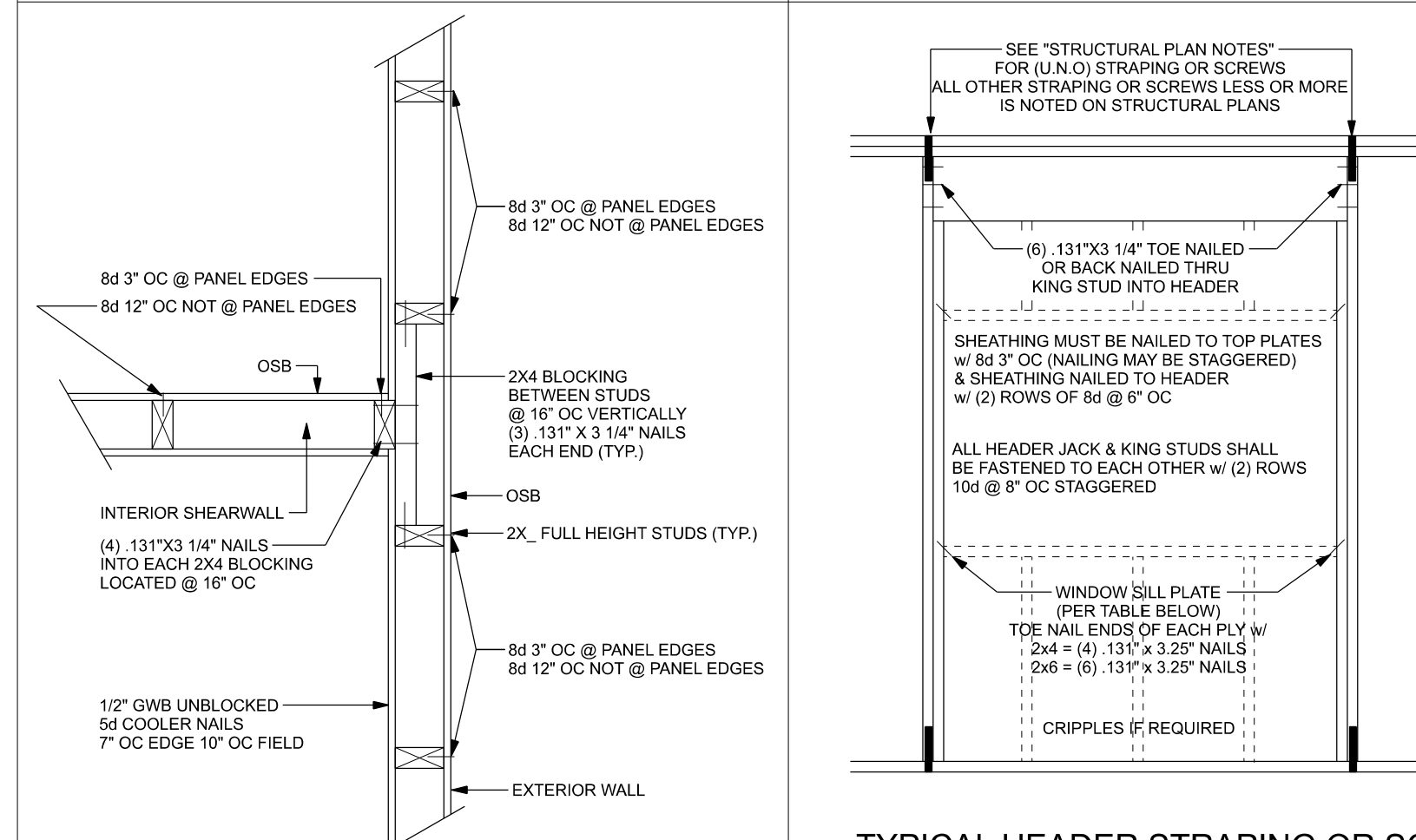
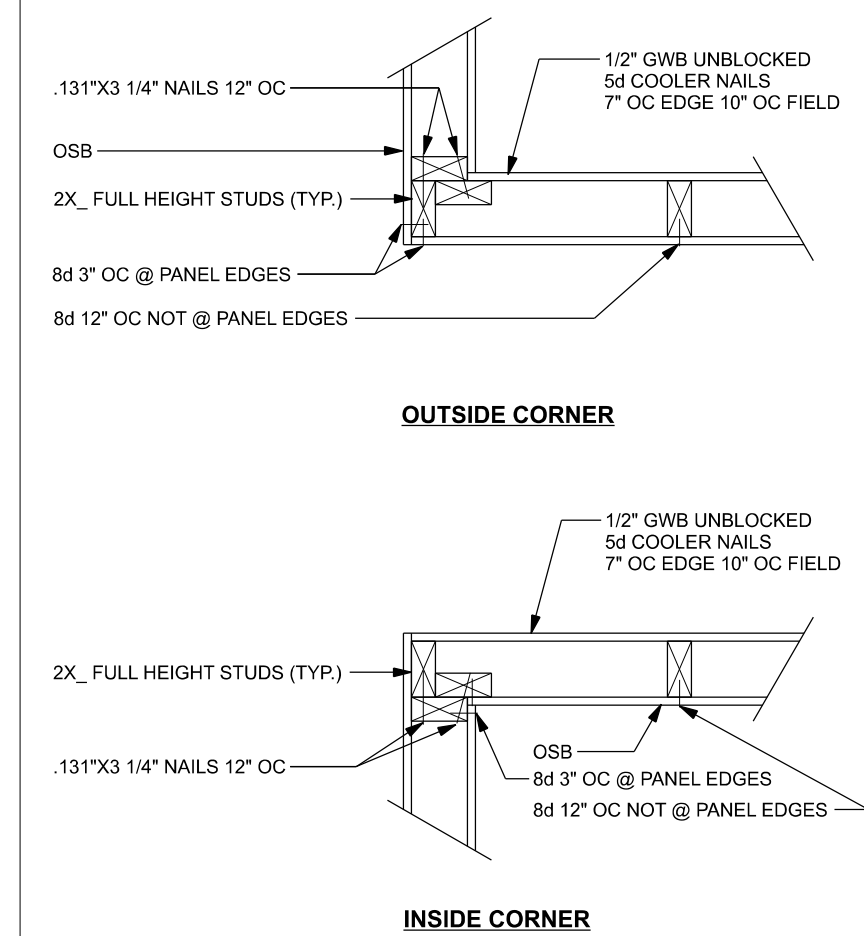


ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

TWO STORY WALL SECTION
SCALE: 3/4" = 1'-0"



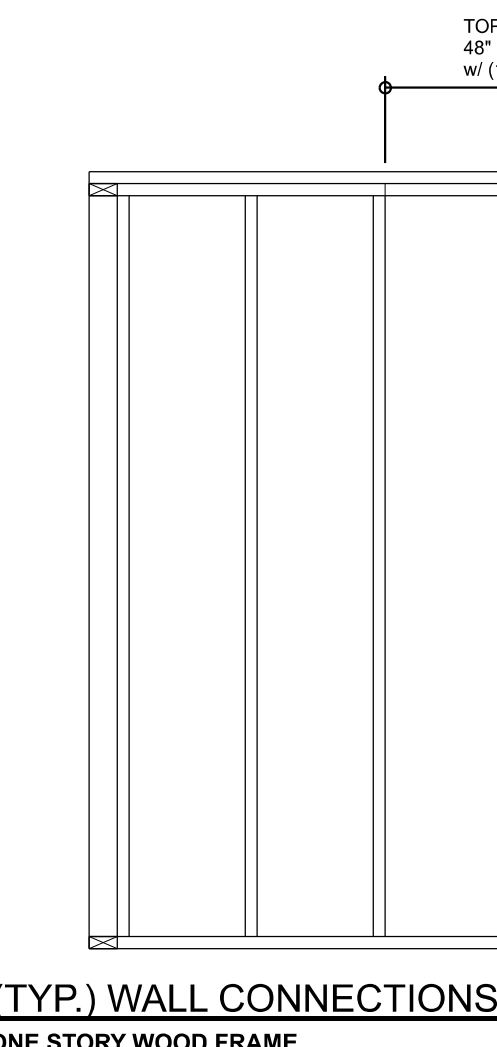
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



(TYP.) CORNER FRAMING
WOOD FRAME

TYPICAL HEADER STRAPING OR SCREWS DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

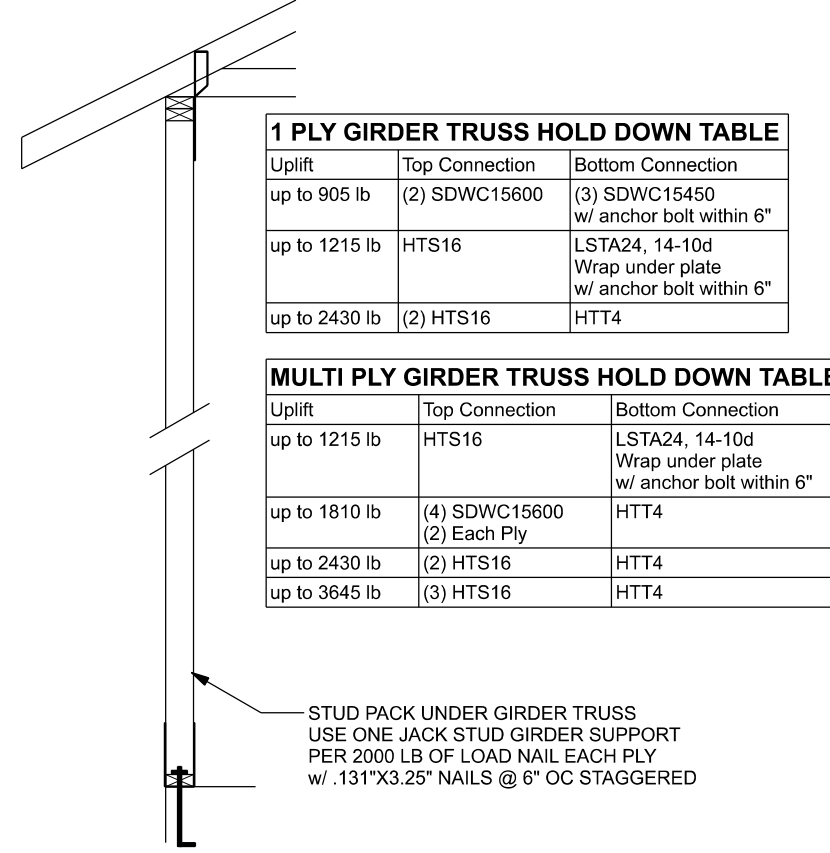


(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME

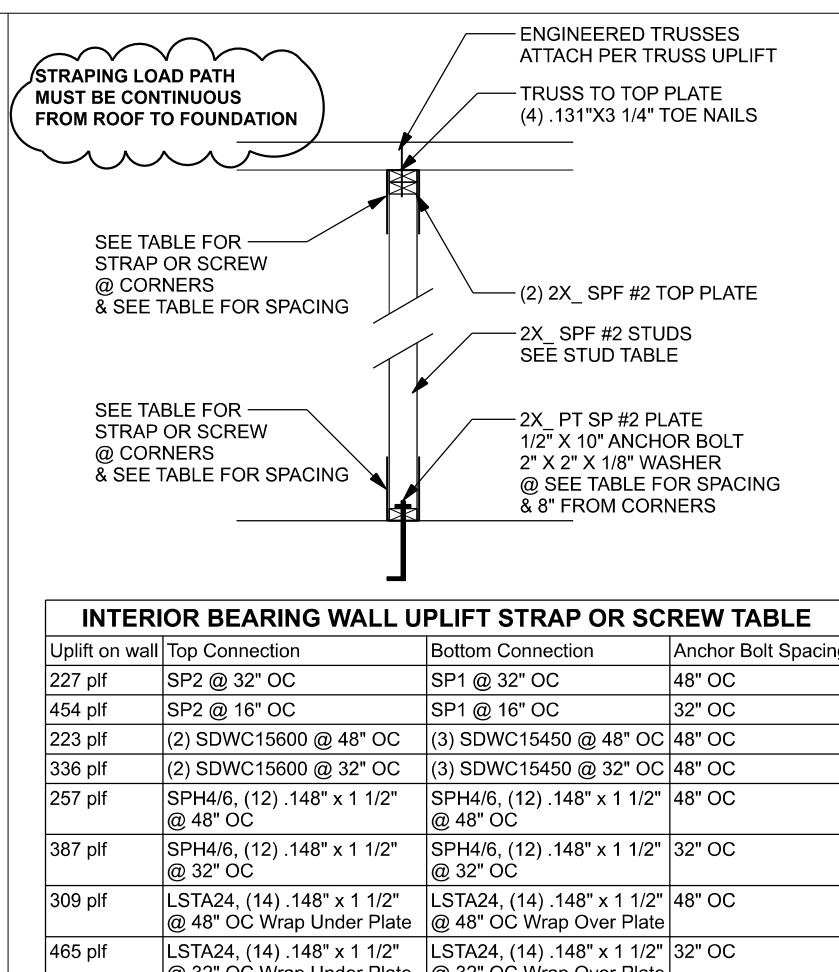
ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)					
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field	
120 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2.38" x 0.131")	6" oc	12" oc	
120 mph Exp. C	7/16"	ASTM F1667 RSR-01 (2.38" x 0.131")	6" oc	6" oc	
120 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (2" x 0.120")	6" oc	6" oc	
130 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2.38" x 0.131")	6" oc	6" oc	
130 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (2" x 0.120")	6" oc	6" oc	
130 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (2" x 0.120")	6" oc	6" oc	
140 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2.38" x 0.131")	6" oc	6" oc	
140 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (2" x 0.120")	6" oc	6" oc	
140 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (2" x 0.120")	6" oc	6" oc	
150 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (2" x 0.120")	6" oc	6" oc	
150 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2.12" x 0.131") or ASTM F1667 RSR-04 (2" x 0.120")	4" oc	4" oc	

Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.

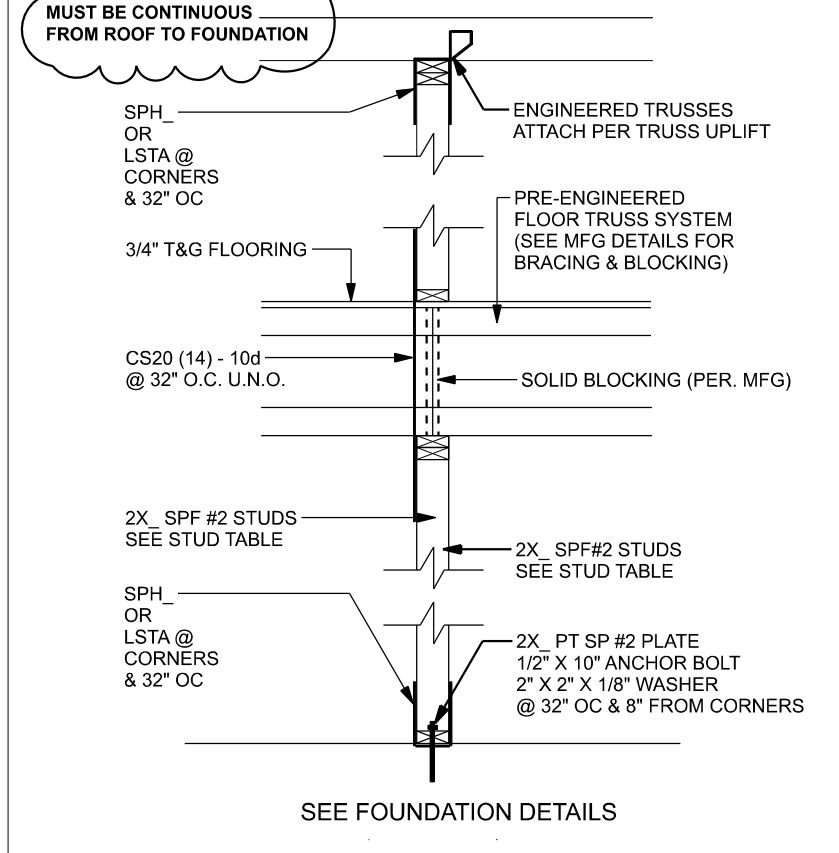
SHEATHING FOR UPLIFT ATTACHMENT DETAILS
ONE STORY WOOD FRAME



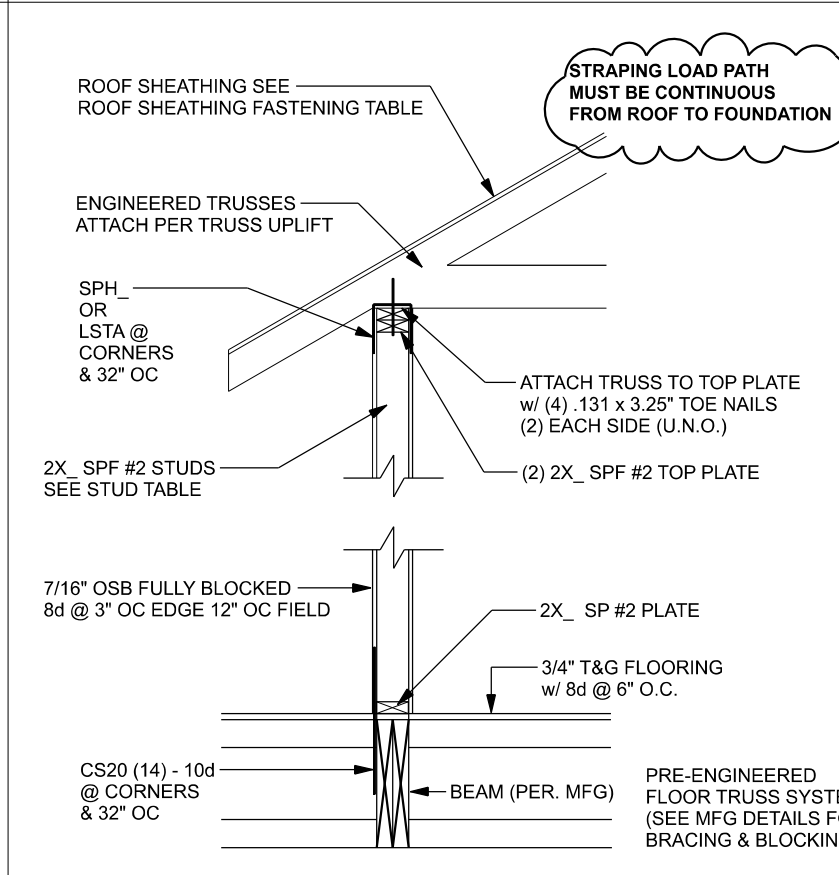
(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



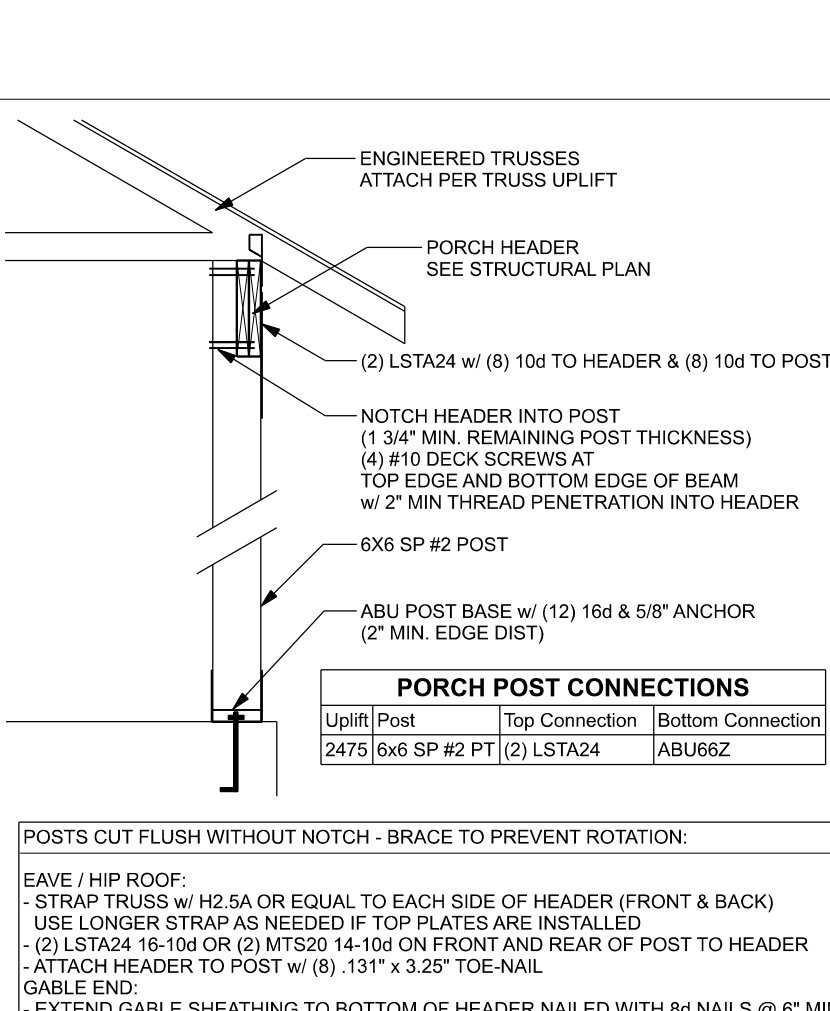
2 STORY INTERIOR BEARING WALL
SCALE: 1/2" = 1'-0"



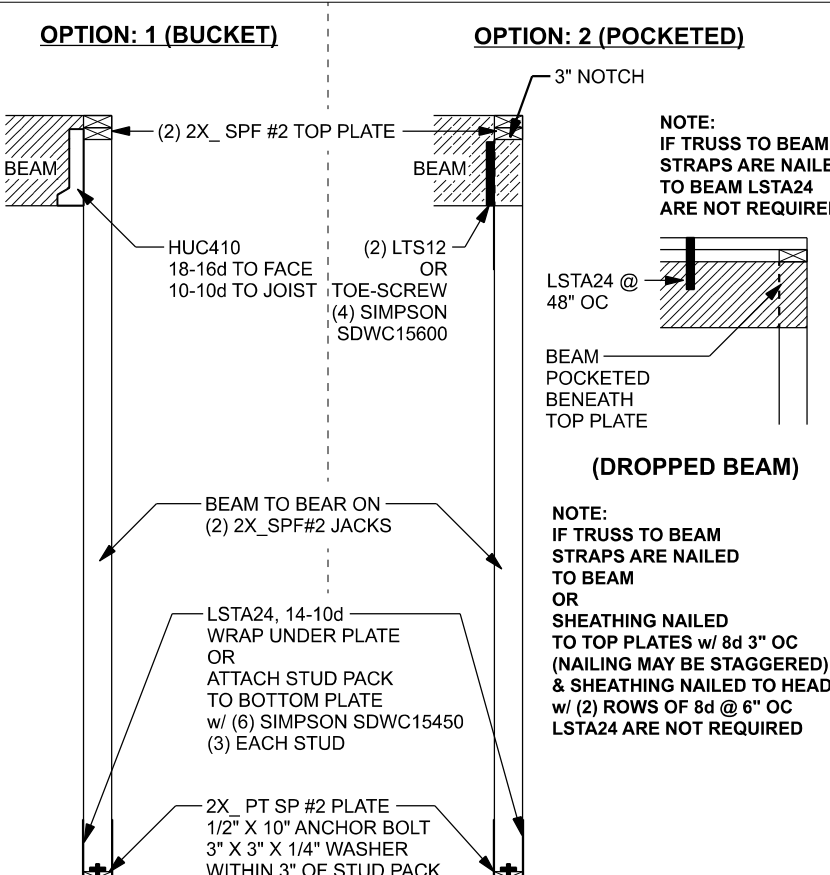
2ND FLOOR WALL TO BEAM / FLOOR TRUSS STRAPING DETAIL
SCALE: 1/2" = 1'-0"



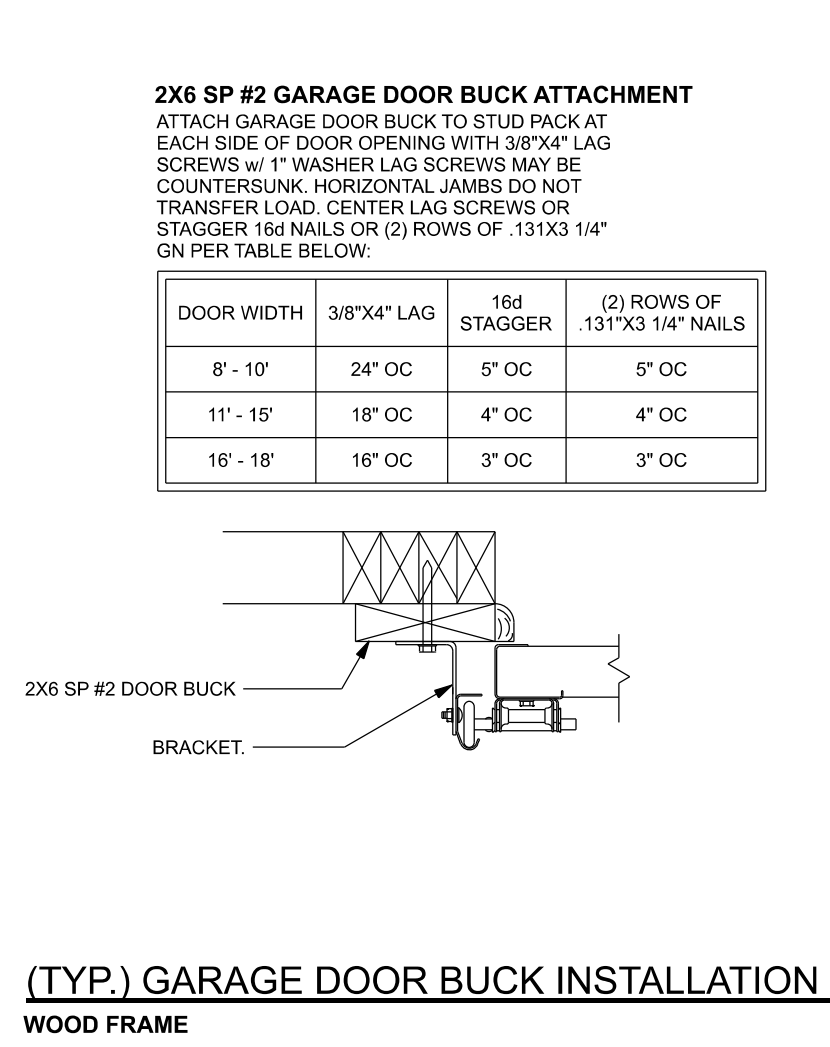
CONNECTOR TABLE					
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter	
805	505	SDWC15600	-	-	
400	290	H3	4-131"x1 1/2"	4-131"x1 1/2"	
625	540	H2.5A	5-131"x1 1/2"	5-131"x1 1/2"	
1040	1015	H104	9-148"x1 1/2"	9-148"x1 1/2"	
645	515	MTS12-20	6-148"x1 1/2"	6-148"x1 1/2"	
990	850	MTS12-30	7-148"x1 1/2"	7-148"x1 1/2"	
1415	1215	HTS20-30	8-148"x1 1/2"	8-148"x1 1/2"	
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member	
1235	1235	LSTA21	6-148"x1 1/2"	6-148"x1 1/2"	
1260	1100	SPH46	12-148"x1 1/2"	wrap under or over plate	
771	771	LSTA24	10-148"x1 1/2"	9-148"x1 1/2"	
1030	1030	CS20	7-148"x1 1/2"	7-148"x1 1/2"	
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate	
555	535	SP1	4-148"x3"	4-148"x3"	
1010	605	SP2	6-148"x3"	6-148"x3"	
1235	1235	LSTA21	12-148"x1 1/2"	wrap under or over plate	
771	771	LSTA24	10-148"x1 1/2"	9-148"x1 1/2"	
1235	1235	LSTA24	14-148"x1 1/2"	wrap under or over plate	
Uplift SP	Uplift SPF	Holdowns @ Stemmwall	To Stud / Post	Anchor	
2145	1835	DTT22	8-SDS 14"x1 1/2"	1/2"x12" Titen HD	
4235	3640	HTT4	16-162"x2 1/2"	1/2"x12" Titen HD	
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor	
2145	1835	DTT22	8-SDS 14"x1 1/2"	1/2"x6" Titen HD	
4235	3640	HTT4	16-162"x2 1/2"	1/2"x12" Titen HD	
Uplift SP	Uplift SPF	Post Bases @ Stemmwall	To Post	Anchor	
1900		ABU42	12-162"x3 1/2"	5/8"x12" Drill & Epoxy	
2475		ABU62	12-162"x3 1/2"	5/8"x12" Drill & Epoxy	
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor	
1900		ABU42	12-162"x3 1/2"	5/8"x12" Drill & Epoxy	
2475		ABU62	12-162"x3 1/2"	5/8"x12" Drill & Epoxy	



(TYP.) PORCH POST
ONE STORY WOOD



(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING SHALL BE THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER HAS SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT THE TRUSS CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVIDES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $f'_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, FB = 85ksi, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 3 INCHES, DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT CUT WITHIN 6" OF REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND ENGINEER'S APPROVAL. CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, Fy = 40 KSI. ALL LAP SPACES 4" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-11, U.N.C.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURERS' INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GRAVOTED CMU.

BUILDER'S RESPONSIBILITY: THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS: FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR. ANCHORS, UPLIFTS AND BEARING LOCATIONS IN TRUSSES ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER, IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED AND SEALED BY A DESIGN MANUFACTURER FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR W WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR. RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE

2x6	SP #2	Fb	E
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LVL	TIMBERSTRAND 1700	1.7	
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

DESIGN CRITERIA & LOADS:

BUILDING CODE	8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023)
CODE FOR DESIGN LOADS	ASCE 7-22
WINDLOADS	
BASIC WIND SPEED (ASCE 7-22, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	
1500 PSF	
FLOOD ZONE	
THIS BUILDING IS NOT IN THE FLOOD ZONE	

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)

EFFECTIVE WIND AREA (FT ²)	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Vasdt) -27.8(Vasdt)	+25.6(Vasdt) -34.2(Vasdt)
0 - 20	+42.6(Vult) -46.2(Vult)	+42.6(Vult) -57(Vult)
GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)		
9x7 GARAGE DOOR	+22.6(Vasdt) -25.5(Vasdt)	
16x7 GARAGE DOOR	+21.7(Vasdt) -24.1(Vasdt)	

Stanley & Beverly Pope Res.
PROJECT ADDRESS:
649 NW Neagel Rd., Lake City, FL 32055

FL PE 53915
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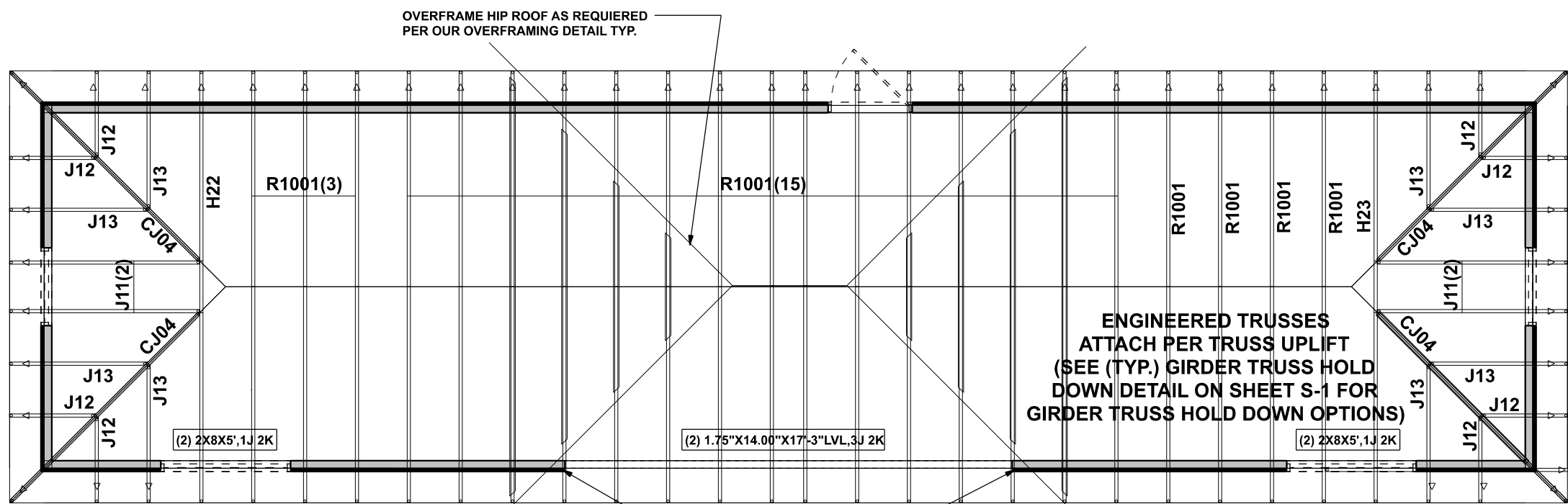
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disoway P.E.
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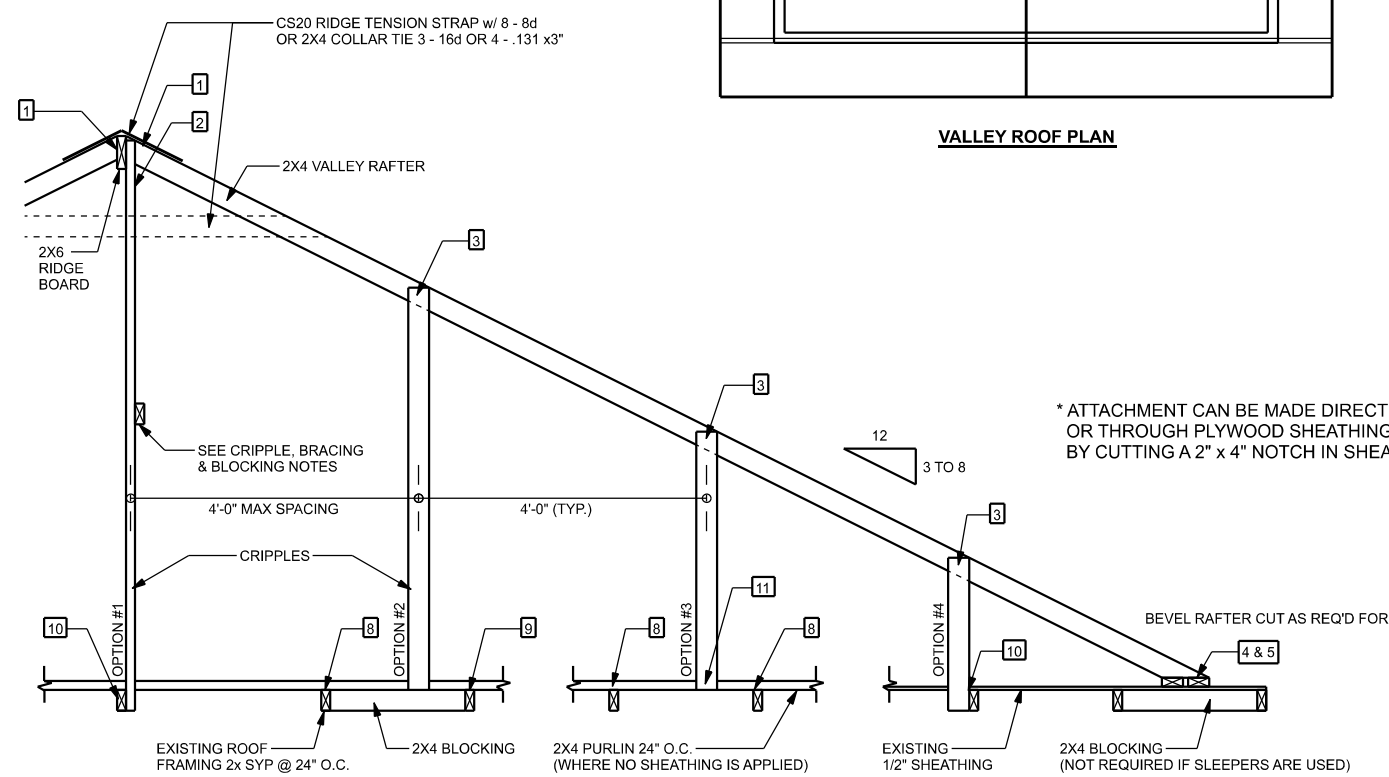
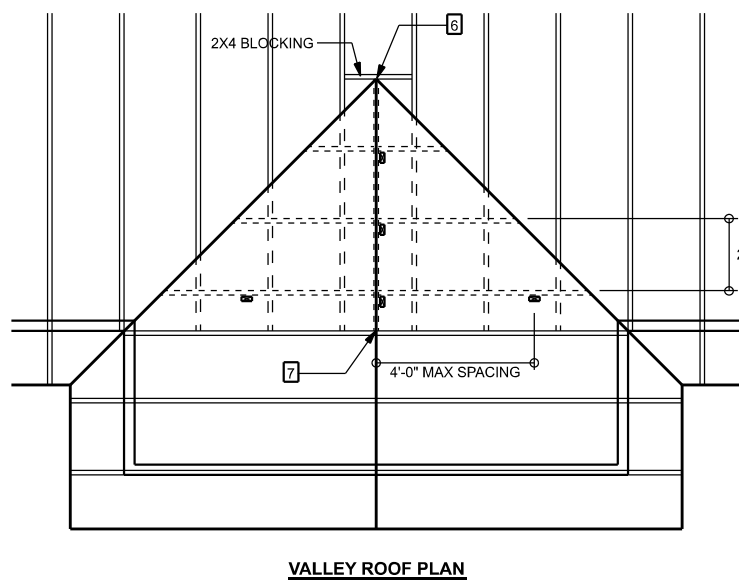
JOB NUMBER:
240444
S-1
OF 4 SHEETS



BONUS ROOM STRUCTURAL PLAN
SCALE: 3/16\"/>

LUMBER SIZE & GRADE MINIMUM REQUIREMENTS

RIDGE BOARD	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
PURLINS / LATERAL BRACING	2X4 SPF #2
SLEEPERS	2X (WIDTH OF RAFTER SEAT CUT) SPF #3 OR 2 PARALLEL 2X4 SPF #3)
CRIPPLES & BLOCKING	2X4 SPF #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



ROOF OVER FRAMING & BRACING DETAIL
SCALE: N.T.S.

VALLEY ROOF PLAN MEMBER LEGEND

---	TRUSS
- - -	TRUSS UNDER VALLEY FRAMING
- . - . -	VALLEY RAFTER OR RIDGE
●	CRIPPLE

CRIPPLES 4'-0\"/>

CONNECTION REQUIREMENT NOTES

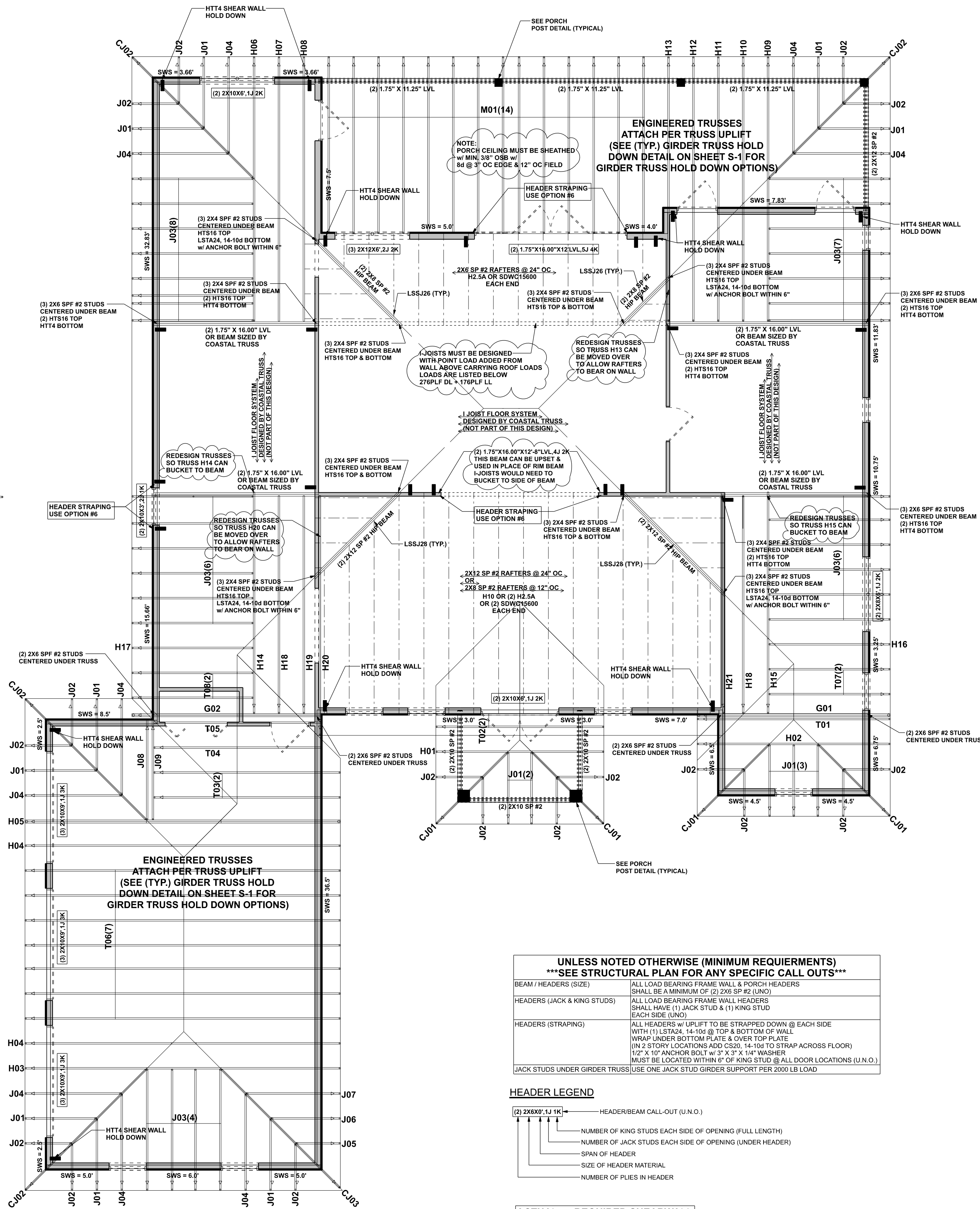
1	2X4 RAFTERS TO RIDGE	3-16d OR 6-131 x 3\"/>
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GENERAL NOTES

MAXIMUM RAFTER SPANS
9'-0\"/>

CRIPPLE, BRACING, & BLOCKING NOTES

2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0\"/>



STRUCTURAL PLAN
SCALE: 3/16\"/>

STRUCTURAL PLAN NOTES

- SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS.
- SN-2 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BC01-03, BC01-01, BC01-02, & BC01-03. BC01-01, BC01-02, & BC01-03 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE.

UNLESS NOTED OTHERWISE (MINIMUM REQUIREMENTS) ***SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS***	
BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (UNO)
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (UNO)
HEADERS (STRAPPING)	ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF PLATE (IN 2-STORY LOCATIONS ADD CS20, 14-10d TO STRAP ACROSS FLOOR) 1/2\"/>

HEADER LEGEND

(2) 2X6X5' L J 1K	HEADER/BEAM CALL-OUT (U.N.O.)
(1) 2X6X5' L J 1K	NUMBER OF KING STUDS EACH SIDE OF OPENING (FULL LENGTH)
(1) 2X6X5' L J 1K	NUMBER OF JACK STUDS EACH SIDE OF OPENING (UNDER HEADER)
(1) 2X6X5' L J 1K	SPAN OF HEADER
(1) 2X6X5' L J 1K	SIZE OF HEADER MATERIAL
(1) 2X6X5' L J 1K	NUMBER OF PILES IN HEADER

ACTUAL vs REQUIRED SHEAR WALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	18479 LBF	21195 LBF
REQUIRED	18499 LBF	18164 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. COASTAL TRUSS & VINYL SIDING JOB #240124-03KM

Stanley & Beverly Pope Res.

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FL PE 53915

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JOB NUMBER:
240444

S-3
OF 4 SHEETS

RAFTER SPAN TABLE (SP#2)
BASED ON WFCM TABLES 3.62A-L & FOOTNOTES

CEILING JOIST SPAN TABLE (SP#2)
BASED ON WFCM TABLES 3.25

MAX. TOP PLATE TO RIDGE HEIGHT	# OF .131" x 2 1/2" TOE-NAILS REQUIRED AT RAFTER TO WALL (FOR SHEAR)															
	DESIGN WIND SPEED (MPH)															
	110	120	130	140	150	160	170	110	120	130	140	150	160			
	RAFTER SPACING = 16" OC								RAFTER SPACING = 24" OC							
10'	4	4	4	4	4	4	4	4	5	5	5	5	5			
15'	4	4	4	4	4	5	5	5	-	-	-	-	-			
20'	5	5	5	5	5	6	6	-	-	-	-	-	-			

FOR CONNECTIONS REQUIRING MORE THAN (6) NAILS "X" USE (4) 131" x 3.25" TOE-
SUPPLY LATERAL / SHEAR LOAD WITH STRAP PER RAFTER TO WALL CONNECTION

RAFTER SPAN TABLE (SP#2)
BASED ON WFCM TABLES 3.62A-L & FOOTNOTES

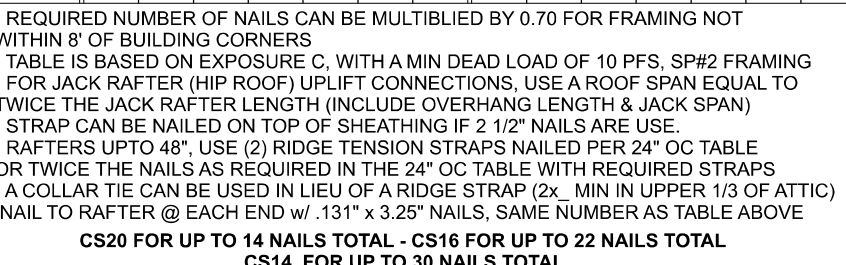
CEILING JOIST SPAN TABLE (SP#2)
BASED ON WFCM TABLES 3.25

	WALL HEIGHT	8'	10'
ROOF PITCH	TOP PLATE TO RIDGE HEIGHT	ADJUSTMENT FACTOR	
<6:12	0' (FLAT)	0.40	0.50
	5'	0.65	0.75
	10'	0.90	1.00
>6:12	5'	0.65	0.75
	10'	0.90	1.00
	15'	1.15	1.25
	20'	1.40	1.50

U = CONNECTOR UPLIFT LOAD
L = CONNECTOR LATERAL LOAD (PERPENDICULAR TO THE WALL)
S = CONNECTOR SHEAR LOAD (PARALLEL TO THE WALL)
R = LW FOR WIND PERPENDICULAR TO THE RIDGE AND WALL FOR RIDGE, WHERE W IS THE BUILDING WIDTH AND L IS THE BUILDING LENGTH
UPLIFT AND LATERAL LOADS CAN BE MULTIPLIED BY 0.75 (WIND PARALLEL) AND 0.92 (LATERAL) FOR FRAMING NOT LOCATED WITHIN 8' OF BUILDING CORNERS.
* UPLIFT LOADS ASSUME A REDUCED ROOF / CEILING ASSEMBLY DEAD LOAD OF 9 psf (6.6 to 15 psf per Rafter).
* UPLIFT LOADS CAN BE REDUCED BY 75% (6.6 to 12.1 psf) FOR EACH WALL ABOVE.
* FOR JACK RAFTER (HIP) UPLIFT CONNECTIONS, USE A ROOF SPAN EQUAL TO TWICE THE JACK RAFTER LENGTH. JACK RAFTER LENGTH = OVERHANG + SPAN

RAFTER EAVE CONNECTION DETAIL

CONVENTIONALLY FRAMED ROOF



FOR JOISTS CROSS BEAMS USE FLAT STRAPS w/ .148 x 2 1/2" NAILS, SAME NUMBER AS TABLE ABOVE IN EACH END OF STRAP.
MAX NAILS IN EACH END OF STRAP: CS20=(6) NAILS, CS16=(10) NAILS, CS14 = (14) NAILS
HEEL JOINT CONNECTIONS ARE NOT REQUIRED WHEN THE RIDGE IS SUPPORTED
FOLLOWING TABLES OUTLINE THE REQUIRED FOR STEEL JOISTS & CROSS

USE MIN OF (1) $31\frac{1}{2} \times 3\frac{1}{2}$ NAIL @ EACH END PER 2' OF CEILING JOIST SPAN (20 PSF L TABULATED HEEL JOINT CONNECTION REQUIREMENTS ASSUME THAT CEILING JOISTS OF TYPE ARE LOCATED AT THE BOTTOM OF THE ATTIC SPACE. WHEN CEILING JOISTS OR TIES ARE LOCATED HIGHER IN THE ATTIC, HEEL JOINT CONNECTION REQUIREMENTS SHALL BE INCREASED BY THE FOLLOWING FACTORS:

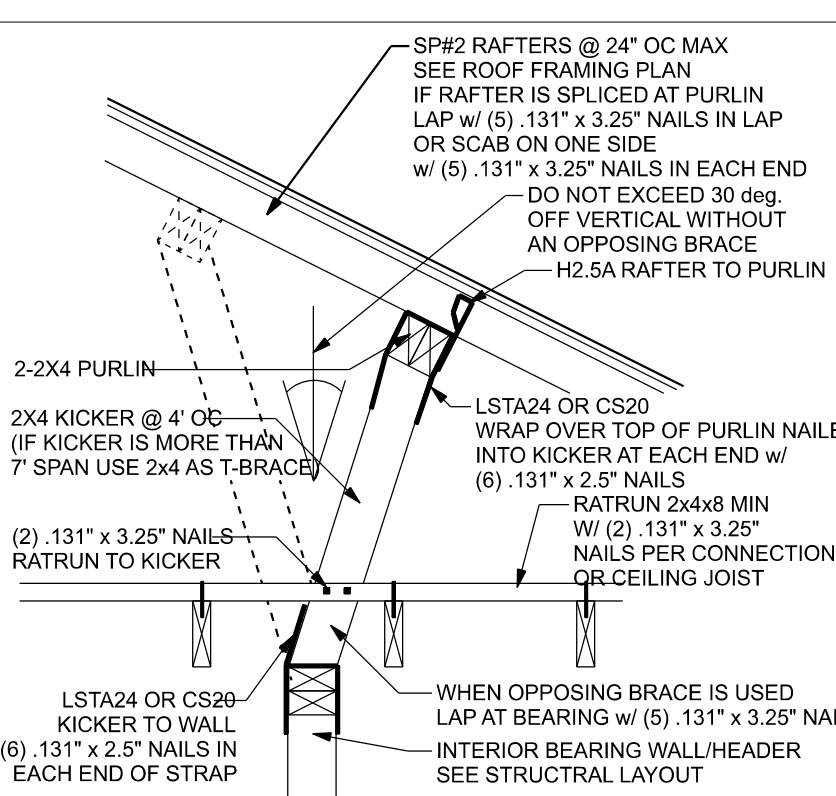
	Hc/Hr	HEEL JOINT CONNECTION ADJUSTMENT FACTOR
Hc = HEIGHT OF CEILING JOISTS OR RAFTER TIES MEASURED VERTICALLY ABOVE TOP OF THE RAFTER SUPPORT WALLS	1/2	2.00
	1/3	1.50
Hr = HEIGHT OF RIDGE MEASURED VERTICALLY ABOVE TOP OF THE RAFTER SUPPORT WALLS.	1/4	1.33
	1/5	1.25

RIDGE TENSION STRAP NAILING

BASED ON WFCM, TABLE 3.6, EXP C

HEEL JOINT CONNECTION

BASED ON WFCM TABLE 3.9



TYPICAL KICKER (RAFTER BRACE) SECTION CONVENTIONALLY FRAMED ROOF

Stanley & Beverly Pope Res.

PROJECT ADDRESS: 649 NW Noegel Rd Lake City, OR

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DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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JOB NUMBER:
240444

S-3.1
OF 4 SHEETS