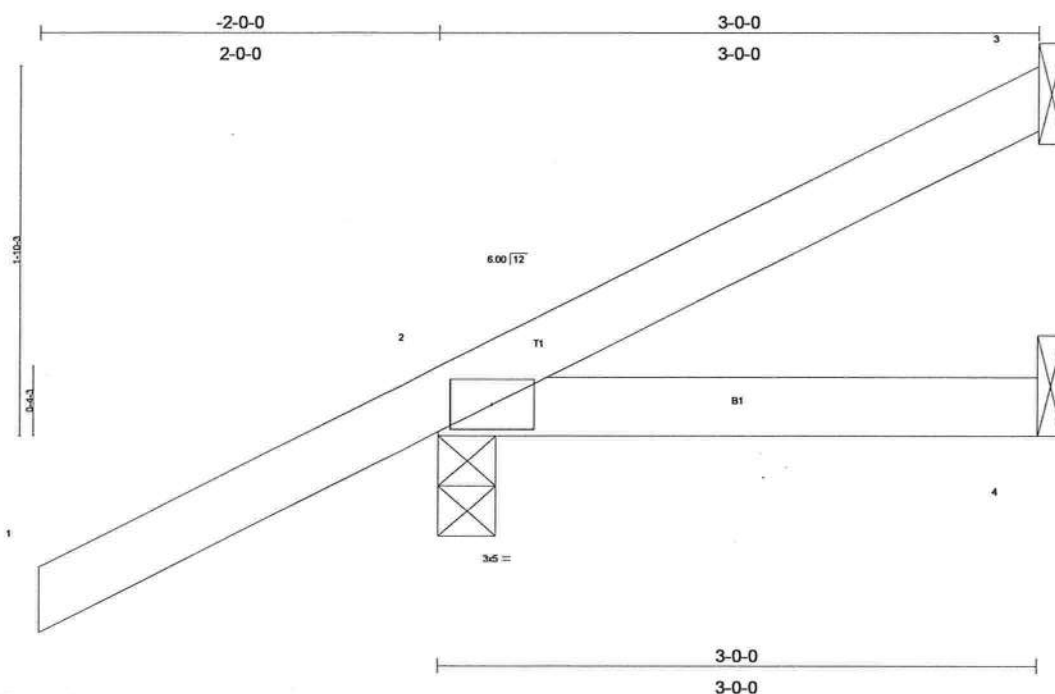


Project Information:				Builder: Aaron Simque Homes, Inc				Builders FirstSource			
				Model: 1448				2525 E. Duval St.			
Builders FirstSource Job #: L282743								Lake City, FL 32055			

Job L282743	Truss CJ3	Truss Type JACK	Qty 4	Ply 1	AARON SIMQUE / LOT 9 SVE
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jul 07 10:34:53 2008 Page 1		



Scale = 1:11.1

LOADING (psf)

TCLL 20.0
TCDL 7.0
BCLL 10.0
BCDL 5.0

SPACING

2-0-0
Plates Increase 1.25
Lumber Increase 1.25
Rep Stress Incr YES
Code FBC2004/TPI2002

CSI

TC 0.29
BC 0.06
WB 0.00
(Matrix)

DEFL

in (loc) l/defl L/d
Vert(LL) -0.00 2-4 >999 360
Vert(TL) -0.01 2-4 >999 240
Horz(TL) -0.00 3 n/a n/a

PLATES

MT20

GRIP 244/190

Weight: 13 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical
Max Horz 2=132(load case 6)
Max Uplift 3=-28(load case 7), 2=-203(load case 6)
Max Grav 3=31(load case 1), 2=250(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3 and 203 lb uplift at joint 2.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L282743	Truss EJ7	Truss Type MONO TRUSS	Qty 25	Ply 1	AARON SIMQUE / LOT 9 SVE
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2008 MiTek Industries, Inc. Mon Jul 07 10:35:37 2008 Page 1		

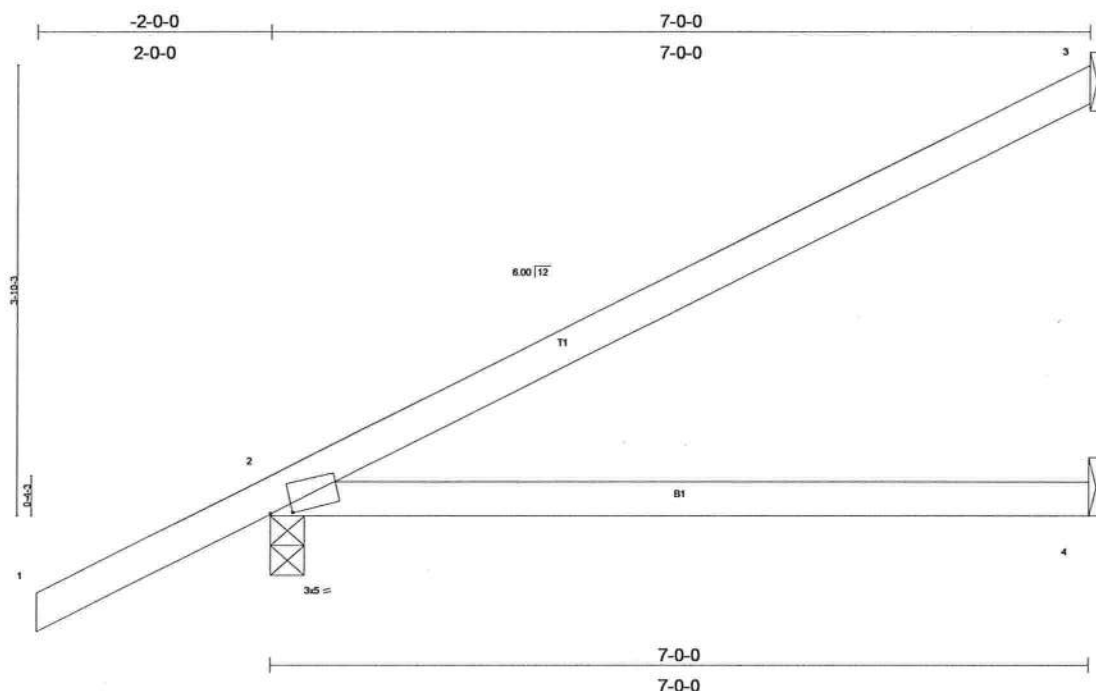


Plate Offsets (X,Y): [2-0-2-5,0-0-7]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.48	Vert(LL)	-0.08	2-4	>999	360	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.28	Vert(TL)	-0.16	2-4	>501	240		
BCLL 10.0 *	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002							Weight: 26 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size)

3=154/Mechanical, 2=352/0-3-8, 4=45/Mechanical
Max Horz 2=161(load case 6)
Max Uplift 3=84(load case 6), 2=-139(load case 6)
Max Grav 3=154(load case 1), 2=352(load case 1), 4=94(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

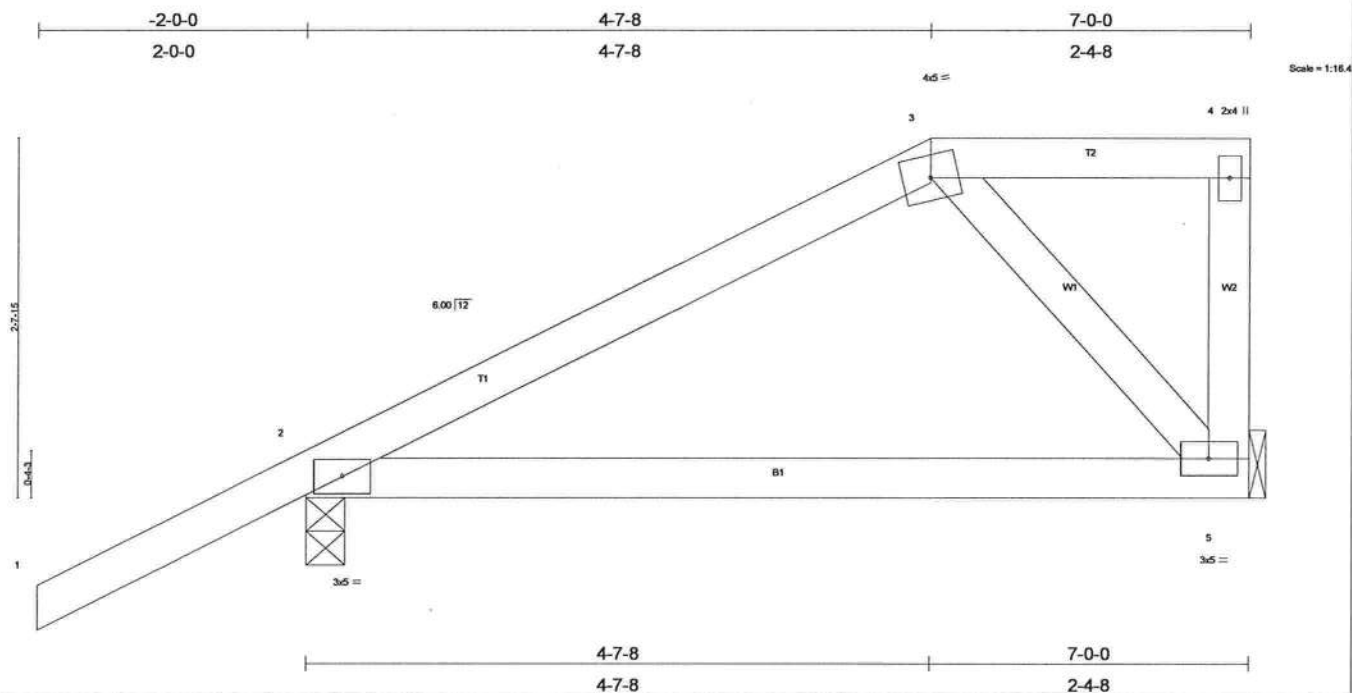
TOP CHORD 1-2=0/47, 2-3=-119/54
BOT CHORD 2-4=0/0

NOTES (5)

- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 3 and 139 lb uplift at joint 2.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L262743	Truss EJ7B	Truss Type MONO HIP	Qty 1	Ply 1	AARON SIMQUE / LOT 9 SVE
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jul 07 10:36:49 2008 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCCL 20.0	2-0-0	TC 0.36	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.14	Vert(LL) -0.04 2-5 >999 360		
BCCL 10.0 *	Lumber Increase 1.25	WB 0.06	Vert(TL) -0.07 2-5 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.00 5 n/a n/a		
	Code FBC2004/TPI2002			Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=196/Mechanical, 2=349/0-3-8
Max Horz 2=125(load case 6)
Max Uplift 5=52(load case 5), 2=150(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=215/95, 3-4=50/0, 4-5=34/31
BOT CHORD 2-5=143/138
WEBS 3-5=178/230

NOTES (6)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 5 and 150 lb uplift at joint 2.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job # L282743	Truss T01	Truss Type COMMON	Qty 1	Ply 1	AARON SIMQUE / LOT 9 SVE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional)		

6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jul 07 10:38:25 2008 Page 1

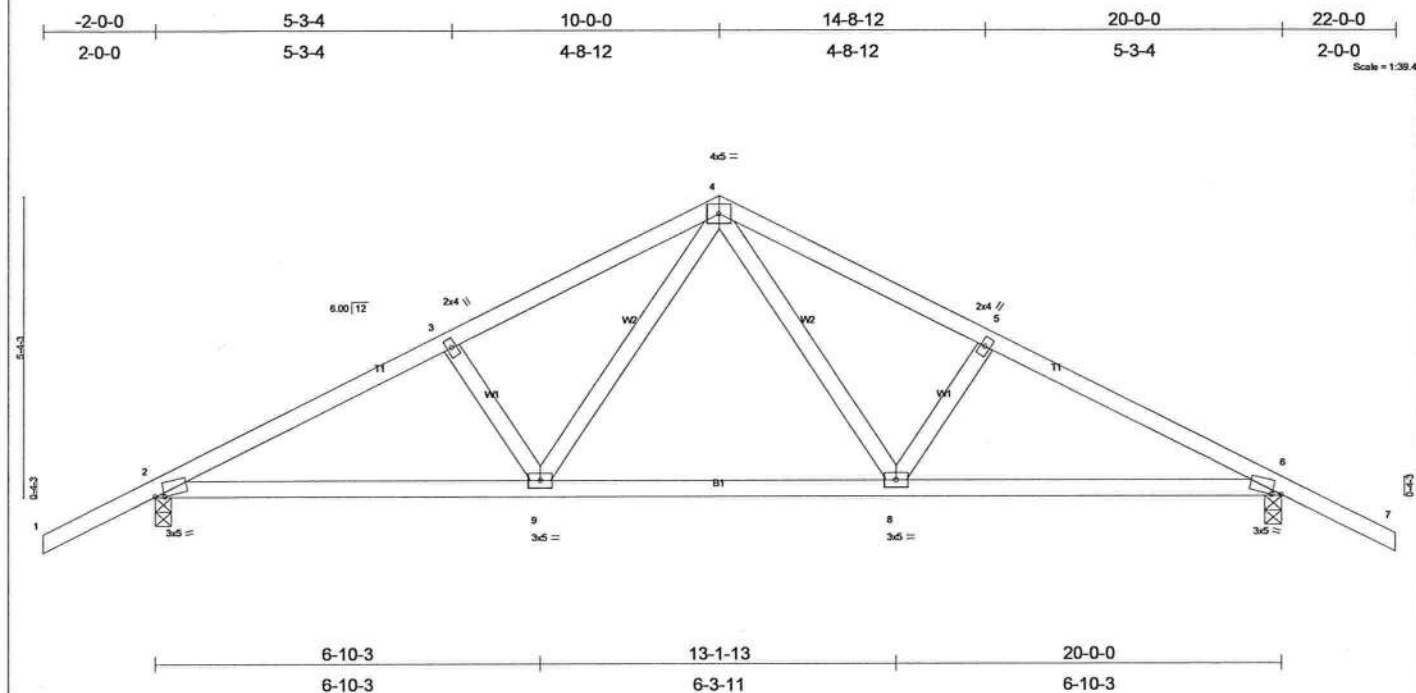


Plate Offsets (X,Y): [2-0-2-1,0-0-7], [6-0-2-1,0-0-7]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.35	Vert(LL)	0.18	8-9	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.56	Vert(TL)	-0.26	8-9	>899	240		
BCLL 10.0	Rep Stress Incr NO	WB 0.18	Horz(TL)	0.04	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)							
								Weight: 95 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-4-10 oc bracing.

REACTIONS (lb/size) 2=936/0-3-8, 6=936/0-3-8
Max Horz 2=95(load case 7)
Max Uplift 2=286(load case 6), 6=286(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1502/801, 3-4=-1358/801, 4-5=-1358/801, 5-6=-1502/801, 6-7=0/47
BOT CHORD 2-9=-546/1271, 8-9=-285/887, 6-8=-546/1271
WEBS 3-9=-213/202, 4-9=-288/540, 4-8=-288/540, 5-8=-213/202

NOTES (6)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2 and 286 lb uplift at joint 6.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert 1-4=-54, 4-7=-54, 2-9=-10, 8-9=-70(F=60), 6-8=-10

Job # L282743	Truss T01G	Truss Type GABLE	Qty 1	Ply 1	AARON SIMQUE / LOT 9 SVE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jul 07 10:39:22 2008 Page 1		

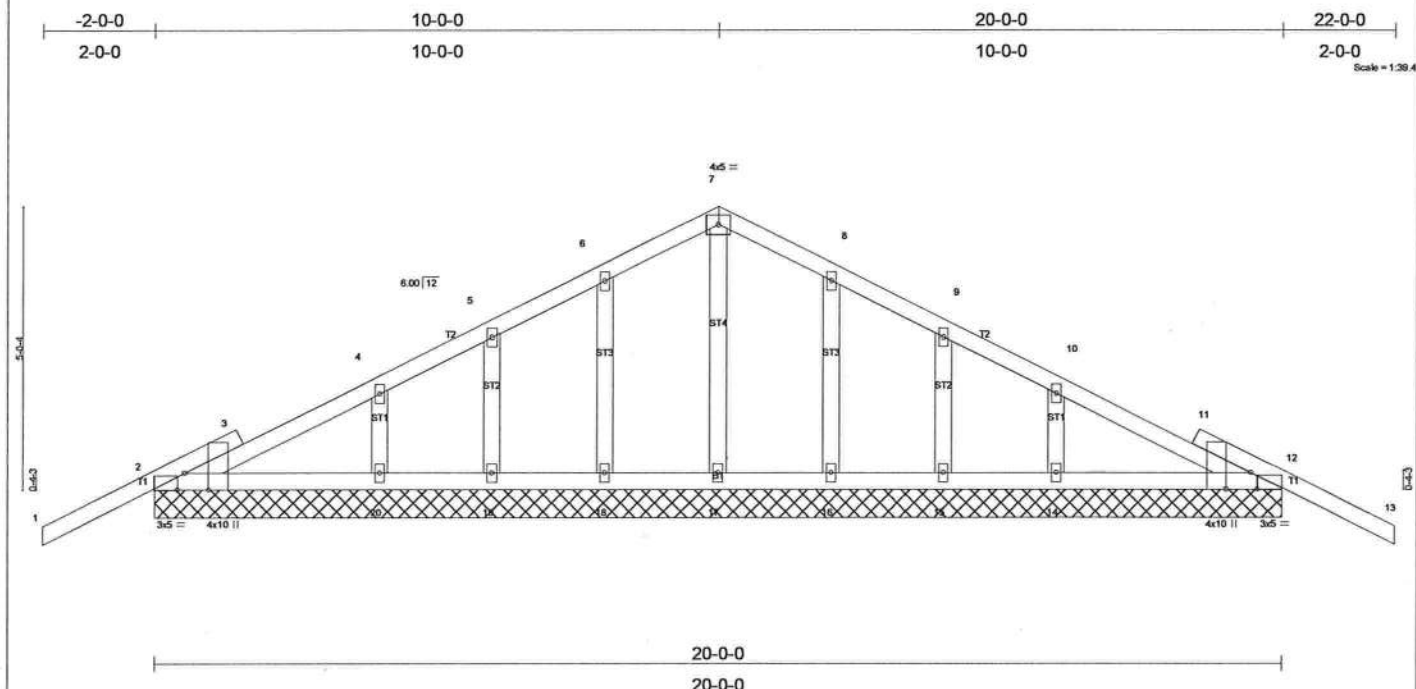


Plate Offsets (X,Y): [2-0-3-8,Edge], [2-0-1-8,Edge], [12-0-3-8,Edge], [12-0-1-8,Edge]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)
TCCL 20.0	Plates Increase	1.25	TC 0.26	Vert(LL)	-0.02 13 n/r 120
TCDL 7.0	Lumber Increase	1.25	BC 0.06	Vert(TL)	-0.02 13 n/r 90
BCLL 10.0	Rep Stress Incr	YES	WB 0.05	Horz(TL)	0.00 12 n/a n/a
BCDL 5.0	Code FBC2004/TP12002		(Matrix)		
			Weight: 102 lb		

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
OTHERS 2 X 4 SYP No.3	

REACTIONS (lb/size) 2=240/20-0-0, 12=240/20-0-0, 17=143/20-0-0, 18=136/20-0-0, 19=100/20-0-0, 20=200/20-0-0, 16=136/20-0-0, 15=100/20-0-0, 14=200/20-0-0
Max Horz 2=104(load case 6)
Max Uplift 2=181(load case 6), 12=197(load case 7), 18=83(load case 6), 19=90(load case 6), 20=87(load case 6), 16=82(load case 7), 15=89(load case 7), 14=92(load case 7)
Max Grav 2=242(load case 10), 12=242(load case 11), 17=143(load case 1), 18=140(load case 10), 19=100(load case 1), 20=201(load case 10), 16=140(load case 11), 15=100(load case 1), 14=201(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-74/35, 3-4=-59/64, 4-5=-15/88, 5-6=-6/137, 6-7=-10/191, 7-8=-10/191, 8-9=-6/137, 9-10=-15/81, 10-11=-22/64, 11-12=-33/18, 12-13=0/47
BOT CHORD 2-20=-20/127, 19-20=-20/127, 18-19=-20/127, 17-18=-20/127, 16-17=-20/127, 15-16=-20/127, 14-15=-20/127, 12-14=-20/127
WEBS 7-17=-124/0, 6-18=-118/104, 5-19=-88/108, 4-20=-163/128, 8-16=-118/104, 9-15=-88/108, 10-14=-163/128

NOTES (9)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS: gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 2, 197 lb uplift at joint 12, 83 lb uplift at joint 18, 90 lb uplift at joint 19, 87 lb uplift at joint 20, 82 lb uplift at joint 16, 89 lb uplift at joint 15 and 92 lb uplift at joint 14.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job #	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 9 SVE
L282743	T02	COMMON	1	2	Job Reference (optional)
Builders FirstSource, Lake City, FI 32055			6.300 s Apr 19 2006 Mitek Industries, Inc. Mon Jul 07 11:13:53 2008 Page 2		

LOAD CASE(S) Standard

- Concentrated Loads (lb)
Vert: 10=48(F) 11=132(F)
- 4) MWFRS Wind Right: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=8, 2-4=15, 4-6=2, 2-10=6, 10-11=15(F=21), 6-11=6
Horz: 1-2=-16, 2-4=-23, 4-6=10
Concentrated Loads (lb)
Vert: 10=48(F) 11=132(F)
- 5) MWFRS 1st Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=42, 2-4=24, 4-6=12, 2-10=6, 10-11=15(F=21), 6-11=6
Horz: 1-2=-51, 2-4=-32, 4-6=20
Concentrated Loads (lb)
Vert: 10=48(F) 11=268(F)
- 6) MWFRS 2nd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=5, 2-4=12, 4-6=24, 2-10=6, 10-11=15(F=21), 6-11=6
Horz: 1-2=-14, 2-4=-20, 4-6=32
Concentrated Loads (lb)
Vert: 10=48(F) 11=268(F)
- 7) MWFRS 3rd Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=42, 2-4=24, 4-6=12, 2-10=6, 10-11=15(F=21), 6-11=6
Horz: 1-2=-51, 2-4=-32, 4-6=20
Concentrated Loads (lb)
Vert: 10=48(F) 11=268(F)
- 8) MWFRS 4th Wind Parallel: Lumber Increase=1.60, Plate Increase=1.60
Uniform Loads (plf)
Vert: 1-2=5, 2-4=12, 4-6=24, 2-10=6, 10-11=15(F=21), 6-11=6
Horz: 1-2=-14, 2-4=-20, 4-6=32
Concentrated Loads (lb)
Vert: 10=48(F) 11=268(F)
- 9) 1st unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-14, 2-10=-10, 10-11=-86(F=-76), 6-11=-10
Concentrated Loads (lb)
Vert: 10=48(F) 11=-968(F)
- 10) 2nd unbalanced Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-14, 4-6=-54, 2-10=-10, 10-11=-86(F=-76), 6-11=-10
Concentrated Loads (lb)
Vert: 10=48(F) 11=-968(F)

Job #	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 9 SVE
L282743	T04	MONO HIP	1	1	L282743013
Builders FirstSource, Lake City, FL 32055			Job Reference (optional)		
			6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jul 07 12:37:35 2008 Page 1		



Scale = 1/56.2

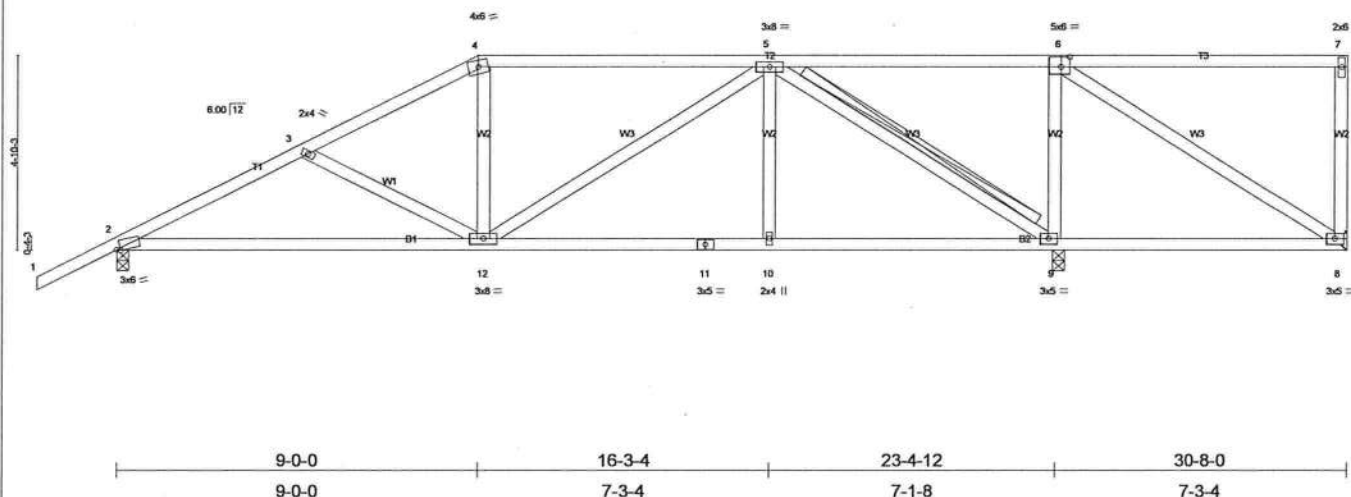


Plate Offsets (X,Y): [2-0-1-1,0-0-7], [6-0-2-8,0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.46	Vert(LL)	-0.13	2-12	>999	360	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.42	Vert(TL)	-0.24	2-12	>999	240		
BCLL 10.0	Lumber Increase 1.25	WB 0.45	Horz(TL)	0.03	9	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 165 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-9-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 8=15/Mechanical, 2=791/0-3-8, 9=1284/0-3-8
Max Horz 2=195(load case 6)
Max Uplift 8=27(load case 4), 2=223(load case 6), 9=338(load case 5)
Max Grav 8=17(load case 6), 2=791(load case 1), 9=1284(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1146/551, 3-4=-916/454, 4-5=-785/463, 5-6=-174/317, 6-7=-37/18, 7-8=-169/120
BOT CHORD 2-12=-638/963, 11-12=-337/595, 10-11=-337/595, 9-10=-337/595, 8-9=-298/161
WEBS 3-12=-203/195, 4-12=0/198, 5-12=-150/224, 5-10=0/195, 5-9=-1084/606, 6-9=-625/411, 6-8=-214/399

NOTES (6-7)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 8, 223 lb uplift at joint 2 and 338 lb uplift at joint 9.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
7) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job #	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 9 SVE
L282743	T06	HIP	1	1	L282743015
Builders FirstSource, Lake City, FL 32055					Job Reference (optional)
					6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jul 07 13:10:25 2008 Page 1

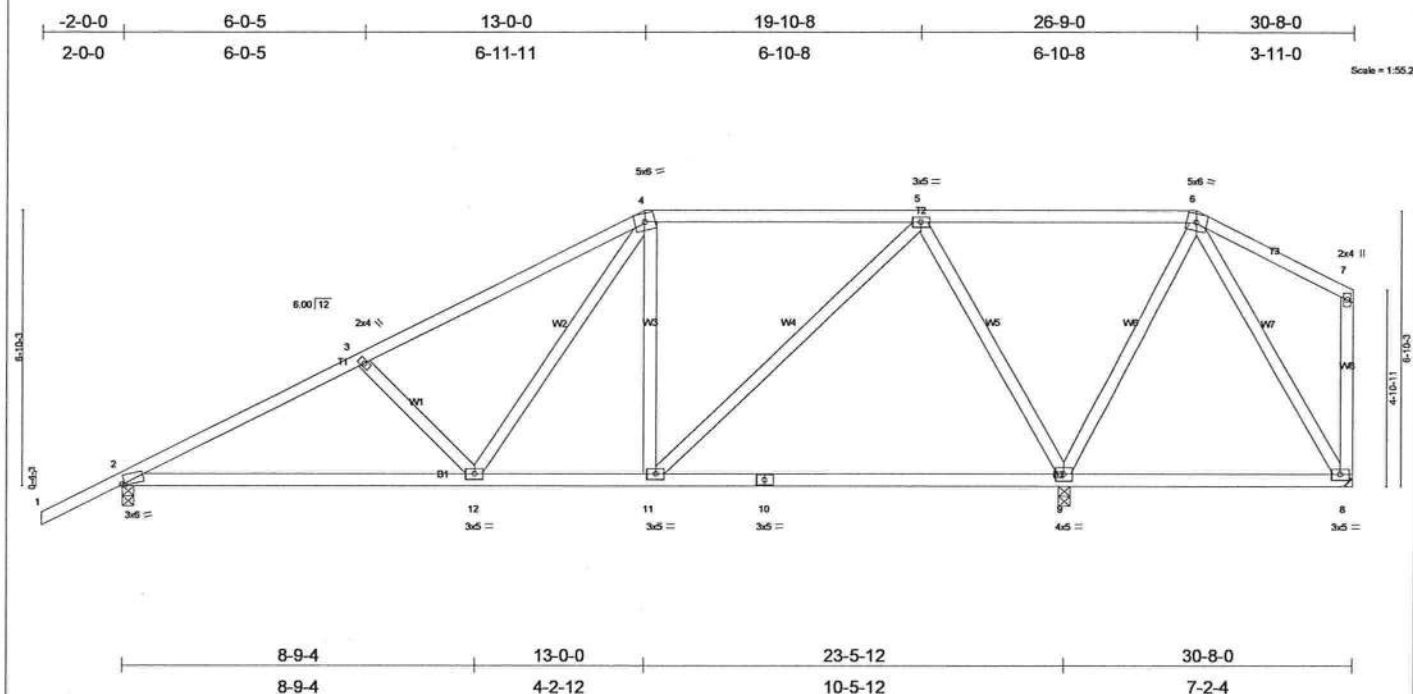


Plate Offsets (X,Y): [2-0-0-13,Edge]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)
TCCL 20.0	Plates Increase	1.25	TC 0.36	Vert(LL)	-0.21 9-11 >999 360
TCCL 7.0	Lumber Increase	1.25	BC 0.48	Vert(TL)	-0.37 9-11 >759 240
BCCL 10.0 *	Rep Stress Incr	YES	WB 0.91	Horz(TL)	0.02 9 n/a n/a
BCCL 5.0	Code FBC2004/TPI2002		(Matrix)		
			Weight: 176 lb		

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 5-10-0 oc purlins, except end verticals.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	2 X 4 SYP No.3		

REACTIONS (lb/size) 2=768/0-3-8, 9=1399/0-3-8, 8=107/Mechanical
Max Horz 2=220(load case 6)
Max Uplift 2=232(load case 6), 9=316(load case 5), 8=218(load case 10)
Max Grav 2=768(load case 1), 9=1399(load case 1), 8=64(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1086/533, 3-4=-850/467, 4-5=-489/371, 5-6=-145/365, 6-7=-34/61, 7-8=-65/72
BOT CHORD 2-12=623/908, 11-12=-280/496, 10-11=-50/118, 9-10=-50/118, 8-9=-132/70
WEBS 3-12=325/322, 4-12=-222/349, 4-11=-330/278, 5-11=-339/584, 5-9=-904/566, 6-9=-572/384, 6-8=-142/256

- NOTES** (7-8)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 232 lb uplift at joint 2, 316 lb uplift at joint 9 and 218 lb uplift at joint 8.
 - 7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
 - 8) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job L282743	Truss T08	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE / LOT 9 SVE L282743017 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2008 MITek Industries, Inc. Mon Jul 07 13:10:53 2008 Page 1		

-2-0-0	6-0-5	11-6-3	17-0-0	22-9-0	30-8-0
2-0-0	6-0-5	5-5-13	5-5-13	5-9-0	7-11-0

Scale = 1:55.2

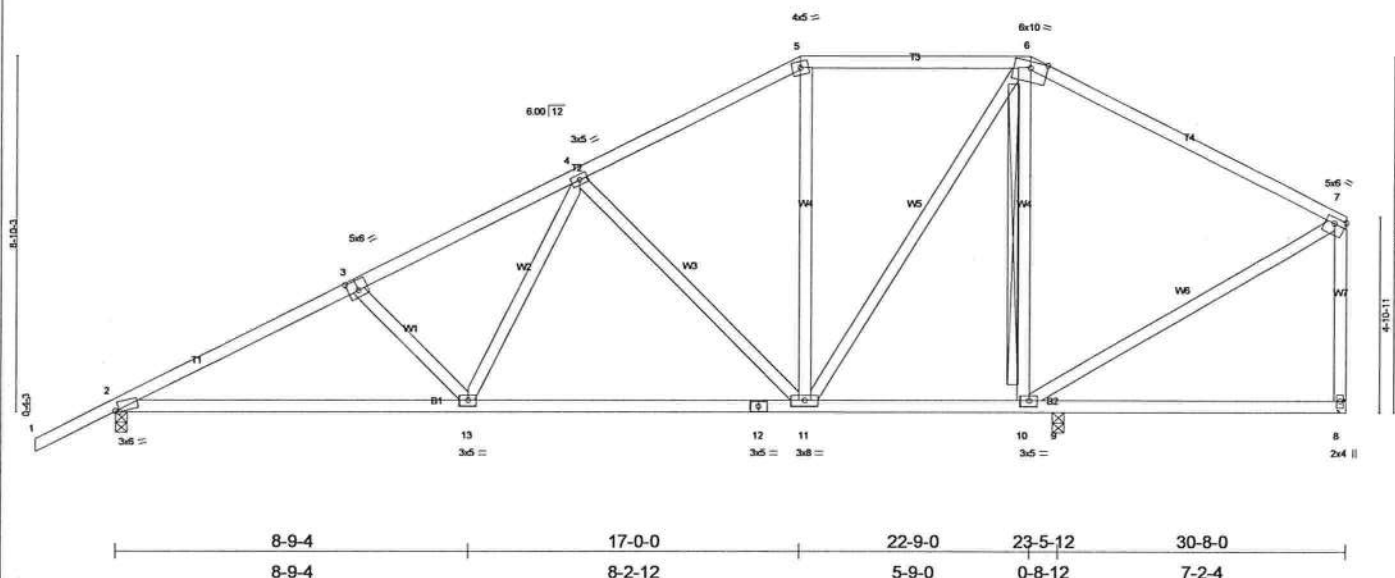


Plate Offsets (X,Y): [2-0-1-1,0-0-7], [3-0-3-0,0-3-0], [7-Edge,0-1-12]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.82	in (loc) l/defl l/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.42	Vert(LL) -0.11 2-13 >999 360		
BCLL 10.0 *	Lumber Increase 1.25	WB 0.56	Vert(TL) -0.22 2-13 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.04 8 n/a n/a		
	Code FBC2004/TPI2002			Weight: 185 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-9-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-7-11 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 6-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1009/0-3-8, 8=691/Mechanical, 9=360/0-3-8
Max Horz 2=244(load case 6)
Max Uplift2=300(load case 6), 8=-171(load case 7), 9=-26(load case 6)
Max Grav 2=1009(load case 1), 8=691(load case 1), 9=407(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1596/858, 3-4=-1377/807, 4-5=-848/597, 5-6=-698/590, 6-7=-603/426, 7-8=-673/479
BOT CHORD 2-13=-906/1355, 12-13=-667/1037, 11-12=-667/1037, 10-11=-266/449, 9-10=-41/48, 8-9=-41/48
WEBS 3-13=-273/262, 4-13=-142/371, 4-11=-487/396, 5-11=-71/133, 6-11=-230/528, 6-10=-549/318, 7-10=-267/480

NOTES (7-8)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone;
Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Refer to girder(s) for truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 300 lb uplift at joint 2, 171 lb uplift at joint 8 and 26 lb uplift at joint 9.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
8) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job L282743	Truss T10	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE / LOT 9 SVE L282743019 Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jul 07 13:11:20 2008 Page 1

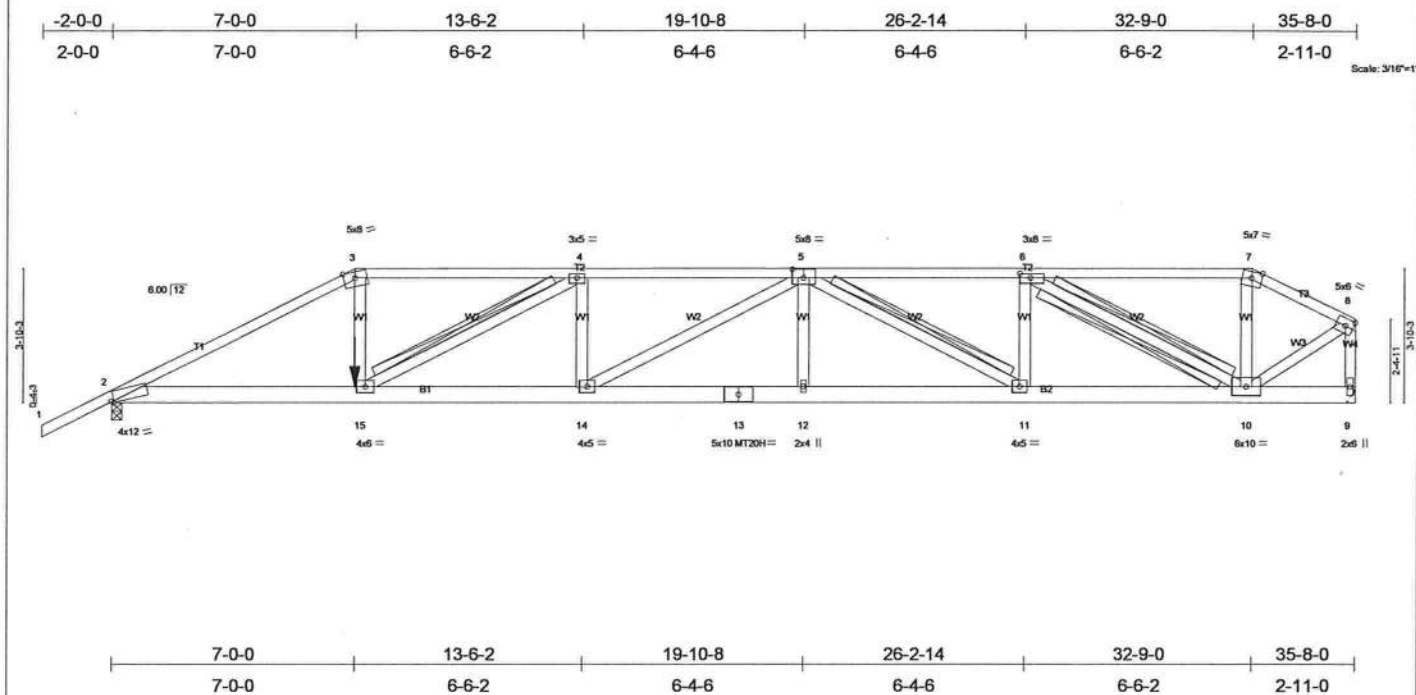


Plate Offsets (X,Y): [2-0-0-13,Edge], [5-0-4-0,0-3-0], [6-0-3-8,0-1-8]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.83	in (loc) l/defl l/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.76	Vert(LL) -0.41 12-14 >999 360	MT20H	187/143
BCLL 10.0	Lumber Increase 1.25	WB 0.95	Vert(TL) -0.78 12-14 >542 240		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.18 9 n/a n/a		
	Code FBC2004/TPI2002			Weight: 213 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2 *Except*	TOP CHORD Structural wood sheathing directly applied or 2-2-2 oc purlins, except end verticals.
T2 2 X 4 SYP No.1D, T2 2 X 4 SYP No.1D	Rigid ceiling directly applied or 5-8-4 oc bracing.
BOT CHORD 2 X 6 SYP No.1D	I-Brace: 2 X 4 SYP No.3 - 6-10
WEBS 2 X 4 SYP No.3	T-Brace: 2 X 4 SYP No.3 - 4-15, 5-11
	Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. with 4in minimum end distance.
	Brace must cover 90% of web length.

REACTIONS (lb/size) 2=2446/0-3-8, 9=2500/Mechanical
Max Horz 2=136(load case 5)
Max Uplift 2=751(load case 4), 9=799(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-4800/1583, 3-4=-4285/1467, 4-5=-6184/2127, 5-6=-5042/1723, 6-7=-2035/694, 7-8=-2237/732, 8-9=-2392/767
BOT CHORD 2-15=-1433/4219, 14-15=-2112/6184, 13-14=-2187/6423, 12-13=-2187/6423, 11-12=-2187/6423, 10-11=-1708/5042, 9-10=-3/14
WEBS 3-15=-465/1581, 4-15=-2255/807, 4-14=0/332, 5-14=-312/109, 5-12=0/271, 5-11=-1582/546, 6-11=-161/863, 6-10=-3438/1191, 7-10=-81/486, 8-10=-819/2454

- NOTES** (12-13)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf, BCDL=3.0psf, Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) All plates are MT20 plates unless otherwise indicated.
 - 6) Refer to girder(s) for truss to truss connections.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 751 lb uplift at joint 2 and 799 lb uplift at joint 9.
 - 8) Girder carries tie-in span(s): 7-0-0 from 32-9-0 to 35-8-0
 - 9) Girder carries hip end with 2-11-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
 - 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 411 lb down and 165 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
 - 13) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-7=-118(F=64), 7-8=-54, 2-15=-10, 10-15=-22(F=-12), 9-10=-85(F=-75)

Concentrated Loads (lb)

Vert: 15=-411(F)

Job L282743	Truss T12	Truss Type HIP	Qty 1	Ply 1	AARON SIMQUE / LOT 9 SVE Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MITek Industries, Inc. Mon Jul 07 11:17:48 2008 Page 1

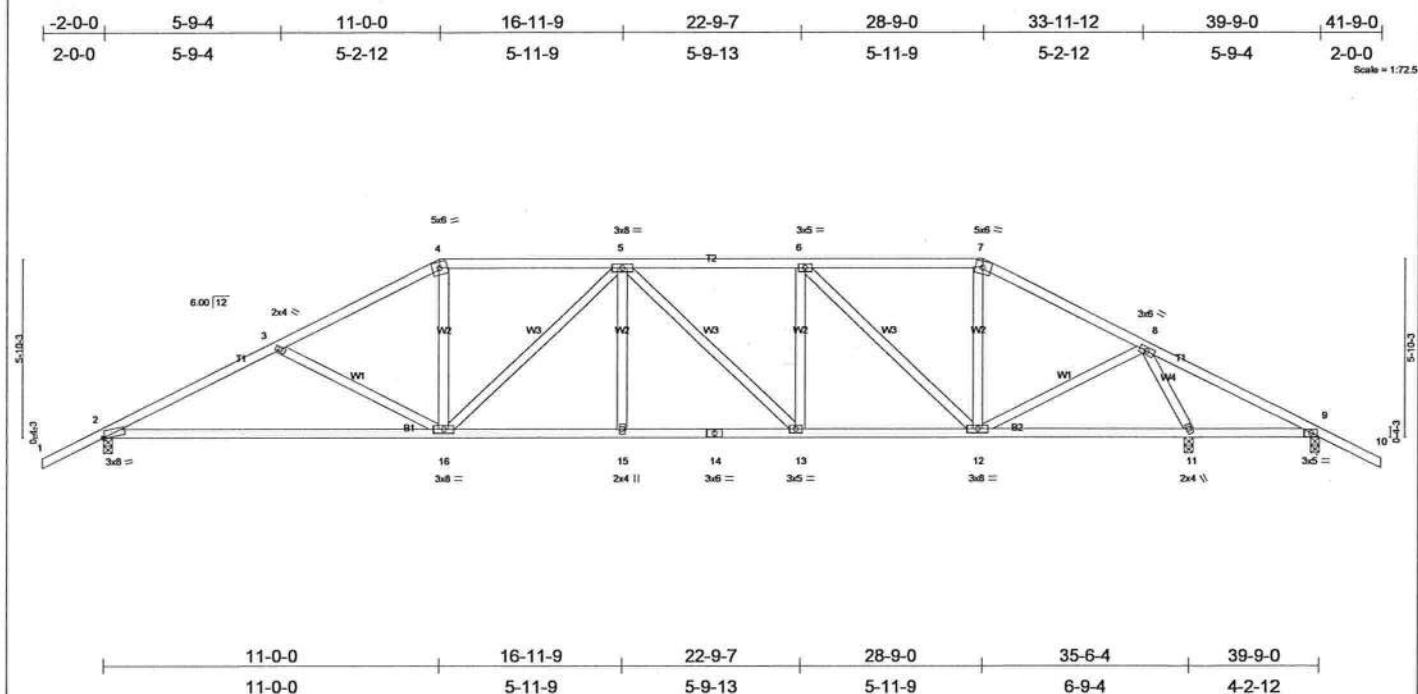


Plate Offsets (X,Y): [2-0-0,10,Edge]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.37	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.68	Vert(LL) -0.31 2-16 >999 360		
BCLL 10.0 *	Lumber Increase 1.25	WB 0.94	Vert(TL) -0.59 2-16 >724 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.08 11 n/a n/a		
	Code FBC2004/TPI2002			Weight: 217 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=1206/0-3-8, 9=153/0-3-8, 11=1705/0-3-8
Max Horz 2=101(load case 6)
Max Uplift 2=296(load case 6), 9=312(load case 10), 11=353(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=2013/1086, 3-4=1704/941, 4-5=-1478/908, 5-6=-1498/933, 6-7=-923/641, 7-8=-1092/650, 8-9=-426/974, 9-10=0/47
BOT CHORD 2-16=-794/1730, 15-16=-664/1675, 14-15=-664/1675, 13-14=-664/1675, 12-13=-573/1498, 11-12=-81/74, 9-11=-804/519
WEBS 3-16=-297/282, 4-16=-176/447, 5-16=-390/162, 5-15=0/127, 5-13=-274/127, 6-13=-53/294, 6-12=-843/404, 7-12=-56/234, 8-12=-385/999, 8-11=-1828/999

NOTES (6)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 296 lb uplift at joint 2, 312 lb uplift at joint 9 and 353 lb uplift at joint 11.
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L282743	Truss T14	Truss Type SPECIAL	Qty 1	Ply 1	AARON SIMQUE / LOT 9 SVE
Builders FirstSource, Lake City, FL 32055			Job Reference (optional)		

6.300 s Apr 19 2006 MITek Industries, Inc. Mon Jul 07 11:39:55 2008 Page 1

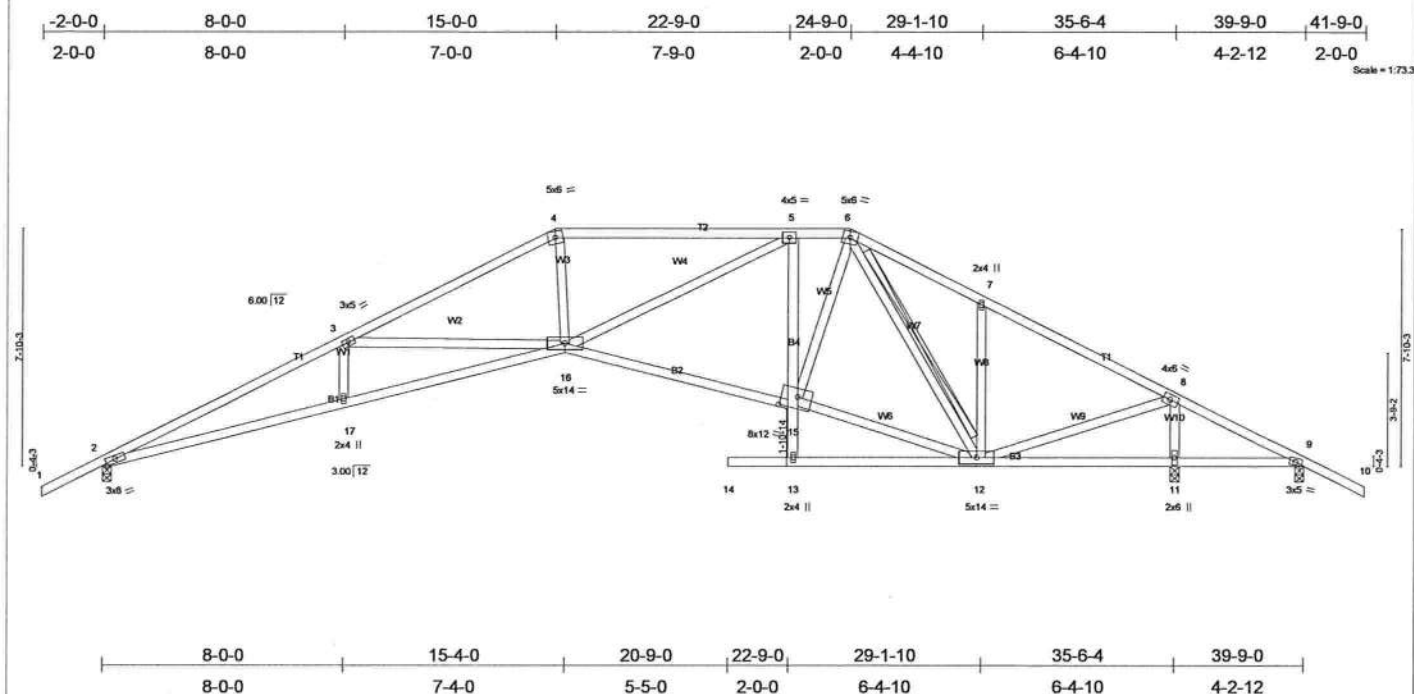


Plate Offsets (X,Y): [2:0-3-12,Edge], [15:0-6-10,0-4-11]					
LOADING (psf)	SPACING	2:0-0	CSI	DEFL	PLATES
TCLL 20.0	Plates Increase	1.25	TC 0.70	in (loc) l/defl L/d	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.86	Vert(LL) 0.35 16-17 >999 360	GRIP
BCLL 10.0	Rep Stress Incr	YES	WB 0.78	Vert(TL) -0.62 16-17 >680 240	244/190
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)	Horz(TL) 0.35 11 n/a n/a	
Weight: 226 lb					

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 2-7-0 oc purtins.
BOT CHORD 2 X 4 SYP No.2 *Except*	Rigid ceiling directly applied or 5-3-14 oc bracing.
B4 2 X 4 SYP No.3	T-Brace: 2 X 4 SYP No.3 - 6-12
WEBS 2 X 4 SYP No.3	Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.
	, with 4in minimum end distance.
	Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1171/0-3-8, 9=491/0-3-8, 11=2099/0-3-8
Max Horz 2=124(load case 7)
Max Uplift 2=307(load case 6), 9=598(load case 10), 11=340(load case 6)
Max Grav 2=1171(load case 1), 9=48(load case 6), 11=2099(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=3530/1703, 3-4=2595/1195, 4-5=2310/1164, 5-6=1346/813, 6-7=819/683, 7-8=839/526, 8-9=656/1579, 9-10=0/47
BOT CHORD 2-17=1363/3155, 16-17=1364/3152, 15-16=378/1421, 13-15=0/156, 5-15=804/385, 13-14=0/0, 12-13=25/29, 11-12=1341/698,
9-11=1341/698
WEBS 3-17=0/236, 3-16=813/636, 4-16=223/718, 5-16=378/1049, 12-15=227/1069, 6-15=345/948, 6-12=791/125, 7-12=315/322,
8-12=936/2130, 8-11=2032/1067

- NOTES** (7)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 307 lb uplift at joint 2, 598 lb uplift at joint 9 and 340 lb uplift at joint 11.
 - 7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L282743	Truss T16	Truss Type SPECIAL	Qty 1	Ply 1	AARON SIMQUE / LOT 9 SVE
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jul 07 11:19:40 2008 Page 1		

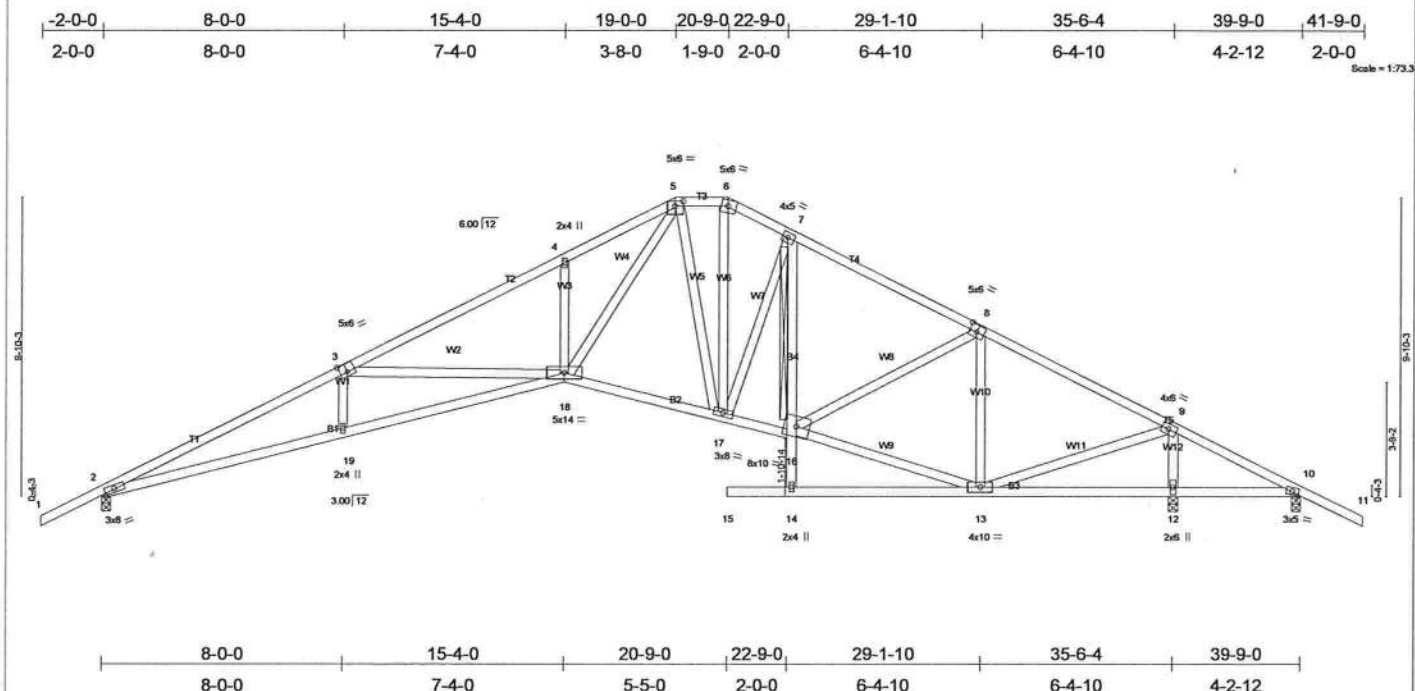


Plate Offsets (X,Y): [2-0-3-12,Edge], [3-0-3-0,0-3-4], [5-0-3-0,0-2-0], [8-0-3-0,0-3-0]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	PLATES
TCLL 20.0	Plates Increase	1.25	TC 0.70	in (loc) l/defl L/d	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.72	Vert(LL) 0.37 18-19 >999 360	GRIP 244/190
BCLL 10.0 *	Rep Stress Incr	YES	WB 0.84	Vert(TL) -0.64 18-19 >665 240	
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)	Horz(TL) 0.34 12 n/a n/a	Weight: 246 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 2-7-2 oc purlins.
BOT CHORD 2 X 4 SYP No.2 *Except*	BOT CHORD Rigid ceiling directly applied or 5-3-3 oc bracing. Except:
B4 2 X 4 SYP No.3	T-Brace: 2 X 4 SYP No.3 - 7-16
WEBS 2 X 4 SYP No.3	JOINTS 1 Brace at Jt(s): 16

REACTIONS (lb/size) 2=1171/0-3-8, 10=492/0-3-8, 12=2100/0-3-8
Max Horz 2=148(load case 7)
Max Uplift 2=323(load case 6), 10=516(load case 10), 12=400(load case 6)
Max Grav 2=1171(load case 1), 10=77(load case 6), 12=2100(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=3544/1756, 3-4=2560/1226, 4-5=2533/1392, 5-6=1144/766, 6-7=1307/839, 7-8=1367/795, 8-9=830/536, 9-10=690/1584, 10-11=0/47
BOT CHORD 2-19=1412/3169, 18-19=1414/3162, 17-18=250/1296, 16-17=265/1171, 14-16=0/160, 7-16=206/66, 14-15=0/0, 13-14=7/30, 12-13=1346/729, 10-12=1346/729
WEBS 3-19=0/247, 3-18=876/665, 4-18=332/338, 5-18=861/1783, 5-17=503/178, 6-17=318/475, 7-17=174/177, 13-16=211/693, 8-16=66/531, 8-13=846/430, 9-13=981/2131, 9-12=2032/1098

- NOTES** (7)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCCL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 323 lb uplift at joint 2, 516 lb uplift at joint 10 and 400 lb uplift at joint 12.
 - 7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jul 07 11:24:49 2008 Page 1



Weight: 41 lb

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

(lb/size) 4=320/0-3-8, 2=460/0-3-8
Max Horz 2=81(load case 6)
Max Uplift 4=-71(load case 7), 2=-174(load case 6)

TOP CHORD 1-2=0/47, 2-3=-467/278, 3-4=-462/270
BOT CHORD 2-5=-156/359, 4-5=-156/359
WEBS 3-5=0/178

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone;
Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 71 lb uplift at joint 4 and 174 lb uplift at joint 2.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	AARON SIMQUE / LOT 9 SVE
L282743	T18G	GABLE	1	1	Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Jul 07 11:25:14 2008 Page 1		

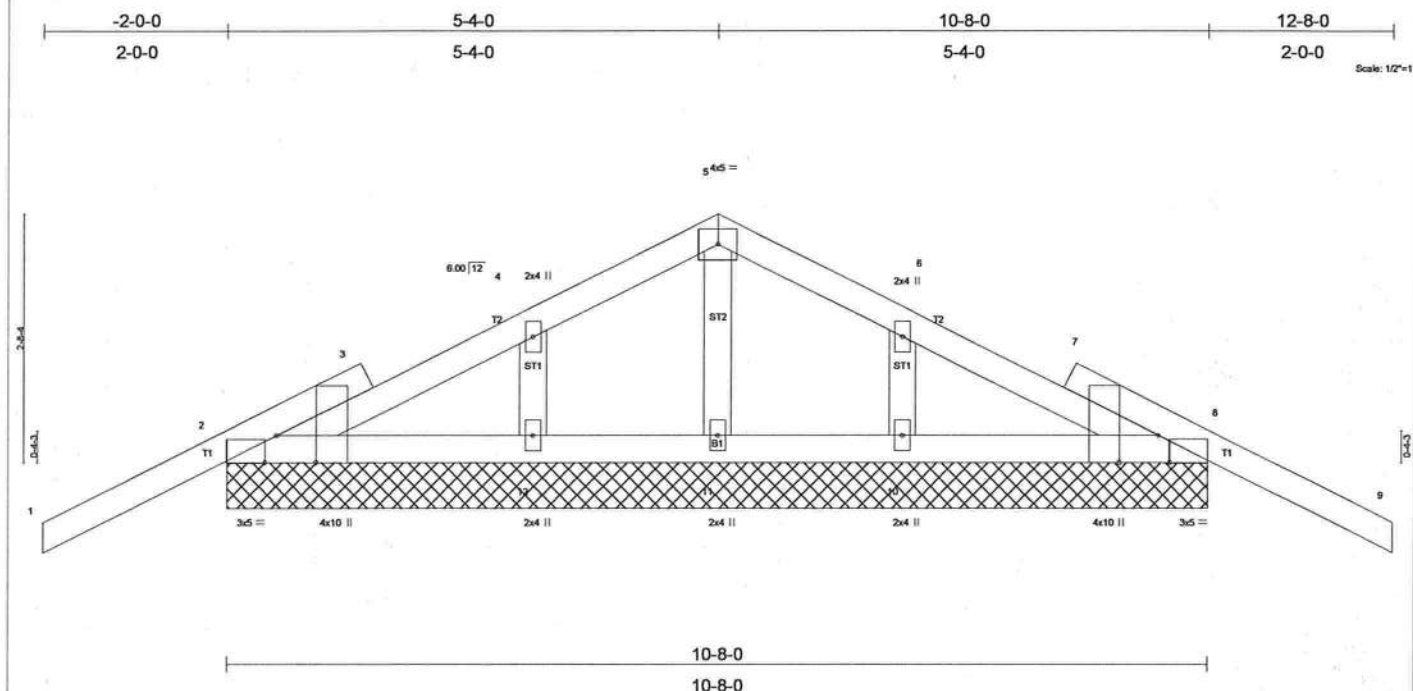


Plate Offsets (X,Y): [2-0-3-8,Edge], [2-0-1-8,Edge], [8-0-3-8,Edge], [8-0-1-8,Edge]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.26	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.04	Vert(LL) -0.02 9 n/r 120		
BCLL 10.0	Lumber Increase 1.25	WB 0.03	Vert(TL) -0.03 9 n/r 90		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 8 n/a n/a		
	Code FBC2004/TP12002			Weight: 50 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
OTHERS 2 X 4 SYP No.3	

REACTIONS (lb/size) 2=234/10-8-0, 8=234/10-8-0, 11=119/10-8-0, 12=156/10-8-0, 10=156/10-8-0
Max Horz 2=71(load case 6)
Max Uplift 2=204(load case 6), 8=214(load case 7), 11=9(load case 6), 12=62(load case 6), 10=66(load case 7)
Max Grav 2=235(load case 10), 8=235(load case 11), 11=119(load case 1), 12=159(load case 10), 10=159(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-27/45, 3-4=-2/61, 4-5=-21/124, 5-6=-21/124, 6-7=0/61, 7-8=-27/45, 8-9=0/47
BOT CHORD 2-12=-12/79, 11-12=-12/79, 10-11=-12/79, 8-10=-12/79
WEBS 5-11=103/13, 4-12=-133/124, 6-10=-133/124

NOTES (8)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 204 lb uplift at joint 2, 214 lb uplift at joint 8, 9 lb uplift at joint 11, 62 lb uplift at joint 12 and 66 lb uplift at joint 10.
8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

MAX GABLE VERTICAL LENGTH														
2X4 GABLE VERTICAL SPACING	BRACE SPECIES	NO BRACES	BRACE *											
			(1) 1X4 "L" BRACE *	(1) 2X4 "L" BRACE *	(2) 2X4 "L" BRACE **	(1) 2X6 "L" BRACE *	(2) 2X8 "L" BRACE *	(2) 2X8 "L" BRACE *						
24" O.C.	SPF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"	
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"	
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"	
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 3"	8' 3"	11' 8"	11' 8"	
	SP	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"	
		#2	3' 7"	6' 10"	6' 3"	6' 11"	7' 6"	8' 3"	8' 11"	10' 10"	11' 8"	12' 11"	13' 11"	
		#3	3' 6"	5' 0"	6' 0"	6' 8"	6' 8"	8' 3"	8' 3"	10' 4"	10' 4"	12' 11"	13' 7"	
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 6"	10' 3"	10' 3"	12' 11"	13' 7"	
	DFL	#1 / #2	3' 4"	6' 8"	6' 10"	6' 11"	7' 11"	8' 1"	9' 6"	9' 5"	12' 6"	12' 9"	14' 0"	
		STANDARD	3' 4"	6' 8"	6' 8"	7' 11"	8' 1"	9' 6"	9' 5"	12' 6"	12' 9"	14' 0"	14' 0"	
		#3	3' 9"	8' 0"	8' 0"	9' 11"	10' 11"	11' 8"	11' 8"	14' 0"	14' 0"	16' 0"	16' 0"	
		STUD	3' 9"	8' 0"	8' 0"	9' 11"	10' 11"	11' 8"	11' 8"	14' 0"	14' 0"	16' 0"	16' 0"	
16" O.C.	SPF	#1	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	
		#2	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	
		#3	4' 0"	8' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 11"	12' 6"	13' 8"	14' 0"	14' 0"	
		STUD	4' 0"	8' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	13' 8"	14' 0"	14' 0"	
	DFL	STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	8' 11"	10' 6"	10' 8"	13' 8"	14' 0"	14' 0"	
		#1 / #2	4' 3"	7' 4"	7' 4"	8' 9"	8' 9"	10' 6"	10' 8"	13' 8"	14' 0"	14' 0"	14' 0"	
		#3	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	14' 0"	14' 0"	14' 0"	
		STUD	4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	14' 0"	14' 0"	14' 0"	
	12" O.C.	SPF	#1	4' 8"	7' 4"	7' 11"	8' 9"	8' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
			#2	4' 7"	7' 4"	7' 11"	8' 9"	8' 5"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"	14' 0"
			#3	4' 4"	7' 2"	6' 9"	8' 9"	8' 2"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
			STUD	4' 4"	7' 1"	6' 8"	8' 2"	7' 8"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"	14' 0"
DFL		STANDARD	4' 3"	6' 1"	6' 1"	8' 0"	8' 0"	10' 5"	10' 8"	12' 6"	12' 6"	14' 0"	14' 0"	

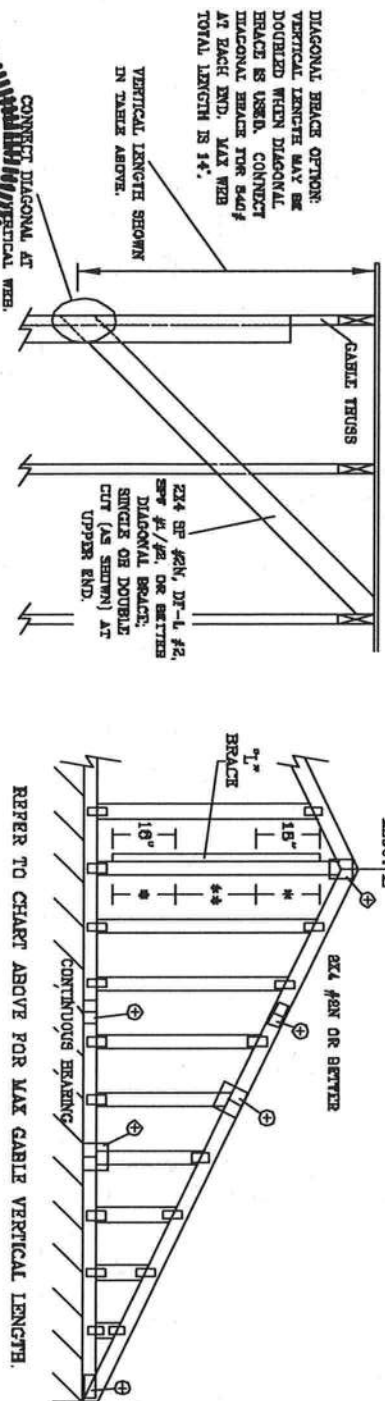
BRAICING GROUP SPECIES AND GRADBS:	
GROUP A:	
SPRICE-PINE-YR	HICK-PFR
#1 / #2	#1
STANDARD	STUD
#3	#3
STUD	STANDARD
DOUGLAS FIR-LARCH	
#2	#2
STUD	STUD
STANDARD	STANDARD
GROUP B:	
HICK-PFR	
#1 & BTR	#1
DOUGLAS FIR-LARCH	
#1	#1
#2	#2

PROVIDE UPLIFT CONNECTIONS FOR 136 PLT OVER CONTINUOUS BEARING (6 PSF MC DEAD LOAD).
CABLE END SUPPORTS LOAD FROM 4" O" OUTLIMBERS WITH 8" O" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH T¹ BRACE WITH 104 NUTS.
 * FOR (1) T¹ BRACE, SPACE NUTS AT 2" O.C.
 IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
 * FOR (2) T¹ BRACES: SPACE NUTS AT 3" O.C.
 IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.
 T¹ BRACING MUST BE A MINIMUM OF 60% OF WEB
 MEMBER LENGTH.

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SLIT
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2, 3X4

+ REFER TO COMMON TRUSS DESIGN FOR
FEAS, SPLICE, AND BEEL PLATES.

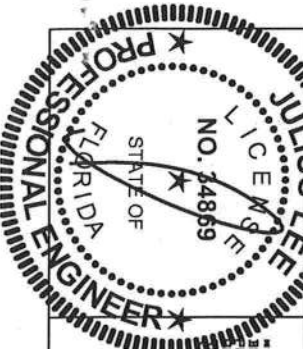


WARDROBE, TRIM, RE-LOCATE EXTREME CARE IN FABRICATING, HANGING, SHIPPING, INSTALLING AND UNLOADING. REFER TO B251 1-042 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE CONSTRUCTION PRODUCTS MANUFACTURERS ASSOCIATION, 393 DOWNEY RD., SUITE 200, WEST VALLEY, N.Y. 11575 AND VITA (WOOD TRUSS CONSULTING FIRM), 6500 ENTERPRISE DR., WILMINGTON, VI 50719 FOR SAFETY PRACTICES PRIOR TO PERFORMING ANY FABRICATING, UNLOADING, SHIPPING, OR INSTALLATION. UNLESS OTHERWISE INDICATED, TOP CODED SHALL HAVE PERPENDICULAR ATTACHED RIGID PANELS AND BOTTOM CODED SHALL HAVE A PARALLEL ATTACHED RIGID CEILING.

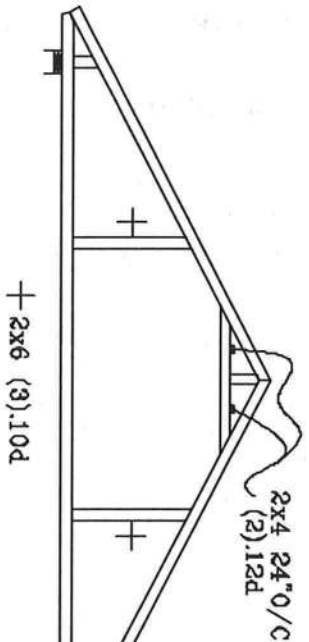
JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

REF	ASCE7-02-CABI3015
DATE	11/26/03
DRWG	WEEK STD CABLE IS E HT
-ENG	

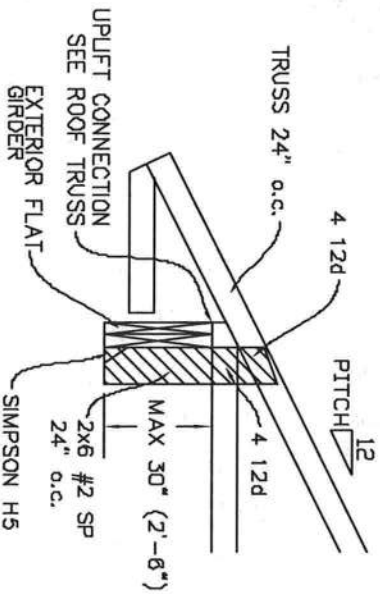
REVIEWED
By Julius Lee at 12:00 pm, Jun 11, 2008



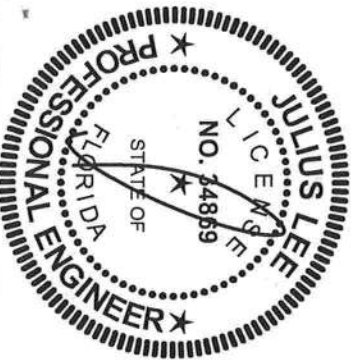
TYPICAL ATTIC TRUSS BRACING



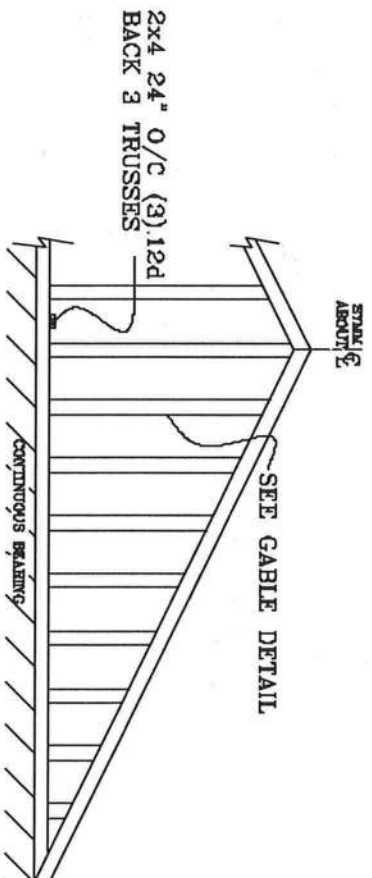
TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

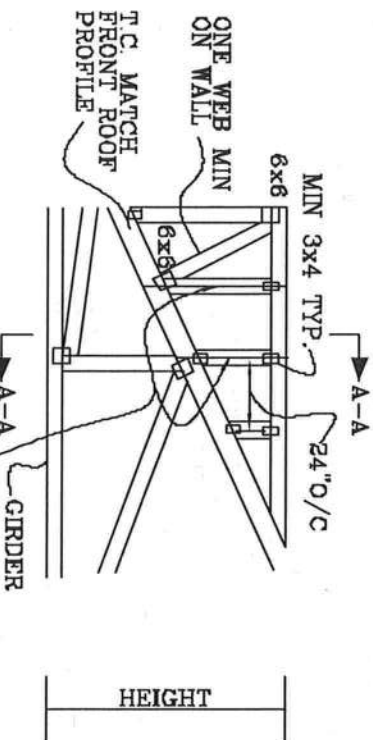


GABLE END TRUSS DETAIL



MINIMUM BC BRACING ON GABLE TRUSS OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

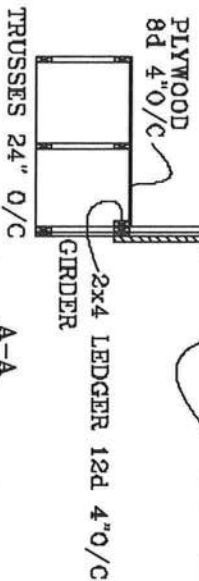
TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



SEE ROOF TRUSSES FOR UPLIFT

ROOF 24" O/C

SEE GABLE END DETAIL FOR T-BRACE BEHIND EACH VERTICAL



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CONS. ENGINEERS P.A.
105 SW 4th AVENUE
DEER BEACH, FL 33444-2161

No. 34869
STATE OF FLORIDA

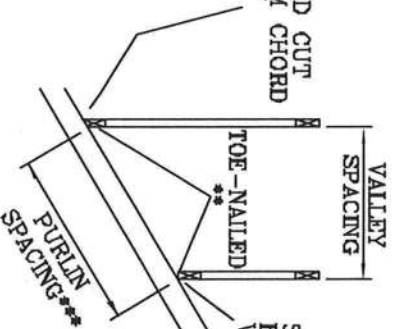
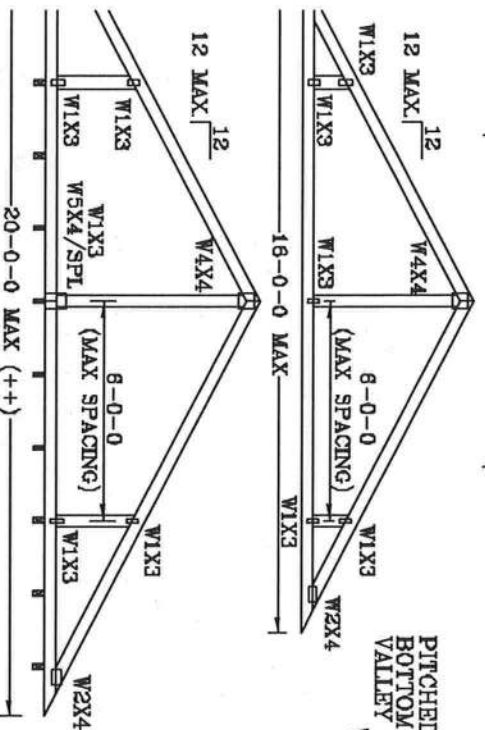
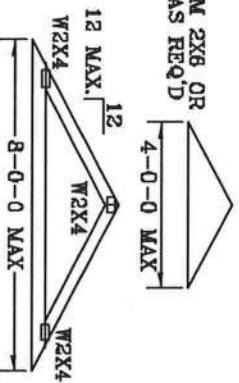
VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:

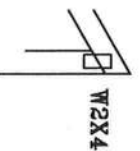
(2) 18d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR
ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED
BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=6 PSF.

CUT FROM 2X6 OR
LARGER AS REQ'D

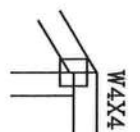


SQUARE CUT
BOTTOM CHORD
VALLEY

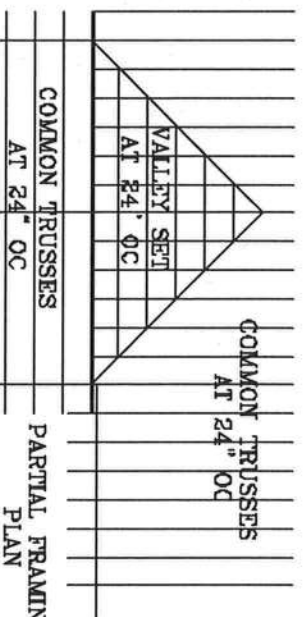
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS
BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES
NOT EXCEED 12'-0".
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



OPTIONAL STUB
END DETAIL



COMMON TRUSSES
AT 24" OC



COMMON TRUSSES
AT 24" OC

PARTIAL FRAMING
PLAN

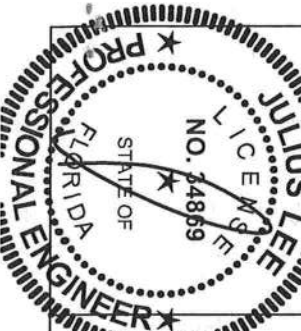
UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80%
LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED
WITH 8d BOX (0.113" X 2.6") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING,
EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'-9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'-0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS
INSTALLATION

OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON
ENGINEERS' SEALED DESIGN.

VARIOUS TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND
MAINTAINING. THE ENGINEER IS NOT RESPONSIBLE FOR DAMAGE TO TRUSSES OR OTHER STRUCTURAL
ELEMENTS DURING TRANSPORTATION OR INSTALLATION. THE ENGINEER IS NOT RESPONSIBLE FOR
DAMAGE TO TRUSSES OR OTHER STRUCTURAL ELEMENTS DURING TRANSPORTATION OR INSTALLATION.
THE ENGINEER IS NOT RESPONSIBLE FOR DAMAGE TO TRUSSES OR OTHER STRUCTURAL ELEMENTS
DURING TRANSPORTATION OR INSTALLATION. THE ENGINEER IS NOT RESPONSIBLE FOR DAMAGE
TO TRUSSES OR OTHER STRUCTURAL ELEMENTS DURING TRANSPORTATION OR INSTALLATION.



REVIEWED

By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.

1655 SW 4th Avenue
DeBary, FL 32713-5101

No. 34869
STATE OF FLORIDA

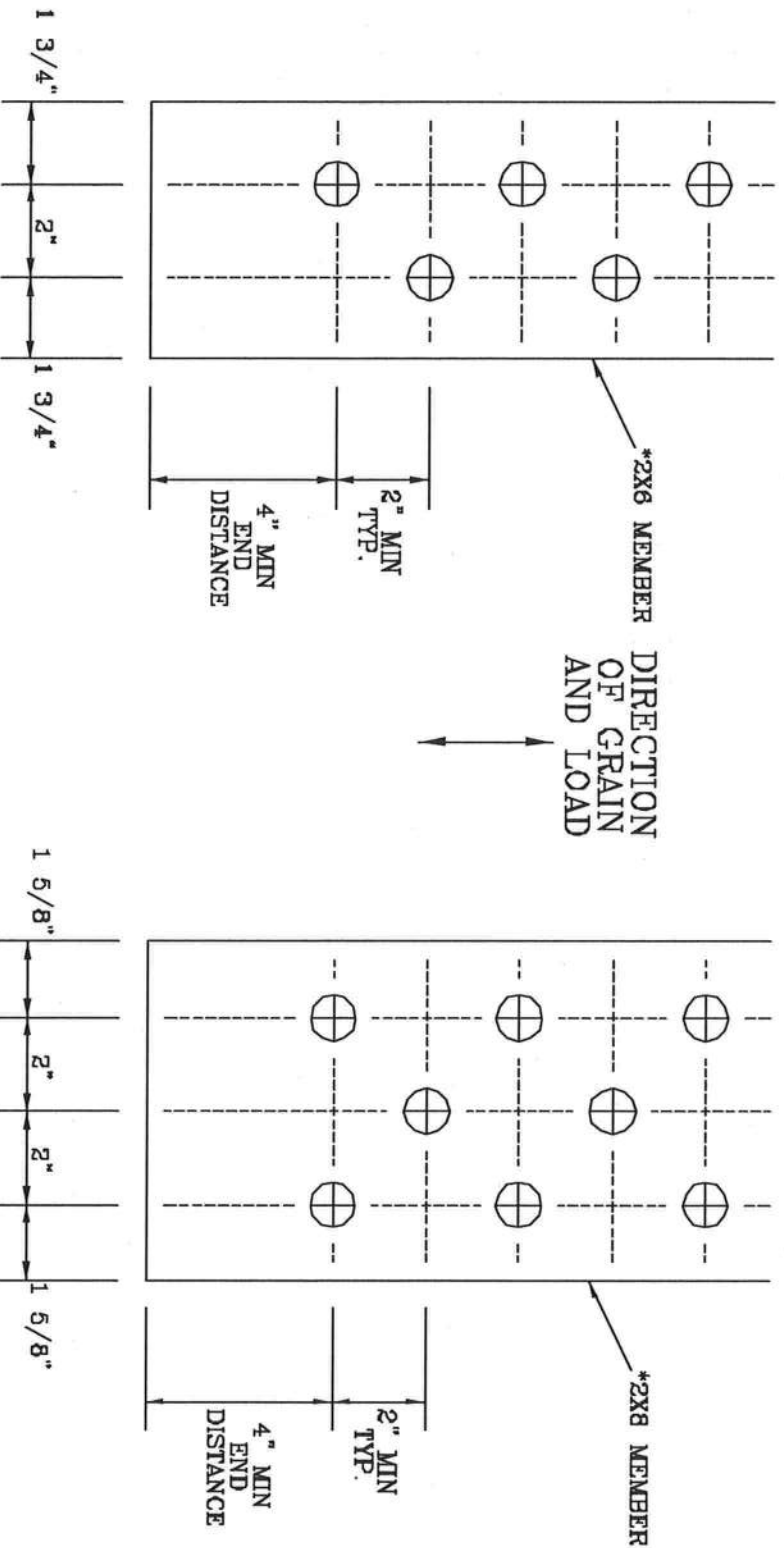
THIS DRAWING REPLACES DRAWING A105

TC LL	20	20	PSF	REF	VALLEY DETAIL
TC DL	7	15	PSF	DATE	11/26/03
BC DL	5	5	PSF	DRWG	VALTRUSS1103
BC LL	0	0	PSF	-ENG	JL
TOT. LD.	32	40	PSF		
DURFAC	1.25	1.25			
SPACING		24"			

1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

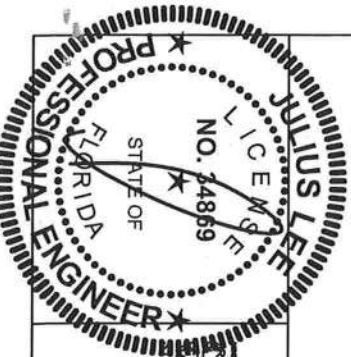
TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL

2X8 DETAIL

THIS DRAWING REPLACES DRAWING A026.016



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND ERECTION. REFER TO POST-1-80 BUILDING DEPARTMENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 3801 E. HIGHWAY 1, SUITE 200, WILMINGTON, VA 22797 AND APCA CODED TRUSS COUNCIL, 1000 17th AVE. N.E., ALBUQUERQUE, NM 87102. THESE PRACTICES MUST BE FOLLOWED PRIOR TO PERFORMING STRUCTURAL PANELS AND SECTION CHORDS SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

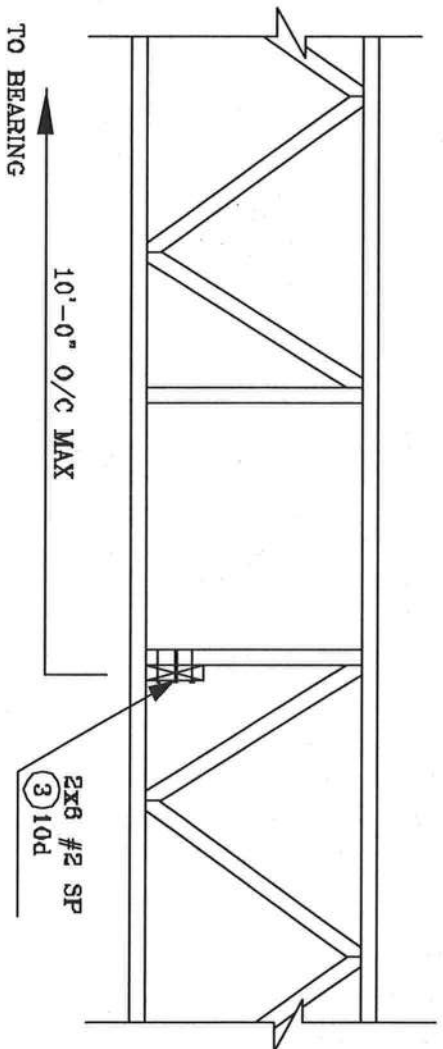
REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 17th AVE. N.E.
ALBUQUERQUE, NM 87102-2161

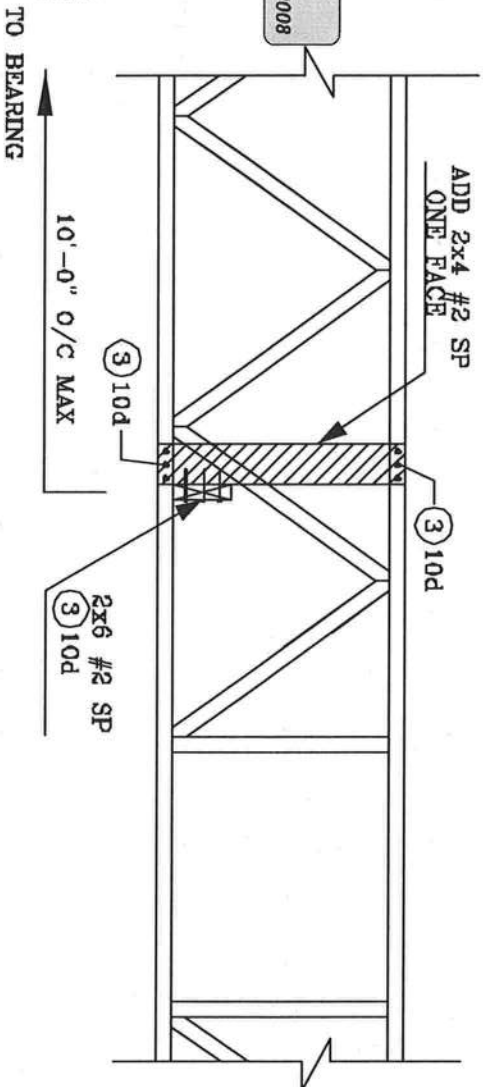
No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTS1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

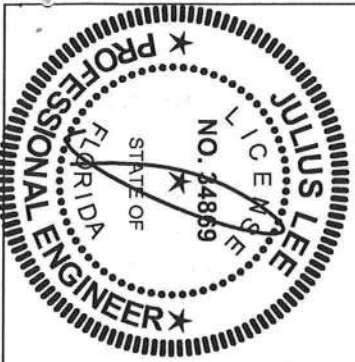
STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008



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CONS. ENGINEERS P.A.
1426 SW 4th AVENUE
DEER BEACH, FL 33444-2161

No. 34869
STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
		3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	835	835	740		
	12	2,225	1,670	1,670	1,485		
	18	3,335	2,505	2,505	2,225		
	24	4,450	3,335	3,335	2,965		
SDS Screws 1/4" x 3 1/2" or WS35 1/4" x 6" or WS6(1)	4	1,915	1,435(4)	1,435	1,275	1,860(2)	1,405(2)
	6	2,870	2,150 (4)	2,150	1,915	2,785(2)	2,110(2)
	8	3,825	2,870 (4)	2,870	2,550	3,715(2)	2,810(2)
	4	2,545	1,910 (4)	1,910	1,695	1,925(2)	1,775(2)
3 3/4" or 5" TrussLok™	6	3,815	2,860 (4)	2,860	2,545	2,890(3)	2,665(2)
	8	5,090	3,815 (4)	3,815	3,390	3,855(2)	3,550(2)

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

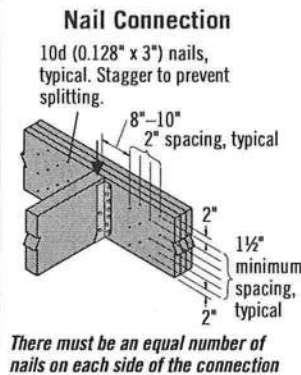
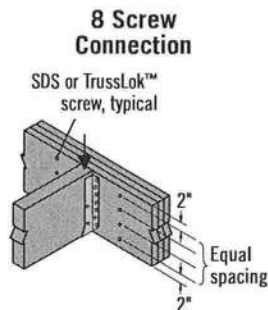
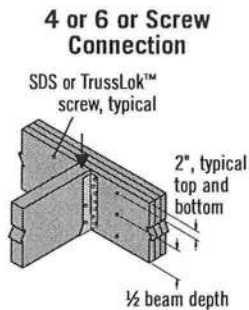
See General Notes on page 38

(2) 6" long screws required.

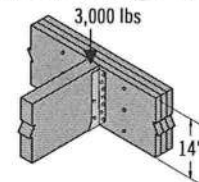
(3) 5" long screws required.

(4) 3 1/2" and 3 3/4" long screws must be installed on both sides.

Connections



Point Load Design Example



First, verify that a 3-ply 1 3/4" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1 3/4" assembly, eight 3 3/4" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1 3/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d-16d (0.148"-0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3 3/4" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed

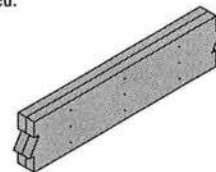
on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3 1/2" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of 1/2" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

L6