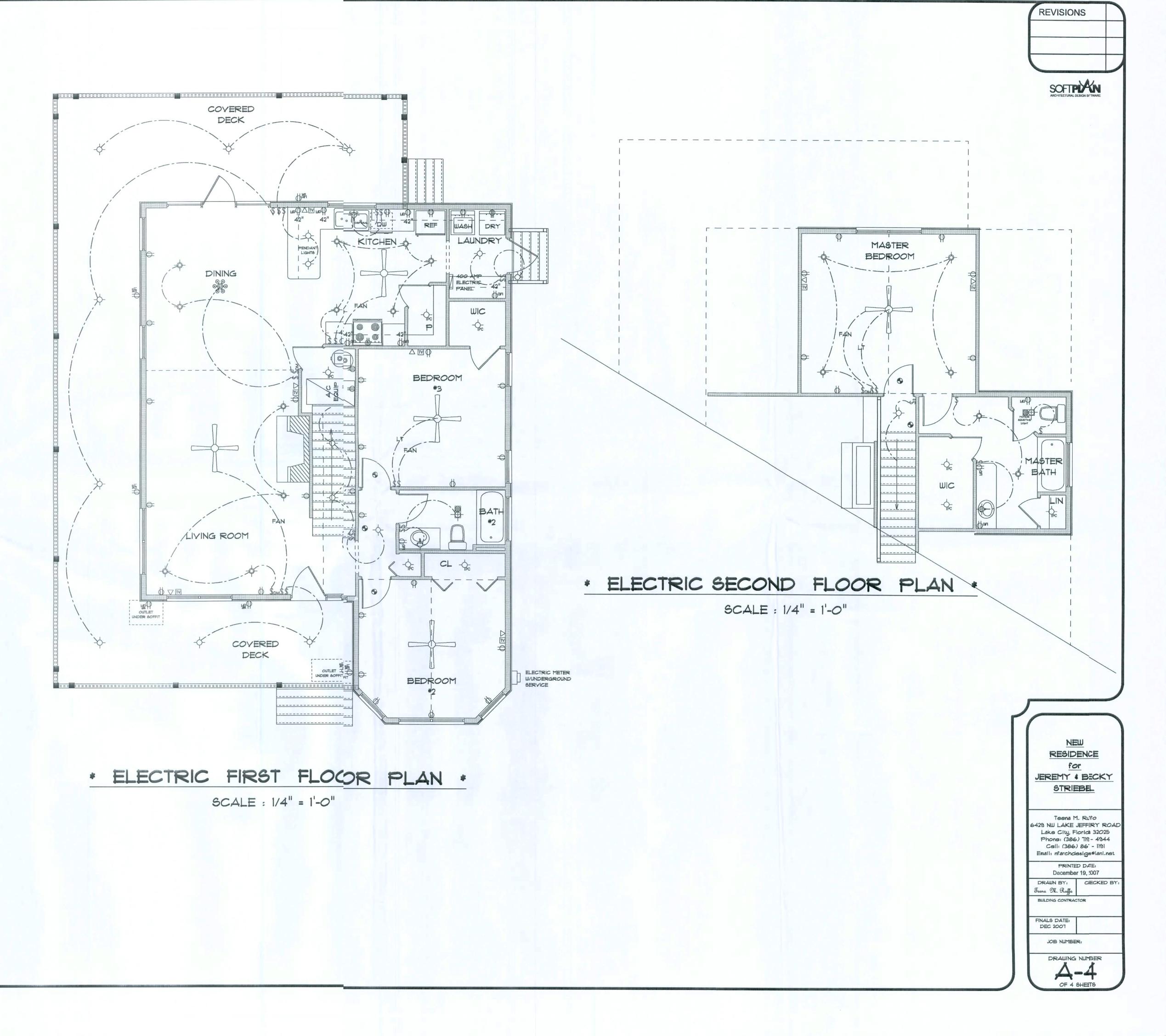


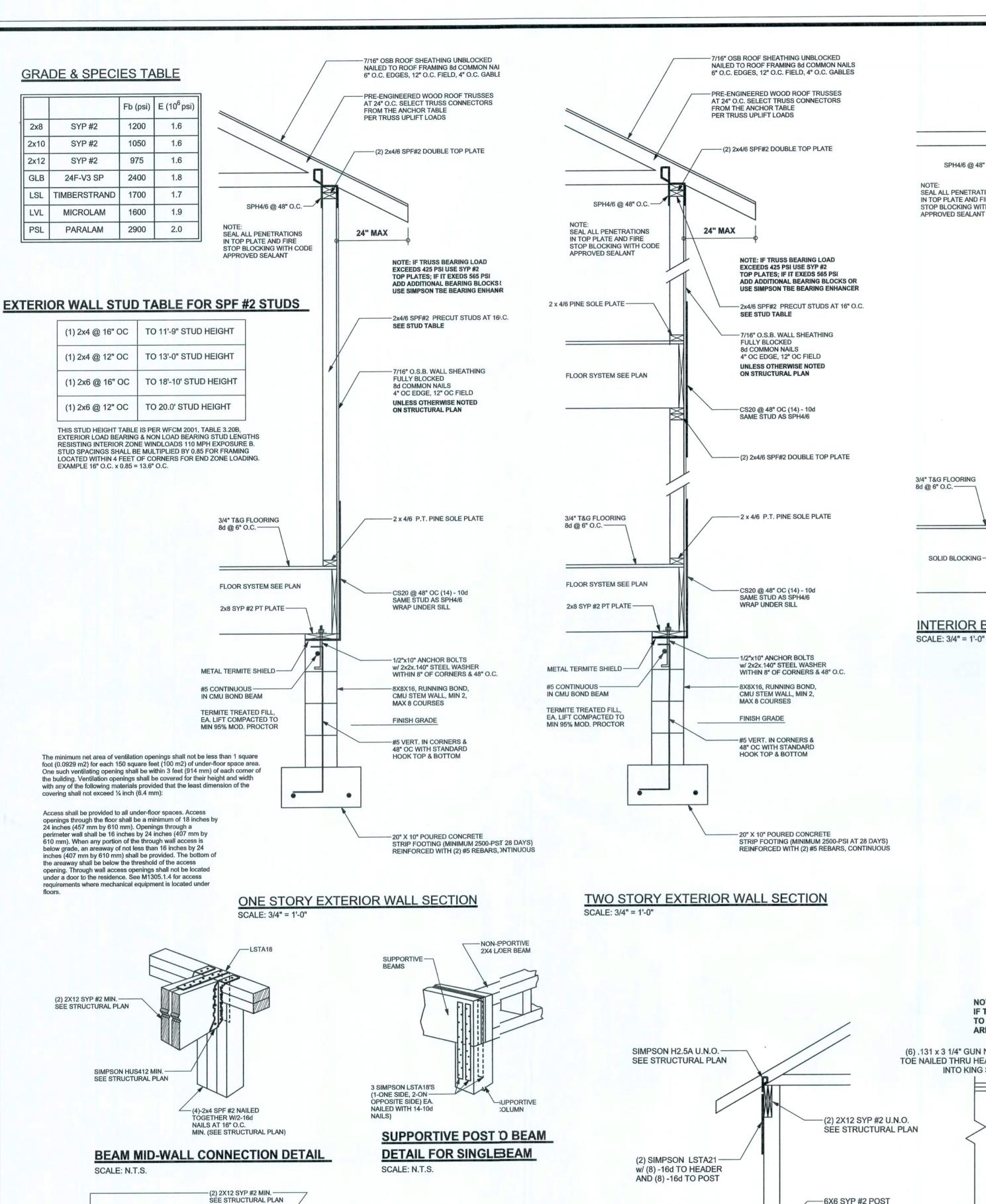
ELECTRICAL	COUNT	SYMBOL
chandelier	1	6 % G 6 % G
400 amp elec panel	1	400 Are
cable tv outlet	7	<u>⊠</u>
ceiling fan	5	
dimmer switch	3	\$ъм
electric meter	1	ELECTRIETER WANDEROUND SERVICE
gfi waterproof outlet	6	Q ₃₀
light	19	
outlet	33	ě.
outlet 220v	4	₫
outlet gfi	10	∯≈
pull chain light	8	4:
recessed can light	17	4
smoke detector	5	9
switch	22	\$
switch 3 way	2	\$,
telephone	7	Δ
vent light combo	2	

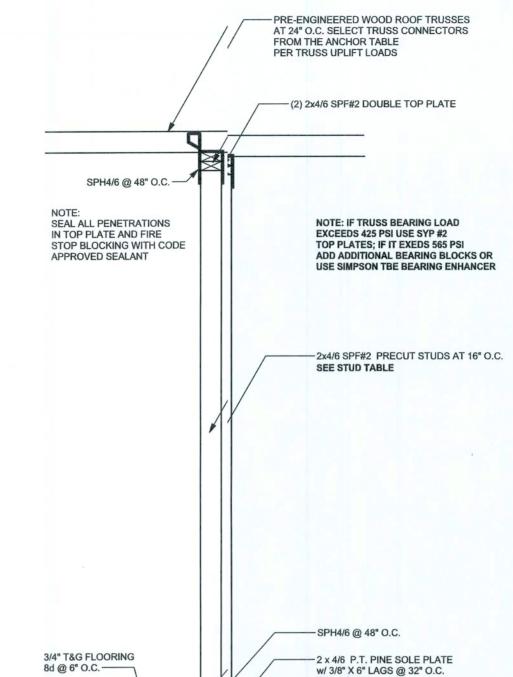
ELECTRICAL PLAN NOTES

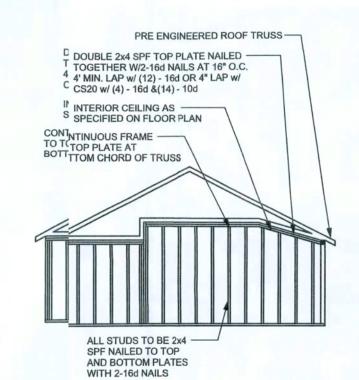
- E-I WIRE ALL APPLIANCES, HYAC UNITS AND OTHER EQUIPMENT PER MANUFACTORS SPECIFICATIONS.
- E-2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHON LINES TO BE INSTALLED.
- E-4 ALL INSTALLATIONS SHALL BE PER NATIONAL ELECTRIC CODE.
- E-3 ALL SMOKE DETECTORS SHALL BE 120V W/BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER, INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E-5 TELEPHONE, TELEVVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNERS DIRECTION AND IN ACCORDANCE WITH APPLICABLE SECTIONS OF NATIONAL ELCT. CODE LATEST EDITION.
- E-6 ELECTRICAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E-7 ENTRY OF SERVICE UNDERGROUND OR OVERHEAD) IS TO BE DETERMINED BY THE POWER COMPANY.
- E-8 ALL BEDROOM RECEPTICALS ARE TO BE AFCI (ARC FAULT CIRCIT INTERRUPT)

NOTE: SEPARATE CIRCUITS FOR EACH ROOT









TO BLOCKING

CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

(1) 2X4 SPF #2 SILL UP TO 7'-3" U.N.O.

(FOR: 110 MPH, 10'-0" WALL HIGHT U.N.O.)

TYPPICAL HEADER STRAPING DETAIL
SCALELE: 1/2" = 1'-0"

IF TRUSS TO WAIALL STRAPS ARE NAILED TO THE HEADER R THE SPH4/6 @ 48" O.C. ARE NOT REQUIFIRED

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END: 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" × 4" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302, JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT, THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT, DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN

ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY: 4"OC. UNO

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/04"; WITH 5/03/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS. FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

OF LIFT LDG. STF	OFLIFT LDS. SFF	TRUSS CONNECTOR	TOPLATES	TO RAFTER/TRUSS	TO STUDS	
< 420	< 245	H5A	3-8d	3-8d		
< 455	< 265	H5	4-8d	4-8d		
< 360	< 235	H4	4-8d	4-8d		
< 455	< 320	H3	4-8d	4-8d		
< 415	< 365	H2.5	5-8d	5-8d		
< 600	< 535	H2.5A	5-8d	5-8d		
< 950	< 820	H6	8-8d	8-8d		
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"		
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"		
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"		
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"		
< 760	< 655	H10-2	6-10d	6-10d		
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"		
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"		
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"		
< 2900	< 2490	2 - HTS24				
< 2050	< 1785	LGT2	14 -16d	14 -16d		
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION	
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT	
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROI 12" EMBEDMENT	
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROI 12" EMBEDMENT	
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROI 12" EMBEDMENT	
		STUD STRAP CONNECTOR*			TO STUDS	
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d	
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d	
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d	
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d	
< 885	< 760	SP4			6-10d, 1 1/2"	
< 1240	< 1065	SPH4			10-10d, 1 1/2"	
< 885	< 760	SP6			6-10d, 1 1/2"	
< 1240	< 1065	SPH6			10-10d, 1 1/2"	
< 1235	< 1165	LSTA18	14-10d			
< 1235	< 1235	LSTA21	16-10d			
< 1030	< 1030	CS20	18-8d			
< 1705	< 1705	CS16	28-8d			
		STUD ANCHORS*	TO STUDS		TO FOUNDATION	
< 1350	< 1305	LTT19	8-16d		1/2" AB	
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB	
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB	
	< 3695	HTT16	18 - 16d		5/8" AB	
< 4175			16-16d			
< 4175 < 1400	< 1400	PAHD42	10-100			
	< 1400 < 3335	PAHD42 HPAHD22	16-16d			
< 1400					1/2" AB	
< 1400 < 3335	< 3335	HPAHD22	16-16d		1/2" AB 1/2" AB	

UPLIFT LBS. SYP UPLIFT LBS. SPF TRUSS CONNECTOR* TO PLATES TO RAFTER/TRUSS TO STUDS

REVISIONS

Zone Effective Wind Area (ft2)

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> CERTIFICATIOI: I hereby certify that I have amined this plin, and that the applicable ortions of the pan, relating to wind engine comply with secon R301.2.1, florida building ode residential?004, to the best of my

WINDLOAD EN INEER: Mark Disosway,

PE No.53915, P)B 868, Lake City, FL

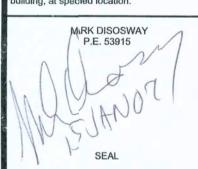
ated dimensions supercede scaled

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mensions. Refr all questions to Mark Disosway, 2.E. for resolution.

32056, 386-7545419

LIMITATION: The design is valid for one building, at spedied location.



Jereny & Rebecca Striebel Residence

ADDRESS: Columia County, Florida

Mark Disosway P.E. P.J. Box 868 Lake Ciy, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: January 15, 2008 CHECKED BY: David Disosway

FINALS DATE: 15 / Jan / (8

> JOE NUMBER 801101 DRAWING NUMBER

> > 5-1 CF 4 SHEETS

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.) BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

) BASIC WIND SPEED = 110 MPH

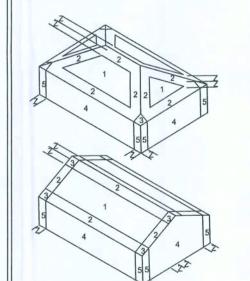
) WIND EXPOSURE = B .) WIND IMPORTANCE FACTOR = 1.0

4.) BUILDING CATEGORY = II 5.) ROOF ANGLE = 10-45 DEGREES

6.) MEAN ROOF HEIGHT = <30 FT

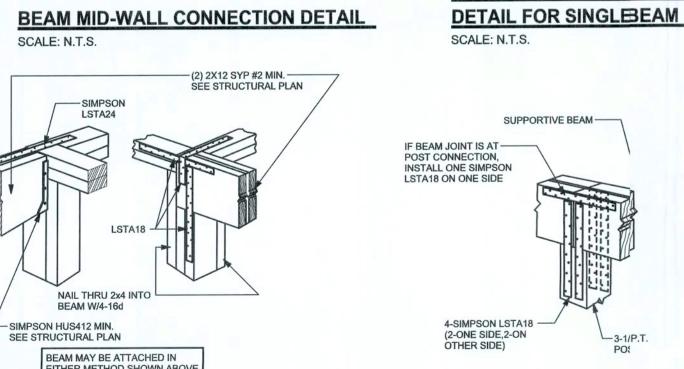
) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



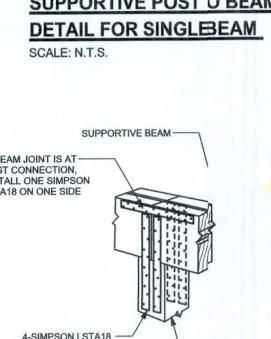
		1	0		100
	1	19.9	-21.8	18.1	-18.1
	2	19.9	-25.5	18.1	-21.8
3	2 O'hg		-40.6		-40.6
	3	19.9	-25.5	18.1	-21.8
	3 O'hg		-68.3		-42.4
	4	21.8	-23.6	18.5	-20.4
	5	21.8	-29.1	18.5	-22.6
	1 2 2 2 2 2 2	& Wind st Cas 5, 10	е	21.8	-29.1
	8x7 Gar	age D	oor	19.5	-22.9
	16x7 Ga	arage [Door	18.5	-21.0

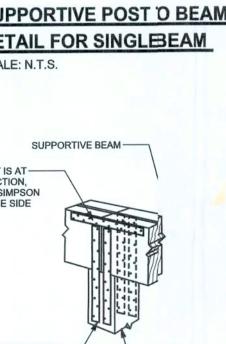
DESIGN	LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)	
	30 PSF (SLEEPING ROOMS)	
	30 PSF (ATTICS WITH STORAGE)	
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)	
ROOF	20 PSF (FLAT OR <4:12)	
	16 PSF (4:12 TO <12:12)	
	12 PSF (12:12 AND GREATER)	
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)	
SOIL BE	EARING CAPACITY 1000PSF	
	FLOOD ZONE (BUILDER TO VERIFY)	



BEAM CORNER CONNECTION. DETAIL

SUPPORTIVE CENTER POSTO BEAM DETAIL SCALE: N.T.S.





(6) .131 x 3 1/4" GUN NAILS--(6) .131 x 3 1/4" GUN NAILS TOE NAILED THRU HEADER TOE NAILED THRU HEADER - SPH4/6 ALL OPENINGS (U.N.O.) INTO KING STUD INTO KING STUD ∠SPH4/6 @ 48" O.C. (U.N.O.) CRIPPLES IF REQUIRED (4) .131 x 3 1/4" GUN NAILS TOE NAILED THRU SILL -INTO JACK STUD U.N.O. -SIMPSON ABU POST BASE TYPICAL STRAPPING (U.N.O.) w/ (12) - 16d & 5/8" x 10" ANCHOR BOLT (SEE STRUCTURAL PLAN) CS20 ALL OPENINGS (U.N.O.) -SEE FOOTING DETAILS (1) 2X6 SPF #2 SILL UP TO 11'-0" U.N.O.

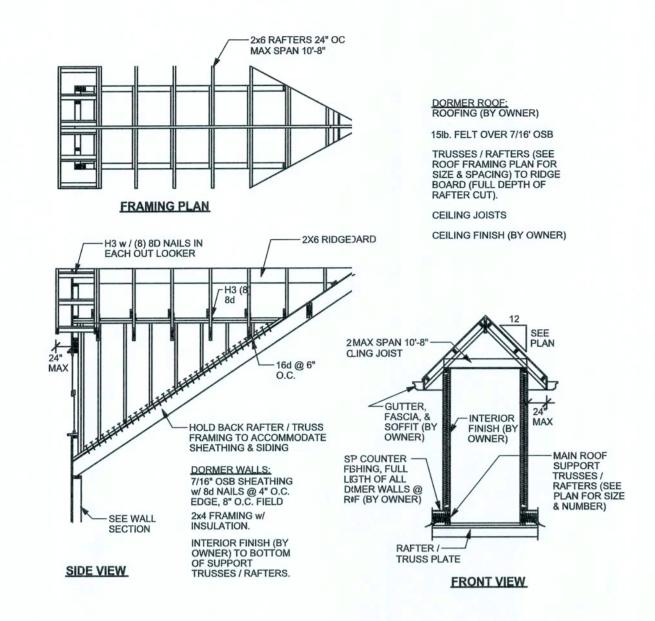
MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

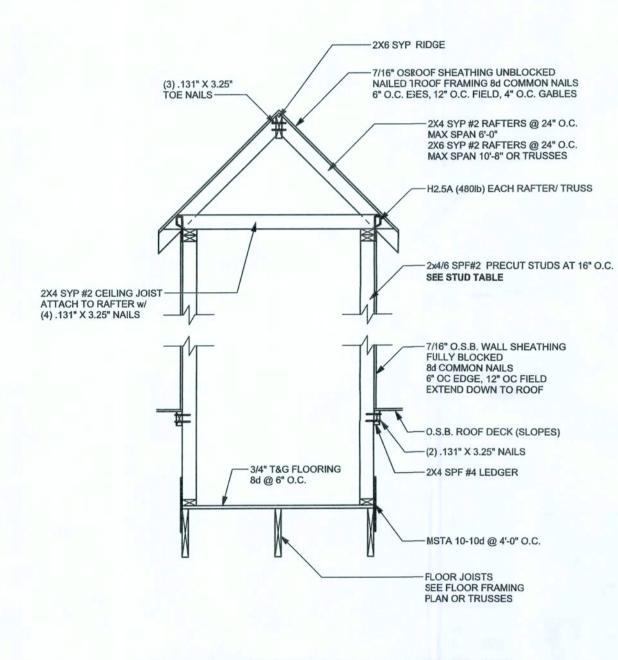
	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

ACI530 1-02 Section Specific Requirement

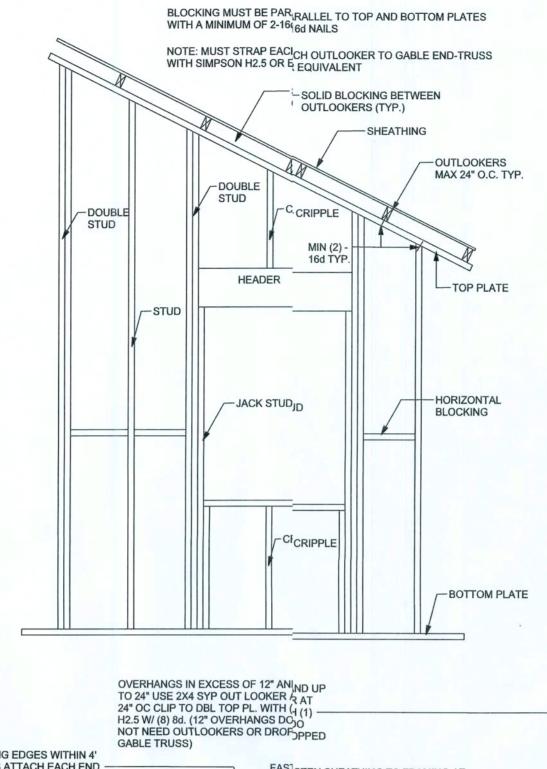
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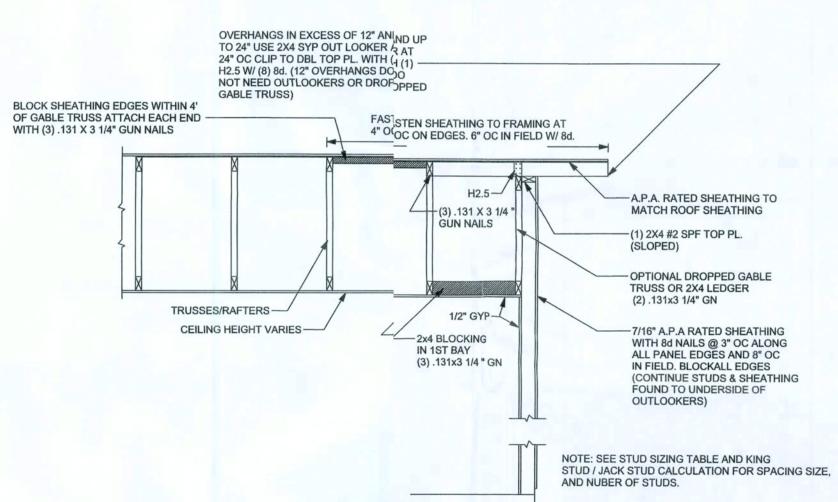


DORMER ANCHORIG DETAIL (ON ROOF)
SCALE: N.T.S.

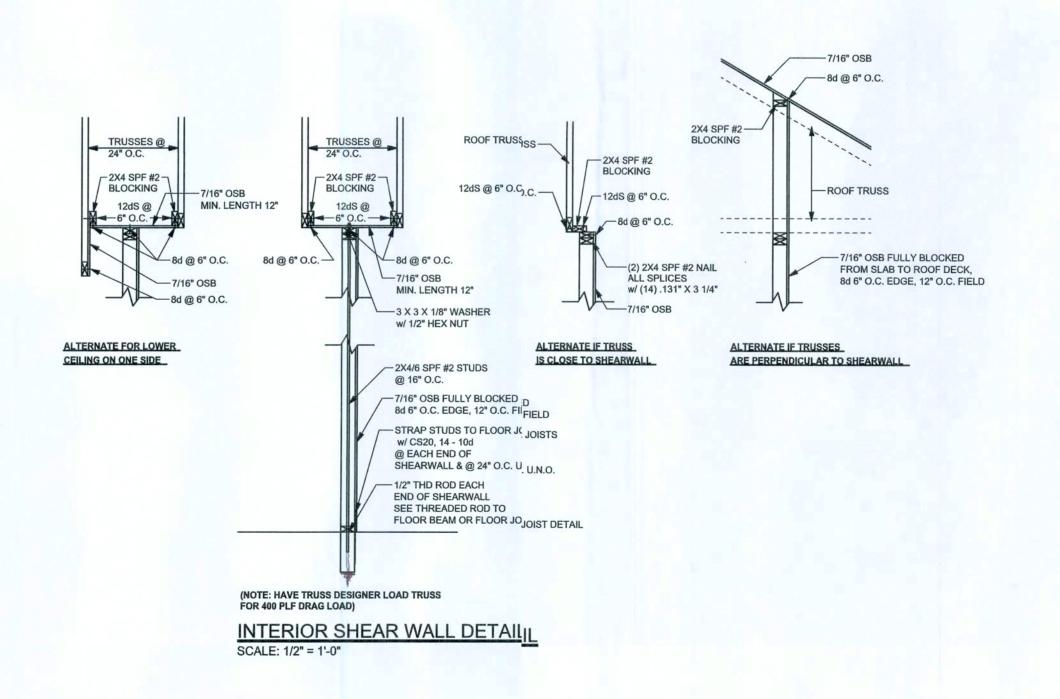


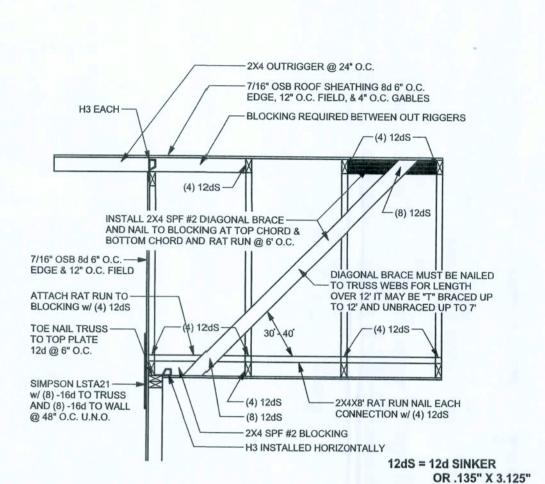
DORMER ANCHORING DETAI (ON FLOOR)
SCALE: N.T.S.





GABLE END WALL BALL LOON FRAMING DETAIL SCALE: 1/2" = 1'-0"





SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C. FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, ENCLOSED

OR .131 X 3.25"

GABLE BRACING DETAIL SCALE: 1/2" = 1'-0"

REVISIONS

SOF PLAN

WINDLOAD ENGINER: Mark Disosway, PE No.53915, POB t68, Lake City, FL 32056, 386-754-541! Stated dimensions spercede scaled dimensions. Refer alquestions to Mark Disosway, P.E.for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E.hereby expressly reserve its common law copyights and property right in these instruments of service. This document is not to be reproduced altered or copied in any form or manner without first the express written permission and consent of Mark Disosway. CERTIFICATION: I breby certify that I have examined this plan, and that the applicable portions of the plan, elating to wind engineering comply with section I301.2.1, florida building code residential 2004 to the best of my LIMITATION: This deign is valid for one building, at specified ocation. MARKDISOSWAY P.I. 53915

Jeremy & Rebecca StriebelResidence

ADDRESS: Columbia County, Florida

Mark Disosway P.E. P.O. 3ox 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

> PRINTED DATE: January 15, 2008

> > CHECKED BY:

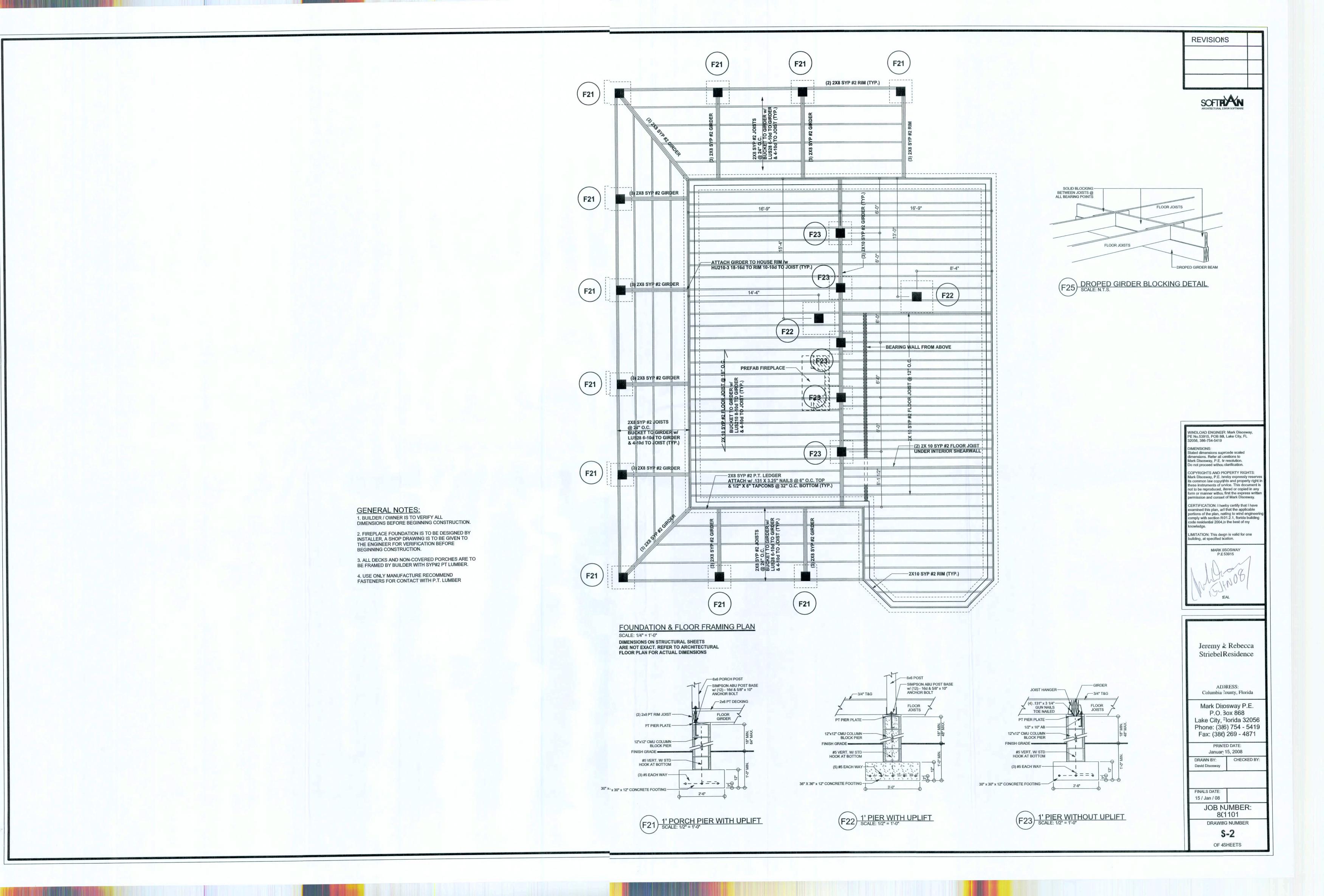
FINALS DATE: 15 / Jan / 08

DRAWN BY:

David Disosway

JOB NUMBER: 801101 DRAWING NUMBER

S1.1 OF 43HEETS



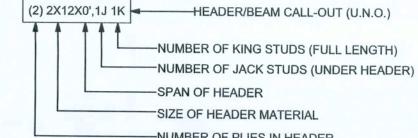
JC3 JC5 STRUCTURAL PLAN NOTES ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.) SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.) JE7A DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS A4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. -2000 LB UPLIFT A5 (3) 2X6 SPF #2 STUDS CENTERED UNDER TRUSS A19 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE (2) 1.75"X12.00"X9'LVL,3J 2K WALL LEGEND (2) SPH6 TOP (2) CS20 BOTTOM —(2) SPH6 TOP (2) CS20 BOTTOM -2000 LB UPLIFT USE H2.5A (535lb) FOR ALL TRUSS TO WALL FRAME AND PORCH BEAM CONNECTIONS UNLESS NOTED OTHERWISE -6X6 SYP #2 POST -(3) HTS24 TOP **ABU66 BOTTOM** A7 A28 -954 LB A12 **HEADER LEGEND** A13 A14 —(2) 2X4 SPF #2 STUDS A15 CENTERED UNDER TRUSS -SPAN OF HEADER -954 LB SIZE OF HEADER MATERIAL A16 UPLIFT A24 ---NUMBER OF PLIES IN HEADER TOTAL SHEAR WALL SEGMENTS SWS = 0.0' INDICATES SHEAR WALL SEGMENTS REQUIRED ACTUAL TRANSVERSE 32.0' 32.0' UPLIFT LONGITUDINAL 30.0' JC5 JE7A JC3 JC5 -SEE PORCH POST DETAIL (TYP.) STRUCTURAL ROOF PLANN SCALE: 1/4" = 1'-0"

REVISIONS

SOFTPIAN ARCHITECTUAL DESIGN SOFTWARE

LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR WALL
SWS = 0.0'	INTERIOR NON BEARING SHEARWALLS SEE DETAILS
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS



WINDLOAD ENGINIER: Mark Disosway, PE No.53915, POB 68, Lake City, FL 32056, 386-754-541

Stated dimensions spercede scaled dimensions. Refer alquestions to Mark Disosway, P.Efor resolution. Do not proceed withut clarification. COPYRIGHTS AND PROPERTY RIGHTS:
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permission and consent of Mark Disosway. CERTIFICATION: I lareby certify that I have examined this plan, and that the applicable portions of the plan, elating to wind engineering comply with section (301.2.1, florida building code residential 200, to the best of my

LIMITATION: This design is valid for one building, at specifiedocation.

MARKDISOSWAY P.I. 53915

Jeremy & Rebecca StriebelResidence

ADDRESS: Columbia County, Florida

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PRIN'ED DATE: Januar 15, 2008 DRAWN BY: CHECKED BY: David Disosway

FINALS DATE: 15 / Jan / 08

JOB NUMBER: 801101 DRAWING NUMBER

CONNECTIONS, WALL, & HEADER DESIGN IS BASED

JOB #5140

ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. W.B. HOWLAND TRUSS

S-3 OF 4SHEETS

STRUCTURAL FLOOR PLAN SCALE: 1/4" = 1'-0"

A7 TRUSS

6X6 SYP #2 POST -

(3) HTS24 TOP **ABU66 BOTTOM**

TRUSSES ARE LOADED CORRECTLY

BUILDERS TO VERIFY THAT

SWS = 5.0

SWS = 12.0'

6X6 SYP #2 POST (3) HTS24 TOP

SWS = 10.0°

SEE DETAIL

INTERIOR SHEARWALL

ABU66 BOTTOM