





& REAR ELEVATIONS

WALL

SOFTPIAN ARCHITECTURAL DESIGN SOFTWARE

Weath ACTING, GIBRALTAR

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JOB NUMBER 20220216

SHEET NUMBER A.1

OF 4 SHEETS



Weath, Lake City Florida

CONTRACTING

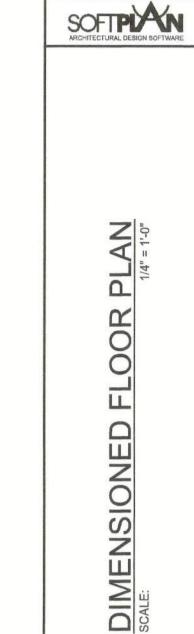
SOFTPIAN ARCHITECTURAL DESIGN SOFTWARE

ELEVATIONS

1/4" = 1'-0"

JOB NUMBER 20220216 SHEET NUMBER

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RALTAR CONTRACTING, LLC.

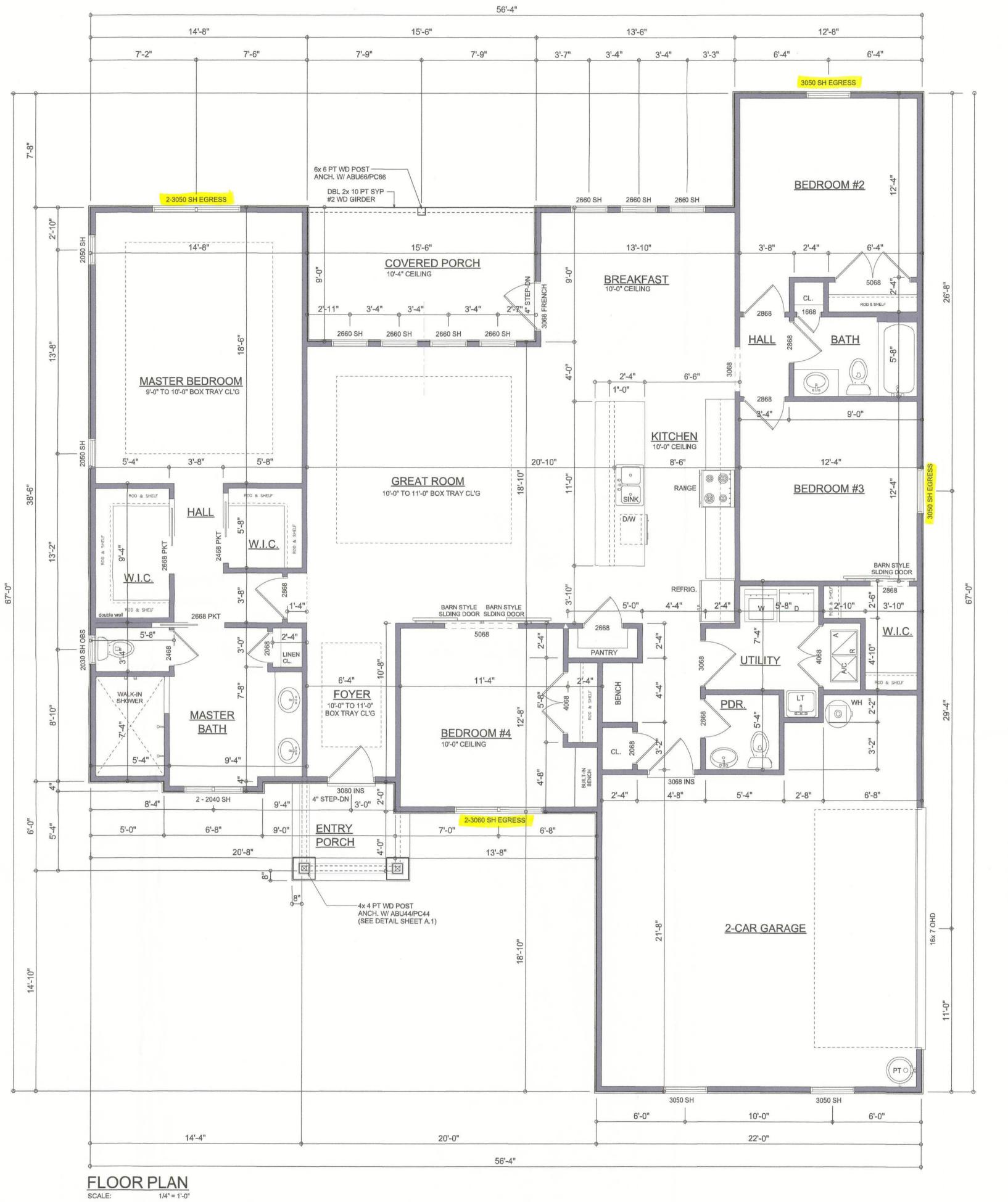
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JOB NUMBER 20220216

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WILL CATY OF 4 SHEETS



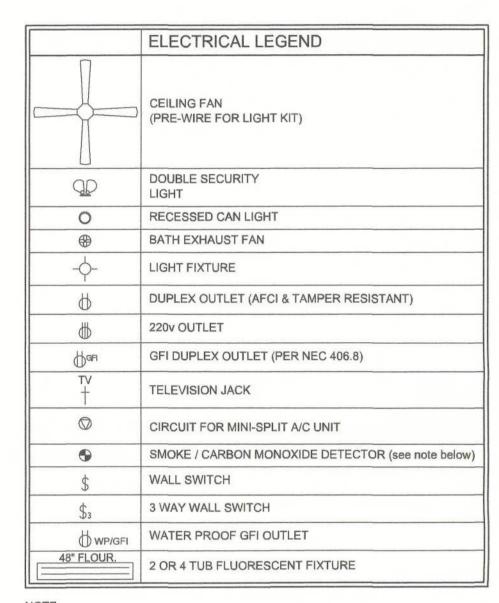
ALL CEILINGS SHALL BE 9'-0" UNLESS OTHERWISE NOTED

Garage fire separations shall comply with the following:

- 1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 13/8 inches (34.9 mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted.
- 2. Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.
- 3. A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.
- 4. When installing an attic access and/or pull-down stair unit in the garage, devise shall have a minimum 20 min. fire rating.

AREA SUMMARY

LIVING AREA	2,105	S.F.
GARAGE AREA	512	S.F.
ENTRY PORCH AREA	50	S.F.
COVERED PORCH AREA	140	S.F.
TOTAL AREA	2,807	S.F.



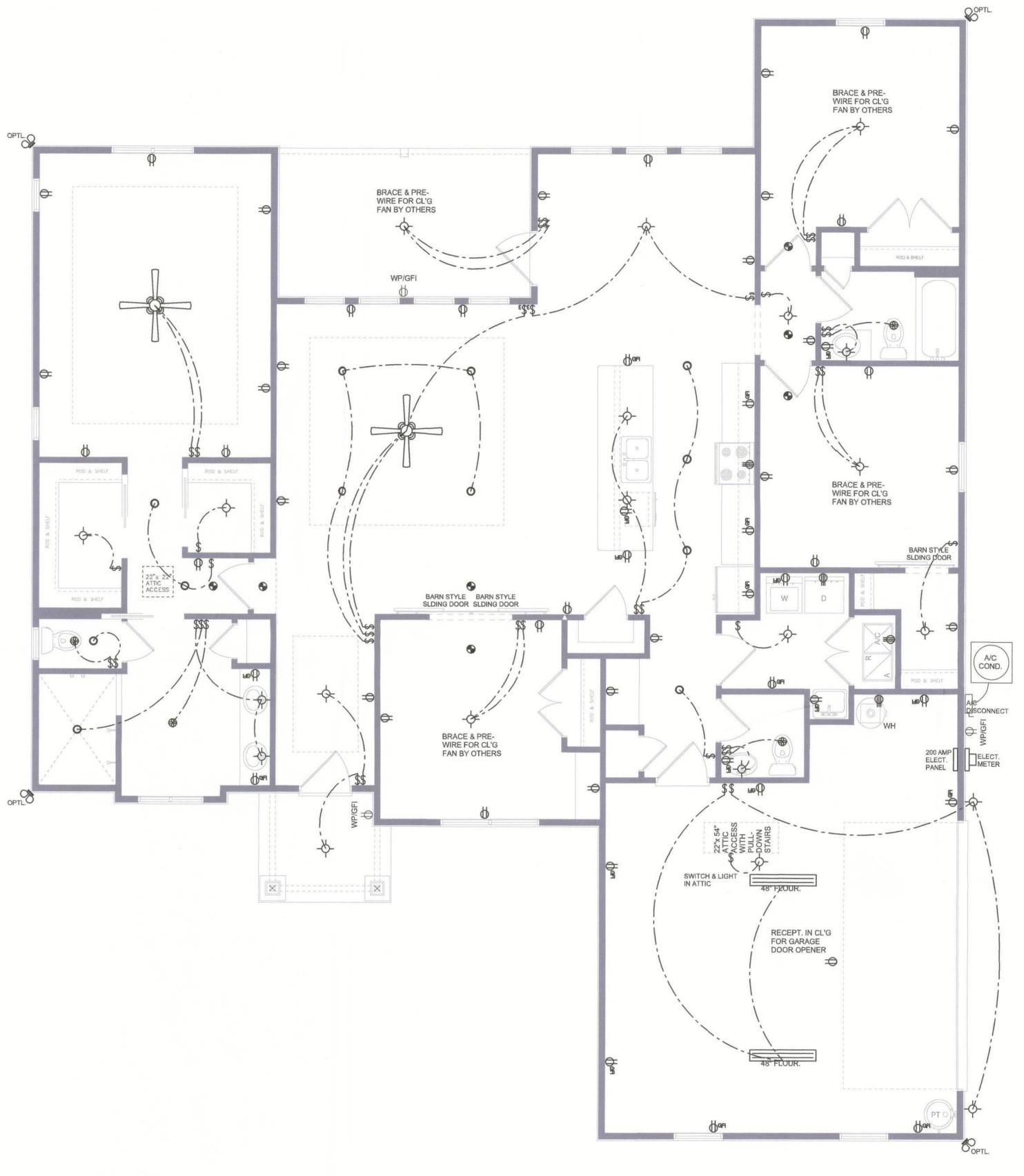
NOTE: ALL INTERIOR RECEPTACLES SHALL BE AFCI (ARC FAMULT CIRCUIT INTERRUPT) PER NEC 210.12 & TAMPER RESISTANT PER

ALL INTERIOR & EXTERIOR LIGHTING SHALL MEET OR EXCEED THE MIN. 75% HIGH-EFFICIENCY LIGHTING PER FBC-ENERGY CONSERVATION R404.

ALL SMOKE DETECTORS BE A COMBO SMOKE & CARBON MONOXIDE DETECTOR AND SHALL HAVE BATTERY BACKUP POWER AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY ALL ACTIVATE.

THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS. CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR SHALL BE USED AS AN EQUIPMENT GROUND.

IT IS THE LICENSED ELECTRICAL CONTRACTORS RESPONSIBILITY TO INSURE THAT ALL WORK PERFORMED AND EQUIPMENT INSTALLED MEETS OR EXCEEDS THE 2017 (NFPA-70) NATIONAL ELECTRIC CODE AND ALL OTHER LOCAL CODES AND ORDINANCES.

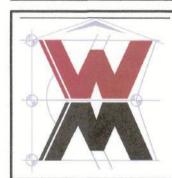


ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

SOFTPIAN ARCHITECTURAL DEPUNIO

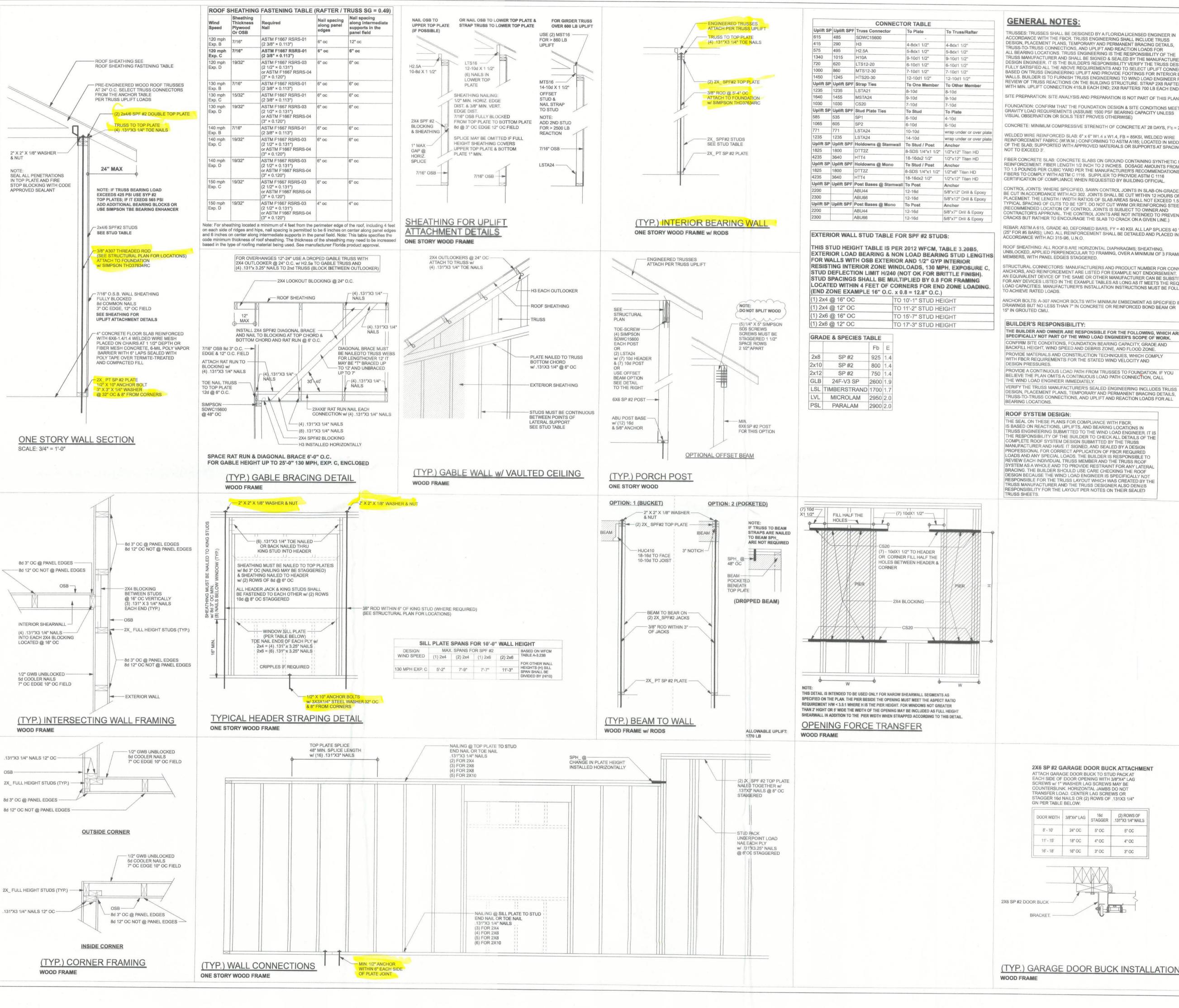
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GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS

BER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND YPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL IREINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOF'S ARE HORIZONTAL DIAPHRAGMS: SHEATHING UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED OAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED O ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN RAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPIEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS; FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED OADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

2X6 SP #2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD PACK AT

EACH SIDE OF DOOR OPENING WITH 3/8"X4" LAG

STAGGER 16d NAILS OR (2) ROWS OF .131X3 1/4"

DOOR WIDTH 3/8"X4" LAG 16d (2) ROWS OF STAGGER .131"X3 1/4" NAILS

5" OC

5" OC

4" OC

3" OC

SCREWS w/ 1" WASHER LAG SCREWS MAY BE

COUNTERSUNK. HORIZONTAL JAMBS DO NOT

TRANSFER LOAD, CENTER LAG SCREWS OF

24" OC

11' - 15' 18" OC 4" OC

16' - 18' 16" OC 3" OC

8' - 10'

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution

DESIGN CRITERIA & LOADS:

CODE FOR DESIGN LOADS

(BUILDER MUST FIELD VERIFY)

(BUILDER MUST FIELD VERIFY)

ENCLOSURE CLASSIFICATION INTERNAL PRESSURE

C&C DESIGN PRESSURES SEE TABLE

SOIL BEARING CAPACITY 1500 PSF

TOPOGRAPHIC FACTOR

FLORIDA BUILDING CODE RESIDENTIAL

ASCE 7-16

-45 DEGREES

40 PSF LIVE LOAD

30 PSF LIVE LOAD

20 PSF LIVE LOAD

16 PSF LIVE LOAD 12 PSF LIVE LOAD

COMPONENT & CLADING DESIGN PRESSURES 120 MPH (EXP C)-

+22.6(Vasd) -25.5(Vasd)

+21.7(Vasd) -24.1(Vasd)

GARAGE DOOR DESIGN PRESSURES 120 MPH (EXP C)-

THIS BUILDING IS NOT IN THE FLOOD ZONE

+25.6(Vasd) -27.8(Vasd) +25.6(Vasd) -34.2(Vasd) +42.6(Vult) -46.2(Vult) +42.6(Vult) -57(Vult)

END 4' FROM ALL

OUTSIDE CORNER

BUILDING CODE

WINDLOADS

BASIC WIND SPEED

WIND EXPOSURE

RISK CATEGORY

COEFFICIENT

MEAN ROOF HEIGHT

FLOOR LOADING

ROOMS OTHER THAN

SLEEPING ROOM

4:12 TO < 12:12

12:12 & GREATER

FLOOD ZONE

WIND AREA (FT2)

9x7 GARAGE DOOR

16x7 GARAGE DOOR

SLEEPING ROOMS

ROOF LOADING FLAT OR < 4:12

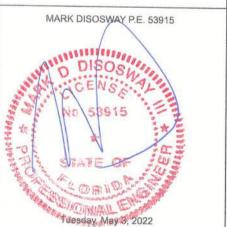
ROOF ANGLE

(ASCE 7-10, 3S GUST)

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

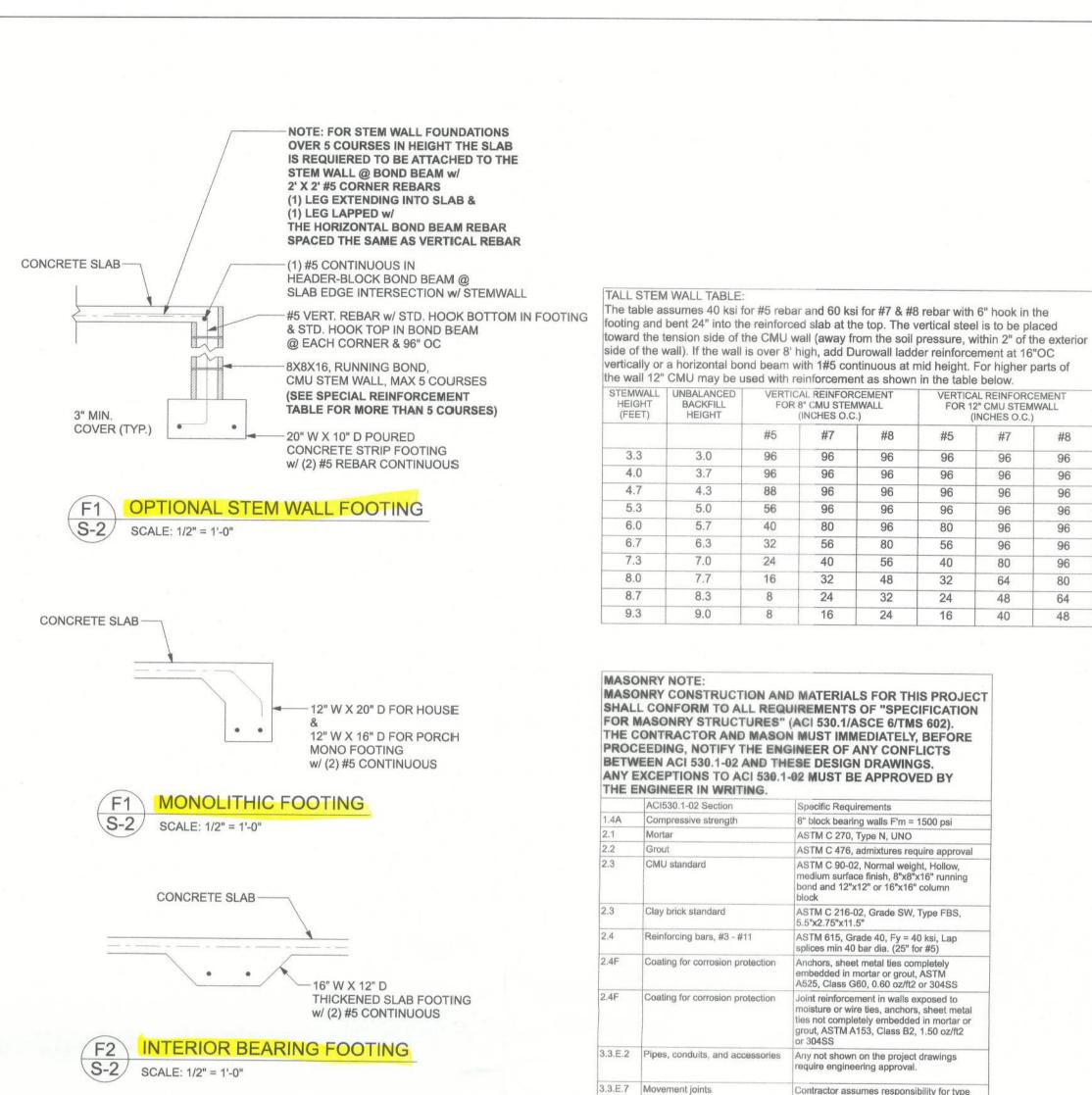


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> JOB NUMBER: 220531 **S-1**

> > OF 3 SHEETS

Mark Disosway P.E.



S-2 SCALE: 1/2" = 1'-0"

4" - 6" CURB (optional)

S-2 SCALE: 1/2" = 1'-0"

S-2 SCALE: 1/2" = 1'-0"

6" MIN. WIDE

F4 MONOLITHIC CURB FOOTING

-12" W X 16" D + CURB

w/ (2) #5 CONTINUOUS

MONO FOOTING

CONCRETE SLAB

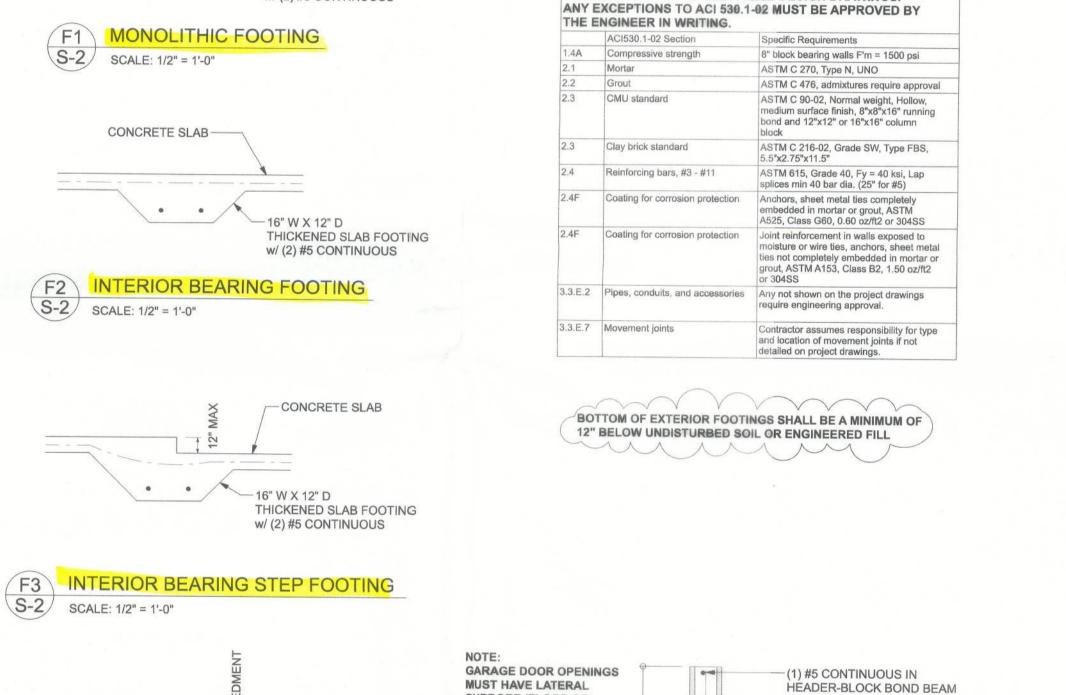
-CONCRETE SLAB

-GARAGE DOOR POCKET

MONO FOOTING w/ (2) #5 CONTINUOUS

• • 12" W X 12" D MIN.

GARAGE DOOR POCKET FOOTING



#5 VERT. REBAR

CMU STEM WALL

OPTIONAL STEM WALL CURB FOOTING

w/ STD HOOK IN FOOTING

@ EACH CORNER & 48" OC

-8X8X16, RUNNING BOND,

-20" W X 10" D POURED

CONCRETE STRIP FOOTING

w/ (2) #5 REBAR CONTINUOUS

SUPPORT (FLOOR OR

CONCRETE SLAB -

PERPENDICULAR WALL)

WITHIN 3'-0" OF OPENING

@ TOP OF RETAINING WALL

COVER (TYP.)

S-2 SCALE: 1/2" = 1'-0"

4.0

9.0

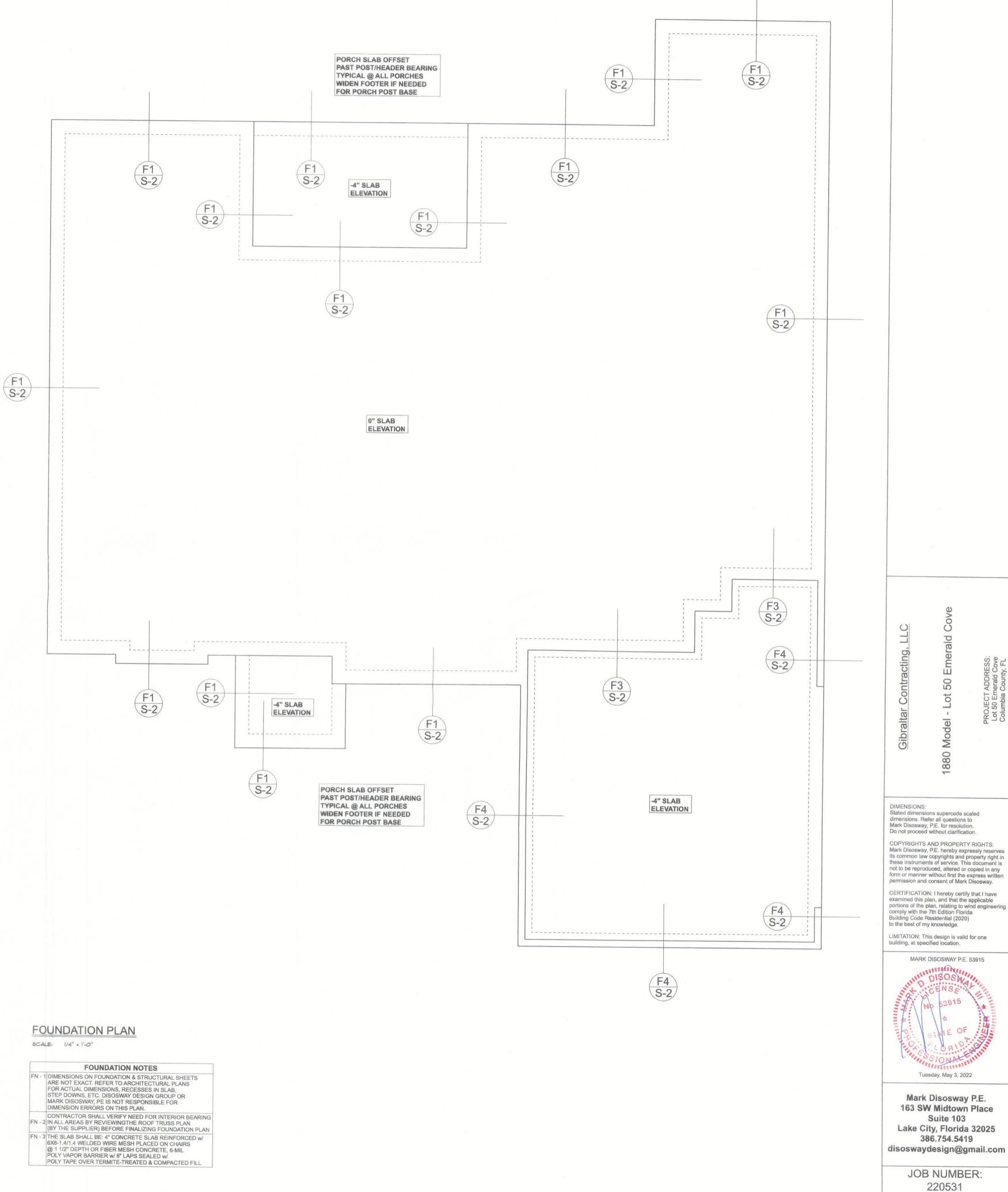
VERTICAL REINFORCEMENT

FOR 8" CMU STEMWALL

VERTICAL REINFORCEMENT

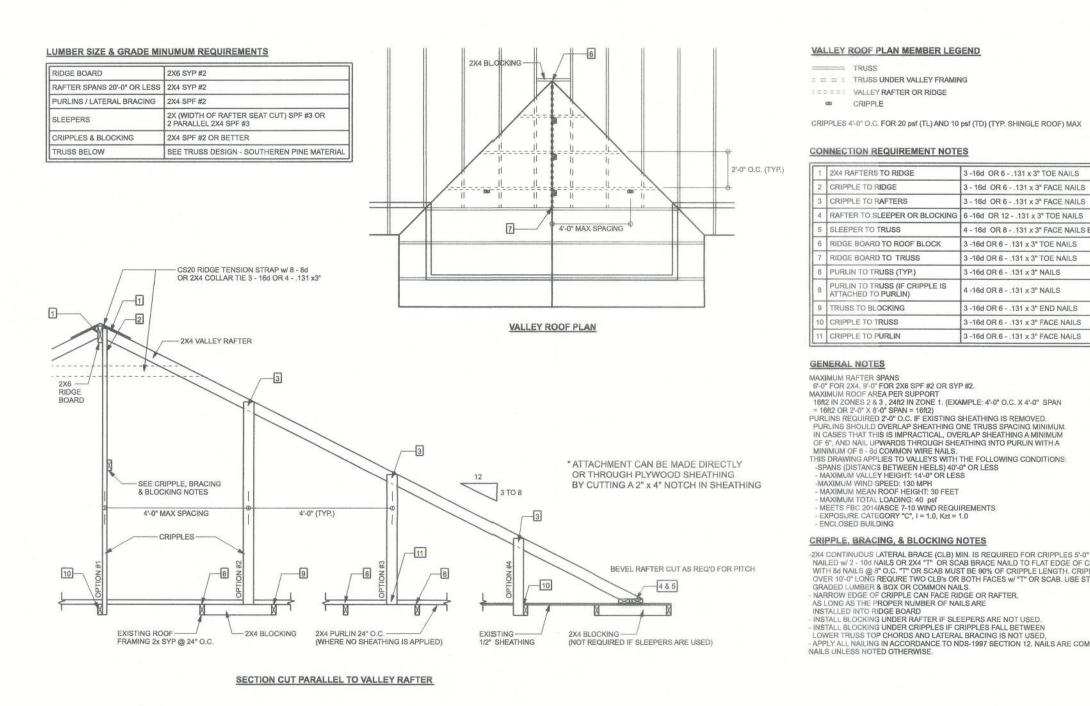
FOR 12" CMU STEMWALL (INCHES O.C.)

#5 #7 #8



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> **S-2** OF 3 SHEETS



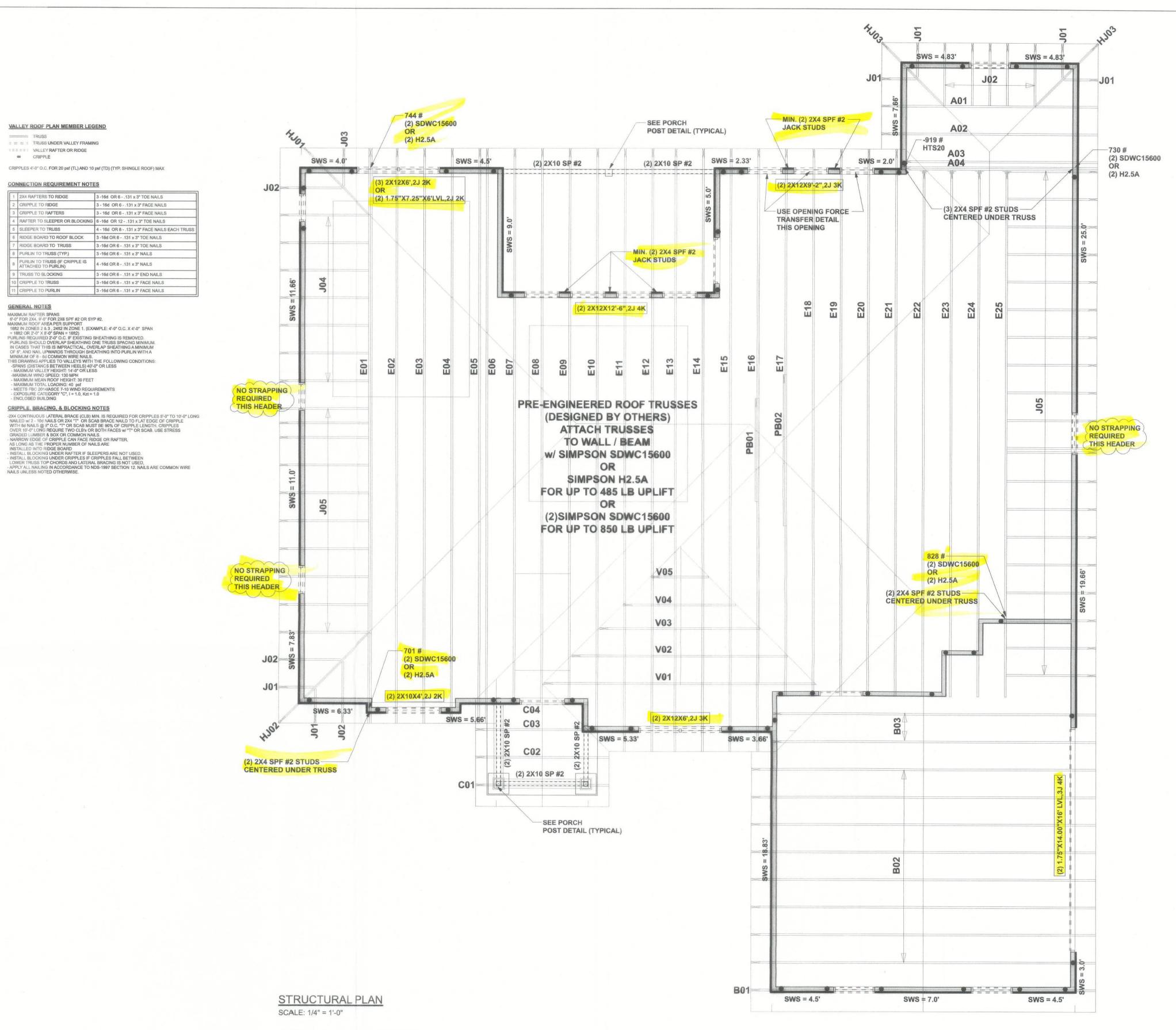
ROOF OVER FRAMING & BRACING DETAIL SCALE: N.T.S

2X4 RAFTERS TO RIDGE

TRUSS TO BLOCKING

CRIPPLE TO PURLIN

3 -16d OR 6 - .131 x 3" END NAILS



STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)

ALL LOAD BEARING FRAME WALL HEADERS SN-2 SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD

DIMENSIONS ON STRUCTURAL SHEETS SN-4 ARE NOT EXACT. REFER TO ARCHITECTURAL

FLOOR PLAN FOR ACTUAL DIMENSIONS PERMANENT TRUSS BRACING IS TO BE INSTALLED AT

LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3, BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND

(2) 2X10X0',1J 1K HEADER/BEAM CALL-OUT (U.N.O.) A A A A A - NUMBER OF KING STUDS (FULL LENGTH) - NUMBER OF JACK STUDS (UNDER HEADER) SPAN OF HEADER - SIZE OF HEADER MATERIAL NUMBER OF PLIES IN HEADER

THREADED ROD LEGEND

- INDICATES LOCATION OF: 3/8" A307 ALL THREADED ROD

ACTUAL vs REQUIRED SHEARWALL LONGITUDUNAL TRANSVERSE

35592 LBF 17841 LBF REQUIRED 13063 LBF 15147 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. W.B. HOWLAND TRUSS CO. JOB #21-5391

Mark Disosway P.E. 163 SW Midtown Place Suite 103 Lake City, Florida 32025 386.754.5419 disoswaydesign@gmail.com

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MARK DISOSWAY P.E. 53915

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> JOB NUMBER: 220531

> > **S-3** OF 3 SHEETS