
GARAGE DOOR OPENINGS MUST HAVE LATERAL SUPPORT (FLOOR OR PERPENDICULAR WALL) @ TOP OF RETAINING WALL WITHIN 3'-0" OF OPENING



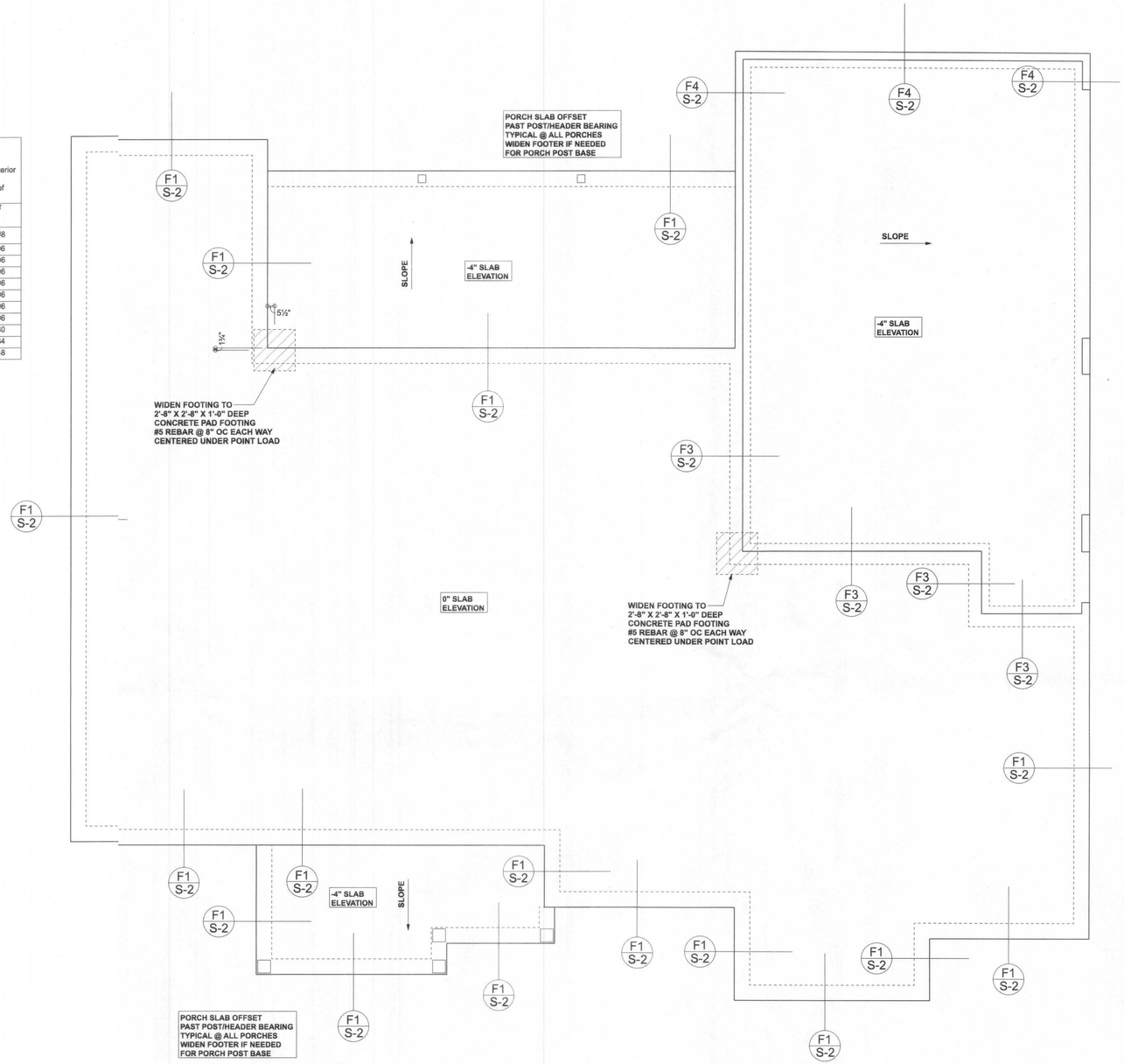
F4 S-2 OPTIONAL STEM WALL CURB FOOTING

SCALE: 1/2" = 1'-0"



F5 S-2 GARAGE DOOR POCKET FOOTING

SCALE: 1/2" = 1'-0"



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES	
FN - 1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, PE IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN - 2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
FN - 3	THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 6X6-1.4/1.4 @ 1'-0" OC, 4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL

Gibraltar Contracting, LLC

Steve & Kendra Condo Res.

PROJECT ADDRESS:
339 SW Manyrik Drive
High Springs, FL 32643

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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Mark Disoway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disoway.

CERTIFIATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the bet of my knowledge.

LIMITATION: This design is valid for one building, t specified location.

MARK DISOSWAY P.E. 53915

MARK DISOSWAY LICENSE No. 53915 STATE OF FLORIDA PROFESSIONAL ENGINEER

Thursday, June 2, 2022

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16 SW Midtown Place
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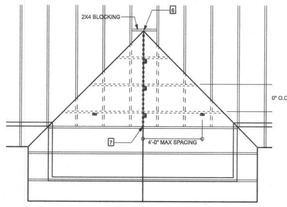
disoswaydesign@gmail.com

JOB NUMBER:
220651

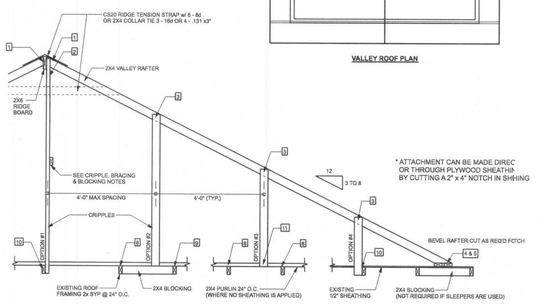
S-2
OF 3 SHEETS

LUMBER SIZE & GRADE MINUMUM REQUIREMENTS

RIDGE BOARD	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
PURLINS / LATERAL BRACING	2X4 SPF #2
SLEEPERS	2X (WIDTH OF RAFTER SEAT CUT) SPF #3 OR 2 PARALLEL 2X4 SPF #3
CRIPPLES & BLOCKING	2X4 SPF #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



VALLEY ROOF PLAN



* ATTACHMENT CAN BE MADE DIREC OR THROUGH PLYWOOD SHEATHI BY CUTTING A 2" x 4" NOTCH IN SHIHING

SECTION CUT PARALLEL TO VALLEY RAFTER

ROOF OVER FRAMING & BRACING DETAIL

SCALE: N.T.S

VALLEY ROOF PLAN MEMBER LEGEND

- TRUSS
- TRUSS UNDER VALLEY FRAMING
- VALLEY RAFTER OR RIDGE
- CRIPPLE

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

CONNECTION REQUIREMENT NOTES

1	2X4 RAFTERS TO RIDGE	3 -16d OR 6 - .131 x 3" TOE NAILS
2	CRIPPLE TO RIDGE	3 - 16d OR 6 - .131 x 3" FACE NAILS
3	CRIPPLE TO RAFTERS	3 - 16d OR 6 - .131 x 3" FACE NAILS
4	RAFTER TO SLEEPER OR BLOCKING	6 -16d OR 12 - .131 x 3" TOE NAILS
5	SLEEPER TO TRUSS	4 - 16d OR 8 - .131 x 3" FACE NAILS EACH TRUSS
6	RIDGE BOARD TO ROOF BLOCK	3 -16d OR 6 - .131 x 3" TOE NAILS
7	RIDGE BOARD TO TRUSS	3 -16d OR 6 - .131 x 3" TOE NAILS
8	PURLIN TO TRUSS (TYP.)	3 -16d OR 6 - .131 x 3" NAILS
8	PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4 -16d OR 8 - .131 x 3" NAILS
9	TRUSS TO BLOCKING	3 -16d OR 6 - .131 x 3" END NAILS
10	CRIPPLE TO TRUSS	3 -16d OR 6 - .131 x 3" FACE NAILS
11	CRIPPLE TO PURLIN	3 -16d OR 6 - .131 x 3" FACE NAILS

GENERAL NOTES

MAXIMUM RAFTER SPANS
6'-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2.
MAXIMUM ROOF AREA PER SUPPORT
WIND ZONE 1 - 2482 sq. ft. ZONE 1: (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN
WIND ZONE 2 & 3 - 1241 sq. ft. ZONE 2 & 3
PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.
PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM.
IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 6", AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 8 - 8d COMMON WIRE NAILS.
THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
- SPANS (DISTANCES BETWEEN HEELS) 40'-0" OR LESS
- ROOF SLOPES 3:12 MINIMUM, 12:12 OR LESS
- MAXIMUM WIND SPEED: 130 MPH
- MAXIMUM MEAN ROOF HEIGHT: 30 FEET
- MAXIMUM TOTAL LOADING: 40 psf
- MEETS FBC 2014/ASCE 7-10 WIND REQUIREMENTS
- EXPOSURE CATEGORY "C", I = 1.0, Kzt = 1.0
- ENCLOSED BUILDING

CRIPPLE, BRACING, & BLOCKING NOTES

-2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG NAILED w/ 2 - 10d NAILS OR 2X4 "T" OR SCAB BRACE NAILD TO FLAT EDGE OF CRIPPLE WITH 8d NAILS @ 8" O.C. "T" OR SCAB MUST BE 80% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO CLB's OR BOTH FACES w/ "T" OR SCAB. USE STRESS GRADE 2X4 SPF #2 OR BETTER FOR CRIPPLES.
- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD
- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.
- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.
- APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.

FRAMING PLAN

DORMER ANCHORING DETAIL (ON ROOF

SCALE: N.T.S.

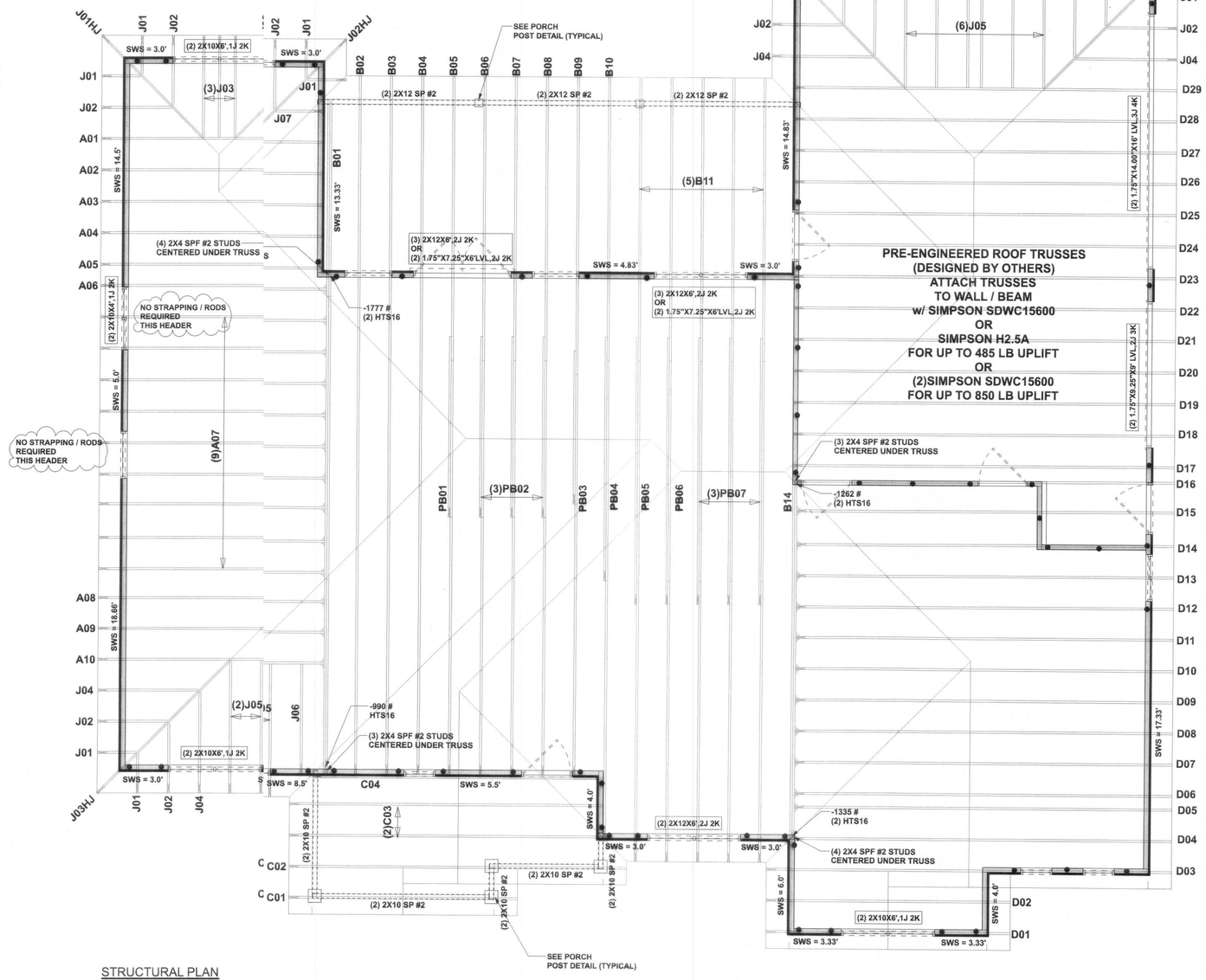
DOER ROOF:
ROWG (BY OWNER)

15lbLT OVER 7/16" OSB

TRIES / RAFTERS (SEE RO'RAMING PLAN FOR SIZ'SPACING) TO RIDGE BO'(FULL DEPTH OF RAFR CUT).

CEI6 JOISTS

CEI6 FINISH (BY OWNER)



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL LL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 10 SP #2 (U.N.O.)

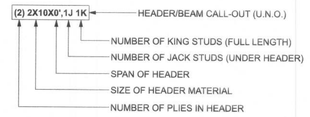
SN-2 ALL LOAD BEARING FRAME WALL LL HEADERS SHALL HAVE (1) JACK STUD & (1) 1) KING STUD EACH SIDE (U.N.O.)

SN-3 USE ONE JACK STUD GIRDER SU SUPPORT PER 2500 LB LOAD

SN-4 DIMENSIONS ON STRUCTURAL SI SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-5 PERMANENT TRUSS BRACING IS IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE S SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RES STRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BC CSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS S SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND



THREADED ROD LEGEND

INDICATES LOCATION OF: 3/8" A307 ALL THREADED ROD

ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDUNAL
ACTUAL	23436 LBF	17598 LBF
REQUIRED	16017 LBF	14148 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. W.B. HOWLAND TRUSS CO. JOB #22-7286

Gibraltar Contracting, LLC

Steve & Kendra Condo Res.

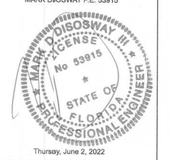
PROJECT ADDRESS:
2351 Alaska Drive
High Springs, FL 32643

DIMENSIONS:
Stated dimension supercede scaled dimensions. Refeall questions to Mark Disoway, PE. for resolution. Do not proceed whout clarification.

CERTIFICATION: hereby certify that I have examined this pla, and that the applicable portions of the pla, relating to wind engineering comply with the 7t Edition Florida Building Code Redential (2020) to the best of my lowledge.

LIMITATION: Thisdesign is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915



Thursay, June 2, 2022

Mark Disosway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disoswaydsign@gmail.com

JOB NUMBER: 20651

S-3

OF3 SHEETS

REAR ELEVATION

SCALE: 1/4" = 1'-0"

FRONT ELEVATION

SCALE: 1/4" = 1'-0"

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

REVISIONS
June 06, 2022

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

FRONT & REAR ELEVATIONS
SCALE: 1/4" = 1'-0"

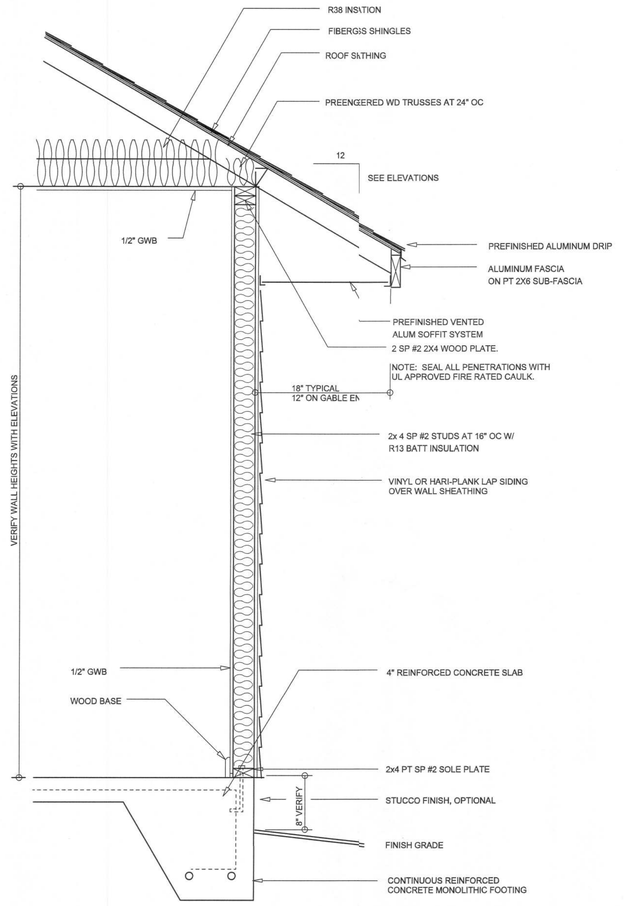
A CUSTOM HOME FOR:
SIEVE & KENDRA CONDO
Property Address: 339 SW Marymik Drive, High Springs, Florida
GIBRALTAR CONTRACTING, LLC.
LIC# 1259633 HIGH SPRINGS, FLORIDA

© WM DESIGN & ASSOCIATES, INC.
426 SW COMMERCE DR, STE 130
LAKE CITY, FL 32025
386) 758-8406
will@willmyers.net

JOB NUMBER
20220212

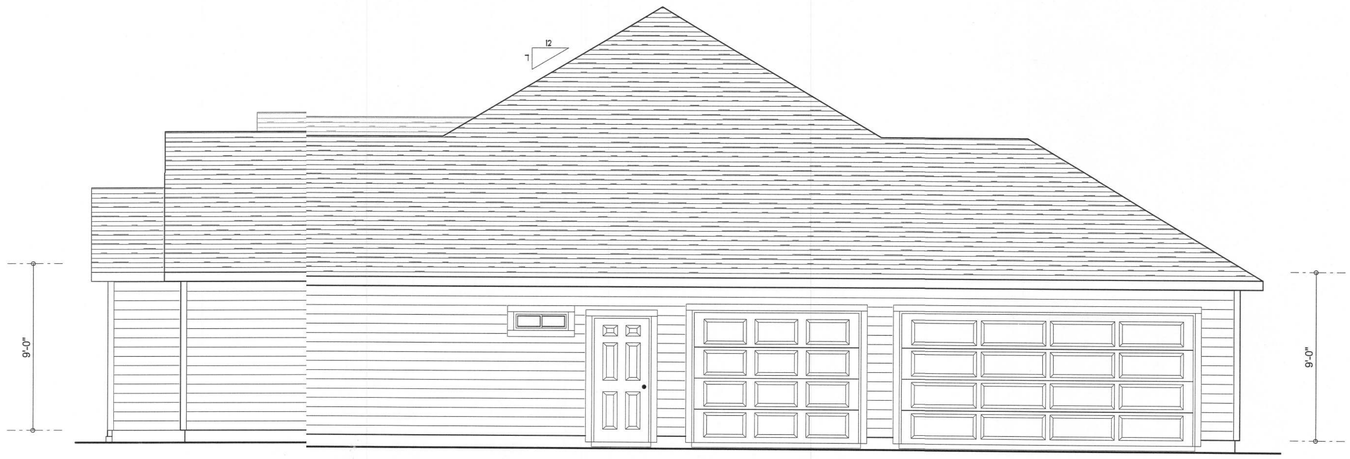
SHEET NUMBER
A.1

COLUMBIA COUNTY BUILDING DEPARTMENT
Received for FILE COPY
Code Compliance
PLANS EXAMINER



TYPICAL WALL SECTION

SCALE: 1" = 1'- 0"



RIGHT ELEVATION

SCALE: 1/4" = 1'-0"

LEFT ELEVATION

SCALE: 1/4" = 1'-0"

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

REVISIONS
June 03, 2022

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

LEFT & RIGHT ELEVATIONS
SCALE: 1/4" = 1'-0"

TYPICAL WALL SECTION
SCALE: 1" = 1'-0"

A CUSTOM HOME FOR:
STEVE & KENDRA CONDO
Property Address: 339 SW Marynik Drive, High Springs, Florida
GIBRALTAR CONTRACTING, LLC.
LIC# 1259633 HIGH SPRINGS, FLORIDA

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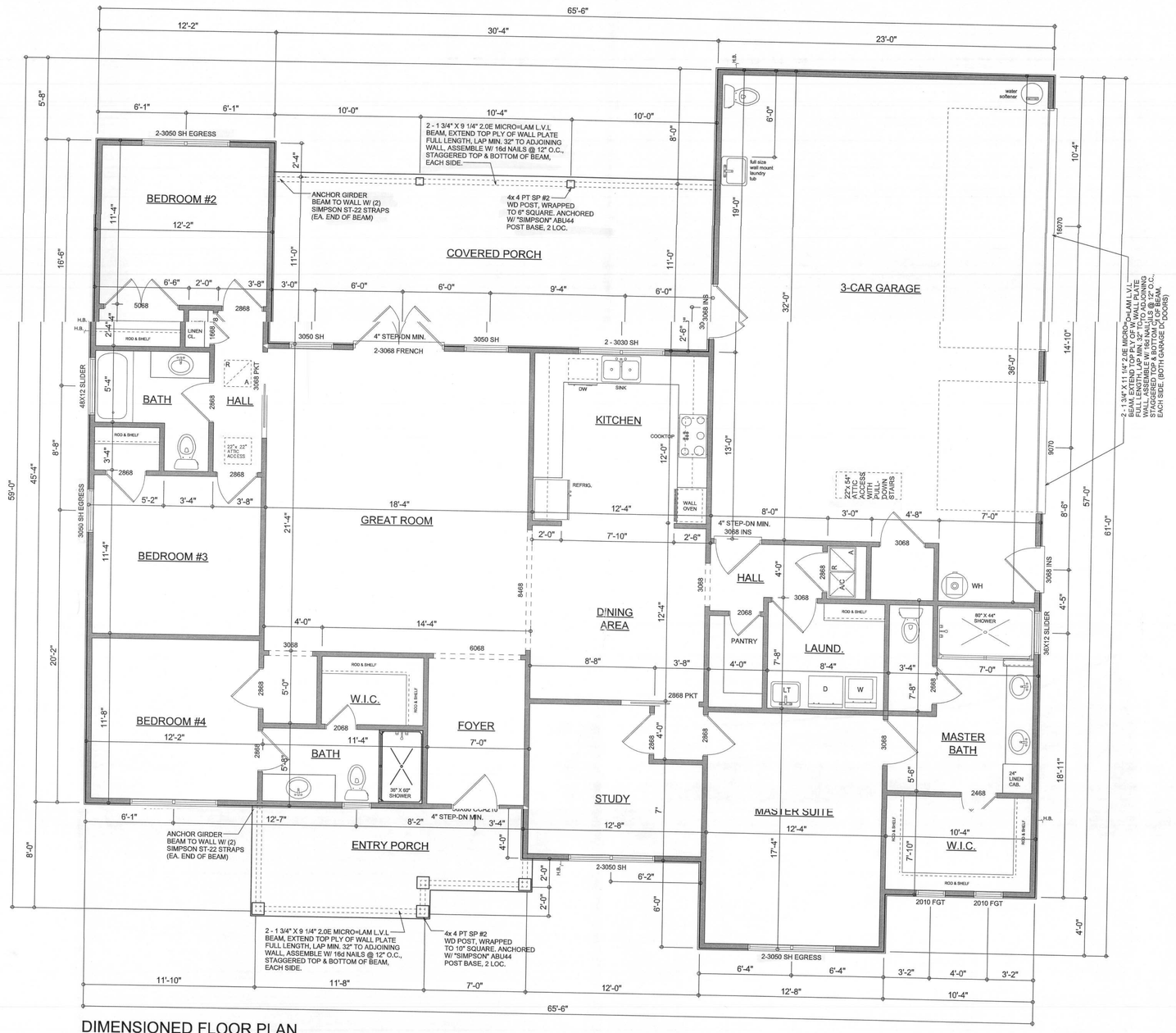
JOB NUMBER
20220212

SHEET NUMBER
A.2



DIMENSIONED FLOOR PLAN

SCALE: 1/4" = 1'-0"

NOTE: ALL WALLS SHALL BE 9'-0" UNLESS OTHERWISE NOTED.

Garage fire separations shall comply with the following:

1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 13/8 inches (34.9 mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted.
2. Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.
3. A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.
4. When installing an attic access and/or pull-down stair unit in the garage, devise shall have a minimum 20 min. fire rating.

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

AREA SUMMARY

LIVING AREA	2,155	S.F.
GARAGE AREA	777	S.F.
COVERED PORCH AREA	334	S.F.
ENTRY PORCH AREA	134	S.F.
TOTAL AREA	3,400	S.F.

REVISIONS
June 06, 2022

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

DIMENSIONED FLOOR PLAN
SCALE: 1/4" = 1'-0"

A CUSTOM HOME FOR
STEVE & KENDRA CONDO
Property Address: 339 S Marynik Drive, High Springs, Florida

GIBRALTAI CONTRACTING, LLC.
LIC# 1259633 HIGHPRINGS, FLORIDA

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JOB NUMBER
20220212

SHEET NUMBER
A.3

	ELECTRICAL LEGEND
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
SEC	DOUBLE SECURITY LIGHT
	RECESSED CAN LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET (AFCI & TAMPER RESISTANT)
	220v OUTLET
GFI	GFI DUPLEX OUTLET (PER NEC 406.8)
TV	TELEVISION JACK
	SPECIALTY CIRCUIT AS REQ'D. VERIFY W/ EQUIP.
	SMOKE / CARBON MONOXIDE DETECTOR (see note below)
$	WALL SWITCH
$3	3 WAY WALL SWITCH
WP/GFI	WATER PROOF GFI OUTLET
48" FLOUR.	2 OR 4 LED STRIP FIXTURE

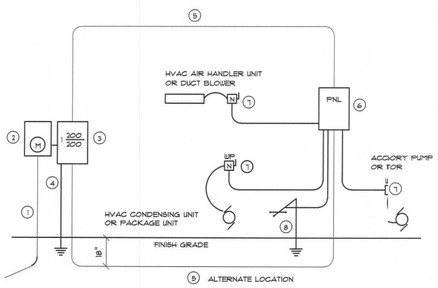
NOTE:
ALL INTERIOR RECEPTACLES SHALL BE AFCI
(ARC FAULT CIRCUIT INTERRUPT) PER NEC 210.12 & TAMPER RESISTANT PER
NEC 406.11

ALL INTERIOR & EXTERIOR LIGHTING SHALL MEET OR EXCEED THE MIN. 75% HIGHEFFICIENCY
LIGHTING PER FBC-ENERGY CONSERVATION R404.

ALL SMOKE DETECTORS BE A COMBO SMOKE & CARBON MONOXIDE DETECTOR
AND SHALL HAVE BATTERY BACKUP POWER
AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY
ALL ACTIVATE.

THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE
INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MNS.
CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANELS SUB
PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR
SHALL BE USED AS AN EQUIPMENT GROUND.

IT IS THE LICENSED ELECTRICAL CONTRACTORS RESPONSIBILITY TO INSURE THALL
WORK PERFORMED AND EQUIPMENT INSTALLED MEETS OR EXCEEDS THE 2017 'PA-70) NATIONAL
ELECTRIC CODE AND ALL OTHER LOCAL CODES AND ORDINANCES.



① Service/Feeder Entrance Conductors: 2" rigid conduit, min. 18" deep, w/ continuous Ground Bonding Conductor, Service/ Entrance Conductors shall not be spliced except that bolted connections at the Meter, Disconnecting Devices and Panel shall be allowed.

② Meter Enclosure, weatherproof, U.L. Listed.

③ Main Disconnect Switch: fused or Main BRKR, weatherproof, U.L. Listed.

④ Service entrance Ground: 1" ~ iron/steel rod x 8'-0" long and/or concrete encased foundation steel rebar x 20'-0" long. Grounding Conductor shall be bonded to each piece of Service/ Entrance Equipment, and shall be sized per item #5, below.

⑤ 200 AMPERE SERVICE: 3-#2/0-USE-Cu, 1-#4-Cu-GND, 2" Conduit.

⑥ House Panel (FNL), U.L. Lised, sized per schedule.

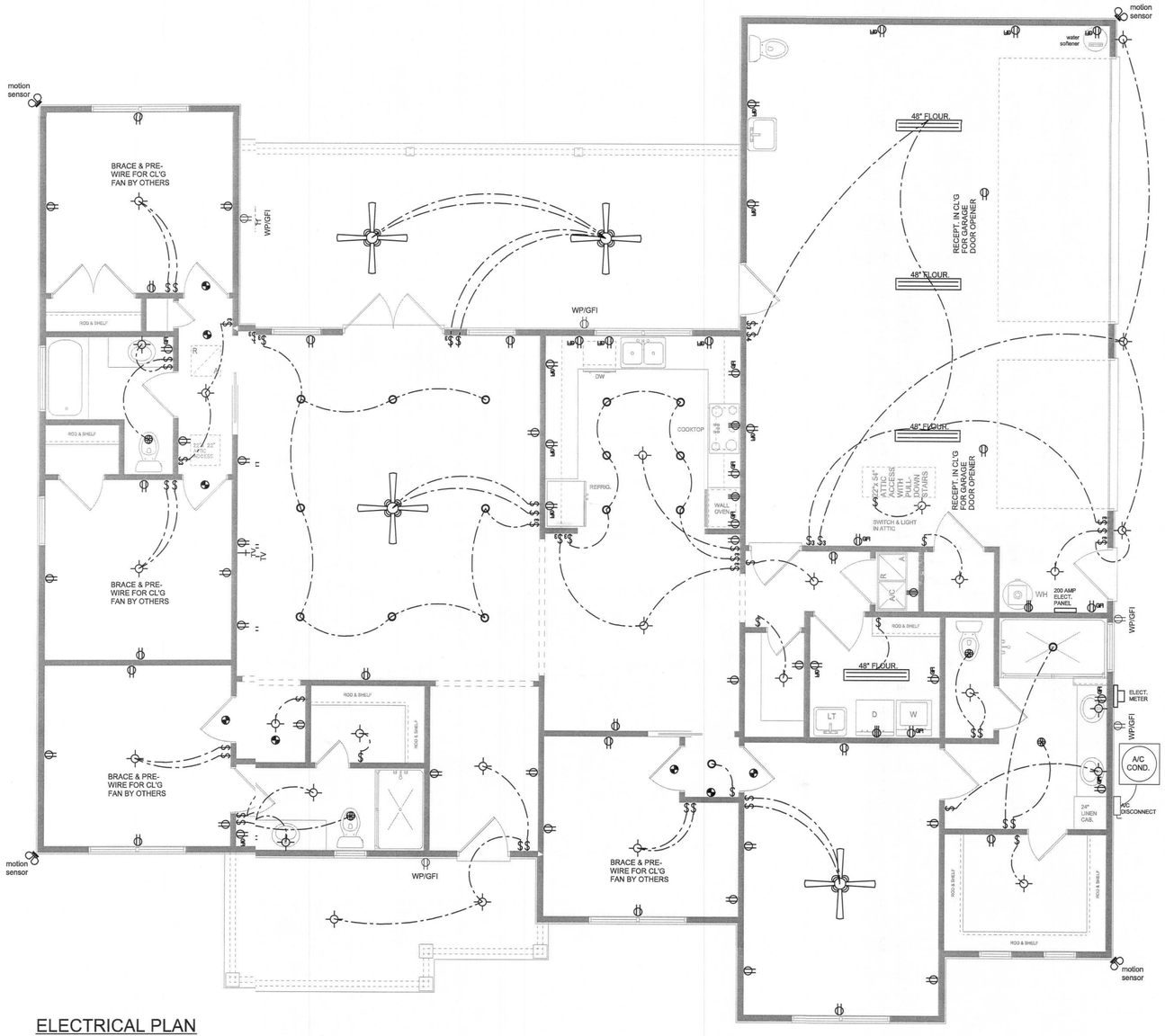
⑦ Equipment Disconnect Switch: non-fused, in weatherproof enclosure, size according to Panel Schedule loads.

⑧ Provide Ground Bond Wire to metal piping, size in accordance with the Service Ground Conductor.

NOTE:
THE MINIMUM AIC RATING FOR PANEL BOARDS, BRKRS
AND DISCONNECT SWITCHES SHALL BE 22,000 AIC.

ELECTRICAL RISER DIAGRAM: 200A

SCALE: NONE



ELECTRICAL PLAN

SCALE: 1/4" = 1'-0"

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

REVISIONS
June 05, 2020

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

A CUSTOM HOME FOR:
STEVE & KENDRA CONDO
Property Address: 339 SW Marynik Drive, High Springs, Florida
LIC# 1259633

GIBRALTAR CONTRACTING, LLC.
HIGH SPRINGS, FLORIDA

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(386) 758-8406
wl@willmyers.net

JOB NUMBER
20220212

SHEET NUMBER
A.4