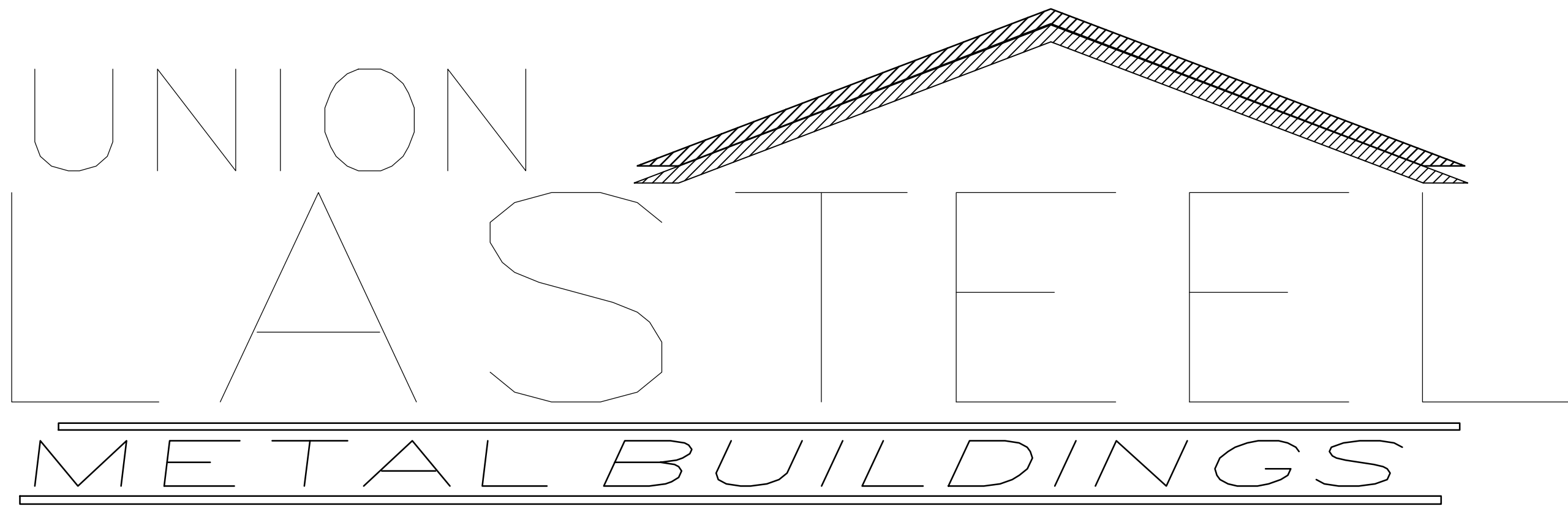




PO BOX 537 Lake Bulter, FL 32054
(386) 496-0681 (386) 496-4705 (fax)

BUILDING LOADS / DESCRIPTION:

WIDTH: 30 LENGTH: 62 HEIGHT: 19 /24
(BUILDING DIMENSIONS ARE NOMINAL. REFER TO PLANS).

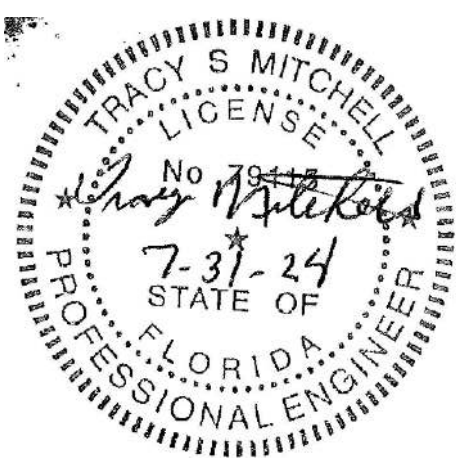
THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED
AND APPLIED AS REQUIRED BY : FBC 23

THE CONTRACTOR IS TO CONFIRM THAT THESE LOADS COMPLY
WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

ROOF DEAD LOAD:	2.00	PSF (ROOF PANELS & PURLINS)			
COLLATERAL LOAD:	1.00	PSF	SNOW EXPOSURE:	1.0000	
ROOF LIVE LOAD:	20.00	PSF	WIND EXPOSURE:	B	
ROOF SNOW LOAD:	0.00	PSF	INTERNAL PRESSURE COEFF.:		
GROUND SNOW LOAD:	0	PSF	0.55	/	-0.55
BASIC WIND SPEED:	130	MPH	SPECTRAL RESPONSE COEFF.		MAPPED SPECTRAL RESPONSE ACC.
SEISMIC ZONE:	B		Sds	0.10	Ss 0.09
THERMAL FACTOR:	1.00		Sd1	0.08	St 0.05
IMPORTANCE FACTORS:			DESIGN BASE SHEAR, V:		
WIND LOAD	1.00	EXPANDED FORMULA	0.667*Ie*Fa*Ss*W/R		
SNOW LOAD	1.0000	LONGITUDINAL	0.49		
SEISMIC LOAD	1.00	TRANSVERSE	0.48		

GENERAL NOTES:

- 1) MATERIALS : MINIMUM YIELD:
- | | |
|------------------------|-----------------------|
| HOT ROLLED BAR | Fy = 50.0000 ksi MIN. |
| STRUCTURAL STEEL SHEET | Fy = 55.0000 ksi MIN. |
| STRUCTURAL STEEL PLATE | Fy = 50.0000 ksi MIN. |
| COLD FORMED SHAPES | Fy = 57.0000 ksi MIN. |
| WALL SHEETING | Fy = 60.0000 ksi MIN. |
| ROOF SHEETING | Fy = 60.0000 ksi MIN. |
| BOLTS | A307 & A325 |
- THE METAL BUILDING MANUFACTURER RESERVES THE RIGHT TO
SUBSTITUTE THE ABOVE MATERIALS WITH EQUAL OR BETTER MATERIAL.
- 2) BOLT TIGHTENING REQUIREMENTS:
- ALL HIGH STRENGTH BOLTS ARE A325 UNLESS NOTED OTHERWISE.
HIGH STRENGTH BOLTS SHALL BE TIGHTENED BY THE TURN OF THE NUT METHOD
IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR
STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". A325 BOLTS SHALL BE
INSTALLED WITH OUT WASHERS WHEN TIGHTENED BY THE "TURN OF THE NUT"
METHOD. ALL BOLTED CONNECTIONS, FOR SHEAR/BEARING CONNECTION TYPE
WITH BOLT THREADS EXCLUDED FROM THE SHEAR PLANE SHALL BE SNUG TIGHT
ONLY.
- 3) ALL STRUCTUAL STEEL TO RECEIVE A RUST INHIBITIVE PRIMER. THIS PAINT
IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS.



ROOF PANELS:

COLOR: Galvalume Plus

WALL PANELS:

COLOR: Need Sill Poly Color

TRIM COLORS:

CABLE: Need Sill Poly Color

CORNER: Need Sill Poly Color

EAVE: Need Sill Poly Color

FRAMED OPENINGS: Need Sill Poly Color

LINER PANELS:

COLOR: N/A

LINER TRIM:

COLOR: N/A

DEFLECTION LIMTS:

EW COL:	180
EW RAF LIVE:	180
EW RAF WIND:	180
WALL GIRT:	90
PURL LIVE:	180
PURL WIND:	150
WALL PANEL:	60
ROOF PANEL LIVE:	60
ROOF PANEL WIND:	60
RF HORIZONTAL:	60
RF VERTICAL:	180
WIND BENT:	60
RF CRANE:	60
RF SEIS:	50
WIND BENT SEIS:	50

IMPORTANT NOTE: FINAL DETAILING, FABRICATION, AND DELIVERY DATE OF THIS PROJECT
CANNOT BE COMPLETED UNTIL THE SIGNED APPROVALS ARE RETURNED TO THE METAL
BUILDING MANUFACTURER.

SHEET 1 OF 13

1992 Bascom Norris Drive
Lake City, FI 32055

△		
△		
△		
△		
△	07/31/24	FOR APPROVAL
REV.	DATE	REVISION

PURCHASER: New Millennium
PROJECT: Pre-Engineered Steel Building
JOB NUMBER: NewMillennium30x62

BUILDER / CONTRACTOR RESPONSIBILITIES

IT IS THE RESPONSIBILITY OF THE BUILDER/CONTRACTOR TO INSURE THAT ALL PROJECT
PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY
GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND
DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN
AGREEMENT THAT THE METAL BUILDING SYSTEM MANUFACTURER OR ITS DESIGN ENGINEER IS
ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION
PROJECT.

THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE
APPROPRIATE AGENCY AS REQUIRED. APPROVAL OF THE METAL BUILDING SYSTEM
MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE METAL BUILDING
SYSTEM MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE
CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.2.1 AISC CODE OF STANDARD
PRACTICES, 9TH ED.) WHERE DISCREPANCIES EXIST BETWEEN THE METAL BUILDING SYSTEM
MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE
STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE
9TH ED.)

DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT
FURNISHED BY THE METAL BUILDING SYSTEM MANUFACTURER ARE THE RESPONSIBILITY OF
THE CONTRACTORS AND ENGINEERS OTHER THAN THE METAL BUILDING SYSTEM
MANUFACTURER'S ENGINEER UNLESS SPECIFICALLY INDICATED.

THE CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK
IN COMPLIANCE WITH THE METAL BUILDING SYSTEM MANUFACTURER "FOR CONSTRUCTION"
DRAWINGS.

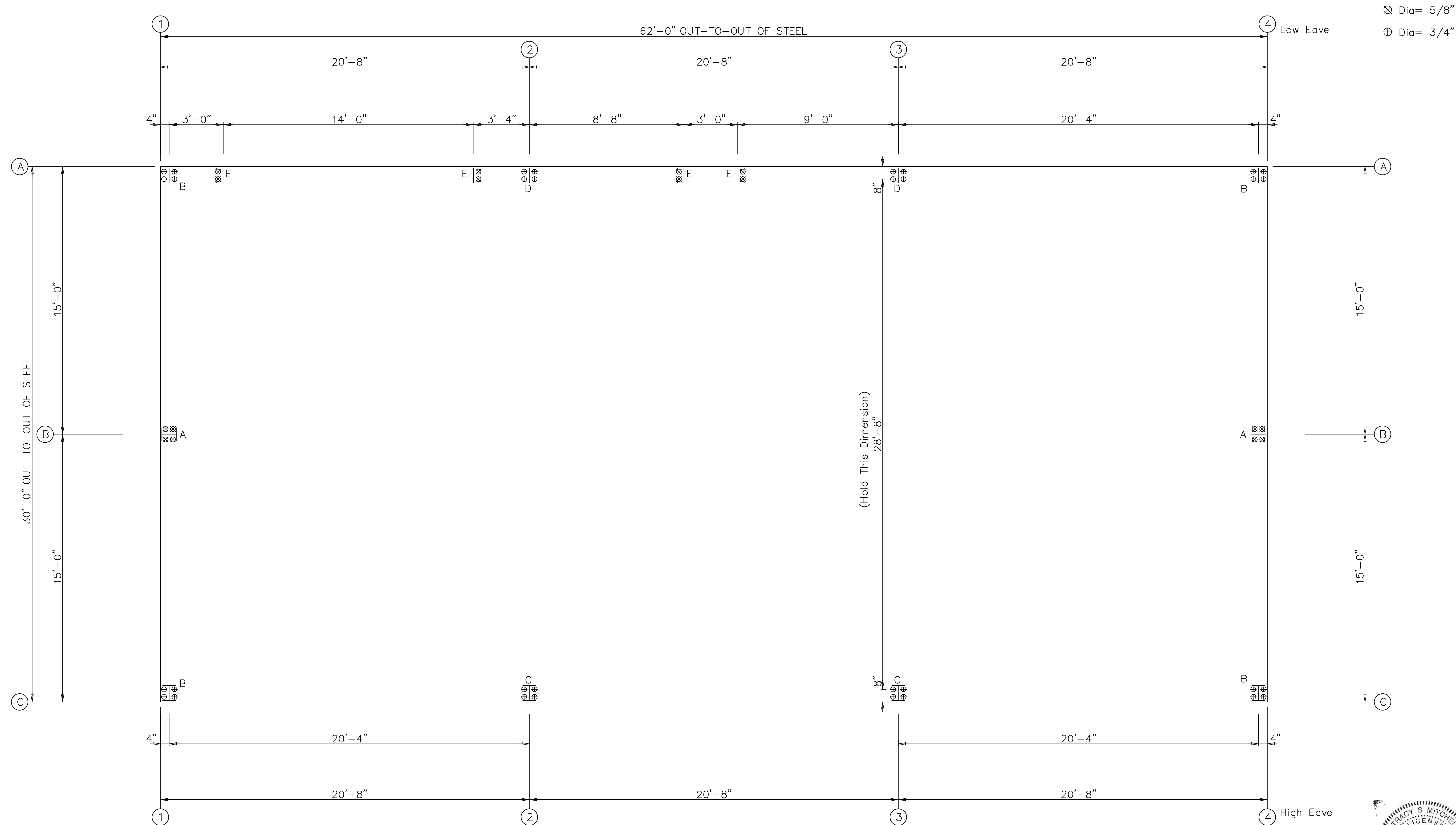
ALL BRACING AS SHOWN AND PROVIDED BY THE METAL BUILDING SYSTEM MANUFACTURER
FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A
PERMANENT PART OF THE STRUCTURE.

TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR
OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND
FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE
THE STEEL FRAMING, OR ANY PARTLY ASSEMBLIED STEEL FRAMING, AGAINST LOADS
COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING
FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING
FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH
UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION, OR COLLISION. (SECT. 7.9.1
AISC CODE OF STANDARD PRACTICE, 9TH ED.)

WARNING: IN NO CASE SHOULD GALVALUME STEEL PANELS BE USED IN CONJUNCTION
WITH LEAD OR COPPER. BOTH LEAD AND COPPER HAVE HARMFUL CORROSION EFFECTS ON
THE ALUMINUM ZINC ALLOY COATING WHEN THEY ARE USED IN CONTACT WITH GALVALUME
STEEL PANELS. EVEN RUN-OFF FROM COPPER FLASHING, WIRING, OR TUBING ONTO
GALVALUME SHOULD BE AVOIDED.

APPROVAL NOTES

THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS
APPROVAL DRAWINGS: IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS BE MADE
IN CONTRASTING INK (PREFERABLY RED INK), HAVE ALL INSTANCES OF CHANGE CLEARLY
INDICATED, AND BE LEGIBLE AND UNAMBIGUOUS. A SIGNATURE AND DATE IS REQUIRED ON
ALL PAGES. MANUFACTURER RESERVES THE RIGHT TO RE-SUBMIT DRAWINGS WITH EXTENSIVE
OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION. THIS MAY IMPACT THE
DELIVERY SCHEDULE. APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE
METAL BUILDING SYSTEM MANUFACTURER HAS CORRECTLY INTERPRETED THE CONTRACT
REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN WITH
INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED BY
MANUFACTURER. ANY CHANGES NOTED ON THHE DRAWINGS NOT IN COMFORMANCE WITH THE
TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER
ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED
AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION.
MANUFACTURER RECONGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING
APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED.
HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTURAL
TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILIAR INDICATION
OF APPROVAL, DISAPPROVAL, ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS
BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS
UNACCEPTABLE ALTERNATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE
CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS
CUSTOMER.



⊗ Dia= 5/8"
⊕ Dia= 3/4"

ANCHOR BOLT PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)



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Union LaSteel				
PROJECT	Pre-Engineered Steel Building	ANCHOR BOLT PLAN		
ID	NewMillennium30x62	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	1992 Bascom Norris Drive Lake City, FL 32055	DATE: 7/25/24	SHEET 2 OF 13	

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft)	=	30.0
Length (ft)	=	62.0
Eave Height (ft)	=	19.0/ 24.0
Roof Slope (rise/12)	=	2.0
Dead Load (psf)	=	2.0
Collateral Load (psf)	=	1.0
Roof Live Load(psf)	=	20.0
Frame Live Load(psf)	=	12.0
Wind Speed (mph)	=	130.0
Wind Code	=	FBC 23 (IBC 18)
Exposure	=	B
Closure	=	Partially Enclosed
Importance Wind	=	1.00
Importance Seismic	=	1.00
Seismic Zone	=	B
Seismic Coeff (Fa*Ss)	=	0.15

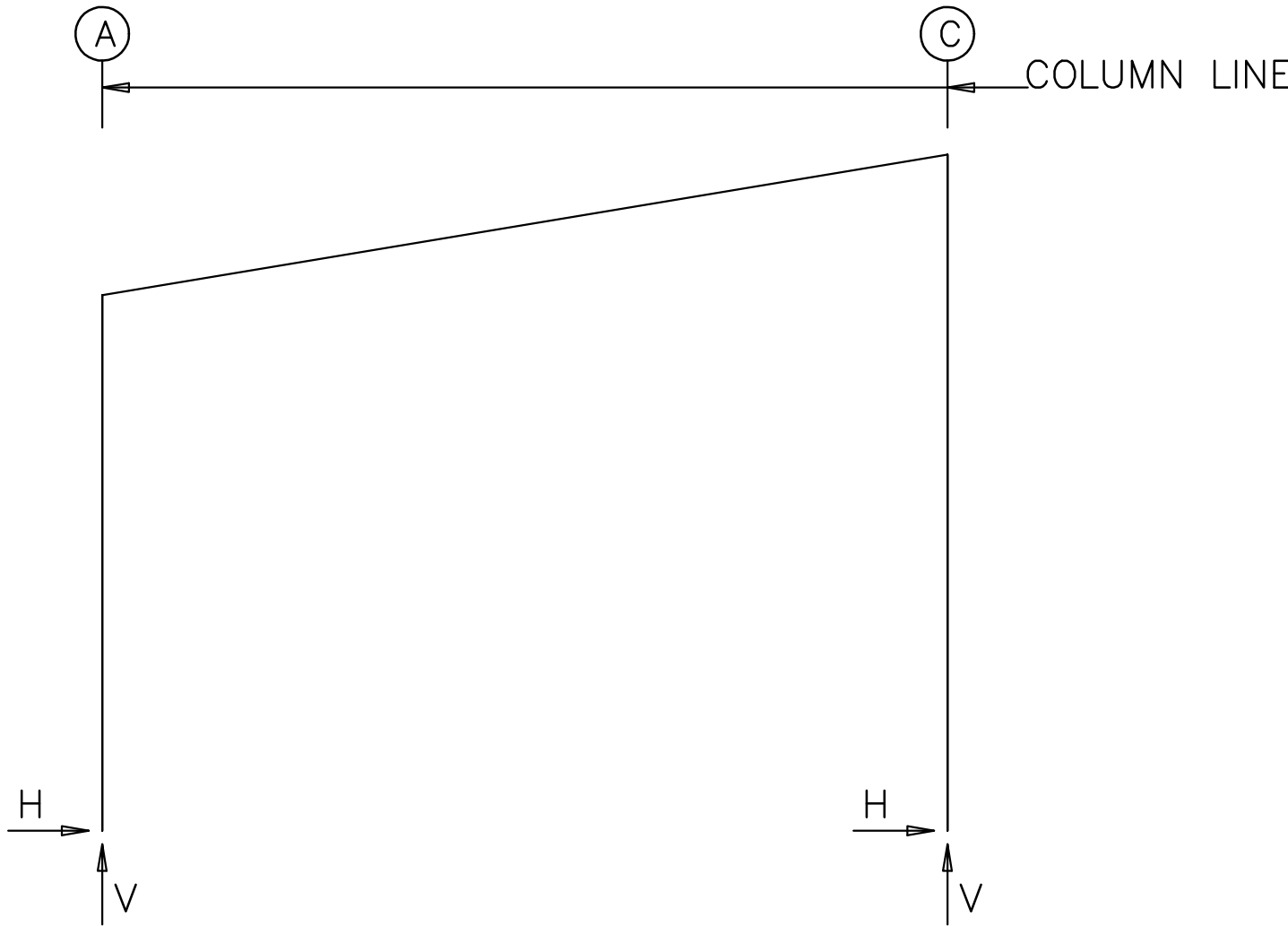
ID	Description
1	Dead+Collateral+Live
2	Dead+0.6Wind_Left1
3	Dead+0.6Wind_Right1
4	Dead+Collateral+0.75Live+0.45Wind_Right2
5	0.6Dead+0.6Wind_Left1
6	0.6Dead+0.6Wind_Right1
7	0.6Dead+0.6Wind_Left2
8	0.6Dead+0.6Wind_Right2
9	0.6Dead+0.6Wind_Long1L
10	0.6Dead+0.6Wind_Left1+0.6Wind_Suction
11	0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
12	Dead+Collateral+0.75Live+0.45Wind_Left2+0.45Wind_Suction
13	0.6Dead+0.6Wind_Right1+0.6Wind_Suction

BUILDING BRACING REACTIONS

Wall		Col Line	± Reactions(k)				Panel Shear (lb/ft)	
Loc	Line		Wind		Seismic		Wind	Seis
			Horz	Vert	Horz	Vert		
L_EW	1						112	10
F_SW	C	3,4	3.3	3.6	0.8	0.8		
R_EW	4						112	10
B_SW	A	4,3	3.0	2.5	0.7	0.6		

Reactions for seismic represent shear force, Eh

FRAME LINES: 2 3



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in) Qty Dia		Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
2*	A	3	4.0	-0.9	7	-4.1	-1.1	4	0.750	8.000	12.00	0.375	0.0
		4	1.7	6.6	9	2.3	-6.5						
2*	C	8	4.5	-0.9	2	-2.8	-3.3	4	0.750	8.000	10.38	0.375	0.0
		1	-0.8	5.5	9	-1.9	-7.4						
2*	Frame lines: 2 3												

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Collateral		Live		Wind_Left1		Wind_Right1		Wind_Left2	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2*	A	0.1	1.1	0.1	0.3	0.6	3.7	-2.7	-11.8	6.5	-3.4	-6.9	-3.0
2*	C	-0.1	1.5	-0.1	0.3	-0.6	3.8	-4.4	-7.9	3.5	-11.7	-0.1	0.8
Frame Line	Column Line	Wind_Right2		Wind_Long1		Wind_Long2		Seismic_Left		Seismic_Right		Seismic_Long	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
2*	A	2.4	5.4	3.7	-11.9	3.7	-10.0	-0.1	-0.1	0.1	0.1	0.0	-0.6
2*	C	7.6	-3.0	-3.1	-13.9	-3.9	-10.7	-0.1	0.1	0.1	-0.1	0.0	-0.8
2*	Frame lines: 2 3												

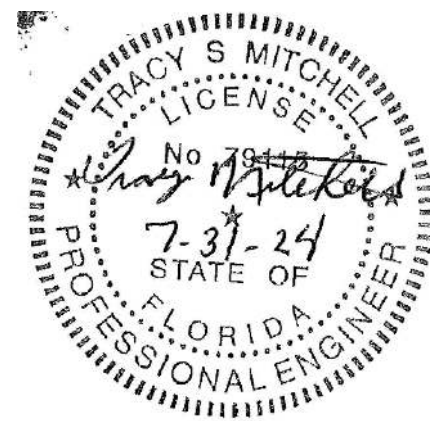
ENDWALL COLUMN REACTIONS(k)

Frame Line	Col Line	Load Id			
1	B	Max Horz	=	2.6	Vert = -3.3 (10)
1	B	Max +Vert	=	4.8	Horz = 0.0 (1)
1	B	Max -Vert	=	-3.3	Horz = 2.6 (10)

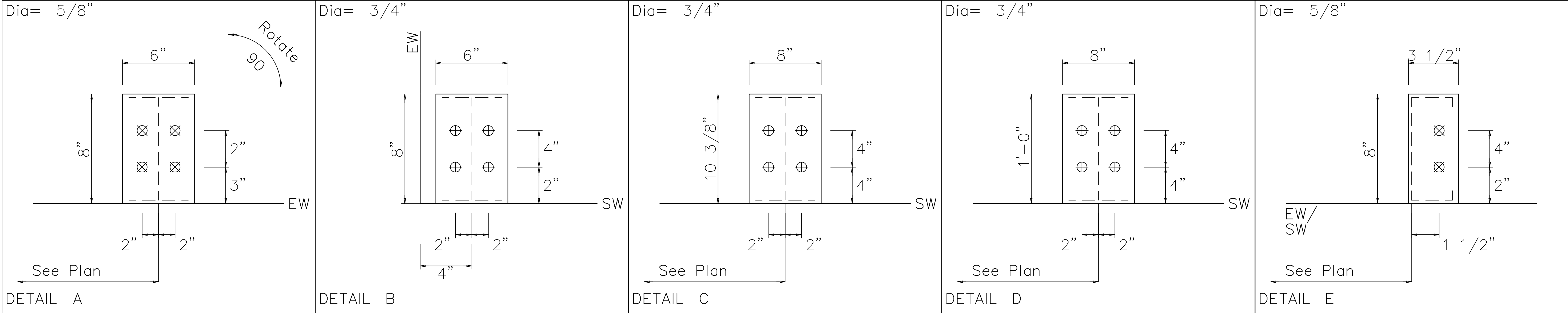
(1)Dead+Collateral+Live
(10)0.6Dead+0.6Wind_Left1+0.6Wind_Suction

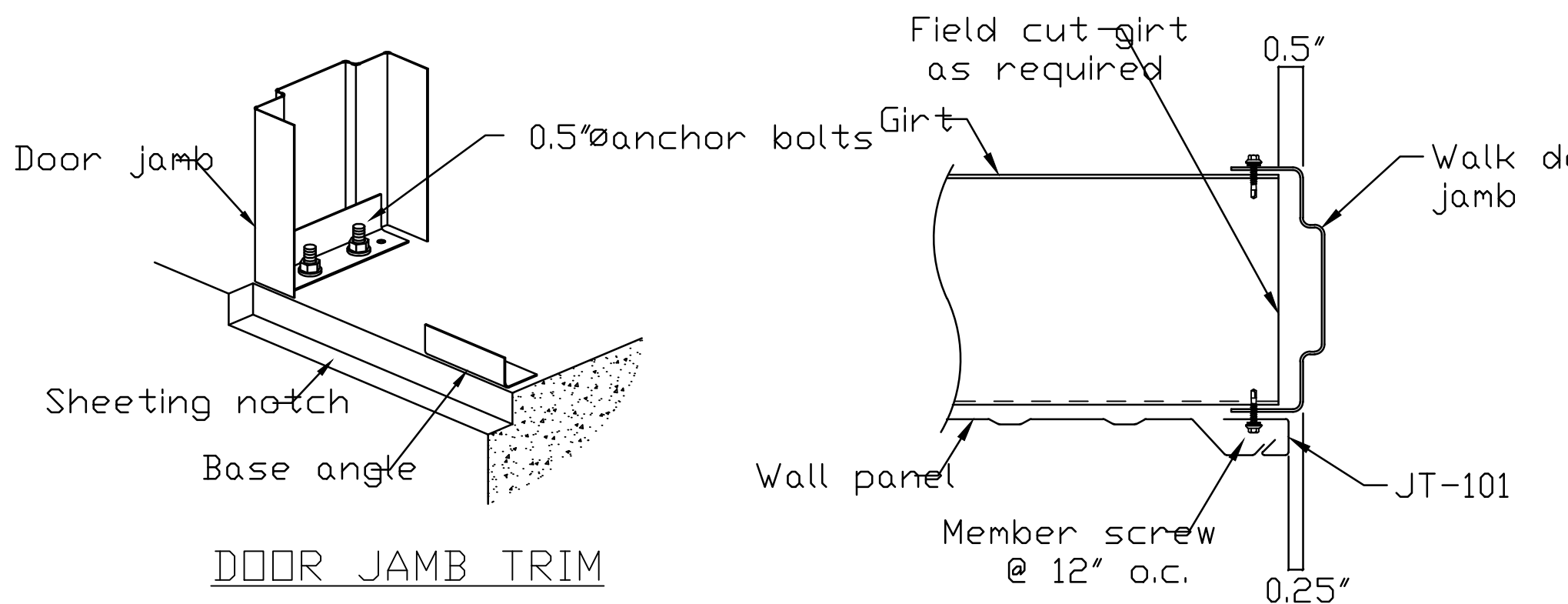
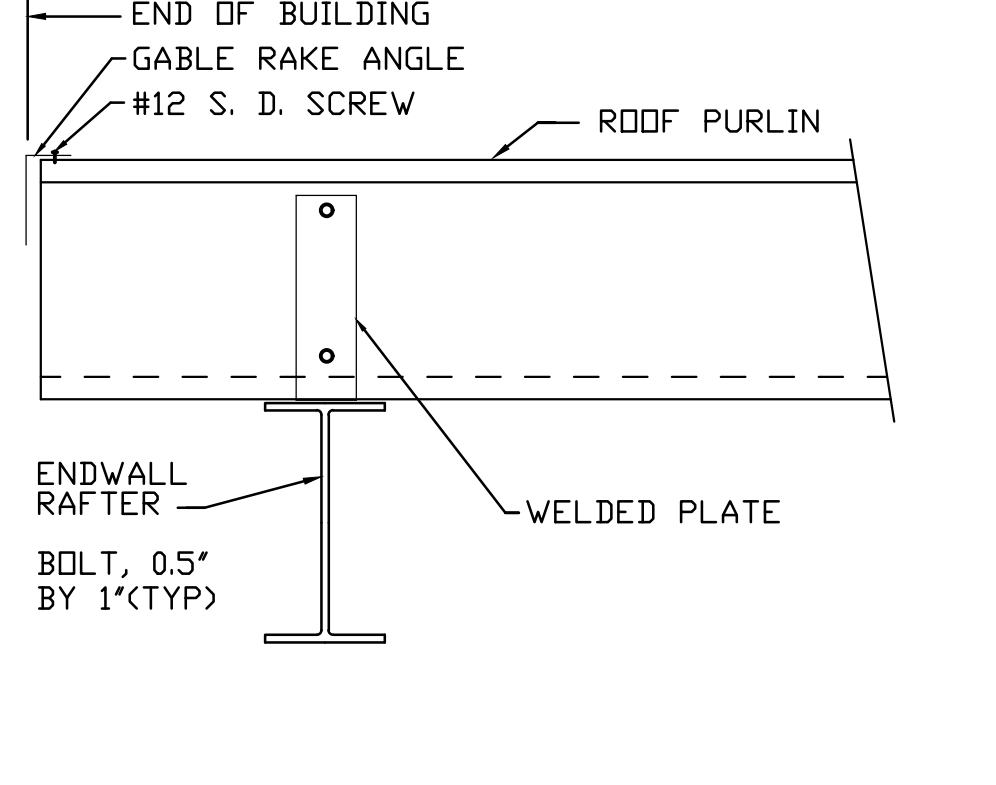
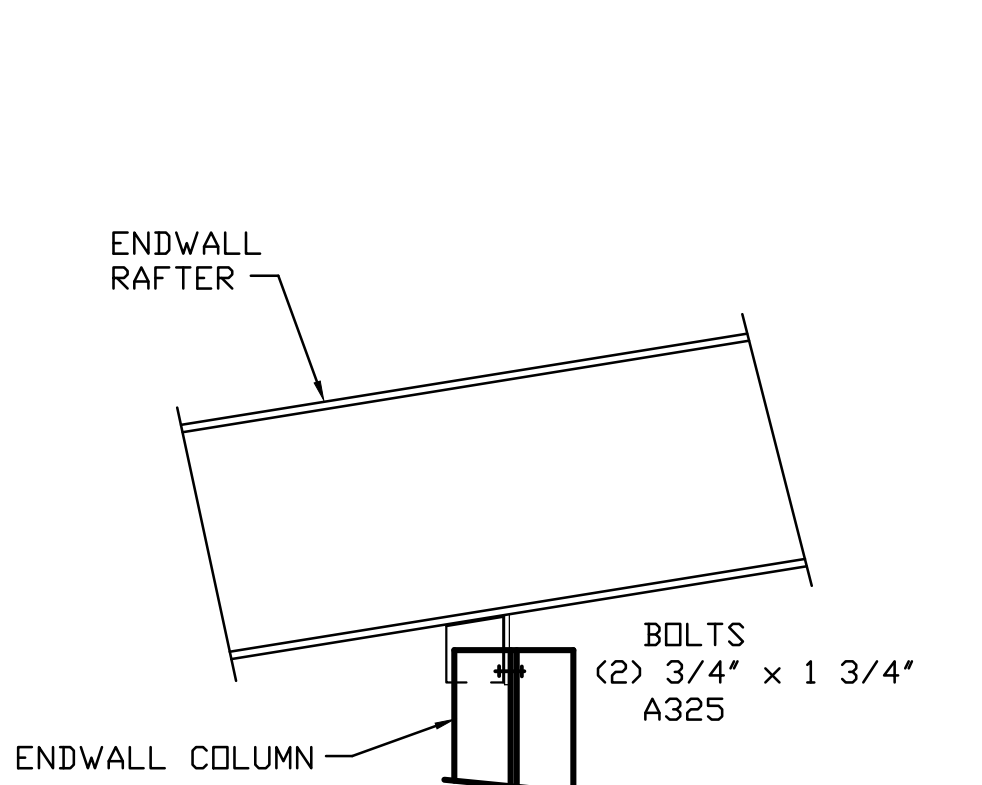
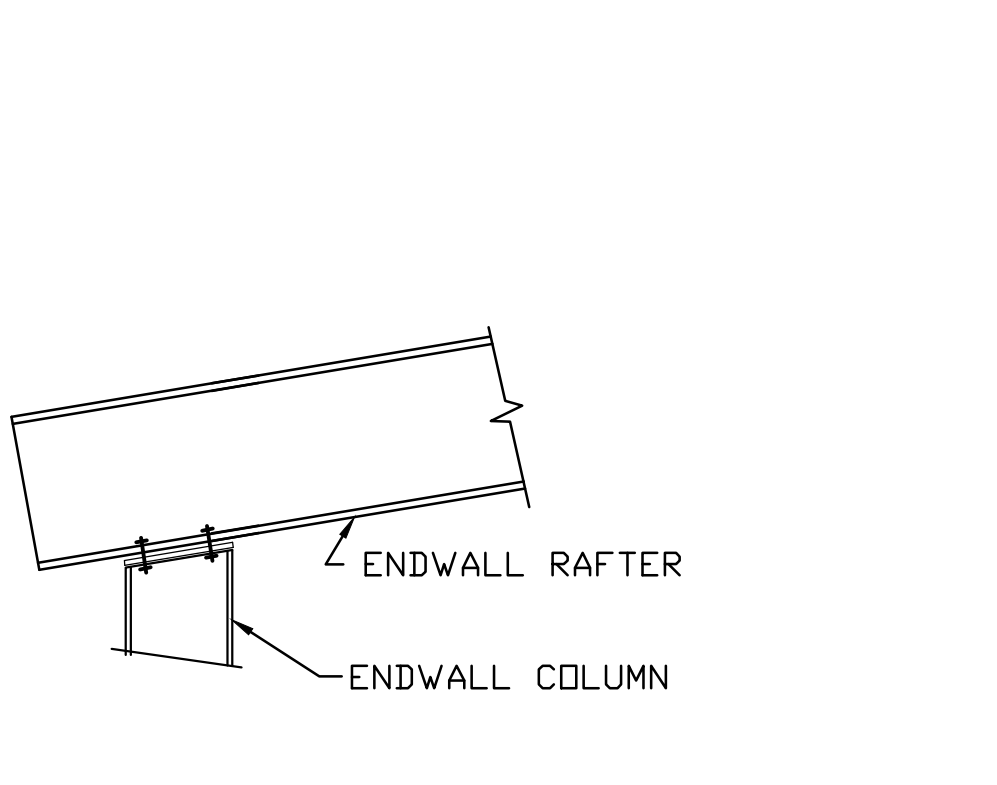
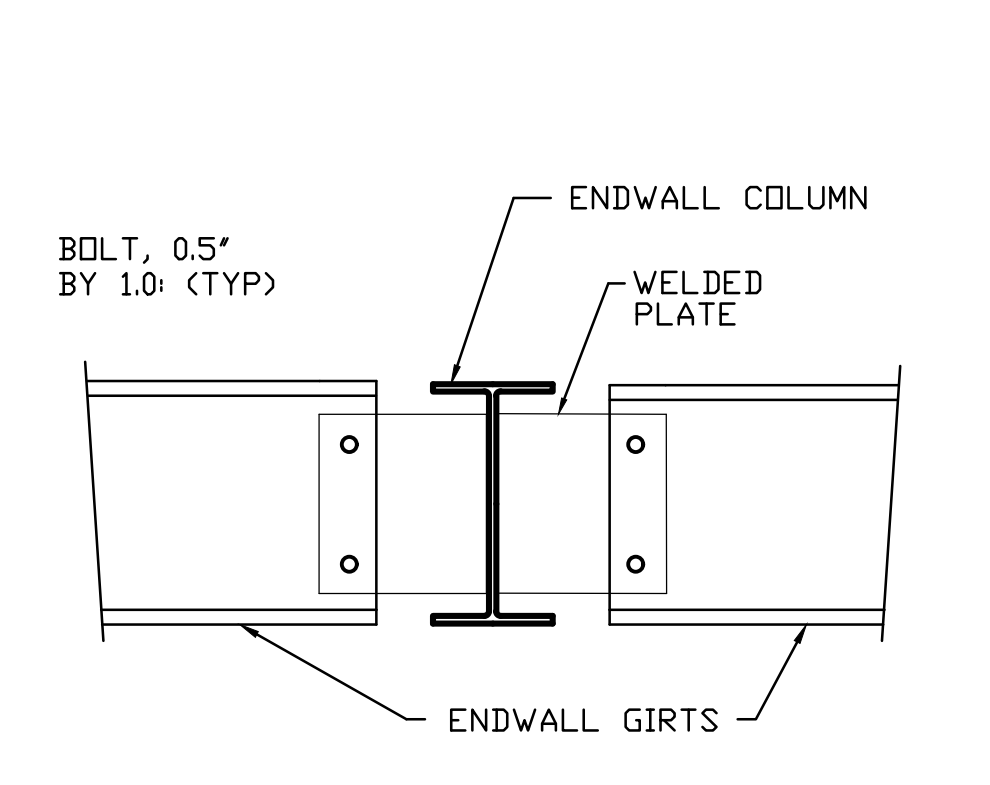
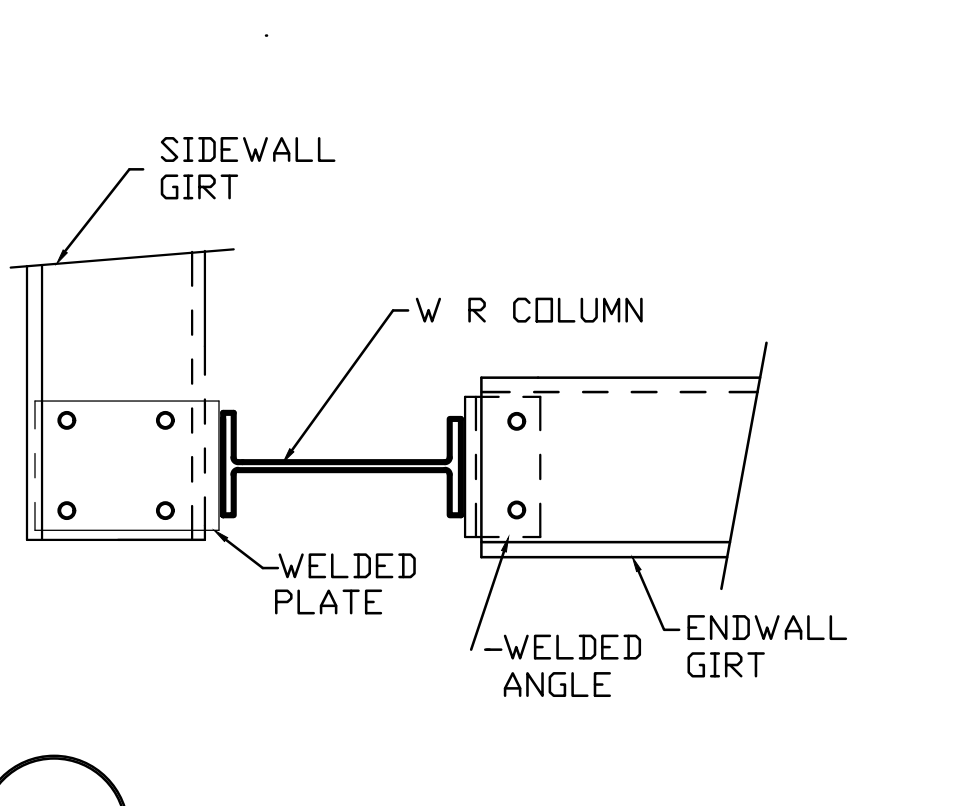
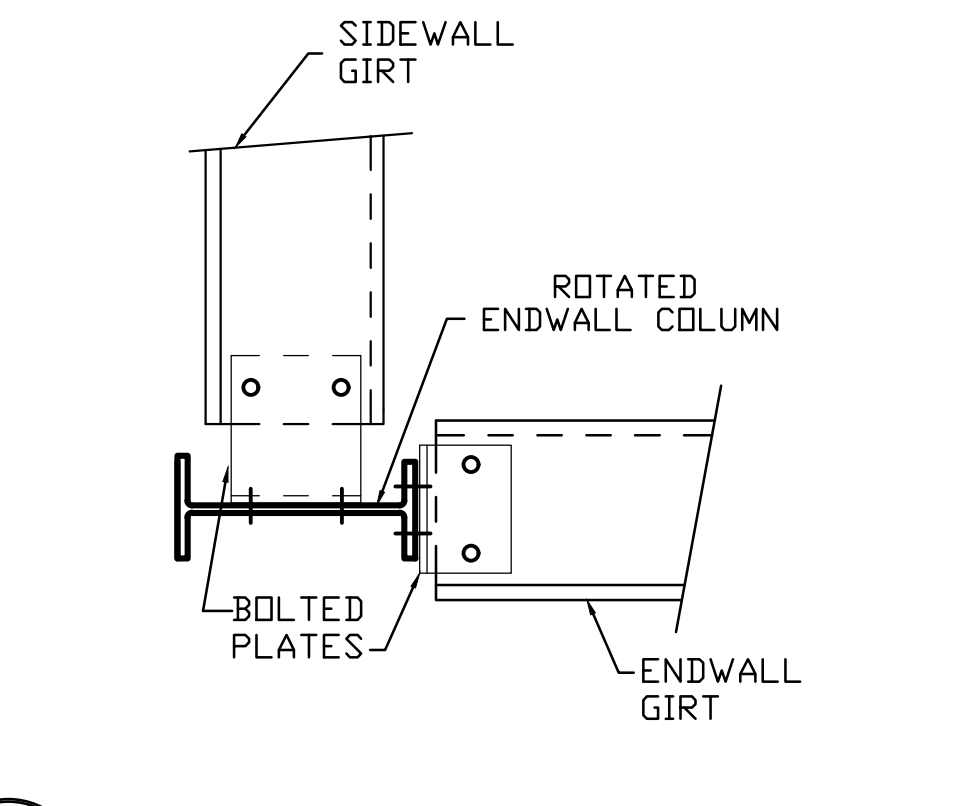
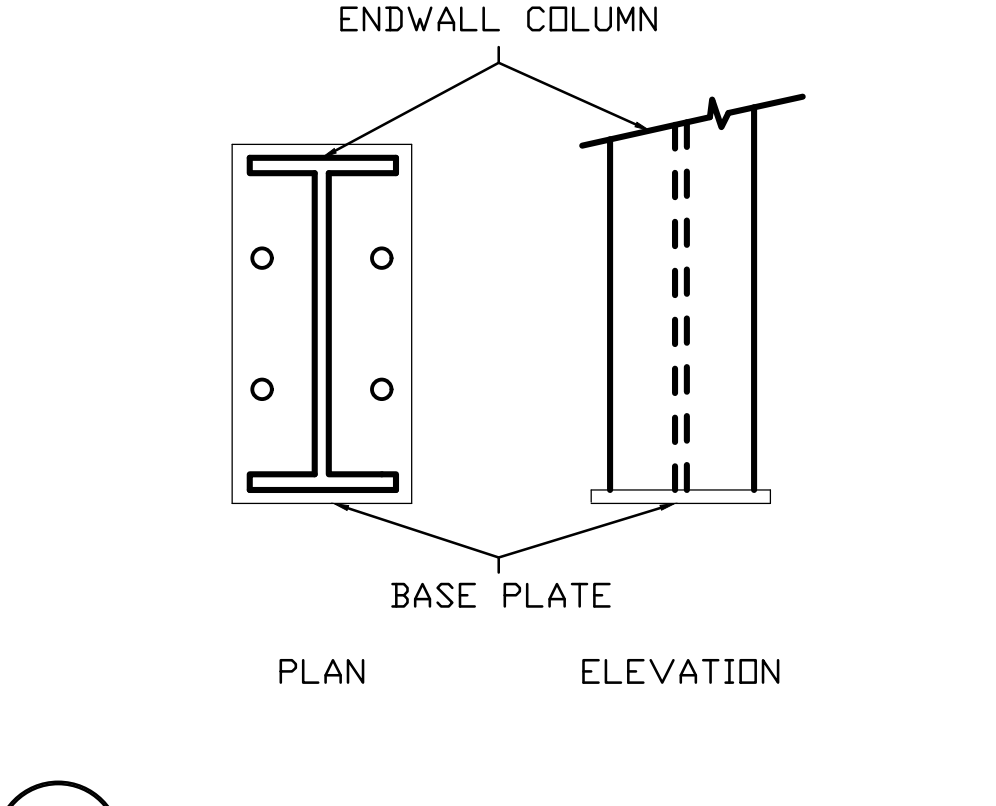
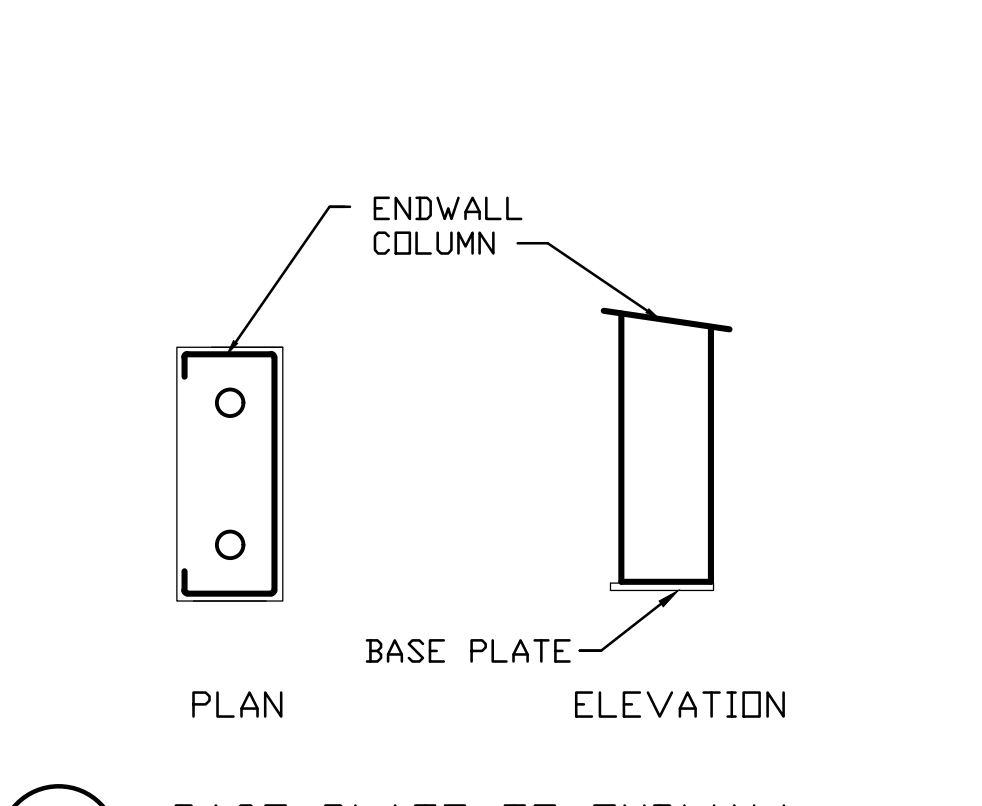
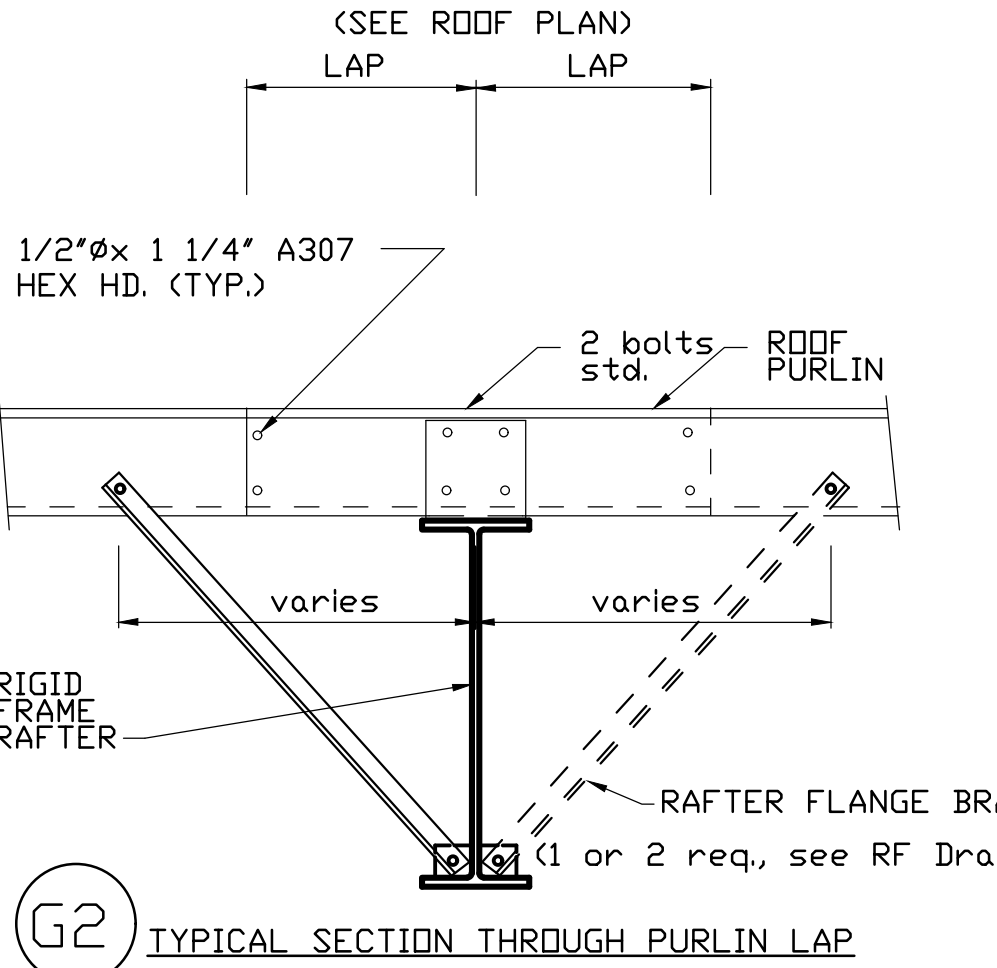
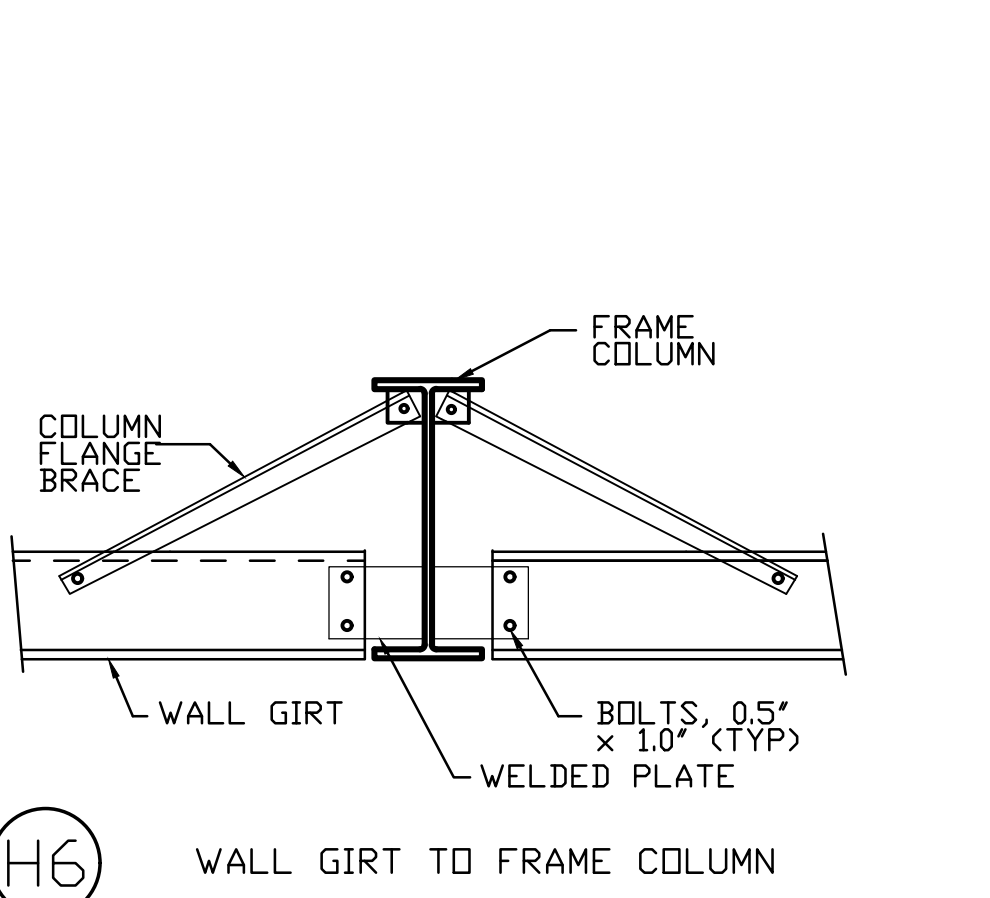
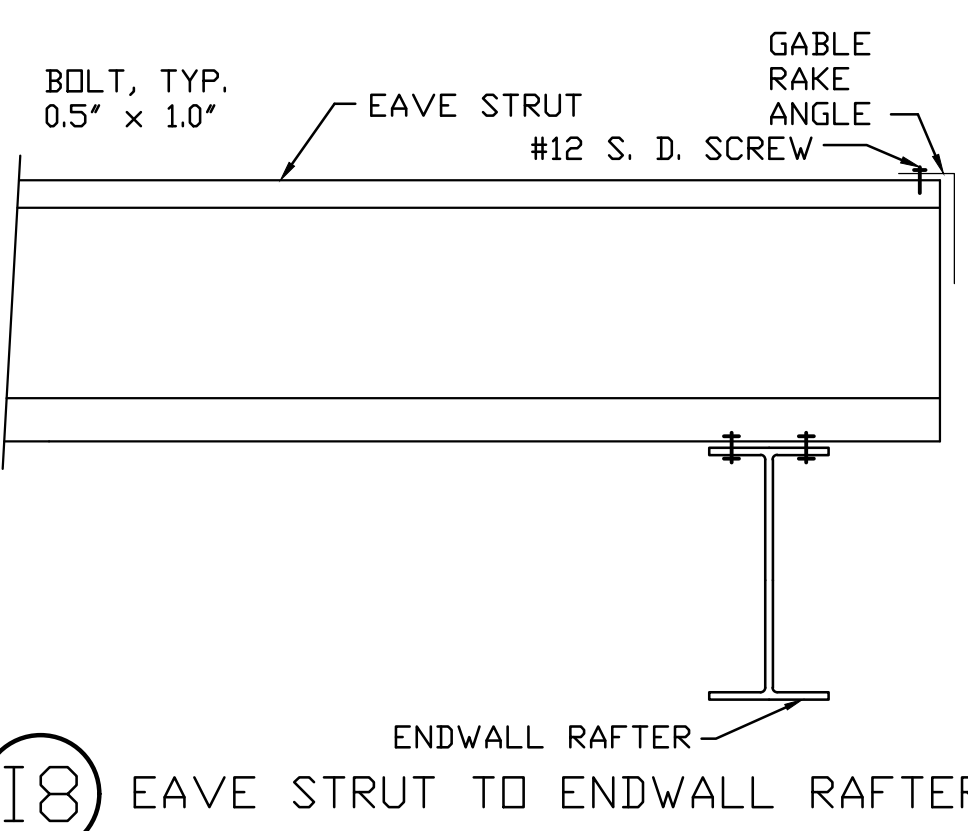
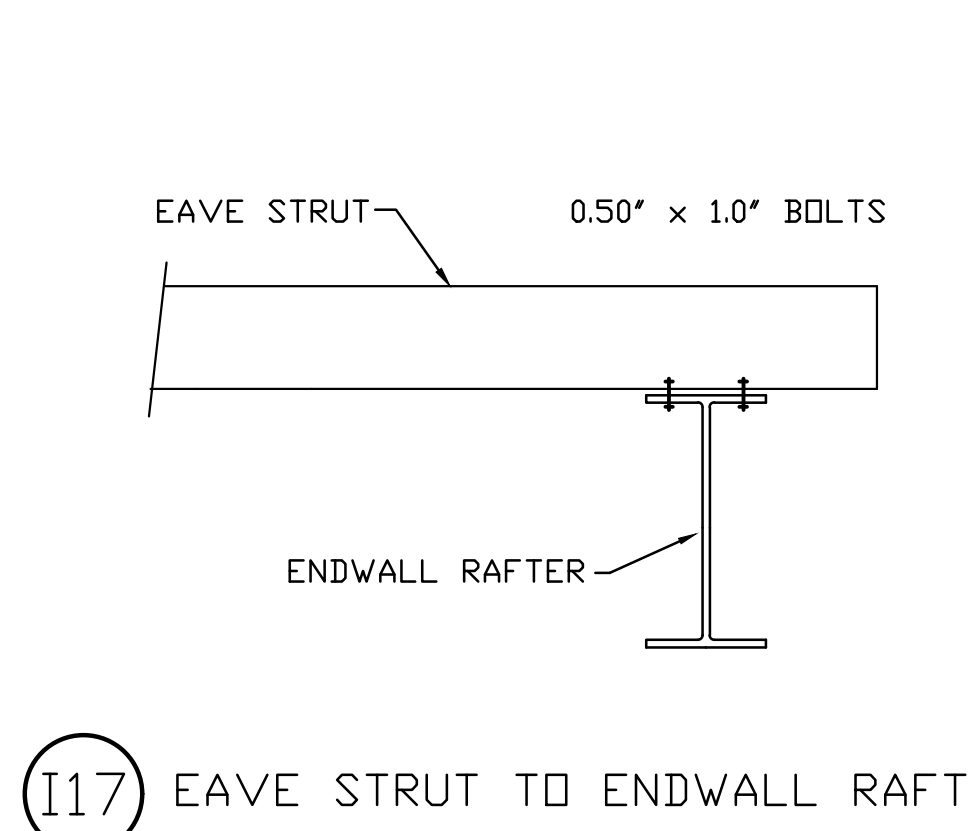
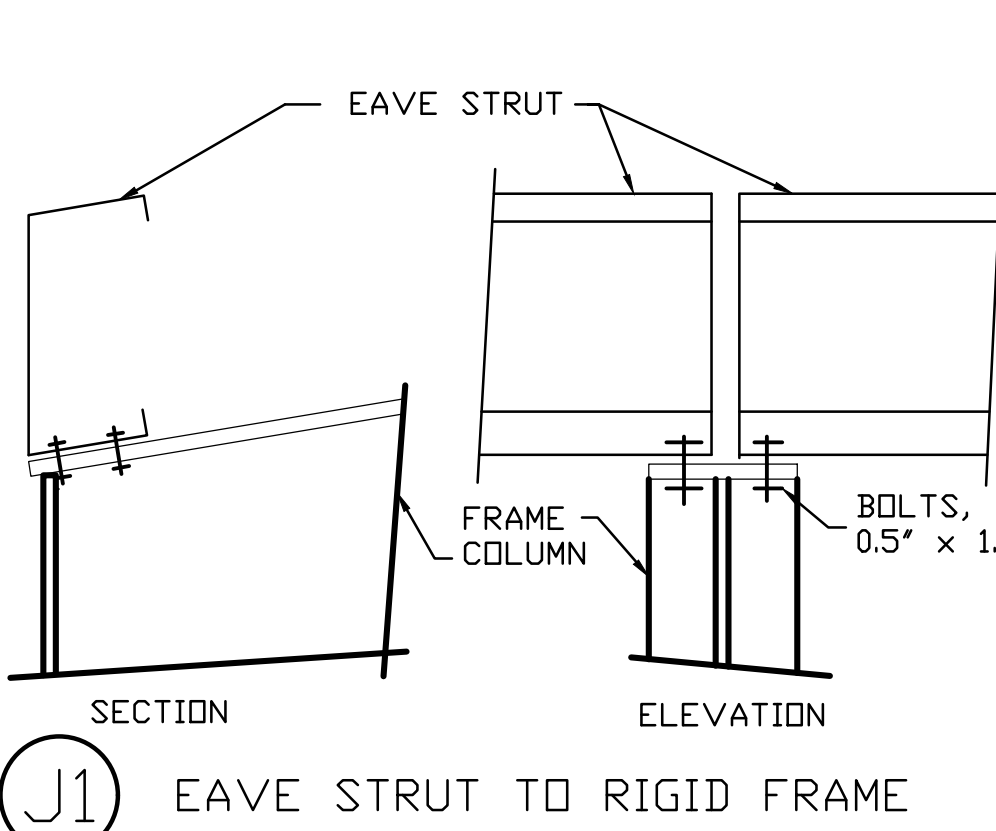
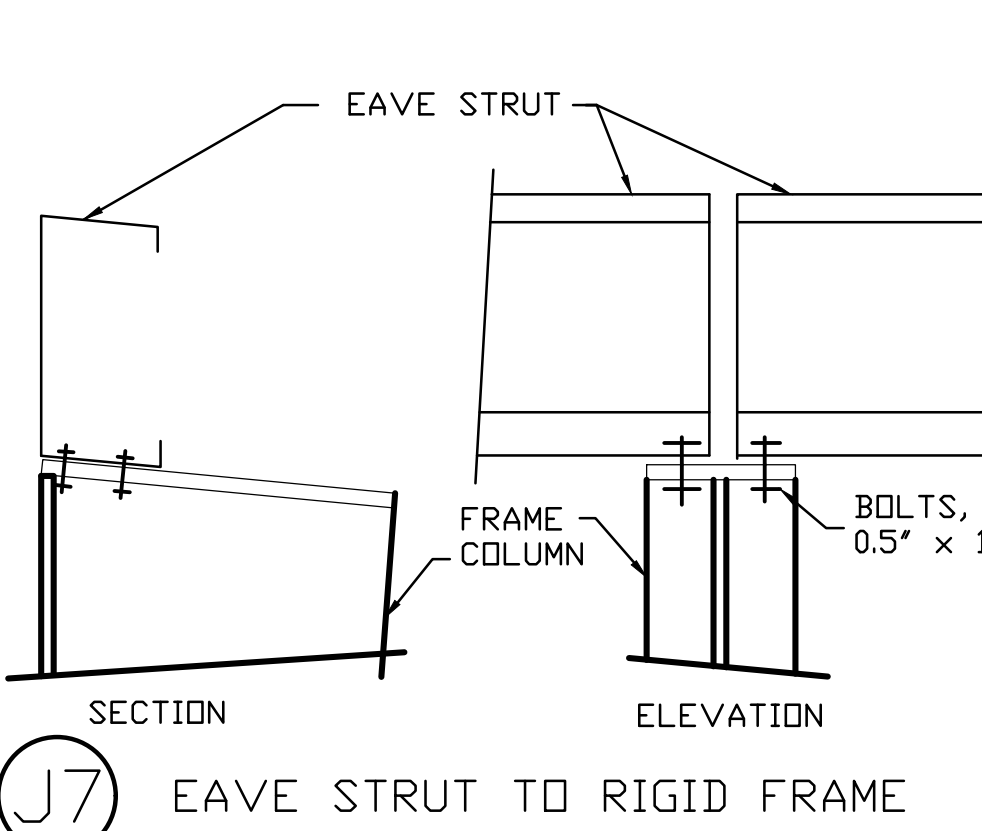
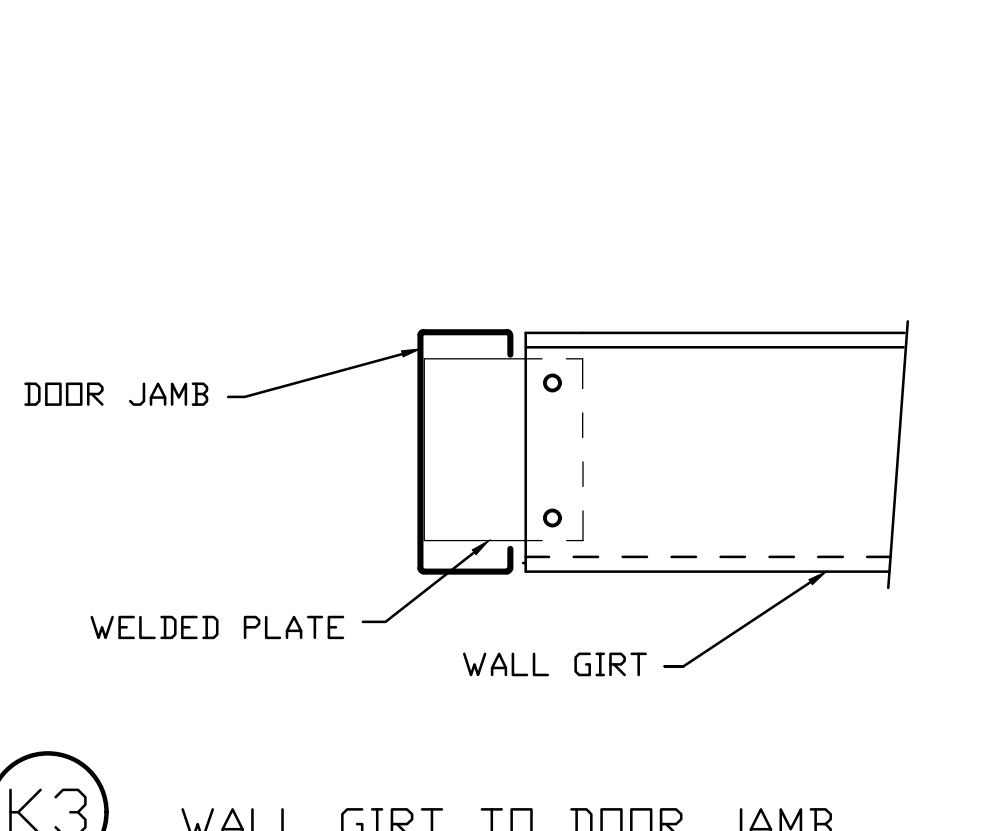
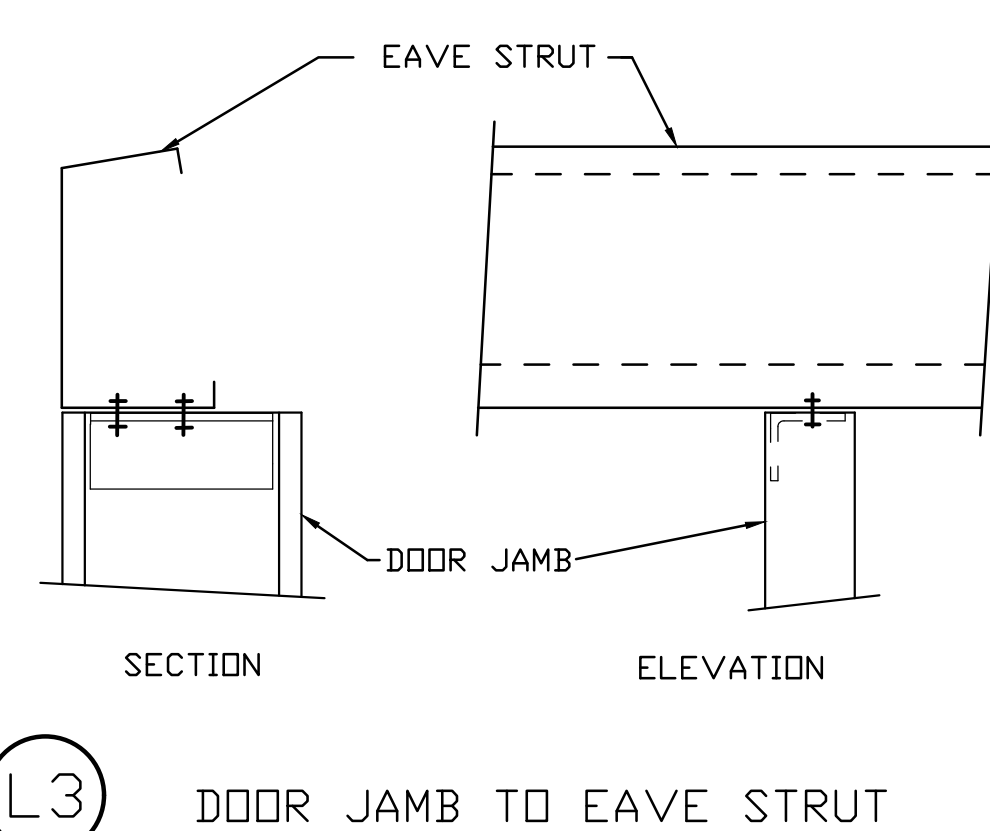
ANCHOR BOLT SUMMARY

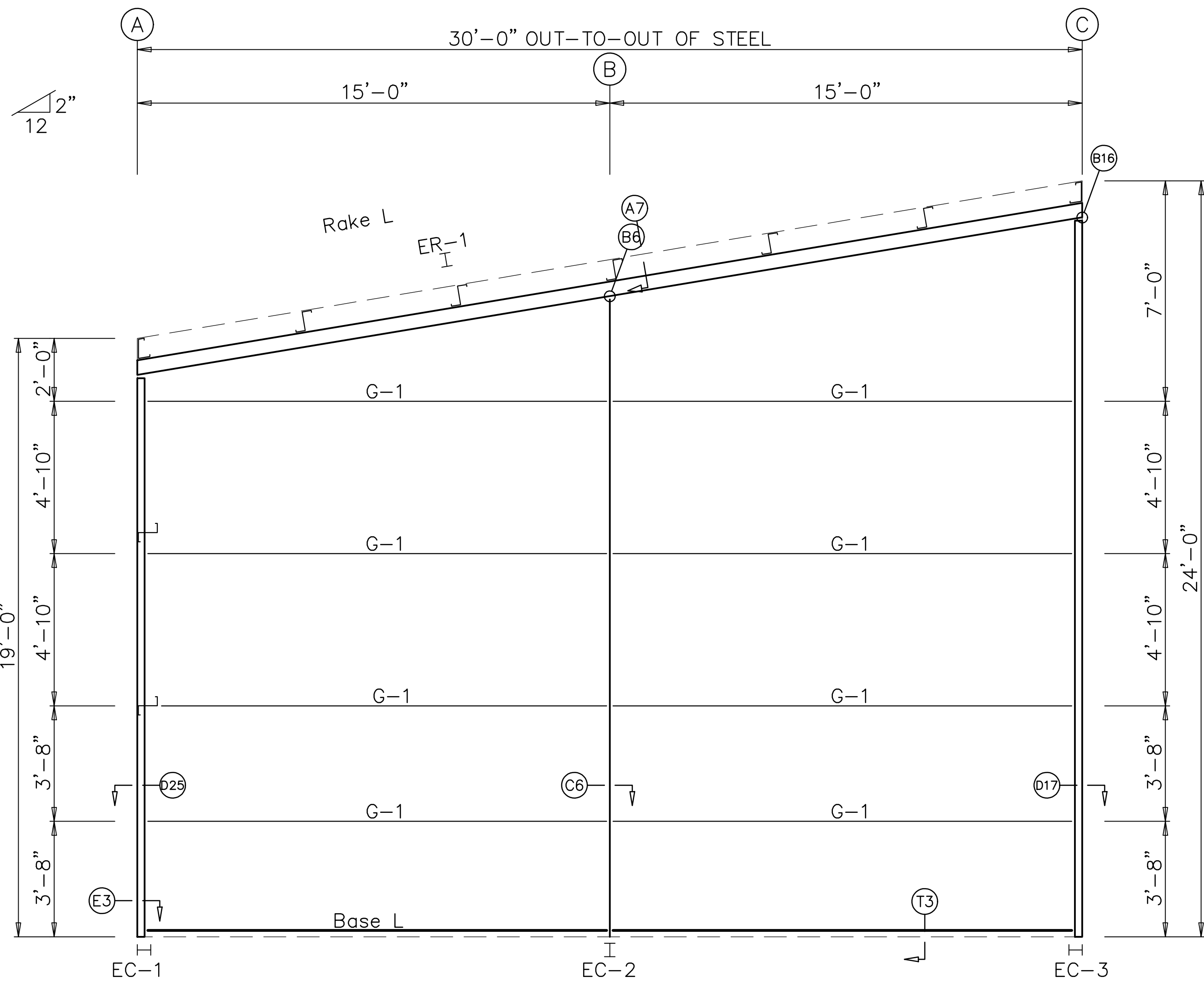
Qty	Locate	Dia (in)	Type	Proj (in)
⊗ 8	Jamb	5/8"	A307	3.00
⊗ 8	Endwall	5/8"	A307	3.00
⊕ 16	Endwall	3/4"	A307	3.00
⊕ 16	Frame	3/4"	A307	3.00



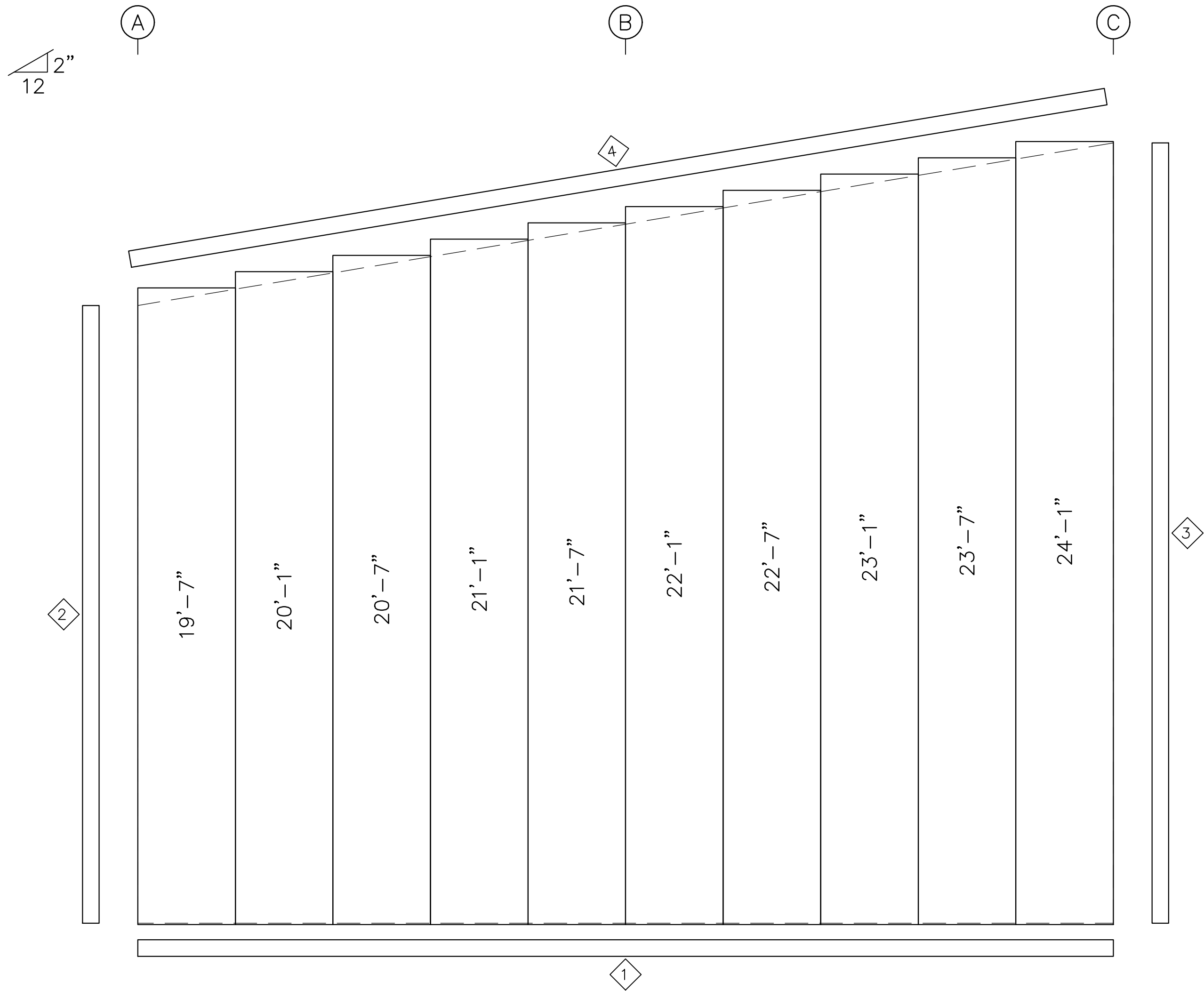
Union LaSteel				
PROJECT	Pre-Engineered Steel Building	ANCHOR BOLT REACTIONS		
ID	NewMillennium30x62	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	1992 Bascom Norris Drive Lake City, FL 32055	DATE: 7/25/24	SHEET 3 OF 13	



 Door jamb 0.5" anchor bolts Sheeting notch Base angle Wall panel Member screw @ 12" o.c. Field cut girt as required JT-101 0.5" 0.25" Walk door jamb D17 DOOR JAMB TRIM	 END OF BUILDING GABLE RAKE ANGLE #12 S. D. SCREW ROOF PURLIN ENDWALL RAFTER WELDED PLATE BOLT, 0.5" BY 1" (TYP) A7 SECTION THRU ENDWALL RAFTER	 ENDWALL RAFTER ENDWALL COLUMN BOLTS (2) 3/4" x 1 3/4" A325 B6 ENDWALL RAFTER TO COLUMN	 ENDWALL RAFTER ENDWALL COLUMN B16 CORNER COLUMN TO ENDWALL RAFTER	 BOLT, 0.5" BY 1.0" (TYP) ENDWALL COLUMN WELDED PLATE ENDWALL GIRTS C6 ENDWALL COLUMN TO WALL GIRT	
 SIDEWALL GIRT W R COLUMN WELDED PLATE WELDED ANGLE ENDWALL GIRT D17 CORNER COLUMN TO WALL GIRT	 SIDEWALL GIRT ROTATED ENDWALL COLUMN BOLTED PLATES ENDWALL GIRT D25 CORNER COLUMN TO WALL GIRT	 ENDWALL COLUMN BASE PLATE PLAN ELEVATION E3 BASE PLATE FOR ENDWALL COLUMN	 ENDWALL COLUMN BASE PLATE PLAN ELEVATION E5 BASE PLATE TO ENDWALL COLUMN OR DOOR JAMB	 (SEE ROOF PLAN) LAP 1/2" x 1 1/4" A307 HEX HD. (TYP.) 2 bolts std. ROOF PURLIN varies RIGID FRAME RAFTER varies RAFTER FLANGE BRACE (1 or 2 req, see RF Drawing) G2 TYPICAL SECTION THROUGH PURLIN LAP	 COLUMN FLANGE BRACE FRAME COLUMN WALL GIRT BOLTS, 0.5" x 1.0" (TYP) WELDED PLATE H6 WALL GIRT TO FRAME COLUMN
 BOLT, TYP. 0.5" x 1.0" EAVE STRUT #12 S. D. SCREW GABLE RAKE ANGLE ENDWALL RAFTER I18 EAVE STRUT TO ENDWALL RAFTER	 EAVE STRUT 0.50" x 1.0" BOLTS ENDWALL RAFTER I17 EAVE STRUT TO ENDWALL RAFTER	 EAVE STRUT FRAME COLUMN BOLTS, 0.5" x 1.5" SECTION ELEVATION J1 EAVE STRUT TO RIGID FRAME	 EAVE STRUT FRAME COLUMN BOLTS, 0.5" x 1.5" SECTION ELEVATION J7 EAVE STRUT TO RIGID FRAME	 DOOR JAMB WELDED PLATE WALL GIRT K3 WALL GIRT TO DOOR JAMB	 EAVE STRUT DOOR JAMB SECTION ELEVATION L3 DOOR JAMB TO EAVE STRUT



ENDWALL FRAMING: FRAME LINE 1

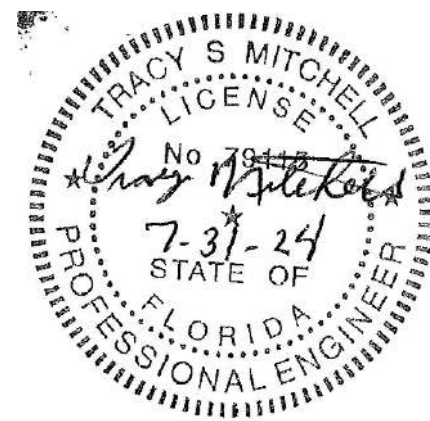


ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 Ga. SX – Need Sill Poly Color

BOLT TABLE FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
Cor_Column/Raf	4	A325	3/4"	2"
EC-2/ER-1	2	A325	5/8"	1 1/2"

TRIM TABLE FRAME LINE 1			
ID	PART	LENGTH	DETAIL
1	BT-102	10'-3"	TRIM_68
2	CT-102	10'-3"	TRIM_323
3	JT-101	10'-3"	TRIM_301
4	RT-101	10'-3"	TRIM_229

MEMBER TABLE FRAME LINE 1		
MARK	PART	LENGTH
EC-1	W8X10	17'-8 15/16"
EC-2	W8X10	20'-1 3/8"
EC-3	W8X10	22'-7 5/8"
ER-1	W8X10	30'-4 15/16"
G-1	8X25Z16	13'-11 7/8"

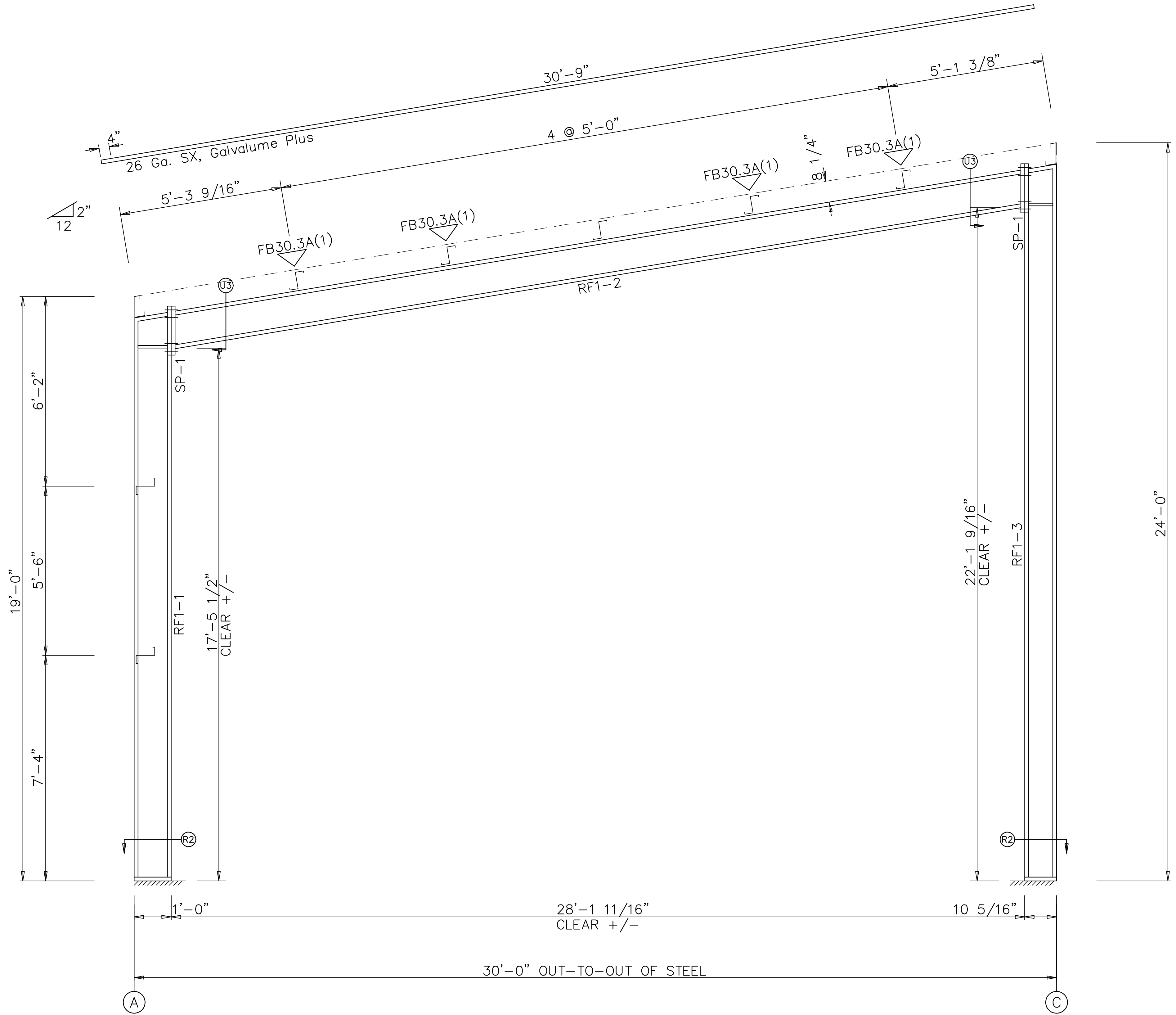


Union LaSteel				
PROJECT	Pre-Engineered Steel Building	ENDWALL FRAMING		
ID	NewMillennium30x62	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	1992 Bascom Norris Drive Lake City, FL 32055	DATE: 7/25/24	SHEET 6 OF 13	

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
	Top	Bot				
SP-1	4	4	0	A325	0.750	2.00

▽FLANGE BRACES: Both Sides(U.N.)
FBxxA(1): xx=length(in)
A - L2x2x14G

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
RF1-1	W12X16	18'-3 5/8"
RF1-2	W12X16	28'-6 1/16"
RF1-3	W10X26	23'-3 3/4"



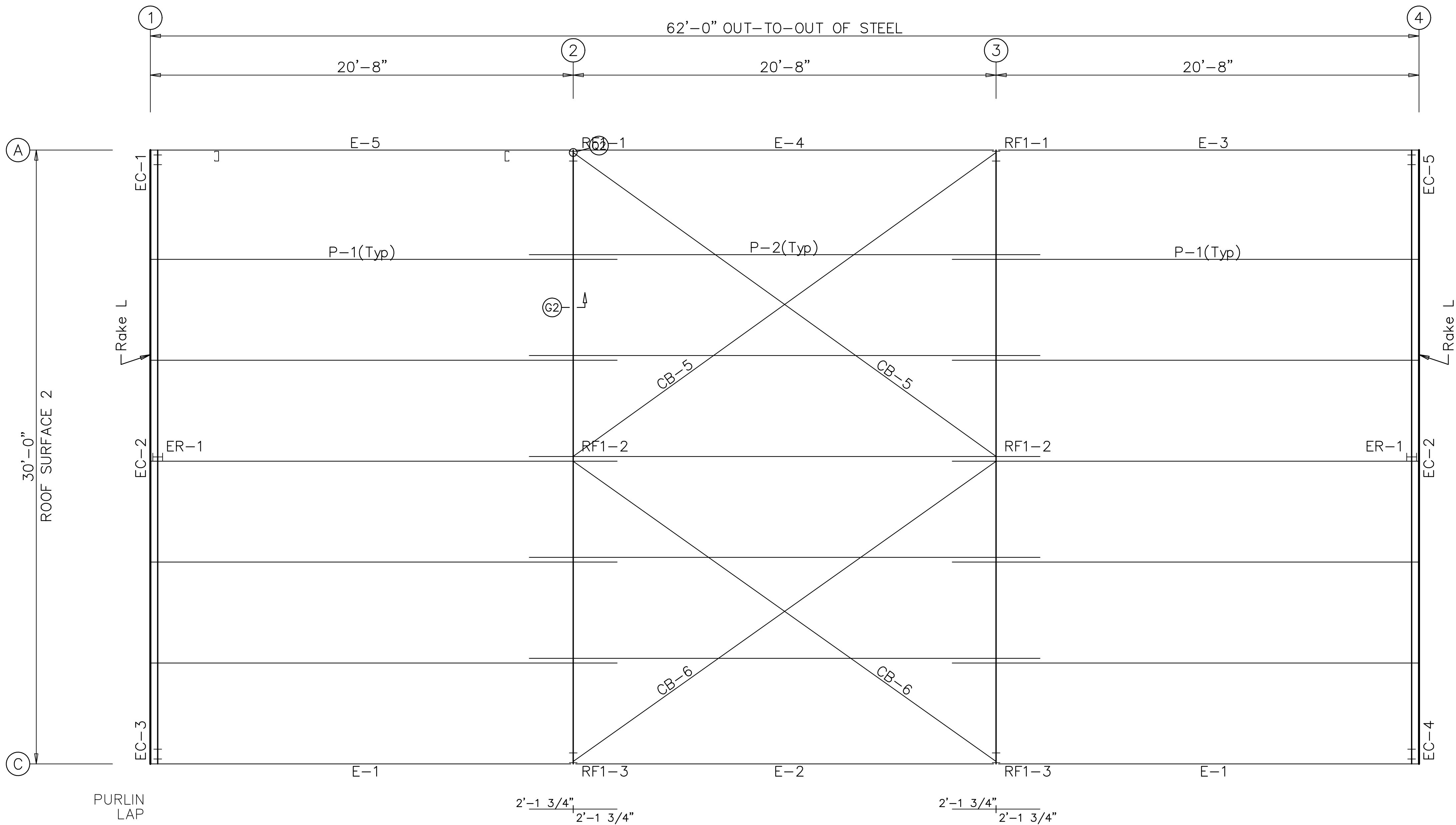
RIGID FRAME ELEVATION: FRAME LINE 2 3



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PROJECT	Pre-Engineered Steel Building	RIGID FRAME ELEVATION		
ID	NewMillennium30x62	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	1992 Bascom Norris Drive Lake City, FL 32055	DATE: 7/25/24	SHEET 8 OF 13	

MEMBER TABLE		
ROOF PLAN		
MARK	PART	LENGTH
P-1	8X25Z14	22'-9 1/2"
P-2	8X25Z16	24'-11 1/2"
E-1	E085342H	20'-7 1/2"
E-2	E085342H	20'-7 1/2"
E-3	E085342L	20'-7 1/2"
E-4	E085342L	20'-7 1/2"
E-5	E085342L	20'-7 1/2"
CB-5	0.25_CBL	23'-10 1/4"
CB-6	0.25_CBL	23'-7 1/4"



ROOF FRAMING PLAN

30'-9" (21)

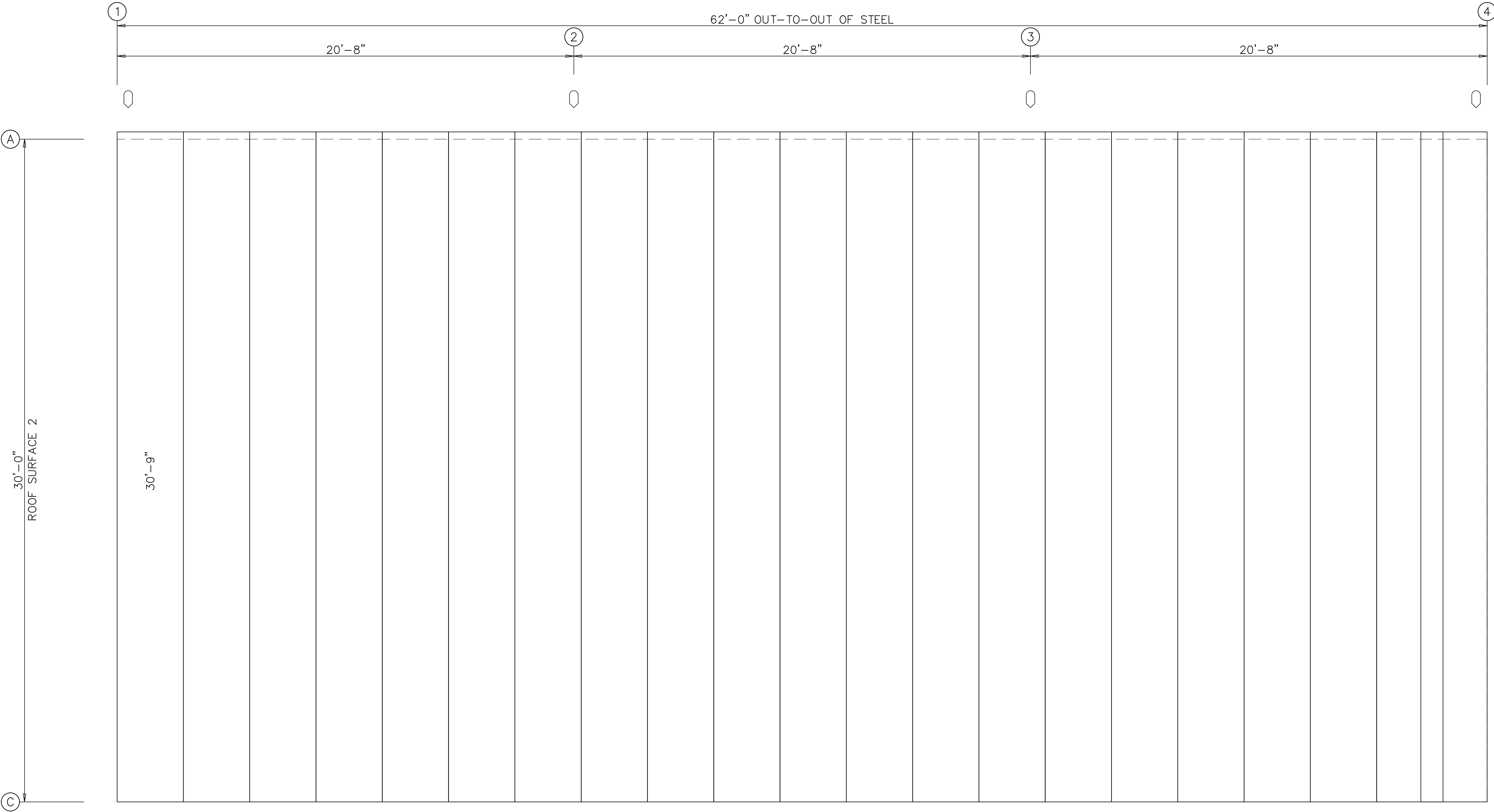
ROOF SHEETING
PANELS: 26 Ga. SX
Galvalume Plus



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PROJECT	Pre-Engineered Steel Building	ROOF FRAMING		
ID	NewMillennium30x62	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	1992 Bascom Norris Drive Lake City, FL 32055	DATE: 7/25/24	SHEET 9 OF 13	

⏏ DOWNSPOUT LOCATIONS

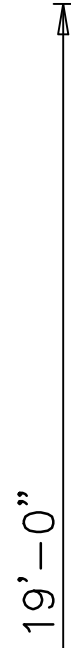


ROOF SHEETING PLAN
PANELS: 26 Ga. SX – Galvalume Plus



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PROJECT	Pre-Engineered Steel Building	ROOF SHEETING		
ID	NewMillennium30x62	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	1992 Bascom Norris Drive Lake City, FL 32055	DATE: 7/25/24	SHEET 10 OF 13	



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A

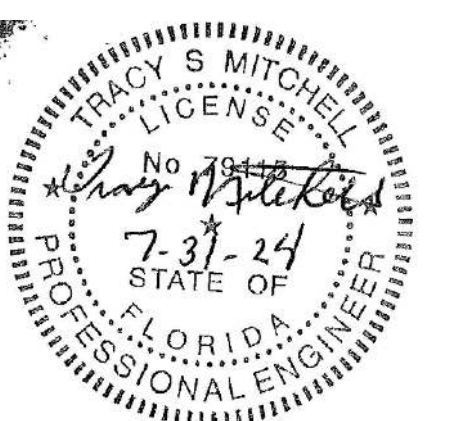
PANELS: 26 Ga. SX — Need Sill Poly Color

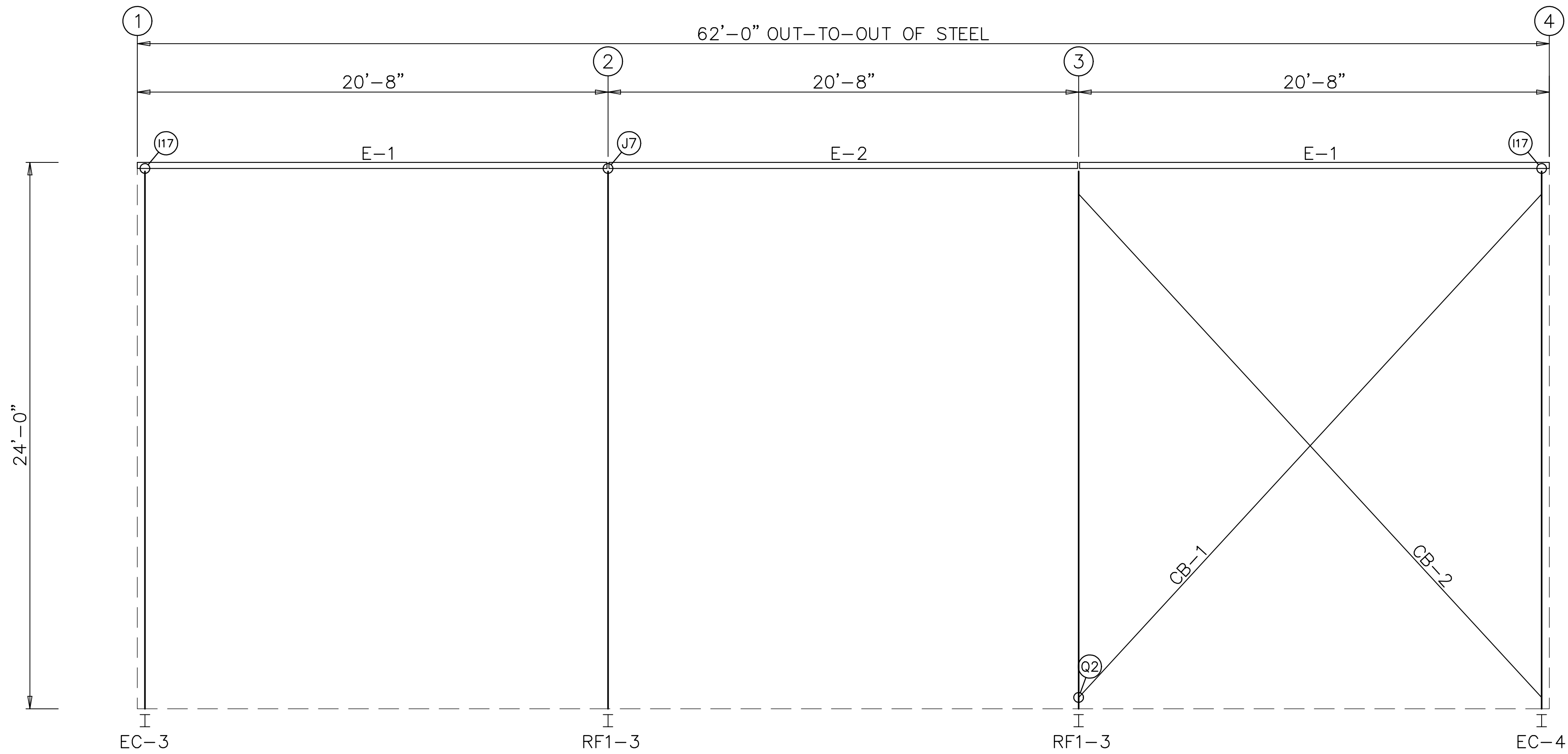


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PROJECT	Pre-Engineered Steel Building	SIDEWALL FRAMING		
ID	NewMillennium30x62	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	1992 Bascom Norris Drive Lake City, Fl 32055	DATE: 7/25/24	SHEET 11 OF 13	

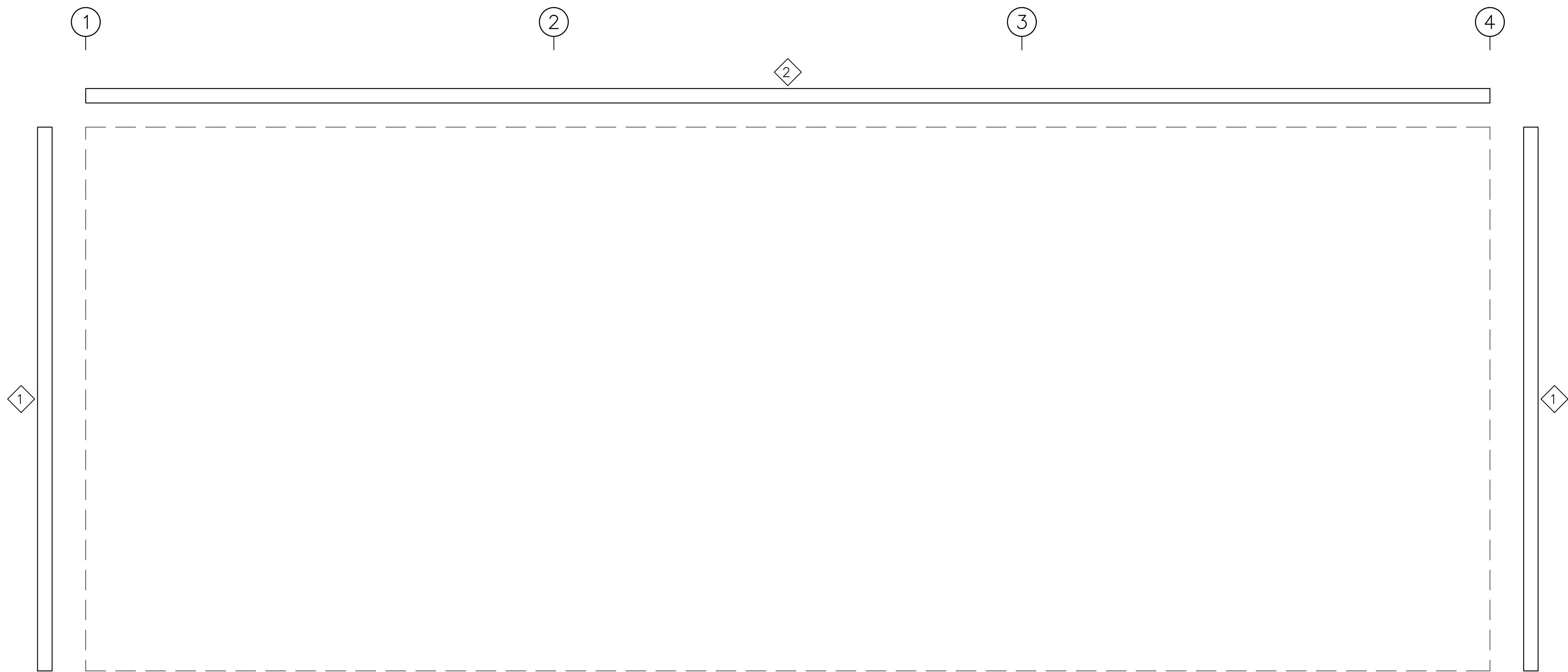
TRIM TABLE			
FRAME LINE A			
◇ID	PART	LENGTH	DETAIL
1	BT-102	10'-3"	TRIM_68
2	CT-102	10'-3"	TRIM_323
3	GU-121	20'-3"	
4	EC-122	6"	
5	SCB	6"	
6	JT-101	10'-3"	TRIM_401
7	HT-101	10'-3"	TRIM_232
8	MT-116B	10'-3"	TRIM_242
9	JT-101	10'-3"	TRIM_239
10	MT-116B	10'-3"	TRIM_235

MEMBER TABLE		
FRAME LINE A		
MARK	PART	LENGTH
DJ-1	8X35C16	7'-4"
DJ-2	8X35C12	18'-5 1/4"
DH-1	8X35C16	3'-0"
DH-2	8X35C16	14'-0"
E-3	E085342L	20'-7 1/2"
E-4	E085342L	20'-7 1/2"
E-5	E085342L	20'-7 1/2"
G-2	8X25Z12	19'-8 3/8"
G-3	8X25Z12	20'-1 1/4"
G-4	8X25Z12	20'-1 1/4"
G-5	8X25Z16	2'-8 3/8"
G-6	8X25Z16	2'-3 1/2"
CB-3	0.25_CBL	25'-4"
CB-4	0.25_CBL	24'-10 3/4"





SIDEWALL FRAMING: FRAME LINE C



SIDEWALL SHEETING & TRIM: FRAME LINE C

TRIM TABLE			
FRAME LINE C			
◇ID	PART	LENGTH	DETAIL
1	JT-101	10'-3"	TRIM_301
2	ET-702	10'-3"	TRIM_316

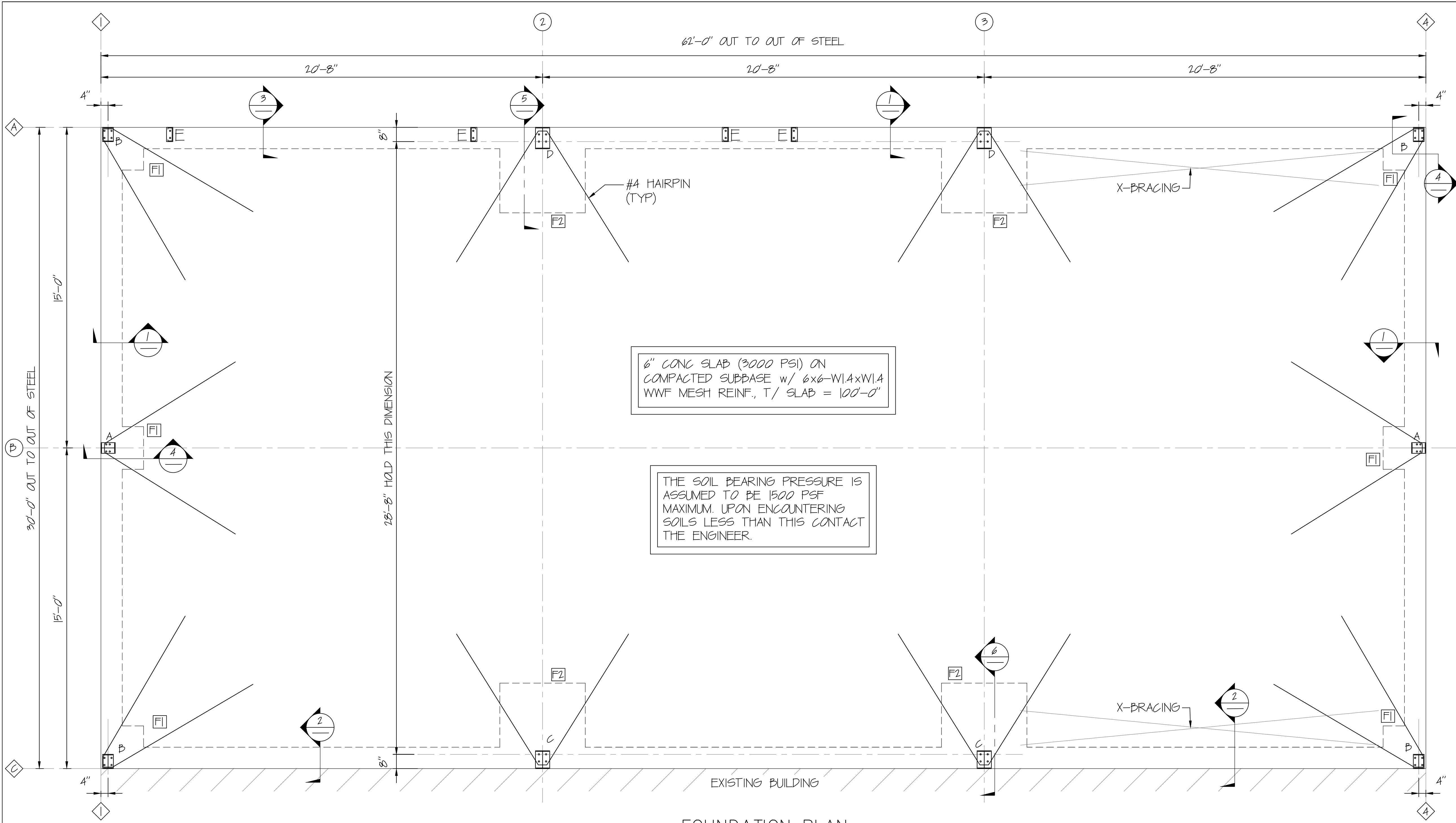
MEMBER TABLE		
FRAME LINE C		
MARK	PART	LENGTH
E-1	E085342H	20'-7 1/2"
E-2	E085342H	20'-7 1/2"
CB-1	0.25_CBL	28'-2 3/4"
CB-2	0.25_CBL	28'-9 1/2"



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ID	NewMillennium30x62	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	1992 Bascom Norris Drive Lake City, FL 32055	DATE: 7/25/24	SHEET 12 OF 13	



6" CONC SLAB (3000 PSI) ON
COMPACTED SUBBASE w/ 6x6-W14xW14
WWF MESH REINF., T/ SLAB = 100'-0"

THE SOIL BEARING PRESSURE IS
ASSUMED TO BE 1500 PSF
MAXIMUM. UPON ENCOUNTERING
SOILS LESS THAN THIS CONTACT
THE ENGINEER.

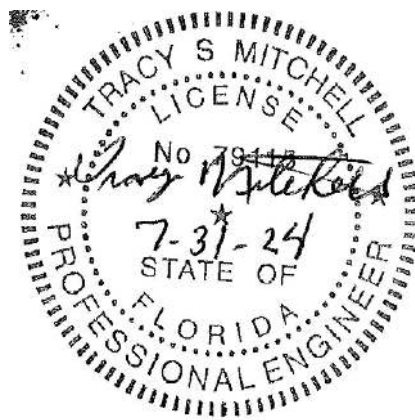
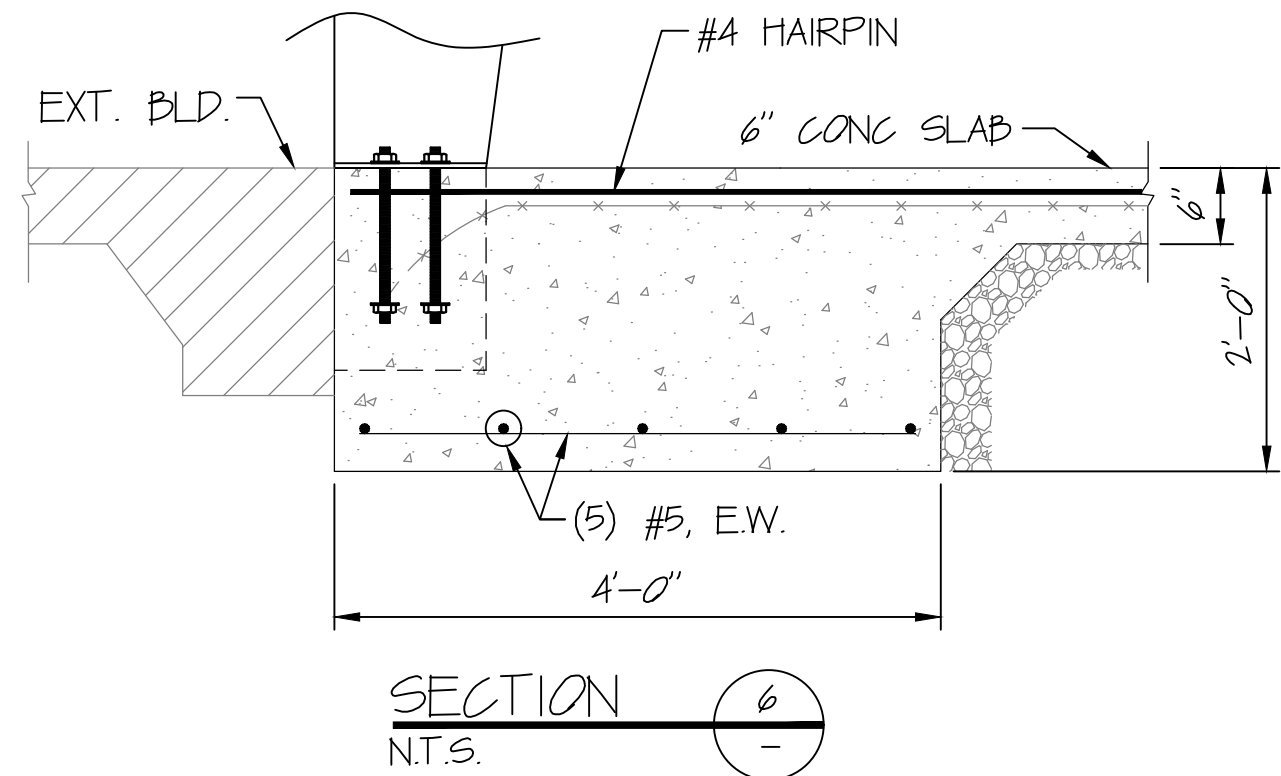
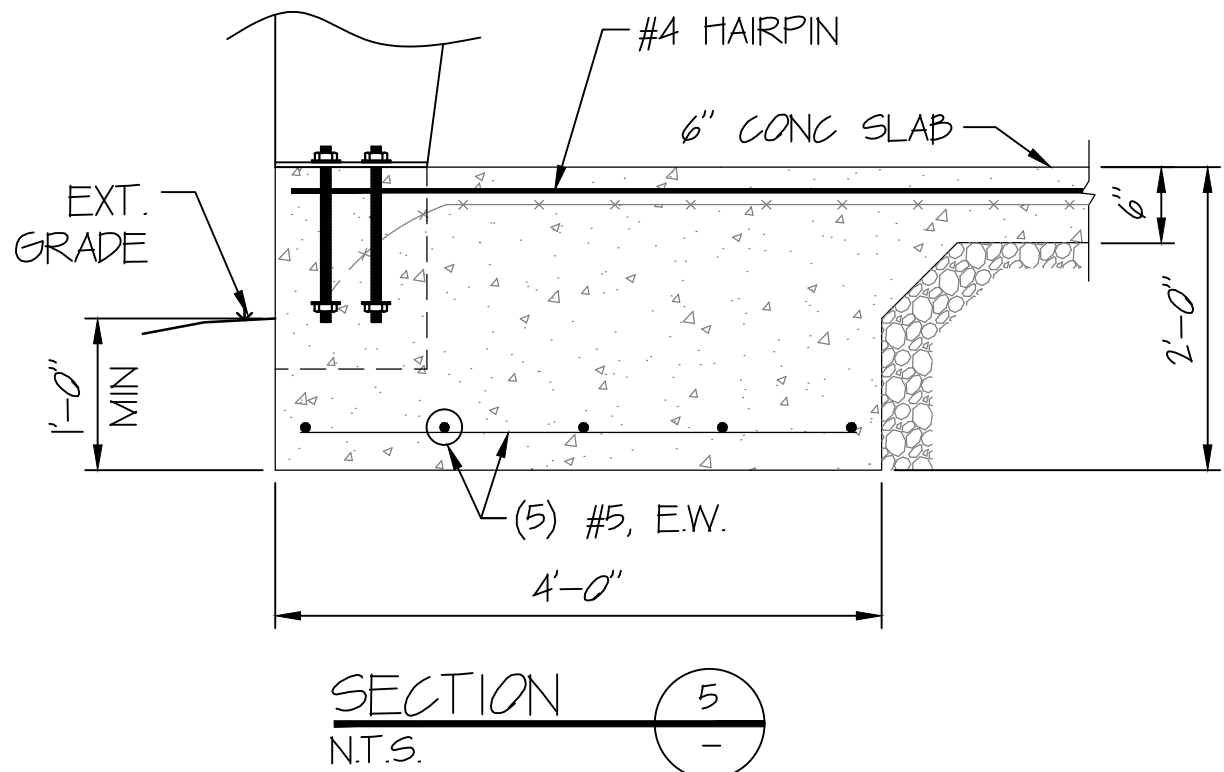
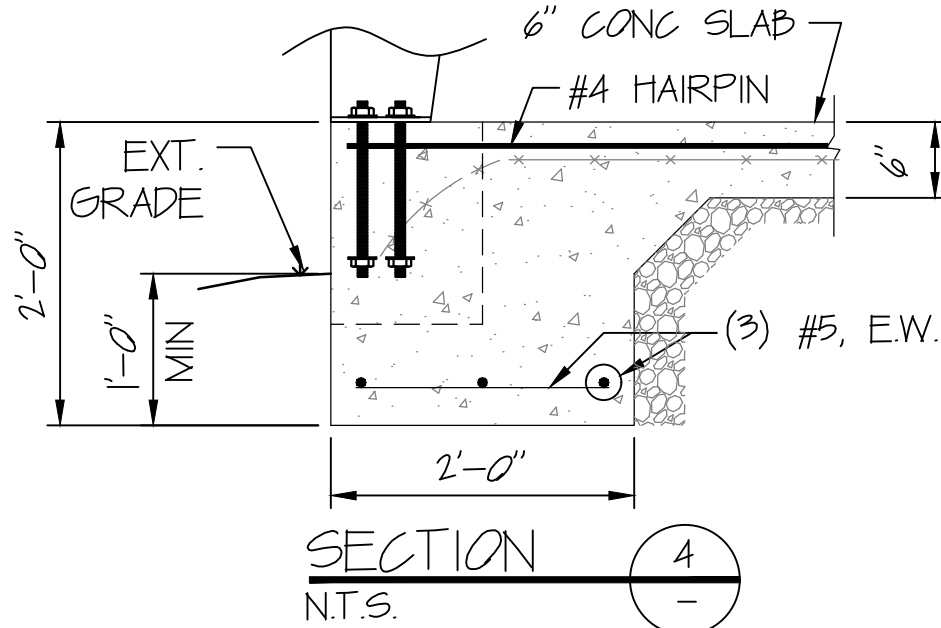
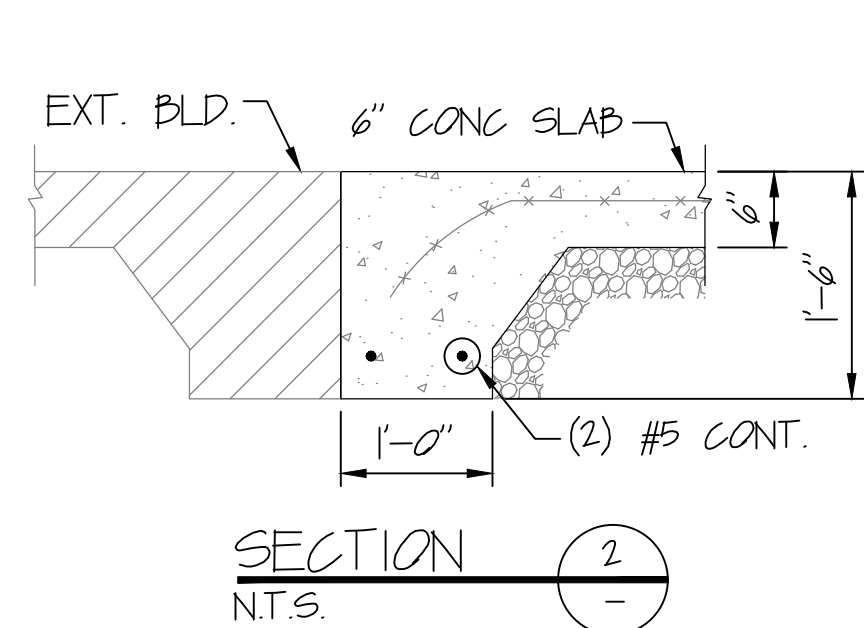
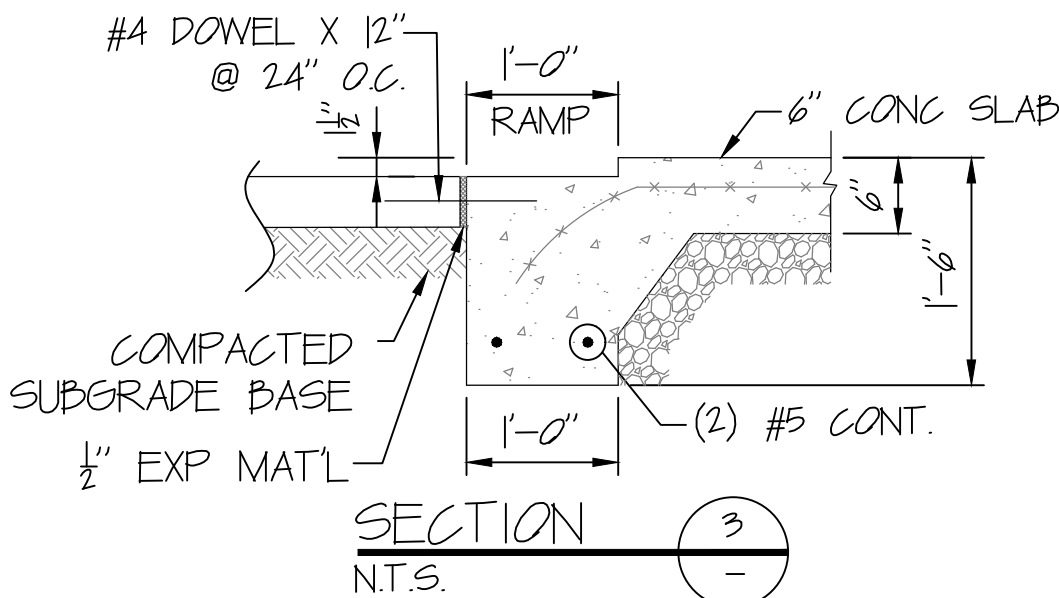
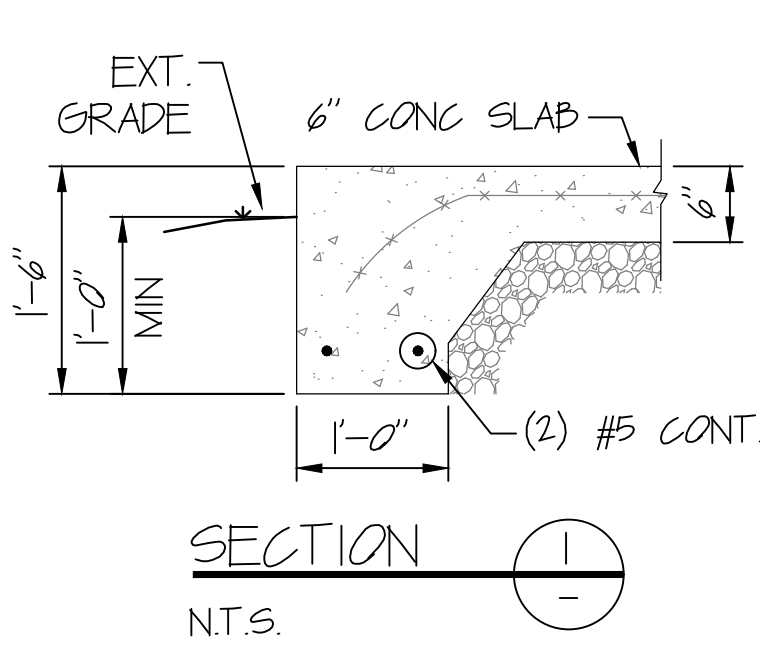
FOUNDATION PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)

CONCRETE

- 1. ALL CONCRETE WORK SHALL CONFORM TO ACI 301-18, "SPECIFICATIONS FOR BUILDINGS", EXCEPT AS NOTED BELOW. REFER TO CHAPTER 16 OF ACI 301 FOR REPORTS FROM TESTS THAT SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER, ARCHITECT, OWNER, ANY APPLICABLE CONTRACTORS, CONCRETE SUPPLIER, AND BUILDING OFFICIAL.
- 2. CONCRETE WORK IN COLD WEATHER SHALL CONFORM TO ALL REQUIREMENTS OF ACI 308R-16 "STANDARD SPECIFICATION FOR COLD WEATHER CONCRETING" AND ACI 308R-16 "COLD WEATHER CONCRETING".
- 3. CONCRETE WORK IN HOT WEATHER SHALL CONFORM TO ALL REQUIREMENTS OF ACI 308R-10 "HOT WEATHER CONCRETING". THE AIR TEMPERATURE, RELATIVE HUMIDITY, CONCRETE TEMPERATURE, AND WIND VELOCITY SHALL BE ENTERED INTO NOMOGRAPH FIG. 2.15 TO DETERMINE IF PRECAUTIONS AGAINST PLASTIC SHRINKAGE ARE REQUIRED.
- 4. REBAR FABRICATION, PLACEMENT, SPLICING, ETC. SHALL BE IN ACCORDANCE WITH THE LATEST EDITIONS OF ACI 318, THE CONCRETE REINFORCING STEEL INSTITUTE (CRSI) HANDBOOK AND ANY OTHER APPLICABLE PUBLICATIONS.
- 5. A SLUMP TEST SHALL BE PERFORMED BEFORE ANY HRWR IS ADDED.
- 6. CONCRETE MATERIAL SPECIFICATION: 3000 PSF f'c BASED ON 28 DAYS STRENGTH.
- 7. UNLESS OTHERWISE NOTED, REINFORCEMENT BARS TO HAVE MINIMUM CONCRETE COVER AS SHOWN BELOW:

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	3 INCHES
CONCRETE EXPOSED TO EARTH OR WEATHER:	
#5 BARS AND SMALLER	1 1/2 INCHES
OTHER	2 INCHES
CONCRETE NOT EXPOSED TO EARTH OR WEATHER:	
BEAM AND COLUMN BARS INCLUDING TIES, STIRRUPS AND SPIRALS	1 1/2 INCHES
SLABS, WALLS AND JOISTS:	
#11 BARS AND SMALLER	3/4 INCHES
OTHERS	1 1/2 INCHES
- 8. ALL EXPOSED EDGES ARE TO BE CHAMFERED 3/4 INCH, UNO.
- 9. LAP SPLICE LENGTHS TO BE OF CLASS "B" PER ACI.
- 10. CONCRETE REINFORCEMENT SHALL CONFORM TO ASTM A615 OR A616, GRADE 60.
- 11. ISOLATED COLUMN FOOTINGS HAVE BEEN DESIGNED WITH AN ASSUMED ALLOWABLE SOIL BEARING PRESSURE OF 1500 PSF, UPON ENCOUNTERING ANY ADVERSE SOIL CONDITIONS CONTACT THE STRUCTURAL ENGINEER.
- 12. DO NOT CAST CONCRETE AGAINST WATER OR FROZEN WATER.

FOOTING SCHEDULE			
FND MARK	FND SIZE (L x B x D)	REINFORCING TOP, E.W. BOTT, E.W.	TOP OF FOOTING
F1	2'-0"x2'-0"x1'-0"	- (3)~#5's	100'-0"
F2	4'-0"x4'-0"x2'-0"	- (5)~#5's	100'-0"



Union LaSteel			
PROJECT	Pre-Engineered Steel Building	FOUNDATION PLAN	
ID	NewMillennium30x62	DESIGN: TSM	DRAFT: GSM CHECK: TSM
PROJECT ADDRESS	1992 Bascom Norris Drive Lake City, FL 32055	DATE: 07/30/24	SHEET 13 OF 13