DATE 11/21/2005

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000023879

APPLICANT JUANITA BRADLEY PHONE 386.497.3520

ADDRESS POB 337 FT. WHITE FL 32038

OWNER JAMES & JUANITA BRADLEY PHONE 386.497.3520

ADDRESS 279 SW MITT COURT FT. WHITE FL 32038

CONTRACTOR JAMES BRADLEY PHONE 497.3520

LOCATION OF PROPERTY FT WHITE EAST ON CR 18 TO 1 1/2 MILES TO MITT CT ON THE NORTH SIDE, DRIVE UP TO THE HOME.

TYPE DEVELOPMENT DETACHED GARAGE ESTIMATED COST OF CONSTRUCTION 10000.00

HEATED FLOOR AREA TOTAL AREA HEIGHT 11.40 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING A-3 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 1 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 34-6S-16-04055-001 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES 30.00

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor
			Juanita A Bradley

Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident
EXISTING	X-05-0330	BLK	JTH	N

COMMENTS: DETACHED GARAGE.

Check # or Cash 1883

FOR BUILDING & ZONING DEPARTMENT ONLY

Temporary Power date/app. by Foundation date/app. by Monolithic (footer/Slab) date/app. by

Under slab rough-in plumbing date/app. by Slab date/app. by Sheathing/Nailing date/app. by

Framing date/app. by Rough-in plumbing above slab and below wood floor date/app. by

Electrical rough-in date/app. by Heat & Air Duct date/app. by Peri. beam (Lintel) date/app. by

Permanent power date/app. by C.O. Final date/app. by Culvert date/app. by

M/H tie downs, blocking, electricity and plumbing date/app. by Pool date/app. by

Reconnection date/app. by Pump pole date/app. by Utility Pole date/app. by

M/H Pole date/app. by Travel Trailer date/app. by Re-roof date/app. by

BUILDING PERMIT FEE $ 50.00 CERTIFICATION FEE $ .00 SURCHARGE FEE $ .00

MISC. FEES $ .00 ZONING CERT. FEE $ 50.00 FIRE FEE $ .00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 125.00

INSPECTORS OFFICE CH CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only Application # 0511-65 Date Received 11/17 By JN Permit # 23879
Application Approved by - Zoning Official BLK Date 12.11.05 Plans Examiner OK JTH Date 11-17-05
Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3
Comments NOC

Applicants Name James M. Bradley Phone 386-497-3520 1st. Inq: 386. 365. 0046
Address P.O. Box 337
Owners Name James M. Bradley Phone
911 Address 279 S.W. MITT COURT. FT. WHITE, FL 32038
Contractors Name OWNER/BUILDER Phone
Address
Fee Simple Owner Name & Address James M. Bradley
Bonding Co. Name & Address NA
Architect/Engineer Name & Address MARTY ESKRIDGE 14952 MAIN ST ALACHUA, FL 386 462.1340
Mortgage Lenders Name & Address None

Property ID Number ✓ 34-6S-16-04055-001 Estimated Cost of Construction $10,000
Subdivision Name — Lot ____ Block ____ Unit ____ Phase ____
Driving Directions FT. WHITE EAST ON CR 18 1 1/2 miles S.W. MITT COURT ON the North Side - Drive up to home -

Type of Construction FRAME - DETACHED GARAGE Number of Existing Dwellings on Property 1
Total Acreage 30 Ac. Lot Size ____ Do you need a - Culvert Permit or Culvert Waiver or ✓ Have an Existing Driv
Actual Distance of Structure from Property Lines - Front 700' Side 300 Side 400 Rear 700'
Total Building Height 11'4" Number of Stories 1 Heated Floor Area 0 Roof Pitch 6/12
GARAGE 672 SQ FT TOTAL 672

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

James M. Bradley
Owner Builder or Agent (Including Contractor) Contractor Signature

GALE TEDDER MY COMMISSION # DD 0 EXPIRES: June 28, 2008 Bonded Thru Notary Public Underwriters

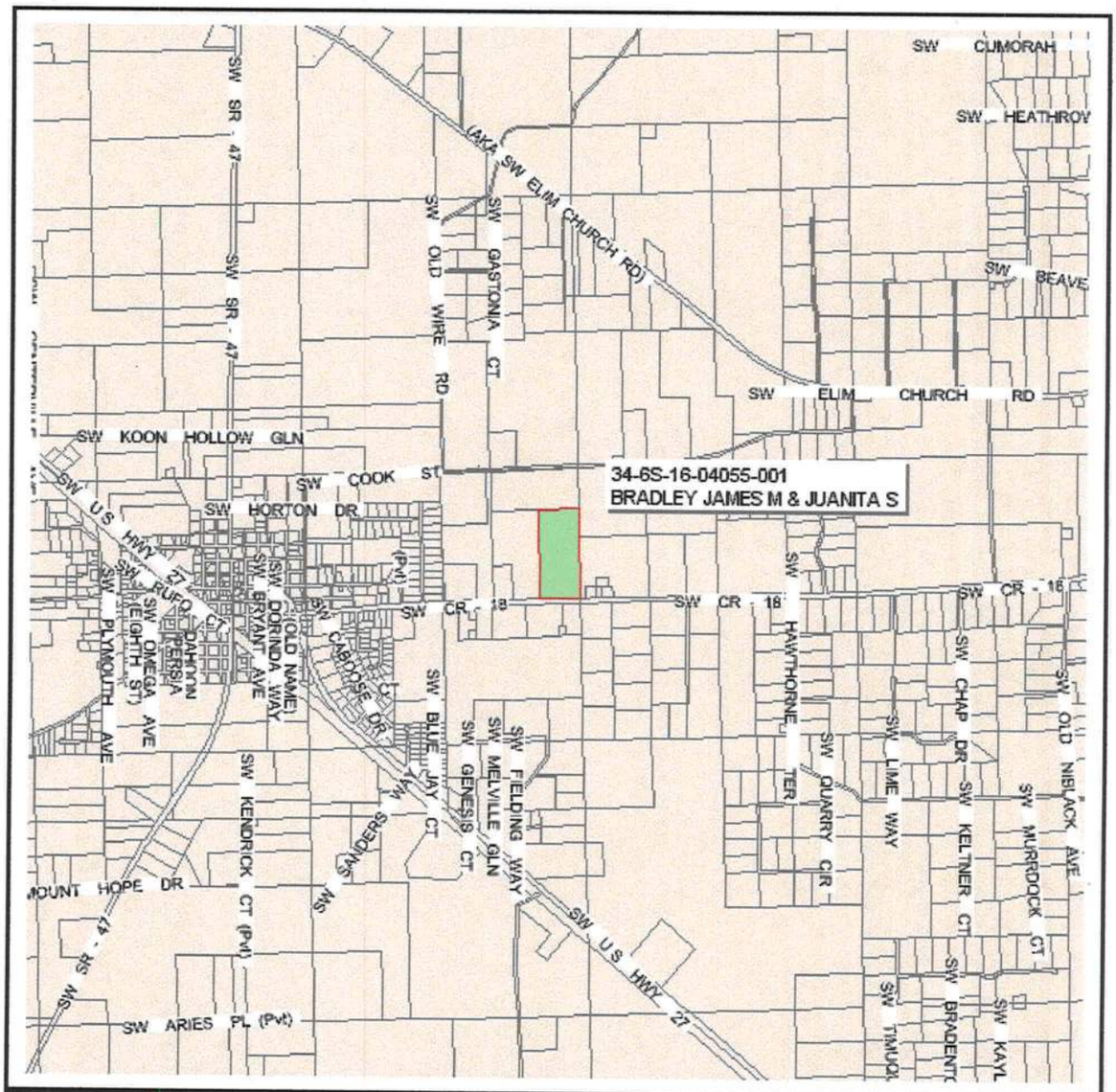
STATE OF FLORIDA
COUNTY OF COLUMBIA

Contractors License Number
Competency Card Number

Sworn to (or affirmed) and subscribed before me
this 17th day of November 20 05.
Personally known ____ or Produced Identification DL

NOTARY STAMP/SEAL
Gale Tedder
Notary Signature

JN LEFT MESSAGE 11.17.05



Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

PARCEL: 34-6S-16-04055-001 HX - IMPROVED A (005000)

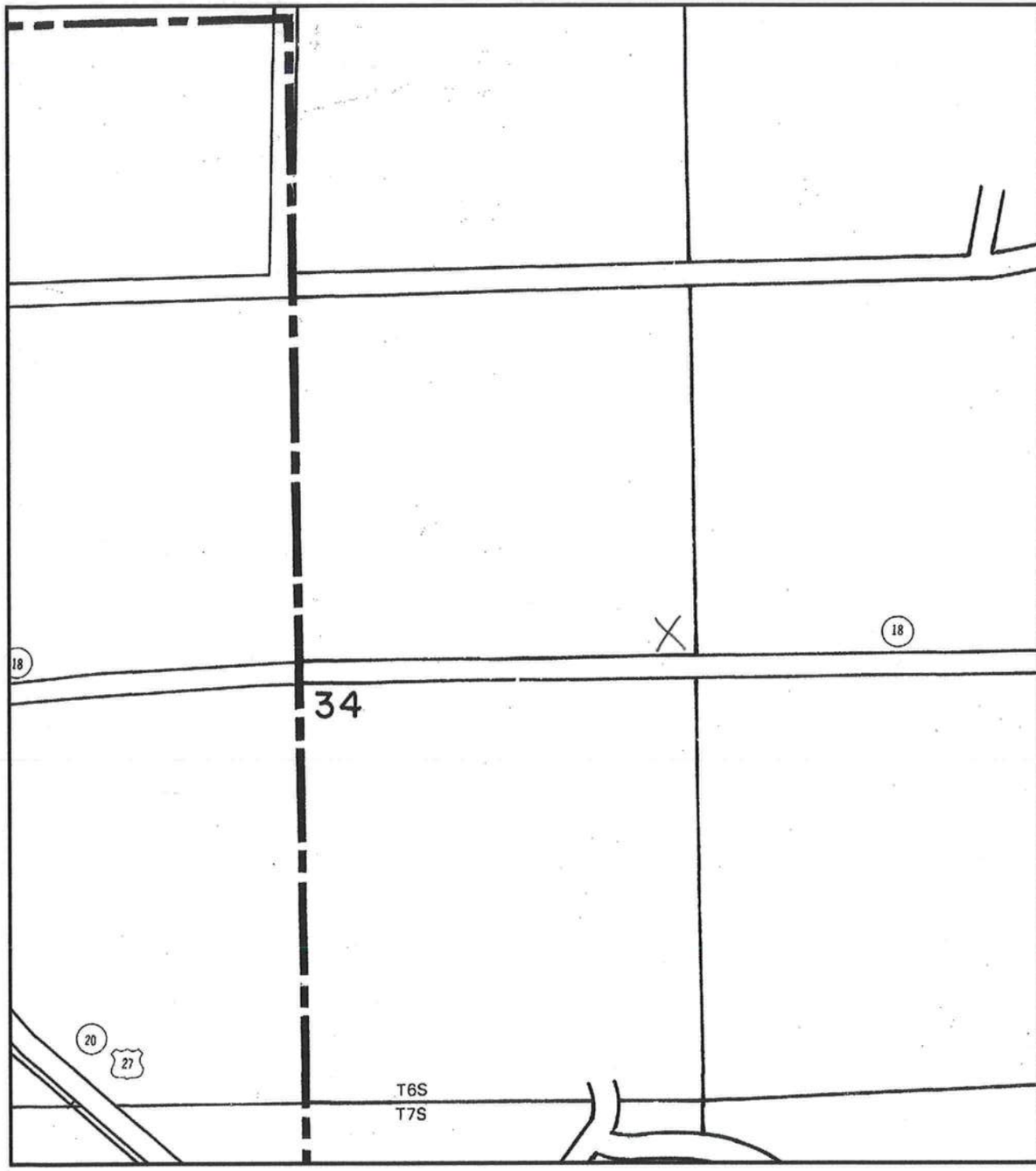
COMM NE COR, RUN S 783.55 FT FOR POB, RUN W 762.40 FT, S 1711.23 FT, E 762.38 FT, N

Name:	BRADLEY JAMES M & JUANITA S		LandVal	$22,400.00
Site:			BldgVal	$194,815.00
Mail:	P O BOX 337 FT WHITE, FL 320380337		ApprVal	$233,728.00
			JustVal	$286,902.00
Sales	1/1/2001	$0.00 V / U	Assd	$221,121.00
Info	9/30/1994	$0.00 V / U	Exmpt	$25,000.00
			Taxable	$196,121.00

0 0.2 0.4 0.6 mi

N W E S

This information, GIS Map Updated: 8/3/2005, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.



APPROXIMATE SCALE IN FEET

1000 0 1000

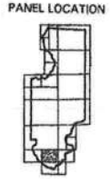
NATIONAL FLOOD INSURANCE PROGRAM

FIRM
FLOOD INSURANCE RATE MAP

COLUMBIA COUNTY, FLORIDA
(UNINCORPORATED AREAS)

PANEL 260 OF 290

PANEL LOCATION



COMMUNITY-PANEL NUMBER
120070 0260 B

EFFECTIVE DATE:
JANUARY 6, 1988

Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT Version 1.0. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. Futher information about National Flood Insurance Program flood hazard maps is available at www.fema.gov/mit/tsd.

Print Date: 11/17/2005 (printed at scale and type A)

DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed $25,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F .I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

TYPE OF CONSTRUCTION

( ) Single Family Dwelling
( ) Farm Outbuilding
( ) New Construction
( ) Two-Family Residence
(✓) Other Detached Garage
( ) Addition, Alteration, Modification or other Improvement

NEW CONSTRUCTION OR IMPROVEMENT

I James M. Bradley **, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number_____**

James M. Bradley 11-17-2005
Signature **Date**

FOR BUILDING USE ONLY

I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7).

Date 11-17-2005 **Building Official/Representative** Daniel L Crumie

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                    Print Key Output
5722SS1 V5R3M0 040528                S109D45B            11/17/05  Page   1
                                                                   11:12:00
Display Device  . . . . . :  WA
User  . . . . . . . . . . :  GAIL

@ CAM112M01     S  CamaUSA Appraisal System                    Columbia County
11/17/2005 11:11   Legal Description Maintenance     26720  Land  002  *
Year T Property                     Sel              4397  AG    001
2006 R 34-6S-16-04055-001                ...        209276  Bldg  002
                                                     11399  Xfea  002
HX      BRADLEY JAMES M & JUANITA S                 251792  TOTAL      B

  1  COMM NE COR, RUN S 783.55 FT     FOR POB, RUN W 762.40 FT, S      2
  3  1711.23 FT, E 762.38 FT, N       1715.40 FT TO POB. ORB 796-977   4
  5  & PROB DIST CS#00-19-CP          QCD 1025-2143.                   6
  7                                                                    8
  9                                                                   10
 11                                                                   12
 13                                                                   14
 15                                                                   16
 17                                                                   18
 19                                                                   20
 21                                                                   22
 23                                                                   24
 25                                                                   26
 27                                                                   28
                                          Mnt  9/30/2004 KYLIE
F1=Task  F3=Exit  F4=Prompt  F10=GoTo  PgUp/PgDn  F24=More
```

RESIDENTIAL WIND DESIGN & ANALYSIS

\ V=110 \ ***NO COPIES ARE TO BE PERMITTED***

PREPARED FOR:

ROSENBOOM CONSTRUCTION \ THE BRADLEY GARAGE

PREPARED BY:

MARTY R. ESKRIDGE
14952 MAIN ST
ALACHUA FL 32615
386-462-1340 / 352-375-6329

OFFICE COPY

SUMMARY
OF WIND DESIGN & ANALYSIS

Trusses: Lumber type So. Pine Grade #1 #2 #3 Size 2 x 4 Spacing 24 in.

Hurricane anchors: Interior: Mfr * Model *
End: Mfr * Model *

Roof sheathing: Type OSB Size 7/16 Fastener type Nails Size 8d/131
Interior zone spacing: Interior 8 in. Periphery 4 in.
Edge and end zone spacing: Interior 8 in. Periphery 4 in.

Top double pl: Type Spruce Grade #1 #2 Size 2 x 4 Nail spacing 12 in.

Studs: Wood or Steel: Wood Type Spruce Grade #1 #2 Size 2 x 4
Interior stud spacing 16 in. Composite (yes or no) Y
End stud spacing 16 in. Composite (yes or no) Y

Shearwall siding: Type OSB / Thickness 7/16 in.
24' Trans: Fastener 8d/131 Spacing: Int 8 in. Edge 4 in.
23' Long: Fastener 8d/131 Spacing: Int 8 in. Edge 4 in.

Wall tension transferred by: Siding nails 8d/131 @ 4 O.C. edges

Foundation anchor bolts: Concrete strength 3000 psi
Size 1/2 in. Shape L Washer 2" Embedment 6 in.
Location of first anchor bolt from corner 8 in.

Anchor Bolts @ 48" O.C.
Hold-down device: Mfr _____ Model A307 Loc. from corner 8 in.

Type of foundation: 1 #5 rebar continuous required in bond beam./FBC-1804.6.2.6.2

Floor slab 4 in. CMU: Size 8 x 16 in. Height 24 in. Reinf. # 5 at 96 in.
Monolithic footing: Depth 18 in. Bottom width 12 in.

Footing: Width 20 in. Depth 10 in. Reinforcing 2 --# 5 bars
Interior Footings: 16" W X 10" D

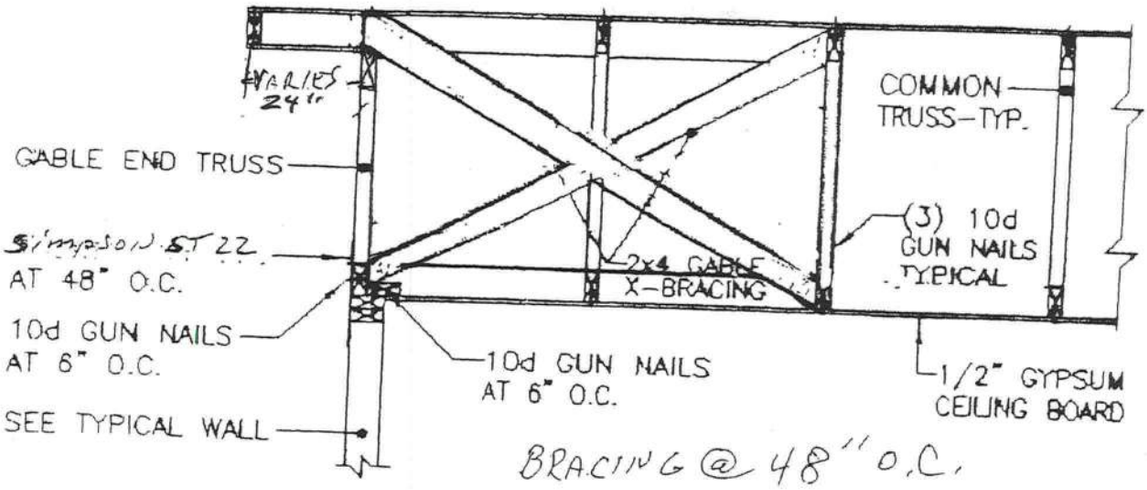
Porch Columns: NA

Porch Column Fasteners: NA

NOTE:
1. Balloon frame ALL gable ends unless this summary is accompanied by Gable End Wall Brace detail.
2. All trusses must bear on exterior walls & porch beams.
3. All walls to be nailed with same nailing pattern as shearwalls.
4. This is a windload only, NOT a structural analysis.
5. This windload is not valid without a raised, embossed seal.
6. It is assumed that ideal soil conditions and pad preparations are provided.
7. Fiber mesh or WWM may be used in concrete slab.
8. Trusses must be anchored and supported in accordance to the truss engineering.
9. Wind design and analysis valid for one use only, no copies permitted.

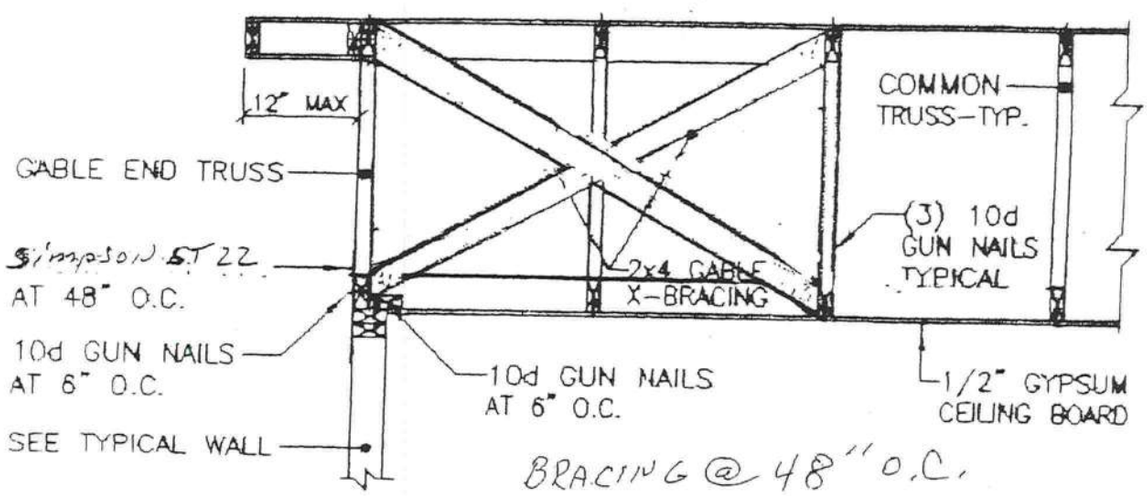
Bracing Gable

9/29/05



GABLE END DETAIL

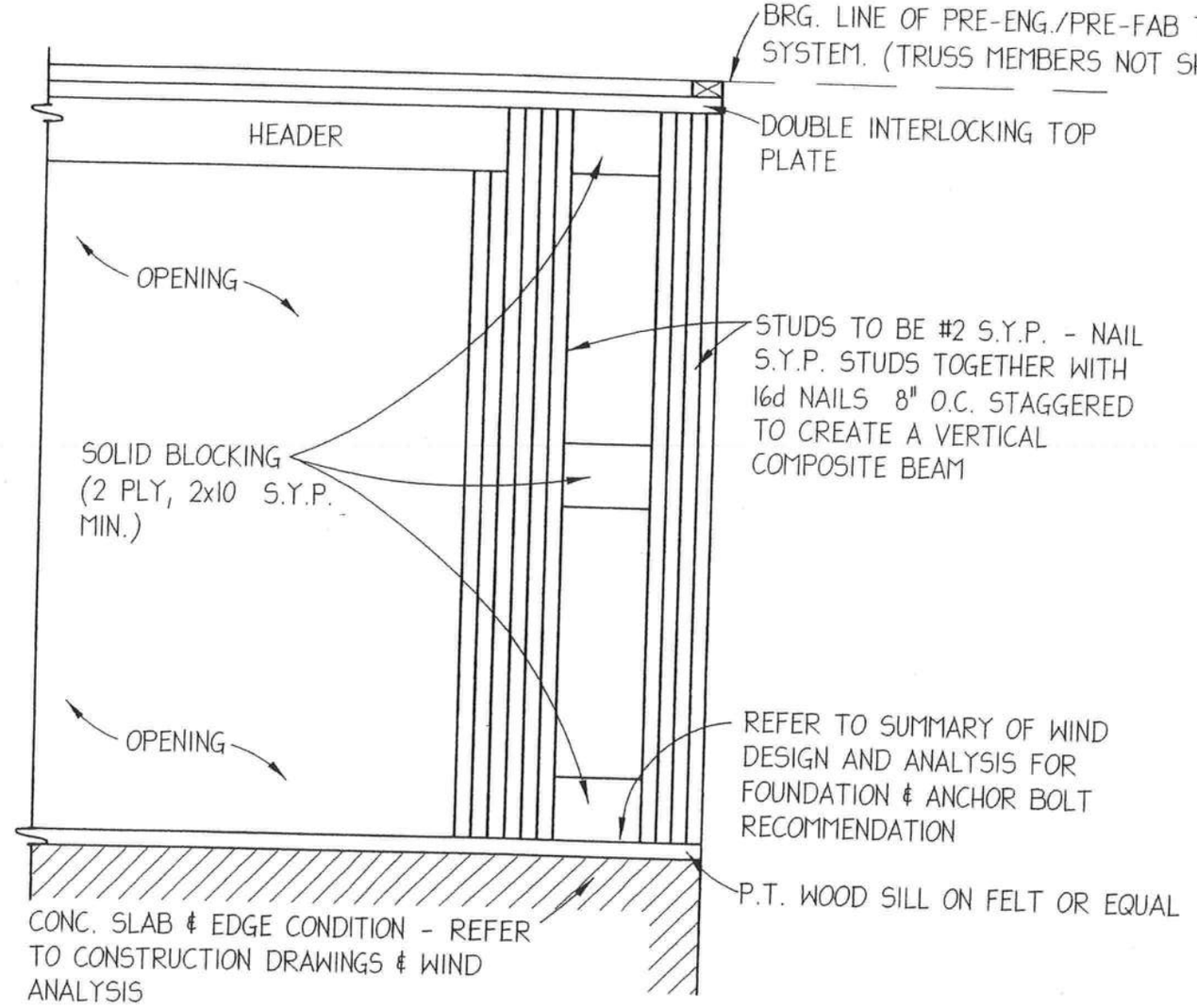
SCALE: NTS



GABLE END DETAIL

SCALE: NTS

AR003285
9/29/05



MINIMUM 2'-0" SHEAR WALL SEGMENT

ALLOWABLE EQUIVALENT SHEAR WALL EQUAL TO 1.5 TIMES THE ACTUAL WALL SEGMENT LENGTH. MINIMUM WALL SEGMENT LENGTH OF 2'-0" WHICH EQUATES TO A 3'-0" SHEAR WALL SEGMENT. SEE WINDLOAD ANALYSIS FOR NAIL SIZES AND SPACING ON SHEATHING.

C:\DeSoto\SHEAR WALL.DWG 01/15/01 13:28

C#C-

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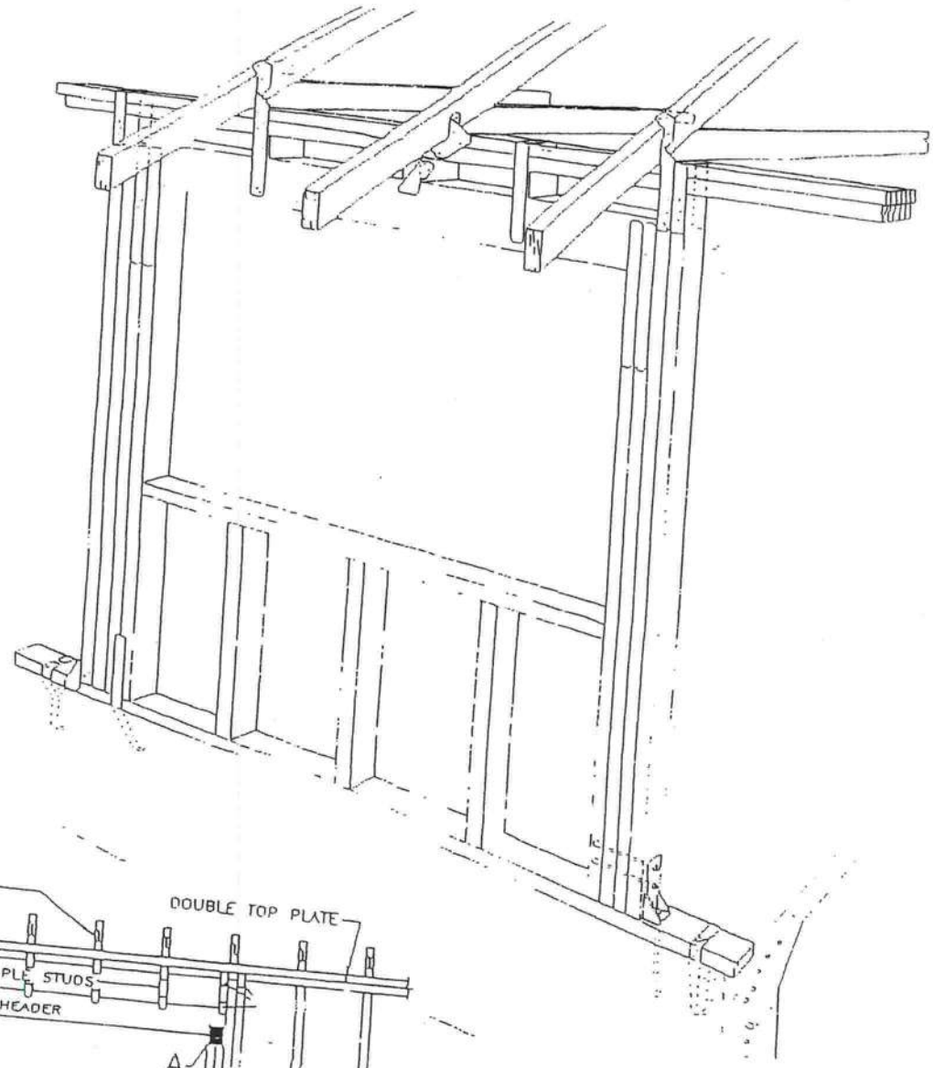
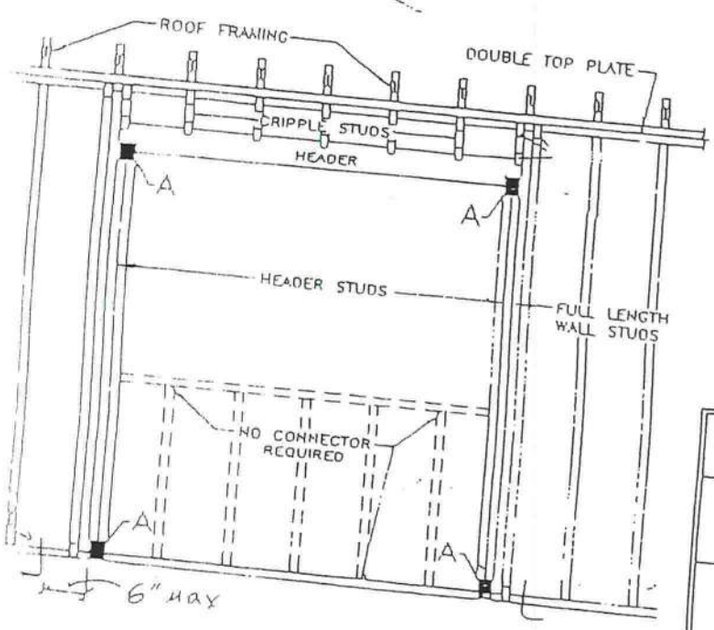




Unsupported Wall Height	Stud Spacing	Maximum Header Span (ft)					
		3'	6'	9'	12'	15'	18'
		Number of Header Studs Supporting End of Header					
		1[1]	1	2	2	2	2
		Number of Full-Length Studs at Each End of Header					
10' or less	12 in.	2	2	3	3	3	3
	16 in.	2	2	3	3	3	3
	24 in.	1	2	2	2	2	2
greater than 10'	12 in.	2	2	3	4	5	5
	16 in.	2	2	3	3	4	4
	24 in.	1	2	2	2	3	3

Total each truss uplift on the header divide by 2 for header anchorage

TIE-DOWN TABLES

HEADERS				
Uplift Force Lbs	Top Connector **	Rating Lbs	Bottom Connector **	Rating Lbs
to 455	LSTA9	725	H3	455
to 910	LSTA12	905	2-H3	910
to 1265	LSTA18	1265	LTT19	1350
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2565
to 2865	3-LSTA18	3255	HD2A-3.5	2865
to 3700	3-LSTA24	3880	HD5A-3	3700

Total uplift for each truss resting on the header and divide by 2 to determine the uplift force.
Use proper bolt anchors sufficient to support required load.

TRUSSES/GIRDERS		
Uplift Force Lbs	Top Connector **	Bottom Connector **
to 500	H2.5	N/A
501-1049	H10	N/A
1050-1350	TS22	LTT19
1351-1750	2-TS22	LTT20
1751-2570	2-TS22	HD2A
2571-3665	3-TS22	HD5A
3666-5260	2-MST148	HTT22
5261-8300	2-MST48	HD10A

Two 12d common toenails are required per truss/rafter per bearing point into plate.
Use proper bolt anchors.
Strap rafters to truss or at each end with minimum uplift resistance of 450# each end.
Strap ridge beam at each end with minimum uplift resistance of 1000#.
It is the contractors responsibility to provide a continuous load path from truss/rafter/ridge beam to foundation.

	Top Connector **	Rating Lbs	Bottom Connector **	Rating
BEAM SEATS	LSTA18*	1200	LTT19*	1250
POSTS (max 17' spacing)	2-LSTA18	2400	ABU44	2300

*or per truss engineering
Use proper bolt anchors
All beams to be sheathed or strapped to Double Top Plate when applicable.

CRIPPLES	Sheathing nailing alone adequate w/8d nails @ 3" O.C.

STUDS
Wall sheathing nailing Adequate exterior walls bottom w/8d nails @ 3" O.C.
Wall sheathing nailing Adequate exterior walls top w/8d nails @ 3" O.C., as long as sheathing covers top plate, otherwise use SP2 @ 32" O.C. in addition to sheathing nailing.
Use SP2 top and SP1 bottom each stud for all interior load bearing walls and anchor bolts @ 32" O.C.
Interior anchor bolts to be ½" x 8" A307 or ½" x 6" wedge anchor or equivalent.

** Equivalent Simpson hardware, or other manufacturer, may be substituted for any of the hardware specified on this page as long as it meets the required load capacities/uplift resistance.
NOTE: For nailing into SPF members, multiply table values by .86

ASCE 7-98

9/30/05

Wind Load Design per ASCE 7-98

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	26.6	Deg
Type of Roof	Gabled	
Eave Height (Eht)	8.00	ft
Ridge Height (RHt)	15.88	ft
Mean Roof Height (Ht)	11.44	ft
Width Perp. to Wind (B)	28.00	ft
Width Parallel to Wind (L)	24.00	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.48
Flexible Structure	No

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	Cc * (33/z)^0.167	0.3048	
Lzm	l*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((B+Ht)/Lzm)^0.63))^0.5	0.9238	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8800	
Gust Factor Category III: Flexible or Dynamically Sensitive Structures			
Vhref	V*(5280/3600)	161.33	ft/s
Vzm	bm*(zm/33)^Am*Vhref	70.89	ft/s
NF1	NatFreq*Lzm/Vzm	4.37	Hz
Rn	(7.47*NF1)/(1+10.302*NF1)^1.667	0.0552	
Nh	4.6*NatFreq*Ht/Vzm	0.74	
Nb	4.6*NatFreq*B/Vzm	1.82	
Nd	15.4*NatFreq*Depth/Vzm	5.21	
Rh	1/Nh-(1/(2*Nh^2)*(1-Exp(-2*Nh)))	0.6453	
Rb	1/Nb-(1/(2*Nb^2)*(1-Exp(-2*Nb)))	0.4029	
Rd	1/Nd-(1/(2*Nd^2)*(1-Exp(-2*Nd)))	0.1734	
RR	((1/Beta)*Rn*Rh*Rb*(0.53+0.47*Rd))^0.5	0.9367	
gg	+(2*LN(3600*n1))^0.5+0.577/(2*LN(3600*n1))^0.5	4.19	
Gust3	0.925*((1+1.7*Izm*(3.4^2*Q^2+GG^2*RR^2)^0.5)/(1+1.7*3.4*Izm))	1.21	

Gust Factor Summary			
Main Wind-force resisting system:		**Components and Cladding:**	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.88	Gust Factor (G)	0.88

ASCE 7-98

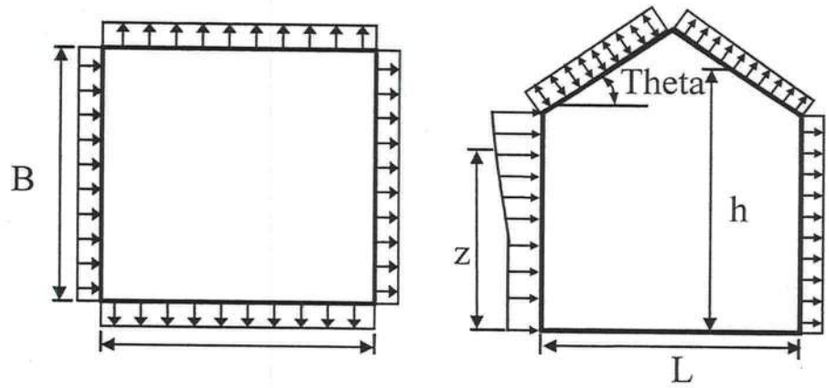
Wind Load Design per ASCE 7-98

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev.	Kz	Kzt	Kd	qz	Pressure (lb/ft^2)	
					Windward Wall*	
ft			1.00	lb/ft^2	+GCpi	-GCpi
15.88	0.70	1.00	1.00	21.70	12.07	18.48
15	0.70	1.00	1.00	21.70	12.07	18.48

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(15/zg)^(2/Alpha)	0.57	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*ImpFac*Kh*Kht*Kd	17.80	psf

Wall Pressure Coefficients, Cp	
Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf) +GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 28 ft wall)	-0.50	-11.04	-4.63
Leeward Walls (Wind Dir Parallel to 24 ft wall)	-0.47	-10.52	-4.11
Side Walls	-0.70	-14.17	-7.76
Roof - Normal to Ridge (Theta>=10)			
Windward - Max Negative	-0.26	-7.30	-0.89
Windward - Max Positive	0.21	0.08	6.48
Leeward Normal to Ridge	-0.60	-12.60	-6.20
Overhang Top	-0.26	-4.10	-4.10
Overhang Bottom	0.80	0.70	0.70
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 5.72 ft	-0.90	-17.30	-10.90
Dist from Windward Edge: 5.72 ft to 11.44 ft	-0.90	-17.30	-10.90
Dist from Windward Edge: 11.44 ft to 22.88 ft	-0.50	-11.04	-4.63

ASCE 7-98

Wind Load Design per ASCE 7-98

Dist from Windward Edge: > 22.88 ft	-0.30	-7.90	-1.50

* Horizontal distance from windward edge

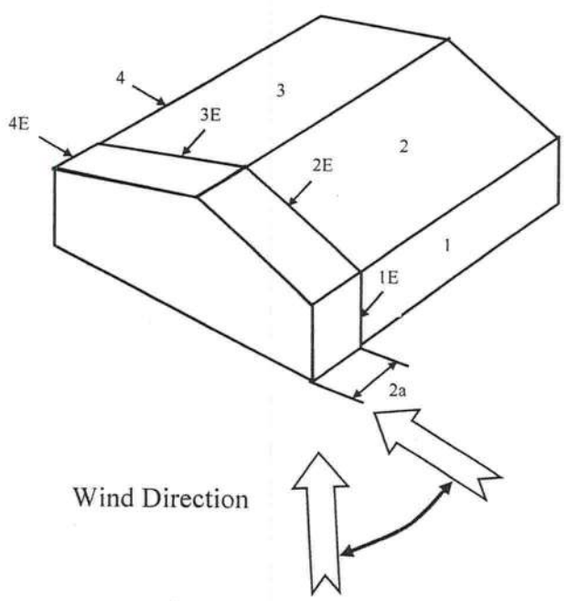
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	17.80

Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.55	0.18	-0.18	21.70	8.03	15.84
2	-0.10	0.18	-0.18	21.70	-5.99	1.82
3	-0.45	0.18	-0.18	21.70	-13.61	-5.79
4	-0.39	0.18	-0.18	21.70	-12.38	-4.57
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.73	0.18	-0.18	21.70	11.88	19.69
2E	-0.19	0.18	-0.18	21.70	-7.93	-0.12
3E	-0.58	0.18	-0.18	21.70	-16.59	-8.78
4E	-0.53	0.18	-0.18	21.70	-15.50	-7.69
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

* p = qh * (GCpf - GCpi)



ASCE 7-98

Wind Load Design per ASCE 7-98

Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(15/zg)^(2/Alpha)	=	0.57
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	17.80

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86
5	0.40	0.18	-0.18	21.70	4.77	12.59
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.61	0.18	-0.18	21.70	9.33	17.14
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43

* p = qh * (GCpf - GCpi)

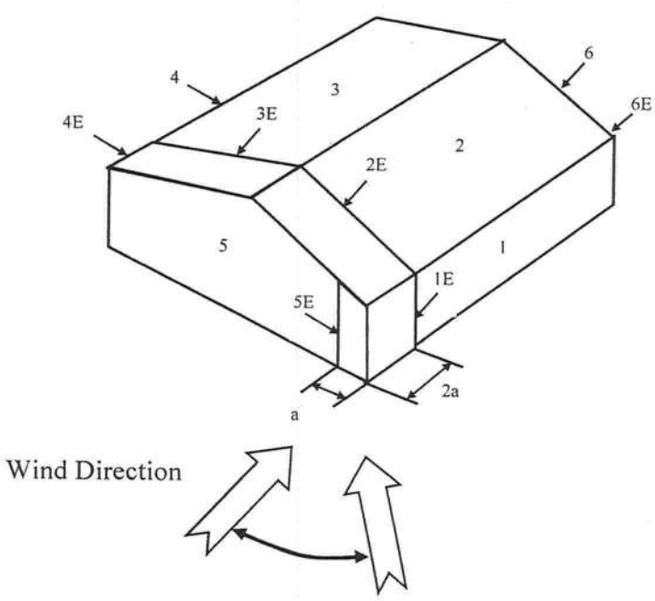


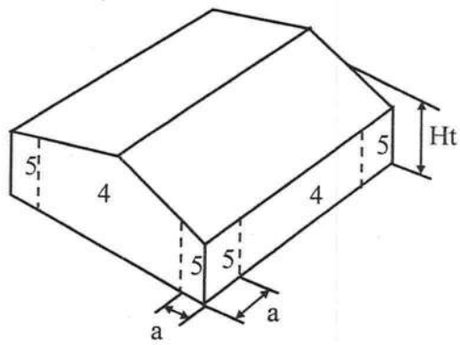
Figure 6-5 - External Pressure Coefficients, GCp

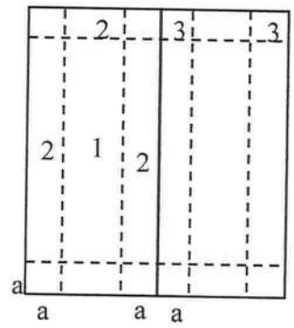
Loads on Components and Cladding for Buildings w/ Ht <= 60 ft

ASCE 7-98

9/30/05

Wind Load Design per ASCE 7-98





Gabled Roof

10 < Theta <= 45

a = 2.4 ==> 3.00 ft

Component	Width (ft)	Length (ft)	Area (ft^2)	Zone	GCp		Wind Press (lb/ft^2)	
					Max	Min	Max	Min
	16	7	112.00	5	0.81	-1.03	17.71	-21.53
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
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	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

Condition	Gcpi	
	Max +	Max -

ASCE 7-98

Wind Load Design per ASCE 7-98

Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	**0.18**	**-0.18**

Table 6-8 External Pressure Coefficients for Arched Roofs, Cp

r (Rise-to-Span Ratio) = 0.3

Condition	Variable	Cp		
		Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	Cp	0.13	-1	-0.5
	P (+GCpi) - psf	-1.25	-18.87	-11.04
	P (-GCpi) -psf	5.16	-12.46	-4.63
Roof Springing from Ground	Cp	0.42	-1	-0.5
	P (+GCpi) - psf	3.38	-18.87	-11.04
	P (-GCpi) -psf	3.38	-18.87	-11.04

Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings, Cf

Variable	Description	Value	
L	Roof dimension normal to wind direction	24.00	ft
B	Roof dimension parallel to wind direction	28.00	ft
L/B	Ratio of L to B	0.857	
Theta	Slope of Roof	26.6	Deg
Cf	Force Coefficient	1.18	
X	Distance to center of pressure from windward edge	0.42	ft

RE: RSNBARN - BARN

MiTek Industries, Inc.
1801 Massaro Blvd.
Tampa, Fl 33619
Phone: 813/675-1200
Fax: 813/675-1148

Site Information:
Project Customer: Project Name:
Lot/Block: Subdivision:
Address:
City: State:

Name Address and License # of Structural Engineer of Record, If there is one, for the building.
Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2004/TPI2002
Wind Code: ASCE 7-98 Wind Speed: 110 mph
Roof Load: 40.0 psf

Design Program: MiTek 20/20 6.2
Design Method: Main Wind Force Resisting System ASCE 7-98
Floor Load: N/A psf

This package includes 2 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Job ID#	Truss Name	Date
1	T1822280	RSNBARN	A	10/25/05
2	T1822281	RSNBARN	AET	10/25/05

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Santa Fe Truss.

Truss Design Engineer's Name: Zhang, Guo-jie
My license renewal date for the state of is February 28, 2007.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Sec. 2.

October 25,2005
Zhang, Guo-jie

Job	Truss	Truss Type	Qty	Ply	BARN
RSNBARN	A	HOWE	11	1	T1822280

Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., p.colacino

6.200 s Oct 18 2005 MiTek Industries, Inc. Tue Oct 25 09:15:17 2005 Page 1

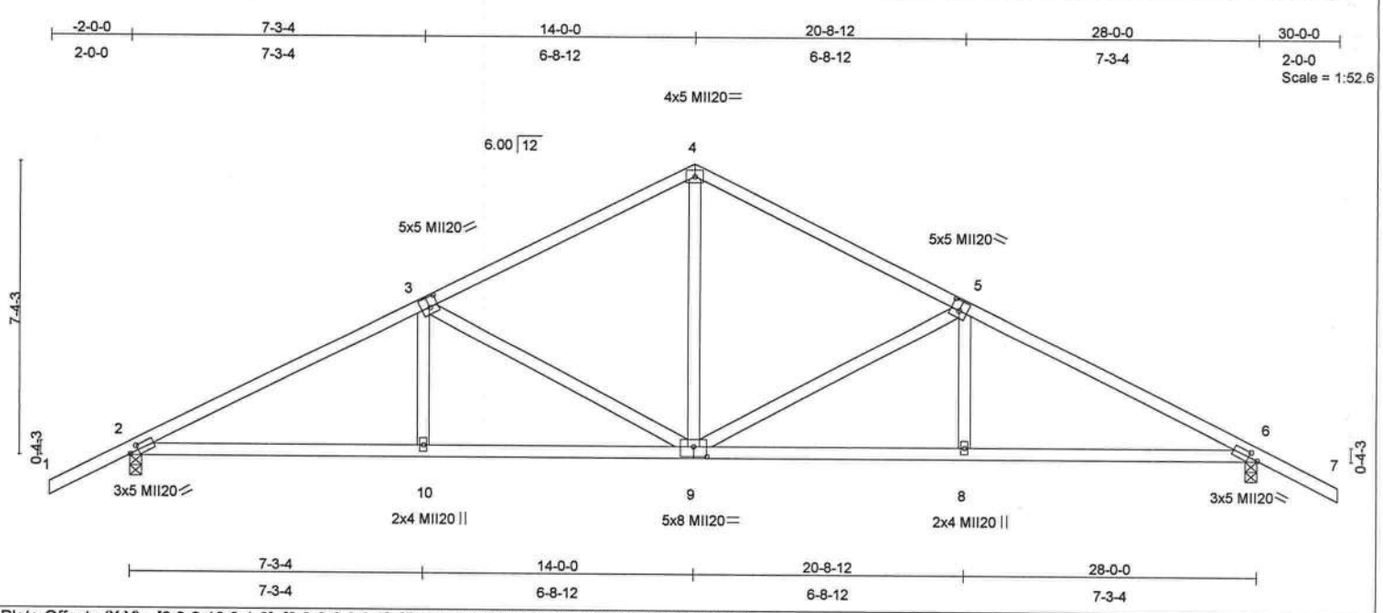


Plate Offsets (X,Y): [2:0-2-10,0-1-8], [3:0-2-8,0-3-0], [5:0-2-8,0-3-0], [6:0-2-10,0-1-8], [9:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.07	9	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.21	2-10	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.65	Horz(TL)	0.08	6	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 137 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1237/0-3-8, 6=1237/0-3-8
Max Horz 2=-149(load case 6)
Max Uplift2=-338(load case 5), 6=-338(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/53, 2-3=-1951/334, 3-4=-1335/291, 4-5=-1335/291, 5-6=-1951/335, 6-7=0/53
BOT CHORD 2-10=-296/1652, 9-10=-296/1652, 8-9=-169/1652, 6-8=-169/1652
WEBS 3-10=0/299, 4-9=-84/717, 5-8=0/299, 3-9=-639/228, 5-9=-639/229

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS interior zone; Lumber DOL=1.33 plate grip DOL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 338 lb uplift at joint 2 and 338 lb uplift at joint 6.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744
MiTek Industries, Inc.
1801 Massaro Blvd
Tampa FL 33619
FL Cert.#6634

October 25,2005

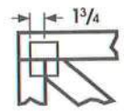
WARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

14515 N. Outer Forty,
Suite #300
Chesterfield, MO 63017

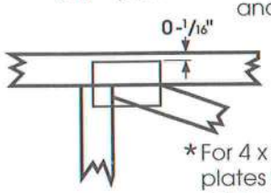
MiTek®

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x,y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0 - 1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

4 x 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

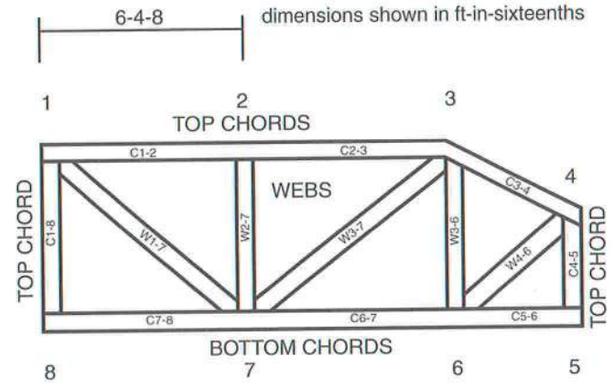
BEARING

Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

TEE-LOK®

⚠General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.

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Job	Truss	Truss Type	Qty	Ply	BARN
RSNBARN	AET	HOWE	2	1	T1822281

Job Reference (optional)

SANTA FE TRUSS, HIGH SPRINGS FL., p.colacino

6.200 s Oct 18 2005 MiTek Industries, Inc. Tue Oct 25 09:15:19 2005 Page 1

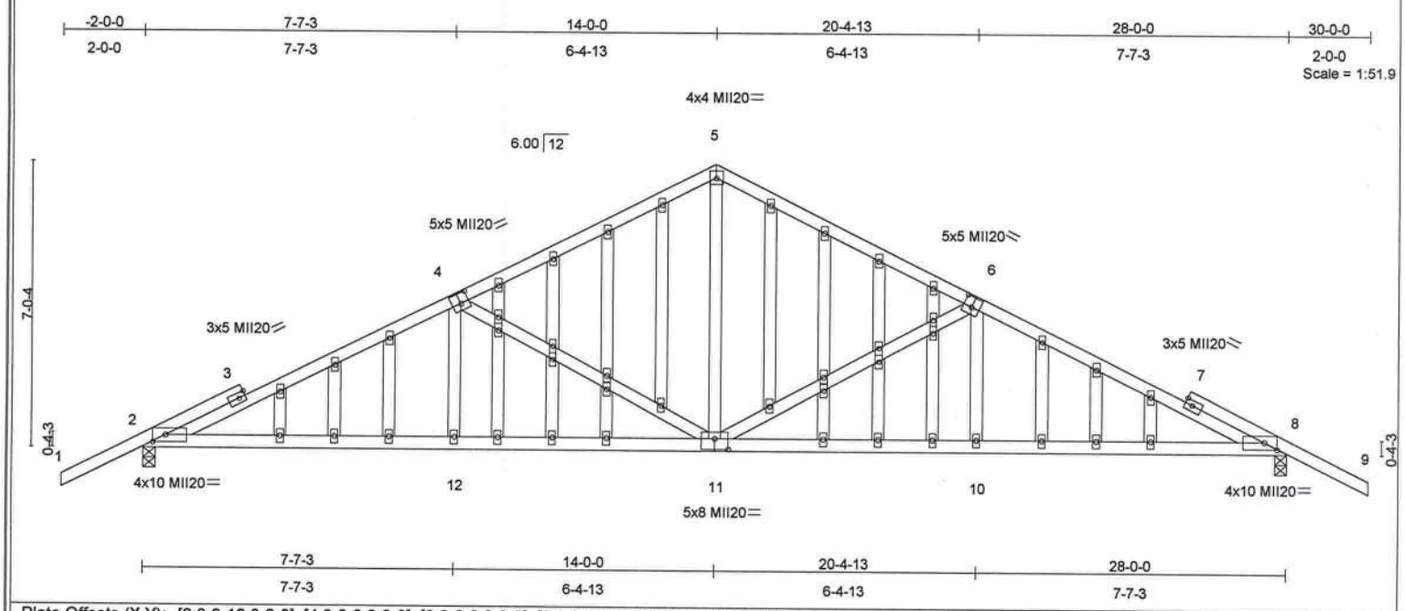


Plate Offsets (X,Y): [2:0-3-12,0-2-0], [4:0-2-8,0-3-0], [6:0-2-8,0-3-0], [8:0-3-12,0-2-0], [11:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.11	2-12	>999	240	MII20	249/190
TCDL	10.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.33	2-12	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.08	8	n/a	n/a		
BCDL	10.0	Code FBC2004/TPI2002		(Matrix)							Weight: 211 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1237/0-3-8, 8=1237/0-3-8
Max Horz 2=144(load case 5)
Max Uplift2=-339(load case 5), 8=-339(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/53, 2-3=-2070/322, 3-4=-2012/356, 4-5=-1384/295, 5-6=-1384/295, 6-7=-2012/357, 7-8=-2070/323, 8-9=0/53
BOT CHORD 2-12=-318/1800, 11-12=-319/1796, 10-11=-191/1796, 8-10=-190/1800
WEBS 4-12=0/312, 5-11=-105/827, 6-10=0/312, 4-11=-740/247, 6-11=-740/248

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=18ft; TCDL=5.0psf; BCDL=5.0psf; Category II; Exp C; enclosed; MWFRS interior zone; Lumber DOL=1.33 plate grip DOL=1.33.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MII20 unless otherwise indicated.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) Gable studs spaced at 1-4-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 339 lb uplift at joint 2 and 339 lb uplift at joint 8.

LOAD CASE(S) Standard

Guo-Jie Zhang, FL Lic #47744
MiTek Industries, Inc.
1801 Massaro Blvd
Tampa FL 33619
FL Cert.#6634

October 25,2005

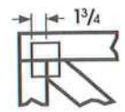
WARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

14515 N. Outer Forty,
Suite #300
Chesterfield, MO 63017

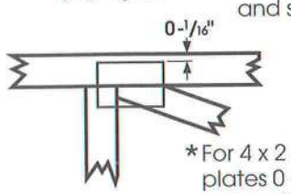
MiTek®

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x,y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0 - 1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

4 x 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

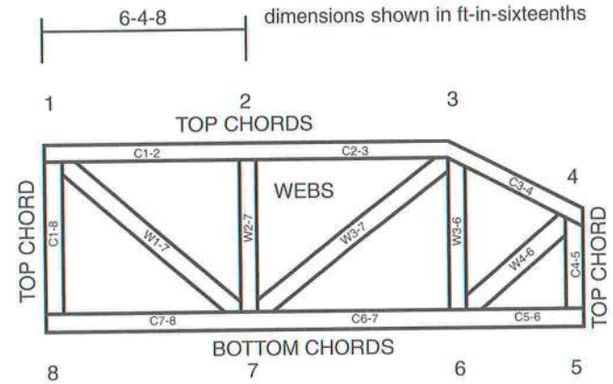
BEARING

Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

TEE-LOK®

⚠General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.

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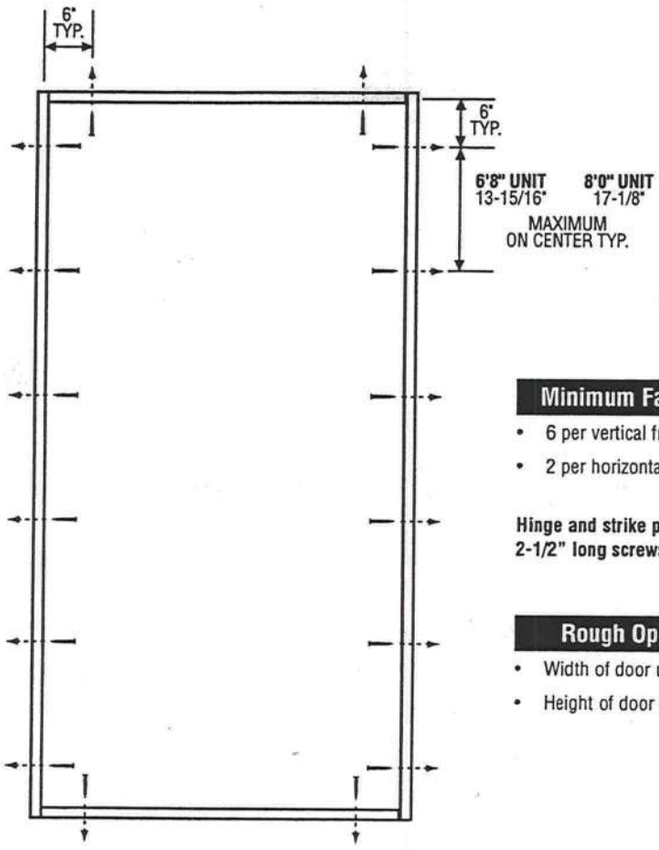
SINGLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 2 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 3146 or 3166**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

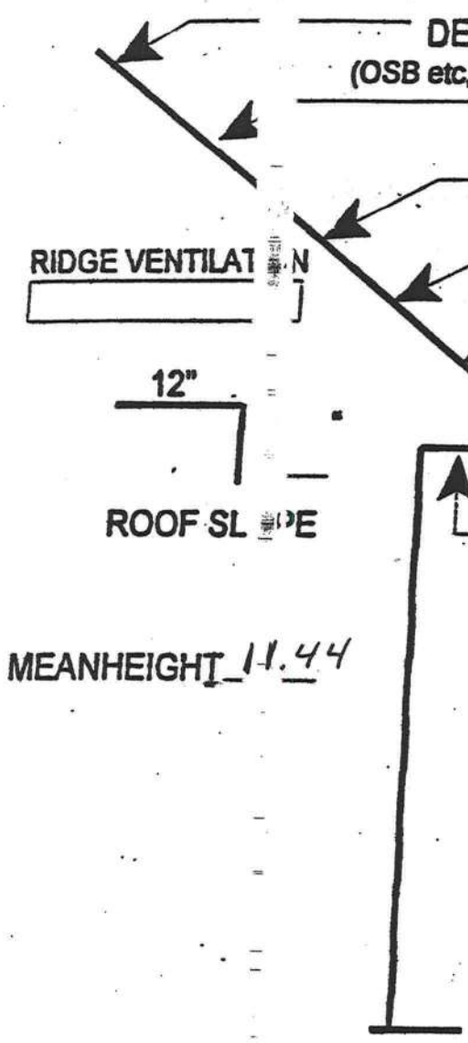
April 29, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection
Premium Quality Doors
Exclusively from
Masonite
Masonite International Corporation

ALACHUA COUNTY OFFICE OF CODES ENFORCEMENT

ROOFING, WINDOW & DOOR REQUIREMENTS

ROOF SYSTEM DESCRIPTION



DECK TYPE: 7/16 OSB 4" EDGE 8" FIELD
(OSB etc, Nail pattern & fastner size)

UNDERLAYMENT: 30 lb FELT
(Felt weight)

FLASHING: 29 GA GAL.

CAP SHEET:
(Flat roof only)

ROOF COVERING: 30yr ARC
(Mfg. & Nailing)

DRIP EDGE MATERIAL: ALUMINUM

EAVE VENTILATION: VENTED ALUMINUM

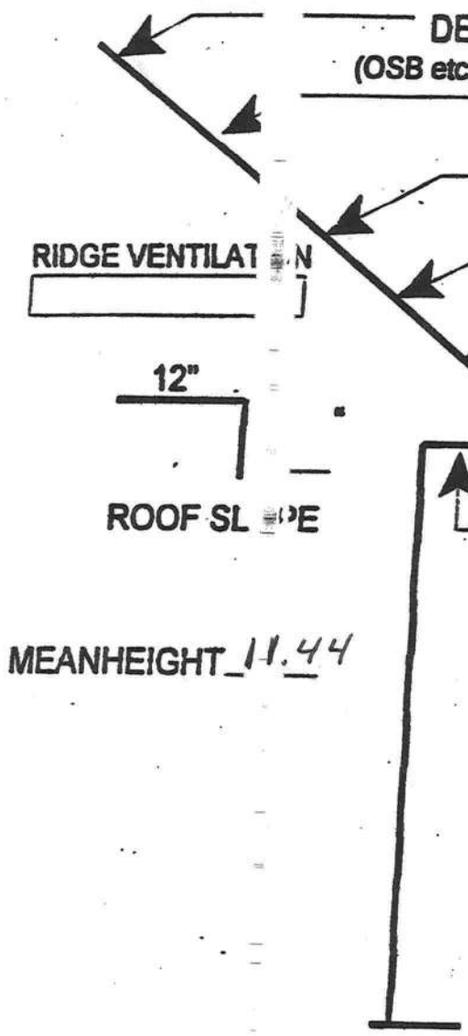
WINDLOAD INFO

1. Basic wind speed ___100 mph ✓110 mph
2. Wind importance factor 1
3. Wind exposure B
4. Internal pressure coefficient -18
5. Components & cladding (design pressure) 21.8

ALACHUA COUNTY OFFICE OF CODES ENFORCEMENT

ROOFING, WINDOW & DOOR REQUIREMENTS

ROOF SYSTEM DESCRIPTION



MEANHEIGHT 11.44

DECK TYPE: 7/16 OSB 4" EDGE 8" FIELD
(OSB etc, Nail pattern & fastner size)

UNDERLAYMENT: 30 lb FELT
(Felt weight)

FLASHING: 29 GA GAL.

CAP SHEET:
(Flat roof only)

ROOF COVERING: 30yr ARC
(Mfg. & Nailing)

DRIP EDGE MATERIAL: ALUMINUM

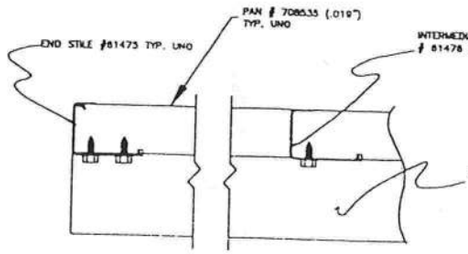
EAVE VENTILATION: VENTED ALUMINUM

WINDLOAD INFO

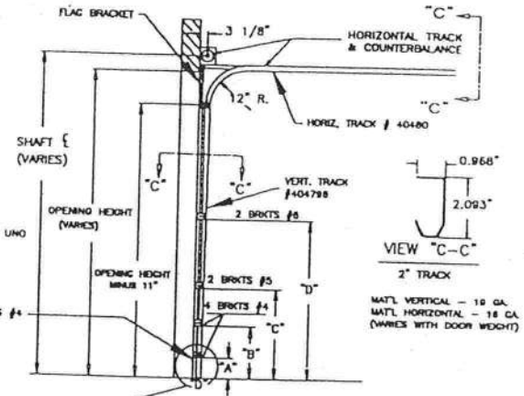
1. Basic wind speed ___100 mph ✓110 mph
2. Wind importance factor 1
3. Wind exposure B
4. Internal pressure coefficient -18
5. Components & cladding (design pressure) 21.8

NOTES:

1. 4 STRUTS POSITIONED AS SHOWN WITH 4 THREAD FORMING SCREWS 1/4"-20 X 3/4" AT EACH END STILE & 2 AT INTERMEDIATE STILES PER SECTION "B-B".
2. FOUR 21" SECTIONS FORM THE 7'-0" HIGH DOOR.
3. ALL SECTIONS EMBOSSED 14.50"x20.375. 25 GA.
4. TORSION SPRINGS SHOWN. EXTENSION SPRINGS AVAILABLE.
5. SIDE LOCKS MUST BE ATTACHED TO EACH SIDE OF DOOR. CAM & BAR LOCKS OR AN OPENER MAY BE SUBSTITUTED.
6. WHERE THIS DRAWING CONFLICTS WITH OTHER INSTALLATION INSTRUCTIONS, THIS DRAWING GOVERNS.



SECTION "B-B"



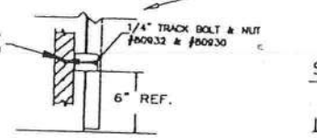
VIEW "C-C"

2" TRACK

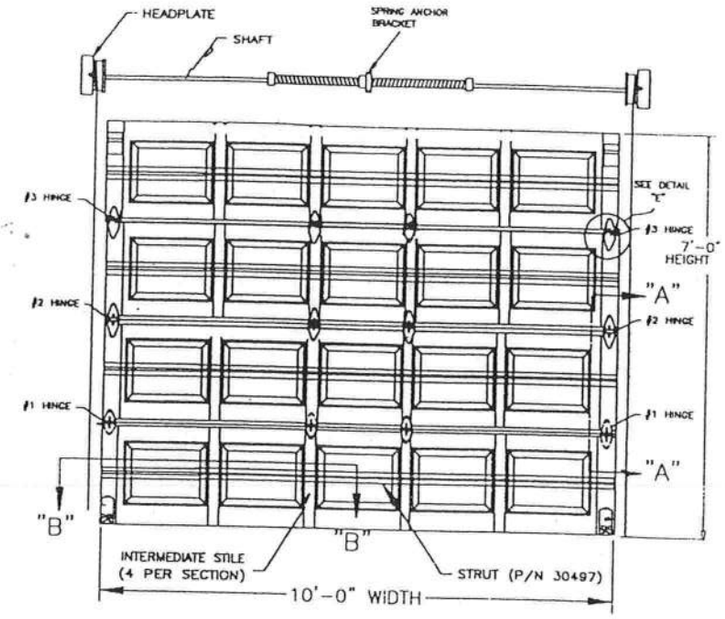
MAT'L VERTICAL - 18 GA.
MAT'L HORIZONTAL - 18 GA.
(VARIES WITH DOOR WEIGHT)

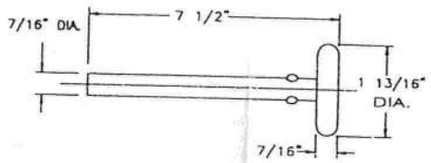
DOOR HEIGHT	"A"	"B"	"C"	"D"
7'-00"	8"	22"	43"	64"

STANDARD TRACK DETAIL

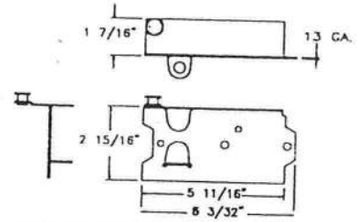


DETAIL "D"





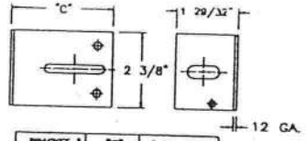
P/N 80441 ROLLER ASSEMBLY



P/N 80262 R + L BOT. FIXTURE

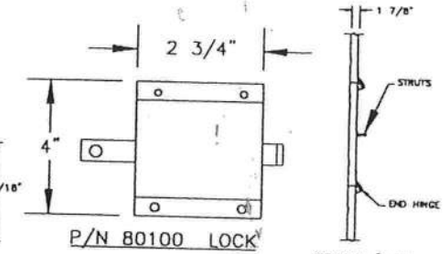
P/N 80298 SCREW

1/4"-20 x 5/8"



BRACKET #	"C"	PART NUMBER
4	3-0"	80800
5	3-1/4"	80801
6	3-1/2"	80802

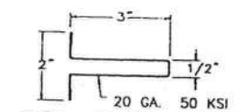
TRACK BRACKET



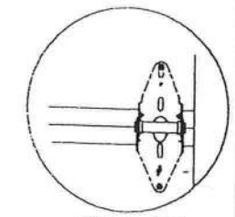
P/N 80100 LOCK

SECTION "A-A"

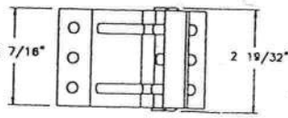
ALL INTERMEDIATE HINGES ARE #1

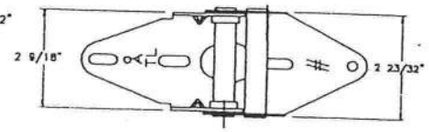


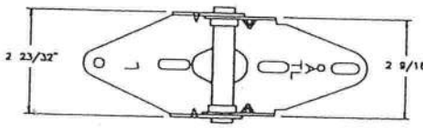
P/N 30497 STRUT

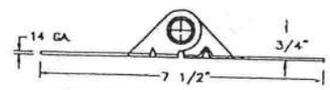


DETAIL "E"

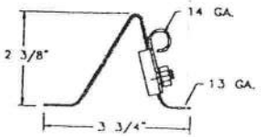




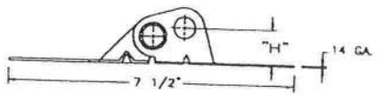




P/N 80428 CTR. HINGE

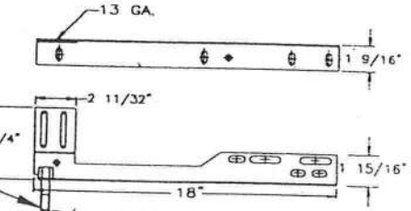


P/N 80448 TOP FIXTURE



END HINGE

HINGE #	GAGE #	"H"	PART NUMBER
1	14	1-0"	80428
2	14	1-1/4"	80429
3	14	1-1/2"	80430



P/N 80485 FLAG BRACKET

PROFESSIONAL ENGINEER'S SEAL PROVIDED ONLY FOR VERIFICATION OF TESTING TO REQUIREMENTS OF ASTM E-330-96.

NOTES: THIS DRAWING AND / OR TECHNICAL INFORMATION ON THIS SHEET IS THE PROPERTY OF MID-AMERICA DOOR COMPANY AND IS LOANED IN CONFIDENCE FOR ENGINEERING AND MUTUAL ASSISTANCE PURPOSES ONLY, AND MAY NOT BE REPRODUCED OR USED TO MANUFACTURE ANYTHING DISCLOSED HEREON WITHOUT THE EXPRESSED PERMISSION OF MID-AMERICA DOOR COMPANY.

JOB NO.	BY	DATE	BY	DATE	BY	DATE	CHKR.	BY	DATE
	DRAWN		INITIAL CHK.		FINAL CHK.			APPROVED	

SCALE	N.T.S.	DATE
DRAWN BY	JLG	07-14-99
INITIAL CHK.	-	
FINAL CHK.	-	
ENGR.	-	
APPR. BY	JR	-

Mid-America Door Company — POWER CITY, OK

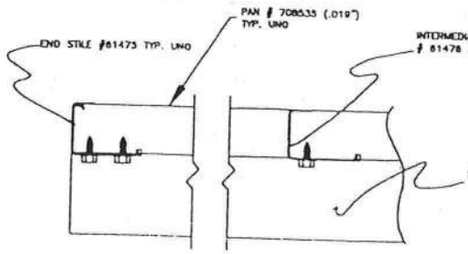
DESCRIPTION: 10'x7' GARAGE DOOR + - 30 PSF DESIGN LOAD

TITLE

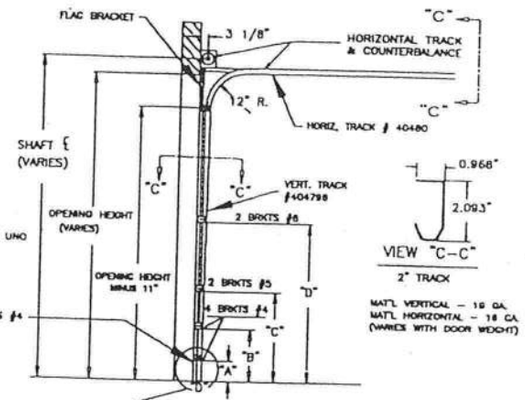
DWG. NO. FL-10700900 | SHT. 1 OF 1 | REV. -

NOTES:

1. 4 STRUTS POSITIONED AS SHOWN WITH 4 THREAD FORMING SCREWS 1/4"-20 X 3/4" AT EACH END STILE & 2 AT INTERMEDIATE STILES PER SECTION "B-B".
2. FOUR 21" SECTIONS FORM THE 7'-0" HIGH DOOR.
3. ALL SECTIONS EMBOSSED 14.50"x20.375. 25 GA.
4. TORSION SPRINGS SHOWN. EXTENSION SPRINGS AVAILABLE.
5. SIDE LOCKS MUST BE ATTACHED TO EACH SIDE OF DOOR. CAM & BAR LOCKS OR AN OPENER MAY BE SUBSTITUTED.
6. WHERE THIS DRAWING CONFLICTS WITH OTHER INSTALLATION INSTRUCTIONS, THIS DRAWING GOVERNS.



SECTION "B-B"



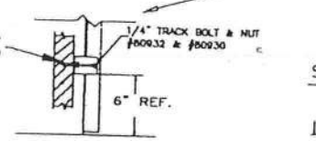
VIEW "C-C"

2" TRACK

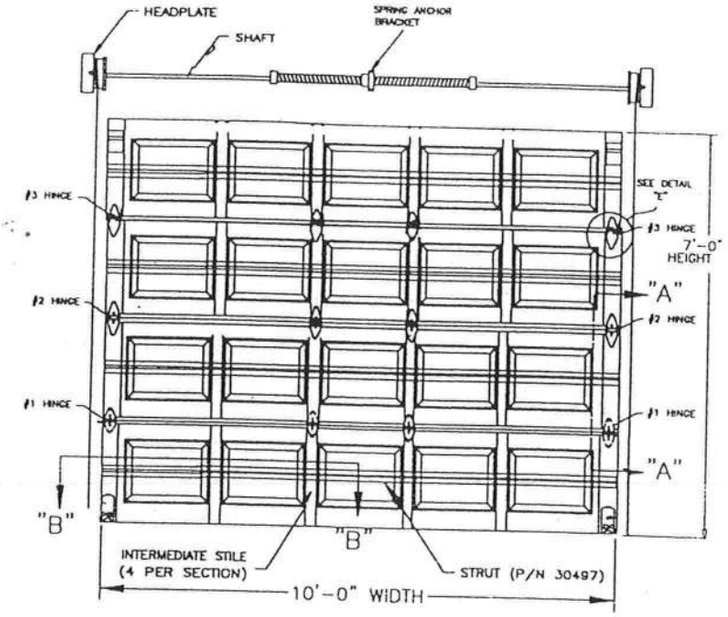
MAT'L VERTICAL - 18 GA.
MAT'L HORIZONTAL - 18 GA.
(VARIES WITH DOOR WEIGHT)

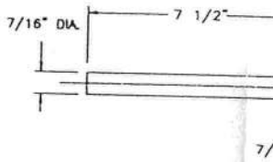
DOOR HEIGHT	"A"	"B"	"C"	"D"
7'-00"	6"	22"	43"	64"

STANDARD TRACK DETAIL

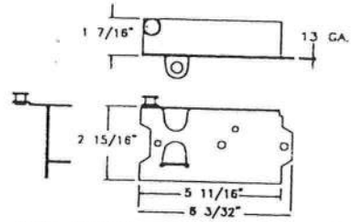


DETAIL "D"

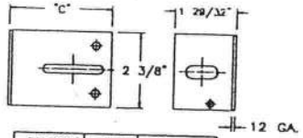




P/N 80441 ROLLER ASSEMBLY



P/N 80262 R + L BOT. FIXTURE

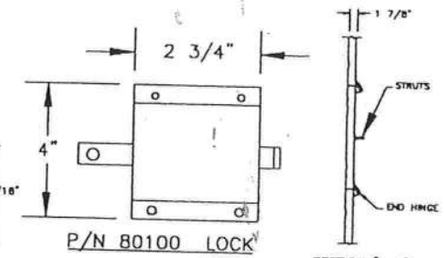


BRACKET #	"C"	PART NUMBER
4	3-0"	80800
5	3-1/4"	80801
6	3-1/2"	80802

TRACK BRACKET

P/N 80298 SCREW

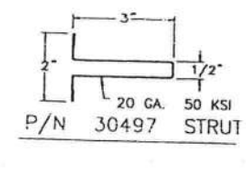
1/4"-20 x 5/8"



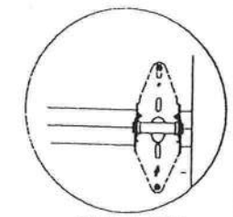
P/N 80100 LOCK

SECTION "A-A"

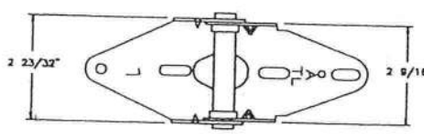
ALL INTERMEDIATE HINGES ARE #1

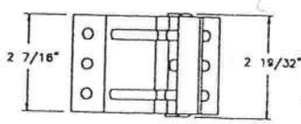


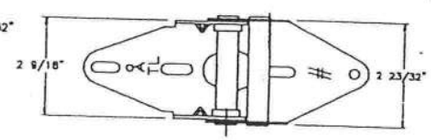
P/N 30497 STRUT

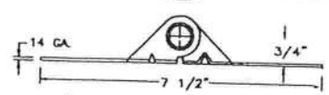


DETAIL "E"

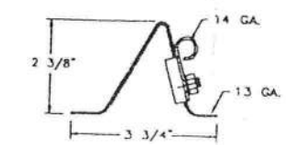




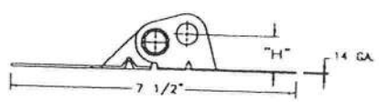




P/N 80428 CTR. HINGE

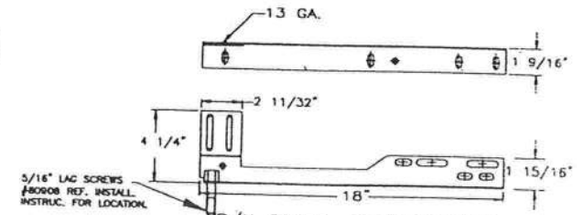


P/N 80448 TOP FIXTURE



END HINGE

HINGE #	GAGE #	"H"	PART NUMBER
1	14	1-0"	80428
2	14	1-1/4"	80429
3	14	1-1/2"	80430



P/N 80485 FLAG BRACKET

PROFESSIONAL ENGINEER'S SEAL PROVIDED ONLY FOR VERIFICATION OF TESTING TO REQUIREMENTS OF ASTM E-330-96.

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JOB NO.	BY	DATE	BY	DATE	BY	DATE	ENGR.	BY	DATE
	DRAWN		INITIAL CHK.		FINAL CHK.			APPROVED	

SCALE	N.T.S.	DATE
DRAWN BY	JLG	07-14-98
INITIAL CHK.		"
FINAL CHK.		"
ENGR.		"
APPR. BY	JR	"

Mid-America Door Company POTEAU CITY, OK

DESCRIPTION: 10'x7' GARAGE DOOR + - 30 PSF DESIGN LOAD

TITLE

DWG. NO. FL-10700900 | SHT. 1 OF 1 | REV. -

DOOR FRAME PLATE CONNECTION SCHEDULE 1.1

WOOD CONSTRUCTION		QUANT. OF NAILS PER JAMB**		
		SYP	DOUG.FIR	SPF
TOTAL DOOR LOAD (LBS.)	LOAD PER JAMB (LBS.)	SPECIFIC GRA.=0.55	SPECIFIC GRA.=0.48	SPECIFIC GRA.=0.42
1000	500	5	7	10
1250	625	7	9	12
1500	750	8	11	15
1750	875	9	13	17
2000	1000	10	14	20
2250	1125	12	16	22
2500	1250	13	18	24
2750	1375	14	20	27
3000	1500	15	21	29
3250	1625	17	23	32
3500	1750	18	25	34
3750	1875	19	26	36
4000	2000	20	28	39
4250	2125	22	30	41
4500	2250	23	32	44
4750	2375	24	33	46
5000	2500	25	35	48

** - BASED ON 16dx 3 1/2" LONG THREADED, HARDENED NAIL ASSUMING 2" MIN. PENETRATION.
1. ANCHORS TO BE EVENLY SPACED BETWEEN THE HEADER AND THE FLOOR.
2. PREDRILL NAIL HOLES TO PREVENT SPLITTING.
3. CHART IS BASED ON 8'-0" MAX. DOOR HEIGHT.

DOOR FRAME PLATE CONNECTION SCHEDULE 1.2

WOOD CONSTRUCTION		QUANT. OF LAG SCREWS PER JAMB**		
		SYP	DOUG.FIR	SPF
TOTAL DOOR LOAD (LBS.)	LOAD PER JAMB (LBS.)	SPECIFIC GRA.=0.55	SPECIFIC GRA.=0.48	SPECIFIC GRA.=0.42
1000	500	3	3	3
1250	625	3	3	3
1500	750	3	3	3
1750	875	3	3	3
2000	1000	3	3	3
2250	1125	3	3	3
2500	1250	3	3	3
2750	1375	3	3	3
3000	1500	3	3	3
3250	1625	3	3	3
3500	1750	3	3	4
3750	1875	3	3	4
4000	2000	3	3	4
4250	2125	3	4	4
4500	2250	3	4	5
4750	2375	3	4	5
5000	2500	4	4	5
5250	2625	4	4	5
5500	2750	4	5	6
5750	2875	4	5	6
6000	3000	4	5	6
6250	3125	4	5	6
6500	3250	4	5	6
6750	3375	5	6	7
7000	3500	5	6	7
7250	3625	5	6	7
7500	3750	5	6	7

** - BASED ON 3/8"ø LAG SCREW ASSUMING 2" MIN. PENETRATION SEASONED WOOD, DRY USE.
1. ANCHORS TO BE EVENLY SPACED BETWEEN THE HEADER AND THE FLOOR.
2. CHART IS BASED ON 8'-0" MAX. DOOR HEIGHT.

DOOR FRAME PLATE CONNECTION SCHEDULE 2.1

MASONRY/CONCRETE CONSTRUCTION		QUANTITY OF TAPCON ANCHORS PER JAMB**	
TOTAL DOOR LOAD (LBS.)	LOAD PER JAMB (LBS.)	HOLLOW BLOCK	3,000 PSI CONCRETE
1000	500	3	3
1250	625	3	3
1500	750	3	3
1750	875	4	3
2000	1000	4	3
2250	1125	5	3
2500	1250	5	3
2750	1375	6	4
3000	1500	6	4
3250	1625	6	4
3500	1750	7	5
3750	1875	7	5
4000	2000	8	5
4250	2125	8	6
4500	2250	9	6
4750	2375	9	6
5000	2500	10	6
5250	2625	10	7
5500	2750	11	7
5750	2875	11	7
6000	3000	11	8
6250	3125	12	8
6500	3250	12	8
6750	3375	13	9
7000	3500	13	9
7250	3625	14	9
7500	3750	14	9

**- BASED ON 1/4"ø TAPCON ANCHOR WITH A 1 1/2" MIN. EMBEDMENT.
1. ANCHORS TO BE EVENLY SPACED BETWEEN THE HEADER AND THE FLOOR.
2. 1/4" WASHERS REQUIRED WITH EACH TAPCON ANCHOR.
3. CHART IS BASED ON 8'-0" MAX. DOOR HEIGHT.

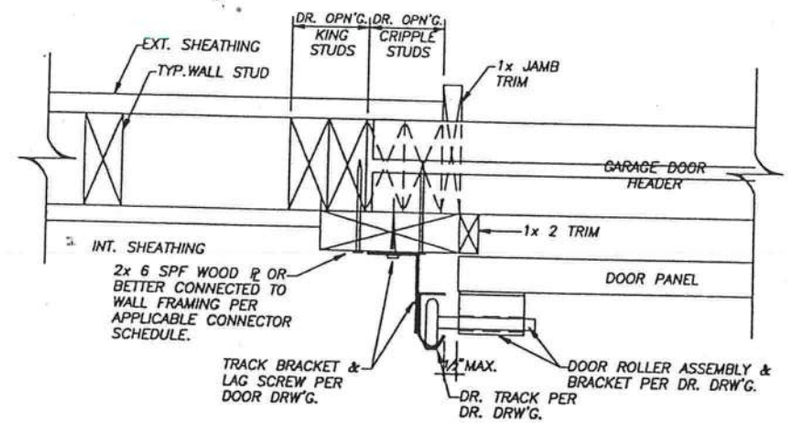
DOOR FRAME PLATE CONNECTION SCHEDULE 3.1

MASONRY/CONCRETE CONSTRUCTION		QUANTITY OF EXPANSION ANCHORS PER JAMB**		
TOTAL DOOR LOAD (LBS.)	LOAD PER JAMB (LBS.)	GROUT-FILLED BLOCK	2,000 PSI CONCRTE	3,000 PSI CONCRTE
1000	500	3	3	3
1250	625	3	3	3
1500	750	3	3	3
1750	875	3	3	3
2000	1000	3	3	3
2250	1125	3	3	3
2500	1250	3	3	3
2750	1375	3	3	3
3000	1500	3	3	3
3250	1625	3	3	3
3500	1750	4	3	3
3750	1875	4	3	3
4000	2000	4	3	3
4250	2125	4	3	3
4500	2250	4	3	3
4750	2375	5	3	3
5000	2500	5	3	3
5250	2625	5	3	3
5500	2750	5	3	3
5750	2875	6	3	3
6000	3000	6	3	3
6250	3125	6	3	3
6500	3250	6	3	3
6750	3375	6	3	3
7000	3500	7	4	3
7250	3625	7	4	4
7500	3750	7	4	4

**- BASED ON 3/8"ø HILTI STUD TYPE EXPANSION ANCHOR WITH A 2 1/2" MIN. EMBEDMENT.
1. ANCHORS TO BE EVENLY SPACED BETWEEN THE HEADER AND THE FLOOR.
2. MINIMUM DISTANCE FROM CONCRETE EDGE TO ANCHOR SHALL BE 3 3/4".
3. CHART IS BASED ON 8'-0" MAX. DOOR HEIGHT.

NOTES

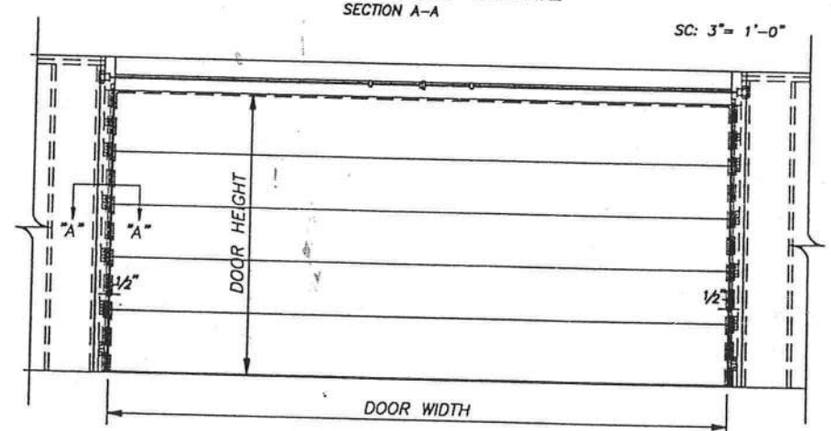
1. WIND LOAD PERFORMANCE LABEL MUST BE AFFIXED TO THE INSIDE OF THE DOOR IN A LOCATION THAT IS EASILY SEEN.
2. WHERE THIS DRAWING CONFLICTS WITH OTHER INSTALLATION INSTRUCTIONS, THIS DRAWING GOVERNS.
3. THE TESTING METHODS FOR THE DOORS PERTAINING TO DRW'S. LISTED HEREIN WERE IN ACCORDANCE WITH DASMA 108, SBC 1999 & ASCE 7-98.



JAMB FASTENER DETAIL

SECTION A-A

SC: 3"= 1'-0"



ELEVATION NOTES:
1. SPACE BETWEEN INSIDE OF VERTICAL TRACK AND END OF DOOR IS 1/2" MAX.

DOOR ELEVATION

(INTERIOR ELEVATION)

SCALE: 1/4"= 1'-0"

02/22/02

SCALE	N.T.S.	DATE	
DRAWN BY	JAR	02-22-02	
INITIAL CHK.			
FINAL CHK.			
ENGR.			

Mid-America Door Company PONCA CITY, OK

DESCRIPTION: INSTALLATION ADDENDUM FOR WIND LOAD DOORS

TITLE

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DOOR FRAME PLATE CONNECTION SCHEDULE 1.1

WOOD CONSTRUCTION		QUANT. OF NAILS PER JAMB**		
		SYP	DOUG.FIR	SPF
TOTAL DOOR LOAD (LBS.)	LOAD PER JAMB (LBS.)	SPECIFIC GRA.=0.55	SPECIFIC GRA.=0.48	SPECIFIC GRA.=0.42
1000	500	5	7	10
1250	625	7	9	12
1500	750	8	11	15
1750	875	9	13	17
2000	1000	10	14	20
2250	1125	12	16	22
2500	1250	13	18	24
2750	1375	14	20	27
3000	1500	15	21	29
3250	1625	17	23	32
3500	1750	18	25	34
3750	1875	19	26	36
4000	2000	20	28	39
4250	2125	22	30	41
4500	2250	23	32	44
4750	2375	24	33	46
5000	2500	25	35	48

** - BASED ON 16dx 3 1/2" LONG THREADED, HARDENED NAIL ASSUMING 2" MIN. PENETRATION.
1. ANCHORS TO BE EVENLY SPACED BETWEEN THE HEADER AND THE FLOOR.
2. PREDRILL NAIL HOLES TO PREVENT SPLITTING.
3. CHART IS BASED ON 8'-0" MAX. DOOR HEIGHT.

DOOR FRAME PLATE CONNECTION SCHEDULE 1.2

WOOD CONSTRUCTION		QUANT. OF LAG SCREWS PER JAMB**		
		SYP	DOUG.FIR	SPF
TOTAL DOOR LOAD (LBS.)	LOAD PER JAMB (LBS.)	SPECIFIC GRA.=0.55	SPECIFIC GRA.=0.48	SPECIFIC GRA.=0.42
1000	500	3	3	3
1250	625	3	3	3
1500	750	3	3	3
1750	875	3	3	3
2000	1000	3	3	3
2250	1125	3	3	3
2500	1250	3	3	3
2750	1375	3	3	3
3000	1500	3	3	3
3250	1625	3	3	3
3500	1750	3	3	4
3750	1875	3	3	4
4000	2000	3	3	4
4250	2125	3	4	4
4500	2250	3	4	5
4750	2375	3	4	5
5000	2500	4	4	5
5250	2625	4	4	5
5500	2750	4	5	6
5750	2875	4	5	6
6000	3000	4	5	6
6250	3125	4	5	6
6500	3250	4	5	6
6750	3375	5	6	7
7000	3500	5	6	7
7250	3625	5	6	7
7500	3750	5	6	7

** - BASED ON 3/8"ø LAG SCREW ASSUMING 2" MIN. PENETRATION SEASONED WOOD, DRY USE.
1. ANCHORS TO BE EVENLY SPACED BETWEEN THE HEADER AND THE FLOOR.
2. CHART IS BASED ON 8'-0" MAX. DOOR HEIGHT.

DOOR FRAME PLATE CONNECTION SCHEDULE 2.1

MASONRY/CONCRETE CONSTRUCTION		QUANTITY OF TAPCON ANCHORS PER JAMB**	
TOTAL DOOR LOAD (LBS.)	LOAD PER JAMB (LBS.)	HOLLOW BLOCK	3,000 PSI CONCRETE
1000	500	3	3
1250	625	3	3
1500	750	3	3
1750	875	4	3
2000	1000	4	3
2250	1125	5	3
2500	1250	5	3
2750	1375	6	4
3000	1500	6	4
3250	1625	6	4
3500	1750	7	5
3750	1875	7	5
4000	2000	8	5
4250	2125	8	6
4500	2250	9	6
4750	2375	9	6
5000	2500	10	6
5250	2625	10	7
5500	2750	11	7
5750	2875	11	7
6000	3000	11	8
6250	3125	12	8
6500	3250	12	8
6750	3375	13	9
7000	3500	13	9
7250	3625	14	9
7500	3750	14	9

**- BASED ON 1/4"ø TAPCON ANCHOR WITH A 1 1/2" MIN. EMBEDMENT.
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DOOR FRAME PLATE CONNECTION SCHEDULE 3.1

MASONRY/CONCRETE CONSTRUCTION		QUANTITY OF EXPANSION ANCHORS PER JAMB**		
TOTAL DOOR LOAD (LBS.)	LOAD PER JAMB (LBS.)	GROUT-FILLED BLOCK	2,000 PSI CONCRTE	3,000 PSI CONCRTE
1000	500	3	3	3
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1500	750	3	3	3
1750	875	3	3	3
2000	1000	3	3	3
2250	1125	3	3	3
2500	1250	3	3	3
2750	1375	3	3	3
3000	1500	3	3	3
3250	1625	3	3	3
3500	1750	4	3	3
3750	1875	4	3	3
4000	2000	4	3	3
4250	2125	4	3	3
4500	2250	4	3	3
4750	2375	5	3	3
5000	2500	5	3	3
5250	2625	5	3	3
5500	2750	5	3	3
5750	2875	6	3	3
6000	3000	6	3	3
6250	3125	6	3	3
6500	3250	6	3	3
6750	3375	6	3	3
7000	3500	7	4	3
7250	3625	7	4	4
7500	3750	7	4	4

**- BASED ON 3/8"ø HILTI STUD TYPE EXPANSION ANCHOR WITH A 2 1/2" MIN. EMBEDMENT.
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NOTES

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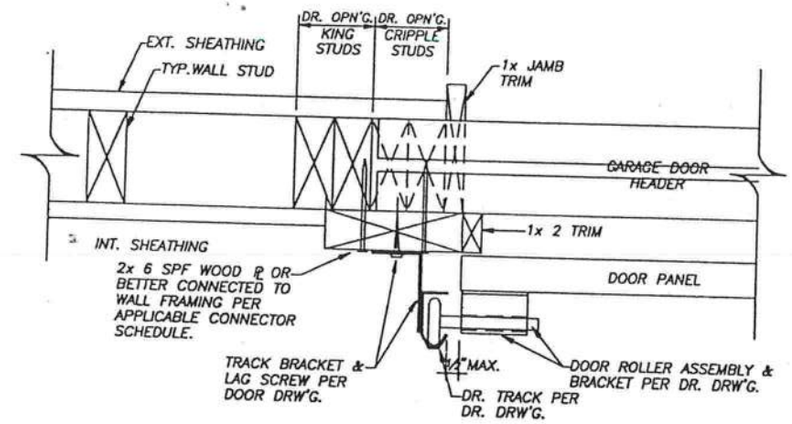
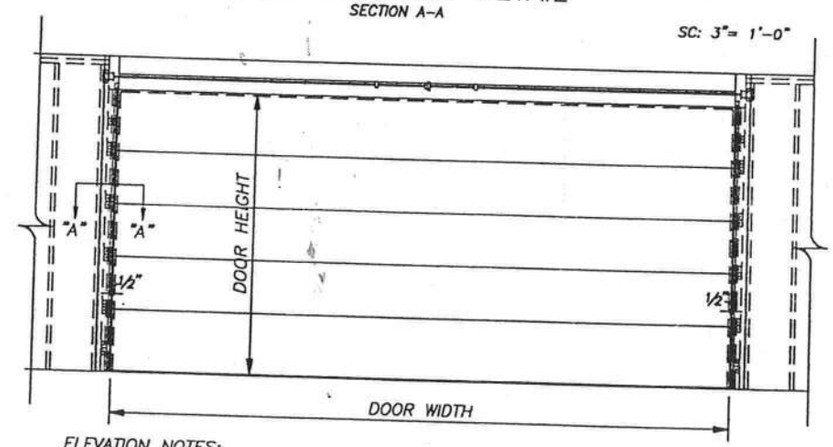


JAMB FASTENER DETAIL

SECTION A-A

SC: 3"= 1'-0"



ELEVATION NOTES:
1. SPACE BETWEEN INSIDE OF VERTICAL TRACK AND END OF DOOR IS 1/2" MAX.

DOOR ELEVATION

(INTERIOR ELEVATION)

SCALE: 1/4"= 1'-0"

02/22/02

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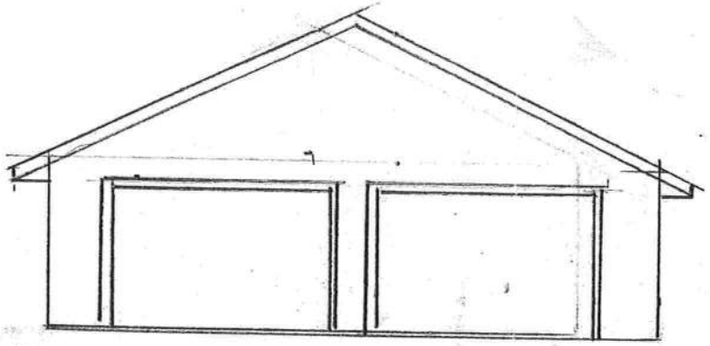
SCALE	N.T.S.
DRAWN BY	JAR
DATE	02-22-02
INITIAL CHK.	
FINAL CHK.	
ENGR.	
APPR. BY	

JOB | BY | DATE | BY | DATE | BY | DATE | BY | DATE

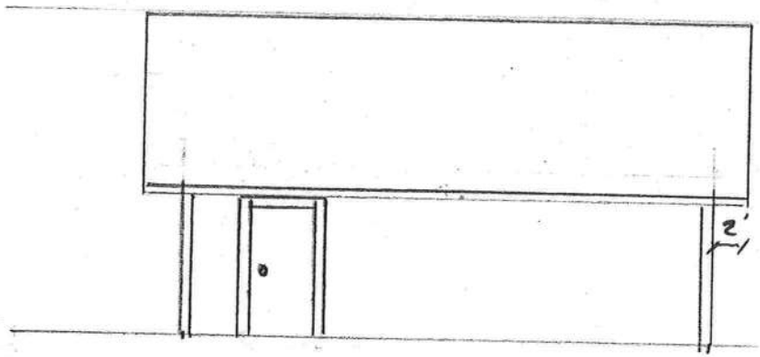
Mid-America Door Company PONCA CITY, OK

DESCRIPTION: INSTALLATION ADDENDUM FOR WIND LOAD DOORS

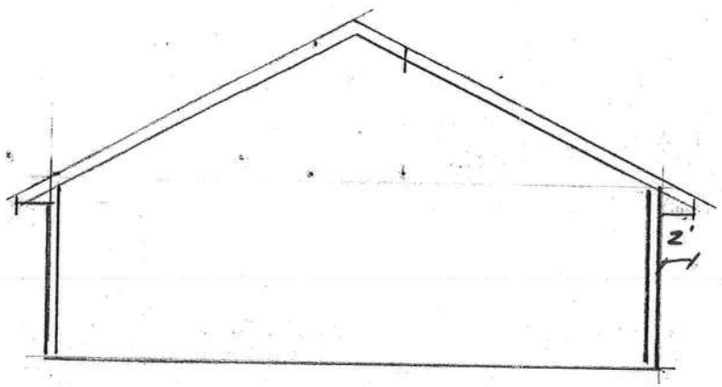
TITLE



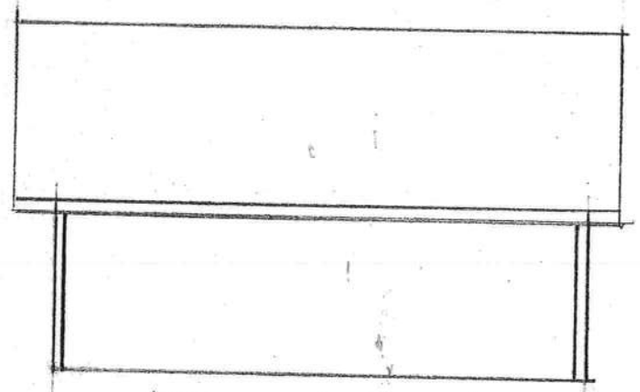
FRONT



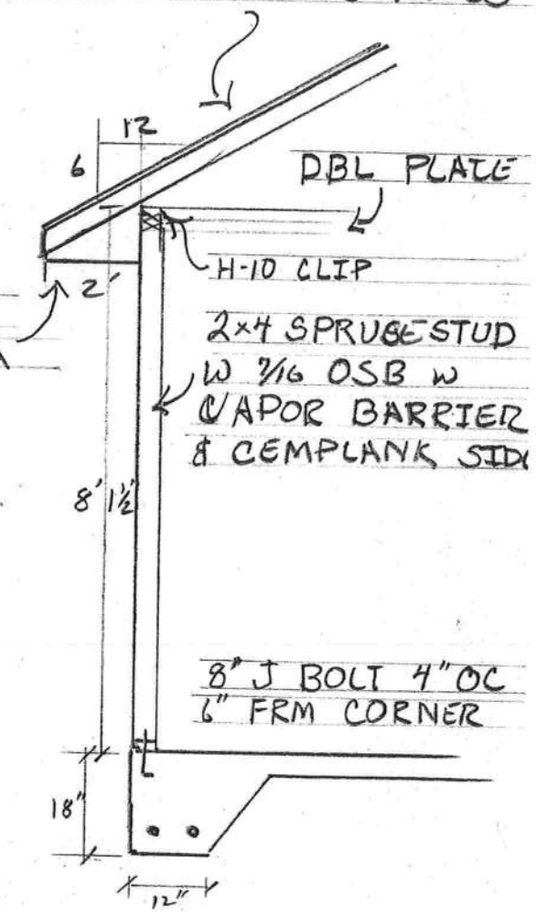
RIGHT



REAR



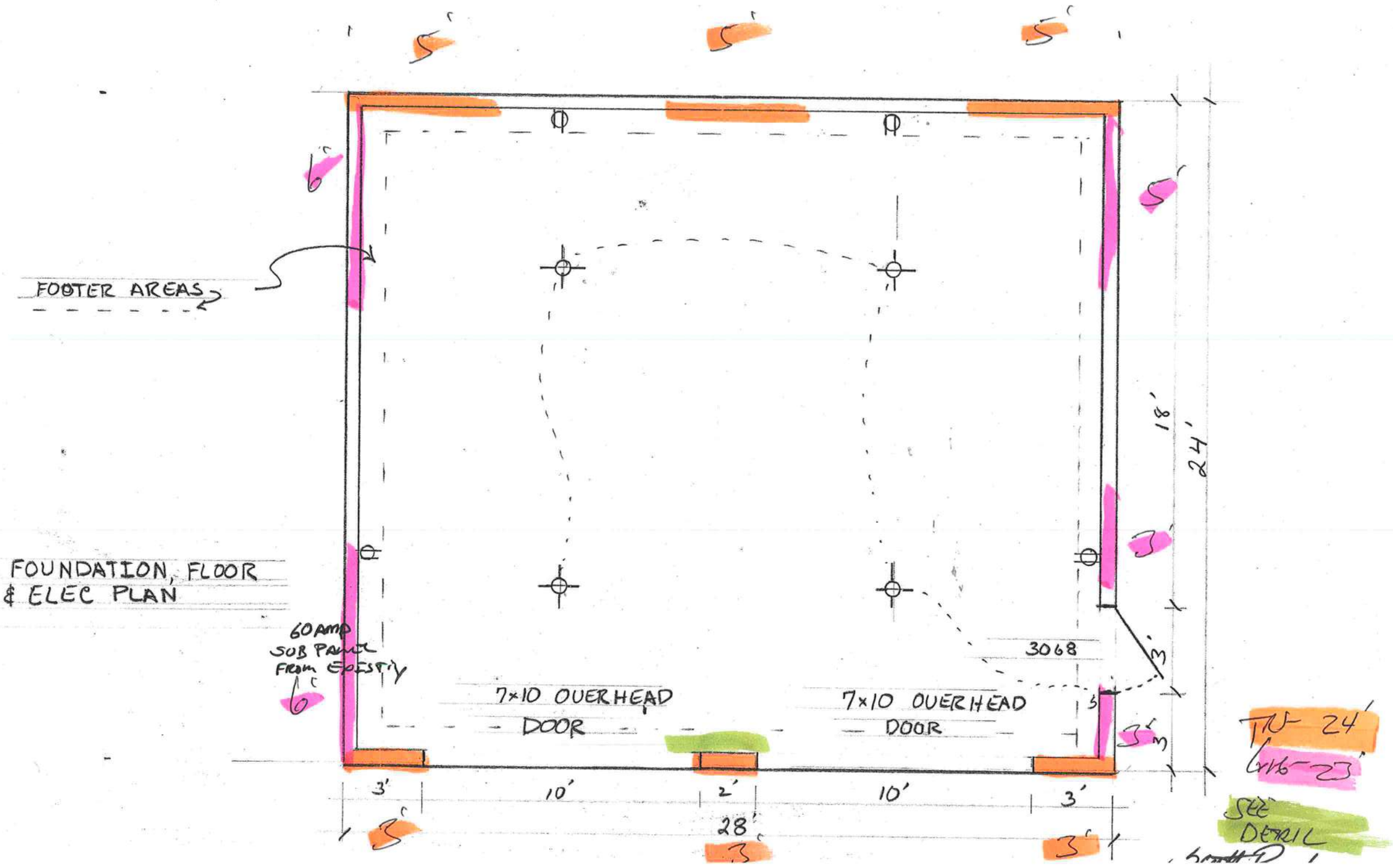
LEFT

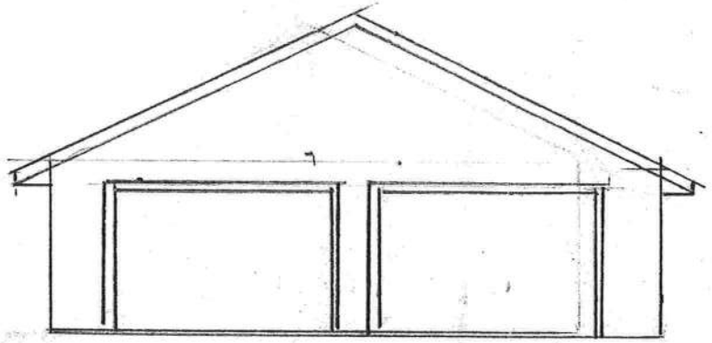


4" 3000 PSI W FIBER OVER 6 Mill VAPOR BARRIER
W .025% SOIL POISION ON CLEAN COMPACED FILL.
FOOTER 14x12 W 2#5 CONT MIN SOIL CAP
1000# PSF

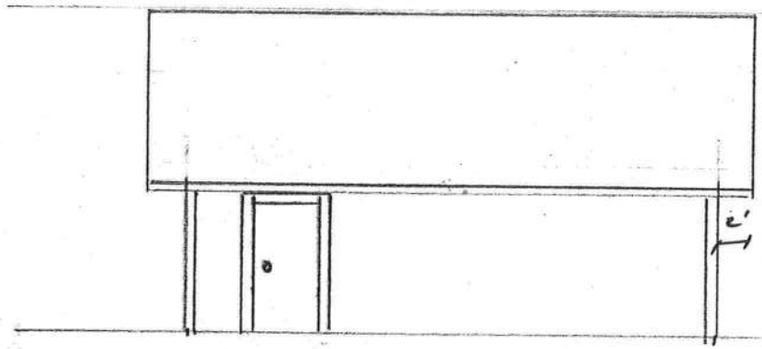
Bradley ~~GARAGE~~

Matt D. 1

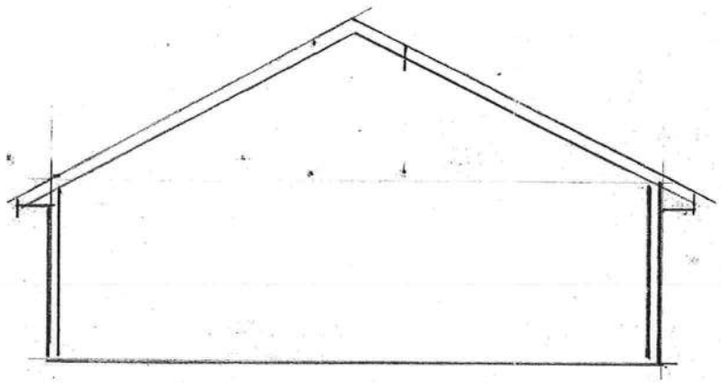

FOOTER AREAS
FOUNDATION, FLOOR & ELEC PLAN
60 AMP SUB PANEL FROM EXISTING
7×10 OVERHEAD DOOR
7×10 OVERHEAD DOOR
3068
18'
24'
3'
10'
2'
10'
3'
28'
5'
6'
TW 24'
GVB 23'
SEE DETAIL



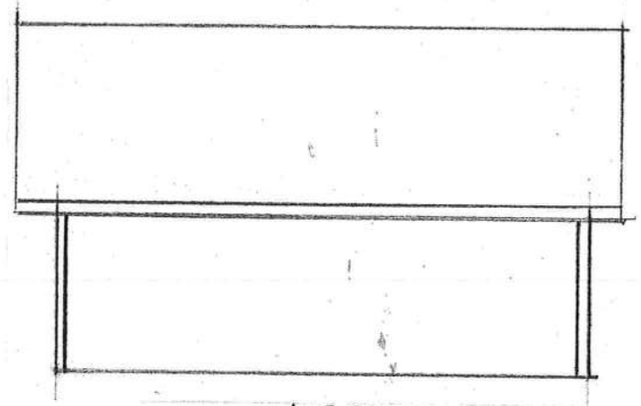
FRONT



RIGHT

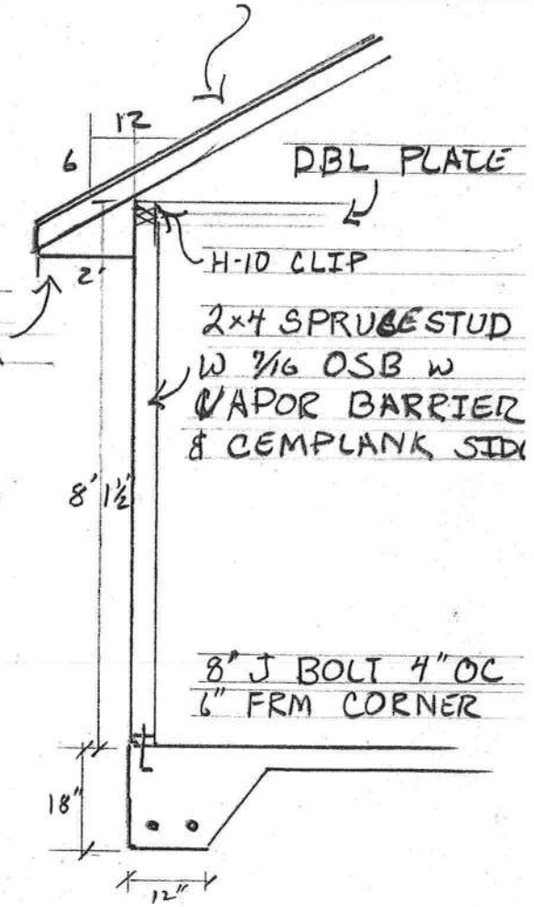


REAR



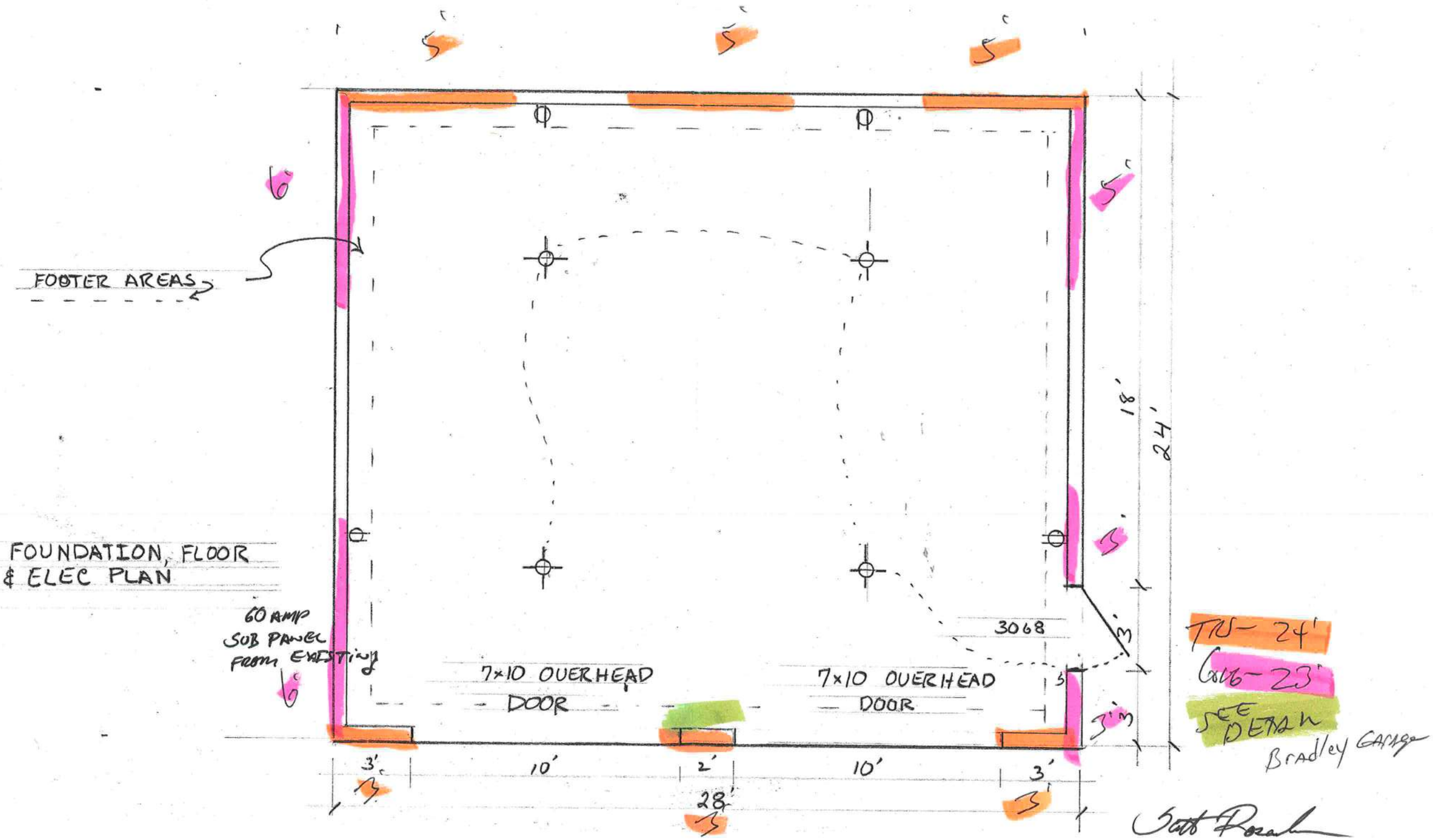
LEFT

30yr ARC SHINGLES OVER
30lb FELT OVER 7/16 OSB
OVER PRE ENG TRUSS

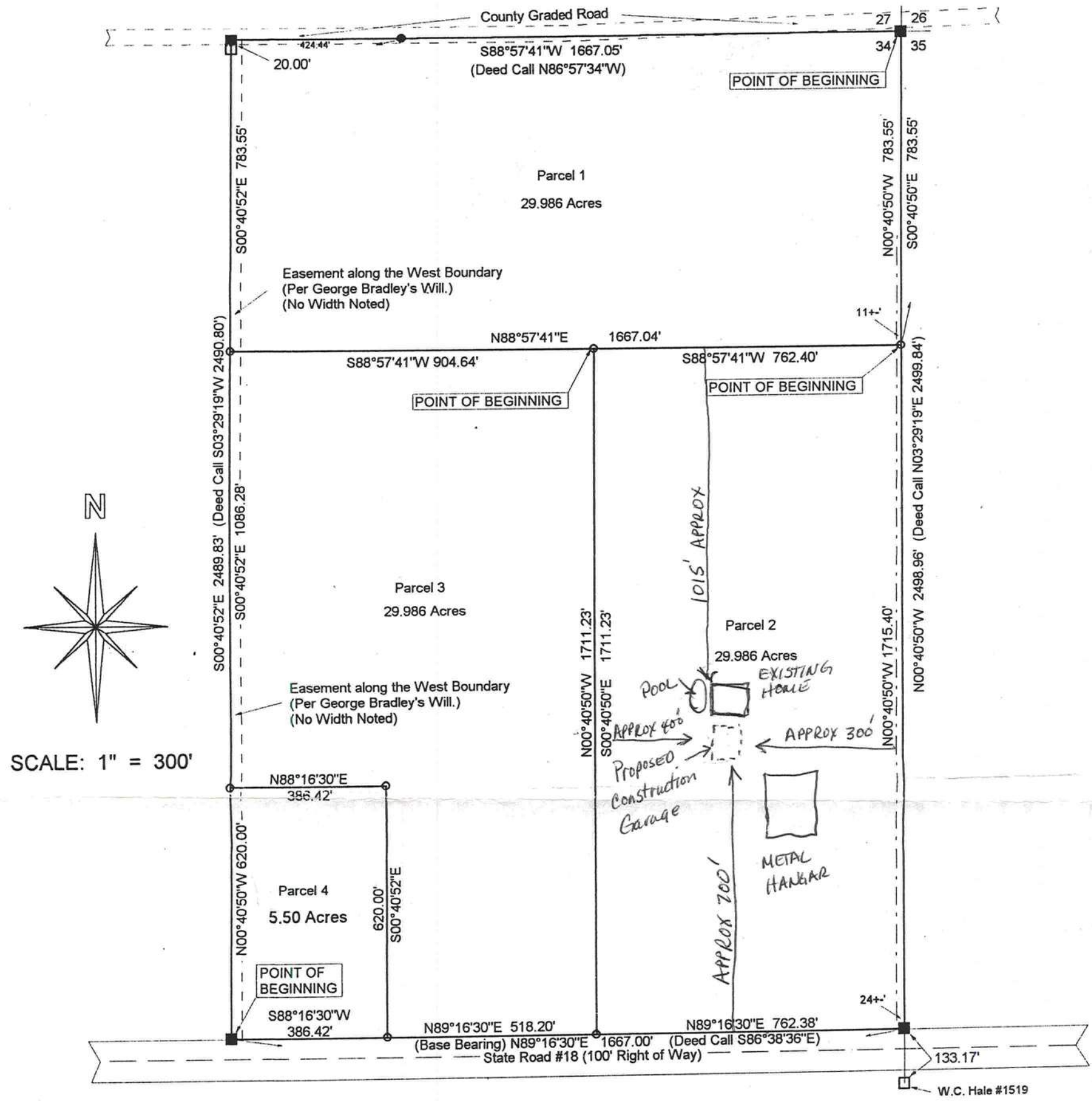


4" 3000 PSI W FIBER OVER 6 Mill VAPOR BARRIER
W .025% SOIL POISION ON CLEAN COMPACED FILL.
FOOTER 14x12 W 2#5 CONT MIN SOIL CAP
1000# PSF

Scott Rosen


FOOTER AREAS
FOUNDATION, FLOOR & ELEC PLAN
60 AMP SUB PANEL FROM EXISTING
7×10 OVERHEAD DOOR
7×10 OVERHEAD DOOR
3068
3'
10'
2'
10'
3'
28'
18'
24'
TW- 24'
GW- 23'
SEE DETAIL
Bradley Garage

BOUNDARY SURVEY FOR BRADLEY ESTATE
LYING IN THE NORTHEAST QUARTER OF SECTION 34, TOWNSHIP 6 SOUTH, RANGE 16 EAST
COLUMBIA COUNTY, FLORIDA
11 SEPTEMBER 2000



LEGEND & NOTES

■ - Denotes 4" X 4" Concrete Marker Found, No Identification.

□ - Denotes 4" X 4" Concrete Marker Found, W.C. Hale Corner.

● - Denotes 1/2" Iron Rod Found #LB6685.

o - Denotes 1/2" Iron Rod Set, #3499.

— - —- Denotes Fence (not to scale).

Bearings projected from DOT right-of-way map.

Improvements not Located.

Reference Surveys by W. C. Hale #1519, Britt Surveying #1079, and Corbett Horne #3048.

THIS SURVEY WAS PREFORMED WITHOUT THE BENEFIT OF A TITLE REPORT AND NO SEARCH OF THE PUBLIC RECORDS WAS MADE FOR OWNERSHIP, EASEMENTS, UTILITIES, RIGHT-OF-WAYS, AND ETC.

THE SURVEY DEPICTED HERE IS NOT COVERED BY PROFESSIONAL LIABILITY INSURANCE.

"UNLESS IT BEARS THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER THIS DRAWING, SKETCH, PLAT OR MAP IS FOR INFORMATIONAL PURPOSES ONLY AND IS NOT VALID."

"SEE ATTACHED SHEET FOR DESCRIPTIONS"

THIS IS TO CERTIFY TO BRADLEY ESTATE **THAT THIS IS A TRUE AND CORRECT REPRESENTATION OF FIELD WORK DEPICTED HEREON AND MEETS THE MINIMUM TECHNICAL STANDARDS SET FORTH BY THE FLORIDA BOARD OF PROFESSIONAL LAND SURVEYORS IN CHAPTER 61G17-6, FLORIDA ADMINISTRATIVE CODE, PURSUANT TO SECTION 472.027 FLORIDA STATUTES, TO THE BEST OF MY KNOWLEDGE AND BELIEF.**

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