

TYPICAL PORCH POST DETAIL

BEAM CORNER CONNECTION. DETAIL

SCALE: N.T.S.

SUPPORTIVE CENTER POST TO BEM DETAIL

SCALE: N.T.S.

TYPICAL 1 STORY HEADER STRAPING DETAIL

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

VASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

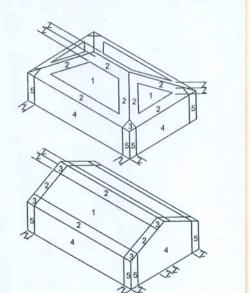
NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

SPECIFICALLY NOT PART O	ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUN BACKFILL HEIGHT, WIND SPEED A	NDATION BEARING CAPACITY, GRADE AND ND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONST REQUIREMENTS FOR THE STATES	RUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PARELIEVE THE PLAN OMITS A CONT THE WIND LOAD ENGINEER IMMED	ATH FROM TRUSSES TO FOUNDATION. IF YOU INJUDIES LOAD PATH CONNECTION, CALL DIATELY.
DESIGN, PLACEMENT PLANS, TEM	ER'S SEALED ENGINEERING INCLUDES TRUSS PORARY AND PERMANENT BRACING DETAILS, AND UPLIFT AND REACTION LOADS FOR ALL

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER, IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

WIND LOADS PER FLORIDA BUILDING COD	
(ENCLOSED SIMPLE DIAPHRAGM BUILDING	SS WITH FLAT, HIPPED, OR GABLE ROOFS; ST HORIZONTAL DIMENSION OR 60 FT; NOT
ON UPPER HALF OF HILL OR ESCARPMENT	60FT IN EXP. B. 30FT IN EXP. C AND >10%
SLOPE AND UNOBSTRUCTED UPWIND FOR	R 50x HEIGHT OR 1 MILE WHICHEVER IS LESS
BUILDING IS NOT IN THE HIGH VELOCITY H	URRICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEB	BRIS REGION
1.) BASIC WIND SPEED = 110 MPH	
2.) WIND EXPOSURE = B	
3.) WIND IMPORTANCE FACTOR = 1.0	
4.) BUILDING CATEGORY = II	
5.) ROOF ANGLE = 10-45 DEGREES	
6.) MEAN ROOF HEIGHT = <30 FT	
7.) INTERNAL PRESSURE COEFFICIENT =	N/A (ENCLOSED BUILDING)
8.) COMPONENTS AND CLADDING DESIGN	N WIND PRESSURES (TABLE R301.2(2))
Æ.	Zone Effective Wind Area (ft2)
	10 100 1 19.9 -21.8 18.1 -18.1
2 2	1 19.9 -21.8 18.1 -18.1 2 19.9 -25.5 18.1 -21.8
	2 O'hg -40.6 -40.6
2 2 2 1	3 19.9 -25.5 18.1 -21.8
4 2 5	3 O'ha 69 2 42 4



		10		100	
1	19.9	-21.8	18.1	-18.1	
2	19.9	-25.5	18.1	-21.8	
2 O'hg		-40.6		-40.6	
3	19.9	-25.5	18.1	-21.8	
3 O'hg		-68.3		-42.4	
4	21.8	-23.6	18.5	-20.4	
5	21.8	-29.1	18.5	-22.6	
	& Wind st Cas 5, 10	e	21.8	-29.1	
8x7 Gar	rage Door		19.5	-22.9	
16x7 Garage Door		18.5	-21.0		

2	40 PSF (ALL OTHER DWELLING ROOMS)	
	30 PSE (SLEEPING ROOMS)	_

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY

GARAGE DOOR BUCK INSTALLATION DETAIL

12 PSF (12:12 AND GREATER) STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

705017

OF 2 SHEETS

PE No.5395, POB 868, Lake City, FL 32056, 38-754-5419

Stated dimensions supercede scaled

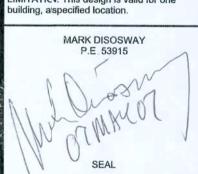
dimension. Refer all questions to

REVISIONS

Mark Disoway, P.E. for resolution. Do not proceed without clarification. COPYRIGITS AND PROPERTY RIGHTS: Mark Disoway, P.E. hereby expressly rese its commo law copyrights and property right in these instriments of service. This document is not to be riproduced, altered or copied in any form or maner without first the express writte permissionand consent of Mark Disosway.

CERTIFIC TION: I hereby certify that I have examined his plan, and that the applicable ortions ofthe plan, relating to wind engine comply wit section R301.2.1, florida building code residntial 2004, to the best of my

LIMITATION: This design is valid for one building, aspecified location.



Istac Construction

Victores Garage

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