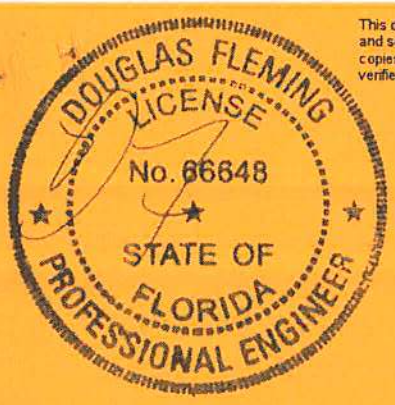


This document has been electronically signed and sealed using a Digital Signature. Printed copies without an original signature must be verified using the original electronic version.

ALPINE
AN ITW COMPANY

COA #0 278
12/03/2020

Alpine, an ITW Company
6750 Forum Drive, Suite 305
Orlando, FL 32821
Phone: (800)755-6001
www.alpineitw.com

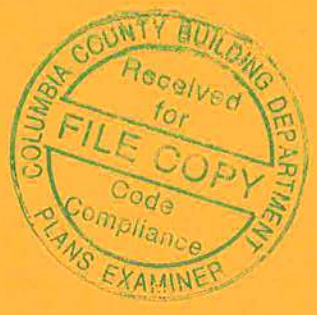
Site Information:	Page 1:
Customer: Seminole Trusses, Inc.	*Job Number:* B52218a
Job Description: Siters Res	
Address: Wester Rd, LAKE CITY, FL	

Job Engineering Criteria:	
Design Code: FBC 2017 RES	*IntelliVIEW Version:* 20.02.00A *JRef #:* 1X0V8570003
Wind Standard: ASCE 7-10 *Wind Speed (mph):* 130 *Building Type:* Closed	*Design Loading (psf):* 37.00

This package contains general notes pages, 12 truss drawing(s) and 6 detail(s).

Item	Drawing Number	Truss
1	337.20.1652.34443	A-1
3	337.20.1652.49480	A-2
5	337.20.1652.53380	T-3
7	337.20.1652.56447	GE2
9	337.20.1652.59367	GE3
11	337.20.1653.51790	GE4
13	BRCLBSUB0119	
15	PB180160118	
17	A14015ENC101014	

Item	Drawing Number	Truss
2	337.20.1652.43350	GE1 Struct
4	337.20.1652.51550	T-2
6	337.20.1652.55100	T-4
8	337.20.1652.57830	T-7
10	337.20.1653.50347	T-8 2-ply
12	337.20.1654.03550	GE5
14	PB160160118	
16	REPCHRD1014	
18	GBLLETIN0118	



General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

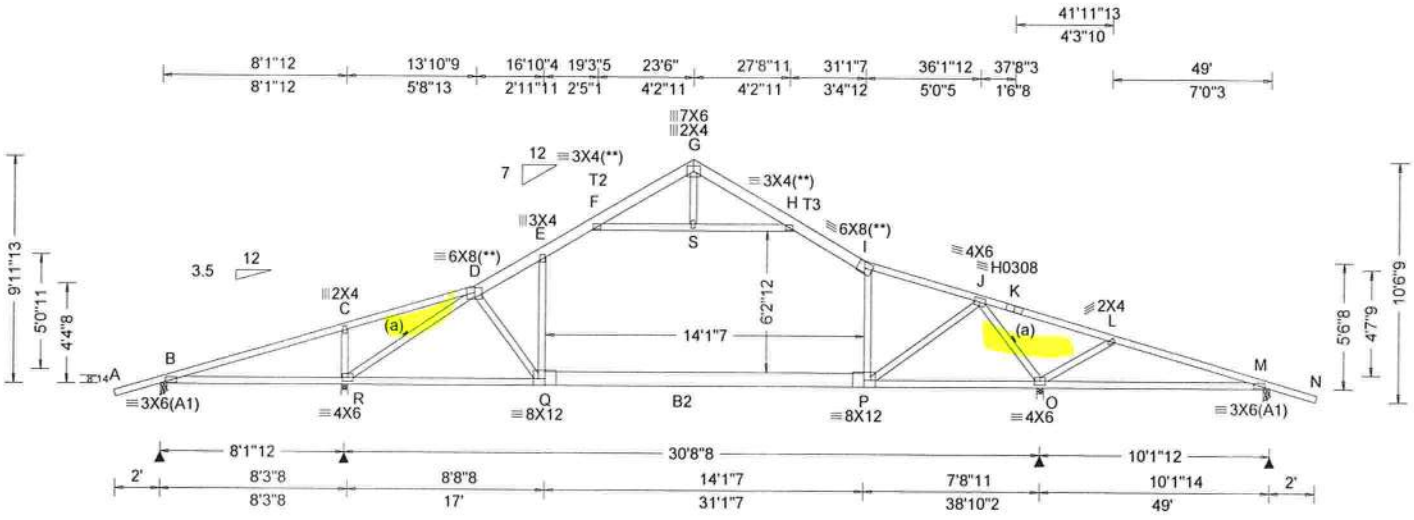
BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcindustry.com.

SEQN: 803486 COMN	Ply: 1	Job Number: B52218a	Cust: R 857 JRef: 1X0V8570003 T14
FROM: mjd	Qty: 8	Siters Res	DrwNo: 337.20.1652.34443
		Truss Label: A-1	SSB / DF 12/02/2020



Loading Criteria (psf)		Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria				
TCLL:	20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in	loc	L/defl	L/#	
TCDL:	7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL):	0.127 Q	999	240	
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL):	0.302 Q	999	180	
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL):	0.056 E	-	-	
Des Ld:	37.00	EXP: C Kzt: NA		HORZ(TL):	0.136 E	-	-	
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0				
Soffit:	0.00	TCDL: 4.2 psf	FBC 2017 RES	Max TC CSI:	0.805			
Load Duration:	1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI:	0.683			
Spacing:	24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Varies by Ld Case	Max Web CSI:	0.997			
		C&C Dist a: 4.90 ft	FT/RT:20(0)/0(0)					
		Loc. from endwall: not in 6.50 ft	Plate Type(s):					
		GCpi: 0.18	WAVE, HS	VIEW Ver: 20.02.00A.1020.20				
		Wind Duration: 1.60						

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	89	/-250	/-	/88	/169	/276
R	2132	/-	/-	/1212	/336	/-
O	2257	/-	/-	/1191	/383	/-
M	162	/-204	/-	/88	/107	/-

Wind reactions based on MWFRS

B	Brg Width = 3.5	Min Req = 1.5
R	Brg Width = 3.5	Min Req = 2.7
O	Brg Width = 3.5	Min Req = 2.8
M	Brg Width = 3.5	Min Req = 1.5

Bearings B, R, O, & M Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	1560	- 402	I - J	307	- 1193
C - D	1554	- 369	J - K	1630	- 343
D - E	372	- 1384	K - L	1611	- 354
E - F	370	- 1140	L - M	1280	- 250
H - I	384	- 1202			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - R	241	- 1443	Q - P	1105	- 96
R - Q	948	- 148	O - M	280	- 1179

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - R	208	- 449	S - H	323	- 1027
R - D	499	- 2730	P - J	1443	- 243
E - Q	468	- 61	J - O	582	- 2254
F - S	323	- 1027	O - L	187	- 504

Lumber

Top chord: 2x4 SP #1; T2,T3 2x6 SP #1;
Bot chord: 2x4 SP #1; B2 2x8 SP SS Dense;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Plates sized for a minimum of 3.50 sq.in./piece.

Loading

Live loads applied in combination per ASCE 7 sec. 2.4.1 use 0.75 factor for multiple live loads.

BC attic loading: LL = 20.00 psf; DL = 10.00 psf; from 17-0-0 to 31-1-7.

Purlins

Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads based on MWFRS with additional C&C member design.

This truss has not been designed for habitation.
Truss not designed to be used as floor.
This truss must never be served by a fixed stairway.

Additional Notes

Negative reaction(s) of -250# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

COA #0 278

12/03/2020

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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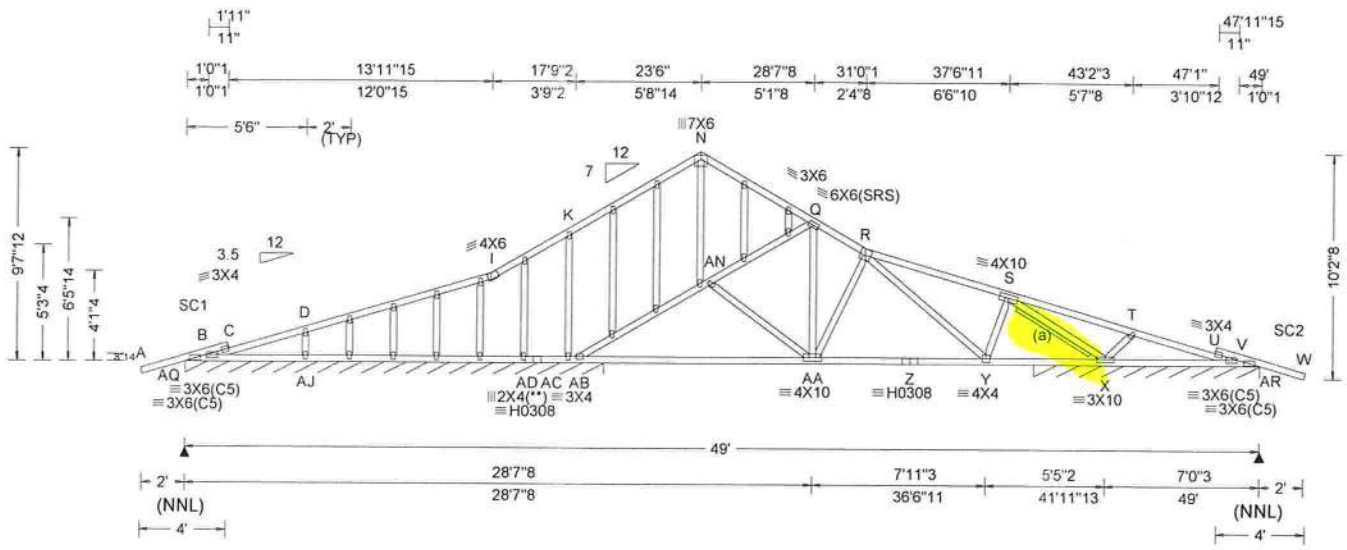
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcindustry.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 803423 GABL FROM: mjd	Ply: 1 Qty: 1	**Job Number:** B52218a Siters Res **Truss Label:** GE1 Struct	Cust: R 857 JRef: 1X0V8570003 T6 DrwNo: 337.20.1652.43350 SSB / DF 12/02/2020	



Loading Criteria (psf)	
TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	0.00
Load Duration: 1.25	
Spacing: 24.0 "	

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 4.90 ft
Loc. from endwall: not in 8.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 2017 RES
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.106	M	999	240
VERT(CL): 0.199	M	999	180
HORZ(LL): 0.058	M	-	-
HORZ(TL): 0.109	M	-	-

Creep Factor: 2.0
Max TC CSI: 0.966
Max BC CSI: 0.634
Max Web CSI: 0.950

VIEW Ver: 20.02.00A.1020.20

▲ Maximum Reactions (lbs), or *=PLF

	Gravity		Non-Gravity		
Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL
AQ*248	/-	/-	/95	/16	/19
AR*310	/-	/-	/125	/-	/-

Wind reactions based on MWFRS
AQ Brg Width = 229 Min Req = -
AR Brg Width = 123 Min Req = -
Bearings AQ & Y Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	555	-683	R - S	830	-2172
K - N	347	-438	S - T	1073	0
N - Q	345	-380	T - U	569	-29
Q - R	754	-2258	U - V	998	-431

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
AB-AA	4712	-636	Y - X	3766	-1067
AA- Z	2280	-497	X - V	129	-460
Z - Y	2280	-497			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
AB-AN	708	-2582	AA- R	438	-1015
AN-AA	38	-628	R - Y	160	-438
AN- Q	675	-1998	S - X	591	-3413
AA- Q	1401	-380	X - T	0	-696

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.	Gables	Tens.	Comp.
D -AJ	0	-485	K -AC	249	-560

Laterally brace chord above/ below filler at 24" OC (or as designed) including a lateral brace on chord directly below both ends of filler (if no rigid diaphragm exists at that point)

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #1;
Stack Chord: SC2 2x4 SP #1;

Bracing

(a) 2x6 #3 or better "T" reinforcement. 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

Plating Notes

All plates are 2X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Plates sized for a minimum of 3.50 sq.in./piece.

Loading

Truss designed to support 2-0-0 top chord outlookers and cladding load not to exceed 6.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Additional Notes

See DWGS A14015ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

COA #0 278

12/03/2020

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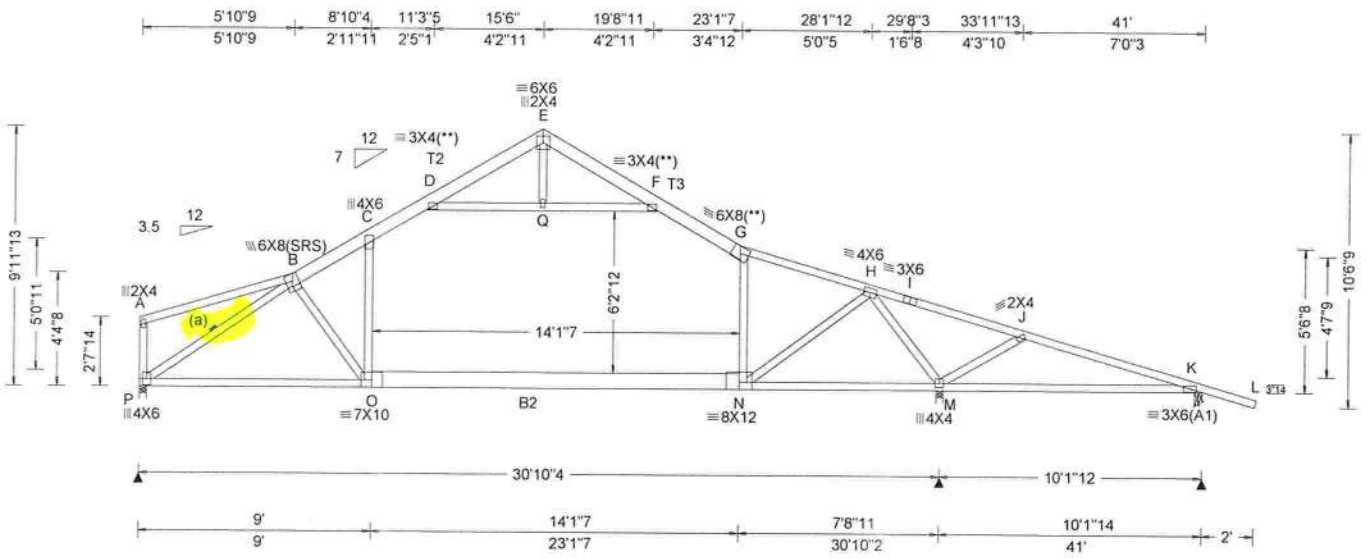
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 803476 COMN	Ply: 1	Job Number: B52218a	Cust: R 857 JRef: 1X0V8570003 T7
FROM: mjd	Qty: 1	Siters Res	DrwNo: 337.20.1652.49480
		Truss Label: A-2	SSB / DF 12/02/2020



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h to 2h
C&C Dist a: 4.10 ft
Loc. from endwall: not in 13.00 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.290 O	999	240
VERT(CL):	0.702 O	527	180
HORZ(LL):	0.163 C	-	-
HORZ(TL):	0.401 C	-	-

Creep Factor: 2.0
Max TC CSI: 0.953
Max BC CSI: 0.704
Max Web CSI: 0.966

VIEW Ver: 20.02.00A.1020.20

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
P	1395	/-	/-	/651	/38	/303
M	2147	/-	/-	/1136	/161	/-
K	327	/-9	/-	/220	/131	/-

Wind reactions based on MWFRS
P Brg Width = 3.5 Min Req = 1.8
M Brg Width = 3.5 Min Req = 2.7
K Brg Width = 3.5 Min Req = 1.5
Bearings P, M, & K Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	476	- 2033	H - I	1164	- 529
C - D	422	- 1503	I - J	1146	- 540
F - G	448	- 1631	J - K	823	- 500
G - H	382	- 1687			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
P - O	1810	- 315	N - M	778	- 344
O - N	1575	- 171	M - K	504	- 740

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
P - B	483	- 2164	Q - F	415	- 1624
B - O	242	- 491	N - H	1336	- 220
C - O	1039	- 143	H - M	540	- 2151
D - Q	415	- 1624	M - J	185	- 496

Lumber

Top chord: 2x4 SP #1; T2,T3 2x6 SP #1;
Bot chord: 2x4 SP #1; B2 2x8 SP SS Dense;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 2x6 #3 or better "T" reinforcement. 80% length of web member. Attached with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

Plating Notes

(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Plates sized for a minimum of 3.50 sq.in./piece.

Loading

Live loads applied in combination per ASCE 7 sec. 2.4.1 use 0.75 factor for multiple live loads.

BC attic loading: LL = 20.00 psf; DL = 10.00 psf; from 9-0-0 to 23-1-7.

Purlins

Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical exposed to wind pressure. Deflection meets L/180.

This truss has not been designed for habitation.
Truss not designed to be used as floor.
This truss must never be served by a fixed stairway.

COA #0 278

12/03/2020

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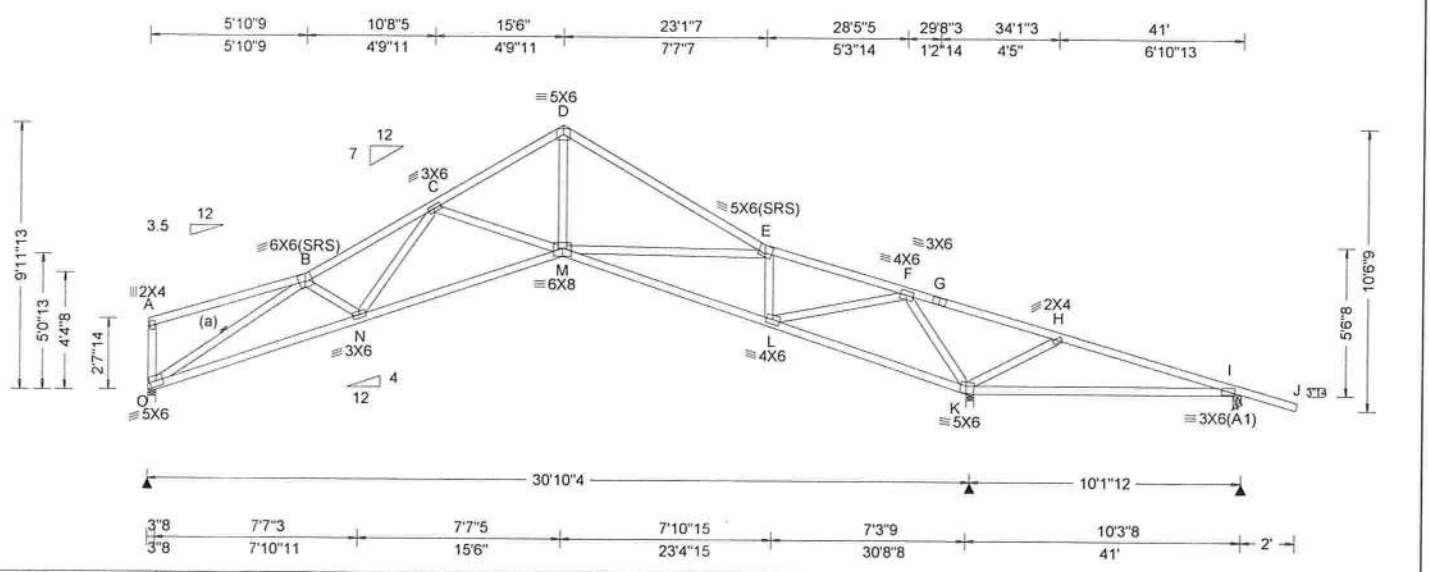
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 803428 COMN FROM: mjd	Ply: 1 Qty: 10	**Job Number:** B52218a Siters Res **Truss Label:** T-2	Cust: R 857 JRef:1X0V8570003 T1 DrwNo: 337.20.1652.51550 SSB / DF 12/02/2020	



Loading Criteria (psf)		**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**				
TCLL:	20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in	loc	L/defl	L/#	
TCDL:	7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL):	0.168 C	999	240	
BCLL:	0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL):	0.320 C	999	180	
BCDL:	10.00	Risk Category: II	Snow Duration: NA	HORZ(LL):	0.139 K	-	-	
Des Ld:	37.00	EXP: C Kzt: NA		HORZ(TL):	0.267 K	-	-	
NCBCLL:	10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0				
Soffit:	0.00	TCDL: 4.2 psf	FBC 2017 RES	Max TC CSI:	0.680			
Load Duration: 1.25		BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI:	0.624			
Spacing: 24.0 "		MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI:	0.771			
		C&C Dist a: 4.10 ft	FT/RT:20(0)/0(0)					
		Loc. from endwall: not in 13.00 ft	Plate Type(s):					
		GCpi: 0.18	WAVE	VIEW Ver: 20.02.00A.1020.20				
		Wind Duration: 1.60						

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
O	1011	/-	/-	/534	/203	/303
K	2237	/-	/-	/1148	/419	/-
I	246	/-288	/-	/142	/147	/-

Wind reactions based on MWFRS
O Brg Width = 3.5 Min Req = 1.5
K Brg Width = 3.5 Min Req = 2.8
I Brg Width = 3.5 Min Req = 1.5
Bearings O, K, & I Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	634	- 2492	F - G	1822	- 384
C - D	360	- 1767	G - H	1804	- 395
D - E	334	- 1819	H - I	1532	- 275
E - F	329	- 1170			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
O - N	2280	- 481	L - K	303	- 888
N - M	2048	- 230	K - I	335	- 1446
M - L	1262	- 174			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
O - B	660	- 2584	E - L	256	- 937
N - C	396	- 170	L - F	1889	- 391
C - M	258	- 469	F - K	484	- 1740
D - M	1344	- 172	K - H	196	- 538
M - E	441	- 176			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 2x6 #3 or better "T" reinforcement. 80% length of web member. Attached with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left end vertical exposed to wind pressure. Deflection meets L/180.

Additional Notes

Negative reaction(s) of -288# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

Shim all supports to solid bearing.

COA #0 278

12/03/2020

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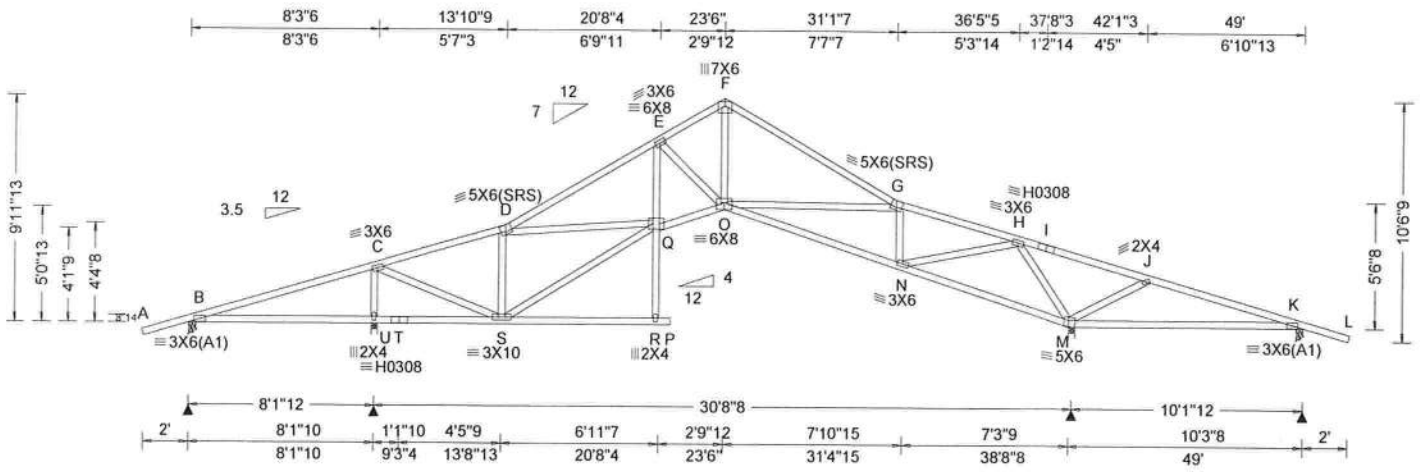
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 803404 COMN	Ply: 1	Job Number: B52218a	Cust: R 857 JRef: 1X0V8570003 T12
FROM: mjd	Qty: 1	Siters Res	DrwNo: 337.20.1652.53380
		Truss Label: T-3	SSB / DF 12/02/2020



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 4.90 ft
Loc. from endwall: not in 6.50 ft
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE, HS

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.121 O	999	240
VERT(CL):	0.227 O	999	180
HORZ(LL):	0.081 M	-	-
HORZ(TL):	0.155 M	-	-

Creep Factor: 2.0
Max TC CSI: 0.619
Max BC CSI: 0.523
Max Web CSI: 0.645

VIEW Ver: 20.02.00A.1020.20

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	301	/-116	/-	/83	/136	/404
U	1679	/-	/-	/1022	/297	/-
M	2028	/-	/-	/1105	/389	/-
K	292	/-193	/-	/157	/96	/-

Wind reactions based on MWFRS

B	Brg Width = 3.5	Min Req = 1.5
U	Brg Width = 3.5	Min Req = 2.1
M	Brg Width = 3.5	Min Req = 2.5
K	Brg Width = 3.5	Min Req = 1.5

Bearings B, U, M, & K Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	918	- 188	G - H	362	- 1218
C - D	337	- 780	H - I	1496	- 327
D - E	430	- 1898	I - J	1478	- 338
E - F	358	- 1535	J - K	1206	- 220
F - G	314	- 1609			

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - U	283	- 858	O - N	1300	- 204
U - T	265	- 799	N - M	263	- 636
T - S	265	- 799	M - K	280	- 1134
Q - O	1626	- 96			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
U - C	470	- 1492	F - O	1205	- 190
C - S	1640	- 387	G - N	242	- 819
S - D	296	- 1007	N - H	1692	- 367
S - Q	864	- 202	H - M	480	- 1596
D - Q	824	- 63	M - J	194	- 531
E - O	210	- 379			

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Wind

Wind loads based on MWFRS with additional C&C member design.

Additional Notes

Negative reaction(s) of -193# MAX. from a non-wind load case requires uplift connection. See Maximum Reactions.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

COA #0 278

12/03/2020

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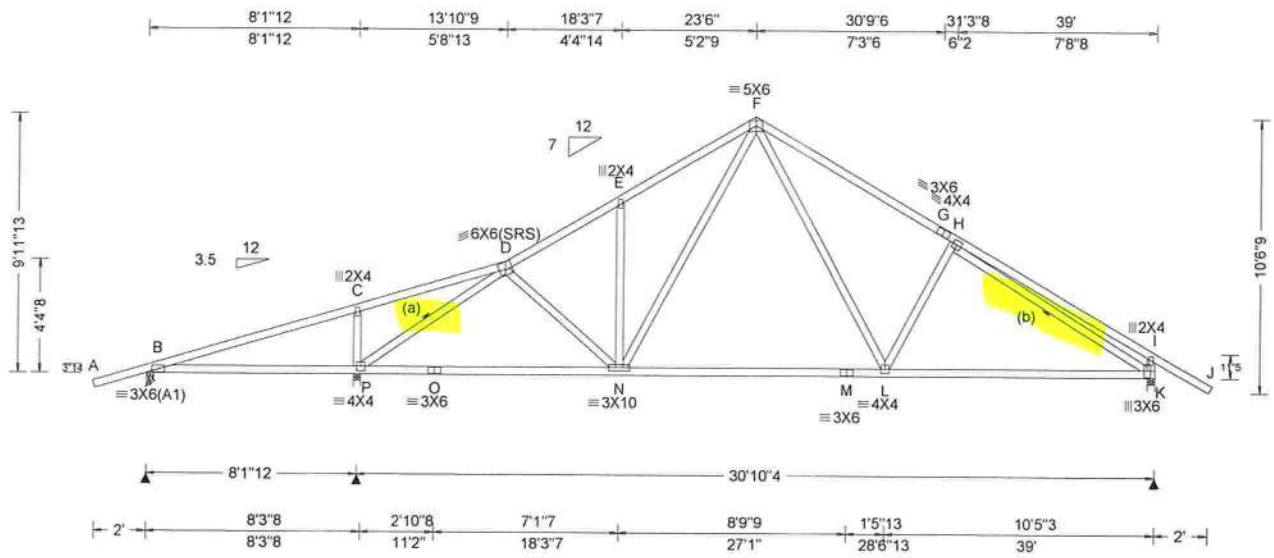
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 803392 SPEC	Ply: 1	Job Number: B52218a	Cust: R 857 JRef: 1X0V8570003 T3
FROM: mjd	Qty: 6	Siters Res	DrwNo: 337.20.1652.55100
		Truss Label: T-4	SSB / DF 12/02/2020



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.90 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL):	0.063 L	999	240
VERT(CL):	0.121 L	999	180
HORZ(LL):	0.032 I	-	-
HORZ(TL):	0.061 I	-	-

Creep Factor: 2.0
Max TC CSI: 0.470
Max BC CSI: 0.805
Max Web CSI: 0.898

VIEW Ver: 20.02.00A.1020.20

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	374	/-	/-	/180	/110	/327
P	1614	/-	/-	/932	/327	/-
K	1255	/-	/-	/742	/267	/-

Wind reactions based on MWFRS
B Brg Width = 3.5 Min Req = 1.5
P Brg Width = 3.5 Min Req = 2.0
K Brg Width = 3.5 Min Req = 1.6
Bearings B, P, & K Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
D - E	555	- 1364	F - G	609	- 1484
E - F	663	- 1363	G - H	573	- 1498

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
P - O	1091	- 375	M - L	892	- 108
O - N	1091	- 375	L - K	1329	- 331
N - M	892	- 108			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - P	339	- 421	F - L	686	- 224
P - D	656	- 1737	H - K	383	- 1421
N - F	508	- 282	I - K	301	- 400

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 2x4 #3 or better "T" reinforcement. 80% length of web member. Attached with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

(b) Continuous lateral restraint equally spaced on member. Or 2x6 #3 or better "T" reinforcement. 80% length of web member. Attached with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure. Deflection meets L/180.

COA #0 278

12/03/2020

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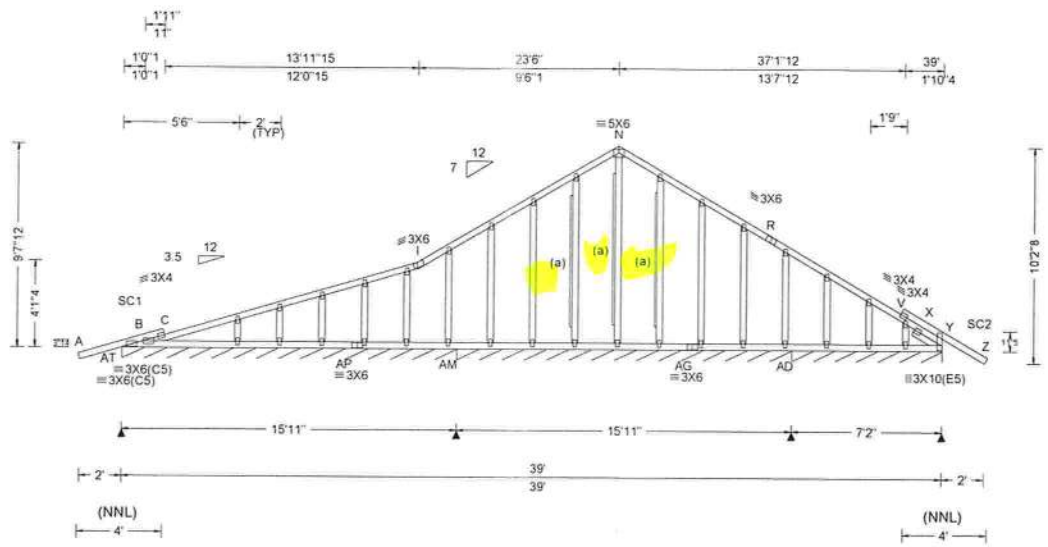
ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821

SEQN: 803396 GABL FROM: mjd	Ply: 1 Qty: 1	**Job Number:** B52218a Siters Res **Truss Label:** GE2	Cust: R 857 JRef: 1X0V8570003 T2 DrwNo: 337.20.1652.56447 SSB / DF 12/02/2020	



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.015 C 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.031 C 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.005 L - -
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.007 L - -
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 0.00	TCDL: 4.2 psf	FBC 2017 RES	Max TC CSI: 0.323
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.141
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Varies by Ld Case	Max Web CSI: 0.105
	C&C Dist a: 3.90 ft	FT/RT:20(0)/0(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 20.02.00A.1020.20
	Wind Duration: 1.60		

▲ Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
AT	*119	/-	/-	/58	/-	/15
AM	*140	/-	/-	/56	/-	/-
AD	*121	/-	/-	/63	/12	/-

Wind reactions based on MWFRS
AT Brg Width = 191 Min Req = -
AM Brg Width = 191 Min Req = -
AD Brg Width = 86.0 Min Req = -
Bearings AT, AM, & AD are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - C	445	-496

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #1;
Stack Chord: SC2 2x4 SP #1;
Rt Slider: 2x4 SP #3; block length = 1.495'

Bracing

(a) 1x4 #3SRB SPF-S or better "L" reinforcement. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

Plating Notes

All plates are 2X4 except as noted.

Plates sized for a minimum of 3.50 sq.in./piece.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 6.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

Additional Notes

See DWGS A14015ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

COA #0 278

12/03/2020

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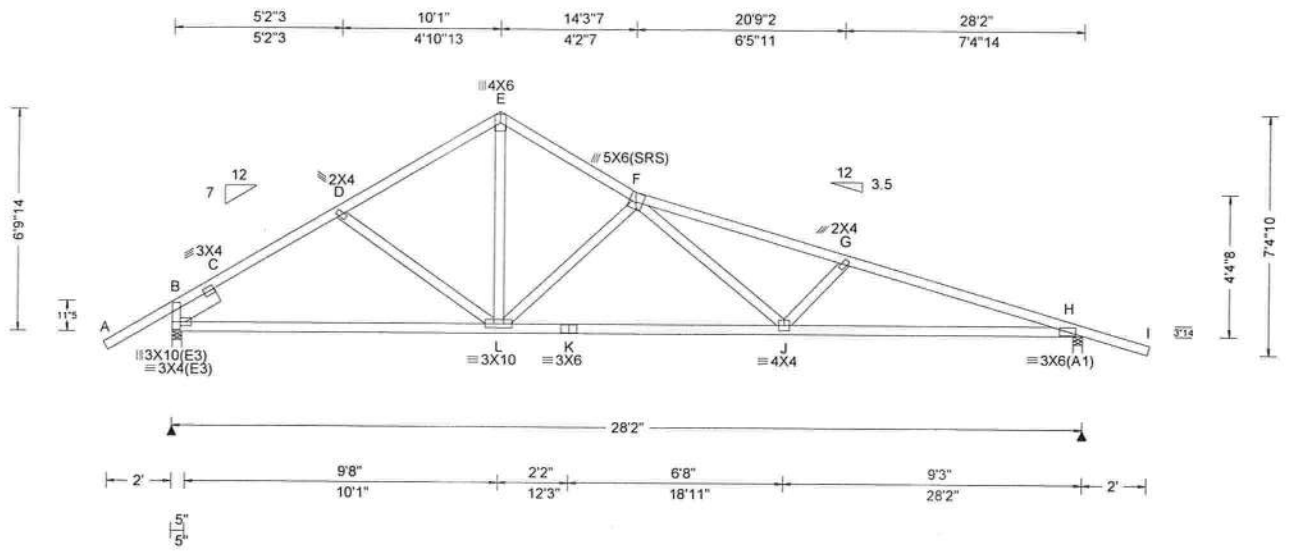
Suite 305
Orlando FL, 32821

SEQN: 803410 COMN FROM: mjd	Ply: 1 Qty: 5	**Job Number:** B52218a Siters Res **Truss Label:** T-7	Cust: R 857 JRef: 1X0V8570003 T5 DrwNo: 337.20.1652.57830 SSB / DF 12/02/2020	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 2017 RES
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.157	J	999	240
VERT(CL): 0.286	J	999	180
HORZ(LL): 0.049	E	-	-
HORZ(TL): 0.088	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.389
Max BC CSI: 0.672
Max Web CSI: 0.670

VIEW Ver: 20.02.00A.1020.20

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1178	/-	/-	/638	/255	/202
H	1173	/-	/-	/655	/265	/-

Wind reactions based on MWFRS
B Brg Width = 3.5 Min Req = 1.5
H Brg Width = 3.5 Min Req = 1.5
Bearings B & H Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	613	- 1594	E - F	604	- 1298
C - D	615	- 1478	F - G	1137	- 2498
D - E	587	- 1303	G - H	1252	- 2767

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - L	1197	- 335	K - J	1799	- 676
L - K	1799	- 676	J - H	2614	- 1116

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
E - L	976	- 467	F - J	730	- 311
L - F	624	- 1013	J - G	314	- 406

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Lt Slider: 2x6 SP #1; block length = 1.617'

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Wind

Wind loads based on MWFRS with additional C&C member design.

COA #0 278

12/03/2020

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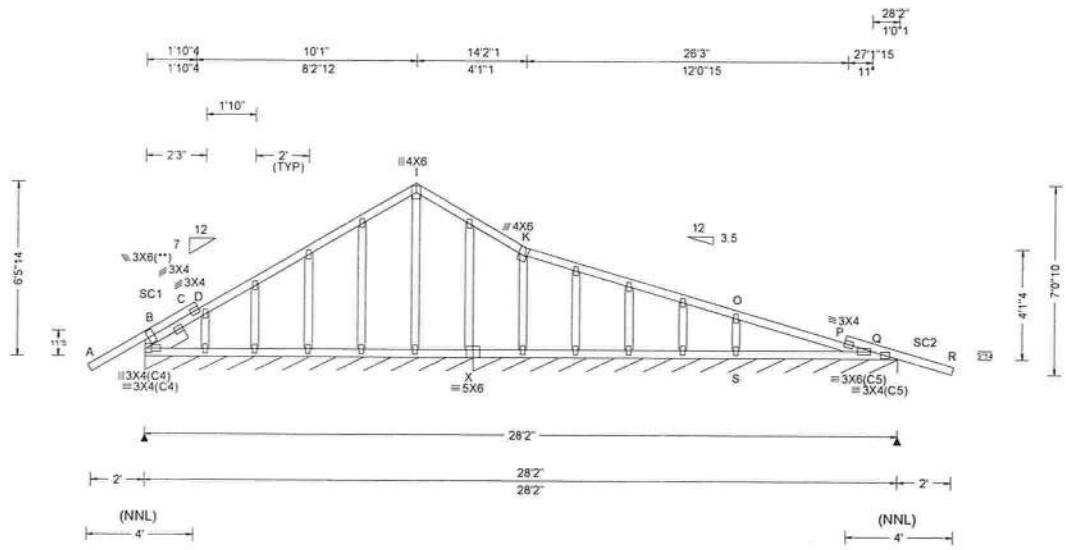
SEQN: 803417 GABL	Ply: 1	Job Number: B52218a	Cust: R 857 JRef: 1X0V8570003 T9
FROM: mjd	Qty: 1	Siters Res	DrwNo: 337.20.1652.59367
		Truss Label: GE3	SSB / DF 12/02/2020



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 2017 RES
TPI Std: 2014
Rep Fac: No
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.035	P	999	240
VERT(CL): 0.062	P	999	180
HORZ(LL): -0.006	P	-	-
HORZ(TL): 0.011	P	-	-

Creep Factor: 2.0
Max TC CSI: 0.476
Max BC CSI: 0.182
Max Web CSI: 0.179

VIEW Ver: 20.02.00A.1020.20

▲ Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	192	/-	/-	/74	/4	/18
X*	143	/-	/-	/63	/-	/-

Wind reactions based on MWFRS
B Brg Width = 147 Min Req = -
X Brg Width = 191 Min Req = -
Bearings B & X are a rigid surface.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	947	-846	P - Q	622	-747
D - I	406	-38			

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.
S - O	40	-549

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #1;
Stack Chord: SC2 2x4 SP #1;
Lt Slider: 2x6 SP #1; block length = 1.571'

Plating Notes

All plates are 2X4 except as noted.

(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Plates sized for a minimum of 3.50 sq.in./piece.

Loading

Truss designed to support 2-0-0 top chord outlookers and cladding load not to exceed 6.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Additional Notes

See DWGS A14015ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

COA #0 248

12/03/2020

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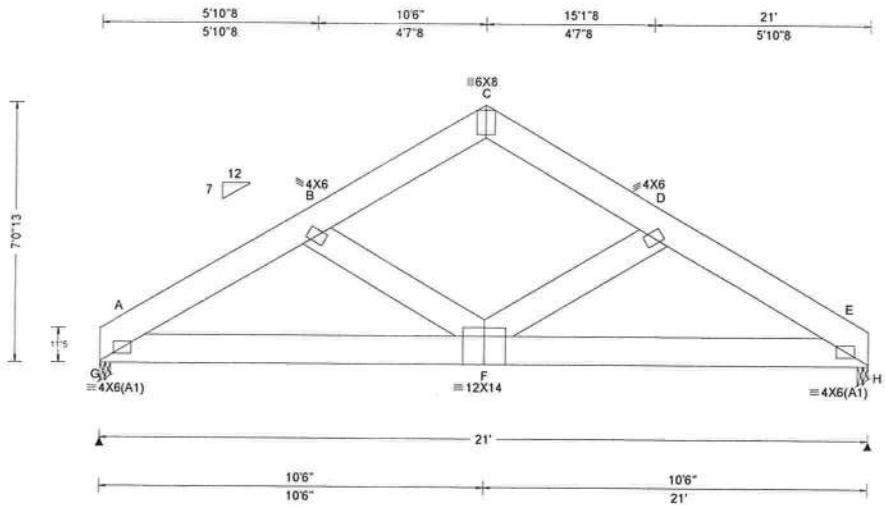
SEQN: 803425 COMN	Ply: 2	Job Number: B52218a	Cust: R 857 JRef: 1X0V8570003 T8
FROM: mjd	Qty: 2	Siters Res	DrwNo: 337.20.1653.50347
		Truss Label: T-8 2-ply	SSB / DF 12/02/2020

2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 7.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.087 F 999 240
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.165 F 999 180
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.028 B - -
Des Ld: 37.00	EXP: C Kzt: NA		HORZ(TL): 0.053 B - -
NCBCLL: 0.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0
Soffit: 0.00	TCDL: 4.2 psf	FBC 2017 RES	Max TC CSI: 0.400
Load Duration: 1.25	BCDL: 5.2 psf	TPI Std: 2014	Max BC CSI: 0.248
Spacing: 48.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: No	Max Web CSI: 0.031
	C&C Dist a: 3.00 ft	FT/RT:20(0)/0(0)	
	Loc. from endwall: Any	Plate Type(s):	
	GCpi: 0.18	WAVE	VIEW Ver: 20.02.00A.1020.20
	Wind Duration: 1.60		

▲ Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
G	1600	/-	/-	/855	/308	/308
H	1600	/-	/-	/855	/308	/-

Wind reactions based on MWFRS
G Brg Width = 3.5 Min Req = 1.5
H Brg Width = 3.5 Min Req = 1.5
Bearings G & H Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - B	305	- 882	C - D	310	- 813
B - C	310	- 813	D - E	305	- 882

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
A - F	659	- 170	F - E	659	- 170

Lumber

Top chord: 2x10 SP #2;
Bot chord: 2x10 SP #2;
Webs: 2x10 SP #2;

Nailnote

Nail Schedule:0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.
(1) 1/2" bolts may be used for
(2) 0.128"x3", min. nails on
Either The Top or Bottom Chords.

Plating Notes

Plates sized for a minimum of 3.50 sq.in./piece.

Wind

Wind loads based on MWFRS with additional C&C member design.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @24" OC.

COA #0 278

12/03/2020

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6750 Forum Drive
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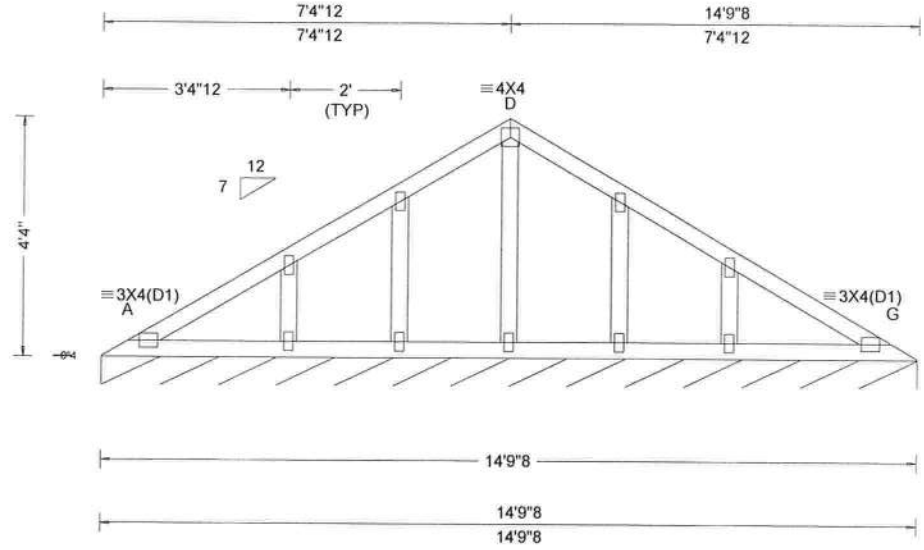
SEQN: 803430 GABL FROM: mjd	Ply: 1 Qty: 1	**Job Number:** B52218a Siters Res **Truss Label:** GE4	Cust: R 857 JRef: 1X0V8570003 T4 DrwNo: 337.20.1653.51790 SSB / DF 12/02/2020	



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 7.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 37.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 4.2 psf BCDL: 5.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/0(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.006 L 999 240 VERT(CL): 0.011 L 999 180 HORZ(LL): -0.004 L - - HORZ(TL): 0.005 H - - Creep Factor: 2.0 Max TC CSI: 0.150 Max BC CSI: 0.109 Max Web CSI: 0.077 VIEW Ver: 20.02.00A.1020.20

▲ Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
G*	142	/-	/-	/46	/67	/14

Wind reactions based on MWFRS
G Brg Width = 177 Min Req = -
Bearing A Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Plates sized for a minimum of 3.50 sq.in./piece.

Loading

Truss designed to support 2-0-0 top chord outlookers and cladding load not to exceed 6.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

Additional Notes

See DWGS A14015ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

COA #0 278

12/03/2020

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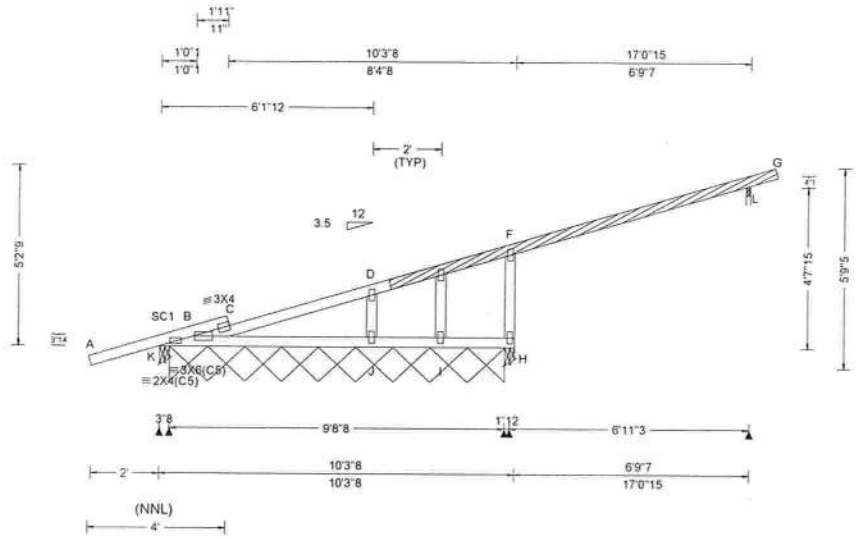
SEQN: 803488 GABL FROM: mjd	Ply: 1 Qty: 1	**Job Number:** B52218a Siters Res **Truss Label:** GE5	Cust: R 857 JRef: 1X0V8570003 T10 DrwNo: 337.20.1654.03550 SSB / DF 12/02/2020	



Loading Criteria (psf)

TCLL:	20.00
TCDL:	7.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	37.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.25
Spacing: 24.0 "

Wind Criteria
Wind Std: ASCE 7-10
Speed: 130 mph
Enclosure: Closed
Risk Category: II
EXP: C Kzt: NA
Mean Height: 15.00 ft
TCDL: 4.2 psf
BCDL: 5.2 psf
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.18
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf: NA Ce: NA
Lu: NA Cs: NA
Snow Duration: NA

Building Code:
FBC 2017 RES
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/0(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.044	C	999	240
VERT(CL): 0.069	C	999	180
HORZ(LL): -0.007	E	-	-
HORZ(TL): 0.011	E	-	-

Creep Factor: 2.0
Max TC CSI: 0.547
Max BC CSI: 0.216
Max Web CSI: 0.153

VIEW Ver: 20.02.00A.1020.20

▲ Maximum Reactions (lbs), or *=PLF

	Gravity		Non-Gravity		
Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL
K	670 /-	/-	/374	/205	/155
K*	57 /-	/-	/32	/-	/-
H	862 /-	/-	/295	/172	/-
L	421 /-	/-	/128	/99	/-
I	/-158				

Wind reactions based on MWFRS
K Brg Width = 3.5 Min Req = 1.5
K Brg Width = 116 Min Req = -
H Brg Width = 3.5 Min Req = 1.5
L Brg Width = 1.5 Min Req = 1.5
Bearings K, K, H, & L Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - C	604	-673

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.	Comp.	Gables	Tens.	Comp.
D - J	0	-587	F - H	178	-849

Lumber

Top chord: 2x4 SP #1;
Bot chord: 2x4 SP #1;
Webs: 2x4 SP #3;
Stack Chord: SC1 2x4 SP #1;

Plating Notes

All plates are 2X4 except as noted.

Plates sized for a minimum of 3.50 sq.in./piece.

Tray Scab(s)

(1) 2x4x11-7-11 x SP #1 scab at right end. Attach scab to face of chord with: 0.128"x3", min. nails @ 8" oc, plus additional nail clusters at: BRG.: (0), heel: (2), 1st panel point: (0).

Loading

Truss designed to support 2-0-0 top chord outlookers and cladding load not to exceed 6.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Purlins

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Shim all supports to solid bearing.

Additional Notes

See DWGS A14015ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

COA #0 278

12/03/2020

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CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

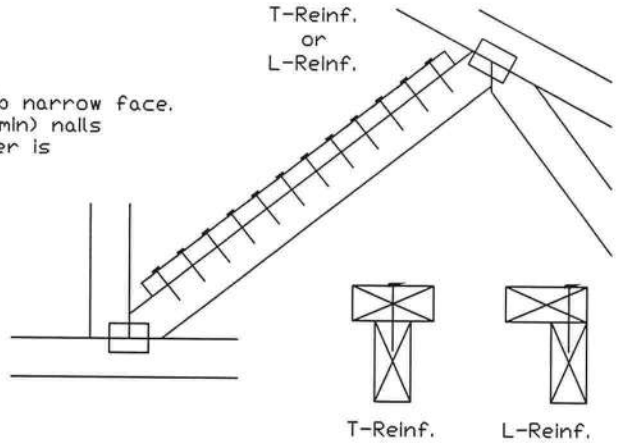
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

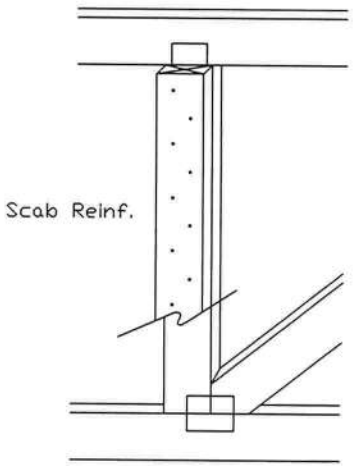
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



DOUGLAS FLEMING LICENSE No. 66848 STATE OF FLORIDA PROFESSIONAL ENGINEER 12/03/2020 #0278

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\\ Suite\ 242
\\ Earth\ City,\ MO\ 63045

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TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 01/02/19
BC DL	PSF	DRWG BRCLBSUB0119
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

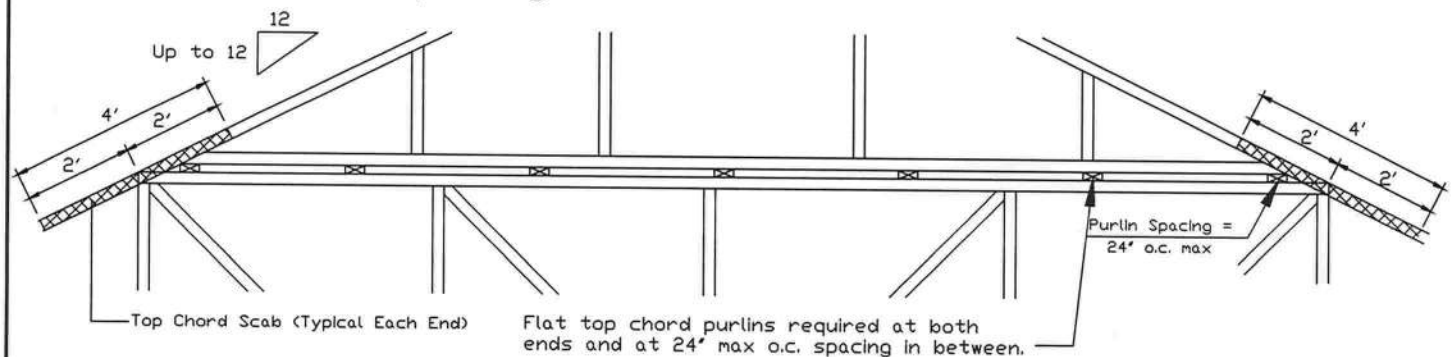
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24" o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

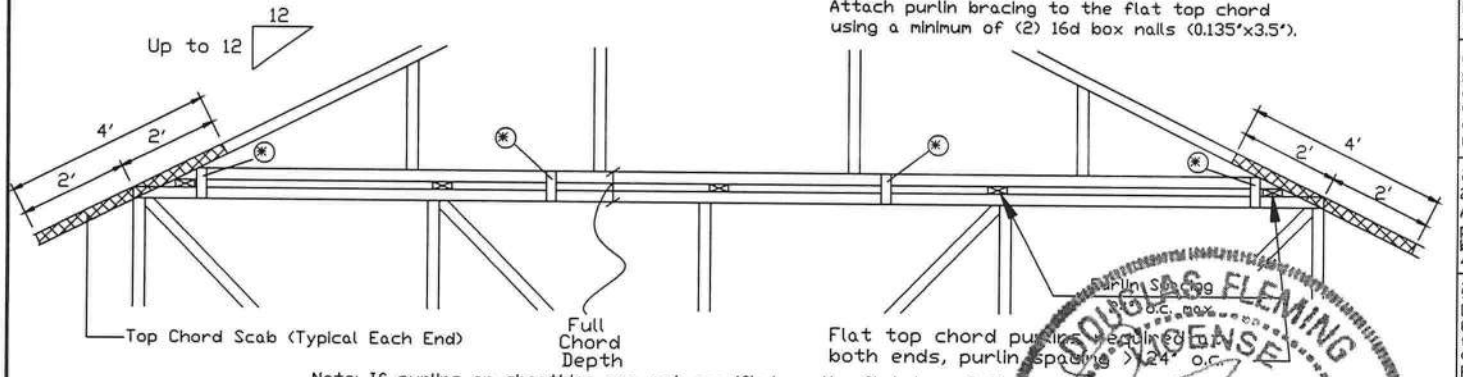
Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4" o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24" o.c. max. and use Detail A

* In addition, provide connection with one of the following methods:

Trulox

Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.

APA Rated Gusset

8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.

2x4 Vertical Scabs

2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

28PB Wave Piggyback Plate

One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

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DOUGLAS FLEMING
LICENSE
No. 86848
STATE OF
FLORIDA
PROFESSIONAL ENGINEER
12/03/2024
CA #0278

SPACING 24.0"

REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB160160118

Piggyback Detail - ASCE 7-16: 180 mph, 30' Mean Hgt, Partially Enclosed, Exp. C, Kzt=1.00

180 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Part. Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
Or 160 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Part. Enclosed Bldg. located anywhere in roof, Exp D, Wind DL= 5.0 psf (min), Kzt=1.0.

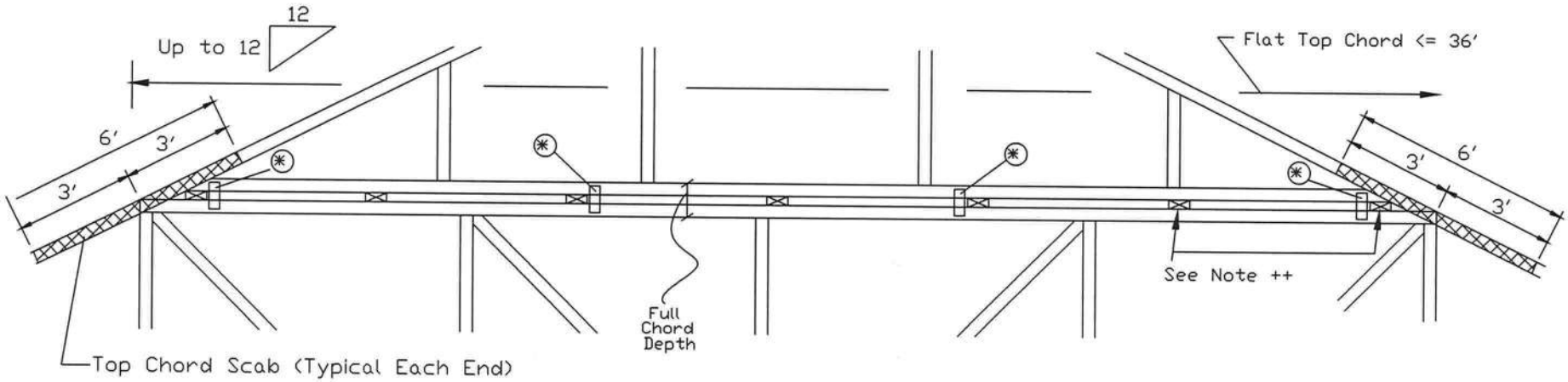
Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24" o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

++ Flat top chord purlins required at both ends and at a maximum of 24' intervals unless otherwise noted on base truss design drawing. Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").



* In addition, provide connection with one of the following methods:	
Trulox Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.	28PB Wave Piggyback Plate One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.
APA Rated Gusset 8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.	2x4 Vertical Scabs 2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.

DOUGLAS DEMING
LICENSE
No. 66848
STATE OF FLORIDA
PROFESSIONAL ENGINEER
12/03/2020
COA #0 278

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\ \ Earth\ City,\ MO\ 63045

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REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB180160118
SPACING	24.0"

Cracked or Broken Member Repair Detail

This drawing specifies repairs for a truss with broken chord or web member.

This design is valid only for single ply trusses with 2x4 or 2x6 broken members. No more than one break per chord panel and no more than two breaks per truss are allowed. Contact the truss manufacturer for any repairs that do not comply with this detail.

(B) = Damaged area, 12" max length of damaged section
(L) = Minimum nailing distance on each side of damaged area (B)
(S) = Two 2x4 or two 2x6 side members, same size, grade, and species as damaged member. Apply one scab per face. Minimum side member length(s) = (2)(L) + (B)

Scab member length (S) must be within the broken panel.

Nail into 2x4 members using two (2) rows at 4" o.c., rows staggered.
Nail into 2x6 members using three (3) rows at 4" o.c., rows staggered.

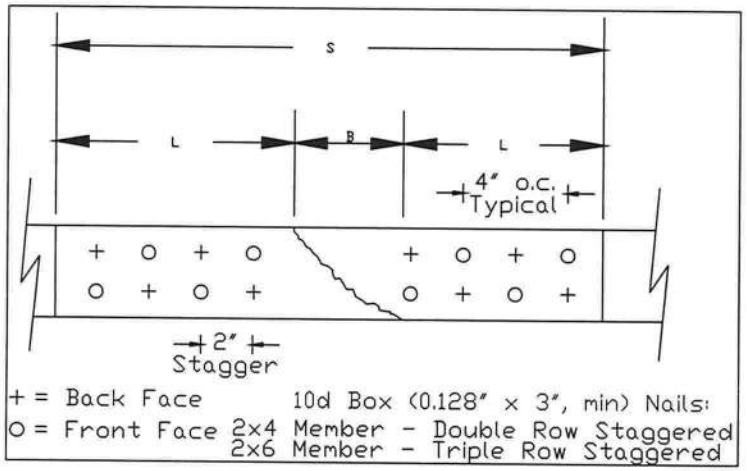
Nail using 10d box or gun nails (0.128"x3", min) into each side member.

The maximum permitted lumber grade for use with this detail is limited to Visual grade #1 and MSR grade 1650f.

This repair detail may be used for broken connector plate at mid-panel splices.

This repair detail may not be used for damaged chord or web sections occurring within the connector plate area.

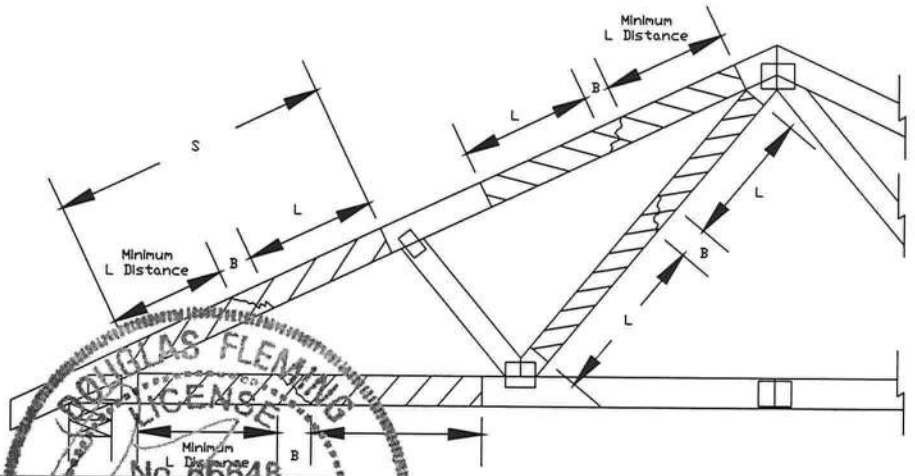
Broken chord may not support any tie-in loads.



Nail Spacing Detail

Load Duration = 0%
Member forces may be increased for Duration of Load

Member	Size	L	Maximum Member Axial Force SPF-C	HF	DF-L	SYP
Web Only	2x4	12"	620#	635#	730#	800#
Web Only	2x4	18"	975#	1055#	1295#	1415#
Web or Chord	2x4	24"	975#	1055#	1495#	1745#
Web or Chord	2x6		1465#	1585#	2245#	2620#
Web or Chord	2x4	30"	1910#	1960#	2315#	2555#
Web or Chord	2x6		2230#	2365#	3125#	3575#
Web or Chord	2x4	36"	2470#	2530#	2930#	3210#
Web or Chord	2x6		3535#	3635#	4295#	4745#
Web or Chord	2x4	42"	2975#	3045#	3505#	3835#
Web or Chord	2x6		4395#	4500#	5225#	5725#
Web or Chord	2x4	48"	3460#	3540#	4070#	4445#
Web or Chord	2x6		5165#	5280#	6095#	6660#



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SPACING 24.0" MAX

REF	MEMBER REPAIR
DATE	10/01/14
DRWG	REPCHRD1014

Gable Stud Reinforcement Detail

ASCE 7-10: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
		#3	4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		Stud	4' 1"	6' 7"	7' 0"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		Standard	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"
	SP DFL	#1	4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
		Stud	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
		Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 2"	11' 0"	11' 10"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) 'L' braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

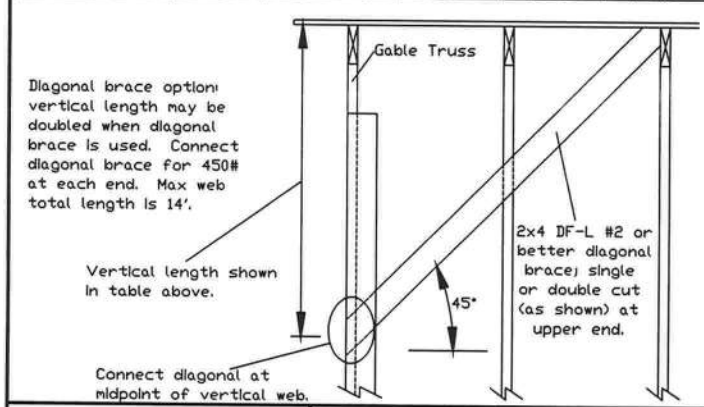
'L' bracing must be a minimum of 80% of web member length.

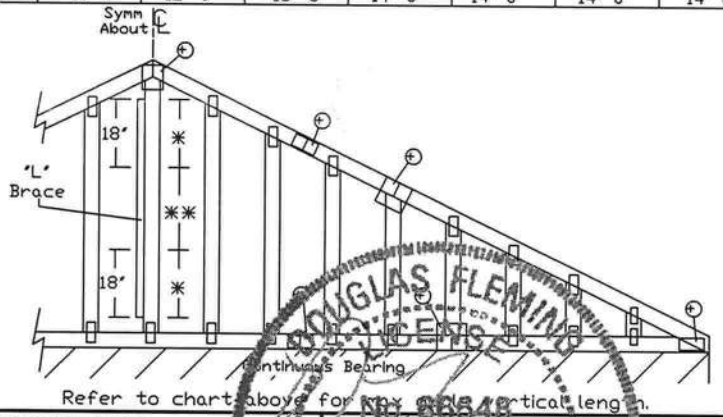
Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.





DOUGLAS FLEMING
No. 66848
STATE OF FLORIDA
PROFESSIONAL ENGINEER
2/03/2020
COA #6278

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WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLER.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec2.

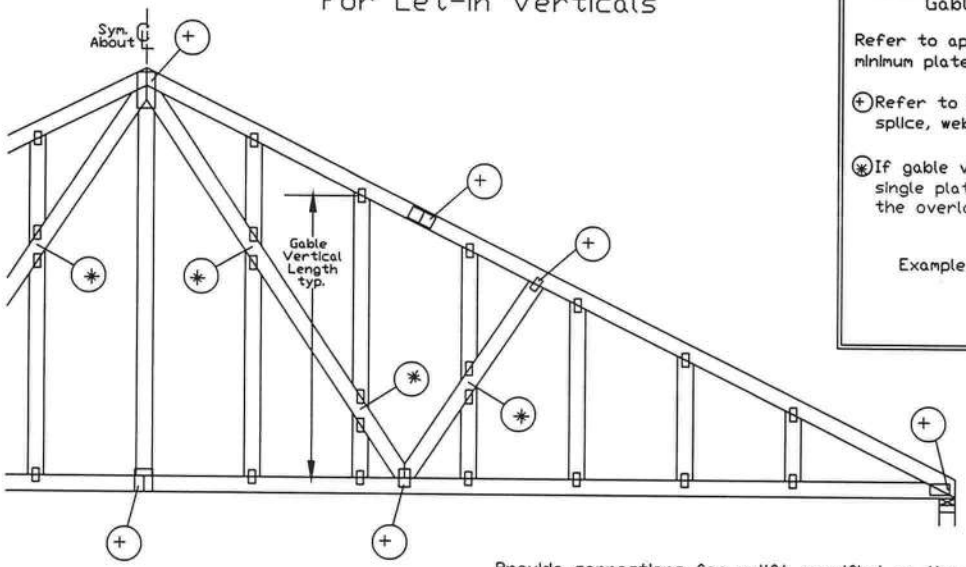
For more information see this Job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.org; ICC: www.iccsafe.org

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-10-GAB14015
DATE	10/01/14
DRWG	A14015ENC101014

Gable Detail
For Let-in Verticals



Gable Truss Plate Sizes

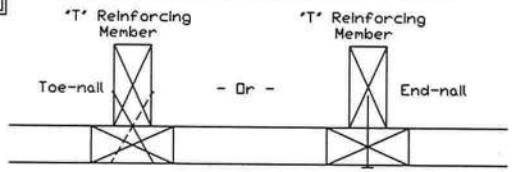
Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

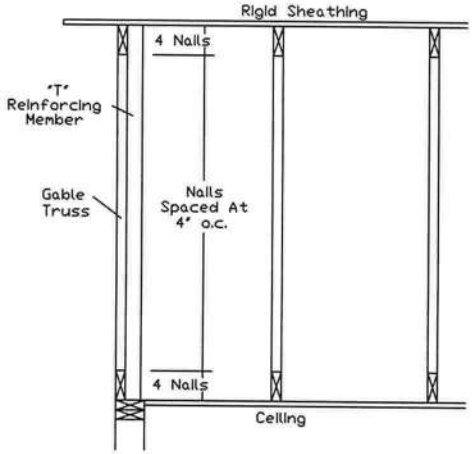
⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

✱ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example: 2X4 2X4 2X8

'T' Reinforcement Attachment Detail





Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:
10d Common (0.148"x 3",min) Nails at 4" o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4" o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118, A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118, A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118, A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118, S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118, S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118, S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118, S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.

To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ "T" Brace

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24'o.c. SP #3
'T' Reinforcing Member Size = 2x4
'T' Brace Increase (From Above) = 30% = 1.30
(1) 2x4 'L' Brace Length = 8' 7"
Maximum 'T' Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

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For more information see this job's general notes page and these web sites:
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DUGGAN FLEMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER
12/03/2020

MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"

REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118