DATE 12/05/2005

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000023927

APPLICANT HAROLD FRANKEL PHONE 752-9592

ADDRESS 1703 SW SISTERS WELCOME RD LAKE CITY FL 32025

OWNER HAROLD FRANKEL PHONE 752-9592

ADDRESS 1703 SW SISTERS WELCOME LAKE CITY FL 32025

CONTRACTOR OWNER PHONE

LOCATION OF PROPERTY 341 ON THE LEFT CORNER OF BUSINESS POINTE AND 341

TYPE DEVELOPMENT HANGAR ESTIMATED COST OF CONSTRUCTION 44000.00

HEATED FLOOR AREA TOTAL AREA 2880.00 HEIGHT 24.00 STORIES 1

FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 4/12 FLOOR SLAB

LAND USE & ZONING RMF-1 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 1 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 13-4S-16-02952-201 SUBDIVISION SOUTHERN APPROACHES

LOT 1 BLOCK PHASE UNIT 1 TOTAL ACRES .50

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor
			Harold Frankel

Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident
EXISTING	05-1058-N	BK	JH	N

COMMENTS: 1ST FLOOR TO BE AT 116.5 FT PER GRADING PLAN BY ENGINEER. NEED PROOF BY ELEVATION LETTER BEFORE SLAB

NOC ON FILE Check # or Cash 1148

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power ____ date/app. by | Foundation ____ date/app. by | Monolithic ____ date/app. by

Under slab rough-in plumbing ____ date/app. by | Slab ____ date/app. by | Sheathing/Nailing ____ date/app. by

Framing ____ date/app. by | Rough-in plumbing above slab and below wood floor ____ date/app. by

Electrical rough-in ____ date/app. by | Heat & Air Duct ____ date/app. by | Peri. beam (Lintel) ____ date/app. by

Permanent power ____ date/app. by | C.O. Final ____ date/app. by | Culvert ____ date/app. by

M/H tie downs, blocking, electricity and plumbing ____ date/app. by | Pool ____ date/app. by

Reconnection ____ date/app. by | Pump pole ____ date/app. by | Utility Pole ____ date/app. by

M/H Pole ____ date/app. by | Travel Trailer ____ date/app. by | Re-roof ____ date/app. by

BUILDING PERMIT FEE $ 220.00 CERTIFICATION FEE $ 14.40 SURCHARGE FEE $ 14.40

MISC. FEES $ .00 ZONING CERT. FEE $ FIRE FEE $ .00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ CULVERT FEE $ **TOTAL FEE** 248.80

INSPECTORS OFFICE L.H. CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525
(exp. 10/31/2005)

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.
Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise. #23927

Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
Company Address: 301 NW Cole Terrace City Lake City State FL Zip 32055
Company Business License No. JB109476 Company Phone No. 386-755-3611
FHA/VA Case No. (if any)

Section 2: Builder Information

Company Name: Harold Frankel Company Phone No.

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 115 SW Bonanza Glen
Lake City, FL 32025
Airplane Hangar
Type of Construction (More than one box may be checked) ☒ Slab ☐ Basement ☐ Crawl ☐ Other
Approximate Depth of Footing: Outside V Inside 2' Type of Fill Sand

Section 4: Treatment Information

Date(s) of Treatment(s) 12/30/05
Brand Name of Product(s) Used Pro Build TC
EPA Registration No. 100-1006
Approximate Final Mix Solution % .5%
Approximate Size of Treatment Area: Sq. ft. 2880 Linear ft. — Linear ft. of Masonry Voids —
Approximate Total Gallons of Solution Applied 290 gals.
Was treatment completed on exterior? ☐ Yes ☒ No
Service Agreement Available? ☐ Yes ☐ No
Note: Some state laws require service agreements to be issued. This form does not preempt state law.

Attachments (List)
Comments

Name of Applicator(s) S. Gregory Certification No. (if required by State law) JF104376

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature Thomas Gregory Date 12/30/05

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

Cal-Tech Testing, Inc.
• Engineering
• Geotechnical
• Environmental
LABORATORIES

P.O. Box 1625 • Lake City, FL 32056-1625
6919 Distribution Avenue S., Unit #5 • Jacksonville, FL 32256

Tel. (904) 755-3633 • Fax (904) 752-5456
Tel. (904) 262-4046 • Fax (904) 262-4047

"Excellence in Engineering & Geoscience"

JOB NO: 05-579

DATE TESTED: 12-30-05

23927

REPORT OF IN-PLACE DENSITY TEST

ASTM METHOD: ✓ **D-2922 (Nuclear)** ___ **D-2937 (Drive Cylinder)** ___ **Other**_____

PROJECT: Fromhel Residence Plome Hangar

CLIENT: Harrold Fromhel

GENERAL CONTRACTOR: S.A.C. **EARTHWORK CONTRACTOR:** S.A.C.

SOIL USE: 1 **SPECIFICATION REQUIREMENTS:** 95%

TECHNICIAN: Chad Day

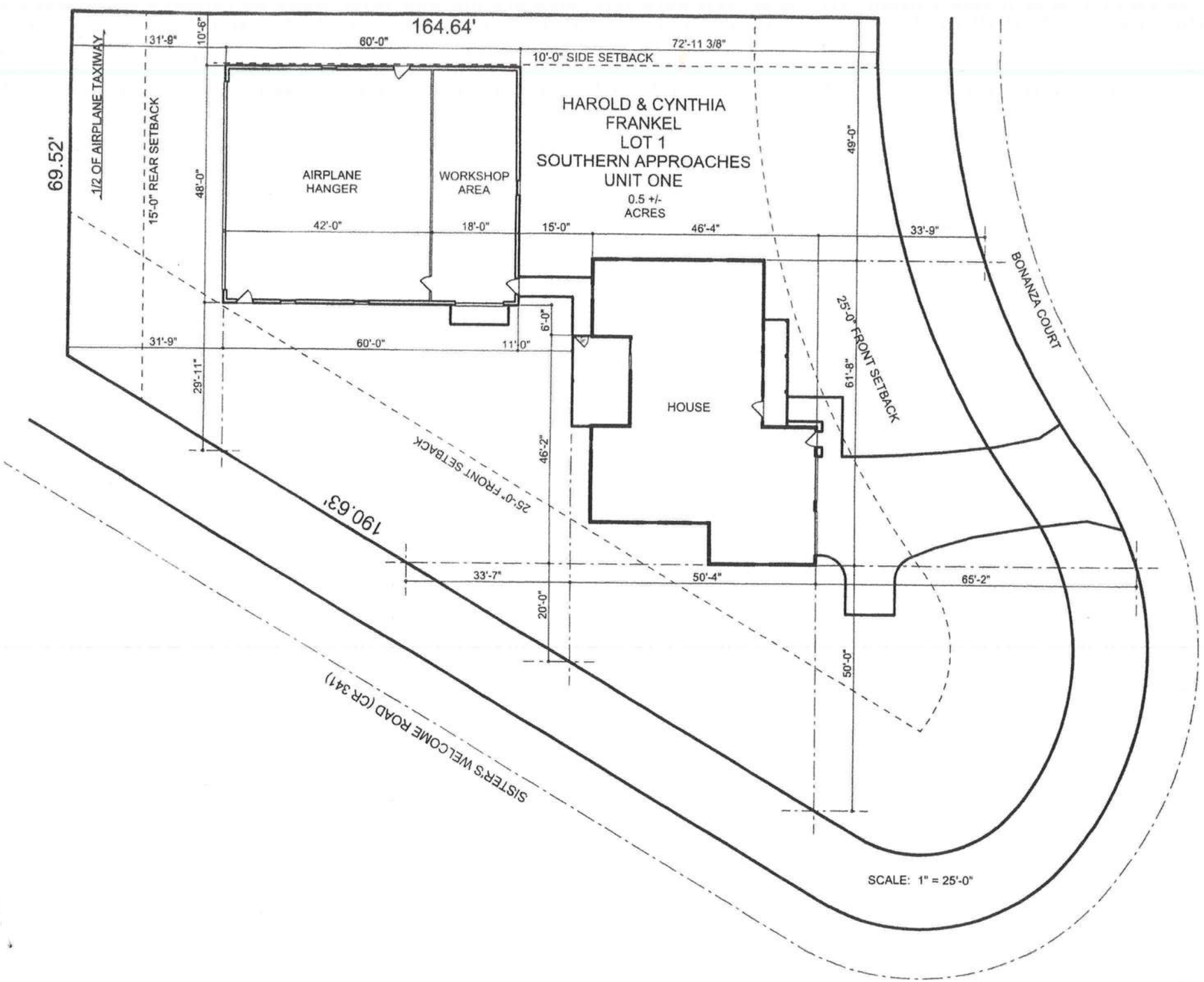
MODIFIED (ASTM D-1557): ✓ **STANDARD (ASTM D-698):** _____

TEST NO.	TEST LOCATION	TEST ✓DEPTH ___ELEV. ___LIFT#	PROCTOR NO.	WET DENS. LBS.CU.FT.	DRY DENS. LBS.CU.FT.	MOIST. PERCENT	% MAX.DENS.
8	N.W. Corner of Pad 4'S. 5'E.	12"	1	108.4	101.8	6.5	95.0
9	S.E. Corner of Pad 12'N. 10'W.	12"	1	115.2	103.8	5.8	101.6
10	South footer @center of Pad	12"	1	106.8	102.2	4.5	95.4
11	N.W. Corner of footer	12"	1	106.9	102.0	5.0	95.2

REMARKS: _____

PROCTOR NO.	SOIL DESCRIPTION	PROCTOR VALUE	OPT.MOIST.
1	Light Tan Sand (Richardson's Pit)	107.1	10.8

NOTE: SOIL USES: 1. Building Pad Fill 2. Trench Backfill 3. Base Course 4. Subbase-Stabilized Subgrade 5. Embankment 6. Subgrade - Natural Soil 7. Other


164.64'
69.52'
1/2 OF AIRPLANE TAXIWAY
15'-0" REAR SETBACK
31'-9"
10'-6"
60'-0"
72'-11 3/8"
10'-0" SIDE SETBACK
HAROLD & CYNTHIA
FRANKEL
LOT 1
SOUTHERN APPROACHES
UNIT ONE
0.5 +/-
ACRES
AIRPLANE
HANGER
WORKSHOP
AREA
48'-0"
42'-0"
18'-0"
15'-0"
46'-4"
33'-9"
49'-0"
BONANZA COURT
25'-0" FRONT SETBACK
61'-8"
6'-0"
11'-0"
29'-11"
HOUSE
46'-2"
25'-0" FRONT SETBACK
190.63'
33'-7"
50'-4"
65'-2"
20'-0"
50'-0"
SISTER'S WELCOME ROAD (CR 341)
SCALE: 1" = 25'-0"

AMERICAN STEEL BLDGS
2865 PLUMMERS COVE RD. SUITE 3
JACKSONVILLE, FL. 32223

DATE: 9/30/05

HAROLD FRANKEL
Re: JOB NO. 15070R1
BUILDING SIZE:
WIDTH : 47.75 ft.
LENGTH : 60 ft.
EAVE HT : 16 ft.

JOBSITE : LAKE CITY, FL

To Whom It May Concern:

This is to certify that the above referenced building is designed in accordance with the order documentation, the Ninth Edition of the American Institute of Steel Construction (AISC) "Manual of Steel Construction" and the 1986 Edition of American Iron and Steel Institute (AISI) " Cold Formed Steel Design Manual. "The basic loads of the subject building meet or exceed the minimum county climatic data as published in the 1996 edition of the MBMA " Low Rise Building Systems Manual ".

The criteria for application of design loads are follows
Governing Code : FBC 04

Roof Dead Load : 2 psf plus wt. of metal bldg structure

Live Load based on the tributary area :

0 - 200 sq. ft.....20 psf
201 - 600 sq. ft.....16 psf
over 600 sq. ft.....12 psf

Collateral Load	: 0 psf	Roof Snow Load	: 0 psf
Wind Load (3 sec gust)	: 100 mph	Snow Exp. Fac	: N/A
Enclosure Type	: Closed	Snow Imp. Fac.	: N/A
Wind Exp. Cat	: B	Seismic Coef SDS	: .08
Wind Imp. Factor	: 1.00	Seismic Coef SDI	: .04
Ground Snow Load	: 0 psf	Seismic Imp. Fac1	: 1.0

This Letter of Certification applies solely to the building and its component parts as furnished by the Metal Building Manufacturer. Doors, windows and louvers are not structural components of the building. It is the responsibility of the owner to determine if wind lock accessories are supplied if required. Certification specifically excludes any foundation, masonry, or general contract work.

Sincerely,

Charles W. Embden

CHARLES W. EMBDEN, P.E.

10/7/05

AMERICAN STEEL BLDGS
2865 PLUMMERS COVE RD. SUITE 3
JACKSONVILLE, FL. 32223

DATE: 9/30/05

HAROLD FRANKEL
Re: JOB NO. 15070R1
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201 - 600 sq. ft.....16 psf
over 600 sq. ft.....12 psf

Collateral Load	: 0 psf	Roof Snow Load	: 0 psf
Wind Load (3 sec gust)	: 100 mph	Snow Exp. Fac	: N/A
Enclosure Type	: Closed	Snow Imp. Fac.	: N/A
Wind Exp. Cat	: B	Seismic Coef SDS	: .08
Wind Imp. Factor	: 1.00	Seismic Coef SDI	: .04
Ground Snow Load	: 0 psf	Seismic Imp. Fac1	: 1.0

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Sincerely,

Charles W. Embden

CHARLES W. EMBDEN, P.E.

10/7/05

AAMA/NWWDA 101/I.S.2-97
TEST REPORT SUMMARY

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window

Title of Test	Results
Rating	H-R40 52 x 72
Overall Design Pressure	+45.0 psf -47.2 psf
Operating Force	11 lb max.
Air Infiltration	0.13 cfm/ft^2
Water Resistance	6.00 psf
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess, Technician

MAH:nlb

ALLEN N. REEVES
CERTIFICATE
NO. 19354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Allen N. Reeves
1 APRIL 2002

Architectural Testing

AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
650 West Market Street
P.O. Box 370
Gratz, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" high

Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Allen N. Reeves
1 APRIL 2002

REEVES
No. 19354
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock with keeper		Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES
CERTIFICATE
NO. 19[illegible]
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	11 lbs	30 lbs max
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.13 cfm/ft^2	0.3 cfm/ft^2 max
	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 25.9 psf (positive)	0.42"*	0.26" max.
	@ 34.7 psf (negative)	0.43"*	0.26" max.
2.1.4.2	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 38.9 psf (positive)	0.02"	0.18" max.
	@ 52.1 psf (negative)	0.02"	0.18" max.

***Note #1:** The tested specimen meets the performance levels specified in AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

**Exceeds L/175 for deflection, but passes all other test requirements.*

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES
CERTIFICATE
NO. 1837
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Test Specimen Description: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	Deglazing Test (ASTM E 987) In operating direction at 70 lbs		
	Meeting rail	0.12"/25%	0.50"/100%
	Bottom rail	0.12"/25%	0.50"/100%
	In remaining direction at 50 lbs		
	Left stile	0.06"/12%	0.50"/100%
	Right stile	0.06"/12%	0.50"/100%
	Forced Entry Resistance (ASTM F 588-97)		
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.00 psf	No leakage	No leakage
	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 33 seconds)		
	@ 45.0 psf (positive)	0.47"*	0.26" max.
	@ 47.2 psf (negative)	0.46"*	0.26" max.

**Exceeds L/175 for deflection, but passes all other test requirements.*

Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
@ 67.5 psf (positive)	0.05"	0.18" max.
@ 70.8 psf (negative)	0.05"	0.18" max.

Allen N. Reeves
1 APRIL 2002

ALLEN N. REEVES
CERTIFICATE NO. 19854
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Technician

Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002

MAH:nlb
01-41134.01

HI-FOLD DOORS

...the ONLY bi-fold doors with high-clearance advantages

As the Hi-Fold Door rises (right), the smooth-rolling steel wheels on its exclusive auxiliary arms contact the jamb posts to guide and support the door into the full-open position. When the door is closed, the auxiliary arms store themselves against the jamb posts and remain flush with the building until the next operation.

Automatic Latching Video

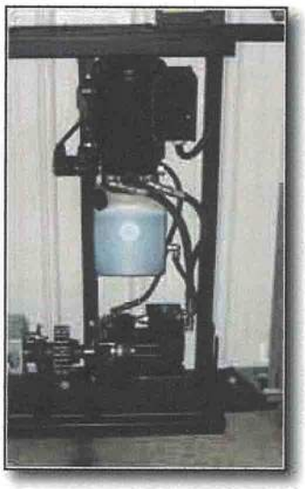
Hand

HI-FOLD DOORS

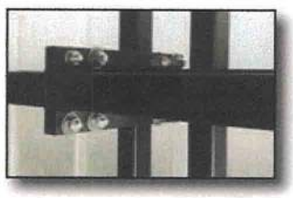
...the ONLY bi-fold doors with high-clearance advantages

New

High speed, hydraulically-powered operator option (patent pending)



Get the speed and power only a hydraulic operator can offer. A two horsepower electric motor runs a hydraulic pump and supplies pressure to a hydraulic wheel motor. System comes complete with a safety brake to lock door in position in the event of hydraulic pressure loss. System is shipped fully assembled, wired, plumbed and tested. Maintenance free operation in all climates.

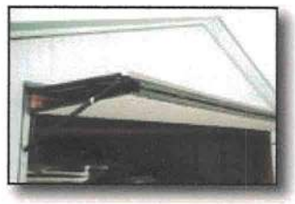


Strong, welded-steel frame assures long life.

Hi-Fold Door panels are built with double, rectangular, structural-steel tubes at the center hinge line (2" x 4" on doors to 46' wide; 2" x 6" on wider doors) and 1 1/2" or 2" square structural-steel tubing, depending on door size, for the remainder of the frame. Doors over 46' wide are made in two sections that bolt together through super-strong, 1/2" steel butt-plate joints. Strong mounting hinges (4,5,7 or 9, depending on door width) are welded to the top rail.

Welded-steel trusses withstand strongest wind loads.

All Hi-Fold Doors receive one to three inside trusses. Made of heavy-gauge, steel tubing with angle-iron webbing, they provide extra strength to control wind-load deflection when the door is closed and to help the "dogleg" center



Double-strength center hinge design eliminates sagging.

Unique, "dogleg" center hinge design, formed by the rectangular-steel members on both the top and bottom Hi-Fold Door panels, provides a "double-strength" beam that virtually eliminates sagging in the full-open position. This strength, combined with the strength of the welded-steel trusses on the inside of the door, provides even greater resistance to wind-load when the door is closed. A floor cane-bolt on all doors over 40' wide further resists wind-loads.

HI-FOLD DOORS

...the ONLY bi-fold doors with high-clearance advantages

Applications Gallery

Hi-Fold Doors fill many needs!

You won't go wrong with quality-built Hi-Fold Doors! They're the only bi-fold doors with high-clearance advantages, requiring less overhead space. Made of heavy-duty steel tubing in sizes up to 70-feet wide and 20-feet high, their rugged, all-welded construction prepares them for many years of day-in and day-out use with minimum maintenance. Cost-saving, standard bi-folds are also available for installations where Hi-Fold advantages are not required. A dependable, three year warranty covers all materials and workmanship.

You may click to open any album by desired subject below, or use the *Search* feature to look for a sample application that fits your needs. The *Slideshow* will provide a random view of the sample application photos.

Search:

4 albums, 12 images [slideshow] [login]

Aircraft Hangars

Last changed on 12/22/04. This album contains 7 items

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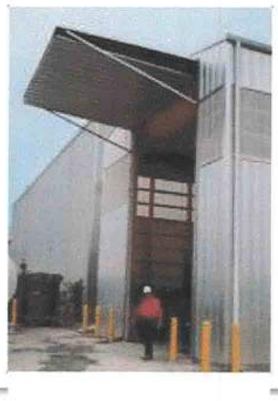

Industrial Buildings

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Farm Buildings

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HI-FOLD DOORS

...the ONLY bi-fold doors with high-clearance advantages

Aircraft Hangars

7 images in this album [slideshow] [login]

Gallery: Applications Gallery

Viewed: 275 times.

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Viewed: 295 times.

Viewed: 277 times.

Forty-two Hi-Fold doors are installed in three T-hangars

Viewed: 278 times.

Viewed: 254 times.

Eight T-hangar buildings, equipped with Hi-Fold doors with walk-in doors.

Viewed: 261 times.

Gallery: Applications Gallery

HI-FOLD DOORS

...the ONLY bi-fold doors with high-clearance advantages

HI-FOLD DOOR SPECIFICATIONS

- **Standard door sizes** - 12' to 70' clear opening width: heights to 20' clear.
- **Main door structure** - Double 2" x 4" Class A500 14-gauge structural center hinge line tubing on doors to 46' wide and double 2" x 6" Class A500 on larger doors.
- **Door frame** - 14-gauge, welded, Class A513 square steel tubing: 1-1/2" x 1-1/2" on doors up to 46' wide: 2" x 2" on doors over 46' wide. Door frame unitized on doors to 46' x 18'. Doors over 18' high and over 46' wide built in right and left halves. Doors over 18' high may have the top and bottom horizontally divided to facilitate freight.
- **Door trusses** - Heavy-gauge, steel tubing with angel iron webbing: 1 to 3 horizontal trusses, 6 1/2" to 12" deep, depending on door width, height and load requirements.
- **Door finish** - All doors primed with black water base oxide and painted with black ester enamel epoxy.
- **Hinges** - Strong factory welded leaf type hinges are installed on the horizontal centerline and top of the door. Top hinge can be ordered with 9-1/4" wide leaf up for wood buildings or 3" wide leaf up for steel buildings. Five top hinges on doors to 34' wide, seven hinges on doors over 34' and nine hinges on doors over 56' wide and 16' tall.
- **Auxiliary arms** - Patented, high-clearance door support arms made of heavy-gauge square-steel tubing with self-concealing chain followers.
- **Wheels** - roller wheels on door bottom and Auxiliary Arms are solid steel with sealed roller bearings inserts.
- **Drive unit** - 1 h.p. to 2 h.p. motor with 45 to 1 speed reducer. Electric brake installed on all doors. Jack shaft with dual chain drives supplied on all doors. Motor mounted to side on doors over 34' wide.
- **Bottom follower system** - Hold bottom of door against building with door closed. Rubber bottom seal - 3" space between door frame and finished floor sealed with standard 12" wide bottom seal.
- **Floor cane-bolt** - Factory installed at center of door over 40' wide. Cane-bolt slides thru sleeve on door into a hole drilled in the floor.
- **Warranty** - 3 years on materials and workmanship.

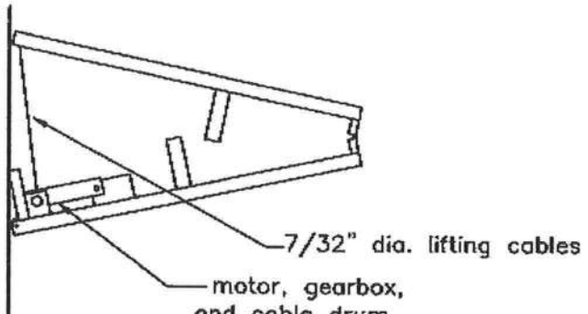
Optional Features

- **Extra-heavy duty trussing** - Standard wind load can be increased or special demands provided.
- **Rubber top seal** - Standard 9" wide rubber roofing membrane with ultraviolet inhibitor to weather proof top of door.
- **Weather-strip package** - Supplied in bulk on a per foot basis for sealing between door frame and building jamb on vertical and horizontal surfaces and at center hinge area.
- **Jamb reinforcement rail** - 10 gauge galvanized steel 5" wide, suitable for doors to 46' wide, 3/16" oxide primed plate 5-1/2" wide available for doors wider than 46'. Wood or concrete jamb buildings require a reinforcing plate for operation.
- **Automatic "knee action" jamb latch** - Patented drive shaft driven latch reaches out and pulls door securely closed before motor shuts off. Automatic cane bolt supplied on doors over 40' wide. Single location latches deleted. Safety edge recommended.
- **Safety edge** - Full width sensing switch to automatically stop and reverse a closing door if obstacle is encountered. Attaches to bottom of door prior to bottom seal.
- **Dead-man operating control** - NEMA

End Views of Bi-fold Door

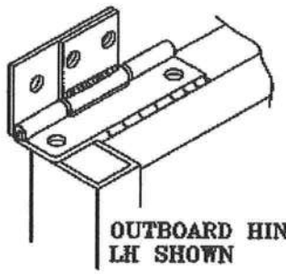
Closed View

Open View

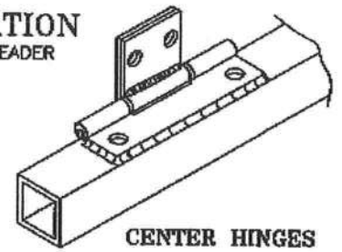


When door is open it is held up by 4 independent lift cables

HINGE INSTALLATION
FOR STEEL STRUCTURE HEADER

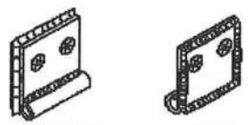


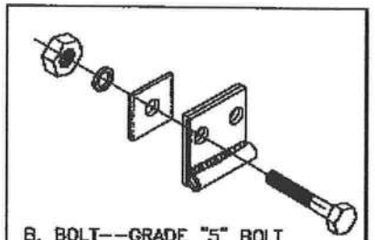
OUTBOARD HINGES
LH SHOWN



CENTER HINGES

A. WELD 4 SIDES AND HOLES ALL AROUND.





B. BOLT--GRADE "5" BOLT
2 BOLTS PER HINGE
DRILL FOR HINGE USING HINGE AS GUIDE

ATTACHENT TO STUB COLUMN:
EACH OUTBOARD HINGE IS SUPPLIED WITH AN EXTENSION FOR 2-BOLT ATTACHMENT TO THE VERTICAL COLUMN.
THERE ARE TWO OPTIONS:
2 BOLTS REQUIRED AT EACH HINGE
OR
WELDING THE HINGE TO THE STUB IS ACCEPTABLE.

NOTE:
WELD THE FULL PERIMETER OF THE HINGE. THE DOOR MUST BE PARTIALLY OPEN TO COMPLETE THE WELD ACROSS THE HINGE BOTTOM. OUTBOARD HINGES WILL EXTEND BEYOND THE VERTICAL COLUMN, MAKING IT NECESSARY TO WELD A VERTICAL LAP JOINT. WELD ON BACK OF HINGE AT COLUMN.

Post-it® Fax Note 7671	Date 12-2	# of pages ▶ 6
To Harold Frankel	From Bill	
Co./Dept.	Co.	
Phone #	Phone #	
Fax # 386-752-9542	Fax #	

BI-FOLD DOOR CORPORATION RIVER FALLS, WI	
PREPARED FOR: Harold Frankel 100 MPH EXPOSURE C WIND LOAD PER FBC	
QUOTATION NO.: FRA1560205.P02	DRAWN BY: TOL
CLEAR OPENING: 42'-0" X 12'-0"	DATE: 12-1-2005
DOOR SERIES: HF61W2	SCALE: NONE
DRAWING NO.:Hinge - FRA1560205.P02	SHEET: 1 of 1

Door Force/Strength Analysis

for Harold Frankel, 42'-0"x12'-0"clear opening
reference FRA1560205.P02 Quote
12/1/2005

page 1/2 This sheet looks at closed door (vertical)

clear width, ft	42
height, ft	12
area, ft^2	504.0
design wind P, PSF	22.6
total force on door, lb	11390

note: hinges at mid-height of door are welded;
hinges at top of door are welded to door and bolted to building
This spreadsheet looks at top hinges, outermost hinges only

Hinge Strength Analysis

	Weight load alone	Wind load alone	Weight and Wind load
	door vertical, both hinge plates vertical		
total wind load on door, lbs, see small table, above		11390	
fraction of windload on all top hinges		0.16	
average wind load, all hinges		1822	
door weight, lbs	2777		
number of hinges	7	7	
avg load per hinge, lb	397	260	
multiplier for end hinge	1.75	1.75	
vertical load on end hinge, est., lb., parallel to plate	694	0	0
horizontal load on end hinge, est., lb., perp. to plate	0	456	0
combined load on end hinge, lb	0	0	830

Maximum Force to Yelding, lbs		Lowest Safety Factors		
A. bolts in tension, perpendicular to hinge plate	9547		21.0	
B1. Bolts in shear, parallel to hinge plate	8645	12.5		
B2. Weld in shear, parallel to hinge plate	10541			
C. bearing stress, holes, parallel to hinge plate	11812			
D. tear-out, parallel to hinge plate	11550			
E. tear apart tube, welded, any direction	19230			
F. pin shear, any direction	8247	11.9	18.1	9.9
See separate hand calculations, for cases A-F, on three pages, dated Dec 3, 2003		Lowest Safety Factor is 9.4, acceptable		

This analysis looks at worst case loading
and compares with forces which cause yielding
in various modes, and calculates safety factors.

JAMB: **Horizontal force on door jambs with door closed:**

decimal portion of wind load on trusses A, B, and hinge tubes		0.72	
conversion to force pushing out on door: x 7/8:		0.875	
decimal portion of force applied to jamb instead of			
being transferred up or down by HiFold door vert frame.		0.6	
force on each building frame door jamb, at midheight, lb		2153	

Door Force/Strength Analysis

for Harold Frankel, 42'-0"x12'-0"clear opening
reference FRA1560205.P02 Quote

page 2/2 This sheet looks at open door: in up position

note: hinges at mid-height of door are welded;
hinges at top of door are welded to door and bolted to building
This spreadsheet looks at top hinges, outermost hinges only

Hinge Strength Analysis

door clear opening is 12'-0", top of door from finish floor is 13'-2", headroom equals 1'-2"

		Note this column is same as on page 1	Refer to item G in hand calcs.	
		Weight load alone (vertical)	horizontal force	combined force on top hinge
		door open		
door weight, lbs			2777	
multiplier of door weight, to get horiz force, lb*			1.35	
total horizontal force on all top hinges, lb			3748.95	
----note this is also force on building frame				
door weight, lbs		2777		
number of hinges		7	7	
avg load per hinge, lb		397	536	
multiplier for end hinge		1.75	1.75	
vertical load on end hinge, est., lb., parallel to plate		694	0	0
horizontal load on end hinge, est., lb., perp. to plate		0	937	0
combined load on end hinge, lb		0	0	1166
Maximum Force to Yelding, lbs		Lowest Safety Factors		
A. bolts in tension, perpendicular to hinge plate	9547		10.2	
B1. Bolts in shear, parallel to hinge plate	8645	12.5		
B2. Weld in shear, parallel to hinge plate	10541			
C. bearing stress, holes, parallel to hinge plate	11812			
D. tear-out, parallel to hinge plate	11550			
E. tear apart tube, welded, any direction	19230			
F. pin shear, any direction	8247	11.9	8.8	7.1
See separate hand calculations, for cases A-F, on three pages, dated Dec 3, 2003		Lowest Safety Factor is 7.1, acceptable		

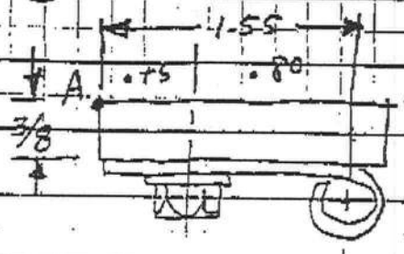
This analysis looks at worst case loading
and compares with forces which cause yielding
in various modes, and calculates safety factors.

JAMB	horizontal force on each side of door, at roller/jamb		1874.475	
CABLE	door weight, from above, lb		3617	
	decimal portion of door weight as cable weight, worst case*		0.6	
	total force on all 4 lifting cables, with door in up position, lb		2170.2	
	average load per cable assuming equal distr., lb		543	
	multiplier to end cables		1.75	
	load on end cables, lb		949.5	
	strength of 7/32" cable, lb		5600	
	safety factor, wi cable, compare to minimum of 5		5.9	

HINGE STRENGTH ANALYSIS

Note three Grade 5, 3/8 ϕ bolts

(A) Bolts in tension



For force perpendicular to hinge plate.

Force $P_\perp$ is multiplied to bolts due to pivot point @ A.

$$F_{bolt} = \frac{P_\perp \times 1.55}{.75} = 2.07\,P_\perp$$

In tension:

$$\sigma_{T\,bolt} = \frac{F_{bolt}}{3A_{cc}} = \frac{2.07(P_\perp)}{3(.0775\,in^2)}$$

note Gr 5 bolt has S_y = yield strength of 85ksi ⇒ force to yield = $P_\perp$

$$P_\perp = \frac{S_y \times 3 \times .0775\,in^2}{2.07} = \boxed{9542\,lb}$$

(B1) Bolts in shear due to parallel-to-plate force P_p

$$\tau = \frac{P_p}{3 \times A_{shear}} = \frac{P_p}{3 \times .0671\,in^2}$$

Shear yield strength = $\frac{S_{yield}}{2}$ = 42,500 psi

Failure, in shear yield = $P_p = 42{,}500\,psi \times 3 \times .0671 = \boxed{8645\,lb}$

$P_\perp$ 2 — see next sheet

(C) Bearing Stress in bolt holes, in 3/8" mat'l (see figure above)

for force P_p parallel to plate:

$$\sigma_c = \frac{P}{3xA} = \frac{P_p}{3 \times 3/8 \times 3/8}$$

Note: 3/8 bolt is in 1/2 ϕ hole, but assume 3/8 ϕ hole (conservative)

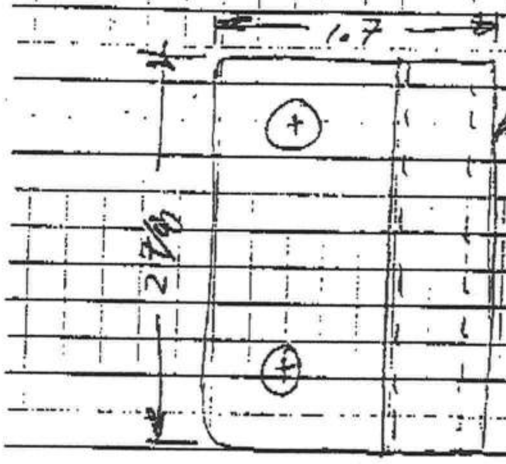
yield strength S_y = 28,000 psi

Force to yield = P_p = 28ksi x 3 x 3/8 x 3/8 = $\boxed{11{,}812\,lb}$

See hinge calcs.xls spreadsheet
Mitchael Brito Hi fold Door, 16X60

HINGE STRENGTH ANALYSIS 12/3/03

(B2) Weld in shear
for force parallel to plate,
Will look @ single center section, which is conservative compared to wider end hinge section. Holes are ignored.



Assume welded all around perimeter

$$P_{erim} = 2\left(2\tfrac{7}{8}\right) + 2(1.7)$$

$$= 9.15''$$

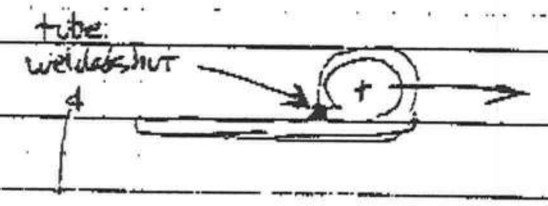
Welds in shear:

$$F_{allowable} = P_p = w \times P_{erim} \times 9600$$

(w = width of weld fillet; 9600 = allowable #/in)

$$= .12\,(9.15)\,9600$$

$$= \boxed{10541\ lb}$$



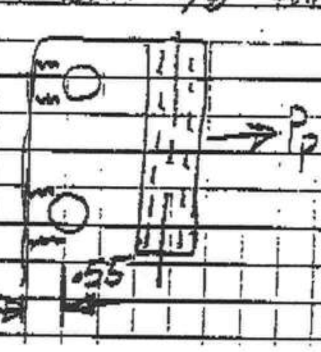
(E) Tear apart Hinge Tube, with tube welded shut
.12 thick mat'l × 2 7/8" long
Area in tension = 2 × .12 × 2 7/8 = .69 in²

$$\sigma_T = \frac{P}{A} \Rightarrow P_{allowed,\ to\ yield} = S_y \times A$$

$$= 28{,}000\,psi \times .69\ in^2$$

$$= \boxed{19230\ lb}$$

(D) Tear out of Holes – for force // to plate
Using only 2 holes (which align w/ center section)
with 3/8 thick section



Shear yield = 28,000 / 2

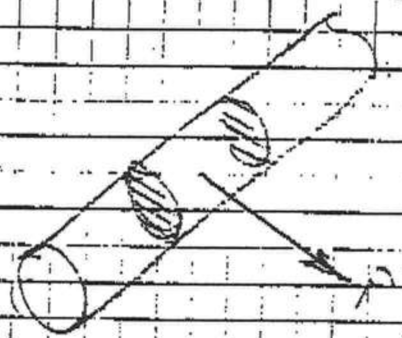
$$\tau = \frac{P_P}{A} = \frac{P_P}{2 \times 2 \times .55 \times \frac{3}{8}}$$

$$P_{P\,allowed,\ to\ yield} = 14{,}000\,psi \times 2 \times 2 \times .55 \times \tfrac{3}{8}$$

$$= \boxed{11550\ lb}$$

HINGE STRENGTH ANALYSIS 12/3/03 [illegible]

(F) Pin Shear, any direction
(2 shear areas)



$$\tau = \frac{P}{2A_{cs}}$$

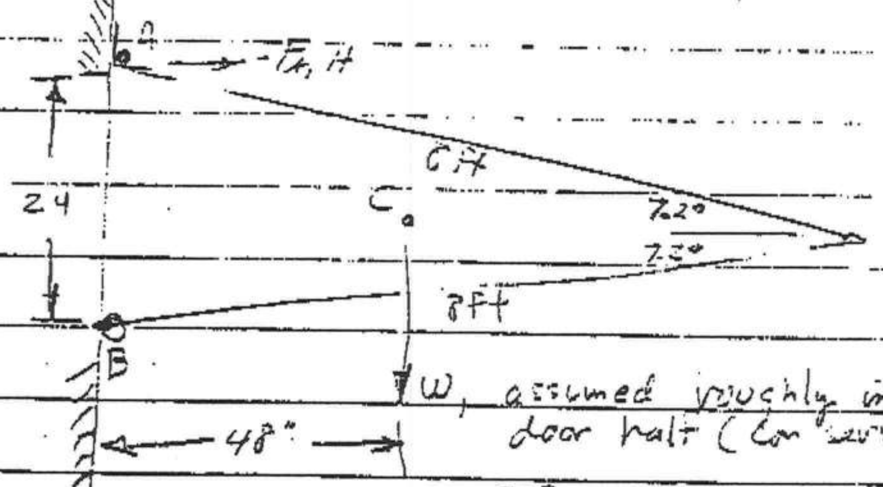
Shear yield strength = 42,000 psi / 2

$$P_{\text{to shear yield}} = S \times 2A$$

$$= 21{,}000 \text{ psi} \times 2 \times \frac{\pi}{4}(.5)^2$$

$$= \boxed{8247.16}$$

(G) Horizontal Force on Top Hinge



$$\Sigma M_o = 0 = W \times 48 - F_{A\text{-}H} \times 24$$

$$\Rightarrow F_{A,H} = W \times \frac{48}{24} \quad \text{or} \quad W \times \underline{\underline{2}}$$

multiplier

END OF HAND CALCULATIONS

HI-FOLD DOOR CORPORATION
N6170 1070th Street, River Falls, Wisconsin 54022
Telephone: (800) 443-6536; FAX: (715) 262-3998
www.hi-fold.com

FRA1560205.P02

Frankel, Mr. Harold
Rt #18, Box 18802

September 6, 2005

Lake City, FL 32025
Att: Harold Frankel
Door Number: 01 Product Number: H4212-2

Called Clear Opening Width	42	Feet	0.000	Inches
Called Clear Opening Height	12	Feet	0.000	Inches
Measured Frame Height	12	Feet	11	Inches
Panel Height	6	Feet	5.5	Inches
Top of Door AFF	13	Feet	2	Inches
Base of Stub column AFF	13	Feet	0	Inches
Clearance Height Required	13	Feet	6	Inches
Measured Frame Width	42	Feet	8	Inches
Clearance Width Required	42	Feet	10	Inches
Arm Location Measurement			30	Inches

Hinge location is based on hinge quantity - Hinge Reverse

Hinge Quantity = 7 If Hinge "B" or "C" is blank then go to "D"

	Standard Wood	Reverse Steel
Edge of door to first Hinge	4.630	2.500
Hinge "A" Measurement	45.370	47.500
Hinge "B" Measurement	132.000	
Hinge "C" Measurement	0.000	
Center of Door to first hinge and Hinge "D"	74.000	

Active weights for the Hi-Fold Door are as follows:

Door Closed

Dead Weight	2203 ~~1873~~	LBS.
Estimated Weight for Exterior covering	504	LBS.
Total Dead Weight less any options	2777 ~~2377~~	LBS.
Wind Load Transferred to Vertical Column	75%	
Wind Load Transferred to Header/hinge mount point	25%	

Door Open - Tends to pull away from building at hinge line.

Horizontal Component - 1.35 times Dead Weight	3749 ~~3209~~	LBS.
Number of hinges	7	Hinges
Horizontal Tension in Pounds per Hinge	536 ~~458~~	LBS.
Horizontal compression At Wheel on each vertical Column.	~~1604~~ 1874	LBS.

. . . the only Bi-Fold Door with high clearance advantages!

HI-FOLD DOOR CORPORATION

N6170 1070th Street, River Falls, Wisconsin 54022
Telephone: (800) 443-6536; FAX: (715) 262-3998
www.hi-fold.com

Quotation and Purchase Agreement

September 6. 2005
Frankel. Mr. Harold
~~Rt #10, Box 10002~~ 1703 SW SISTERS WELCOME RD
Lake City, FL 32025
Attention: Harold Frankel
Phone: 386-752-9592

Quotation Number: FRA1560205.P02

Ship To:
Lake City. FL
Attention:

This Quotation shall be in effect for 30 days. If this is acceptable. please sign it. the attached transmittal print and the information sheet and return one copy of each to us. Each must be returned for the manufacturing of your door to start. This Agreement includes the terms shown on the reverse hereof.

Hi-Fold Doors: Size is Measured Clear Opening after installed and in open position - FOB River Falls, Wi 54022

Product Number	Door Number	Product Description and Size	Quantity	Unit Price	Amount
H4212-2	01	42' 0.000" Wide X 12' 0.000" High Standard	1	3,975.00	3.975.00
Special	Increased loading	100 MPH wind load per FBC 2001 exp C 3 sec gust	1.0	680.000	680.00
Option	Stamped Print	Engineer verified wind load	1.0	312.000	~~312.00~~
Option	Top Seal	12" wide rubber top seal	43.0	1.050	45.15
Option	Weather Stripping	3/4" X 5/8" open cell foam with adhesive back	69.0	.150	10.35
				Package Total	5,022.50 4710.5
				Freight Total	775.00
				Quote Total	5,797.50 5485.5
				Down Payment	1,255.63
				Balance Due	4,541.88

Anticipated Shipping Date: 4-8 Weeks from receipt of signed agreement.
Shipping Method: Contract FT

All doors are custom made for your specific application; therefore, our terms are 25% down payment with order. Remaining balance C.O.D. Advance payment by check or bank letter of credit including freight will save C.O.D. charges. Open account terms require management authorization and credit department approval. Any applicable taxes will be added unless exemption certificate is supplied or on file.

Customer Signature Harold J Frankel Date: 9/8/05

Sincerely,
Hi-Fold Door Corporation
Bill Bakalich
Bill Bakalich
Marketing Manager

Hi-Fold Acceptance _____ Date: _____

Post-it® Fax Note 7671	Date 9-6	# of pages ▶ 6
To Harold Frankel	From Bill	
Co./Dept.	Co.	
Phone #	Phone #	
Fax # 386-752-9592	Fax #	

. . . the only Bi-Fold Door with high clearance advantages!

PREPARED FOR Harold Frankel

QUOTE NUMBER FRA1510205.R02

HINGE LOCATION MEASURED FROM DOOR C/L:
A 47½" B 132" C 74"

HINGE QUANTITY 7

DOOR CLEAR OPENING HEIGHT 12'-0"

MEASURED DOOR FRAME HEIGHT 12'-11"

DOOR PANEL HEIGHT FOR SKIN:
TOP 6'-9½" BOTTOM 6'-5½"

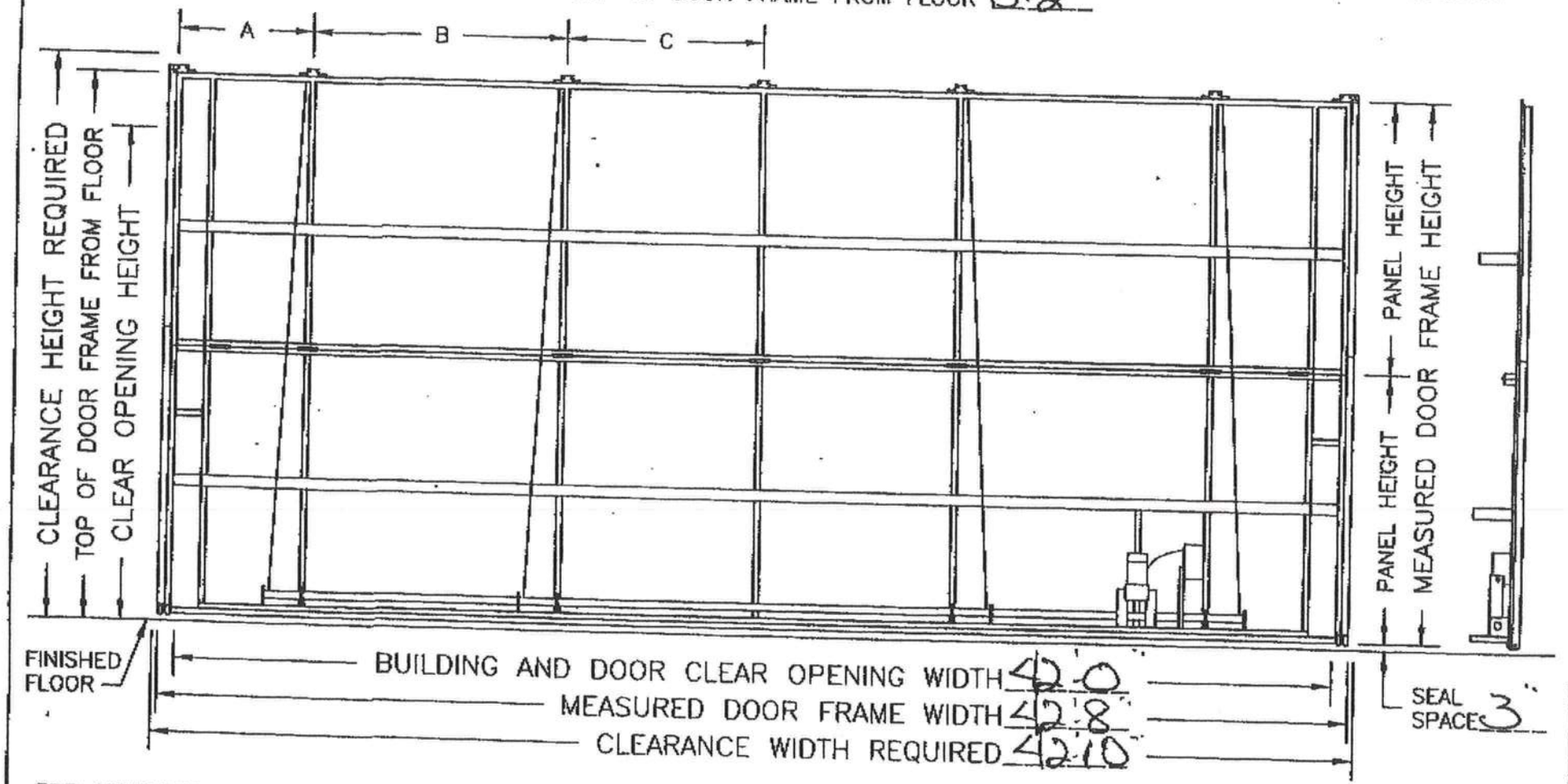
TOP OF DOOR FRAME FROM FLOOR 13'-2"

BUILDING MEASUREMENTS BELOW:

BOTTOM OF STUB COLUMN 13'-0"

CLEARANCE HEIGHT REQUIRED 13'-6"

CLEARANCE HEIGHT IS BOTTOM OF SHEETING AND TRIM ABOVE DOOR



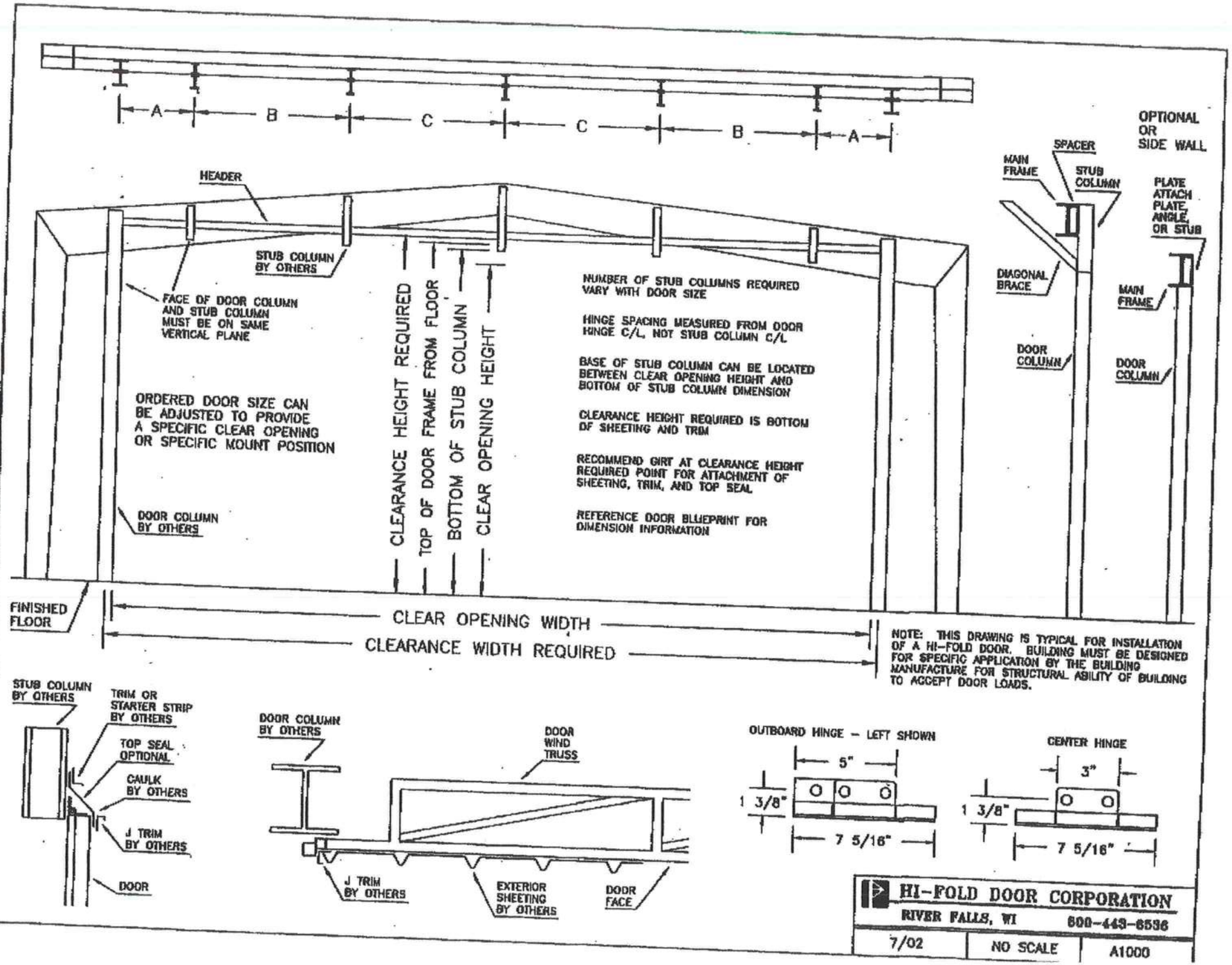
FOR APPROVAL PLEASE RETURN ONE SET OF DRAWINGS FOR PROMPT SCHEDULING

APPROVED FOR CONSTRUCTION AS SHOWN ✓ AS NOTED ____

DATED 9/9/05 SIGNED Harold J Frankel

HI-FOLD DOOR CORPORATION
RIVER FALLS, WI 800-443-6536

6/01	NO SCALE	HF42W2-01



HI-FOLD DOOR CORPORATION
RIVER FALLS, WI 800-443-6536

7/02	NO SCALE	A1000

1. All doors shipped fully assembled. Units over 26' to 46' wide may require a vertical split. All doors over 48' must be vertically split. Units over 18' in height may require each half to be split horizontally. Vertically split doors to require field bolting, horizontally split doors to require field welding.
2. Doors over 46' wide supplied with 1.5 or 2 horsepower motor wired for 220 volt single phase and require separate circuit with circuit interrupt (provided by others) to protect the required 13 amps. Most doors 46' and under supplied with a 1 horsepower motor wired 110 volts and provided with individual circuit protection to protect the required 20 amps. "Up-Stop-Down" control wired 24 volts with cable long enough to place control five feet above floor on the side of door to be specified by customer. Flexible power cable 4' longer than height of door and requires service at that point. Hi-Fold electrical system meets the requirements of the National Electrical Code for typical applications hence it is the customers responsibility to provide instructions to meet special applications and local codes.
3. The electrical system requires a grounded circuit and substantial wiring. Extension cords and generators may not be adequate for door operation, and are not recommended.
4. Optional jamb reinforcement rails are available for wood or concrete jamb building applications.
5. Hi-Fold Door exerts considerable horizontal loads on the building structure in the open position. Loads specific to this application are provided to the purchaser. Purchaser is responsible to insure that the buildings structural design is capable of handling the imposed loads.
6. Operating times vary with door size. Contact Hi-Fold Door for operating speed for a particular size.
7. Purchaser required to inform Hi-Fold Door if any weight in addition to standard 26 gauge roll formed steel exterior covering (provided by others) is to be applied to and lifted by door operating system. Excessive weight will void warranty.
8. Hi-Fold Door is welded steel primed with a water base oxide primer and painted with black epoxy ester enamel and is not intended to by left exposed to the elements. In a highly corrosive atmosphere it may be necessary to field apply finish paint. Operating components must be checked frequently and properly lubricated as necessary.
9. Defective components will be replaced upon receipt of the defective component. Immediate replacement of defective components will be made by UPS or freight shipment on a COD basis with reimbursement upon return of the defective part following analysis and evaluation if deemed necessary by Hi-Fold Door.
10. Field alteration or repair of a door must be authorized in advance in writing. Unauthorized alteration will void warranty.
11. Hi-Fold is not responsible for the installation of the Hi-Fold Door, nor for installation errors or normal installation adjustments as might be required. Compliance with all applicable state and local codes is the sole responsibility of purchaser. Optional walk doors may not meet state or local life safety code requirements for exit doors.
12. Hi-Fold will attempt to answer any installation problem by phone. Factory service will be supplied when all field effort can not rectify the problem. Hi-Fold must be allowed reasonable time within which to travel to the job site. Abnormal cost incurred to answer demand for immediate service must be paid by purchaser. Purchaser must pay for total service cost if Hi-Fold is not found responsible for defect or defective operation.
13. Customer must thoroughly check shipments for damage. Significant damage is reason to refuse shipment of the product. Minor damage must be noted on the bill of lading and claim filed with the carrier. Hi-Fold is not responsible for freight damage. Neither Hi-Fold nor the trucking company can be held responsible for damage caused during unloading. Unloading is the responsibility of the purchaser. Shipment must be checked for road salt during winter months, if shipment comes in contact with salt it must be washed thoroughly prior to unloading truck and noted on the bill of lading.
14. Delivery to be accepted on confirmed delivery date. Any delay caused by purchaser shall require an additional amount paid to cause deposit to equal fifty percent of the order value.
15. Delivery delay exceeding 30 days caused by purchaser will require payment to equal 90 percent of order value and a storage fee may be imposed. Hi-Fold is not responsible for exposure deterioration caused by delivery delays in excess of 30 days
16. Delivery date to be confirmed when all components of the contract are received. Any change order must be received at least two weeks prior to confirmed delivery date.
17. Issuance of a purchase order alone is not adequate for door to be manufactured. This contract and size print must be signed and returned.
18. Cancellation policy: Once manufacturing has begun there will be no cancellations made. If cancellation is prior to manufacturing deposit will be returned in full, less any engineering that has been done.

Customer Signature 

G

STATE OF FLORIDA
DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 05-1058N

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.

See Attached

Notes: _____

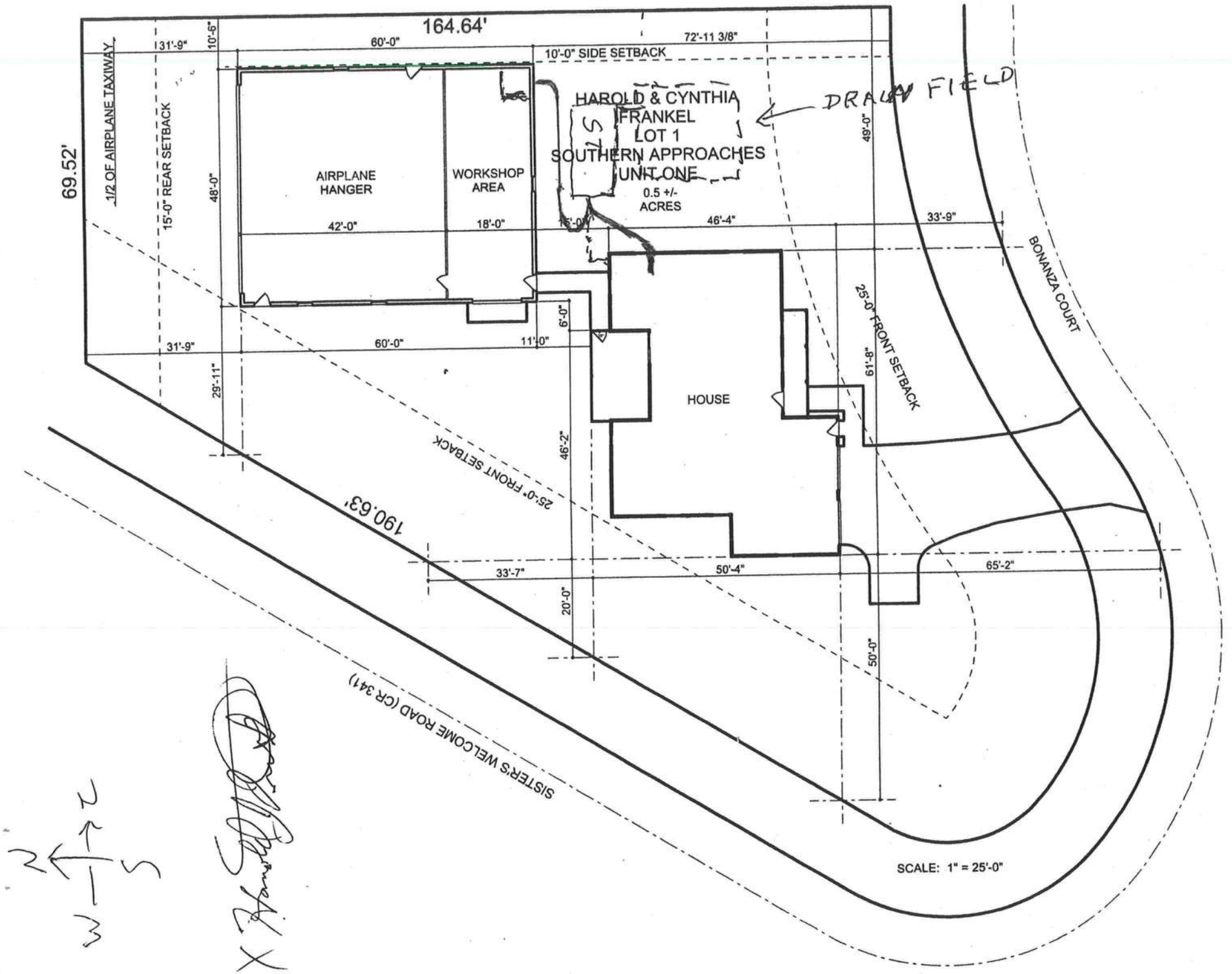
Site Plan submitted by: _____ (Signature) OWNER (Title)

Plan Approved _____ Not Approved _____ Date 10/13/05

By _____ County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

DH 4015, 10/96 (Replaces HRS-H Form 4015 which may be used)
(Stock Number: 5744-002-4015-6)


164.64'
69.52'
190.63'
1/2 OF AIRPLANE TAXIWAY
15'-0" REAR SETBACK
10'-0" SIDE SETBACK
AIRPLANE HANGER
WORKSHOP AREA
HAROLD & CYNTHIA FRANKEL
LOT 1
SOUTHERN APPROACHES UNIT ONE
0.5 +/- ACRES
DRAW FIELD
HOUSE
25'-0" FRONT SETBACK
BONANZA COURT
SISTER'S WELCOME ROAD (CR 341)
31'-9"
60'-0"
72'-11 3/8"
10'-6"
48'-0"
42'-0"
18'-0"
46'-4"
33'-9"
49'-0"
6'-0"
11'-0"
29'-11"
46'-2"
61'-8"
33'-7"
50'-4"
65'-2"
20'-0"
50'-0"
SCALE: 1" = 25'-0"

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0511-77 Date Received 11-21-05 By LH Permit # 23927

Application Approved by - Zoning Official BLK Date 29.11.05 Plans Examiner OK JTH Date 12-02-05

Flood Zone X Development Permit N/A Zoning RMF-1 Land Use Plan Map Category RES. MOD DEV.

Comments 1st Floor to be at 116.5 ft per grading plan by Engineer.

Applicants Name HAROLD J FRANKEL Phone 386-752-9592 cell# 344-3701

Address 1703 SW SISTERS WELCOME RD LAKE CITY FL 32025

Owners Name SAME Phone

911 Address 115 SW Bonanza Glen, Lake City, FL 32025

Contractors Name SAME Phone

Address

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address CHARLES W. EMBDEN JAX. FL - NICHOLAS GEISLER LKCY.

Mortgage Lenders Name & Address

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progressive Energy

Property ID Number 13-4S-16-02952-201 Estimated Cost of Construction 44,000

Subdivision Name SOUTHERN APROCES Lot # 1 Block Unit #1 Phase

Driving Directions SISTERS WELCOME RD TO BUSSINESS POINT RD ON CORNER

Type of Construction STEEL Hangar Number of Existing Dwellings on Property 1

Total Acreage .5 AC Lot Size Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 72'11" Side 10' 6" Side 29'11" Rear 31' 9"

Total Building Height 24 FT Number of Stories 1 ~~Heated~~ Floor Area 2880 Roof Pitch 4/12

STORAGE = 720

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Harold J Frankel

Owner Builder or Agent (Including Contractor)

Contractor Signature

Contractors License Number

Competency Card Number

NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

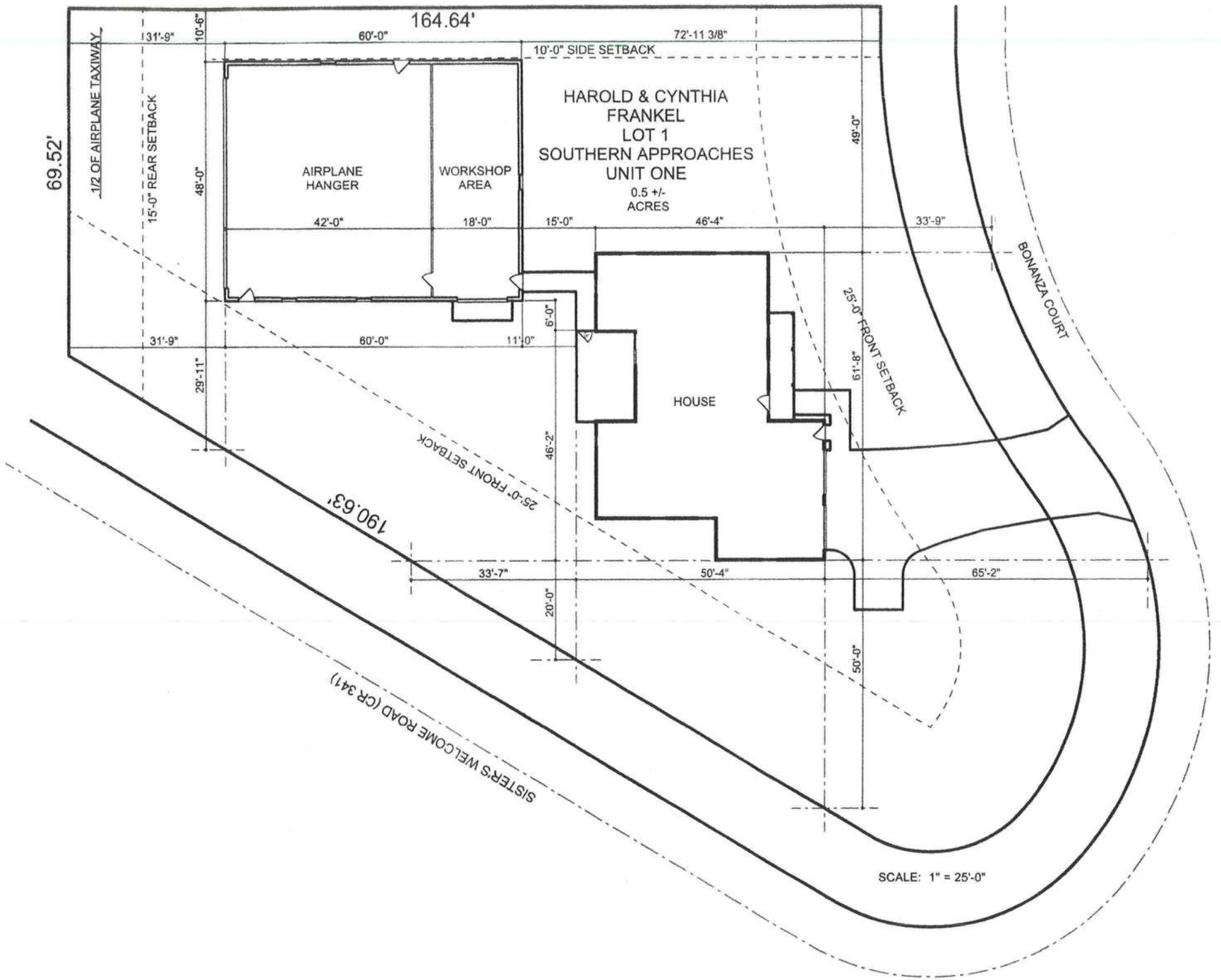
LAURIE HODSON
MY COMMISSION # DD 333503
EXPIRES: June 28, 2008
Bonded Thru Notary Public Underwriters

Sworn to (or affirmed) and subscribed before me this 21 day of November 20 05.

Personally known ✓ or Produced Identification

Laurie Hodson

Notary Signature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72'-11 3/8"
10'-0" SIDE SETBACK
HAROLD & CYNTHIA
FRANKEL
LOT 1
SOUTHERN APPROACHES
UNIT ONE
0.5 +/-
ACRES
AIRPLANE
HANGER
WORKSHOP
AREA
48'-0"
42'-0"
18'-0"
15'-0"
46'-4"
49'-0"
33'-9"
BONANZA COURT
25'-0" FRONT SETBACK
61'-8"
6'-0"
11'-0"
31'-9"
60'-0"
29'-11"
HOUSE
46'-2"
25'-0" FRONT SETBACK
190.63'
33'-7"
50'-4"
65'-2"
20'-0"
50'-0"
SISTER'S WELCOME ROAD (CR 341)
SCALE: 1" = 25'-0"

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION.

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 13-4S-16-02952-201

1. **Description of property: (legal description of the property and street address or 911 address)** LOT 1 UNIT 1 SOUTHERN APPROACHES COLUMBIA COUNTY 115 SW BONANZA GLEN - COURT
2. **General description of improvement:** NEW HOUSE & HANGAR
3. **Owner Name & Address** HAROLD & CYNTHIA FRANKEL = 1703 SW SISTERS WELCOME RD. LAKE CITY. FL. 32025 **Interest in Property** —
4. **Name & Address of Fee Simple Owner (if other than owner):** —
5. **Contractor Name** OWNER CONTRACTOR **Phone Number** (386) 752-9592
   Address —
6. **Surety Holders Name** — Inst:200502186 Date:09/08/2005 Time:11:16
   Address — DC,P.DeWitt Cason,Columbia County B:1057 P:1591
   Amount of Bond —
7. **Lender Name** — **Phone Number:** —
   Address —
8. **Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:**
   Name — **Phone Number** —
   Address —
9. **In addition to himself/herself the owner designates** — **of** — **to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) –**
   (a) 7. Phone Number of the designee —
10. **Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified)** —

NOTICE AS PER CHAPTER 713, Florida Statutes:
The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Signature of Owner

Sworn to (or affirmed) and subscribed before day of August 29, 2005

NOTARY STAMP/SEAL

DONNA L. PIEPER
MY COMMISSION # DD 128093
EXPIRES: October 20, 2006
Bonded Thru Notary Public Underwriters

Signature of Notary

DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed $25,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F .I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

TYPE OF CONSTRUCTION

( ) Single Family Dwelling
( ) Two-Family Residence
( ) Farm Outbuilding
(X) Other HANGER
( ) New Construction
( ) Addition, Alteration, Modification or other Improvement

NEW CONSTRUCTION OR IMPROVEMENT

I HAROLD J FRANKEL, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number_____

Signature 11/21/05
Date

FOR BUILDING USE ONLY

I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7).

Date 11-21-05 Building Official/Representative

From: The Columbia County Building Department
Plans Review
135 NE Hernando Av.
P. O Box 1529
Lake City Florida, 32056-1529

Reference to: Build permit application Number: 0511-77

Harold Frankel Owner/Builder lot 1 Unit 1 Southern Approaches

On the date of November 22, 2005 application 0511-77 and plans for construction of a residential aircraft hanger were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0511-77 when making reference to this application.

1. Please have Mr. Nicholas Geisler show on the plans a design detail for the interior 1 hour fire wall which will provide separation between the aircraft hanger area and the storage area.
2. Please submit product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 for all windows and doors which includes the hanger door that will be placed in the sheer walls of the hanger.
3. Please show compliance with the FBC-2004 section 412.3.1 residential aircraft hangars: An accessory building less than 2,000 square feet (186 m2) and 20 feet (6096 mm) in height, constructed on a one- or two-family residential property where aircraft are stored. Such use will be considered as a residential accessory use incidental to the dwelling. As shown on the floor plan submitted in

application 0511-77 the acceptable square footage of a residential aircraft hangar is exceed by 304 square feet.

4. Show the method of attachment of the hanger door and supports to the structure also include the method of attachment of these supports to the foundation. Provide the wind-load rating for the hanger door and all components of the hanger door.
5. Provide the manufacture specification and fire rating on the door which will be placed within the one hour fire rated wall.
6. If any penetration is made through the 1 hour fire wall, show the method to be used to maintain the firewall integrity.
7. Please provide a copy of the release, of the waste water disposal system from the Columbia County Environmental Department.
8. Please verify the requirement of Mr. Disosway grading plan for your property which stipulates that the first floor elevation of the hanger building to be elevated to 116.5 feet.

Thank you,

Joe Haltiwanger
Plan Examiner
Columbia County Building Department

#23927

The hanger next to Frankels house @ Cannon Creek Airpark is alledged not to meet wind load standards. Apparantly, the bolt patterns did not match and therefore were altered. Please check on this.

Dale

at request from Dale I made a Spotcheck at Mr. Frankel's Airplane Hanger. I Found that there we several Places where the Bolts in the Slab did not match the Main Steel Beam's holes. I spoke with Mr Frankel + he said that he would get his Engineer to approve the Alteration of the Bolts.

2.23.20

Century 21
2.23.20


2. 23. 20

2.23.20

2.23.20

2.23.20

2.23.20

2.23.20

2.23.20


2.23.20


2. 23. 20

NICHOLAS
PAUL
GEISLER
ARCHITECT
N.C.A.R.B. Certified

1758 NW Brown Road
Lake City, FL 32055
386/755-9021

17 JULY 2006

JOHNNY KEARSE, BUILDING OFFICIAL
COLUMBIA COUNTY, BUILDING DEPT.
COLUMBIA COUNTY COURTHOUSE ANNEX
LAKE CITY, FLORIDA 32055

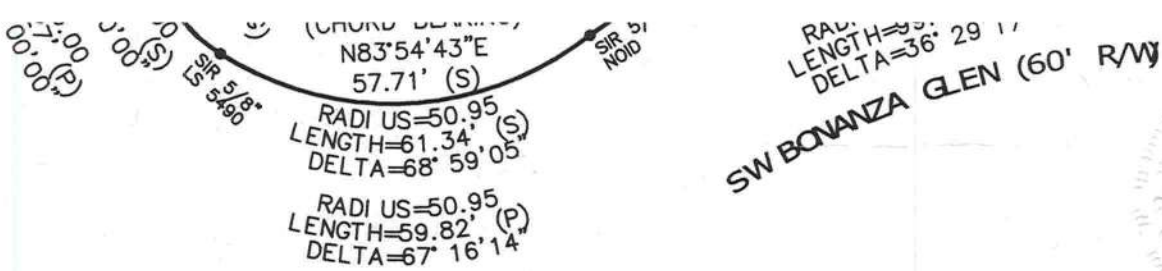
RE: HAROLD FRANKEL, METAL BUILDING
PERMIT Nr.: 23927

DEAR SIR:

PLEASE BE ADVISED THAT I HAVE INSPECTED THE ABOVE REFERENCED METAL BUILDING AND HAVE FOUND THAT IT IS CONSTRUCTED IN ACCORDANCE WITH THE ENGINEERED METAL BUILDING SHOP DRAWINGS AS PREPARED BY THE BUILDING'S MANUFACTURER. WHILE I HAVE NOT INSPECTED THE PROJECT DURING CONSTRUCTION, THE SLAB APPEARS TO BE IN GOOD CONDITION, WITH NO APPEARENT CRACKING OR SETTLEMENT.

SHOULD YOU HAVE ANY FURTHER QUESTIONS WITH THIS, PLEASE CALL FOR ASSISTANCE.

YOURS TRULY,
NICHOLAS PAUL GEISLER, ARCHITECT AR0007005



I HEREBY CERTIFY THIS SURVEY WAS DONE UNDER MY DIRECT SUPERVISION AND IT MEETS THE MINIMUM TECHNICAL STANDARDS FOR LAND SURVEYING PURSUANT TO CHAPTER 61G17-6, FLORIDA ADMINISTRATION CODE, CHAPTER 472, FLORIDA STATUES.

WILLIAM N. KITCHEN PSM 5490

William N. Kitchen
10-18-2005

SURVEYORS NOTES

1. BEARING BASED ON PLAT.
2. THIS SURVEY BASED ON LEGAL DESCRIPTION FURNISHED. THE PUBLIC RECORDS, WERE NOT SEARCHED BY THIS SURVEYOR FOR EASEMENTS, TITLE, COVENANTS, RESTRICTIONS, CLOSURES, TAKINGS OR ORDINANCES, ETC., THERE COULD BE OTHER MATTER OF RECORD THAT EFFECT THIS PARCEL.

CERTIFIED TO:
1. HAROLD FRANKEL

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER.

REV:		

WILLIAM N. KITCHEN
PROFESSIONAL SURVEYOR AND MAPPER
152 N MARION AVENUE
LAKE CITY, FLORIDA 32055
PHONE (386) 755-7786

DRAWN BY: RI	FIELD BOOK: 05406
SCALE: 1" = 30'	
SURVEY DATE: OCTOBER 5, 2005	

JOB NUMBER	SHEET
05406	1 OF 1

CLIENT: HAROLD FRANKEL

LEGEND

(P) = PLAT
(S) = SURVEY MEASUREMENT
FCM = FOUND CONCRETE MONUMENT
SIR = SET IRON ROD
LS = LAND SURVEYOR
NOID = NO IDENTIFICATION
R/W = RIGHT OF WAY

ENGINEERING DESIGN CRITERIA

AMERICAN STEEL BLDGS
Job Number: 15070R1
End User: HAROLD FRANKEL
Job Location: LAKE CITY, FL

DATE: 9/30/05
DESIGNED BY: VMM
DETAILED BY: SPW
CHECKED BY:

SPECIAL NOTE:

BUILDING IS NOT STRUCTURALLY SOUND UNTIL ALL WALL COVERING, ROOF SHEETS, AND PERMANENT BRACING IS INSTALLED. BUILDER / CONTRACTOR IS RESPONSIBLE FOR SUPPORTS OR TEMPORARY BRACING DURING ERECTION, HE SHALL FURNISH, AND INSTALL THESE TEMPORARY SUPPORTS WHERE NECESSARY. TEMPORARY SUPPORTS ARE NOT PROVIDED BY THE METAL BUILDING MANUFACTURER.

DESIGN PARAMETERS

BUILDING DESCRIPTION:

NOMINAL WIDTH:	47.75	feet
NOMINAL LENGTH:	60	feet
EAVE HEIGHT, BACK S.W:	16	feet
EAVE HEIGHT, FRONT S.W:	16	feet
ROOF SLOPE, LEFT:	4.0:12	
ROOF SLOPE, RIGHT:	4.0:12	

SNOW :

FLAT ROOF SNOW LOAD Pf :	0	psf
GROUND SNOW LOAD Pg :	0	psf
SNOW LOAD IMP. FACTOR	N/A	
THERMAL FACTOR Ct :	N/A	
SNOW EXP. FACTOR Ce :	N/A	

DESIGN LOADS

BUILDING CODE:	FBC 04	
FRAME SELF WEIGHT:	INCLUDED	
ROOF DEAD LOAD:	2	psf
COLLATERAL LOAD:	0	psf
LIVE LOAD:	20	psf
SNOW LOAD, ROOF:	0	psf
WIND SPEED:	100	mph
	3 sec gust	
WIND EXPOSURE:	B	
CLOSURE "C, O, P" :	Closed	
IMP. FACTOR, WIND :	1.00	
BUILDING CATEGORY :	I	
SEISMIC PARAMETERS:		
SEISMIC USE GROUP:	I	
SEISMIC DESIGN CATEGORY:	A	
SPECTRUAL RESPONSE COEF. "SDS" :	.08	
(2 SEC PERIOD)		
SPECTRUAL RESPONSE COEF. "SD1" :	.04	
(1 SEC PERIOD)		
SITE CLASS (ASSUMED) :	D	

BASIC SEISMIC-FORCE RESISTING SYSTEM MOMENT RESISTING FRAME
DESIGN BASE SHEAR : .1 HORIZ.
ANALYSIS PROCEDURE : EQUIVALENT LATERAL FORCE PROCEDURE

ENGINEER'S STAMP

FLORIDA PRODUCT APPROVAL INFORMATION

	APPROVAL #
26 GA. PBR PANEL ~ Fern Green	FL3407.1
26 GA. R PANEL ~ Desert Sand	FL3408.1
26 GA. R PANEL ~ Fern Green	FL3408.1

GENERAL NOTES

1. MATERIALS

	ASTM DESIGNATION	
STRUCTURAL STEEL PLATE	A529 OR A572 OR A570 OR A607	GRADE 50
COLD FORMED LIGHT GAGE SHAPES	A570 OR A607	GRADE 55
BRACE RODS	A572	GRADE 65, UNLESS NOTED
HOT ROLLED MILL SHAPES	A36 OR A572	FY= 36 KSI OR GRADE 50
ROOF AND WALL SHEETS	A653 OR A792	GRADE 50
BOLTS	A307, A325, AND A490	A307 UNLESS NOTED
BUILT-UP MEMBERS	A570	GRADE 50 OR 55

2. STRUCTURAL PRIMER

SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER AND IT'S COLOR IS RED OXIDE. THIS PAINT IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS.

3. A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE.

STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE TURN-OF-THE-NUT METHOD IN ACCORDANCE WITH THE NINTH EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". PER SECTION 8D.1, A325 BOLTSMAY BE INSTALLED WITHOUT WASHERS WHEN TIGHTENED BY THE TURN-OF-THE NUT METHOD.

ALL HIGH STRENGTH BOLTS, EXCEPT AS NOTED OTHERWISE, ARE SUBJECT TO DIRECT TENSION AND MAY REQUIRE INSPECTION AS DEFINED BY THE APPLICABLE BUILDING CODE OR STANDARD. IT IS THE RESPONSIBILITY OF THE ERECTOR TO ASSURE PROPER TIGHTNESS.

4. BUILDER/CONTRACTOR RESPONSIBILITIES

THE METAL BUILDING MANUF.'S STANDARD PRODUCT SPECIFICATIONS APPLY AND UNLESS STIPULATED OTHERWISE IN THE CONTRACT DOCUMENTS, THE METAL BUILDING MANUF.'S DESIGN, FABRICATION, QUALITY CRITERIA STANDARDS AND TOLERANCES WILL GOVERN THE WORK.

IN CASE OF DISCREPANCIES BETWEEN METAL BUILDINGS MANUF. STRUCTURAL PLANS AND PLANS FOR OTHER TRADES. THE METAL BUILDING MANUF.'S PLANS SHALL GOVERN.

IT IS THE RESPONSIBILITY OF THE BUILDER / CONTRACTOR TO OBTAIN APPROPRIATE APPROVALS AND NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES, AS REQUIRED.

APPROVAL OF METAL BUILDING MANUF.'S DRAWINGS CONSTITUTES THE BUILDER / CONTRACTOR'S ACCEPTANCE OF THE METAL BUILDING MANUF.'S INTERPRETATION OF THE CONTRACT PURCHASE ORDER.

ONCE THE BUILDER / CONTRACTOR OR A/E FIRM HAS SIGNED MANUF.'S APPROVAL PACKAGE, CHANGES FROM THE PURCHASE ORDER BY THE BUILDER WILL BE BILLED TO THE BUILDER / CONTRACTOR FOR MATERIAL, ENGINEERING AND HANDLING FEES. SUCH CHANGES MAY CAUSE THE PROJECT TO BE MOVED FROM THE FABRICATION AND / OR SHIPPING SCHEDULE. A PENALTY FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND / OR SHIPPING SCHEDULE, AS LONG AS THE MANUF.'S DESIGN AND DETAILING APPROACH COMPLIES WITH THE PURCHASE ORDER.

THE BUILDER / CONTRACTOR OR A/E FIRM ARE RESPONSIBLE FOR THE OVERALL PROJECT CONDITION. ALL INTERFACE AND COMPATIBILITY CONCERNING ANY MATERIALS NOT FURNISHED BY THE MANUF. ARE TO BE CONSIDERED AND COORDINATED BY THE BUILDER / CONTRACTOR OR A/E FIRM. UNLESS SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS IS FURNISHED AS PART OF THE PURCHASE ORDER. THE METAL BUILDING MANNUF.'S ASSUMPTIONS WILL GOVERN.

THE BUILDER / CONTRACTOR IS RESPONSIBLE TO INSURE THAT ALL OTHER PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITY. SUPPLYING SEALED ENGINEERING DESIGN DATA AND DRAWINGS BY THE BUILDING MANUF. DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE BUILDING MANUF. OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR THE CONSTRUCTION PROJECT. THESE DRAWINGS AND DESIGN DATA ARE SEALED AS TO THE STRUCTURAL SYSTEM FURNISHED BY THE METAL BUILDING MANUF. IN COMPLIANCE WITH ALL REQUIREMENTS OF THE PURCHASE ORDER.

THE BUILDER / CONTRACTOR IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL BUILDING COMPONENTS IN ACCORDANCE WITH THE METAL BUILDING MANUF.'S "FOR CONSTRUCTION" DRAWINGS. TEMPORARY SUPPORTS OR BRACING REQUIRED FOR THE BUILDING ERECTION WILL BE THE RESPONSIBILITY OF THE ERECTOR TO DETERMINE, FURNISH, AND INSTALL.

THE METAL BUILDING MANUF. DOES NOT WARRANT STRUCTURAL INTEGRITY OF ANY COMPONENTS FIELD MODIFIED OR DESIGNED AND FABRICATED BY OTHERS. NEITHER DO WE ACCEPT DESIGN RESPONSIBILITY FOR THE EFFECTS NON STANDARD COMPONENTS DESIGNED BY OTHERS MAY HAVE ON THE SYSTEM IN GENERAL.

SHEETING

6 GA.	PBR PANEL ~ Fern Green	ROOF
6 GA.	R PANEL ~ Desert Sand	WALL
6 GA.	R PANEL ~ Fern Green	DOOR

TRIM

6 GA.	Fern Green	GUTTER
6 GA.	Desert Sand	DOWNS
6 GA.	Fern Green	RAKE
6 GA.	N/A	EAVE
6 GA.	Desert Sand	CORNER
6 GA.	Desert Sand	ACCESS.

SKYLIGHTS

NO.	DESCRIPTION	COLOR
N/A	3' X 10'-8 W/TRIM	WHITE

WALL-LIGHTS

NO.	DESCRIPTION	COLOR
N/A	N/A	WHITE

BASE CONDITION

ANGLE ☒ NOTCH ☒ SPC TRIM ☐
TRIM ☐ FL-CEE ☐ NONE ☐

FRAMED OPENINGS

NO.	SIZE	DESCRIPTION
2	3' X 7'	FO.
1	42' X 14'	FO.
1	9' X 9'	FO.
1	3' X 3'	FO.
6	3' X 5'	FO.

DOORS/WINDOWS

NO.	DESCRIPTION
2	3070 WALK DOOR
N/A	ROLL-UP DOOR

VENTILATORS

NO.	DESCRIPTION
2	9" X 10' VENTS W/BS & DMP.

BLANKET TYPE INSULATION

NONE ☒ BY MFG ☐ BY OTHERS ☐

ROOF	WALL
PAINT: RED OXIDE	SCREWS:
	ROOF: #12x1.5LLSD
	WALLS:#12x1.5 STD

LINER PANELS

WALL	THICK	TYPE	LIN FT.	HT.	COLOR
L.E.W.	26 GA	"R"	0'	0'	PW
F.S.W.	26 GA	"R"	0'	0'	PW
R.E.W.	26 GA	"R"	0'	0'	PW
B.S.W.	26 GA	"R"	0'	0'	PW

MISC. ACCESSORIES

NO.	DESCRIPTION
N/A	N/A

DRAWING RELEASE HISTORY

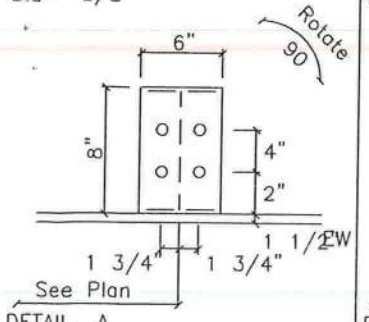
DATE SENT	REASON FOR RELEASE
	A
	B
	C
	D
	E
	F
	G
	H
	I
	J
	K
	L
	M

DRAWING REVISIONS

REVISION	DATE	BY	DESCRIPTION

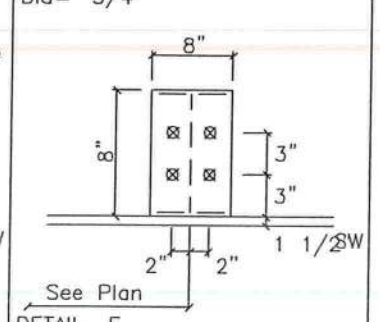
10/7/05

Dia= 5/8"



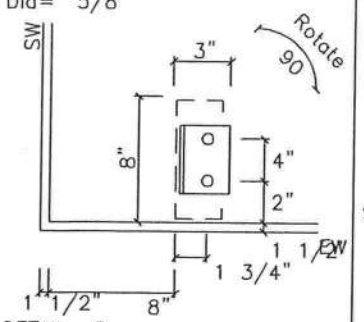
DETAIL A

Dia= 3/4"



DETAIL E

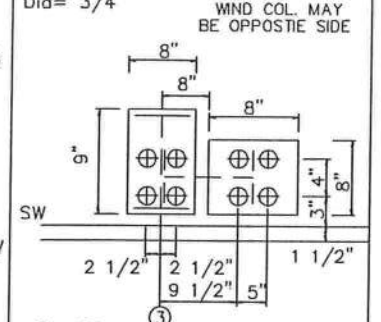
Dia= 5/8"



DETAIL B

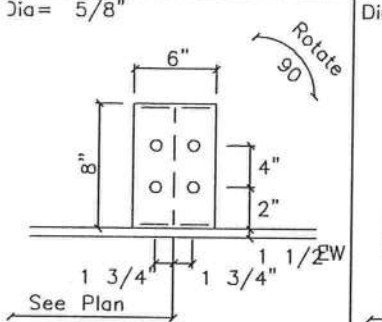
Dia= 3/4"

WIND COL. MAY BE OPPOSITE SIDE



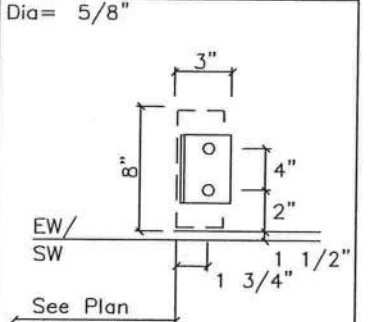
DETAIL F

Dia= 5/8"



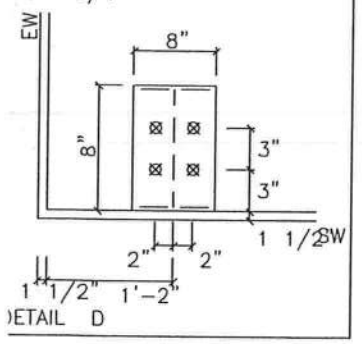
DETAIL C

Dia= 5/8"



DETAIL G

Dia= 3/4"



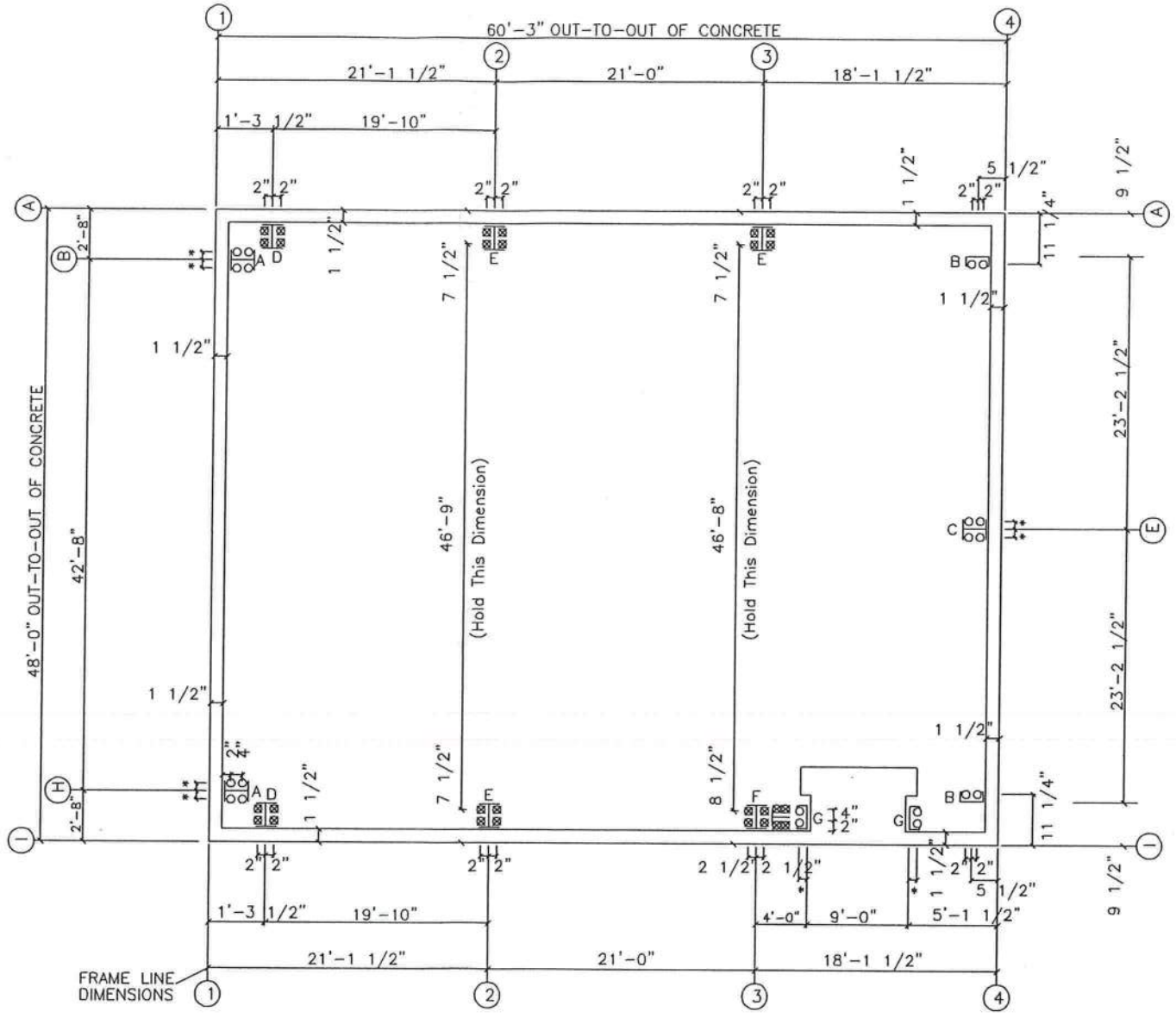
DETAIL D

FIELD LOCATE:
(2) 3' X 7' F.O.
(1) 3' X 3' F.O.
(6) 3' X 5' F.O.

ANCHOR BOLT NOTES:
VALUES GIVEN FOR BENDS AND ANCHOR BOLT TOTAL LENGTHS ARE SUGGESTED LENGTHS ONLY. IT IS THE RESPONSIBILITY OF THE FOUNDATION ENGINEER TO DETERMINE THESE VALUES SINCE THEY ARE A FUNCTION OF CONCRETE STRENGTH AS WELL AS OTHER FACTORS.

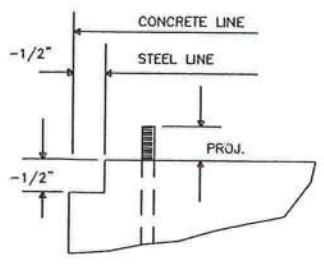
ANCHOR BOLT SUMMARY

Qnt	Loc	Dia (in)	Total Len (in)	Bend Len (in)	Proj (in)
O 4	DJ	5/8"	9.00	3.00	3.00
O 16	EW	5/8"	12.00	3.00	3.00
⊠ 24	RF	3/4"	18.00	3.00	3.00
⊠ 4	WF	3/4"	18.00	3.00	3.00

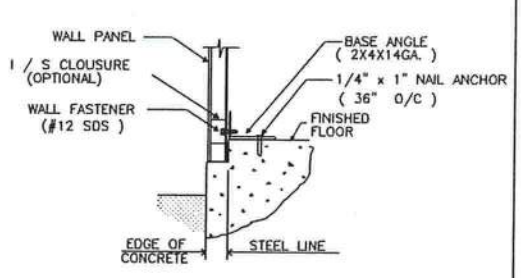


ANCHOR BOLT PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)

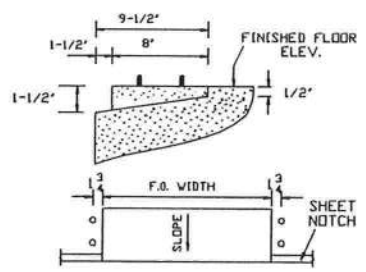
*= 1 3/4"



CONCRETE NOTCH AND ANCHOR BOLT PROJECTION



BASE ANGLE WITH SHEET NOTCH (NO TRIM)



TYP. O.H. DOOR RECESS DETAIL

Base_a_s

REVISIONS						
REV.	DESCRIPTION:	DATE	DTLR	DATE	CHKR	APPD

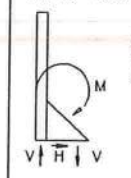
DRAWING STATUS
[X] FOR CONSTRUCTION
[ ] FOR PERMIT ONLY
[ ] FOR APPROVAL
[ ] OTHER, EXPLAIN

AMERICAN STEEL BLDGS		HAROLD FRANKEL	
PROJECT	47.7 x 60.0 x 16.0	ANCHOR BOLT PLAN	
ID	15070R1	DESIGN:	DRAFT: SPW CHECK:
PROJECT ADDRESS	LAKE CITY, FL	DATE: 9/30/05	SHEET 1

SEAL

10/7/05

WIND COLUMN REACTIONS



Loc	Wall Line	Col Line	R/L	Load_Id	± Reactions Horz (k)	± Reactions Vert (k)	Moment (f-k)	Anc_Bolt No	Anc_Bolt Dia	Base_Plate(in) Width	Base_Plate(in) Length	Base_Plate(in) Thick
F_SW	I	3	R	Wind	2.74	41.14	41.14	4	0.750	8.00	8.00	0.75
				Seismic	0.11	1.59	1.59					

GENERAL NOTES

(1.) APPLICATION OF ENGINEERS SEAL IS FOR METAL BUILDING ONLY AND DOES NOT REPRESENT THE PROFESSIONAL OF RECORD.

(2.) FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF THE METAL BUILDING MANUFACTURER.

(3.) ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.

(4.) THE BUILDING REACTION DATA REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION. THE FOUNDATION IS TO BE DESIGNED BY A QUALIFIED ENGINEER TO SUPPORT THE BUILDING REACTIONS IN ADDITION TO OTHER LOADS IMPOSED BY THE BUILDING USE OR OCCUPANCY WITH RESPECT TO JOB SITE CONDITIONS.

(5.) ALL ANCHOR BOLTS TO BE ASTM SPECIFICATION A307 UNLESS OTHERWISE NOTED.

(6.) VALUES GIVEN FOR BENDS AND ANCHOR BOLT TOTAL LENGTHS ARE SUGESTED LENGTHS ONLY. IT IS THE RESPONSIBILITY OF THE FOUNDATION ENGINEER TO DETERMINE THESE VALUES SINCE THEY ARE A FUNCTION OF CONCRETE STRENGTH AS WELL AS OTHER FACTORS.

ENDWALL COLUMN: REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Dead Vert	Coll Vert	Live Vert	Wind-Left Horiz	Wind-Left Vert	Wind-Right Horiz	Wind-Right Vert	Out-Of-Plane Wd P Horiz	Out-Of-Plane Wd S Horiz	Anc. Bolt NoD(in)	Base Plate (in) Wid	Base Plate (in) Len	Base Plate (in) Thk	Grout (in)
1	H	0.0	1.0	0.0	0.0	0.0	0.0	0.0	-0.1	0.1	4 0.625	6.000	8.000	0.500	0.0
1	B	0.0	1.0	0.0	0.0	0.0	0.0	0.0	-0.1	0.1	4 0.625	6.000	8.000	0.500	0.0
4	I	0.3	0.0	1.4	0.0	-1.8	0.0	-1.3	0.0	0.0	2 0.625	3.000	8.000	0.188	0.0
4	E	1.0	0.0	3.7	0.0	-3.3	0.0	-3.3	-3.8	4.1	4 0.625	6.000	8.000	0.500	0.0
4	A	0.3	0.0	1.4	0.0	-1.3	0.0	-1.8	0.0	0.0	2 0.625	3.000	8.000	0.188	0.0

BRACING REACTIONS, PANEL SHEAR

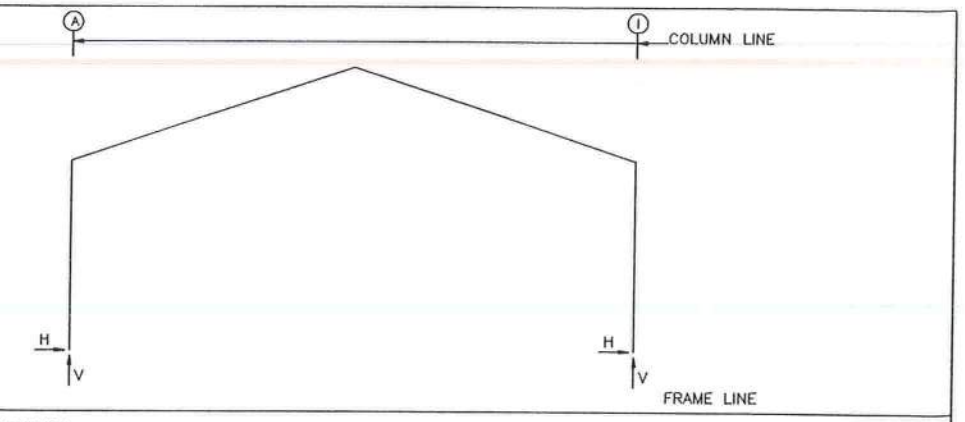
Loc	Wall Line	Col Line	± Reactions (k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel Shear (lb/ft)
L_EW	1	Rigid Frame At Endwall					
F_SW	I	Wind Column In Wall					
R_EW	4						27
B_SW	A	3 ,2	2.7	1.7	0.1	0.1	

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft)	= 47.8
Length (ft)	= 60.0
Eave Height (ft)	= 16.0/16.0
Roof Slope (rise/12)	= 4.0/ 4.0
Dead Load (psf)	= 2.0
Collateral Load (psf)	= 0.0
Roof Live Load (psf)	= 20.0
Frame Live Load (psf)	= 12.0
Wind Speed (mph)	= 100.0(3 SEC GUST)
Wind Code	= FBC 04
Exposure	= B
Closed/Open	= C
Importance - Wind	= 1.00
Importance - Seismic	= 1.00
Seismic Coeff (Fa*Ss)	= 0.08

Id	Description
1	DL+CL+LL
2	DL+CL+0.75LL+0.75WL1
3	DL+CL+0.75LL+0.75WR1
4	0.60DL+WL1
5	0.60DL+WR1
6	0.60DL+WL2
7	0.60DL+WR2
8	DL+CL+0.75LL+0.75WR2+0.75WS
9	0.60DL+WP
10	DL+CL+0.75LL+0.75WL2+0.75WS
11	0.60DL+WR1+WS
12	0.60DL+WL1+WS



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt NoD(in)	Base Plate (in) Wid	Base Plate (in) Len	Base Plate (in) Thk	Grout (in)
1	A	4 0.750	8.000	8.001	0.500	0.0
1	I	4 0.750	8.000	8.001	0.500	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt NoD(in)	Base Plate (in) Wid	Base Plate (in) Len	Base Plate (in) Thk	Grout (in)
2	A	4 0.750	8.000	8.001	0.500	0.0
2	I	4 0.750	8.000	8.001	0.500	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt NoD(in)	Base Plate (in) Wid	Base Plate (in) Len	Base Plate (in) Thk	Grout (in)
3	A	4 0.750	8.000	8.001	0.500	0.0
3	I	4 0.750	8.000	9.000	0.750	0.0

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Wind_L1 Horiz	Wind_L1 Vert	Wind_R1 Horiz	Wind_R1 Vert	Wind_L2 Horiz	Wind_L2 Vert
1	A	0.71	1.78	0.00	0.00	3.84	8.57	-7.35	-11.69	0.03	-7.92	-7.22	-8.42
1	I	-0.71	1.78	0.00	0.00	-3.84	8.57	-0.03	-7.92	7.35	-11.69	-0.16	-4.64
2	A	0.61	1.63	0.00	0.00	2.80	6.44	-4.48	-7.62	0.56	-5.31	-4.59	-4.87
2	I	-0.61	1.63	0.00	0.00	-2.80	6.44	-0.56	-5.31	4.48	-7.62	-0.50	-2.56
3	A	0.57	1.53	0.00	0.00	2.58	5.97	-4.83	-7.42	0.95	-4.57	-4.94	-4.87
3	I	-0.57	1.53	0.00	0.00	-2.58	5.98	-0.96	-4.58	4.83	-7.43	-0.90	-2.02

Frame Line	Column Line	Wind_R2 Horiz	Wind_R2 Vert	Seismic_L Horiz	Seismic_L Vert	Seismic_R Horiz	Seismic_R Vert	LnWind_L Horiz	LnWind_L Vert	LnWind_R Horiz	LnWind_R Vert	LnSeis Horiz	LnSeis Vert
1	A	0.16	-4.64	-0.04	-0.03	0.04	0.03	-0.68	-9.46	-2.31	-8.36		
1	I	7.22	-8.42	-0.04	0.03	0.04	-0.03	2.31	-8.36	0.68	-9.46		
2	A	0.50	-2.56	-0.03	-0.02	0.03	0.02	-0.06	-7.44	-0.87	-6.89		
2	I	4.59	-4.87	-0.03	0.02	0.03	-0.02	0.87	-5.15	0.06	-5.70		
3	A	0.90	-2.01	-0.02	-0.01	0.02	0.01	-0.04	-7.02	-0.80	-6.51		
3	I	4.94	-4.88	-0.02	0.01	0.02	-0.01	0.80	-4.78	0.04	-5.29		

Protection of Opening

This building is located in a wind-borne debris region. Exterior glazing is assumed to be impact resistant and meet the provisions of the missile test, or they should be protected by impact resistant covering meeting the reqirements of SSTD 12, ASTM E 1886 and ASTME 1996 or Miami-Dade PA 201, 202, &203.
Openings may also be protected by structural wood panels having a min. thickness of 7/16" and maximum panel span of 8 feet. Attachment hardware and fastening schedule shall be in accordance with the following table.

WIND-BORNE DEBRIS PROTECTION FASTENING SCHEDULE
FOR WOOD STRUCTURAL PANELS (PLYWOOD)

FASTENER TYPE	FASTENER SPACING (in.) [1,2] PANEL SPAN ≤ 2ft	2ft< PANEL SPAN <4 ft.	4ft< PANEL SPAN <6 ft.	6ft< PANEL SPAN <8 ft.
2 1/2 #6 Wood Screw [3,5]	16	16	12	9
2 1/2 #8 Wood Screw [3]	16	16	16	12
Double-Headed nails [4]	12	6	4	3

SI: 1 inch=25.4 mm 1 foot=305 mm

Notes:

1.) This table is based on a maximum wind speed of 130 m.p.h. (58 m/s) and mean roof height of 33 feet (10 m) or less.
2.) Fasteners shall be installed at opposing ends of the wood structural panel.
3.) Where screws are attached to masonry or masonry/stucco, they shall be attached using vibration-resistant anchors having a minimum withdrawal capacity of 490 lb. (2180 kN).
4.) Nails shall be 10d common or 12d box double-headed nails.
5.) Where screws are attached to pre-engineered metal building components, i.e. Door Jambs, framed openings, etc., they shall be #12 self drilling screws secured to a minimum 16 ga. material. Screws should have a min. withdrawal strength of 500 lbs.

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10/7/05

REVISIONS						
REV.	DESCRIPTION:	DATE	DTLR	DATE	CHKR	APPD

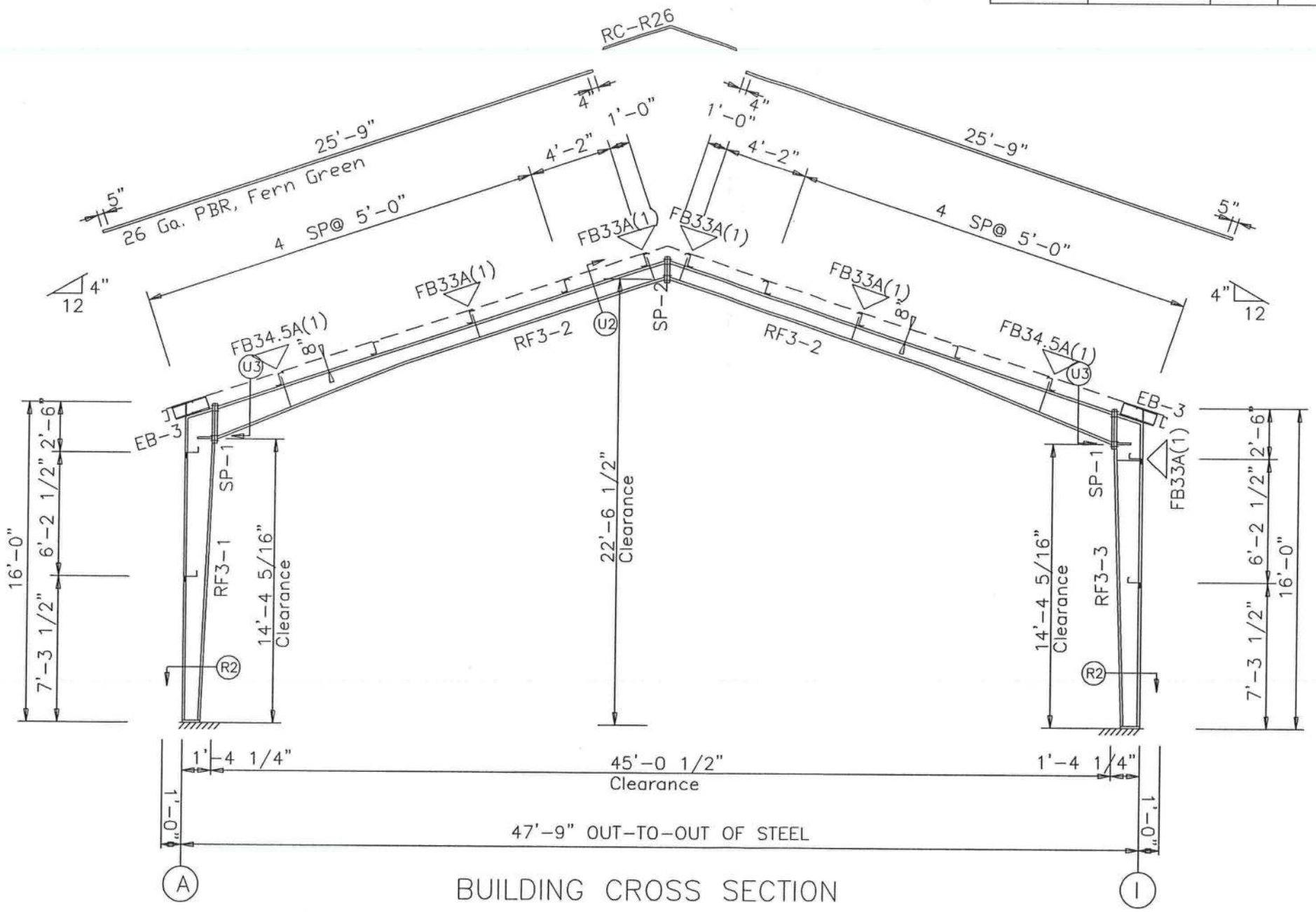
DRAWING STATUS
[X] FOR CONSTRUCTION
[ ] FOR PERMIT ONLY
[ ] FOR APPROVAL
[ ] OTHER, EXPLAIN

AMERICAN STEEL BLDGS		HAROLD FRANKEL	
PROJECT	47.7 x 60.0 x 16.0	ANCHOR BOLT REACTIONS	
ID	15070R1	DESIGN:	DRAFT: SPW CHECK:
PROJECT ADDRESS	LAKE CITY, FL	DATE: 9/30/05	SHEET 2

SPLICE BOLTS						
Splice Mark	Quan Top/Bot/Int			Bolt Type	Dia	Len
SP- 1	4	4	0	A325	0.625	1.75
SP- 2	4	4	0	A325	0.625	1.75

▽FLANGE BRACES:
FBxxA(1): xx=length(in)
A - L2X2X14G

MEMBER SIZE TABLE (in)					
PIECE	WEB DEPTH START/END	WEB PLATE THICK	WEB PLATE LENGTH	OUTSIDE FLANGE W x T x LEN	INSIDE FLANGE W x T x LEN
RF3-1	7.6/15.8	0.120	188.2	5x3/16"x182.9 5x3/16"x 16.8	5x1/4" x167.5
RF3-2	15.6/ 7.6	0.120	121.6	5x3/16"x240.0	5x1/4" x121.8
	7.6/ 7.6	0.120	167.5	5x3/16"x 43.9	5x3/16"x164.9
RF3-3	15.8/ 8.6	0.120	188.0	5x3/16"x 16.8 5x3/16"x182.6	5x1/4" x167.2



BUILDING CROSS SECTION

FOR FRAME LINE 3

GENERAL NOTES:

* NOTICE TO ERECTOR *

(A)It is IMPORTANT that for members exceeding 30 ft. in length that a spreader bar be used when lifting.

(B)ALL flange braces and wind bracing must be installed prior to exterior finishes being applied.

REVISIONS						
REV.	DESCRIPTION:	DATE	DTLR	DATE	CHKR	APPD

DRAWING STATUS
[ ] FOR CONSTRUCTION
[X] FOR PERMIT ONLY
[ ] FOR APPROVAL
[ ] OTHER, EXPLAIN

AMERICAN STEEL BLDGS			
PROJECT	47.7 x 60.0 x 16.0		
ID	15070R1		
PROJECT ADDRESS	LAKE CITY, FL		

SEAL

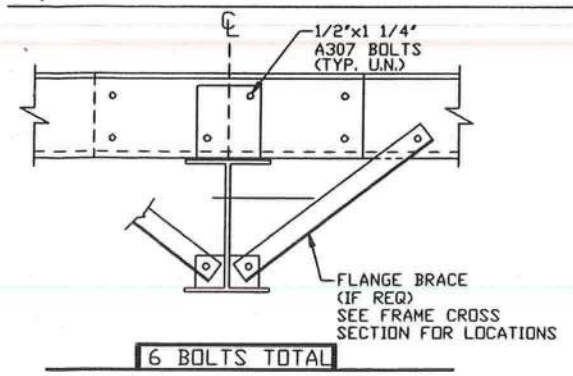
10/7/05

HAROLD FRANKEL

RIGID FRAME ELEVATION

DESIGN: | DRAFT: SPW | CHECK:

DATE: 9/30/05 | SHEET 5

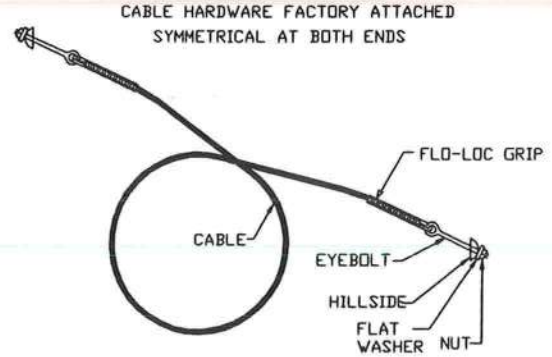


6 BOLTS TOTAL

ADDITIONAL HOLES ARE PRESENT FOR ALIGNMENT PURPOSES ONLY

BRACE CABLE ASSEMBLY

CABLE HARDWARE FACTORY ATTACHED SYMMETRICAL AT BOTH ENDS



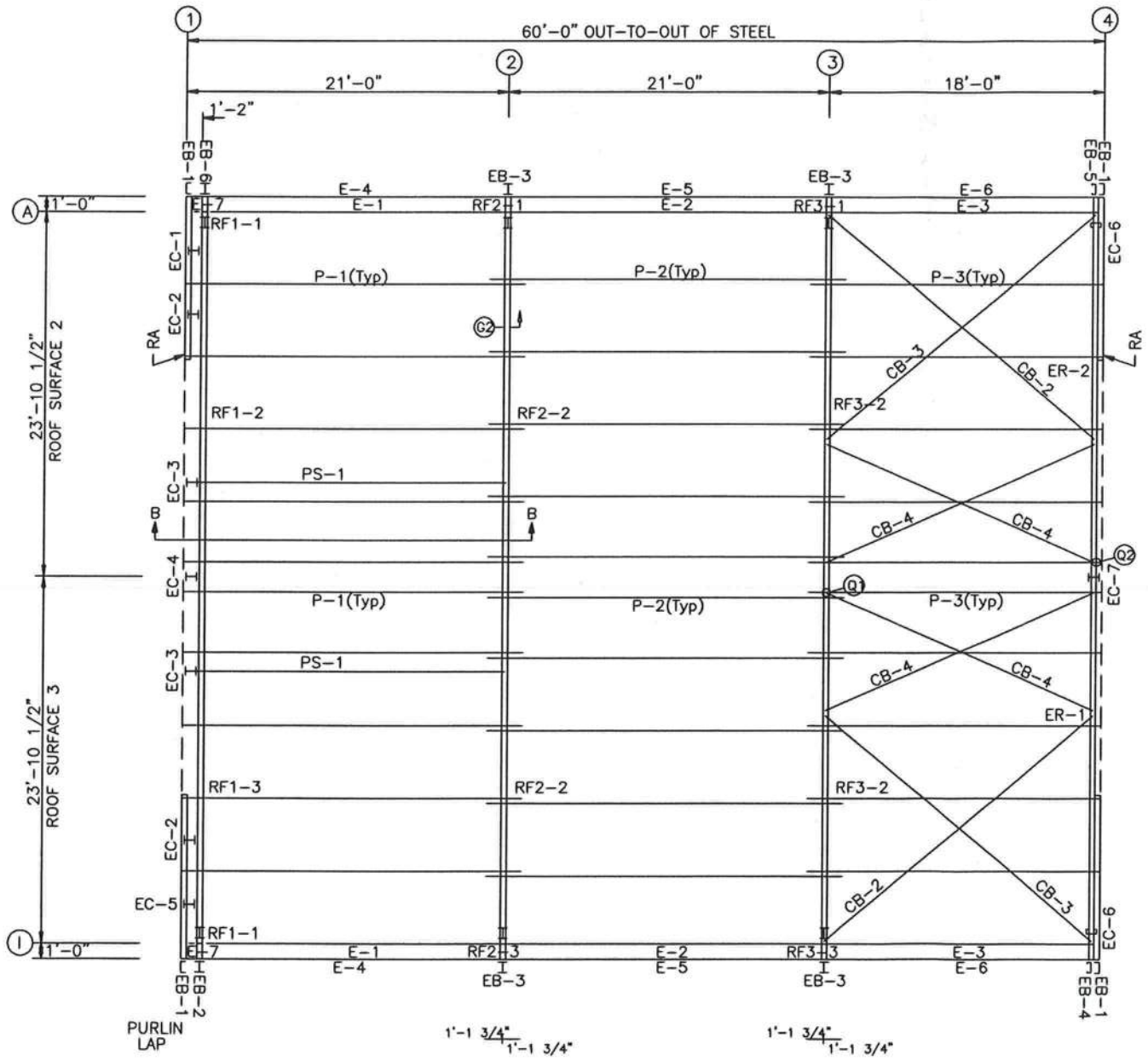
SEE SHEET 14 FOR RIDGEVENT DETAIL

EXTENSION/CANOPY BOLTS ROOF PLAN				
MARK	QUAN	TYPE	DIA	LENGTH
EB-2	4	A325	5/8"	1 3/4"
EB-3	4	A325	5/8"	1 3/4"
EB-4	2	A325	1/2"	1 1/4"
EB-5	2	A325	1/2"	1 1/4"
EB-6	4	A325	5/8"	1 3/4"

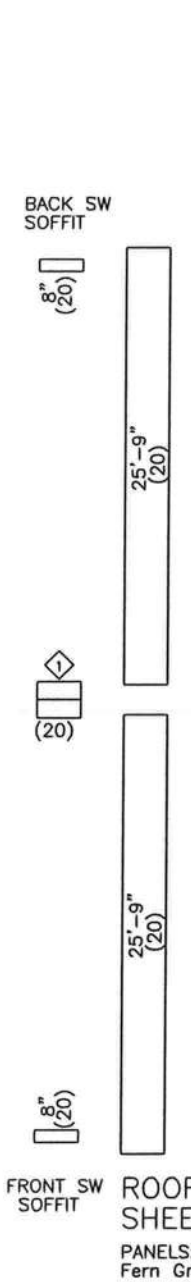
TRIM TABLE ROOF PLAN			
◇ID	MARK	LENGTH	DETAIL
1	RC-R26	3'-0"	TRIM_19

MEMBER TABLE ROOF PLAN		
MARK	PART	LENGTH
EB-1	8X4CH16	11'-5 1/2"
EB-2	W8X10	2'-4 5/8"
EB-3	W8X10	2'-4 5/8"
EB-4	8x35C16	2'-4 5/8"
EB-5	8x35C16	2'-4 5/8"
EB-6	W8X10	2'-4 5/8"
P-1	8X25Z16	22'-1 1/2"
P-2	8X25Z16	23'-3 1/2"
P-3	8X25Z16	19'-1 1/2"
E-1	8E16	19'-2 1/2"
E-2	8E16	20'-4 1/2"
E-3	8E16	17'-0 1/2"
E-4	8X25C16	20'-8"
E-5	8X25C16	20'-4 1/2"
E-6	8X25C16	17'-8"
E-7	8E16	10"
CB-2	0.250CBL	23'-9"
CB-3	0.250CBL	23'-0"
CB-4	0.250CBL	19'-11"

SEE SHEET 6A FOR SECTION "B-B"

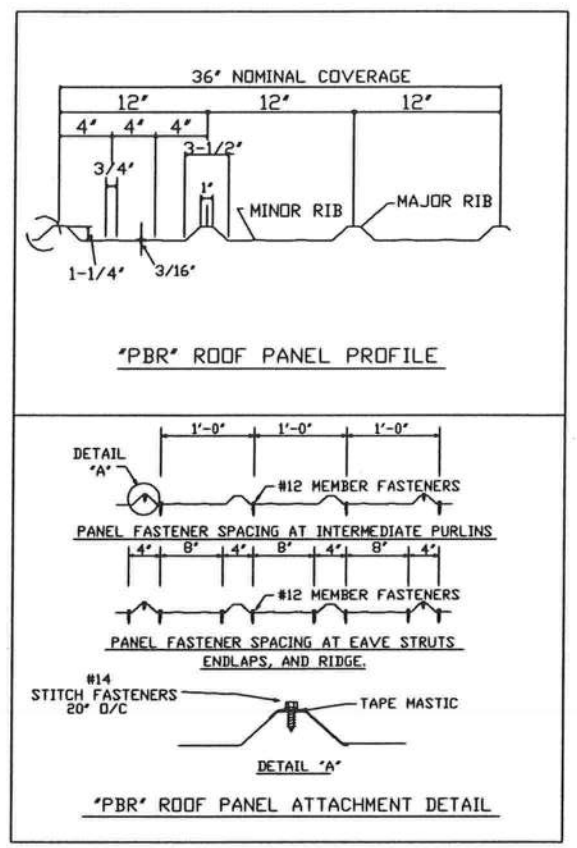


ROOF FRAMING PLAN



ROOF SHEETING

PANELS: 26 Ga. PBR
Fern Green



SEAL

10/7/05

REVISIONS						
REV.	DESCRIPTION:	DATE	DTLR	DATE	CHKR	APPD

DRAWING STATUS
[ ] FOR CONSTRUCTION
[X] FOR PERMIT ONLY
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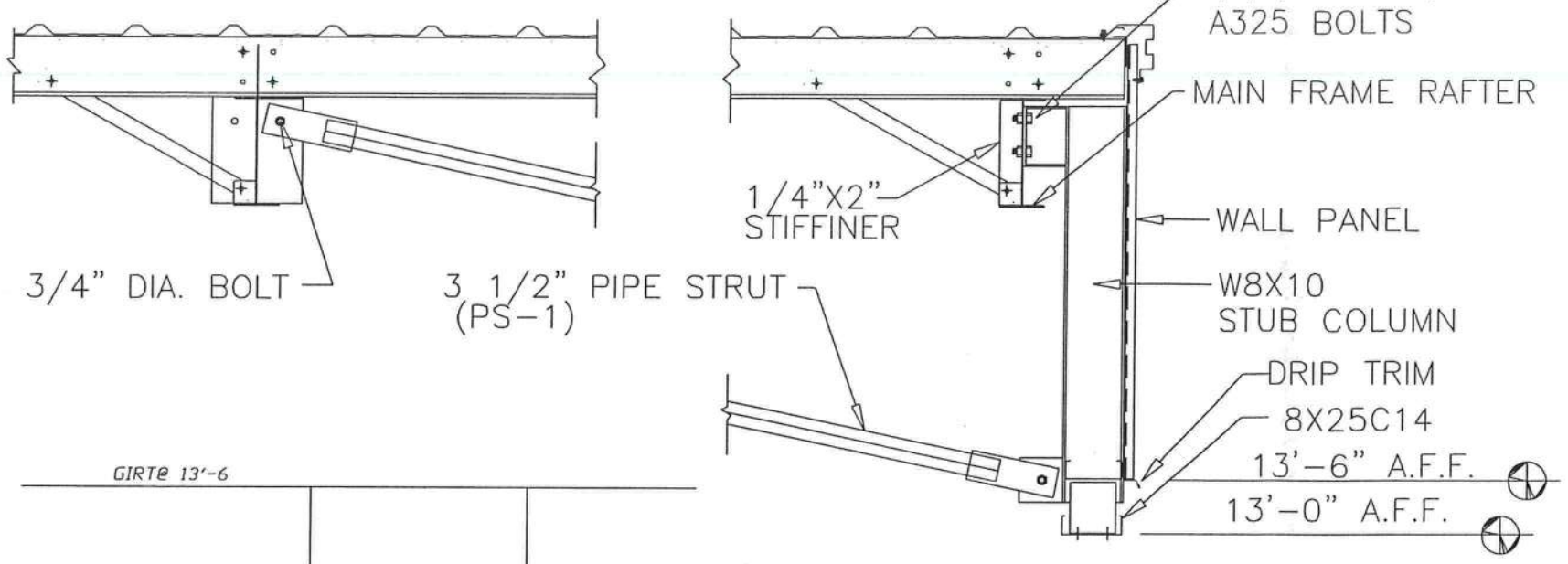
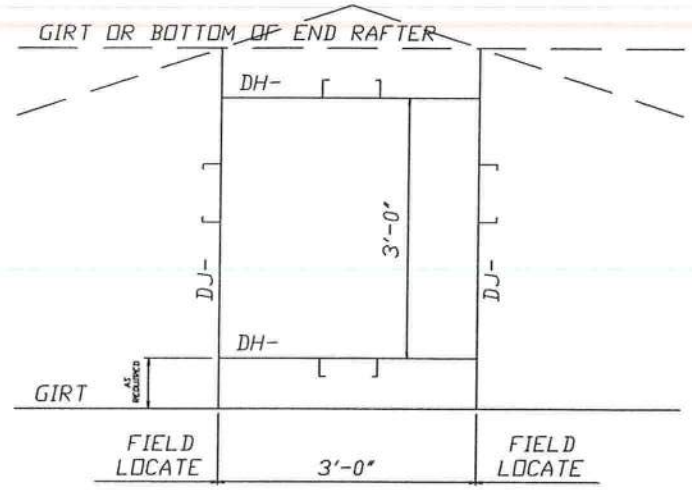
AMERICAN STEEL BLDGS		HAROLD FRANKEL	
PROJECT	47.7 x 60.0 x 16.0	ROOF FRAMING	
ID	15070R1	DESIGN:	DRAFT: SPW CHECK:
PROJECT ADDRESS	LAKE CITY, FL	DATE: 9/30/05	SHEET 6



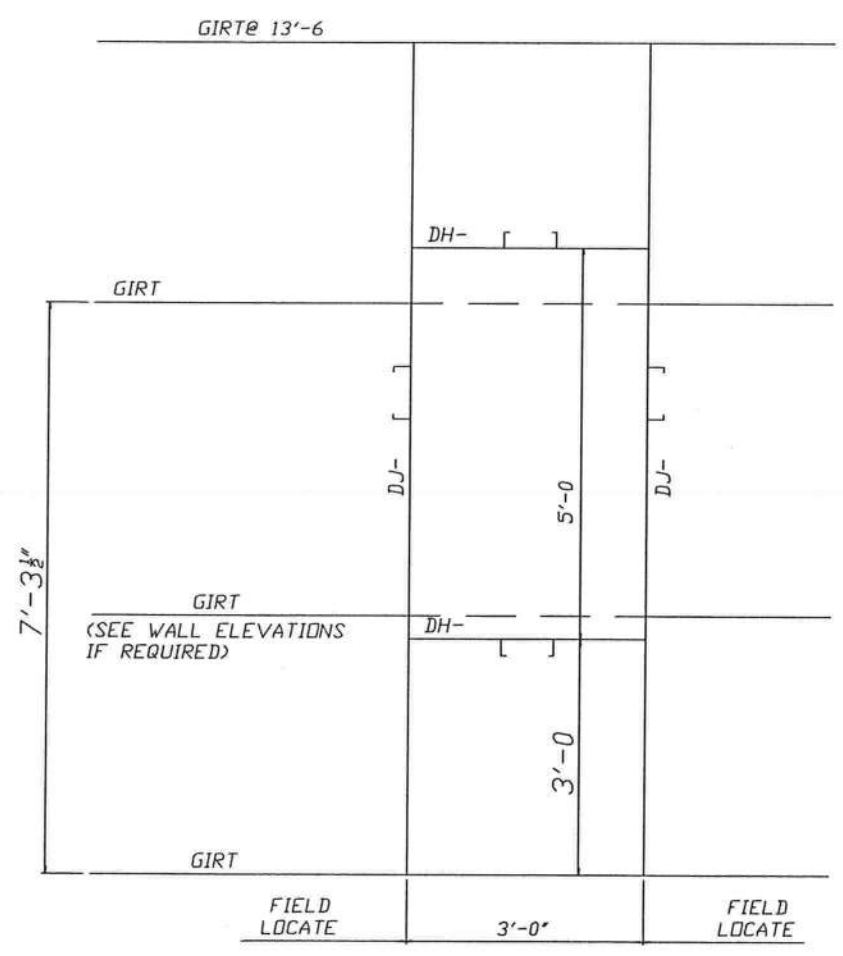
SECTION B-B



1. SEE FRAMING ELEVATIONS FOR GIRT QUANTITY AND ELEVATIONS
2. SEE DETAILS FOR BOLT AND FRAMING REQUIREMENTS.
3. FIELD CUT AND WORK GIRTS, PANELS, AND TRIM AS REQUIRED.

HEAD TRIM: HTX3
JAMB TRIM: JTRX3
SILL TRIM: HTX3

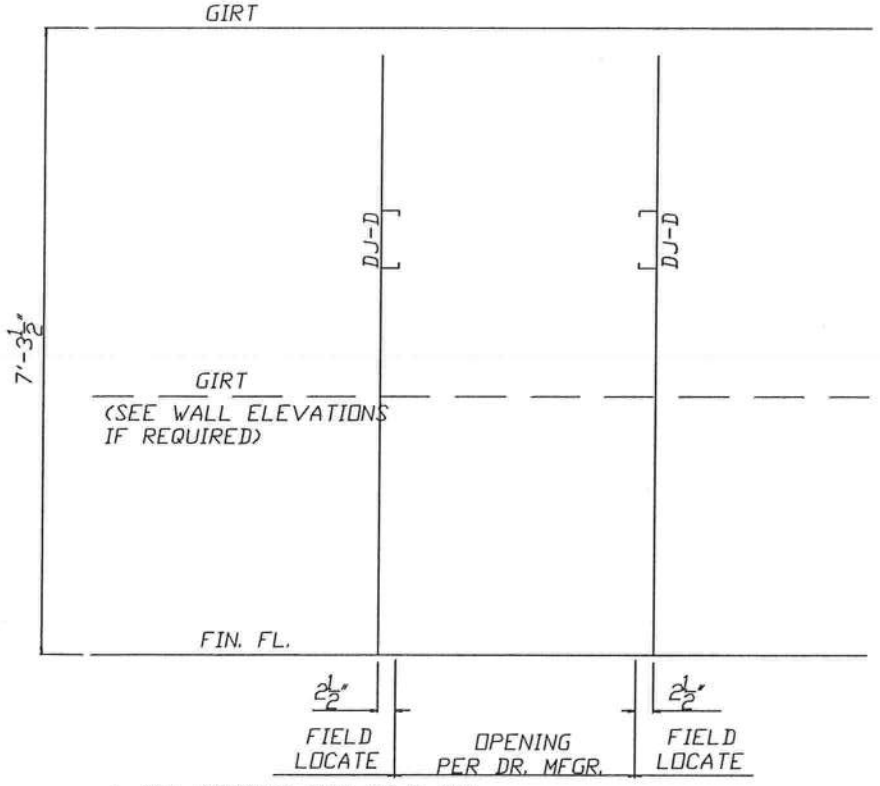
(1) FIELD LOCATED 3' x 3' FRAMED OPENING



1. SEE FRAMING ELEVATIONS FOR GIRT QUANTITY AND ELEVATIONS
2. SEE DETAILS FOR BOLT AND FRAMING REQUIREMENTS.
3. FIELD CUT AND WORK GIRTS, PANELS, AND TRIM AS REQUIRED.

HEAD TRIM: HTX3
JAMB TRIM: JTRX5
SILL TRIM: HTX3

(6) FIELD LOCATED 3' x 5' FRAMED OPENING



1. SEE DETAILS FOR BOLT AND FRAMING REQUIREMENTS.
2. FIELD CUT AND WORK GIRTS, PANELS, AND TRIM AS REQUIRED.

HEAD TRIM: HTX3
JAMB TRIM: JTRX7

(2) FIELD LOCATED 3070 SUB-JAMBS

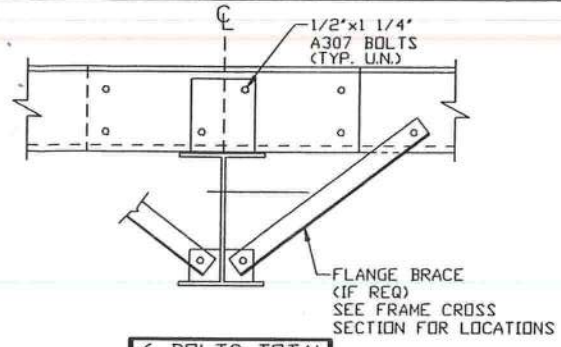
REVISIONS						
REV.	DESCRIPTION:	DATE	DTLR	DATE	CHKR	APPD

DRAWING STATUS
[ ] FOR CONSTRUCTION
[X] FOR PERMIT ONLY
[ ] FOR APPROVAL
[ ] OTHER, EXPLAIN

AMERICAN STEEL BLDGS		HAROLD FRANKEL	
PROJECT	47.7 x 60.0 x 16.0	PIPE STRUT & F.O. DETAILS	
ID	15070R1	DESIGN:	DRAFT: SPW CHECK:
PROJECT ADDRESS	LAKE CITY, FL	DATE: 9/13/05	SHEET 6A

SEAL

10/7/05



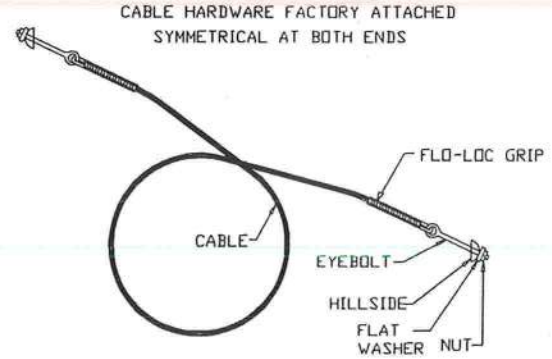
6 BOLTS TOTAL

ADDITIONAL HOLES ARE PRESENT FOR ALIGNMENT PURPOSES ONLY

BRACE CABLE ASSEMBLY

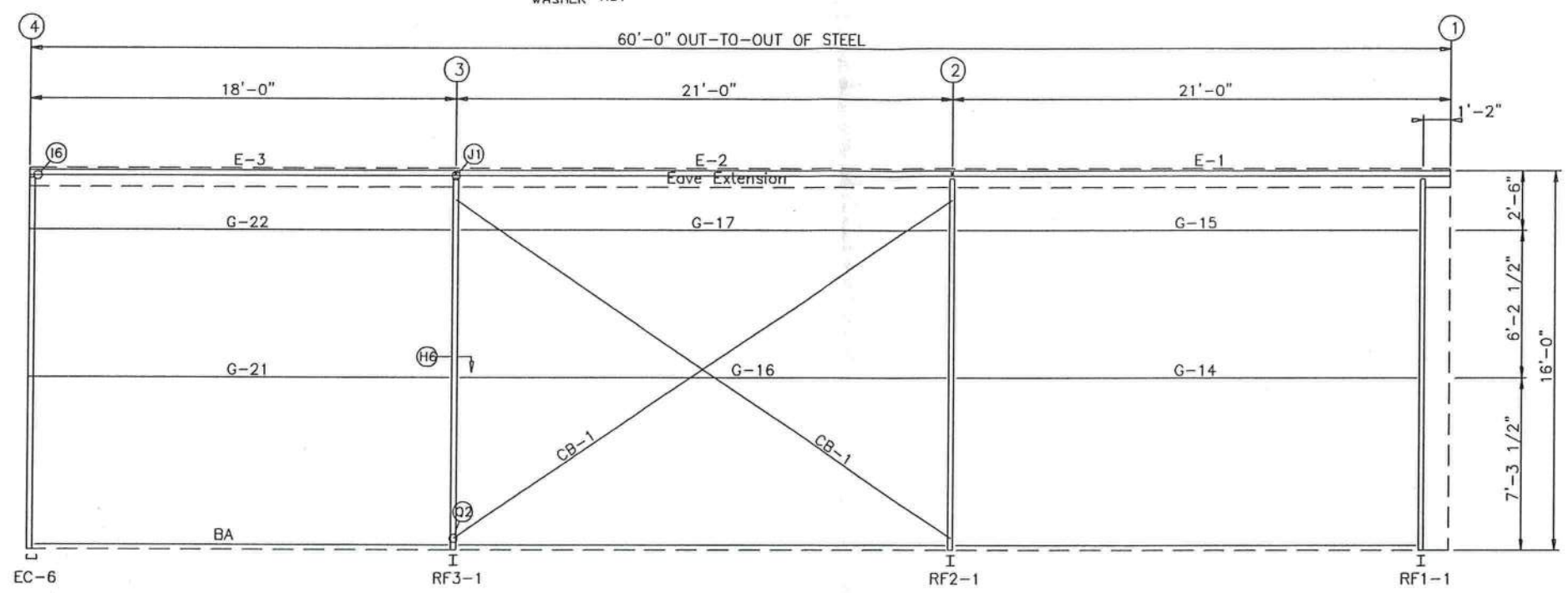
CABLE HARDWARE FACTORY ATTACHED

SYMMETRICAL AT BOTH ENDS

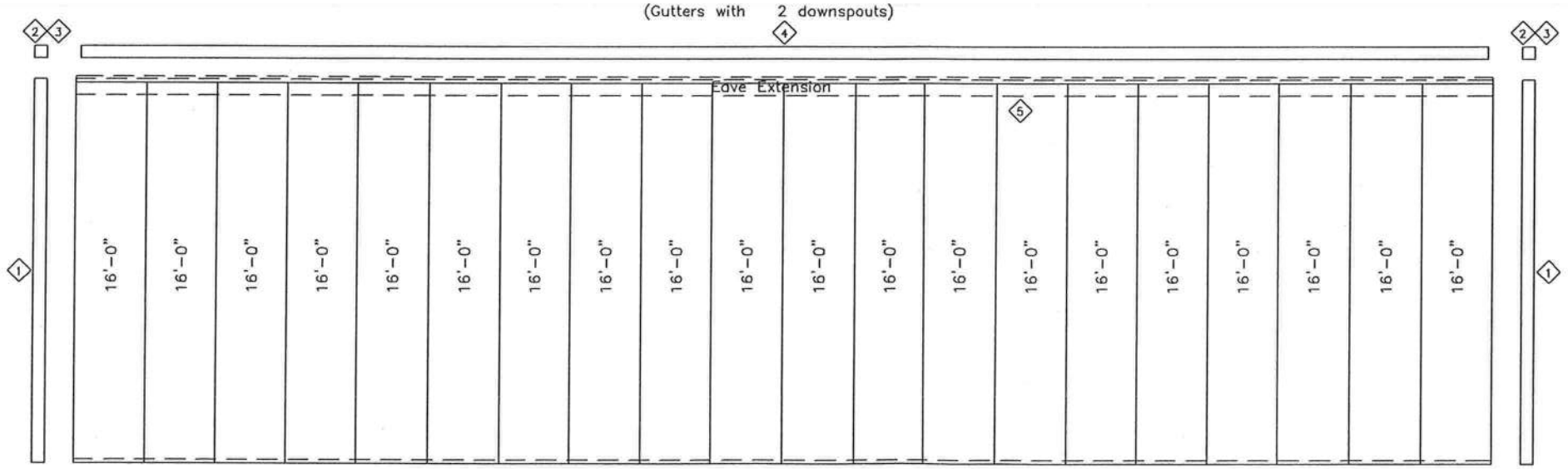


TRIM TABLE FRAME LINE A			
◇ID	MARK	LENGTH	DETAIL
1	CTR	16'-2"	TM29
2	RVCB	6"	TM38
3	VGE	6"	
4	RVEG	20'-3"	TM50
5	RSRT	20'-3"	

MEMBER TABLE FRAME LINE A		
MARK	PART	LENGTH
E-1	8E16	19'-2 1/2"
E-2	8E16	20'-4 1/2"
E-3	8E16	17'-0 1/2"
G-14	8X25Z13	19'-1 1/4"
G-15	8X25Z16	19'-1 1/4"
G-16	8X25Z13	20'-3 1/4"
G-17	8X25Z16	20'-3 1/4"
G-21	8X25Z14	17'-7 3/8"
G-22	8X25Z16	17'-7 3/8"
CB-1	0.250CBL	25'-6"

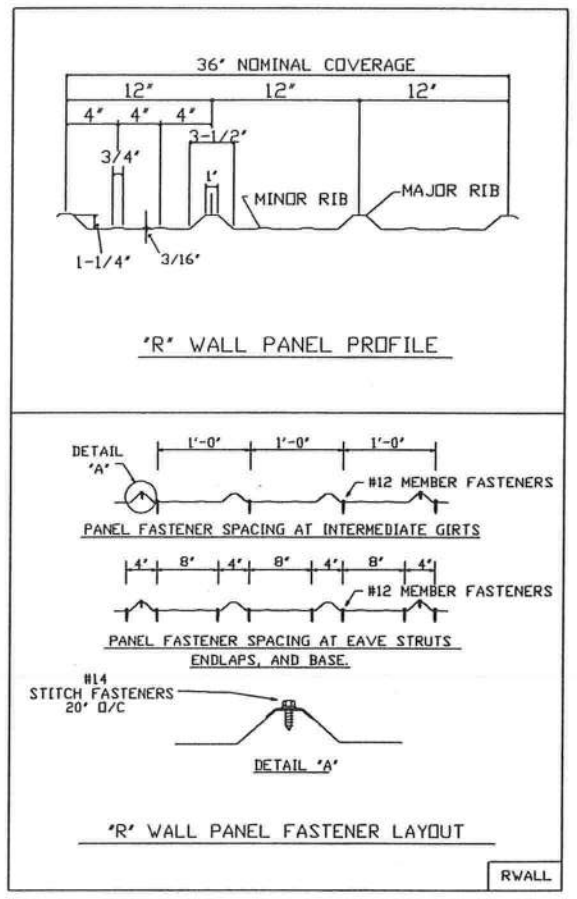


SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Ga. R – Desert Sand



SEAL

10/7/05

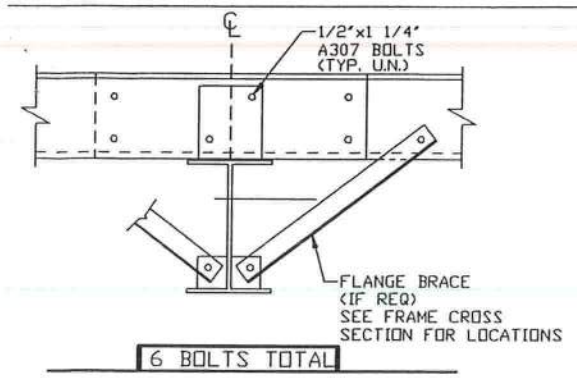
GENERAL NOTES:

(1.) IF CABLE BRACING, WIND BENTS, WIND COLUMNS, OR WEAK AXIS DESIGN OF SIDE WALL COLUMNS WERE NOT PROVIDED IT HAS BEEN DETERMINED THAT DIAPHRAGM PANEL ACTION IS SUFFICIENT TO RESIST LONGITUDINAL FORCES. TEMPORARY BRACING SHOULD BE PROVIDED BY ERECTOR UNTIL ALL WALL AND ROOF PANELS ARE INSTALLED.

REVISIONS						
REV.	DESCRIPTION:	DATE	DTLR	DATE	CHKR	APPD

DRAWING STATUS
[ ] FOR CONSTRUCTION
[X] FOR PERMIT ONLY
[ ] FOR APPROVAL
[ ] OTHER, EXPLAIN

AMERICAN STEEL BLDGS		HAROLD FRANKEL	
PROJECT	47.7 x 60.0 x 16.0	SIDEWALL FRAMING	
ID	15070R1	DESIGN:	DRAFT: SPW CHECK:
PROJECT ADDRESS	LAKE CITY, FL	DATE: 9/30/05	SHEET 7



6 BOLTS TOTAL

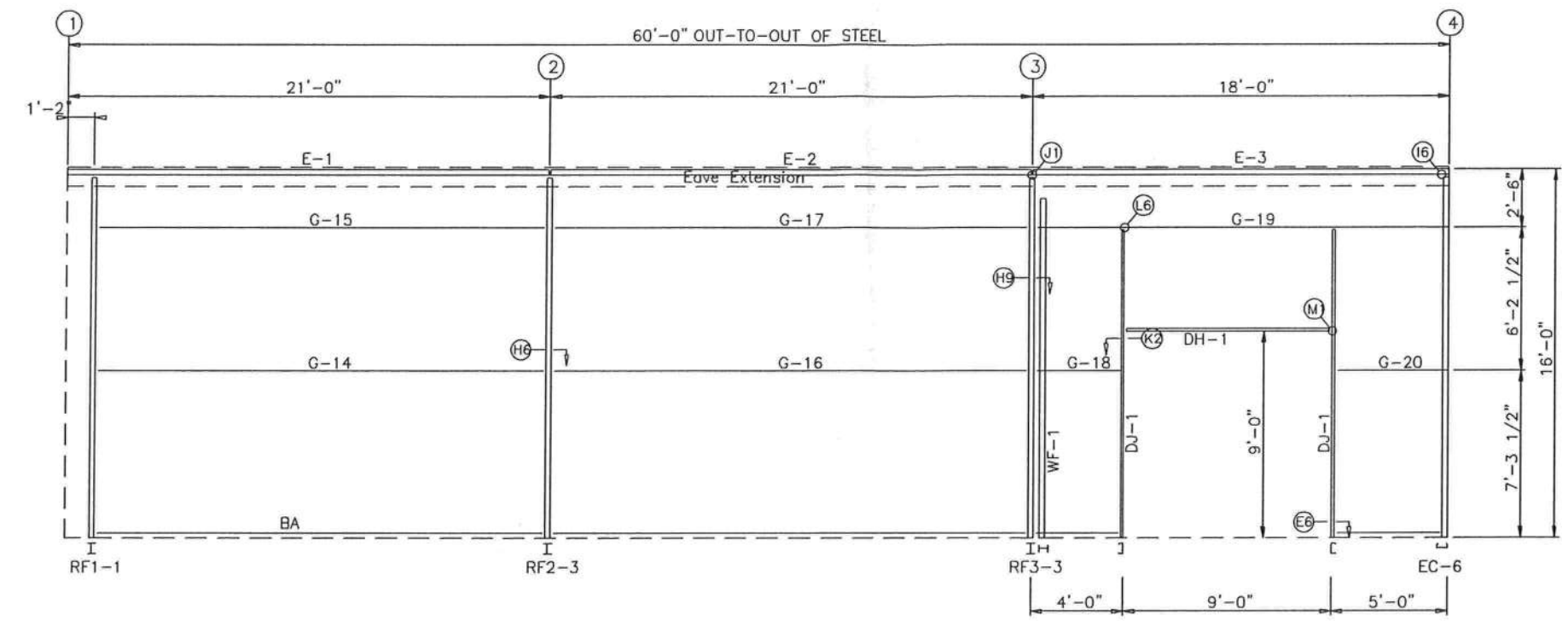
ADDITIONAL HOLES ARE PRESENT FOR ALIGNMENT PURPOSES ONLY

NOTE:
ATTACH WF-1 TO COLUMN
W/ 5/8x1 1/2
A325 BOLTS

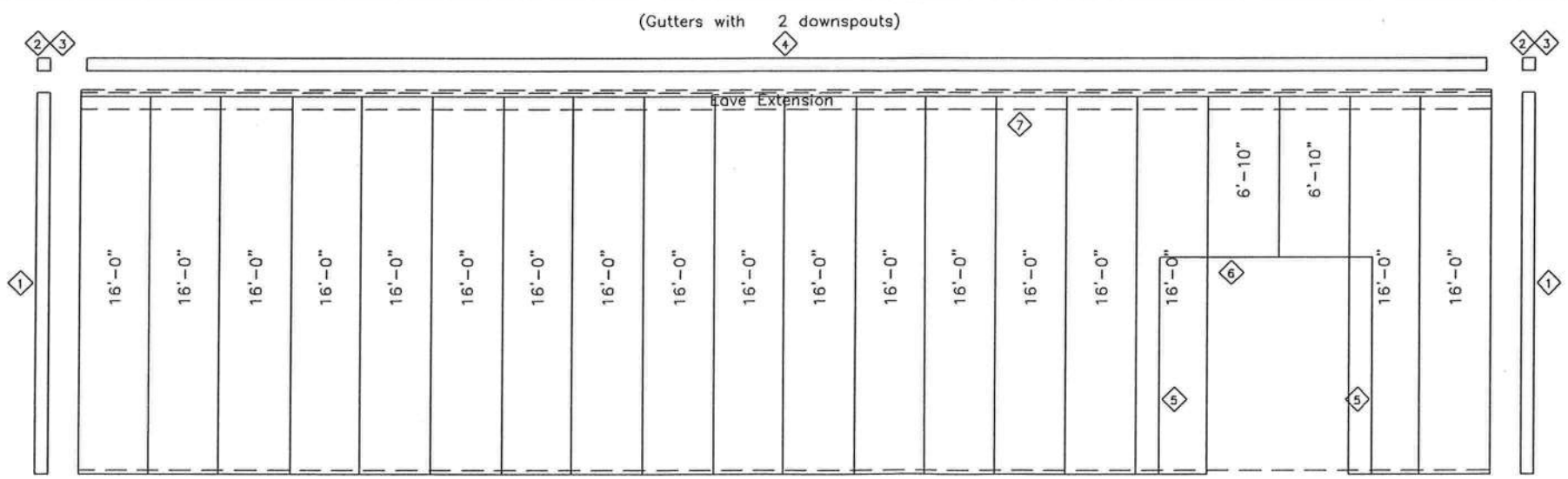
BOLT TABLE FRAME LINE I				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 – RF3-3	4	A325	5/8"	1 1/2"

TRIM TABLE FRAME LINE I			
◇ID	MARK	LENGTH	DETAIL
1	CTR	16'-2"	TM29
2	RVCB	6"	TM38
3	VGE	6"	
4	RVEG	20'-3"	TM50
5	JTR	9'-2"	TM31
6	HT	9'-3"	TM33
7	RSRT	20'-3"	

MEMBER TABLE FRAME LINE I		
MARK	PART	LENGTH
WF-1	W12641	15'-0"
DJ-1	8x35C16	13'-2"
DH-1	8x35C16	9'-0"
E-1	8E16	19'-2 1/2"
E-2	8E16	20'-4 1/2"
E-3	8E16	17'-0 1/2"
G-14	8X25Z13	19'-1 1/4"
G-15	8X25Z16	19'-1 1/4"
G-16	8X25Z13	20'-3 1/4"
G-17	8X25Z16	20'-3 1/4"
G-18	8X25Z16	2'-8"
G-19	8X35Z16	16'-11 1/2"
G-20	8X25Z16	4'-8"



SIDEWALL FRAMING: FRAME LINE I



SIDEWALL SHEETING & TRIM: FRAME LINE I

PANELS: 26 Ga. R – Desert Sand

GENERAL NOTES:

1.) IF CABLE BRACING, WIND BENTS, WIND COLUMNS, OR WEAK AXIS DESIGN OF SIDE WALL COLUMNS WERE NOT PROVIDED IT HAS BEEN DETERMINED THAT DIAPHRAGM PANEL ACTION IS SUFFICIENT TO RESIST LONGITUDINAL FORCES. TEMPORARY BRACING SHOULD BE PROVIDED BY ERECTOR UNTIL ALL WALL AND ROOF PANELS ARE INSTALLED.

REVISIONS						
REV.	DESCRIPTION:	DATE	DTLR	DATE	CHKR	APPD

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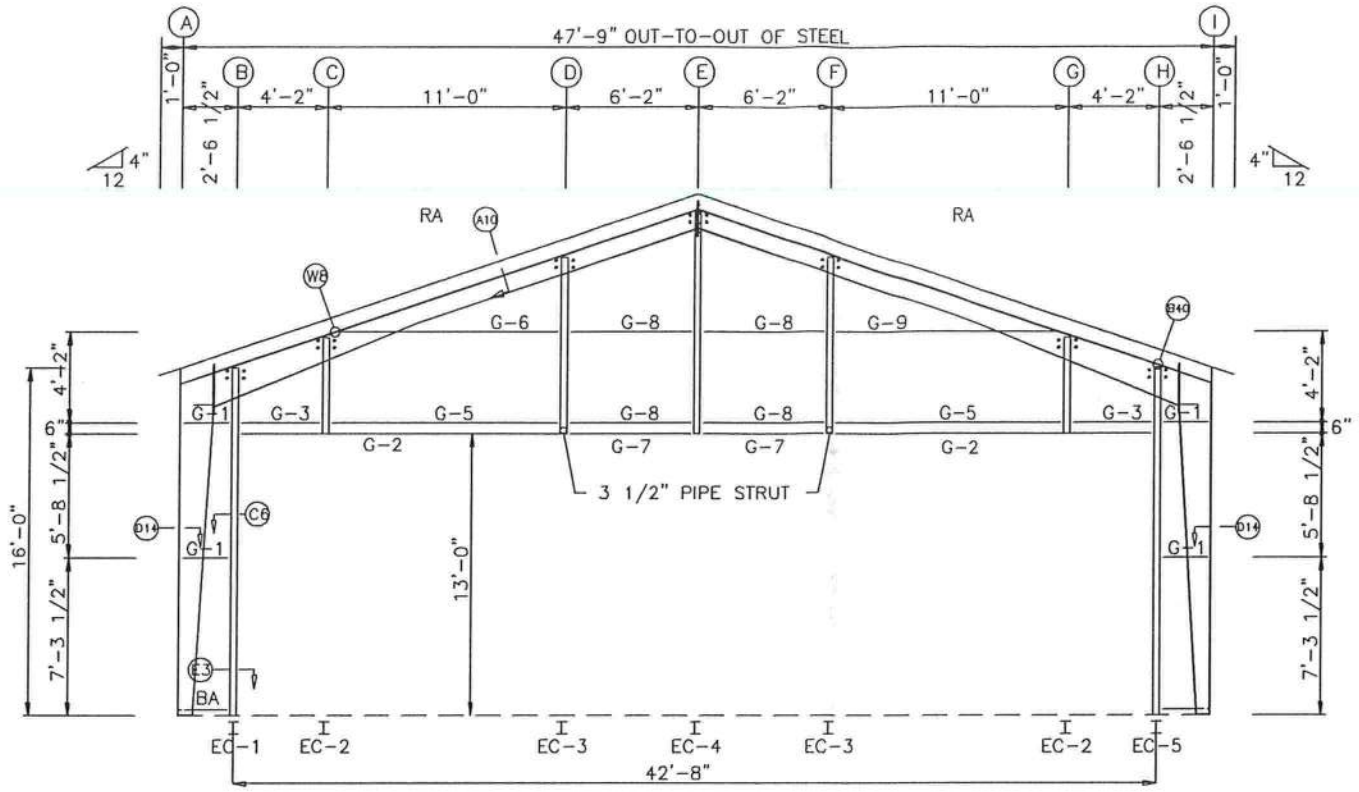
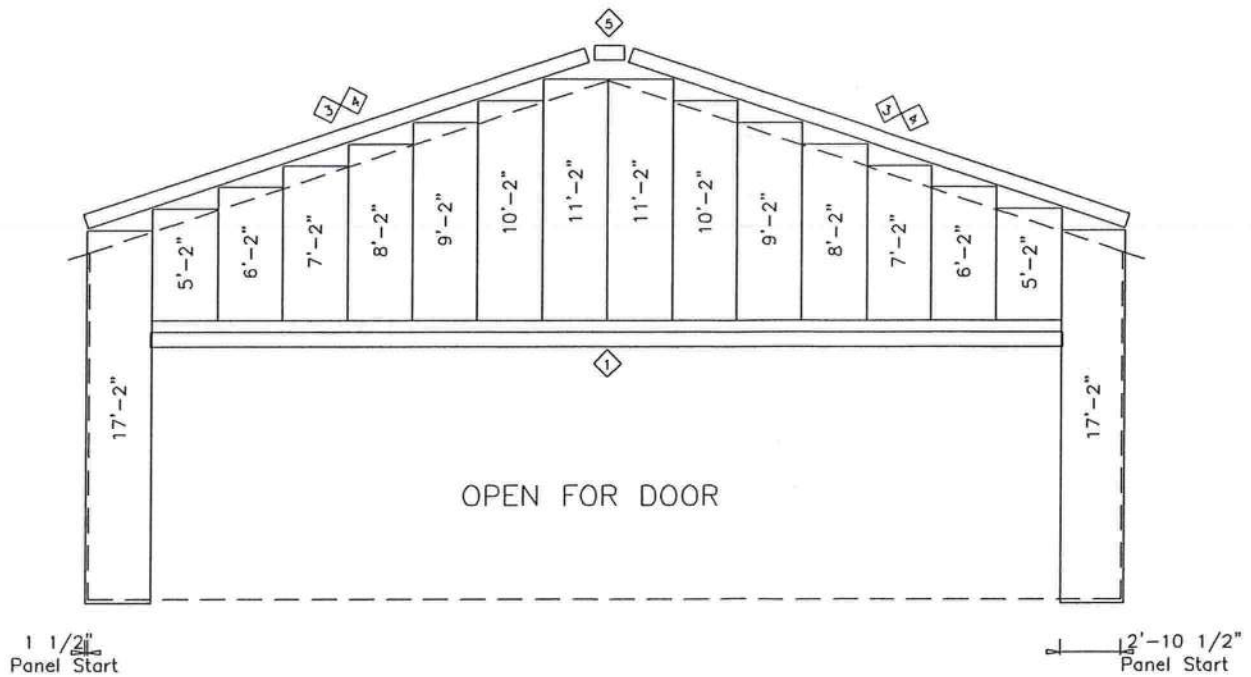
AMERICAN STEEL BLDGS		HAROLD FRANKEL	
PROJECT	47.7 x 60.0 x 16.0	SIDEWALL FRAMING	
ID	15070R1	DESIGN:	DRAFT: SPW / CHECK:
PROJECT ADDRESS	LAKE CITY, FL	DATE: 9/30/05	SHEET 8

SEAL

10/7/05



ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. R – Desert Sand

BOLT TABLE FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
Columns	4	A325	1/2"	1 1/4"

TRIM TABLE FRAME LINE 1			
◇ID	MARK	LENGTH	DETAIL
1	DTR	14'-6"	
3	VRT	20'-3"	TM11
4	VRT	6'-6"	TM11
5	VPB	1'-4"	TM39

MEMBER TABLE FRAME LINE 1		
MARK	PART	LENGTH
EC-1	W8841	15'-11 5/8"
EC-2	W8X10	4'-4 5/16"
EC-3	W8X10	8'-0 5/16"
EC-4	W8X10	10'-0 15/16"
EC-5	W8841	15'-11 5/8"
G-1	8X25Z16	2'-2"
G-2	8X25C14	14'-9 1/2"
G-3	8X25Z16	3'-6"
G-5	8X25Z16	10'-4 1/2"
G-6	8X25Z16	10'-3 7/16"
G-7	8X25C14	12'-3 1/2"
G-8	8X25Z16	5'-6 1/2"
G-9	8X25Z16	10'-3 7/16"

SEE SHEET 11 FOR DOOR FRAMING

GENERAL NOTES:

(1.) IF CABLE BRACING FOR END WALL IS NOT SHOWN ON ERECTION DRAWINGS IT HAS BEEN DETERMINED THAT DIAPHRAGM PANEL ACTION IS SUFFICIENT TO RESIST LONGITUDINAL FORCES. TEMPORARY BRACING SHOULD BE PROVIDED BY ERECTOR UNTIL ALL WALL AND ROOF PANELS ARE INSTALLED.

REVISIONS						
REV.	DESCRIPTION:	DATE	DTLR	DATE	CHKR	APPD

DRAWING STATUS
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[ ] FOR APPROVAL
[ ] OTHER, EXPLAIN

AMERICAN STEEL BLDGS		HAROLD FRANKEL	
PROJECT	47.7 x 60.0 x 16.0	ENDWALL FRAMING	
ID	15070R1	DESIGN:	DRAFT: SPW / CHECK:
PROJECT ADDRESS	LAKE CITY, FL	DATE: 9/30/05	SHEET 9

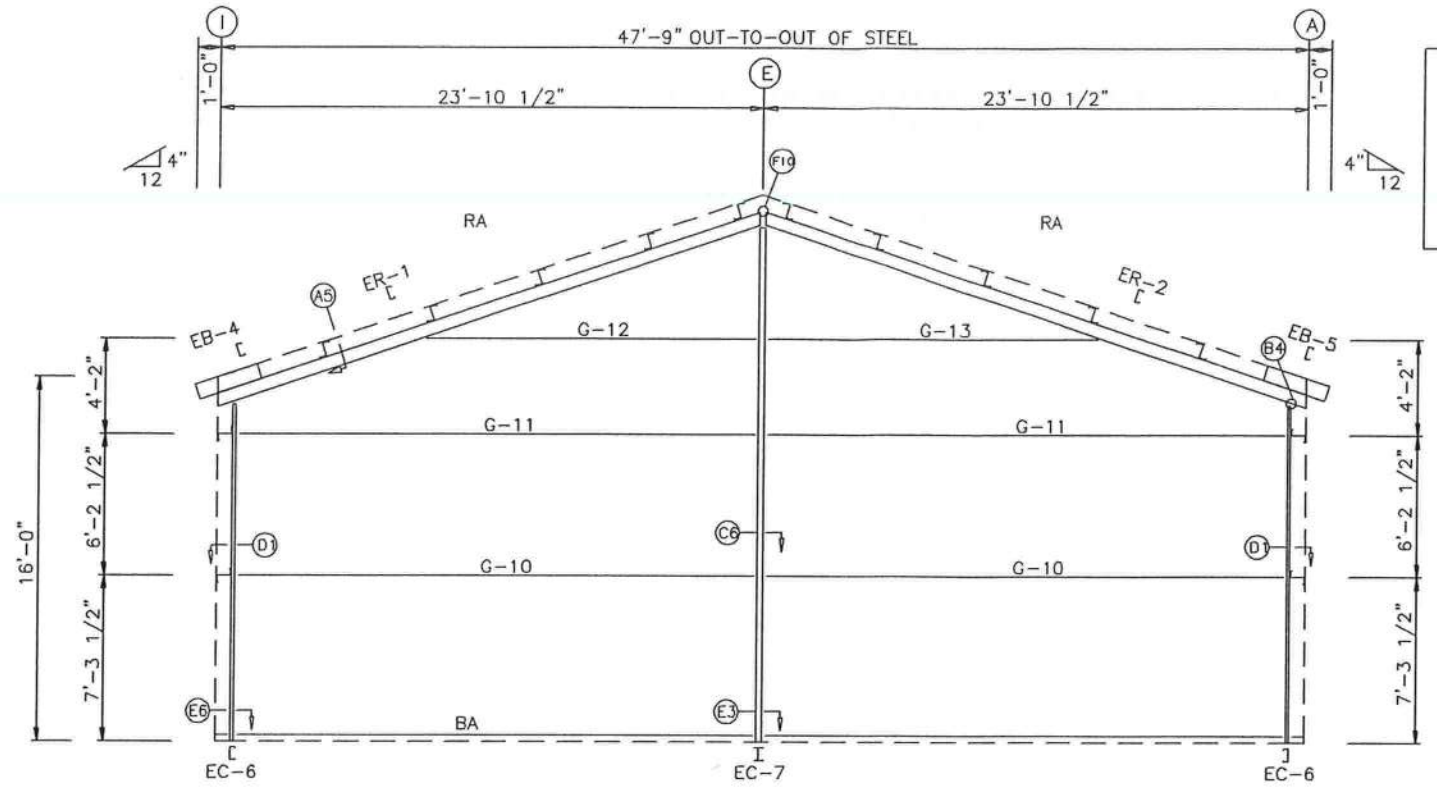
SEAL

10/1/05

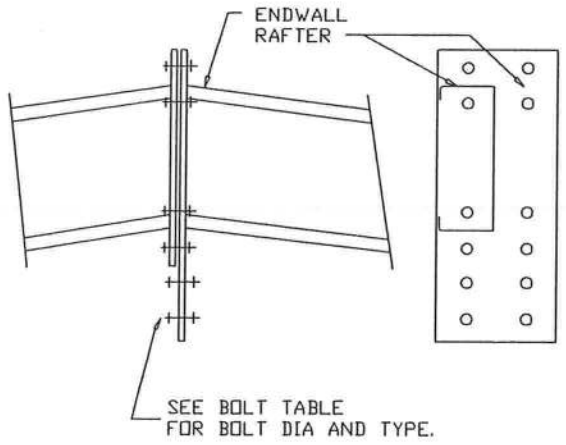
BOLT TABLE FRAME LINE 4				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-2	8	A325	5/8"	2"
Cor_Column/Raf	4	A325	1/2"	1 1/4"
Int_Column/Raf	4	A325	5/8"	1 1/4"

TRIM TABLE FRAME LINE 4			
◇ID	MARK	LENGTH	DETAIL
1	VRT	20'-3"	TM11
2	VRT	6'-6"	TM11
3	VPB	1'-4"	TM39

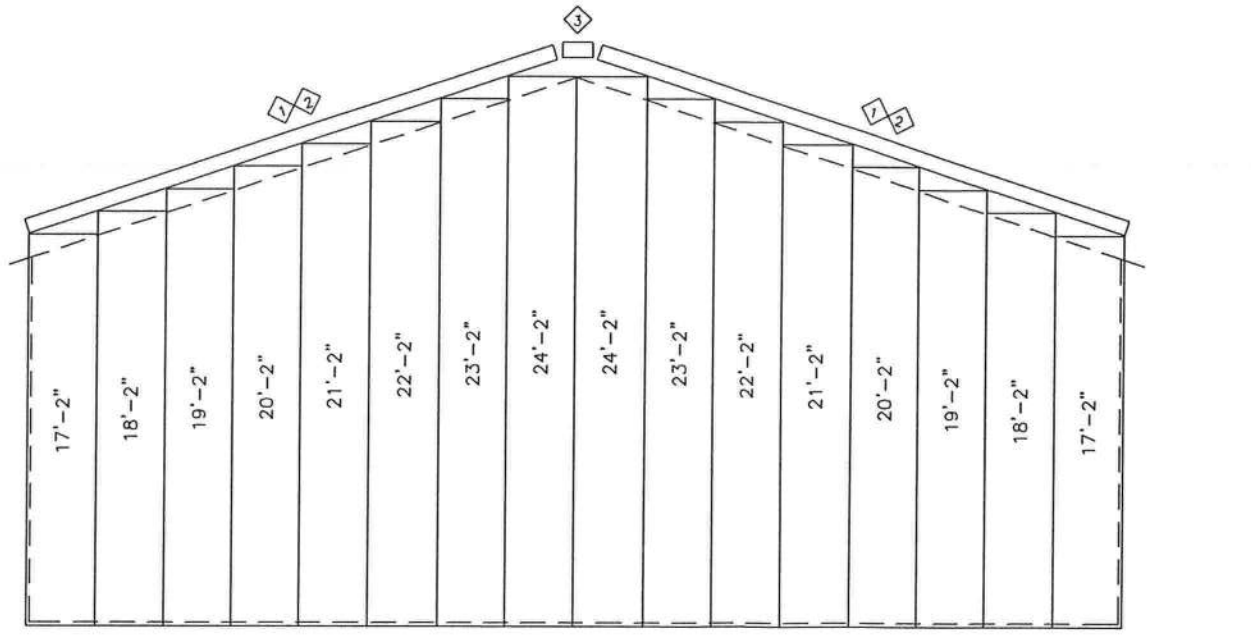
MEMBER TABLE FRAME LINE 4		
MARK	PART	LENGTH
EB-4	8x35C16	2'-4 5/8"
EB-5	8x35C16	2'-4 5/8"
EC-6	8x35C16	14'-6 11/16"
EC-7	W8X13	22'-0 1/4"
ER-1	10x25c13	25'-2"
ER-2	10x25c13	25'-2"
G-10	8X35Z12	22'-7"
G-11	8X25Z13	22'-7"
G-12	8X25Z16	22'-7"
G-13	8X25Z16	18'-6 1/2"



ENDWALL FRAMING: FRAME LINE 4

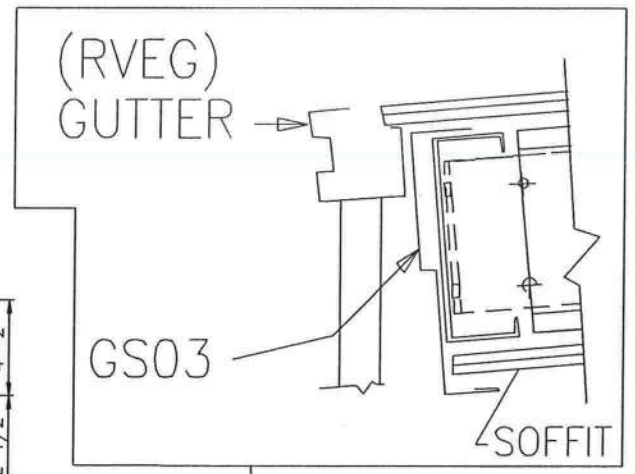


F10 RAFTER SPLICE AT SURFACE CHANGE

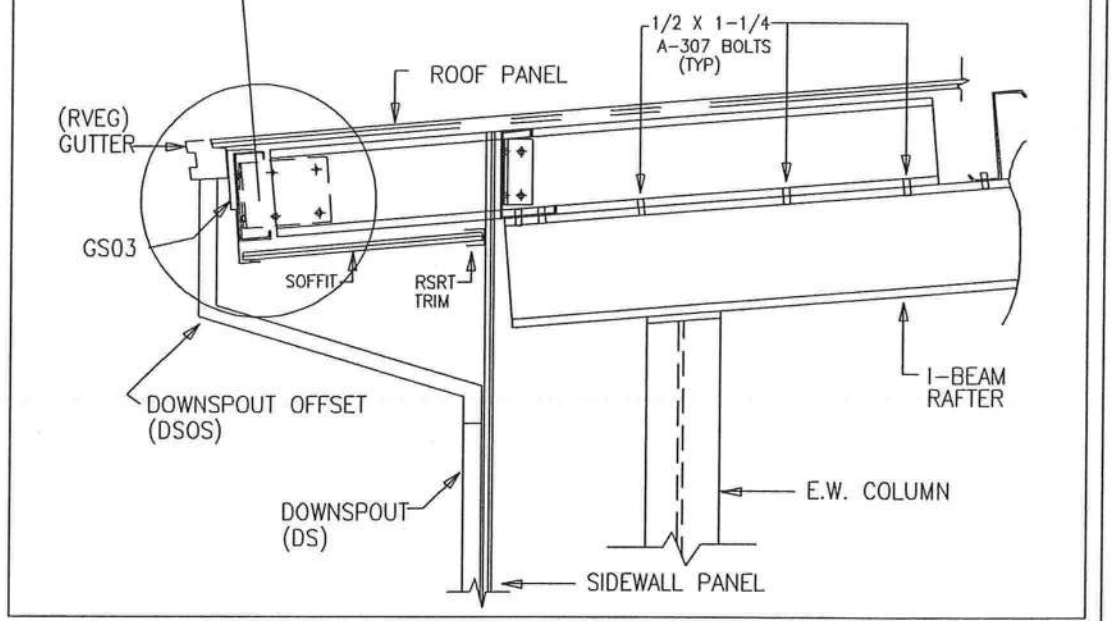


ENDWALL SHEETING & TRIM: FRAME LINE 4

PANELS: 26 Ga. R – Desert Sand



EAVE EXTENSION DETAIL AT ENDWALL



SEAL

10/7/07

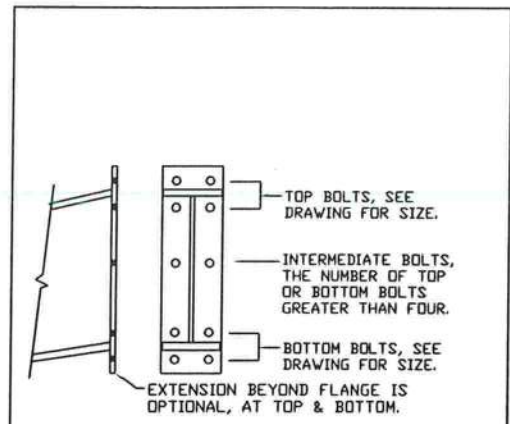
GENERAL NOTES:

1.) IF CABLE BRACING FOR END WALL IS NOT SHOWN ON ERECTION DRAWINGS IT HAS BEEN DETERMINED THAT DIAPHRAGM PANEL ACTION IS SUFFICIENT TO RESIST LONGITUDINAL FORCES. TEMPORARY BRACING SHOULD BE PROVIDED BY ERECTOR UNTIL ALL WALL AND ROOF PANELS ARE INSTALLED.

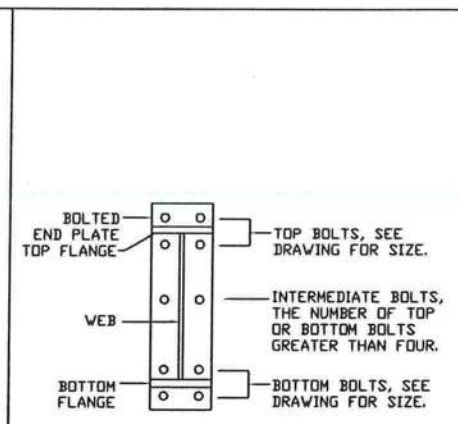
REVISIONS						
REV.	DESCRIPTION:	DATE	DTLR	DATE	CHKR	APPD

DRAWING STATUS
[ ] FOR CONSTRUCTION
[X] FOR PERMIT ONLY
[ ] FOR APPROVAL
[ ] OTHER, EXPLAIN

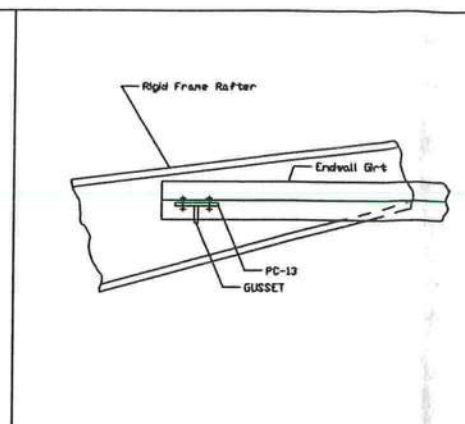
AMERICAN STEEL BLDGS		HAROLD FRANKEL	
PROJECT	47.7 x 60.0 x 16.0	ENDWALL FRAMING	
ID	15070R1	DESIGN:	DRAFT: SPW CHECK:
PROJECT ADDRESS	LAKE CITY, FL	DATE: 9/30/05	SHEET 10



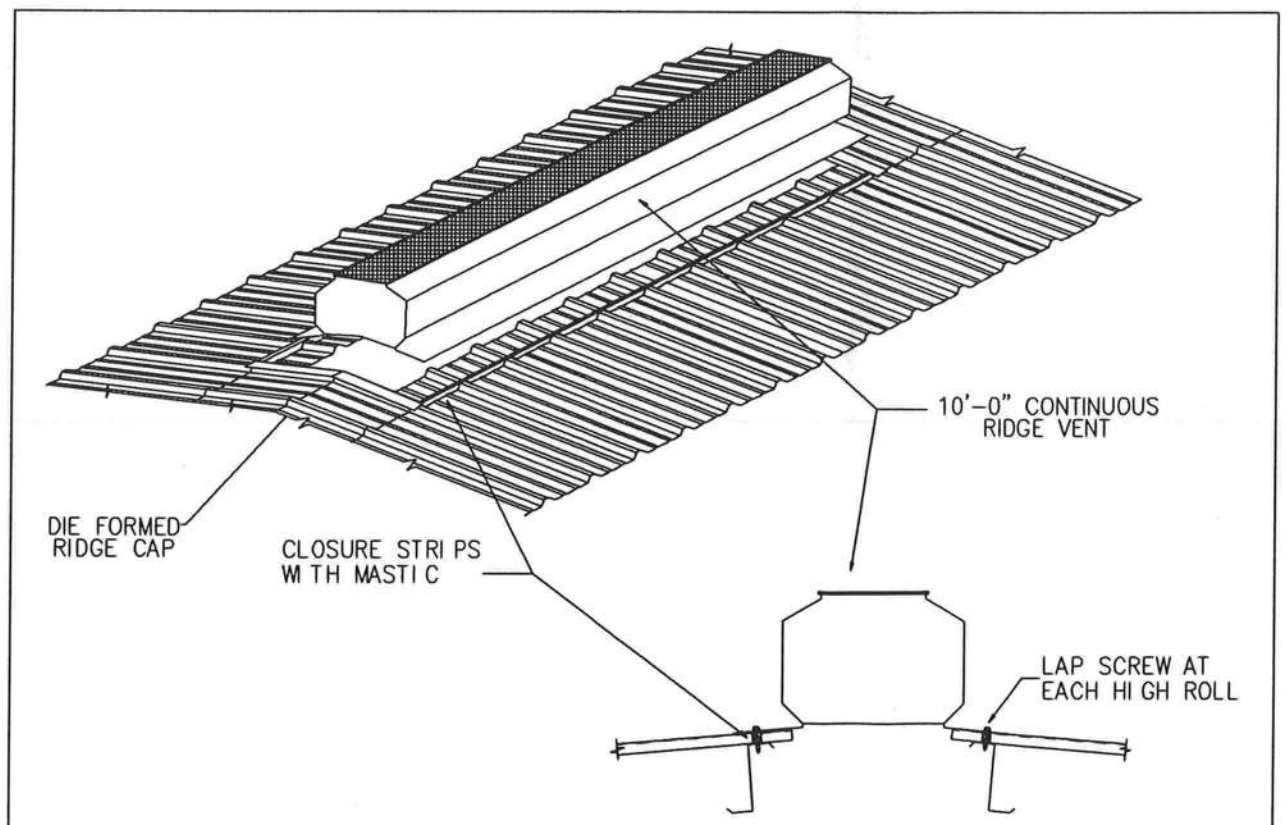
U2 BOLTED END PLATE CONNECTION AT BUILDING PEAK



U3 BOLTS FOR RAFTER TO COLUMN CONNECTION



W8 SECTION OF ENDWALL GIRT TO RAFTER



RIDGE VENT INSTALLATION DETAIL

ridgevent

REVISIONS						
REV.	DESCRIPTION:	DATE	DTLR	DATE	CHKR	APPD

DRAWING STATUS
[ ] FOR CONSTRUCTION
[X] FOR PERMIT ONLY
[ ] FOR APPROVAL
[ ] OTHER, EXPLAIN

AMERICAN STEEL BLDGS		HAROLD FRANKEL	
PROJECT	47.7 x 60.0 x 16.0	Detail Page	
ID	15070R1	DESIGN:	DRAFT: SPW / CHECK:
PROJECT ADDRESS	LAKE CITY, FL	DATE: 9/30/05	SHEET 14

SEAL

10/7/05