

STILL NEEDED - DOES NOT
MATCH PARCEL

Columbia County New Building Permit Application

☒ Cody Barrs Needs to Sign Subform

For Office Use Only Application # 44001 Date Received 11/7 By MG Permit # 38946
Zoning Official LW Date 11-12-19 Flood Zone X Land Use Ag Zoning A-3
FEMA Map # _____ Elevation _____ MFE _____ River _____ Plans Examiner T.C. Date 11-20-19
Comments 1' above road EXISTING
☐ NOC ☐ EH ☐ Deed or PA ☐ Site Plan ☐ State Road Info ☐ Well letter ☒ 911 Sheet ☐ Parent Parcel # _____
☐ Dev Permit # _____ ☐ In Floodway ☐ Letter of Auth. from Contractor ☐ F W Comp. letter
☐ Owner Builder Disclosure Statement ☐ Land Owner Affidavit ☐ Ellisville Water ☒ App Fee Paid ☒ Sub VF Form

Septic Permit No., 19-0859 OR City Water ☐ Fax _____ *not complete

Applicant (Who will sign/pickup the permit) Cecilio Garcia Phone 352 494-4657

Address 7490 NE 30 ST High Springs FL 32643

Owners Name MARVIN ARAGON Phone 352 281-7095

911 Address 316 SW NEWPORT LANE FORT WHITE FL 32038

Contractors Name Cecilio Garcia Phone 352 494-4657

Address 7490 NE 30 ST High Springs FL 32643

Contractor Email KGconstructionllc@yahoo.com ***Include to get updates on this job.

Fee Simple Owner Name & Address _____

Bonding Co. Name & Address _____

Architect/Engineer Name & Address _____

Mortgage Lenders Name & Address _____

Circle the correct power company ☐ FL Power & Light ☒ Clay Elec. ☐ Suwannee Valley Elec. ☐ Duke Energy

Property ID Number 29-65-16-03970-005 Estimated Construction Cost \$139,000

Subdivision Name Rolling ACRES Lot 5 Block _____ Unit _____ Phase _____

Driving Directions from a Major Road ROAD 27 To CENTERVILLE AVE
Right, And LEFT on NEWPORT LANE

Construction of Single Family Home Commercial OR ☒ Residential

Proposed Use/Occupancy _____ Number of Existing Dwellings on Property 0

Is the Building Fire Sprinkled? NO If Yes, blueprints included _____ Or Explain _____

Circle Proposed ☐ Culvert Permit or ☐ Culvert Waiver or ☐ D.O.T. Permit or ☒ Have an Existing Drive

Actual Distance of Structure from Property Lines - Front _____ Side _____ Side _____ Rear _____

Number of Stories 1 Heated Floor Area 1450 Total Floor Area 2290 Acreage 2.48

Zoning Applications applied for (Site & Development Plan, Special Exception, etc.) N/A

Columbia County Building Permit Application

CODE: Florida Building Code 2014 and the 2011 National Electrical Code.

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless pursued in good faith or a permit has been issued.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO CONTRACTOR AND AGENT: **YOU ARE HEREBY NOTIFIED** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. You must verify if your property is encumbered by any restrictions or face possible litigation and/or fines.

Marvin Aragon
Print Owners Name

Marvin Aragon
Owners Signature

****Property owners must sign here before any permit will be issued.**

****If this is an Owner Builder Permit Application then, ONLY the owner can sign the building permit when it is issued.**

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

[Signature]
Contractor's Signature

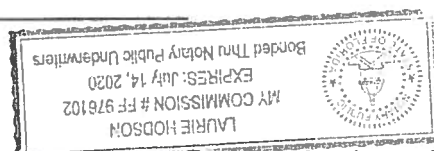
Contractor's License Number CRC1329233
Columbia County
Competency Card Number 1699 Exemption

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 7th day of November 2019.

Personally known _____ or Produced Identification FIDL

[Signature]
State of Florida Notary Signature (For the Contractor)

SEAL:



SUBCONTRACTOR VERIFICATION

APPLICATION/PERMIT # 44001 JOB NAME MARVIN ARAGON
THIS FORM MUST BE SUBMITTED BEFORE A PERMIT WILL BE ISSUED *HG Const.*

Columbia County issues combination permits. One permit will cover all trades doing work at the permitted site. It is **REQUIRED** that we have records of the subcontractors who actually did the trade specific work under the general contractors permit.

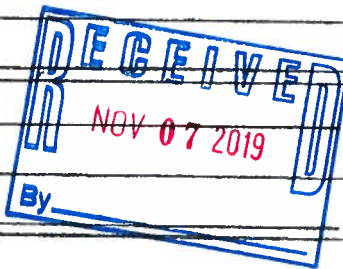
NOTE: It shall be the responsibility of the general contractor to make sure that all of the subcontractors are licensed with the Columbia County Building Department.

Use website to confirm licenses: <http://www.columbiacountyfla.com/PermitSearch/ContractorSearch.aspx>

NOTE: If this should change prior to completion of the project, it is your responsibility to have a corrected form submitted to our office, before that work has begun.

Violations will result in stop work orders and/or fines.

ELECTRICAL <input checked="" type="checkbox"/>	Print Name <u>LEE A. Holloway</u> Signature <u>[Signature]</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Lab <input type="checkbox"/> W <input type="checkbox"/> EX <input type="checkbox"/> DE
CC# <u>891</u>	Company Name: <u>PENINSULAR ELECT. CO., INC.</u> License #: <u>EC 0000668</u> Phone #: <u>352-665-1775</u>	
MECHANICAL/ A/C <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Lab <input type="checkbox"/> W <input type="checkbox"/> EX <input type="checkbox"/> DE
PLUMBING/ GAS <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Lab <input type="checkbox"/> W <input type="checkbox"/> EX <input type="checkbox"/> DE
ROOFING <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Lab <input type="checkbox"/> W <input type="checkbox"/> EX <input type="checkbox"/> DE
SHEET METAL <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Lab <input type="checkbox"/> W <input type="checkbox"/> EX <input type="checkbox"/> DE
FIRE SYSTEM/ SPRINKLER <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Lab <input type="checkbox"/> W <input type="checkbox"/> EX <input type="checkbox"/> DE
SOLAR <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Lab <input type="checkbox"/> W <input type="checkbox"/> EX <input type="checkbox"/> DE
STATE SPECIALTY <input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Lab <input type="checkbox"/> W <input type="checkbox"/> EX <input type="checkbox"/> DE



Columbia County Property Appraiser

Jeff Hampton

2019 Preliminary Certified Values

updated: 8/14/2019

Retrieve Tax Record

2019 TRIM (pdf)

Property Card

Parcel List Generator

Show on GIS Map

Print

Parcel: << 29-6S-16-03970-005 >>

Aerial Viewer

Pictometry

Google Maps

Owner & Property Info

Show Search Results

Owner	ARAGON MARVIN P O BOX 171 HIGH SPRINGS, FL 32655		
Site			
Description*	LOT 5 ROLLING ACRES ADDITION. ORB 662-689, 663-363, 830-1053 834-737, WD 1029-2123, DC 1342-1858, QC 1342-1860, WD 1344- 1150,		
Area	2.48 AC	S/T/R	29-6S-16
Use Code**	VACANT (000000)	Tax District	3

*The Description above is not to be used as the Legal Description for this parcel in any legal transaction.

**The Use Code is a FL Dept. of Revenue (DOR) code and is not maintained by the Property Appraiser's office. Please contact your city or county Planning & Zoning office for specific zoning information.

Property & Assessment Values

2018 Certified Values		2019 Preliminary Certified	
Mkt Land (1)	\$18,392	Mkt Land (1)	\$18,392
Ag Land (0)	\$0	Ag Land (0)	\$0
Building (0)	\$0	Building (0)	\$0
XFOB (0)	\$0	XFOB (0)	\$0
Just	\$18,392	Just	\$18,392
Class	\$0	Class	\$0
Appraised	\$18,392	Appraised	\$18,392
SOH Cap [?]	\$0	SOH Cap [?]	\$0
Assessed	\$18,392	Assessed	\$18,392
Exempt	\$0	Exempt	\$0
Total Taxable	county:\$18,392 city:\$18,392 other:\$18,392 school:\$18,392	Total Taxable	county:\$18,392 city:\$18,392 other:\$18,392 school:\$18,392



Sales History

Show Similar Sales within 1/2 mile

Fill out Sales Questionnaire

Sale Date	Sale Price	Book/Page	Deed	V/I	Quality (Codes)	RCode
9/15/2017	\$17,000	1344/1150	WD	V	Q	01
8/8/2017	\$6,000	1342/1860	QC	V	U	11
10/25/2004	\$18,000	1029/2123	WD	V	Q	
1/29/1997	\$17,800	834/0737	WD	I	Q	
8/1/1996	\$17,900	830/1053	QC	I	U	01
9/28/1988	\$19,400	663/0363	CD	V	U	
10/12/1987	\$20,300	662/0689	QC	V	U	
1/1/1986	\$21,500	584/0523	WD	V	U	01
10/1/1984	\$1,200	548/0546	WD	V	U	01
2/1/1982	\$5,700	485/0345	WD	V	Q	

Building Characteristics

Bldg Sketch	Bldg Item	Bldg Desc*	Year Blt	Base SF	Actual SF	Bldg Value
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NONE

District No. 1 - Ronald Williams
District No. 2 - Rusty DePratter
District No. 3 - Bucky Nash
District No. 4 - Everett Phillips
District No. 5 - Tim Murphy

BOARD OF COUNTY COMMISSIONERS • COLUMBIA COUNTY



Address Assignment and Maintenance Document

To maintain the county wide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for addressing and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Services Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County

Date/Time Issued: **2/19/2018 10:50:22 AM**

Address: **316 SW NEWPORT PI**

City: **FORT WHITE**

State: **FL**

Zip Code **32038**

Parcel ID **03970-005**

REMARKS: Address for proposed structure on parcel.

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION AND ACCESS INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION AND/OR ACCESS INFORMATION BE FOUND TO BE IN ERROR OR CHANGED, THIS ADDRESS IS SUBJECT TO CHANGE.

Address Issued By: **Signed:/ Matt Crews**

Columbia County GIS/911 Addressing Coordinator

**COLUMBIA COUNTY
911 ADDRESSING / GIS DEPARTMENT**

**263 NW Lake City Ave., Lake City, FL 32055 Telephone: (386) 758-1125
Email: gis@columbiacountyfla.com**

Legend

Parcels

2018 Flood Zones

0.2 PCT ANNUAL CHANCE

- A
- AE
- AH

2018 Aerials

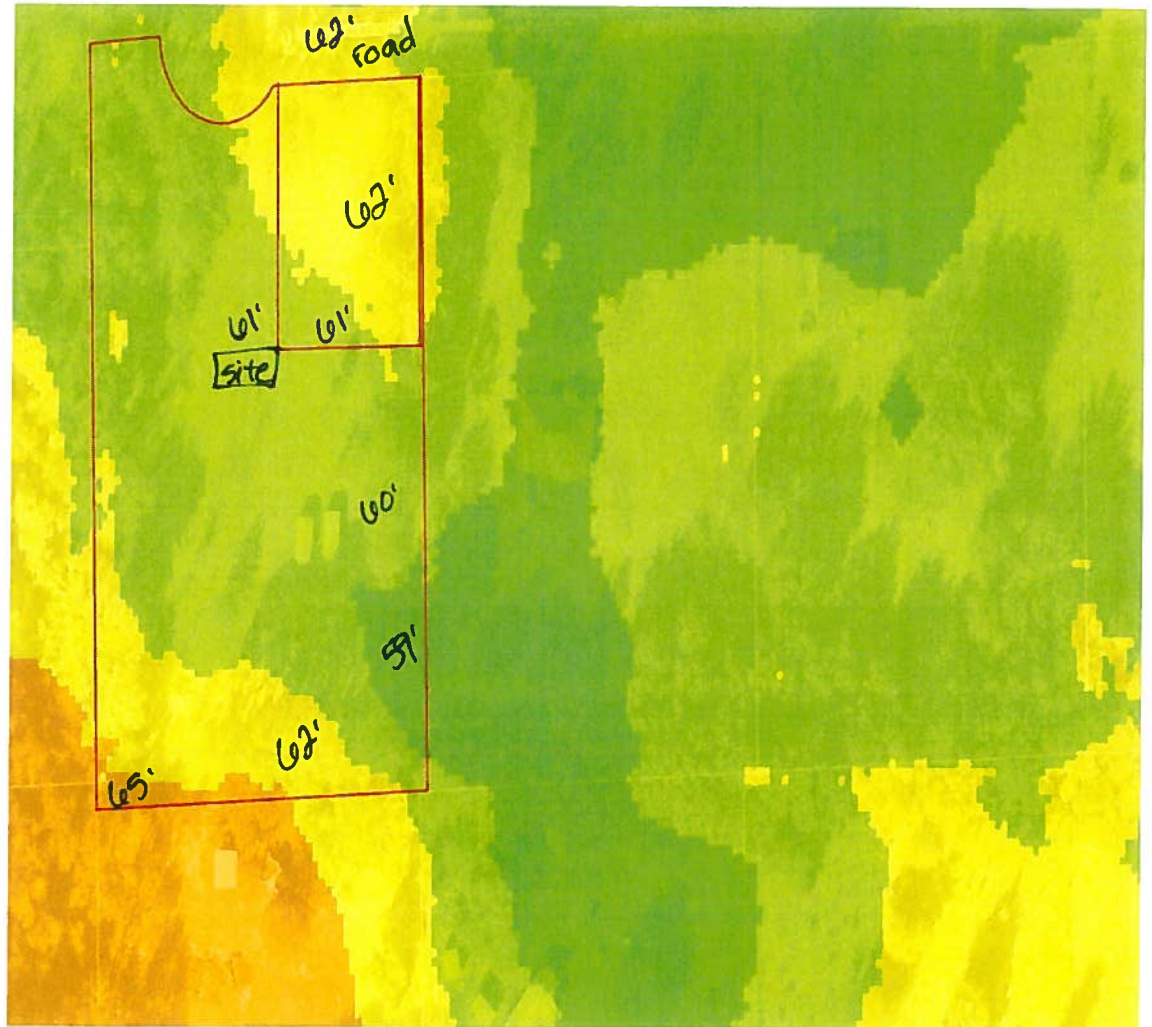
SRWMD Wetlands

Roads

- Roads
- others
- Dirt
- Interstate
- Main
- Other
- Paved
- Private
- Lidar Elevations

Columbia County, FLA - Building & Zoning Property Map

Printed: Tue Nov 12 2019 10:25:47 GMT-0500 (Eastern Standard Time)



Parcel Information

Parcel No: 29-6S-16-03970-005

Owner: ARAGON MARVIN

Subdivision: ROLLING ACRES ADDITION

Lot: 5

Acres: 2.49519944

Deed Acres: 2.48 Ac

District: District 2 Rocky Ford

Future Land Uses: Agriculture - 3

Flood Zones:

Official Zoning Atlas: A-3

SUBCONTRACTOR VERIFICATION

APPLICATION/PERMIT # Aragon JOB NAME _____

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ELECTRICAL	<input checked="" type="checkbox"/>	Print Name <u>Lee A. Holloway</u> Signature <u>[Signature]</u> Company Name: <u>PENINSULAR ELECTRIC COMPANY INC.</u> License #: <u>EC0000128</u> Phone #: <u>352 665-1775</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
MECHANICAL/	<input checked="" type="checkbox"/>	Print Name <u>Richard Touchstone</u> Signature <u>[Signature]</u> Company Name: <u>TOUCHSTONE AC/HEAT INC.</u> License #: <u>CACB 58099</u> Phone #: <u>386 867-0625</u>	Need <input type="checkbox"/> Lic <input checked="" type="checkbox"/> Liab <input checked="" type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
PLUMBING/	<input checked="" type="checkbox"/>	Print Name <u>CODY BARRS</u> Signature <u>[Signature]</u> Company Name: <u>BARRS Plumbing</u> License #: <u>CFC 1427145</u> Phone #: <u>386 623-0509</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
ROOFING	<input checked="" type="checkbox"/>	Print Name <u>Cecilio GARCIN</u> Signature <u>[Signature]</u> Company Name: <u>KG construction</u> License #: <u>CRC 1329233</u> Phone #: <u>352 494 4657</u>	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input checked="" type="checkbox"/> EX <input type="checkbox"/> DE
SHEET METAL	<input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
FIRE SYSTEM/	<input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
SOLAR	<input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
STATE	<input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE
SPECIALTY	<input type="checkbox"/>	Print Name _____ Signature _____ Company Name: _____ License #: _____ Phone #: _____	Need <input type="checkbox"/> Lic <input type="checkbox"/> Liab <input type="checkbox"/> W/C <input type="checkbox"/> EX <input type="checkbox"/> DE



STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE TREATMENT AND DISPOSAL
SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT

PERMIT NO. 19-0859
DATE PAID: 11/23/19
FEE PAID: 200.00
RECEIPT #: 1454466

APPLICATION FOR:

[] New System [☒] Existing System [] Holding Tank [] Innovative
[] Repair [] Abandonment [] Temporary []

APPLICANT: MARVIN ARAGON
AGENT: CECILIO GARCIA / KG CONSTRUCTION TELEPHONE: 352 494 4651
MAILING ADDRESS: 316 SW NEWPORT PL FORT WHITE, FL 32038

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTED BY A PERSON LICENSED PURSUANT TO 489.105(3)(m) OR 489.552, FLORIDA STATUTES. IT IS THE APPLICANT'S RESPONSIBILITY TO PROVIDE DOCUMENTATION OF THE DATE THE LOT WAS CREATED OR PLATTED (MM/DD/YY) IF REQUESTING CONSIDERATION OF STATUTORY GRANDFATHER PROVISIONS.

PROPERTY INFORMATION

LOT: 5 BLOCK: _____ SUBDIVISION: Rolling Acres Add. PLATTED: 1978
PROPERTY ID #: 29-65-16-03970-005 ZONING: _____ I/M OR EQUIVALENT: [Y] [☒ N]
PROPERTY SIZE: 2.49 ACRES WATER SUPPLY: [☒] PRIVATE PUBLIC [] <=2000GPD [] >2000GPD
IS SEWER AVAILABLE AS PER 381.0065, FS? [Y] [☒ N] DISTANCE TO SEWER: _____ FT
PROPERTY ADDRESS: 316 SW NEWPORT PL. FORT WHITE FL 32038
DIRECTIONS TO PROPERTY: NORTH of FORT WHITE on SR 27
Right on CENTERVILLE THEN LEFT ON NEWPORT
LAST LOT ON LEFT

BUILDING INFORMATION

[] RESIDENTIAL [] COMMERCIAL

Unit No	Type of Establishment	No. of Bedrooms	Building Area Sqft	Commercial/Institutional System Design Table 1, Chapter 64E-6, FAC
1	<u>RESIDENTIAL</u>	<u>3</u>	<u>1450</u>	<u>ORIGINAL ATTACHED</u>
2				
3				
4				

[] Floor/Equipment Drains [] Other (Specify) _____

SIGNATURE: _____

DATE: 11-21-19

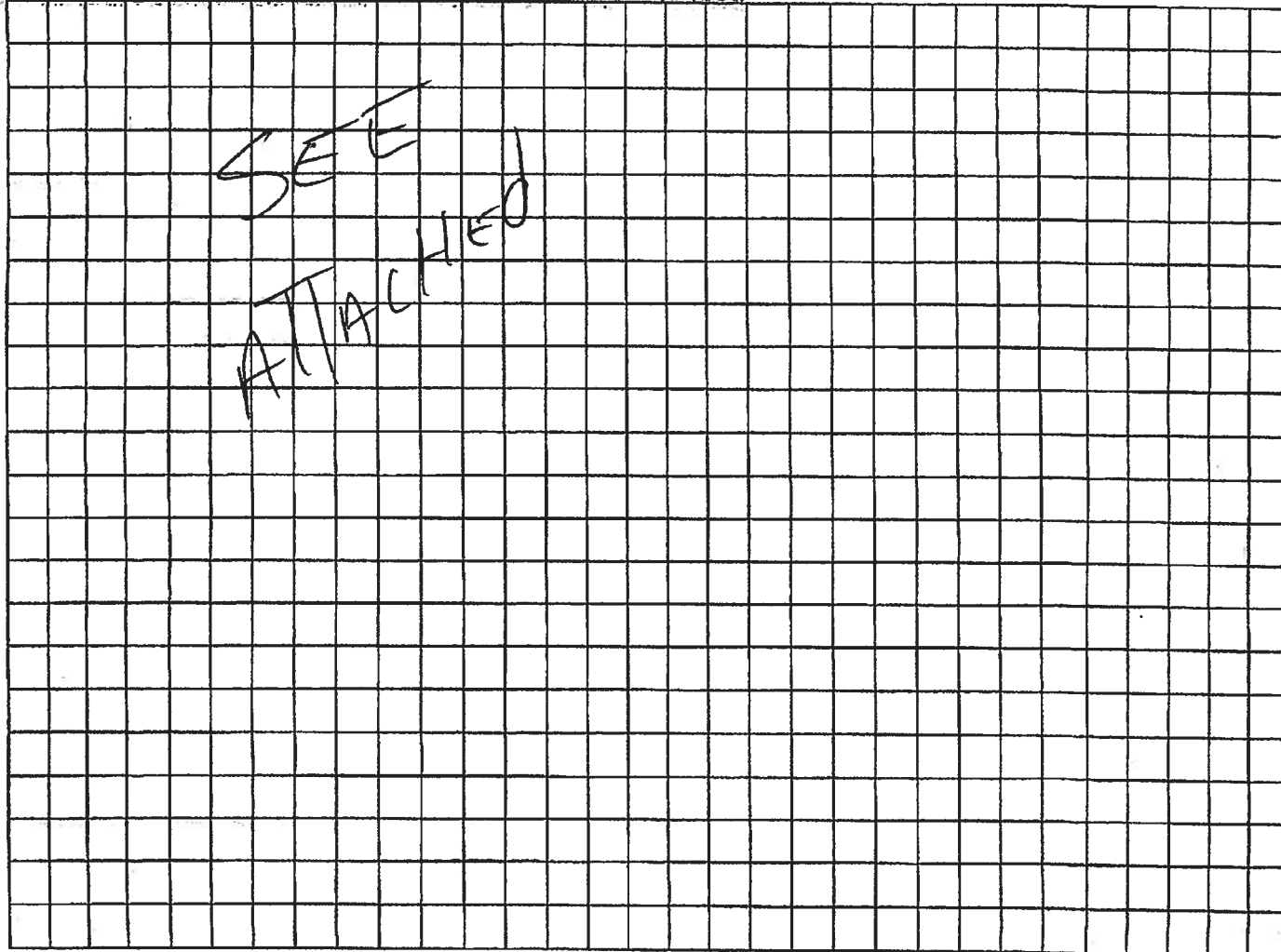
STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR CONSTRUCTION PERMIT

Permit Application Number

19-0859

----- PART II - SITEPLAN -----

Scale: Each block represents 10 feet and 1 inch = 40 feet.



Notes: _____

Site Plan submitted by: _____

TITLE _____

DATE: 11-21-19

Plan Approved _____

Not Approved _____

Date 11/23/19

By _____

Columbin

County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT



STATE OF FLORIDA
DEPARTMENT OF HEALTH
ON-SITE SEWAGE TREATMENT AND DISPOSAL
SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT

PERMIT NO. 19-0859
DATE PAID: 11/23/19
FEE PAID: 200.00
RECEIPT #: 1754466

APPLICATION FOR:
☒ New System ☐ Existing System ☐ Holding Tank ☐ Innovative
☐ Repair ☐ Abandonment ☐ Temporary ☐

APPLICANT: MARVIN ARAGON

AGENT: CECILIO GARCIA / K&G Construction TELEPHONE: 352 494 4657

MAILING ADDRESS: 316 SW NEWPORT BL FORT WHITE, FL 32038

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTED BY A PERSON LICENSED PURSUANT TO 489.105 (3) OR 489.552, FLORIDA STATUTES. IT IS THE APPLICANT'S RESPONSIBILITY TO PROVIDE DOCUMENTATION OF THE DATE THE LOT WAS CREATED OR PLATTED (MM/DD/YY) IF REQUESTING CONSIDERATION OF STATUTORY GRANDFATHER PROVISIONS.

PROPERTY INFORMATION

LOT: 5 BLOCK: Rolling Acres Add. PLATTED: 1978

PROPERTY ID #: 29-68-16-03970-005 ZONING: 1/M OR EQUIVALENT: (Y/N)

PROPERTY SIZE: 2.49 ACRES WATER SUPPLY: ☒ PRIVATE PUBLIC ☐ <2000 GPD ☐ >2000 GPD
IS SEWER AVAILABLE AS PER 381.0065, FSP? ☒ YES ☐ NO DISTANCE TO SEWER: FT

PROPERTY ADDRESS: 316 SW NEWPORT BL. FORT WHITE FL 32038

DIRECTIONS TO PROPERTY: NORTH of FORT WHITE on R 27
RIGHT on CENTERVILLE then LEFT on NEWPORT
LAST LOT on LEFT

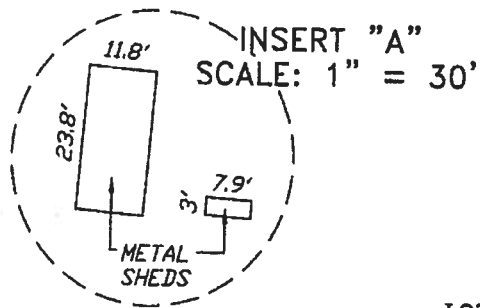
BUILDING INFORMATION		<input type="checkbox"/> RESIDENTIAL	<input type="checkbox"/> COMMERCIAL
Unit No.	Type of Establishment	No. of Bedrooms	Building Area Sqft
1	<u>RESIDENTIAL</u>	<u>3</u>	<u>1450</u>
2			ORIGINAL ATTACHED
3			
4			

☐ Floor/Equipment Drains ☐ Other (Specify) _____

SIGNATURE: _____ DATE: 11-21-19

DR 4015, 08/09 (Obsolesces previous editions which may not be used)
Incorporated 64E-6.001, FAC

17-0854
19-0850



LOT 6

MELVIN E. & MARY S. MOCK
29-6S-16-03974-006

P.L.S. 3048

SW NEWPORT PLACE (DIRT/PUBLIC)

FENCE CORNER
IS 29.91' SOUTH
& 9.58' EAST

NOT LABELED
FENCE CORNER
IS 0.03' NORTH
& 0.44' WEST

(BEARING BASIS)
N.01°17'18"W.
439.03' (PLAT)
N.01°17'18"W.
438.64' (FIELD)

LOT 5
2.49 ACRES ±
1,450

LOT 4

STEVEN D. & DIANNE S. LUTZ
29-6S-16-03969-012

SEE INSERT "A"

SEPTIC TANK

FENCE CORNER
IS 0.08' NORTH
& 1.63' WEST
NOT LABELED

DAYNA M. CLIFFORD
29-6S-16-03969-016

P.R.M. 3
P.L.S. 1950

FENCE CORNER
IS 0.22' NORTH
& 0.02' WEST

1/2" IRON ROD

S.01°08'59"E.
408.84' (FIELD)
S.01°17'18"E.
409.03' (PLAT)

P.L.S. 3048

FENCE CORNER
IS 1.38' NORTH
& 0.15' EAST

CERTIFIED TO:

MARVIN ARAGON

SURVEYOR'S C

I HEREBY CERTIFY THAT THIS SURVEY WAS MADE UP
TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA
IN CHAPTER 5J-17, FLORIDA ADMINISTRATIVE CODE, 1

10/24/2019
FIELD SURVEY DATE

10/28/2019
DRAWING DATE

NOTE: UNLESS IT BEARS THE ORIGINAL SIGNATURE AND 1
AND MAPPER THIS DRAWING, SKETCH, PLAT OR MAP IS F1

FIELD BOOK: SEF

PAGE(S): FILE



COLUMBIA COUNTY BUILDING DEPARTMENT RESIDENTIAL CHECK LIST

MINIMUM PLAN REQUIREMENTS: FLORIDA BUILDING CODE RESIDENTIAL 2017 EFFECTIVE 1 JANUARY 2018
AND THE NATIONAL ELECTRICAL 2014 EFFECTIVE 1 JANUARY 2018

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE WITH THE CURRENT FLORIDA BUILDING CODES RESIDENTIAL AND THE NATIONAL ELECTRICAL CODE. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS, FBC 1609.3.1 THRU 1609.3.3.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FLORIDA BUILDING CODE FIGURE 1609-A THROUGH 1609-C ULTIMATE DESIGN WIND SPEEDS FOR RISK CATEGORY AND BUILDINGS AND OTHER STRUCTURES

Revised 7/1/18

Website: <http://www.columbiacountyfla.com/BuildingandZoning.asp>

Items to Include-
Each Box shall be
Circled as
Applicable

GENERAL REQUIREMENTS:

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

Select From Drop down

1	Two (2) complete sets of plans containing the following:	<input checked="" type="checkbox"/>			
2	All drawings must be clear, concise, drawn to scale, details that are not used shall be marked void	<input type="checkbox"/>			
3	Condition space (Sq. Ft.) 1450	Total (Sq. Ft.) under roof 2290	Yes	No	NA

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES RESIDENTIAL 107.1.

Site Plan information including:

4	Dimensions of lot or parcel of land	Yes		
5	Dimensions of all building set backs	Yes		
6	Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.	Yes		
7	Provide a full legal description of property.	Yes		

Wind-load Engineering Summary, calculations and any details are required.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
8	Plans or specifications must show compliance with FBCR Chapter 3	Yes	No	NA
		Select From Drop down		
9	Basic wind speed (3-second gust), miles per hour	Yes		
10	(Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated)	Yes		
11	Wind importance factor and nature of occupancy	Yes		
12	The applicable internal pressure coefficient, Components and Cladding	Yes		
13	The design wind pressure in terms of psf (kN/m ²), to be used for the design of exterior component, cladding materials not specifiably designed by the registered design professional.	Yes		

Elevations Drawing including:

14	All side views of the structure	Yes		
15	Roof pitch	Yes		
16	Overhang dimensions and detail with attic ventilation	Yes		
17	Location, size and height above roof of chimneys	Yes		
18	Location and size of skylights with Florida Product Approval	NA		
19	Number of stories	NA		
20	Building height from the established grade to the roofs highest peak	Yes		

Floor Plan Including:

21	Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies	Yes		
22	Raised floor surfaces located more than 30 inches above the floor or grade	NA		
23	All exterior and interior shear walls indicated	Yes		
24	Shear wall opening shown (Windows, Doors and Garage doors)	Yes		
25	Show compliance with Section FBCR 310 Emergency escape and rescue opening shown in each bedroom (net clear opening shown) and Show compliance with Section FBC 1405.13.2 where the opening of an operable window is located more than 72 inches above the finished grade or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches above the finished floor of the room in which the window is located. Glazing between the floor and 24 inches shall be fixed or have openings through which a 4-inch-diameter sphere cannot pass.	Yes		
26	Safety glazing of glass where needed	Yes		
27	Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 and chapter 24 of FBCR)	NA		
28	Show stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails	NA		
29	Identify accessibility of bathroom (see FBCR SECTION 320)	Yes		

All materials placed within opening or onto/into exterior walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable
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FBCR 403: Foundation Plans

Select From Drop down

30	Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.	Yes		
31	All posts and/or column footing including size and reinforcing	NA		
32	Any special support required by soil analysis such as piling.	NA		
33	Assumed load-bearing value of soil _____ Pound Per Square Foot	NA		
34	Location of horizontal and vertical steel, for foundation or walls (include # size and type) For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an grounding electrode system. Per the National Electrical Code article 250.52.3	Yes		

FBCR 506: CONCRETE SLAB ON GRADE

35	Show Vapor retarder (6mil. Polyethylene with joints taped 6 inches and sealed)	Yes		
36	Show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and Supports	Yes		

FBCR 318: PROTECTION AGAINST TERMITES

37	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or Submit other approved termite protection methods. Protection shall be provided by registered termiticides	Yes		
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FBCR 606: Masonry Walls and Stem walls (load bearing & shear Walls)

38	Show all materials making up walls, wall height, and Block size, mortar type	No		
39	Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement	Yes		

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

40	Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer	NA		
41	Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or piers	NA		
42	Girder type, size and spacing to load bearing walls, stem wall and/or piers	Yes		
43	Attachment of joist to girder	Yes		
44	Wind load requirements where applicable	Yes		
45	Show required under-floor crawl space	NA		
46	Show required amount of ventilation opening for under-floor spaces	NA		
47	Show required covering of ventilation opening	NA		
48	Show the required access opening to access to under-floor spaces	NA		
49	Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing	NA		
50	Show Draftstopping, Fire caulking and Fire blocking	Yes		
51	Show fireproofing requirements for garages attached to living spaces, per FBCR section 302.6	Yes		
52	Provide live and dead load rating of floor framing systems (psf).	NA		

FBCR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
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Select from Drop down

53	Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls	Yes		
54	Fastener schedule for structural members per table FBC-R602.3.2 are to be shown	Yes		
55	Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing	Yes		
56	Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems	Yes		
57	Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FBC-R602.7.	Yes		
58	Indicate where pressure treated wood will be placed	Yes		
59	Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas	Yes		
60	A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail	Yes		

FBCR :ROOF SYSTEMS:

61	Truss design drawing shall meet section FBC-R 802.10. 1 Wood trusses	Yes		
62	Include a layout and truss details, signed and sealed by Florida Professional Engineer	Yes		
63	Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters	Yes		
64	Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	Yes		
65	Provide dead load rating of trusses	Yes		

FBCR 802:Conventional Roof Framing Layout

66	Rafter and ridge beams sizes, span, species and spacing	NA		
67	Connectors to wall assemblies' include assemblies' resistance to uplift rating	NA		
68	Valley framing and support details	NA		
69	Provide dead load rating of rafter system	NA		

FBCR 803 ROOF SHEATHING

70	Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness	Yes		
71	Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas	Yes		

ROOF ASSEMBLIES FRC Chapter 9

72	Include all materials which will make up the roof assemblies covering	Yes		
73	Submit Florida Product Approval numbers for each component of the roof assemblies covering	Yes		

FBCR Chapter 11 Energy Efficiency Code for Residential Building

Residential construction shall comply with this code by using the following compliance methods in the FBCR Chapter 11 Residential buildings compliance methods. **Two of the required forms are to be submitted, N1100.1.1.1 As an alternative to the computerized Compliance Method A, the Alternate Residential Point System Method hand calculation, Alternate Form 600A, may be used. All requirements specific to this calculation are located in Sub appendix C to Appendix G. Buildings complying by this alternative shall meet all mandatory requirements of this chapter. Computerized versions of the Alternate Residential Point System Method shall not be acceptable for code compliance.**

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Circled as Applicable		
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Select from Drop Down

74	Show the insulation R value for the following areas of the structure	Yes		
75	Attic space	Yes		
76	Exterior wall cavity	Yes		
77	Crawl space	NA		

HVAC information

78	Submit two copies of a Manual J sizing equipment or equivalent computation study	Yes		
79	Exhaust fans shown in bathrooms Mechanical exhaust capacity of 50 cfm intermittent or 20 cfm continuous required	Yes		
80	Show clothes dryer route and total run of exhaust duct	Yes		

Plumbing Fixture layout shown

81	All fixtures waste water lines shall be shown on the foundation plan	Yes		
82	Show the location of water heater	Yes		

Private Potable Water

83	Pump motor horse power	Yes		
84	Reservoir pressure tank gallon capacity	Yes		
85	Rating of cycle stop valve if used	Yes		

Electrical layout shown including

86	Show Switches, receptacles outlets, lighting fixtures and Ceiling fans	Yes		
87	Show all 120-volt, single phase, 15- and 20-ampere branch circuits outlets required to be protected by Ground-Fault Circuit Interrupter (GFCI) Article 210.8 A	Yes		
88	Show the location of smoke detectors & Carbon monoxide detectors	Yes		
89	Show service panel, sub-panel, location(s) and total ampere ratings	Yes		
90	On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type. For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an Grounding electrode system. Per the National Electrical Code article 250.52.3	Yes		
91	Appliances and HVAC equipment and disconnects	Yes		
92	Show all 120-volt, single phase, 15- and 20-ampere branch circuits supplying outlets installed in dwelling unit family rooms, dining rooms, living rooms, parlors, libraries, dens, bedrooms, sunrooms, recreation rooms, closets, hallways, or similar rooms or areas shall be protected by a listed Combination arc-fault circuit interrupter, Protection device.	Yes		

Notice Of Commencement:

A notice of commencement form **RECORDED** in the Columbia County Clerk Office is required to be filed with the Building Department **BEFORE ANY INSPECTIONS** can be performed.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable
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****ITEMS 95, 96, & 98 Are Required After APPROVAL from the ZONING DEPT.****

Select from Drop down

93	Building Permit Application A current Building Permit Application is to be completed, by following the Checklist all supporting documents must be submitted. There is a \$15.00 application fee. The completed application with attached documents and application fee can be mailed.	Yes		
94	Parcel Number The parcel number (Tax ID number) from the Property Appraisers Office (386) 758-1083 is required. A copy of property deed is also required. www.columbiacountyfla.com	No		
95	Environmental Health Permit or Sewer Tap Approval A copy of a approved Columbia County Environmental Health (386) 758-1058	Yes		
96	City of Lake City A City Water and/or Sewer letter. Call 386-752-2031	NA		
97	Toilet facilities shall be provided for all construction sites	Yes		
98	Town of Fort White (386) 497-2321 If the parcel in the application for building permit is within the Corporate city limits of Fort White, an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit.	NA		
99	Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations (Municode.com)	NA		
100	CERTIFIED FINISHED FLOOR ELEVATIONS will be required on any project where the approved FIRM Flood Maps show the property is in a AE, Floodway, and AH flood zones. Additionally One Foot Rise letters are required for AE and AH zones. In the Floodway Flood zones a Zero Rise letter is required.	NA		
101	A Flood development permit is also required for AE, Floodway & AH. Development permit cost is \$50.00	-		
102	Driveway Connection: If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. County Public Works Dept. determines the size and length of every culvert before instillation and completes a final inspection before permanent power is granted. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00) Separate Check when issued. If the project is to be located on an F.D.O.T. maintained road, then an F.D.O.T. access permit is required.	Yes		
103	911 Address: An application for a 911 address must be applied for and received through the Columbia County Emergency Management Office of 911 Addressing Department (386) 758-1125.	Yes		

Ordinance Sec. 90-75. - Construction debris. (e) It shall be unlawful for any person to dispose of or discard solid waste, including construction or demolition debris at any place within the county other than on an authorized disposal site or at the county's solid waste facilities. The temporary storage, not to exceed seven days of solid waste (excluding construction and demolition debris) on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance, shall not be deemed a violation of this section. The temporary storage of construction and demolition debris on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance shall not be deemed in violation of this section; provided, however, such construction and demolition debris must be disposed of in accordance with this article prior to the county's issuance of a certificate of occupancy for the premises. The burning of lumber from a construction or demolition project or vegetative trash when done so with legal and proper permits from the authorized agencies and in accordance with such agencies' rules and regulations, shall not be deemed a violation of this section. No person shall bury, throw, place, or deposit, or cause to be buried, thrown, placed, or deposited, any solid waste, special waste, or debris of any kind into or on any of the public streets, road right-of-way, highways, bridges, alleys, lanes, thoroughfares, waters, canals, or vacant lots or lands within the county. No person shall bury any vegetative trash on any of the public streets, road right-of-way, highways, bridges, lanes, thoroughfares, waters, canals, or lots less than ten acres in size within the county.

Disclosure Statement for Owner Builders:

If you as the Applicant will be acting as your own contractor or owner/builder under section 489.103(7) Florida Statutes, you must submit the required notarized Owner Builder Disclosure Statement form.

****This form can be printed from the Columbia County Website on the Building and Zoning page under Documents. Web address is - <http://www.columbiacountyfla.com/BuildingandZoning.asp>**

Section 105 of the Florida Building Code defines the:

Time limitation of application.

An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

Single-family residential dwelling.

Section 105.3.4 A building permit for a single-family residential dwelling must be issued within 30 working days of application therefor unless unusual circumstances require a longer time for processing the application or unless the permit application fails to satisfy the Florida Building Code or the enforcing agency's laws or ordinances.

Permit intent.

Section 105.4.1: A permit issued shall be constructed to be a license to proceed with the work and not as authority to violate, cancel, alter or set aside any of the provisions of the technical codes, nor shall issuance of a permit prevent the building official from thereafter requiring a correction of errors in plans, construction or violations of this code. Every permit issued shall become invalid unless the work authorized by such permit is commenced within six months after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of six months after the time the work is commenced.

If work has commenced.

Section 105.4.1.1: If work has commenced and the permit is revoked, becomes null and void, or expires because of lack of progress or abandonment, a new permit covering the proposed construction shall be obtained before proceeding with the work.

New Permit.

Section 105.4.1.2: If a new permit is not obtained within 180 days from the date the initial permit became null and void, the building official is authorized to require that any work which has been commenced or completed be removed from the building site. Alternately, a new permit may be issued on application, providing the work in place and required to complete the structure meets all applicable regulations in effect at the time the initial permit became null and void and any regulations which may have become effective between the date of expiration and the date of issuance of the new permit.

Work Shall Be:

Section 105.4.1.3: Work shall be considered to be in active progress when the permit has received an approved inspection within 180 days. This provision shall not be applicable in case of civil commotion or strike or when the building work is halted due directly to judicial injunction, order or similar process.

The Fee:

Section 105.4.1.4: The fee for renewal reissuance and extension of a permit shall be set forth by the administrative authority.

Notification:

When the application is approved for permitting the applicant will be notified by phone as to the status by the Columbia County Building & Zoning Department.

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING	MASONITE INT.	SWINGING EXTERIOR DOOR ASSEMBLY	FL 29847
B. SLIDING			
C. SECTIONAL/ROLL UP			
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	ALL AMERICAN WINDOW COMP.	SINGLE HUNG WINDOW	FL 23890
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING	JAMES HARDIE	HARDI PLANK Lap siding	FL 13192
B. SOFFITS			
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES	GAF	ARCHIT SHINGLES	ASTM D3462
B. NON-STRUCTURAL METAL			
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCTURAL COMPONENTS			
A. WOOD CONNECTORS	SIMPSON TIES	WOOD CONNECTORS	FL 15363
B. WOOD ANCHORS			
C. TRUSS PLATES	MAYO TRUSS	WOOD TRUSSES	ASTM A 653
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

Contractor OR Agent Signature

Date

11-6-19

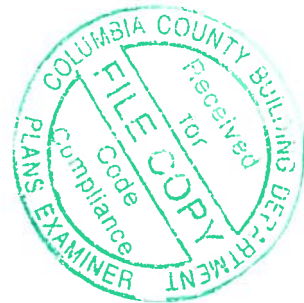
NOTES: _____

VICTOR RAYMOS ARCHITECT INC.

9/5/2019

**WIND LOAD ANALYSIS
Per
2017 Florida Building Codes, Sixth Edition
ASCE 7-10**

*Aragon Wind Load
316 SW Newport Lane
Ft. White, FL 32038*



Victor Ramos
9-6-19

SHEAR FORCES PERPENDICULAR AND PARALLEL TO RIDGE ARE RESISTED WITH THE REQUIRED AMOUNT OF SHEARWALL.

ROOF FRAMING : Pre-engineered wood trusses @ 24" o.c. .

ROOF SHEATHING: 7/16" or 15/32" Exterior APA structural plywood or OSB sheathing.
Fasten with 8d/ .113 ring shank nails.

Interior zone spacing: Interior 6 inches-- Periphery 3 inches

Edge and end zone spacing: Interior 6 inches—Periphery 3 inches

SHEARWALL SIDING: (at exterior studd walls and at Exterior face gable end) 7/16" or 15/32" Exterior APA structural plywood or OSB sheathing.

Spacing: Interior 8 inches-- Edge 3 inches

Fasten with 8d/.131 ring shank nails. **All exterior walls are shearwalls.**

ANCHOR BOLTS: 2X4 PT bottom plate attached to concrete w/ 1/2" bolts @ 32" o.c. and at corners w/ min. 7" imbedment . Provide 2" x 2" x 1/8" washers. Where wall length is less than 32" provide bolt 5" from each end. If bolt is missing after concrete is poured then the 1/2" x 6" Simpson Titen HD bolt can be used.

STUDS: 2X4 #2 Souther Yellow Pine . Exterior spacing 16" o.c..

LOAD TRANSFER FASTENERS (Simpson or equal)

Provide Simpson H2.5A truss anchors at each truss bearing point.

WOOD COLUMNS AND BEAMS (Simpson or equal)

At 4x4 posts to (2) 2x10 and 2x12 porch roof beams use LCE4 corner post caps (add 1/2" plywd blocking to match 4x4 width. Dbl 2x's to be nailed together at 16" o.c. staggerd w/ 10d nails) .
At all 4X4 columns use ABA44Z column plinths. At (2) 2x porch roof beams to stud wall connections, pack 2 studs under beam bearing and attached beam to studs with 3 Simpson SDW Strong Drive Screws 6" long on each side of beams.**Anchor beam bearing studs to king studs snug on each side w/ (5) 10d nails.**

1. All trusses must bear on exterior walls and porch beams.
2. All walls to be nailed with the same patterns as shearwalls.
4. This wind load is not valid without a verified signature.
5. It is assumed that soil conditions will support a minimum 2000 psf.
6. Fiber mesh or WWM may be used in concrete slab.
7. Trusses must be anchored and supported in accordance with truss engineering.
8. Wind design and analysis is valid for one use only.
9. All headers over 12' span must be pre-engineered.
10. Any discrepancies in this wind load shall be brought to the attention of the architect before proceeding with construction.

FOUNDATIONS

Monolithic thickened edge slab footing 1'-8" deep x 1'-4" wide with 2 # 5 rebars continuous. Slab to be 4" thick. All concrete to be 3000 psi with either 6x6 #10 Ga wwm or fibermesh.

VICTOR RAYMOS ARCHITECT INC.

Limits of Liability

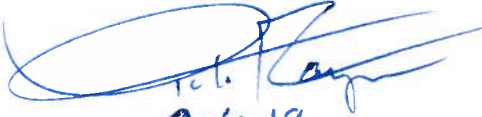
WARRANTY AND STANDARD OF CARE – Services provided by the Consultant (Architect) under this Professional Services Agreement will be performed in a manner consistent with that degree of care and skill ordinarily exercised by members of the same profession currently practicing under similar circumstances. Upon written notice to the Consultant prior to three months after completion of Services, and by mutual agreement between the Consultant and Client, the Consultant will correct those services not meeting such a standard without additional compensation. This Warranty and Standard of Care is in lieu of all other warranties and standards of care. No other warranty or standard of care, express or implied, including warranties of merchantability and fitness for a particular purpose is made or intended by the proposal, by oral communication or by any representations made regarding the services included in this agreement.

LIMITATION OF LIABILITY – Consultant and Client mutually agree that the services provided pursuant to this Agreement involve risks of liability, which cannot be adequately compensated for by the payments Client will make under this Agreement. Therefore, the total cumulative liability of Consultant, its agents, employees and subcontractors whether in contract, tort including negligence (whether sole or concurrent) and strict liability, or otherwise arising out of, connected with or resulting from the services providing pursuant to this Agreement shall not exceed the total fees paid by Client. Client is advised to carefully review Client's risks of liability related to this contract and address such risks through Client's insurance or other means.

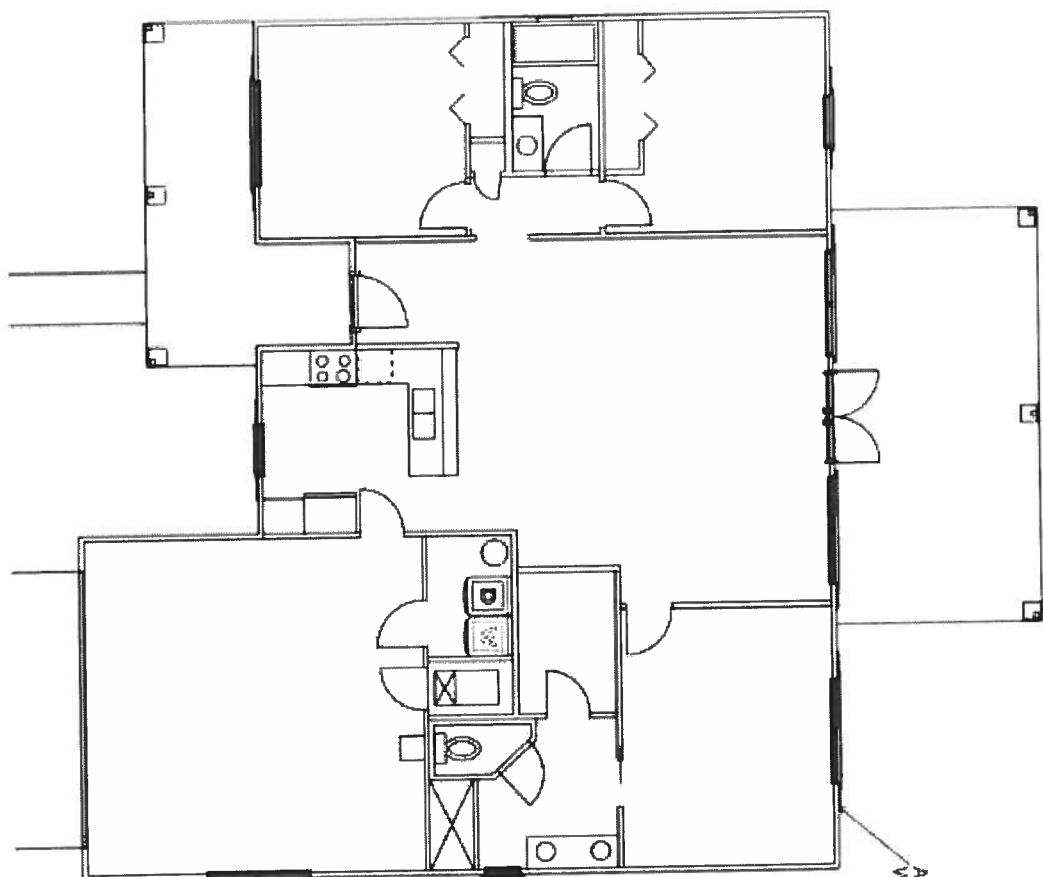
DISCLAIMER OF CONSEQUENTIAL DAMAGES – In no event shall Consultant or Client be liable to the other for any special, indirect, incidental or consequential loss or damages, including lost profits and loss of use.

Acceptance of this WIND LOAD document for a project use, confirms that the client is aware of the Limits of Liability listed herein and is in agreement with them.

Victor Raymos Architect



9-6-19
3



ALL EXTERIOR 2X4 STUD
WALLS ARE SEARWALLS

MecaWind v2329

Software Developer: Meca Enterprises Inc., www.meca.biz, Copyright 2015

Calculations Prepared by:

Date: Sep 05, 2019

Designer: Victor Raymos Architect

Calculations Prepared For:

Client: Marvin Aragon

Project #: 583 Aragon

Location: 316 SW Newport Ln, Ft White Fl 32038

Description:

Home

FileLocation : C:\Users\vraymos\ACTIVE DESK 1A\583 Aragon\WORKING\Wind Load\Mecca\583.wnd

Basic Wind Parameters

Wind Load Standard	= ASCE 7-10	Exposure Category	= C
Wind Design Speed	= 130.0 mph	Risk Category	= II
Structure Type	= Building	Building Type	= Enclosed

General Wind Settings

Incl_LF	= Include ASD Load Factor of 0.6 in Pressures	= True
DynType	= Dynamic Type of Structure	= Rigid
NF	= Natural Frequency of Structure (Mode 1)	= 1.000 Hz
Alt	= Altitude (Ground Elevation) above Sea Level	= 0.000 ft
Bdist	= Base Elevation of Structure	= 0.000 ft
SDB	= Simple Diaphragm Building	= False
MWFRSType	= MWFRS Method Selected	= Ch 27 Pt 1

Topographic Factor per Fig 26.8-1

Topo	= Topographic Feature	= None
Kzt	= Topographic Factor	= 1.000

Building Inputs

RoofType: Building Roof Type	= Hipped	W	: Width Perp to Ridge = 50.000 ft
L	: Length Along Ridge = 50.000 ft	Eht	: Eave Height = 9.000 ft
Hip	: Ridge Hipped Length = 20.000 ft	RE	: Roof Entry Method = Slope
Slope	: Slope of Roof = 6.0 :12	Theta	: Roof Slope = 26.57 Deg
Par	: Is there a Parapet = False		

Exposure Constants per Table 26.9-1:

Alpha: Const from Table 26.9-1=	9.500	Zg: Const from Table 26.9-1=	900.000 ft
At: Const from Table 26.9-1=	0.105	Bt: Const from Table 26.9-1=	1.000
Am: Const from Table 26.9-1=	0.154	Bm: Const from Table 26.9-1=	0.650
C: Const from Table 26.9-1=	0.200	Eps: Const from Table 26.9-1=	0.200

Overhang Inputs:

Std	= Overhangs on all sides are the same	= True
OHType	= Type of Roof Wall Intersections	= Soffit
OH	= Overhang of Roof Beyond Wall	= 2.000 ft

Main Wind Force Resisting System (MWFRS) Calculations per Ch 27 Part 1:

Eht	= Eave Height	= 9.000 ft
Rht	= Ridge Height	= 21.500 ft
h	= Mean Roof Height: $0.5 \cdot (Eht + Rht)$	= 15.250 ft
Zh	= Mean Roof Height for Kh: $h + \text{Base_Dist}$	= 15.250 ft
Kh	= Since 15 ft [4.572 m] < Zh < Zg --> $2.01 \cdot (Zh/zg)^{(2/\text{Alpha})}$	= 0.852
Kzt	= Topographic Factor is 1 since no Topographic feature specified	= 1.000
Kd	= Wind Directionality Factor per Table 26.6-1	= 0.85
GCPi	= Ref Table 26.11-1 for Enclosed Building	= +/-0.18
RA	= Roof Area	= 3336.43 sq ft
LF	= Load Factor based upon ASD Design	= 0.60
qh	= $(0.00256 \cdot Kh \cdot Kzt \cdot Kd \cdot V^2) \cdot LF$	= 18.80 psf
qin	= For Negative Internal Pressure of Enclosed Building use $qh \cdot LF$	= 18.80 psf
qip	= For Positive Internal Pressure of Enclosed Building use $qh \cdot LF$	= 18.80 psf

Gust Factor Calculation:

Gust Factor Category I Rigid Structures - Simplified Method		
G1	= For Rigid Structures (Nat. Freq. > 1 Hz) use 0.85	= 0.85
Gust Factor Category II Rigid Structures - Complete Analysis		
Zm	= $0.6 \cdot Ht$	= 15.000 ft
Izm	= $Cc \cdot (33 / Zm)^{0.167}$	= 0.228
Lzm	= $L \cdot (Zm / 33)^{\text{Epsilon}}$	= 427.057
Q	= $\{1 / (1 + 0.63 \cdot ((B + Ht) / Lzm)^{0.63})\}^{0.5}$	= 0.916

G2 = $0.925 * ((1 + 1.7 * 1zm * 3.4 * Q) / (1 + 1.7 * 3.4 * 1zm))$ = 0.881
 Gust Factor Used in Analysis
 G = Lessor Of G1 Or G2 = 0.850

MWFRS Wind Normal to Ridge (Ref Fig 27.4-1)

h = Mean Roof Height Of Building = 15.250 ft
 RHt = Ridge Height Of Roof = 21.500 ft
 B = Horizontal Dimension Of Building Normal To Wind Direction = 50.000 ft
 L = Horizontal Dimension Of building Parallel To Wind Direction = 50.000 ft
 L/B = Ratio Of L/B used For Cp determination = 1.000
 h/L = Ratio Of h/L used For Cp determination = 0.305
 Slope = Slope of Roof = 26.57 Deg
 OH_Top_+Y = Overhang Top +Y (Leeward) = -0.6, -0.6
 OH_Top_-Y = Overhang Top Windward Edge = 0.28, -0.22
 OH_X_1 = Overhang Top +/-X Coeff (0 to h) (0.000 ft to 15.250 ft) = -0.18, -0.9
 OH_X_2 = Overhang Top +/-X Coeff (0 to h) (0.000 ft to 15.250 ft) = -0.18, -0.9
 OH_X_3 = Overhang Top +/-X Coeff (h to 2h) (15.250 ft to 30.500 ft) = -0.18, -0.5
 OH_X_4 = Overhang Top +/-X Coeff (h to 2h) (15.250 ft to 30.500 ft) = -0.18, -0.5
 OH_X_5 = Overhang Top +/-X Coeff (>2h) (>30.500 ft) = -0.18, -0.3
 OH_X_6 = Overhang Top +/-X Coeff (>2h) (>30.500 ft) = -0.18, -0.3
 Roof_LW = Roof (Leeward) = -0.6, -0.6
 Roof_WW = Roof (Windward) = 0.28, -0.22
 Roof_X_1 = Roof +/-X Coeff (0 to h) (0.000 ft to 15.250 ft) = -0.18, -0.9
 Roof_X_2 = Roof +/-X Coeff (0 to h) (0.000 ft to 15.250 ft) = -0.18, -0.9
 Roof_X_3 = Roof +/-X Coeff (h to 2h) (15.250 ft to 30.500 ft) = -0.18, -0.5
 Roof_X_4 = Roof +/-X Coeff (h to 2h) (15.250 ft to 30.500 ft) = -0.18, -0.5
 Roof_X_5 = Roof +/-X Coeff (>2h) (>30.500 ft) = -0.18, -0.3
 Roof_X_6 = Roof +/-X Coeff (>2h) (>30.500 ft) = -0.18, -0.3
 Soffit_-Y = Overhang Soffit -Y = 0.8, 0.8

 Cp_WW = Windward Wall Coefficient (All L/B Values) = 0.80
 Cp_LW = Leeward Wall Coefficient Using L/B = -0.50
 Cp_SW = Side Wall Coefficient (All L/B values) = -0.70
 GCpn_WW = Parapet Combined Net Pressure Coefficient (Windward Parapet) = 1.50
 GCpn_LW = Parapet Combined Net Pressure Coefficient (Leeward Parapet) = -1.00

Wall Wind Pressures based On Positive Internal Pressure (+GCPi) ~ Normal to Ridge
 All wind pressures include a load factor of 0.6

Elev	Kz	Kzt	qz	GCPi	Windward Press	Leeward Press	Side Press	Total Press	Minimum Pressure*
ft			psf	psf	psf	psf	psf	psf	psf
9.00	0.849	1.000	18.73	0.18	9.35	-11.37	-14.57	20.72	9.60

Wall Wind Pressures based on Negative Internal Pressure (-GCPi) ~ Normal to Ridge
 All wind pressures include a load factor of 0.6

Elev	Kz	Kzt	qz	GCPi	Windward Press	Leeward Press	Side Press	Total Press	Minimum Pressure*
ft			psf	psf	psf	psf	psf	psf	psf
9.00	0.849	1.000	18.73	-0.18	16.12	-4.60	-7.80	20.72	9.60

Notes Wall Pressures:

Kz = Velocity Press Exp Coeff Kzt = Topographical Factor
 $qz = 0.00256 * Kz * Kzt * Kd * V^2$ GCPi = Internal Press Coefficient
 Side = $qh * G * Cp_{SW} - qip * +GCPi$ Windward = $qz * G * Cp_{WW} - qip * +GCPi$
 Leeward = $qh * G * Cp_{LW} - qip * +GCPi$ Total = Windward Press - Leeward Press
 * Minimum Pressure: Para 27.4.7 no less than 9.60 psf (Incl LF) applied to Walls
 + Pressures Acting TOWARD Surface - Pressures Acting AWAY from Surface

Roof Wind Pressures for Positive & Negative Internal Pressure (+/- GCPi) ~ Normal to Ridge
 All wind pressures include a load factor of 0.6

Roof Var	Start Dist	End Dist	Cp_min	Cp_max	GCPi	Pressure Pn_min*	Pressure Pp_min*	Pressure Pn_max	Pressure Pp_max
	ft	ft				psf	psf	psf	psf
OH_Top_+Y	N/A	N/A	-0.600	-0.600	0.180	-6.20	-12.97	-6.20	-12.97
OH_Top_-Y	N/A	N/A	0.280	-0.220	0.180	7.86	1.09	-0.13	-6.90

OH_X_1 (+X)	0.000	15.250	-0.180	-0.900	0.180	0.51	-6.26	-11.00	-17.76
OH_X_2 (-X)	0.000	15.250	-0.180	-0.900	0.180	0.51	-6.26	-11.00	-17.76
OH_X_3 (+X)	15.250	30.500	-0.180	-0.500	0.180	0.51	-6.26	-4.60	-11.37
OH_X_4 (-X)	15.250	30.500	-0.180	-0.500	0.180	0.51	-6.26	-4.60	-11.37
OH_X_5 (+X)	30.500	54.000	-0.180	-0.300	0.180	0.51	-6.26	-1.41	-8.18
OH_X_6 (-X)	30.500	54.000	-0.180	-0.300	0.180	0.51	-6.26	-1.41	-8.18
Roof_LW	N/A	N/A	-0.600	-0.600	0.180	-6.20	-12.97	-6.20	-12.97
Roof_WW	N/A	N/A	0.280	-0.220	0.180	7.86	1.09	-0.13	-6.90
Roof_X_1 (+X)	0.000	15.250	-0.180	-0.900	0.180	0.51	-6.26	-11.00	-17.76
Roof_X_2 (-X)	0.000	15.250	-0.180	-0.900	0.180	0.51	-6.26	-11.00	-17.76
Roof_X_3 (+X)	15.250	30.500	-0.180	-0.500	0.180	0.51	-6.26	-4.60	-11.37
Roof_X_4 (-X)	15.250	30.500	-0.180	-0.500	0.180	0.51	-6.26	-4.60	-11.37
Roof_X_5 (+X)	30.500	52.000	-0.180	-0.300	0.180	0.51	-6.26	-1.41	-8.18
Roof_X_6 (-X)	30.500	52.000	-0.180	-0.300	0.180	0.51	-6.26	-1.41	-8.18
Sofit_Y	N/A	N/A	0.800	0.800	0.180	16.16	9.40	16.16	9.40

Notes Roof Pressures:

Start Dist = Start Dist from Windward Edge End Dist = End Dist from Windward Edge

Cp_Max = Largest Coefficient Magnitude Cp_Min = Smallest Coefficient Magnitude

Pp_max = $q_h * G * C_{p_max} - q_{ip} * (+GCPi)$ Pn_max = $q_h * G * C_{p_max} - q_{in} * (-GCPi)$ Pp_min* = $q_h * G * C_{p_min} - q_{ip} * (+GCPi)$ Pn_min* = $q_h * G * C_{p_min} - q_{in} * (-GCPi)$

OH = Overhang X = Dir along Ridge Y = Dir Perpendicular to Ridge Z = Vertical

* The smaller uplift pressures due to Cp_Min can become critical when wind is combined with roof live load or snow load; load combinations are given in ASCE 7

+ Pressures Acting TOWARD Surface - Pressures Acting AWAY from Surface

MWFRS Wind Parallel to Ridge (Ref Fig 27.4-1)

h	= Mean Roof Height Of Building	= 15.250 ft
RHt	= Ridge Height Of Roof	= 21.500 ft
B	= Horizontal Dimension Of Building Normal To Wind Direction	= 50.000 ft
L	= Horizontal Dimension Of building Parallel To Wind Direction	= 50.000 ft
L/B	= Ratio Of L/B used For Cp determination	= 1.000
h/L	= Ratio Of h/L used For Cp determination	= 0.305
Slope	= Slope of Roof	= 26.57 Deg
Hip_End_1	= Hipped End Coeff (0 to h) (0.000 ft to 15.250 ft)	= -0.18, -0.9
Hip_End_2	= Hipped End Coeff (h to 2h) (15.250 ft to 30.500 ft)	= -0.18, -0.5
Hip_End_3	= Hipped End Coeff (>2h) (>30.500 ft)	= -0.18, -0.3
OH_Bot	= Soffit (Windward Face Only)	= 0.8, 0.8
OH_Top_1	= Overhang Top Coeff (0 to h/2) (0.000 ft to 7.625 ft)	= -0.18, -0.9
OH_Top_2	= Overhang Top Coeff (0 to h) (0.000 ft to 15.250 ft)	= -0.18, -0.9
OH_Top_3	= Overhang Top Coeff (0 to h) (0.000 ft to 15.250 ft)	= -0.18, -0.9
OH_Top_4	= Overhang Top Coeff (h to 2h) (15.250 ft to 30.500 ft)	= -0.18, -0.5
OH_Top_5	= Overhang Top Coeff (h to 2h) (15.250 ft to 30.500 ft)	= -0.18, -0.5
OH_Top_6	= Overhang Top Coeff (>2h) (>30.500 ft)	= -0.18, -0.3
OH_Top_7	= Overhang Top Coeff (>2h) (>30.500 ft)	= -0.18, -0.3
OH_Top_8	= Overhang Top Coeff (>2h) (>30.500 ft)	= -0.18, -0.3
Roof_1	= Roof Coeff (0 to h) (0.000 ft to 15.250 ft)	= -0.18, -0.9
Roof_2	= Roof Coeff (0 to h) (0.000 ft to 15.250 ft)	= -0.18, -0.9
Roof_3	= Roof Coeff (h to 2h) (15.250 ft to 30.500 ft)	= -0.18, -0.5
Roof_4	= Roof Coeff (h to 2h) (15.250 ft to 30.500 ft)	= -0.18, -0.5
Roof_5	= Roof Coeff (>2h) (>30.500 ft)	= -0.18, -0.3
Roof_6	= Roof Coeff (>2h) (>30.500 ft)	= -0.18, -0.3
Cp_WW	= Windward Wall Coefficient (All L/B Values)	= 0.80
Cp_LW	= Leeward Wall Coefficient Using L/B	= -0.50
Cp_SW	= Side Wall Coefficient (All L/B values)	= -0.70
GCpn_WW	= Parapet Combined Net Pressure Coefficient (Windward Parapet)	= 1.50
GCpn_LW	= Parapet Combined Net Pressure Coefficient (Leeward Parapet)	= -1.00

Wall Wind Pressures based On Positive Internal Pressure (+GCPi) - Parallel to Ridge

All wind pressures include a load factor of 0.6

Elev	Kz	Kzt	qz	GCPi	Windward Press	Leeward Press	Side Press	Total Press	Minimum Pressure*
ft			psf	psf	psf	psf	psf	psf	psf
9.00	0.849	1.000	18.73	0.18	9.35	-11.37	-14.57	20.72	9.60

Wall Wind Pressures based on Negative Internal Pressure (-GCPi) - Parallel to Ridge

All wind pressures include a load factor of 0.6

Elev	Kz	Kzt	qz	GCPI	Windward	Leeward	Side	Total	Minimum
ft			psf	psf	Press	Press	Press	Press	Pressure*
-----	-----	-----	-----	-----	psf	psf	psf	psf	psf
9.00	0.849	1.000	18.73	-0.18	16.12	-4.60	-7.80	20.72	9.60

Notes Wall Pressures:

Kz = Velocity Press Exp Coeff Kzt = Topographical Factor
 qz = $0.00256 * Kz * Kzt * Kd * V^2$ GCPI = Internal Press Coefficient
 Side = $q_h * G * C_{p_SW} - q_{ip} * +GCPI$ Windward = $q_z * G * C_{p_WW} - q_{ip} * +GCPI$
 Leeward = $q_h * G * C_{p_LW} - q_{ip} * +GCPI$ Total = Windward Press - Leeward Press
 * Minimum Pressure: Para 27.4.7 no less than 9.60 psf (Incl LF) applied to Walls
 + Pressures Acting TOWARD Surface - Pressures Acting AWAY from Surface

Roof Wind Pressures for Positive & Negative Internal Pressure (+/- GCPI) - Parallel to Ridge
All wind pressures include a load factor of 0.6

Roof Var	Start	End	Cp_min	Cp_max	GCPI	Pressure	Pressure	Pressure	Pressure
	Dist	Dist				Pn_min*	Pp_min*	Pn_max	Pp_max
-----	ft	ft	-----	-----	-----	psf	psf	psf	psf
Hip_End_1 (-X)	0.000	15.250	-0.180	-0.900	0.180	0.51	-6.26	-11.00	-17.76
Hip_End_2 (-X)	15.250	30.500	-0.180	-0.500	0.180	0.51	-6.26	-4.60	-11.37
Hip_End_3 (+X)	30.500	52.000	-0.180	-0.300	0.180	0.51	-6.26	-1.41	-8.18
OH_Bot	N/A	N/A	0.800	0.800	0.180	16.16	9.40	16.16	9.40
OH_Top_1 (-X)	0.000	7.625	-0.180	-0.900	0.180	0.51	-6.26	-11.00	-17.76
OH_Top_2 (-Y)	0.000	15.250	-0.180	-0.900	0.180	0.51	-6.26	-11.00	-17.76
OH_Top_3 (+Y)	0.000	15.250	-0.180	-0.900	0.180	0.51	-6.26	-11.00	-17.76
OH_Top_4 (-Y)	15.250	30.500	-0.180	-0.500	0.180	0.51	-6.26	-4.60	-11.37
OH_Top_5 (+Y)	15.250	30.500	-0.180	-0.500	0.180	0.51	-6.26	-4.60	-11.37
OH_Top_6 (+X)	30.500	54.000	-0.180	-0.300	0.180	0.51	-6.26	-1.41	-8.18
OH_Top_7 (-Y)	30.500	54.000	-0.180	-0.300	0.180	0.51	-6.26	-1.41	-8.18
OH_Top_8 (+Y)	30.500	54.000	-0.180	-0.300	0.180	0.51	-6.26	-1.41	-8.18
Roof_1 (+Y)	0.000	15.250	-0.180	-0.900	0.180	0.51	-6.26	-11.00	-17.76
Roof_2 (-Y)	0.000	15.250	-0.180	-0.900	0.180	0.51	-6.26	-11.00	-17.76
Roof_3 (+Y)	15.250	30.500	-0.180	-0.500	0.180	0.51	-6.26	-4.60	-11.37
Roof_4 (-Y)	15.250	30.500	-0.180	-0.500	0.180	0.51	-6.26	-4.60	-11.37
Roof_5 (+Y)	30.500	52.000	-0.180	-0.300	0.180	0.51	-6.26	-1.41	-8.18
Roof_6 (-Y)	30.500	52.000	-0.180	-0.300	0.180	0.51	-6.26	-1.41	-8.18

Notes Roof Pressures:

Start Dist = Start Dist from Windward Edge End Dist = End Dist from Windward Edge
 Cp_Max = Largest Coefficient Magnitude Cp_Min = Smallest Coefficient Magnitude
 Pp_max = $q_h * G * C_{p_max} - q_{ip} * (+GCPI)$ Pn_max = $q_h * G * C_{p_max} - q_{in} * (-GCPI)$
 Pp_min* = $q_h * G * C_{p_min} - q_{ip} * (+GCPI)$ Pn_min* = $q_h * G * C_{p_min} - q_{in} * (-GCPI)$
 OH = Overhang X = Dir along Ridge Y = Dir Perpendicular to Ridge Z = Vertical
 * The smaller uplift pressures due to Cp_Min can become critical when wind is combined
 with roof live load or snow load; load combinations are given in ASCE 7
 + Pressures Acting TOWARD Surface - Pressures Acting AWAY from Surface

		Maximum Header Span (ft.)					
		3'	6'	9'	12'	15'	18'
		Number of Header Studs Supporting End of Header					
		1	1	2	2	2	2
Unsupported Wall Height	Stud Spacing	Number of Full-Length Studs at Each End of Header					
10' or less	12 in.	2	2	3	3	3	3
	16 in.	2	2	3	3	3	3
	24 in.	1	2	2	2	2	2
greater than 10'	12 in.	2	2	3	4	5	5
	16 in.	2	2	3	3	4	4
	24 in.	1	2	2	2	3	3

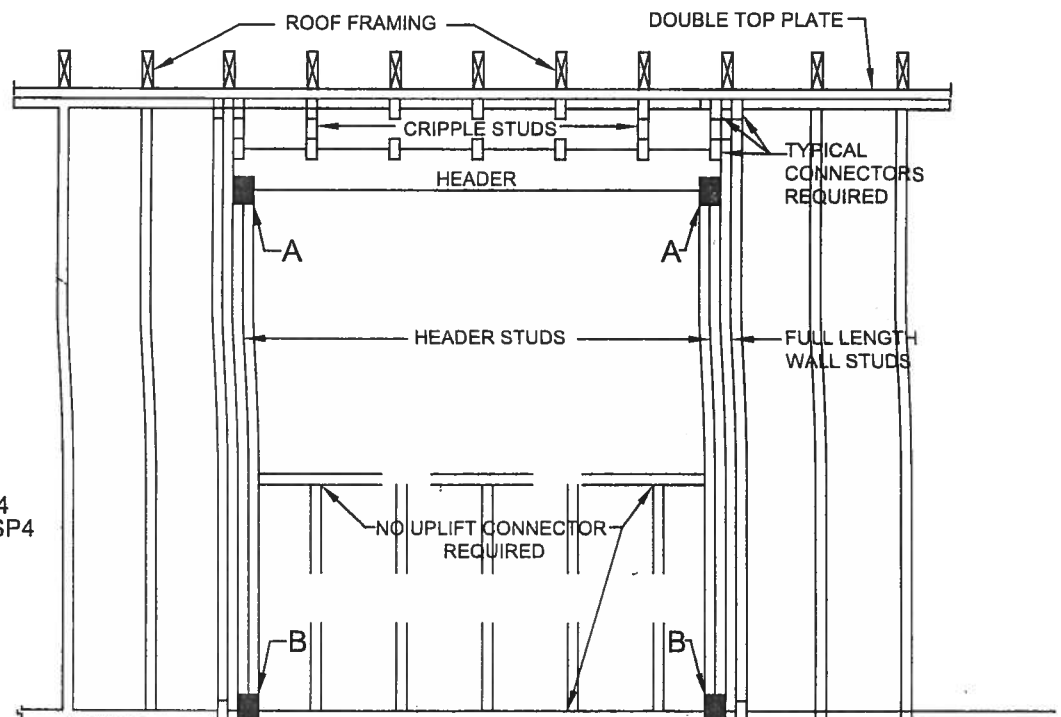
1. The header stud shall not be required if the header is supported by a suitable framing anchor.

Uplift connection
Requirement at points A & B

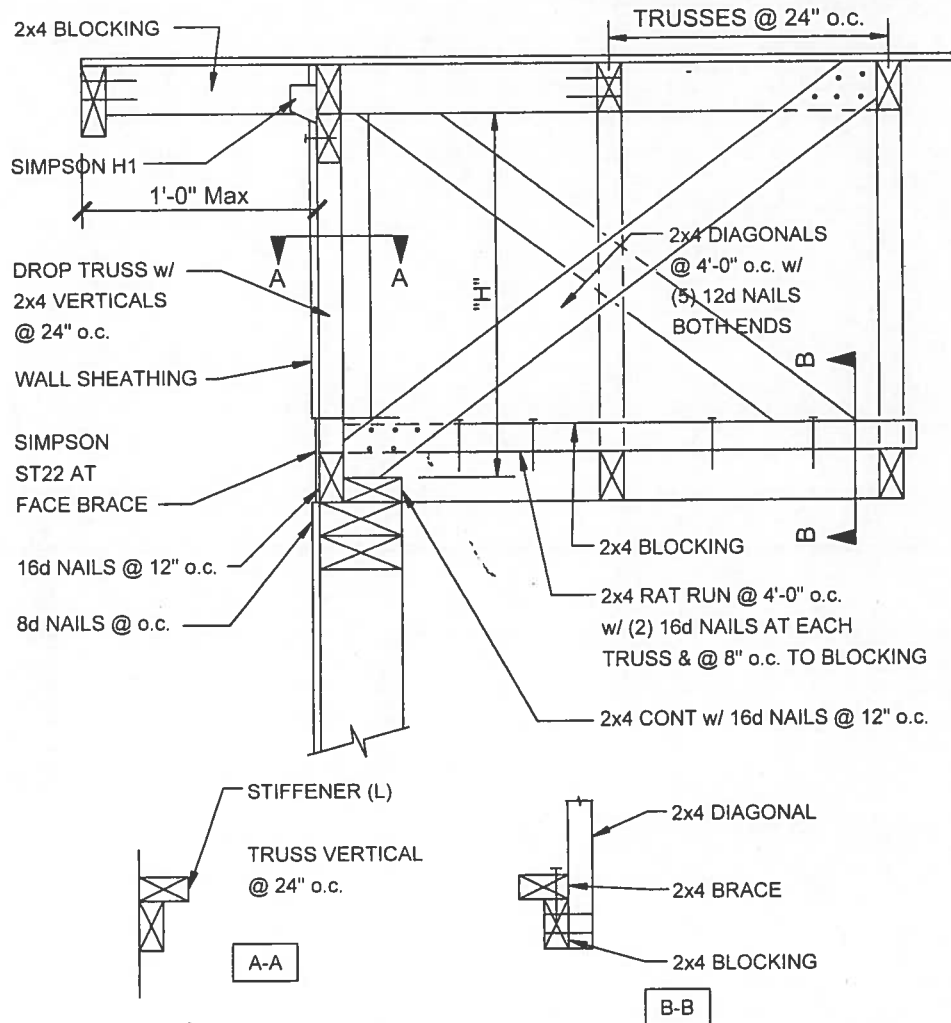
NOTE. Uplift connection IS required at each end of header and at bottom of header studs in addition to connectors at wall studs and at top and bottom of cripples.

All Simpson or equal.

At A : simpson MSTA21
At B : simpson RSP4 per stud
At Top of cripples : Simpson RSP4
At bottom of cripples: Simpson RSP4



DIAGONALS TO BE AT APPROX 45
DEGREE ANGLE. "H" = 48" DIAGONALS
SPAN 2 TRUSS SPACES, "H" = 8'-0"
DIAGONALS SPAN 4 TRUSS SPACES



TYPICAL GABLE END BRACING



Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

RE: Aragon - Aragon

MiTek USA, Inc.

6904 Parke East Blvd.
Tampa, FL 33610-4115

Site Information:

Customer Info: KG Construction Project Name: . Model: .
Lot/Block: . Subdivision: .
Address: ., .
City: Ft. White State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: License #:
Address:
City: State:

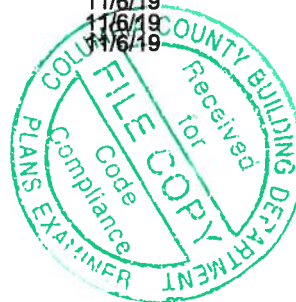
General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2017/TPI2014 Design Program: MiTek 20/20 8.2
Wind Code: ASCE 7-10 Wind Speed: 130 mph
Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 30 individual, Truss Design Drawings and 0 Additional Drawings.

With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	T18581562	A1GIR	11/6/19	23	T18581584	D2	11/6/19
2	T18581563	A2	11/6/19	24	T18581585	D3	11/6/19
3	T18581564	A3	11/6/19	25	T18581586	D4	11/6/19
4	T18581565	A4	11/6/19	26	T18581587	D5GIR	11/6/19
5	T18581566	A5	11/6/19	27	T18581588	J1	11/6/19
6	T18581567	A6	11/6/19	28	T18581589	J2	11/6/19
7	T18581568	A7	11/6/19	29	T18581590	J3	11/6/19
8	T18581569	A8	11/6/19	30	T18581591	J4	11/6/19
9	T18581570	A9	11/6/19				
10	T18581571	A10	11/6/19				
11	T18581572	A11	11/6/19				
12	T18581573	A12	11/6/19				
13	T18581574	A13GIR	11/6/19				
14	T18581575	B1GE	11/6/19				
15	T18581576	B2	11/6/19				
16	T18581577	B3	11/6/19				
17	T18581578	C1GE	11/6/19				
18	T18581579	C2	11/6/19				
19	T18581580	C3	11/6/19				
20	T18581581	C4GIR	11/6/19				
21	T18581582	CJ01	11/6/19				
22	T18581583	D1GE	11/6/19				

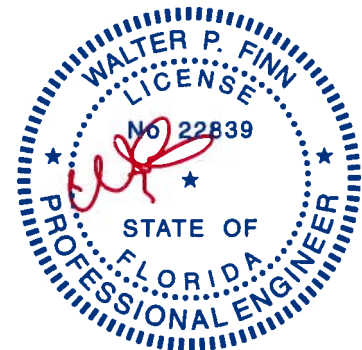


The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Mayo Truss Company, Inc..

Truss Design Engineer's Name: Finn, Walter

My license renewal date for the state of Florida is February 28, 2021.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Walter P. Finn PE No. 22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

November 6, 2019

Finn, Walter

1 of 1

Job Aragon	Truss A1GIR	Truss Type Half Hip Girder	Qty 1	Ply 2	Aragon	T18581562
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Mayo Truss Company, Inc., Mayo, FL - 32066,

8.240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:23:58 2019 Page 1

ID:31mchhOHyrVRSWwdKMvL?uyMA?e-9aEVGcZoB?AHM8kvQ9uSe_XR3gqYoJhubd0WTrYLzgf

-2-0-0	7-0-0	13-8-12	20-3-12	26-10-12	33-7-8
2-0-0	7-0-0	6-8-12	6-7-0	6-7-0	6-8-12

"Special" indicates special hanger(s) or other connection device(s) required at location(s) shown. The design/selection of such special connection device(s) is the responsibility of others. This applies to all applicable truss designs in this job.

Scale = 1:60.0

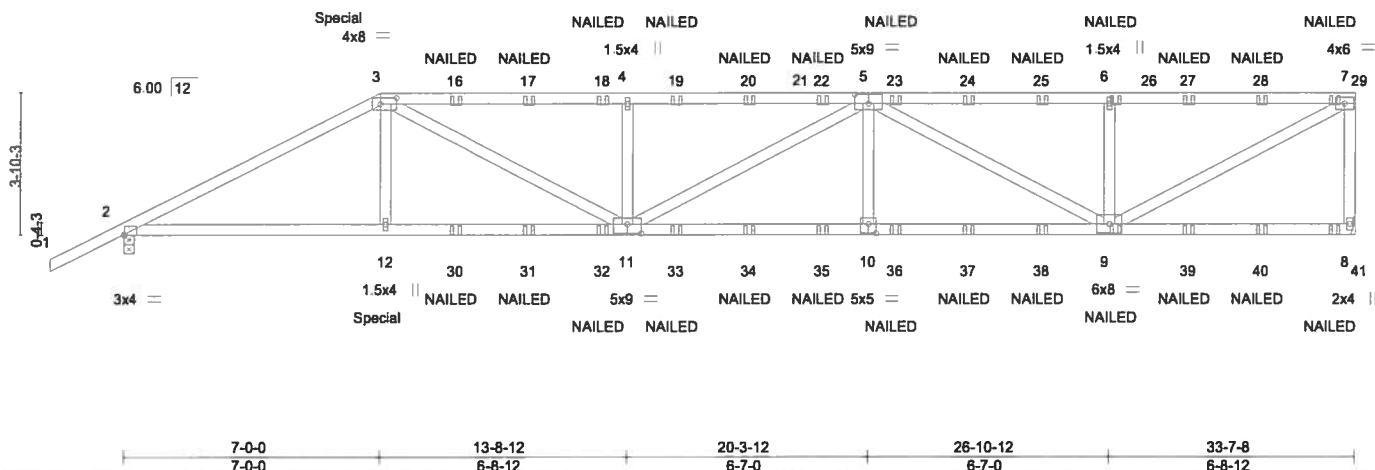


Plate Offsets (X, Y) - [2:0-0-4, Edge], [3:0-5-4, 0-2-0], [5:0-4-8, 0-3-0], [10:0-2-8, 0-3-0], [11:0-4-8, 0-3-0]

LOADING (psf)	SPACING	CSL	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.91	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.25	BC 0.94	Vert(LL) -0.23 10-11 >999 240		
BCLL 0.0 *	Lumber DOL 1.25	WB 0.56	Vert(CT) -0.49 10-11 >814 180		
BCDL 10.0	Rep Stress Incr NO	Matrix-MS	Horz(CT) 0.12 8 n/a n/a		
	Code FBC2017/TPI2014			Weight: 342 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.1 *Except!
1-3: 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(lb/size) 8=2937/Mechanical, 2=2726/0-3-8
Max Horz 2=120(LC 24)
Max Uplift 8=10(LC 8), 2=18(LC 8)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-5257/0, 3-4=-6754/10, 4-5=-6754/10, 5-6=-4453/45, 6-7=-4453/45, 7-8=-2764/93
BOT CHORD 2-12=0/4620, 11-12=0/4642, 10-11=0/6601, 9-10=0/6601
WEBS 3-12=0/691, 3-11=-69/2457, 4-11=-876/193, 5-10=0/522, 5-9=-2441/0, 6-9=-861/198, 7-9=-11/4973

NOTES-

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=34ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 2.
- "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 227 lb down and 134 lb up at 7-0-0 on top chord, and 319 lb down at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

November 6, 2019

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd.
Tampa, FL 36610

Job Aragon	Truss A1GIR	Truss Type Half Hip Girder	Qty 1	Ply 2	Aragon Job Reference (optional)	T18581562
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Mayo Truss Company, Inc., Mayo, FL - 32066,

8.240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:23:58 2019 Page 2
ID:31mchhOHyrVRSWwdKMvL?uyMA7e-9aEVGcZoB7AHM8kvQ9uSe_XR3gqYoJhubd0WTryLzgF

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-60, 3-7=-60, 8-13=-20

Concentrated Loads (lb)

Vert: 3=-180(B) 12=-319(B) 6=-121(B) 9=-59(B) 16=-121(B) 17=-121(B) 18=-121(B) 19=-121(B) 20=-121(B) 22=-121(B) 23=-121(B) 24=-121(B) 25=-121(B)
27=-121(B) 28=-121(B) 29=-139(B) 30=-59(B) 31=-59(B) 32=-59(B) 33=-59(B) 34=-59(B) 35=-59(B) 36=-59(B) 37=-59(B) 38=-59(B) 39=-59(B) 40=-59(B)
41=-65(B)



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6904 Parke East Blvd.
Tampa, FL 38610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581563
Aragon	A2	Half Hip	1	1		

Mayo Truss Company, Inc., Mayo, FL - 32066,

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ID:31mchhOHyrVRSWdKMvL?uyMA?e-dmotUyZQyIl7ziJ5_sPhBC4k44FHxfl.1qHl4?HyLzgE

-2-0-0	4-9-4	9-0-0	15-2-12	21-3-12	27-4-12	33-7-8
2-0-0	4-9-4	4-2-12	6-2-12	6-1-0	6-1-0	6-2-12

Scale = 1:60.0

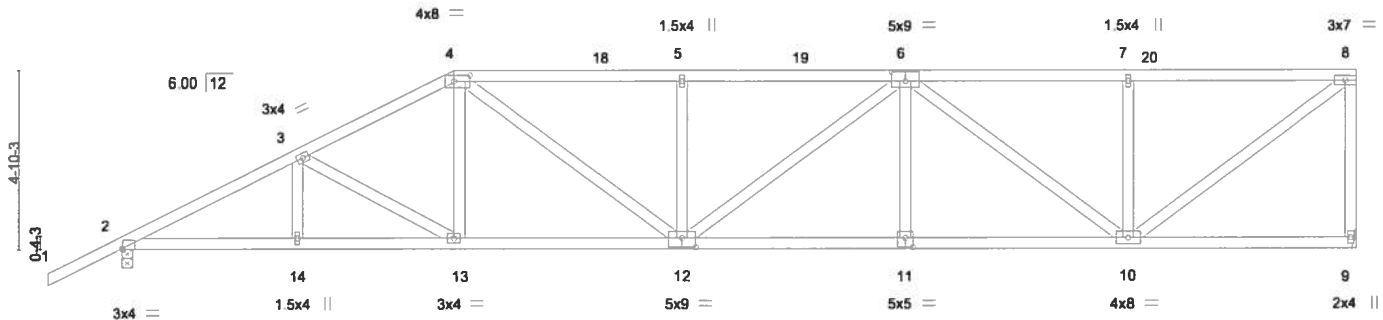


Plate Offsets (X,Y) -		[2:0-0-4,Edge], [4:0-5-4,0-2-0], [6:0-4-8,0-3-0], [11:0-2-8,0-3-0], [12:0-4-8,0-3-0]
LOADING (psf)	SPACING-	2-0-0
TCLL 20.0	Plate Grip DOL	1.25
TCDL 10.0	Lumber DOL	1.25
BCLL 0.0 *	Rep Stress Incr	YES
BCDL 10.0	Code FBC2017/TPI2014	
	CSI.	
	TC 0.44	
	BC 0.59	
	WB 0.98	
	Matrix-AS	
	DEFL.	
	in (loc) l/defl L/d	
	Vert(LL) -0.15 11-12 >999 240	
	Vert(CT) -0.32 11-12 >999 180	
	Horz(CT) 0.10 9 n/a n/a	
	PLATES	GRIP
	MT20	244/190
	Weight: 190 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied.

REACTIONS.

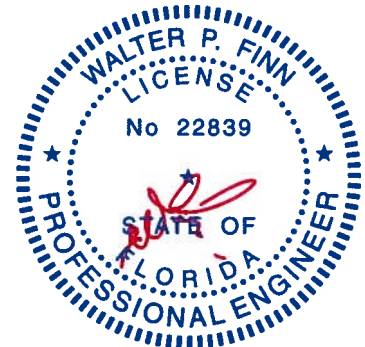
(lb/size) 9=1336/Mechanical, 2=1463/0-3-8
Max Horz 2=150(LC 11)
Max Uplift 2=49(LC 12)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2560/494, 3-4=-2228/485, 4-5=-2481/569, 5-6=-2481/569, 6-7=-1515/370, 7-8=-1515/370, 8-9=-1276/308
BOT CHORD 2-14=-644/2237, 13-14=-644/2237, 12-13=-528/1953, 11-12=-544/2314, 10-11=-544/2314
WEBS 3-13=-341/133, 4-13=-3/349, 4-12=-122/739, 5-12=-394/179, 6-10=-998/229, 7-10=-391/186, 8-10=-397/1857

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=34ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Walter P. Finn PE No 22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

November 6, 2019

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6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581565
Aragon	A4	Hip	1	1		

Mayo Truss Company, Inc., Mayo, FL - 32066,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:01 2019 Page 1

ID:31mchhOHyrVRSWwdKMVL?uyMA?e-a9vdvebgTwZrDcSU6HS9Gd972uwH?fbKHbEA4AylZgC

6-9-4	13-0-0	20-7-8	26-10-4	33-7-8
6-9-4	6-2-12	7-7-8	6-2-12	6-9-4

Scale = 1.56 9

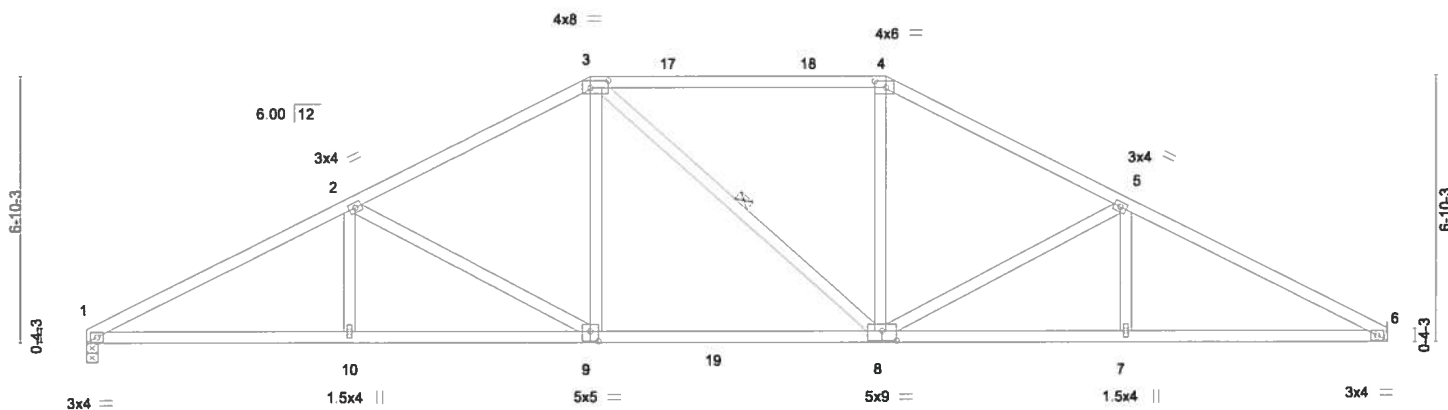


Plate Offsets (X,Y)	[3:0-5-8,0-2-4], [4:0-3-8,0-2-0], [8:0-4-8,0-3-0], [9:0-2-8,0-3-0]
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LOADING (psf)	SPACING	CSL	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.73	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.25	BC 0.68	Vert(LL) -0.16 8-9 >999 240		
BCLL 0.0 *	Lumber DOL 1.25	WB 0.55	Vert(CT) -0.34 8-9 >999 180		
BCDL 10.0	Rep Stress Incr YES	Matrix-AS	Horz(CT) 0.11 6 n/a n/a		
	Code FBC2017/TPI2014			Weight: 168 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 3-8

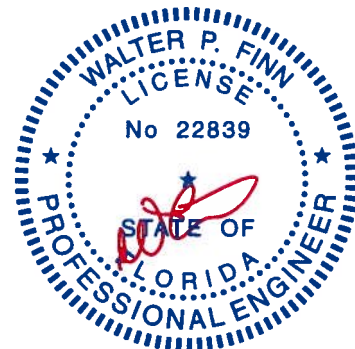
REACTIONS. (lb/size) 1=1345/0-3-8, 6=1345/Mechanical
Max Horz 1=115(LC 10)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2523/588, 2-3=-1962/512, 3-4=-1678/504, 4-5=-1957/510, 5-6=-2523/588
BOT CHORD 1-10=-439/2208, 9-10=-439/2208, 8-9=-227/1676, 7-8=-438/2208, 6-7=-438/2208
WEBS 2-10=0/260, 2-9=-608/241, 3-9=-30/509, 4-8=-28/496, 5-8=-611/242, 5-7=0/262

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=34ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Refer to girder(s) for truss to truss connections.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

November 6,2019

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6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581566
Aragon	A5	Hip	1	1	Job Reference (optional)	

Mayo Truss Company, Inc., Mayo, FL - 32066,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:03 2019 Page 1

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Scale = 1:58.2

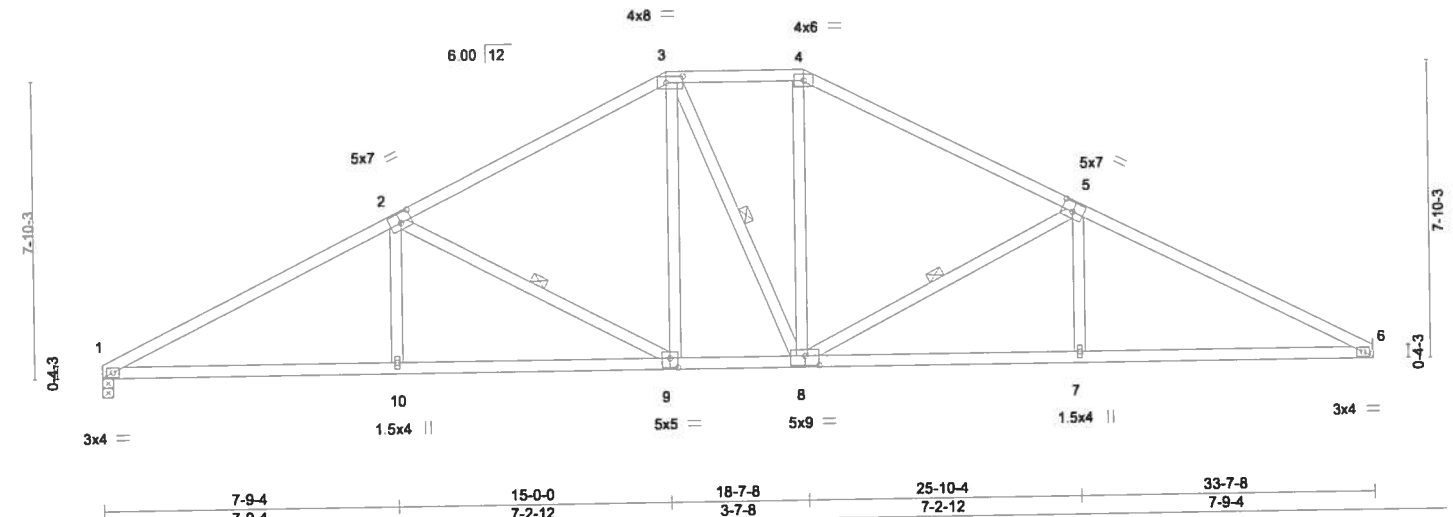


Plate Offsets (X,Y) = [2:0-3-8,0-3-0], [3:0-5-4,0-2-0], [5:0-3-8,0-3-0], [8:0-4-8,0-3-0], [9:0-2-8,0-3-0]

LOADING (psf)	SPACING-		CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	2-0-0	TC 0.58	Vert(LL)	-0.12	10-13	>999	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.75	Vert(CT)	-0.28	10-13	>999		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.24	Horz(CT)	0.11	6	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-AS					Weight: 174 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 2-9, 3-8, 5-8

REACTIONS. (lb/size) 1=1345/0-3-8, 6=1345/Mechanical
Max Horz 1=133(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2475/590, 2-3=-1789/503, 3-4=-1502/500, 4-5=-1786/502, 5-6=-2475/589
BOT CHORD 1-10=-428/2160, 9-10=-430/2156, 8-9=-179/1500, 7-8=-429/2156, 6-7=-428/2159
WEBS 2-10=0/329, 2-9=-752/286, 3-9=-75/468, 4-8=-74/469, 5-8=-753/286, 5-7=0/329

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=34ft; eave=4ft; Cat. II; Exp B; End., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581567
Aragon	A6	Common	2	1		

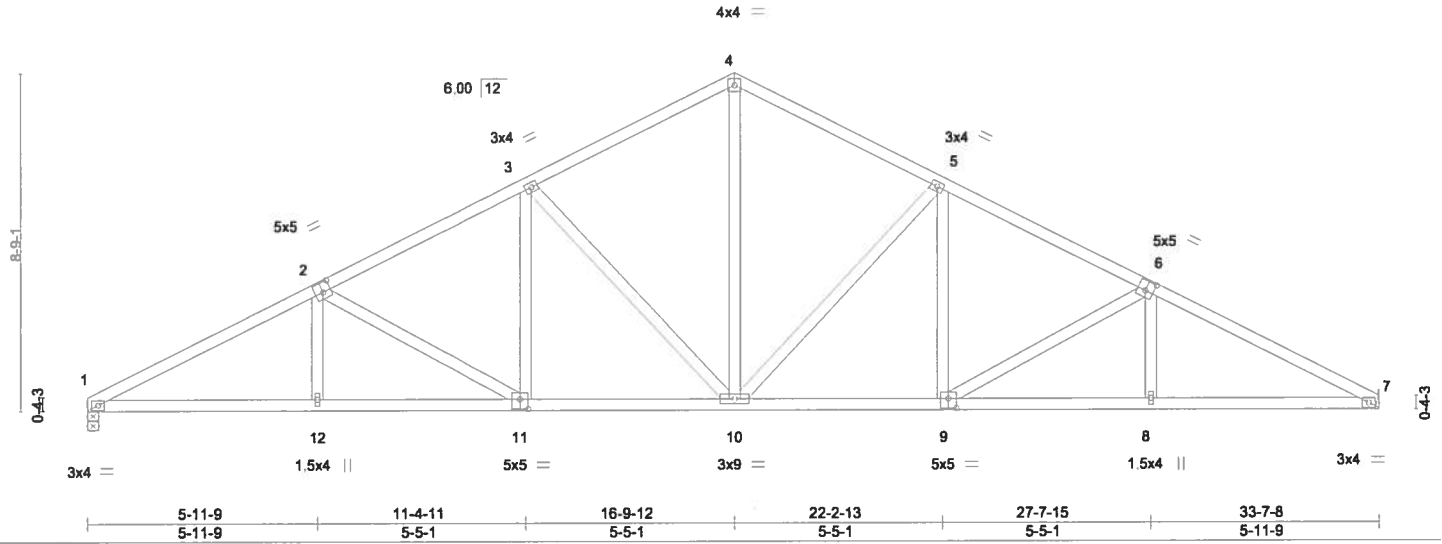
Mayo Truss Company, Inc., Mayo, FL - 32066,

8.240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:04 2019 Page 1

ID:31mchhOHyrVRSWwdKMvL7uyMA7e-jbmXfdZmrXQ43B3nP7suFnbI5ypC_6m_ZTqgVylZg9

5-11-9	11-4-11	16-9-12	22-2-13	27-7-15	33-7-8
5-11-9	5-5-1	5-5-1	5-5-1	5-5-1	5-11-9

Scale = 1:57.2



Job	Truss	Truss Type	Qty	Ply	Aragon	T18581568
Aragon	A7	Common	5	1		

Mayo Truss Company, Inc., Mayo, FL - 32066,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:06 2019 Page 1

ID:31mchhOHyrVRSWwdKMvL?uyMA7e-w6jWylfplSB8JNLsuq1LzgsxGvelgub3RtyxINyLzg7

5-11-9	11-4-11	16-9-12	22-2-13	27-7-15	33-7-8	35-7-8	2-0-0
5-11-9	5-5-1	5-5-1	5-5-1	5-5-1	5-11-9		

Scale = 1:58.8

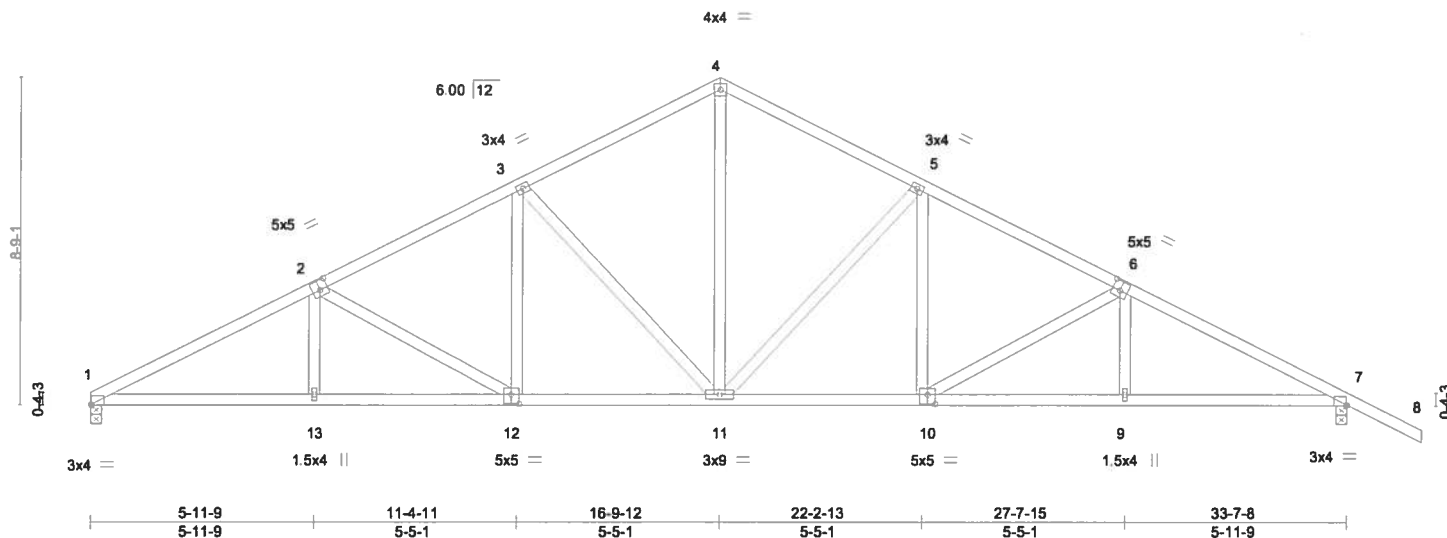


Plate Offsets (X,Y) -		[1:0-0-4,Edge], [2:0-2-8,0-3-0], [6:0-2-8,0-3-0], [7:0-0-4,Edge], [10:0-2-8,0-3-0], [12:0-2-8,0-3-0]									
LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0		Plate Grip DOL	1.25	TC 0.38		Vert(LL)	-0.13 11	>999	240	MT20	244/190
TCDL 10.0		Lumber DOL	1.25	BC 0.63		Vert(CT)	-0.27 11-12	>999	180		
BCLL 0.0 *		Rep Stress Incr	YES	WB 0.69		Horz(CT)	0.11 7	n/a	n/a		
BCDL 10.0		Code FBC2017/TPI2014		Matrix-AS						Weight: 186 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS.

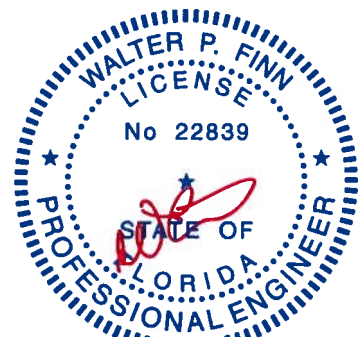
(lb/size) 1=1341/0-3-8, 7=1469/0-3-8
Max Horz 1=162(LC 10)
Max Uplift 7=51(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2554/613, 2-3=-2083/559, 3-4=-1586/504, 4-5=-1587/504, 5-6=-2075/555,
6-7=-2535/594
BOT CHORD 1-13=-432/2240, 12-13=-433/2237, 11-12=-263/1789, 10-11=-260/1784, 9-10=-413/2202,
7-9=-412/2205
WEBS 4-11=-278/1037, 5-11=-638/235, 5-10=-24/399, 6-10=-478/177, 3-11=-646/240,
3-12=-32/403, 2-12=-513/197

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=34ft; eave=4ft; Cat. II; Exp B; Encl., GCPl=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

November 6, 2019

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6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581569
Aragon	A8	Common	2	1		

Mayo Truss Company, Inc., Mayo, FL - 32066,

8.240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:09 2019 Page 1

ID:31mchhOHyrVRSWwdKMvL?uyMA7e-LhOfaNhnbNZjAq40ayb2bJUS86h9tFHV7rAbMiyLzg4

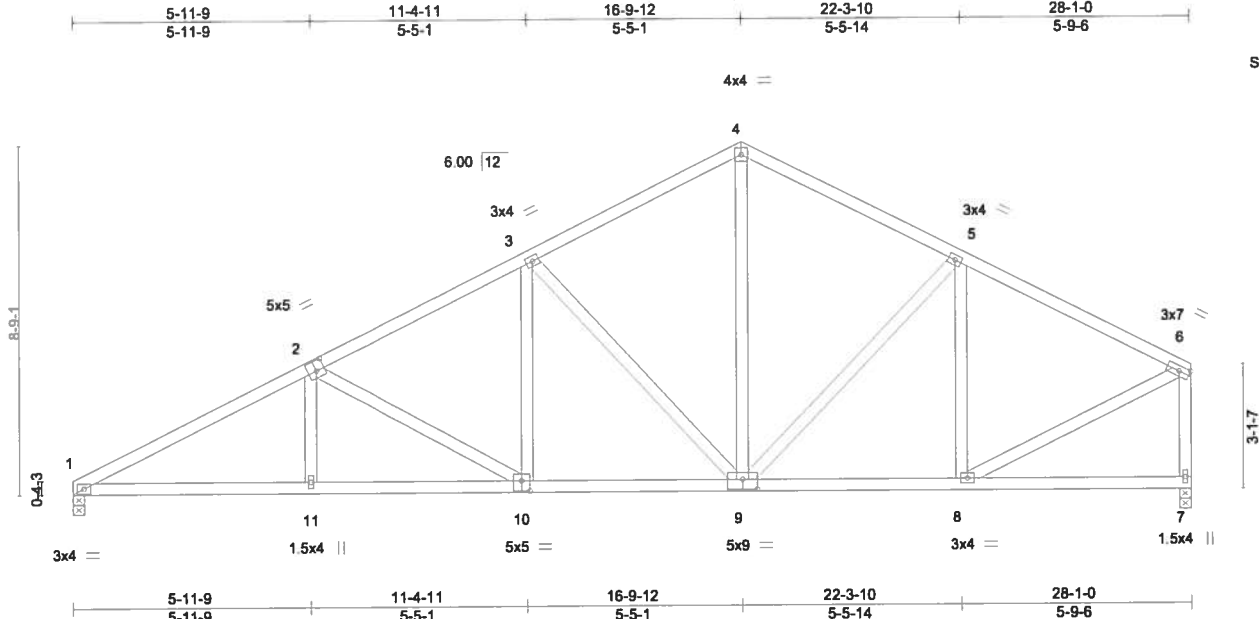


Plate Offsets (X, Y)		[2:0-2-8, 0-3-0], [9:0-4-8, 0-3-0], [10:0-2-8, 0-3-0]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES
TCLL 20.0	Plate Grip DOL	1.25	TC 0.34	Vert(LL)	-0.07	10	>999	240	GRIP
TCDL 10.0	Lumber DOL	1.25	BC 0.55	Vert(CT)	-0.15	9-10	>999	180	MT20
BCLL 0.0	Rep Stress Incr	YES	WB 0.70	Horz(CT)	0.05	7	n/a	n/a	
BCDL 10.0	Code	FBC2017/TPI2014	Matrix-AS						Weight: 165 lb FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied.

REACTIONS.

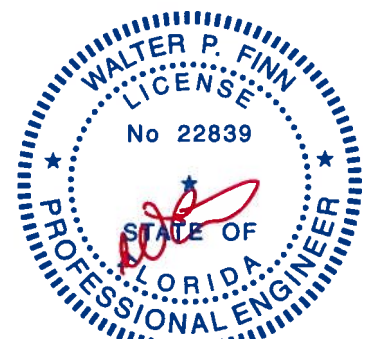
(lb/size) 1=1118/0-3-8, 7=1118/0-3-8
Max Horz 1=181(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2064/497, 2-3=-1587/442, 3-4=-1087/387, 4-5=-1091/388, 5-6=-1098/319,
6-7=-1062/297
BOT CHORD 1-11=-523/1803, 10-11=-524/1799, 9-10=-353/1345, 8-9=-238/921
WEBS 2-10=-517/198, 3-10=-32/407, 3-9=-648/240, 4-9=-177/597, 5-8=-339/177,
6-8=-222/987

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=28ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
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November 6, 2019

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6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Aragon	
Aragon	A9	Hip	1	1		T18581570

Mayo Truss Company, Inc., Mayo, FL - 32066,

8.240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:12 2019 Page 1

ID:31mchhOHyrVRSWwdKMvL?uyMA7e-Ig4oCPjauixH1lobF58lCx6wqJgp4j5ypoPGy1yLzg1

7-9-4	15-0-0	18-7-8	23-2-8	28-1-0
7-9-4	7-2-12	3-7-8	4-7-0	4-10-8

Scale = 1:50.5

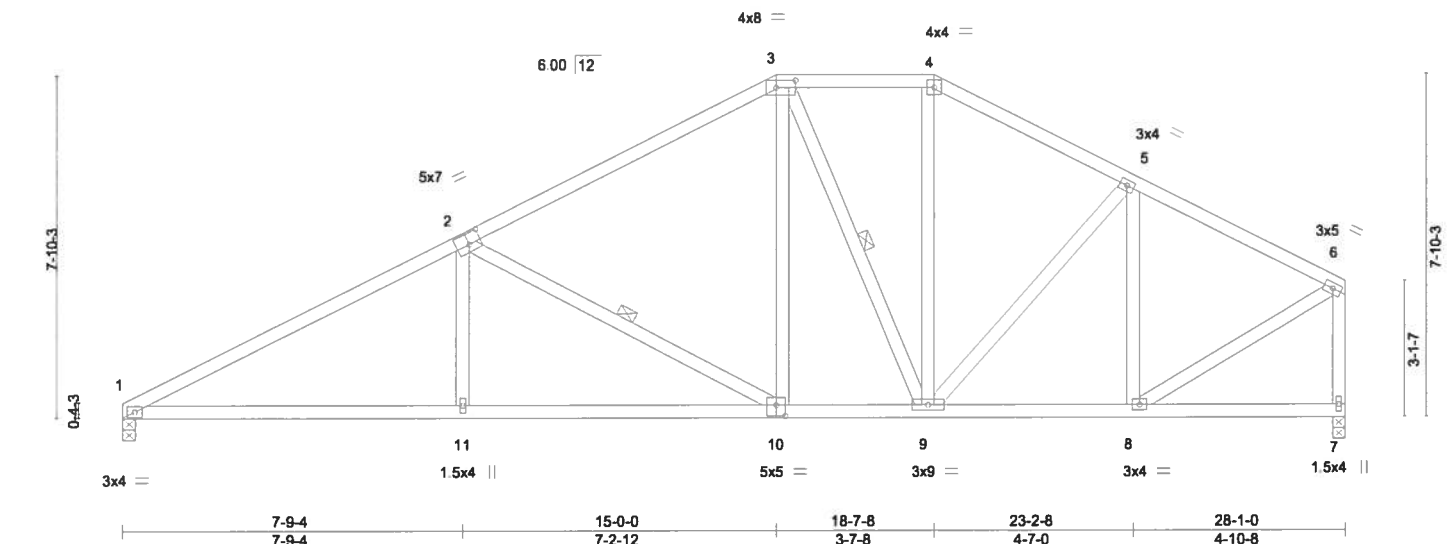


Plate Offsets (X,Y) - [2:0-3-8,0-3-0], [3:0-5-4,0-2-0], [10:0-2-8,0-3-0]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	In (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.57	Vert(LL)	-0.10 11-14	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.68	Vert(CT)	-0.23 11-14	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.25	Horz(CT)	0.05 7	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-AS						
								Weight: 168 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 2-10, 3-9

REACTIONS.

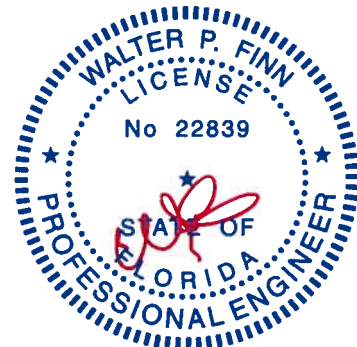
(lb/size) 1=1118/0-3-8, 7=1118/0-3-8
Max Horz 1=166(LC 11)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-1974/472, 2-3=-1284/383, 3-4=-934/367, 4-5=-1109/372, 5-6=-1027/293,
6-7=-1071/284
BOT CHORD 1-11=-480/1712, 10-11=-481/1708, 9-10=-231/1050, 8-9=-223/867
WEBS 2-11=0/329, 2-10=-754/287, 3-10=-64/490, 3-9=-352/73, 4-9=-68/276, 5-8=-412/184,
6-8=-219/979

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=28ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Walter P. Finn PE No 22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
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November 6, 2019

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6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581571
Aragon	A10	Hip	1	1		

Mayo Truss Company, Inc., Mayo, FL - 32066,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:23:49 2019 Page 1

ID:31mchhOHyrVRSWwdKMvL?uyMA?e-wrB5NXS8IEZYnmYAPmELn5fzm2rhBEXZjKYesyLzgO

6-9-4	13-0-0	20-7-8	28-1-0
6-9-4	6-2-12	7-7-8	7-5-8

Scale: 1/4"=1'

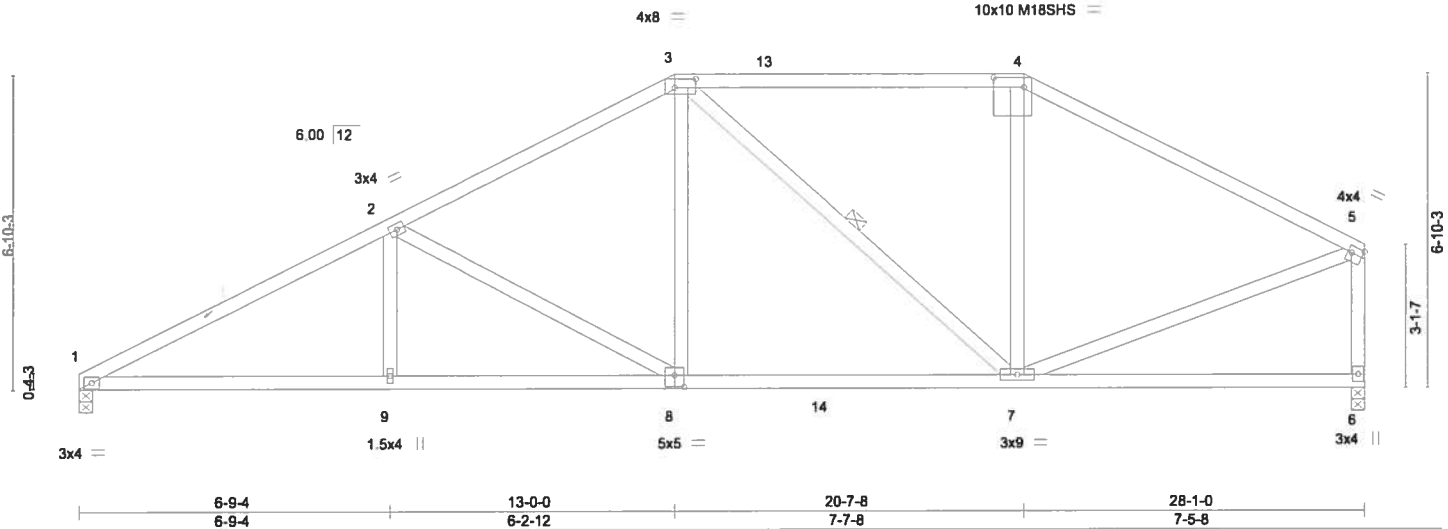


Plate Offsets (X, Y) - [3:0-5-8,0-2-4], [4:0-8-0,0-2-8], [5:Edge,0-1-12], [8:0-2-8,0-3-0]					
LOADING (psf)	SPACING	2-0-0	CSL	DEFL.	PLATES
TCLL 20.0	Plate Grip DOL	1.25	TC 0.62	in (loc) l/defl L/d	MT20 244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.60	Vert(LL) -0.11 7-8 >999 240	M18SHS 244/190
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.56	Vert(CT) -0.21 7-8 >999 180	
BCDL 10.0	Code FBC2017/TP12014		Matrix-AS	Horz(CT) 0.05 6 n/a n/a	
					Weight: 150 lb FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 3-7

REACTIONS.

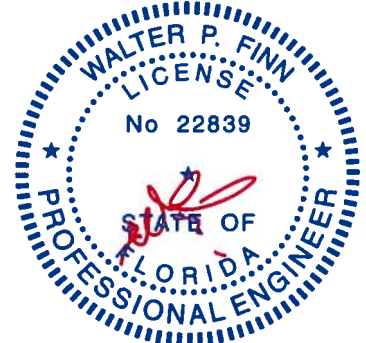
(lb/size) 1=1118/0-3-8, 6=1118/0-3-8
Max Horz 1=149/LC 11)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-2024/473, 2-3=-1455/396, 3-4=-979/344, 4-5=-1188/318, 5-6=-1047/292
BOT CHORD 1-9=-493/1765, 8-9=-493/1765, 7-8=-281/1244
WEBS 2-9=0/266, 2-8=-618/243, 3-8=-34/505, 3-7=-400/87, 5-7=-172/964

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=28ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Walter P. Finn PE No 22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
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6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581572
Aragon	A11	Hip	1	1		

Mayo Truss Company, Inc., Mayo, FL - 32066,

8.240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:23:50 2019 Page 1

ID:31mchhOHyrVRSWwdKMvL?uyMA?e-O1TbtSm3XAPOv7NyTlaJICczSC1wkJimN45BJLzgN

5-9-4	11-0-0	16-9-12	22-7-8	28-1-0
5-9-4	5-2-12	5-9-12	5-9-12	5-5-8

Scale: 1/4"=1'

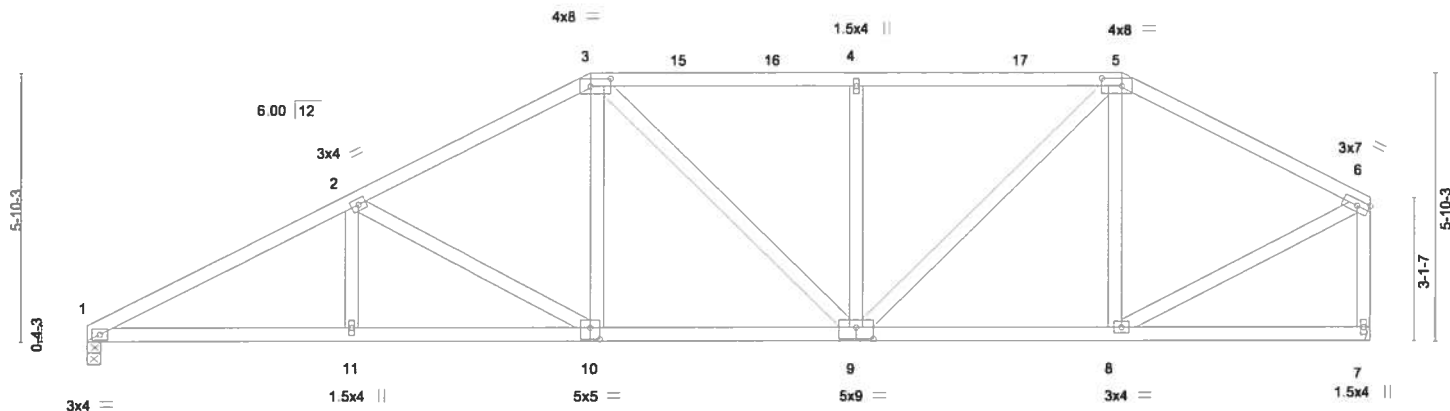


Plate Offsets (X,Y) -	[3:0-5-4,0-2-0], [5:0-5-4,0-2-0], [9:0-4-8,0-3-0], [10:0-2-8,0-3-0]
-----------------------	---

LOADING (psf)	SPACING-	CSL	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 2-0-0	TC 0.34	Vert(LL) -0.07	10	>999	240		MT20	244/190
TCDL 10.0	Lumber DOL 1.25	BC 0.53	Vert(CT) -0.16	9-10	>999	180			
BCLL 0.0 *	Rep Stress Incr YES	WB 0.33	Horz(CT) 0.05	7	n/a	n/a			
BCDL 10.0	Code FBC2017/TPI2014	Matrix-AS							
								Weight: 158 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied.

REACTIONS.

(lb/size) 1=1118/0-3-8, 7=1118/Mechanical
Max Horz 1=132(LC 11)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-2079/472, 2-3=-1607/409, 3-4=-1390/402, 4-5=-1390/402, 5-6=-1083/288,
6-7=-1065/274
BOT CHORD 1-11=-504/1819, 10-11=-504/1819, 9-10=-327/1368, 8-9=-205/897
WEBS 2-10=-517/204, 3-10=-31/407, 4-9=-386/165, 5-9=-154/722, 5-8=-343/163,
6-8=-189/977

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=28ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
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6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581573
Aragon	A12	Half Hip	1	1		

Mayo Truss Company, Inc., Mayo, FL - 32066,

8.240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:23:51 2019 Page 1

ID:31mchhOHyrVRSWwdKMvL?uyMA7e-sDJroDTPqrIG03iZWAGpsWIN8sYHfA8s_1pejlyLzgM



Scale = 1:50.6

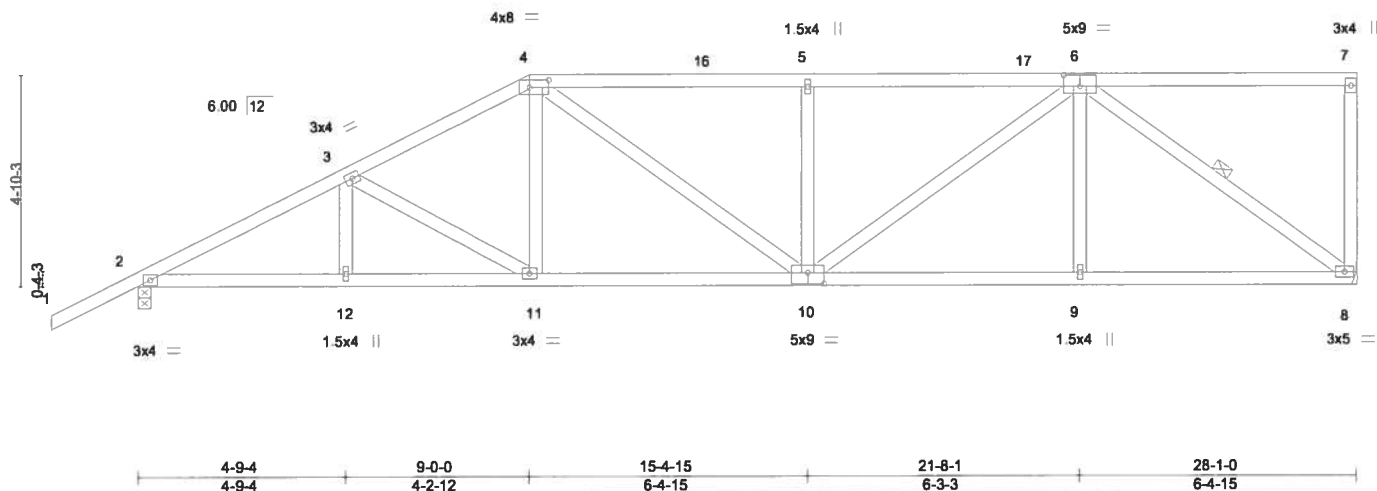


Plate Offsets (X,Y) - [4:0-5-4,0-2-0], [6:0-4-8,0-3-0], [10:0-4-8,0-3-0]

LOADING (psf)	SPACING-	CSL	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.38	Vert(LL)	-0.08	10	>999	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.25	BC 0.47	Vert(CT)	-0.19	10-11	>999		
BCLL 0.0 *	Lumber DOL 1.25	WB 0.42	Horz(CT)	0.07	8	n/a		
BCDL 10.0	Rep Stress Incr YES	Matrix-AS						
	Code FBC2017/TPI2014						Weight: 157 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied.
WEBS 1 Row at midpt 6-8

REACTIONS.

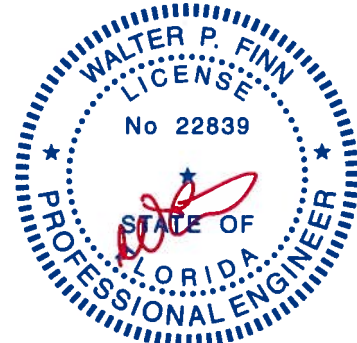
(lb/size) 8=1113/Mechanical, 2=1242/0-3-8
Max Horz 2=150(LC 11)
Max Uplift 2=50(LC 12)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2079/399, 3-4=-1741/389, 4-5=-1741/423, 5-6=-1741/423
BOT CHORD 2-12=-556/1808, 11-12=-556/1808, 10-11=-438/1518, 9-10=-317/1246, 8-9=-317/1246
WEBS 3-11=-345/135, 4-11=-1/355, 4-10=-48/376, 5-10=-398/180, 6-10=-163/612, 6-9=0/265, 6-8=-1503/336

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=28ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
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6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581574
Aragon	A13GIR	Half Hip Girder	1	2		

Mayo Truss Company, Inc., Mayo, FL - 32066,

8.240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:23:54 2019 Page 1

ID:31mchhOHyrVRSWwdKMvL?uyMA7e-Ho_RFVH7mgrtXQ8BJqWU8Mr83YMsXDlg72JK4yLzgJ

-2-0-0	7-0-0	12-4-2	17-6-8	22-8-14	28-1-0
2-0-0	7-0-0	5-4-2	5-2-6	5-2-6	5-4-2

Scale = 1:51.0

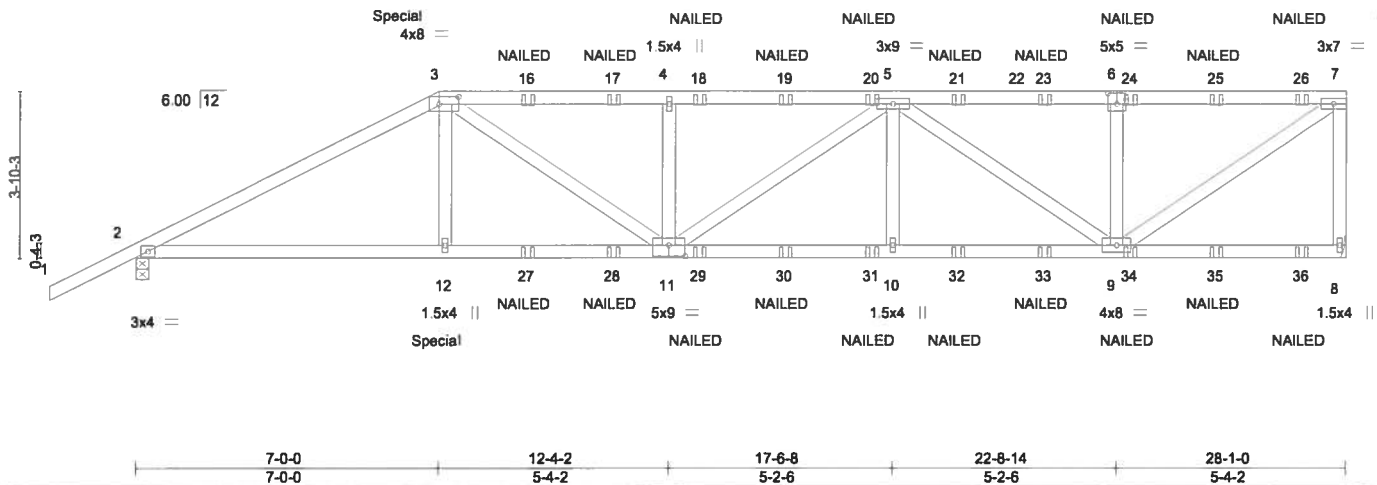


Plate Offsets (X,Y)- [3:0-5-4,0-2-0], [6:0-2-8,0-3-0], [11:0-4-8,0-3-0]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.52	Vert(LL)	-0.12	10-11	>999	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.63	Vert(CT)	-0.26	10-11	>999		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.40	Horz(CT)	0.07	8	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-MS						
								Weight: 295 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-1-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(lb/size) 8=2410/Mechanical, 2=2251/0-3-8
Max Horz 2=120(LC 24)
Max Uplift 8=2(LC 8), 2=-17(LC 8)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-4195/0, 3-4=-4790/0, 4-5=-4790/0, 5-6=-2971/33, 6-7=-2971/33, 7-8=-2284/62
BOT CHORD 2-12=0/3669, 11-12=0/3690, 10-11=0/4507, 9-10=0/4507
WEBS 3-12=0/641, 3-11=-56/1394, 4-11=-688/151, 5-11=0/343, 5-10=0/415, 5-9=-1861/0, 6-9=-680/157, 7-9=0/3539

NOTES-

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=28ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 2.
- "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 227 lb down and 133 lb up at 7-0-0 on top chord, and 319 lb down at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

November 6, 2019

Continued on page 2

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581574
Aragon	A13GIR	Half Hip Girder	1	2	Job Reference (optional)	

Mayo Truss Company, Inc., Mayo, FL - 32066,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:23:54 2019 Page 2
ID:31mchhOHyrVRSWwdKMvL?uyMA7e-Ho__RFVH7mgtXQ8BJqWU8Mr63YMsXDlg?2JK4yLzgJ

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-3=-60, 3-7=-60, 8-13=-20

Concentrated Loads (lb)

Vert: 3=-180(F) 12=-319(F) 16=-121(F) 17=-121(F) 18=-121(F) 19=-121(F) 20=-121(F) 21=-121(F) 23=-121(F) 24=-121(F) 25=-121(F) 26=-125(F) 27=-59(F)
28=-59(F) 29=-59(F) 30=-59(F) 31=-59(F) 32=-59(F) 33=-59(F) 34=-59(F) 35=-59(F) 36=-60(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

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6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581575
Aragon	B1GE	Common Supported Gable	1	1		

Mayo Truss Company, Inc., Mayo, FL - 32066,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:16 2019 Page 1

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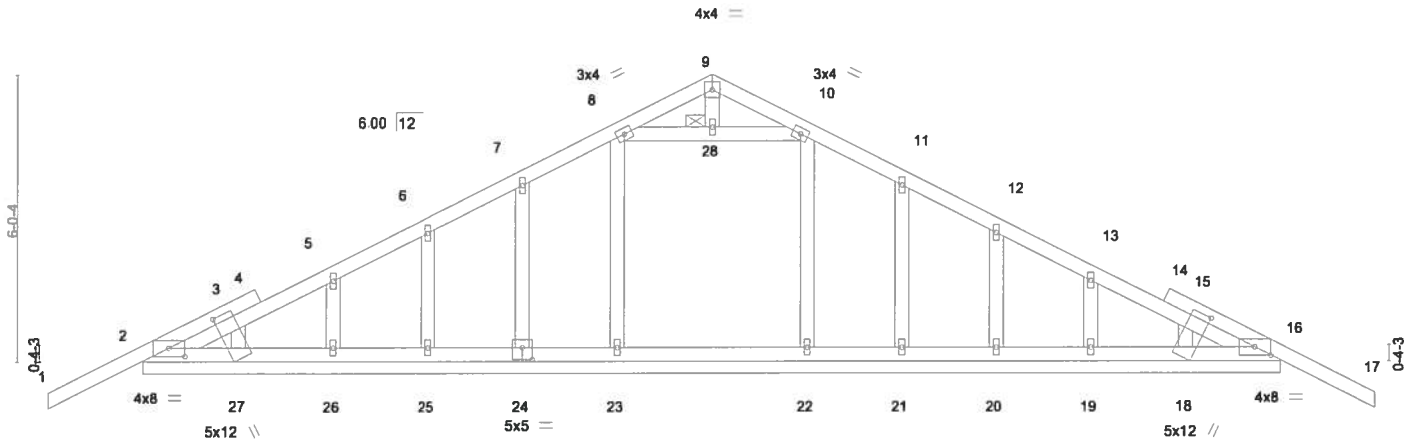


Plate Offsets (X,Y)- [2:0-4-0,0-2-1], [3:0-0-0,0-1-15], [15:0-0-0,0-1-15], [16:0-4-0,0-2-1], [18:0-1-8,1-1-2], [24:0-2-8,0-3-0], [27:0-1-8,1-1-2]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.26	Vert(LL)	-0.02	17	n/r	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.14	Vert(CT)	-0.03	17	n/r		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.07	Horz(CT)	0.00	16	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S					Weight: 132 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 28

REACTIONS.

All bearings 24-0-0.
(lb) - Max Horz 2=110(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 2, 16, 24, 25, 26, 21, 20, 19
Max Grav All reactions 250 lb or less at joint(s) 24, 25, 26, 27, 21, 20, 19, 18 except 2=257(LC 1), 16=257(LC 1), 23=321(LC 17), 22=303(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 1.5x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 16, 24, 25, 26, 21, 20, 19.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 16.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
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November 6, 2019

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6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581576
Aragon	B2	Common	5	1		

Mayo Truss Company, Inc., Mayo, FL - 32066,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:17 2019 Page 1
ID:31mchhOHyrVRSWwdKMvL7uyMA7e-6EthF6niqaa83hZ2ekwv?pqkKQdlxhz460dEylZfy

-2-0-0	6-3-4	12-0-0	17-8-12	24-0-0	26-0-0
2-0-0	6-3-4	5-8-12	5-8-12	6-3-4	2-0-0

Scale = 1.44.9

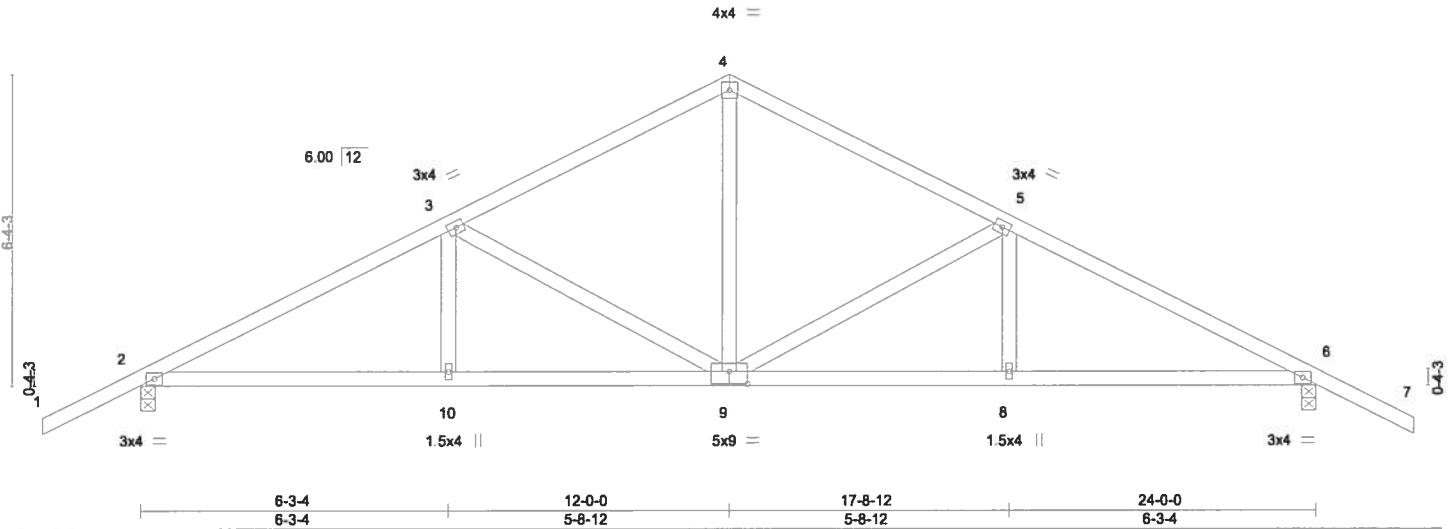


Plate Offsets (X,Y)- [9-0-4-8 0-3-0]									
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.37	Vert(LL)	0.11 9-10	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.45	Vert(CT)	-0.13 9-10	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.41	Horz(CT)	0.05 6	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-AS					Weight: 118 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS.

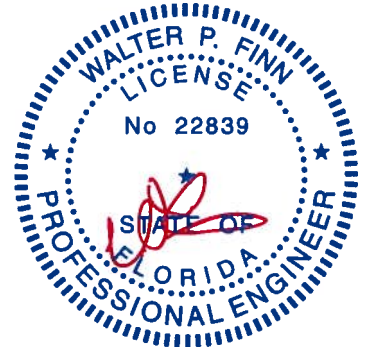
(lb/size) 2=1080/0-3-8, 6=1080/0-3-8
Max Horz 2=116(LC 11)
Max Uplift 2=256(LC 12), 6=256(LC 12)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1677/1276, 3-4=-1157/930, 4-5=-1157/931, 5-6=-1677/1276
BOT CHORD 2-10=-1009/1439, 9-10=-1009/1439, 8-9=-1027/1439, 6-8=-1027/1439
WEBS 4-9=-656/642, 5-9=-560/508, 3-9=-560/508

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=256, 6=256.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

November 6, 2019

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6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581577
Aragon	B3	Common	1	1		

Mayo Truss Company, Inc., Mayo, FL - 32066,

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ID:31mchhOHyrVRSWwdKMvL?uyMA?e-2c?RgopzESqINNx93mO_Qv9s869Dqr_QOb7h7yLzfw

6-3-4	12-0-0	17-8-12	24-0-0
6-3-4	5-8-12	5-8-12	6-3-4

Scale = 1:41.5

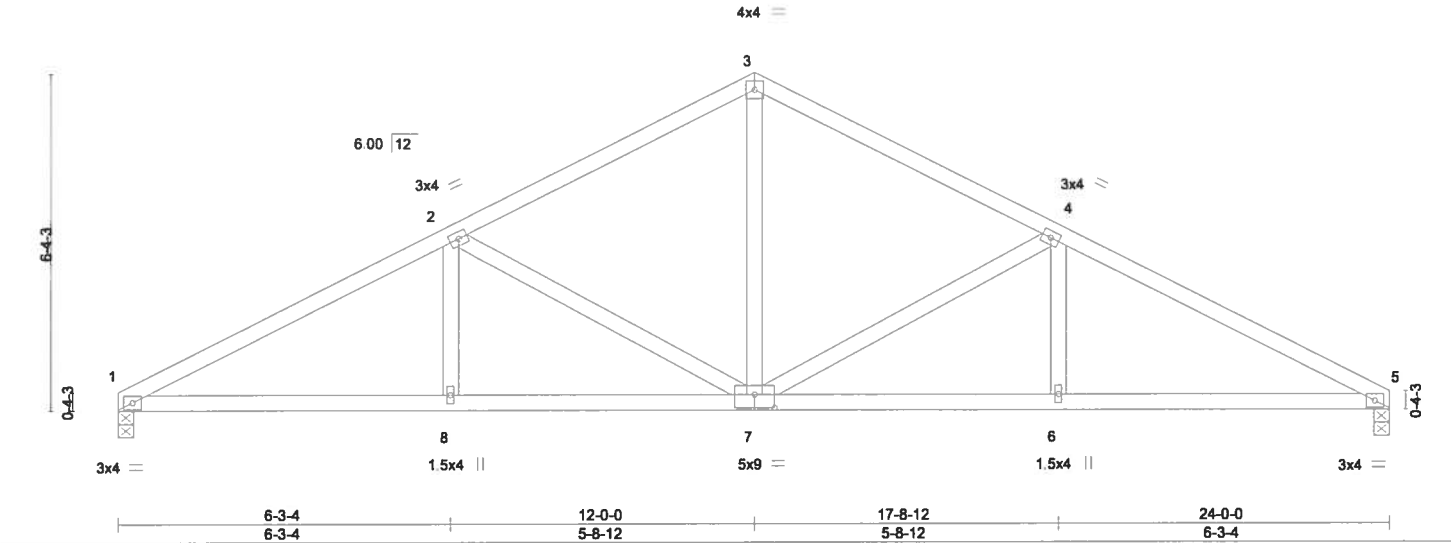


Plate Offsets (X,Y) -- [7:0-4-8,0-3-0]									
LOADING (psf)	SPACING-		CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES
TCLL 20.0	Plate Grip DOL	1.25	TC 0.39	Vert(LL)	0.12	8-11	>999	240	MT20
TCDL 10.0	Lumber DOL	1.25	BC 0.51	Vert(CT)	-0.14	8-11	>999	180	
BCLL 0.0	Rep Stress Incr	YES	WB 0.44	Horz(CT)	0.05	5	n/a	n/a	
BCDL 10.0	Code FBC2017/TPI2014		Matrix-AS						
									Weight: 112 lb FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS.

(lb/size) 1=960/0-3-8, 5=960/0-3-8
Max Horz 1=99(LC 10)
Max Uplift 1=207(LC 12), 5=207(LC 12)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-1710/1310, 2-3=-1178/949, 3-4=-1178/949, 4-5=-1710/1310
BOT CHORD 1-8=-1097/1487, 7-8=-1097/1487, 6-7=-1097/1487, 5-6=-1097/1487
WEBS 3-7=-679/667, 4-7=-594/528, 4-6=-209/253, 2-7=-594/528, 2-8=-209/253

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 1=207, 5=207.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
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6904 Parke East Blvd.
Tampa, FL 36610

Job Aragon	Truss C1GE	Truss Type Common Supported Gable	Qty 1	Ply 1	Aragon	T18581578
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Mayo Truss Company, Inc., Mayo, FL - 32066,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:21 2019 Page 1

ID:31mchhOHyrVRSWwdKMvL?uyMA7e-77B5UqDm340cg_KHUp4r_WNyuhqUHui4Em0yLzfu

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2-0-0 9-11-8 9-11-8 2-0-0

Scale = 1:39.6

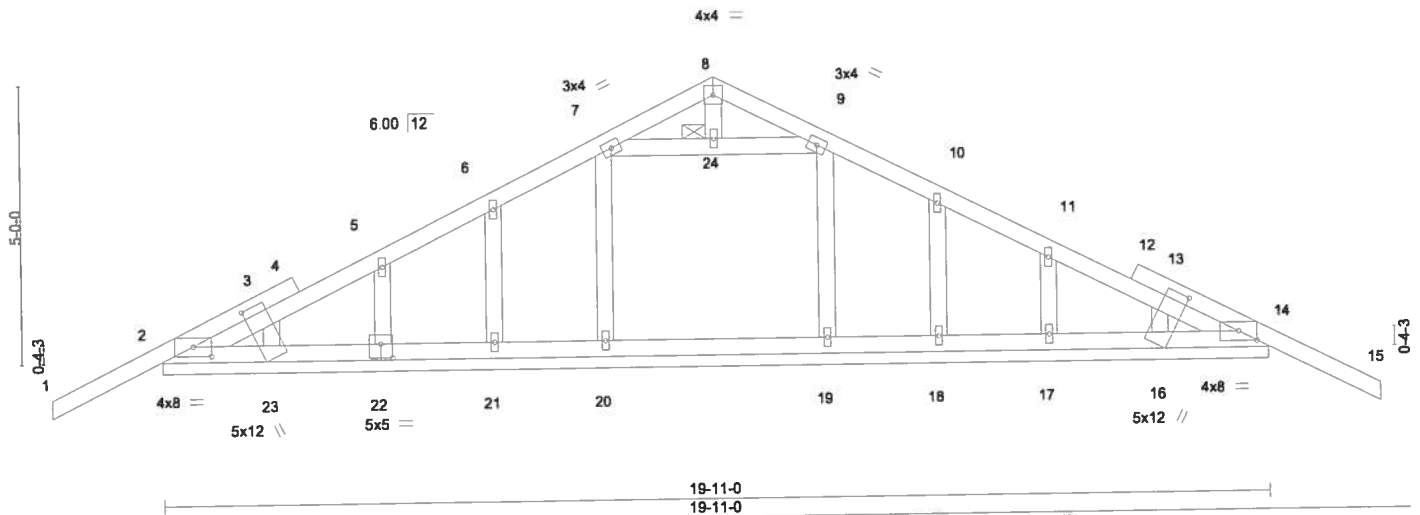


Plate Offsets (X,Y)- [2:0-4-0,0-2-1], [3:0-0-0,0-1-15], [13:0-0-0,0-1-15], [14:0-4-0,0-2-1], [16:0-1-12,1-0-11], [22:0-2-8,0-3-0], [23:0-1-12,1-0-11]

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	In	(loc)	L/def	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.26	Vert(LL)	-0.02	15	n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.09	Vert(CT)	-0.03	15	n/r	120		
BCLL 0.0	Rep Stress Incr	YES	WB 0.04	Horz(CT)	0.00	14	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S						Weight: 106 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 24

REACTIONS.

All bearings 19-11-0.
(lb) - Max Horz 2=93(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 2, 14, 21, 22, 18, 17
Max Grav All reactions 250 lb or less at joint(s) 20, 21, 22, 23, 19, 18, 17, 16 except 2=256(LC 1),
14=257(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Encl., GCPI=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 1.5x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 14, 21, 22, 18, 17.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
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November 6,2019

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6904 Parke East Blvd.
Tampa, FL 36610

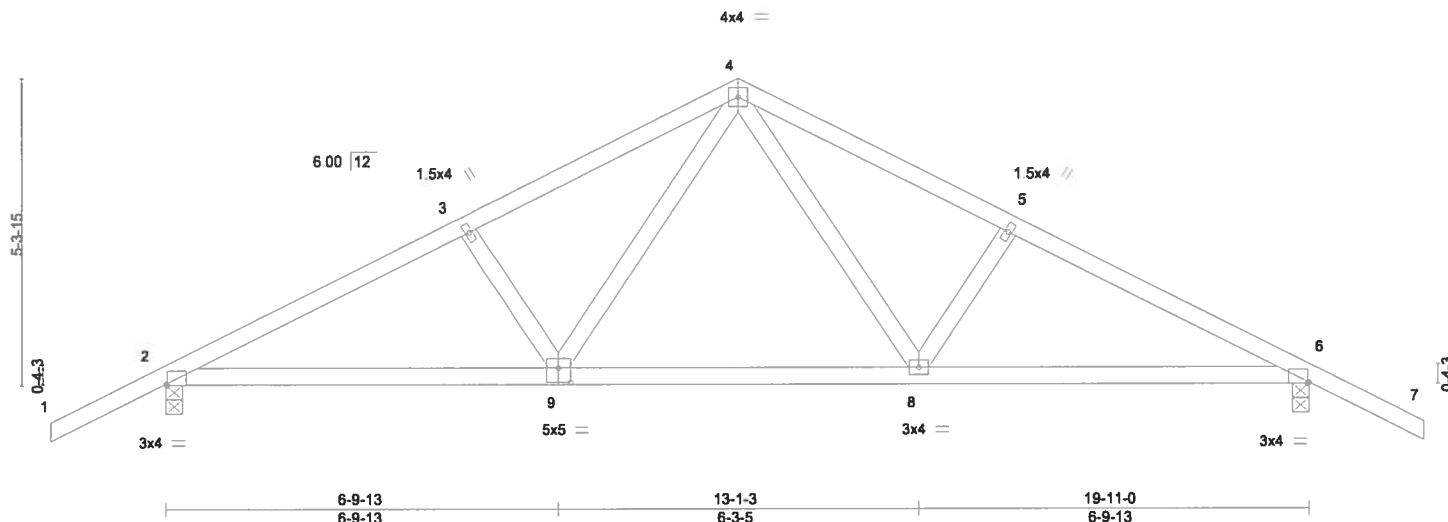
Job	Truss	Truss Type	Qty	Ply	Aragon	T18581579
Aragon	C2	Common	4	1	Job Reference (optional)	

Mayo Truss Company, Inc., Mayo, FL - 32066,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:22 2019 Page 1
ID:31mchhOHyrVRSWwdKMvL?uyMA7e-SBhaJpnXNCIEqZWqBK5c2XiFL9rQGqQ6MqniSylZft

-2-0-0	5-3-0	9-11-8	14-8-0	19-11-0	21-11-0
2-0-0	5-3-0	4-8-8	4-8-8	5-3-0	2-0-0

Scale = 1:38.3



LOADING (psf)	SPACING-	CSL	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.25	in (loc) l/defl L/d	MT20	244/190
TCDL 10.0	Plate Grip DOL 1.25	BC 0.45	Vert(LL) -0.05 9-12 >999 240		
BCLL 0.0 *	Lumber DOL 1.25	WB 0.10	Vert(CT) -0.11 9-12 >999 180		
BCDL 10.0	Rep Stress Incr YES	Matrix-AS	Horz(CT) 0.03 6 n/a n/a		
	Code FBC2017/TPI2014			Weight: 95 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS.

(lb/size) 2=917/0-3-8, 6=917/0-3-8
Max Horz 2=99(LC 10)
Max Uplift 2=49(LC 12), 6=49(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1345/321, 3-4=-1194/329, 4-5=-1194/329, 5-6=-1345/321
BOT CHORD 2-9=-160/1155, 8-9=-36/776, 6-8=-178/1155
WEBS 4-8=-89/444, 5-8=-287/177, 4-9=-89/444, 3-9=-287/177

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

November 6, 2019

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Job	Truss	Truss Type	Qty	Ply	Aragon	T18581580
Aragon	C3	Common	1	1		

Mayo Truss Company, Inc., Mayo, FL - 32066,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:23 2019 Page 1

ID:31mchhOHyrVRSWwdKMvL?uyMA7e-wNEyW9sTlgKks_BjOvrK9G3solTr9jzZL0ZLquyLzfs



Scale = 1:36 3

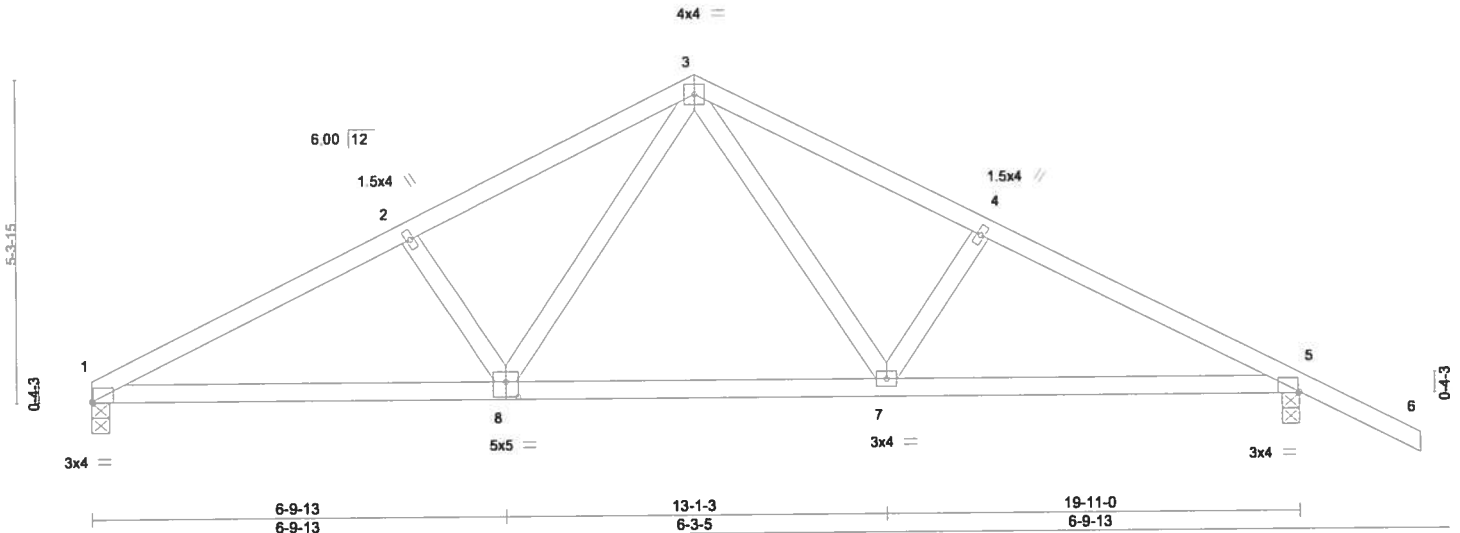


Plate Offsets (X,Y)- [1:0-0-4,Edge], [5:0-0-4,Edge], [8:0-2-8,0-3-0]

LOADING (psf)	SPACING-	CSL	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.26	Vert(LL) -0.05	8-11	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL 1.25	BC 0.46	Vert(CT) -0.12	8-11	>999	180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.11	Horz(CT) 0.03	5	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014	Matrix-AS					Weight: 92 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS.

(lb/size) 1=791/0-3-8, 5=923/0-3-8
Max Horz 1=95(LC 10)
Max Uplift 5=52(LC 12)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-1370/350, 2-3=-1229/357, 3-4=-1206/344, 4-5=-1357/336
BOT CHORD 1-8=-208/1195, 7-8=-51/789, 5-7=-192/1166
WEBS 3-7=-86/443, 4-7=-288/179, 3-8=-108/477, 2-8=-307/189

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; End., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

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6904 Parke East Blvd.
Tampa, FL 33610

Job Aragon	Truss C4GIR	Truss Type COMMON GIRDER	Qty 1	Ply 3	Aragon	T18581581
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Mayo Truss Company, Inc., Mayo, FL - 32066,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:35 2019 Page 1
ID:31mchhOHyrVRSWwdKMvL7uyMA7e-ahzU1G??TMr1lq305Q38eoZlOba1z21K6iTzFCyLzfg

3-8-3	6-9-13	9-11-8	13-1-3	16-2-13	19-11-0
3-8-3	3-1-11	3-1-11	3-1-11	3-1-11	3-8-3

Scale = 1:37.1

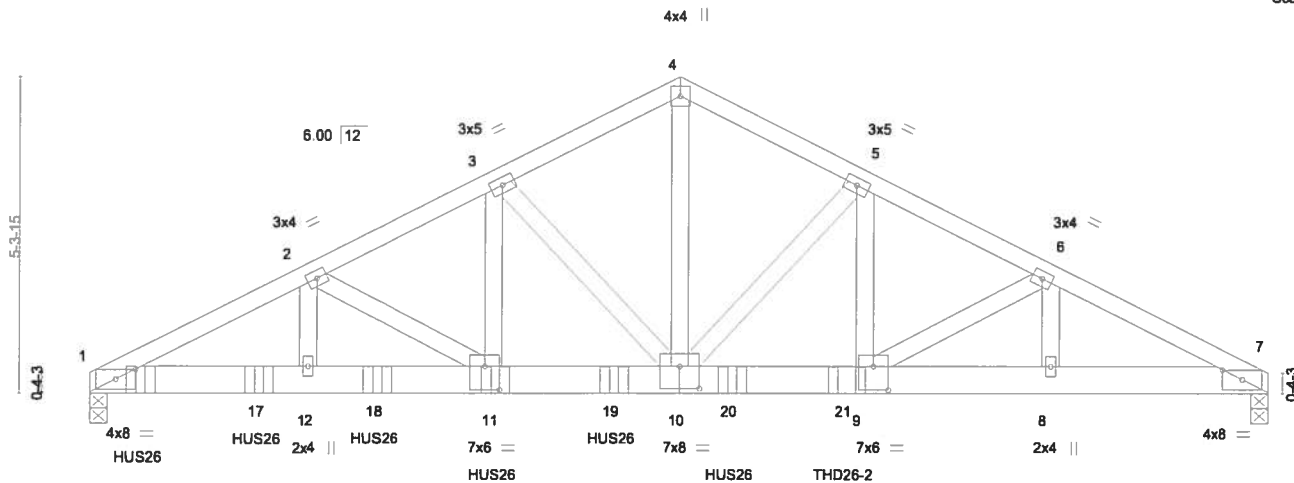


Plate Offsets (X,Y)-	[1:0-4-0,0-1-15], [7:0-4-0,0-1-15], [9:0-3-0,0-4-12], [10:0-4-0,0-4-8], [11:0-3-0,0-4-12]
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LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.48	Vert(LL)	-0.12	10-11	>999	240	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.50	Vert(CT)	-0.23	9-10	>999	180	
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.55	Horz(CT)	0.06	7	n/a	n/a	
BCDL 10.0	Code FBC2017/TPI2014		Matrix-MS						
								Weight: 367 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP SS
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-6-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=7450/0-3-8, 7=4999/0-3-8
Max Horz 1=82(LC 23)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

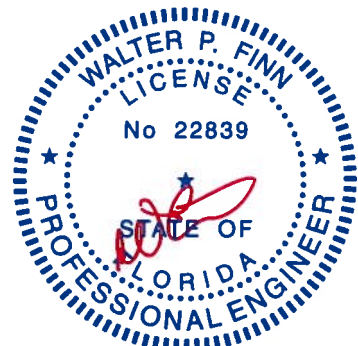
TOP CHORD 1-2=-13762/0, 2-3=-11053/0, 3-4=-8474/0, 4-5=-8473/0, 5-6=-10771/0, 6-7=-10737/0
BOT CHORD 1-12=0/12309, 11-12=0/12309, 10-11=0/9858, 9-10=0/9606, 8-9=0/9577, 7-8=0/9577
WEBS 4-10=0/7304, 5-10=-3002/0, 5-9=0/3055, 3-10=-3371/0, 3-11=0/3450, 2-11=-2827/0,
2-12=0/2391

NOTES-

- 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-6-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc, Except member 5-9 2x4 - 2 rows staggered at 0-7-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft. Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Use USP HUS26 (With 14-16d nails into Girder & 6-16d nails into Truss) or equivalent spaced at 2-0-0 oc max. starting at 0-10-4 from the left end to 10-10-4 to connect truss(es) to back face of bottom chord.
- Use USP THD26-2 (With 18-16d nails into Girder & 12-10d nails into Truss) or equivalent at 12-9-8 from the left end to connect truss(es) to back face of bottom chord.
- Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

- Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-60, 4-7=-60, 1-7=-20
Concentrated Loads (lb)
Vert: 11=-1325(B) 14=-1326(B) 17=-1325(B) 18=-1325(B) 19=-1321(B) 20=-1316(B) 21=-2917(B)



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
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Job	Truss	Truss Type	Qty	Ply	Aragon	T18581582
Aragon	CJ01	Diagonal Hip Girder	2	1		

Mayo Truss Company, Inc., Mayo, FL - 32066,

8.240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:36 2019 Page 1
ID:31mchhOHyrVRSWwdKMvL7uyMA7e-2tXsFc0dEgztw_eD7aNB76707diaVUKXDxoeyLzff

Job Reference (optional)

-2-9-15 5-2-5 9-10-13
2-9-15 5-2-5 4-8-7

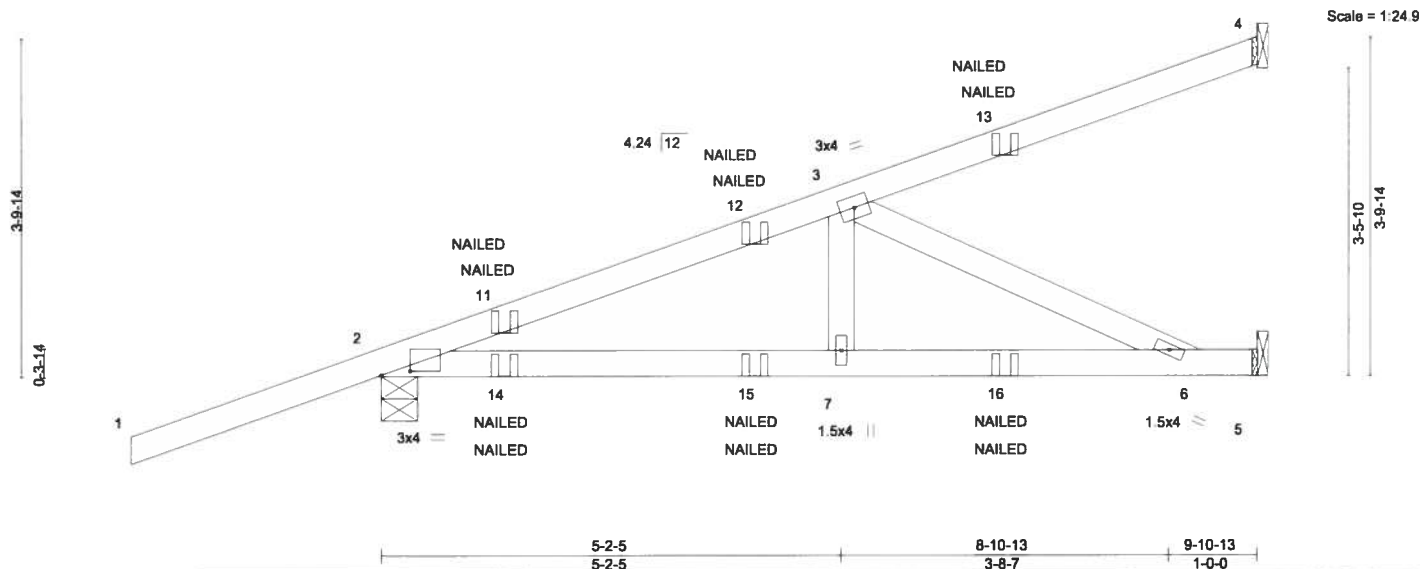


Plate Offsets (X,Y)- [2:0-3-15,0-0-9]

LOADING (psf)	SPACING-		CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL 1.25		TC 0.62	Ver(LL)	-0.06	7-10	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL 1.25		BC 0.61	Ver(CT)	-0.11	6-7	>999	180		
BCLL 0.0	Rep Stress Incr NO		WB 0.22	Horz(CT)	0.01	5	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-MS							
									Weight: 44 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(lb/size) 4=144/Mechanical, 2=487/0-4-15, 5=288/Mechanical
Max Horz 2=120(LC 8)
Max Uplift 4=32(LC 8), 2=152(LC 8)
Max Grav 4=144(LC 1), 2=544(LC 28), 5=303(LC 29)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-732/0
BOT CHORD 2-7=-35/641, 6-7=-35/641
WEBS 3-7=0/275, 3-6=-706/38

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cal. II; Exp B; End.; GCpi=0.18; MWFRS (directional); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=152.
- 6) "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidelines.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-60, 5-8=-20

Concentrated Loads (lb)

Vert: 11=72(F=36, B=36) 13=-71(F=-36, B=-36) 14=82(F=41, B=41) 15=4(F=2, B=2) 16=-49(F=-24, B=-24)



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
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November 6,2019

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6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581583
Aragon	D1GE	Common Supported Gable	1	1	Job Reference (optional)	

Mayo Truss Company, Inc., Mayo, FL - 32066,

8.240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:39 2019 Page 1

ID:31mchhOHyrVRSWwdKMvL?uyMA7e-SSC?td2WXbLSnRMnKG74oekbqC1Ovzww1VRBPzylZfc

-2-0-0	10-0-0	20-0-0	22-0-0
2-0-0	10-0-0	10-0-0	2-0-0

Scale = 1:39.7

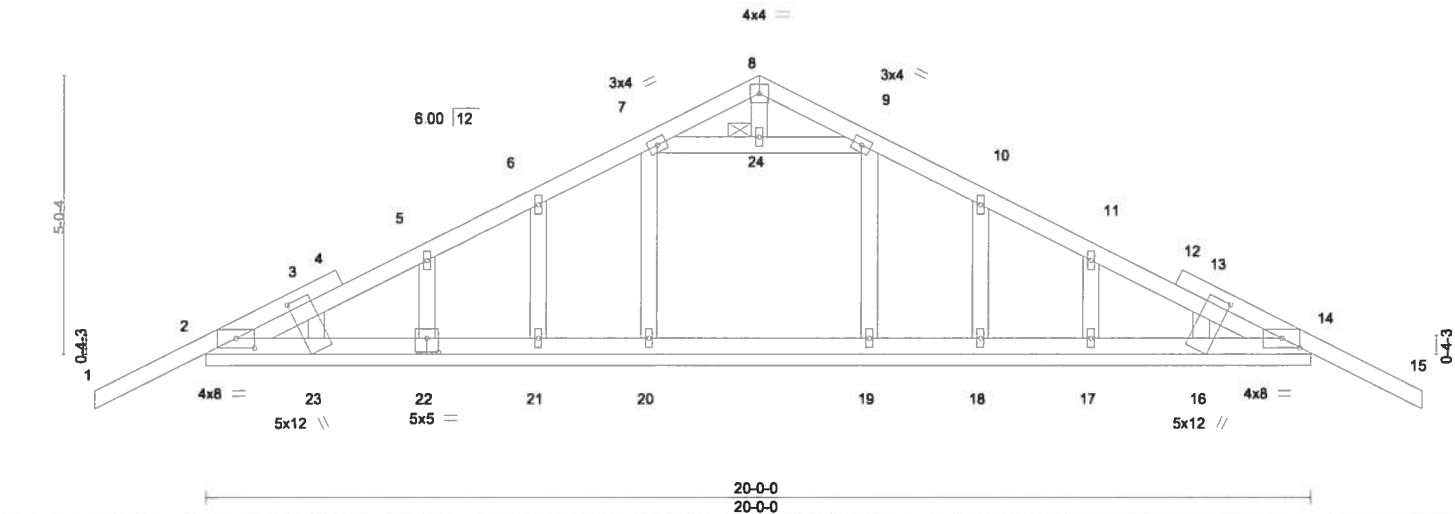


Plate Offsets (X,Y) - [2:0-4-0,0-2-1], [3:0-0-0,0-1-15], [13:0-0-0,0-1-15], [14:0-4-0,0-2-1], [16:0-1-8,1-1-2], [22:0-2-8,0-3-0], [23:0-1-8,1-1-2]									
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES
TCLL 20.0	Plate Grip DOL	1.25	TC 0.26	Vert(LL)	-0.02	15	n/r	120	MT20
TCDL 10.0	Lumber DOL	1.25	BC 0.09	Vert(CT)	-0.03	15	n/r	120	
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.04	Horz(CT)	0.00	14	n/a	n/a	
BCDL 10.0	Code FBC2017/TPI2014		Matrix-S						
									Weight: 106 lb FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 24

REACTIONS.

All bearings 20-0-0.
(lb) - Max Horz 2=94(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 2, 14, 21, 22, 18, 17
Max Grav All reactions 250 lb or less at joint(s) 20, 21, 22, 23, 19, 18, 17, 16 except 2=257(LC 1), 14=257(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 1.5x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 14, 21, 22, 18, 17.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
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6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581584
Aragon	D2	Common	2	1		

Mayo Truss Company, Inc., Mayo, FL - 32066,

8.240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:40 2019 Page 1
ID:31mchhOHyrVRSWwdKMvL7uyMA7e-wfmN4z38lvTJObx_uzfJLrGmwch_eM83F9BkxPyLzfb

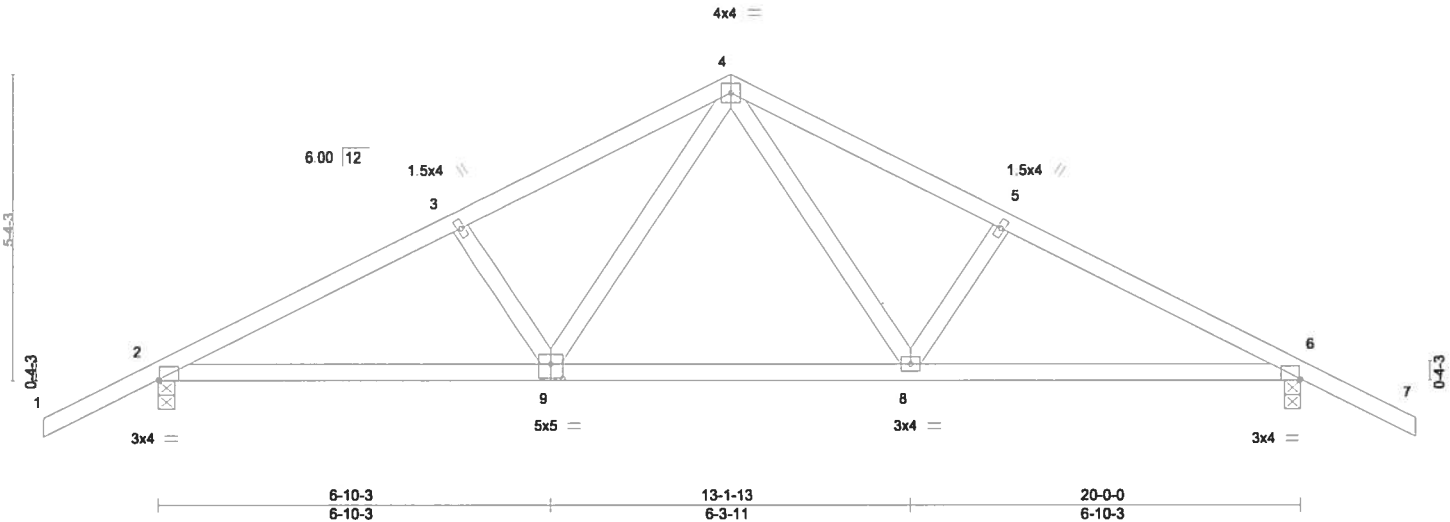


Plate Offsets (X,Y)=[2:0-0-4,Edge], [6:0-0-4,Edge], [9:0-2-8,0-3-0]									
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc) l/defl L/d		PLATES GRIP	
TCLL	20.0	Plate Grip DOL 1.25		TC	0.30	Vert(LL)	0.11 8-15 >999 240	MT20	244/190
TCDL	10.0	Lumber DOL 1.25		BC	0.45	Vert(CT)	-0.12 8-15 >999 180		
BCLL	0.0 *	Rep Stress Incr YES		WB	0.30	Horz(CT)	0.03 6 n/a n/a		
BCDL	10.0	Code FBC2017/TPJ2014		Matrix-AS				Weight: 95 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS.

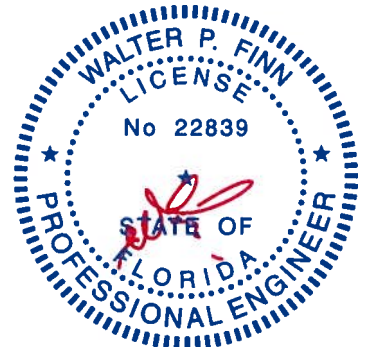
(lb/size) 2=920/0-3-8, 6=920/0-3-8
Max Horz 2=99(LC 11)
Max Uplift 2=221(LC 12), 6=221(LC 12)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1351/1067, 3-4=-1199/1072, 4-5=-1199/1072, 5-6=-1351/1067
BOT CHORD 2-9=-838/1160, 8-9=-486/780, 6-8=-856/1160
WEBS 4-8=-487/446, 5-8=-289/210, 4-9=-487/446, 3-9=-289/210

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=221, 6=221.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Walter P. Finn PE No. 22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

November 6, 2019

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6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581585
Aragon	D3	Common	1	1		

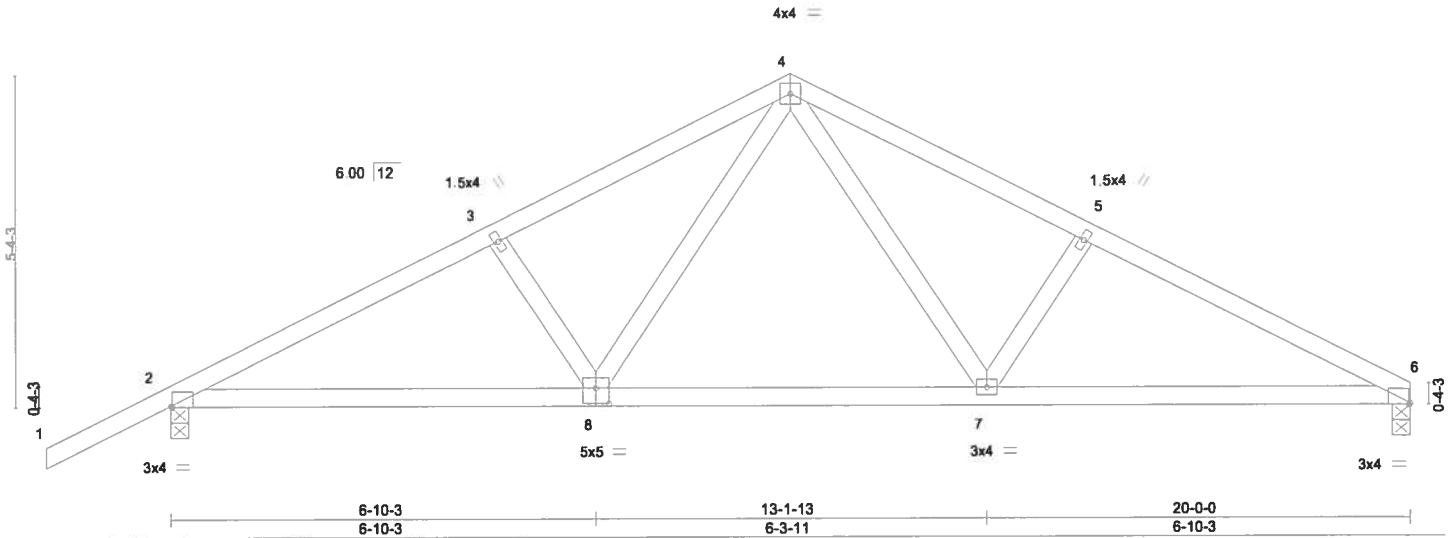
Mayo Truss Company, Inc., Mayo, FL - 32066,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:42 2019 Page 1

ID:31mchhOHyrVRSWwdKMvL?uyMA?e-11u7Vf5OpWj1ev5M?OhnQGM5pPyE6GTMjTgr?lyLzfZ

2-0-0	5-3-4	10-0-0	14-8-12	20-0-0
2-0-0	5-3-4	4-8-12	4-8-12	5-3-4

Scale = 1:35.5



LOADING (psf)		SPACING-		CSL		DEFL		PLATES		GRIP	
TCLL	20.0	Plate Grip DOL	1.25	TC	0.34	Vert(LL)	0.12	MT20	244/190		
TCDL	10.0	Lumber DOL	1.25	BC	0.47	Vert(CT)	-0.12				
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.31	Horz(CT)	0.03				
BCDL	10.0	Code FBC2017/TPI2014		Matrix-AS							
								Weight: 92 lb		FT = 0%	

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS.

(lb/size) 6=794/0-3-8, 2=926/0-3-8
Max Horz 2=95(LC 11)
Max Uplift 6=169(LC 12), 2=225(LC 12)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1364/1081, 3-4=-1211/1086, 4-5=-1234/1099, 5-6=-1376/1095
BOT CHORD 2-8=-907/1171, 7-8=-538/792, 6-7=-923/1200
WEBS 4-7=-505/479, 5-7=-308/222, 4-8=-483/445, 3-8=-289/211

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=169, 2=225.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
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6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581586
Aragon	D4	Common	2	1		

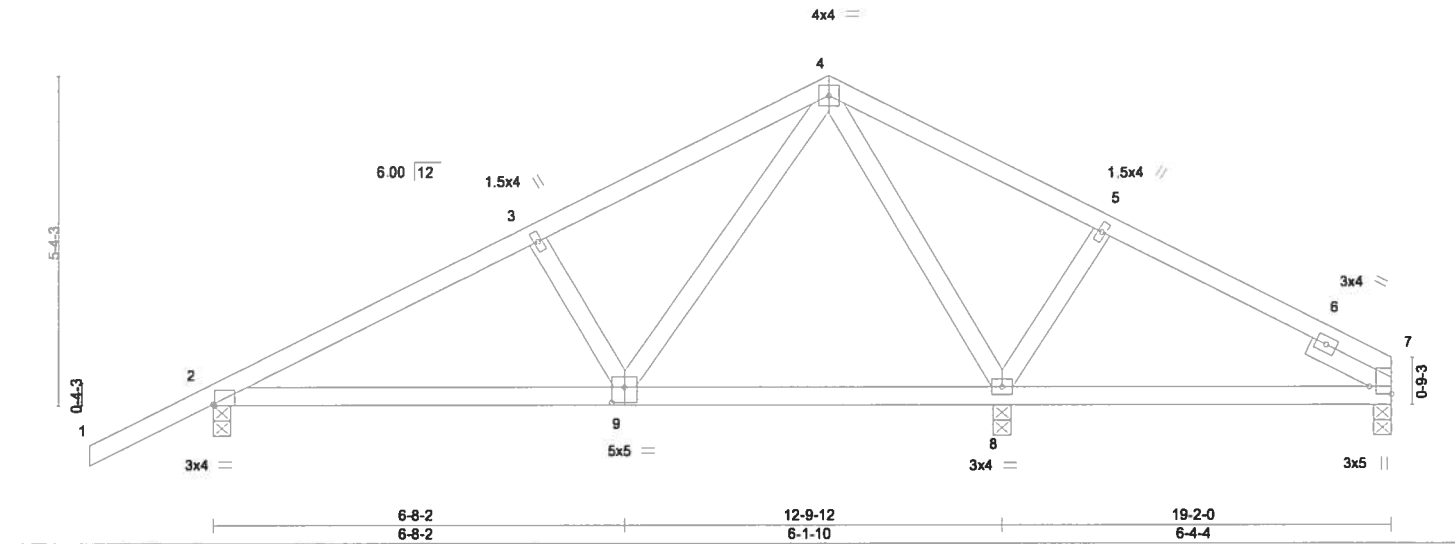
Mayo Truss Company, Inc., Mayo, FL - 32066,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:43 2019 Page 1

ID:31mchhOHyrVRSWwdKMvL?uyMA?e-LESWj?750aqruf3gZZ5C0zUulypK2rjIWx7PPYkyLzfY

-2-0-0	5-3-4	10-0-0	14-5-4	19-2-0
2-0-0	5-3-4	4-8-12	4-5-4	4-8-12

Scale = 1:35.8



LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES		GRIP	
TCLL	20.0	Plate Grip DOL	1.25	TC	0.25	Vert(LL)	-0.04	MT20	244/190		
TCDL	10.0	Lumber DOL	1.25	BC	0.36	Vert(CT)	-0.09				
BCCL	0.0 *	Rep Stress Incr	YES	WB	0.28	Horz(CT)	0.01				
BCDL	10.0	Code FBC2017/TPI2014		Matrix-AS							
								Weight: 92 lb		FT = 0%	

LUMBER-
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.2
SLIDER Right 2x4 SP No.2 1-6-0

BRACING-
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS. (lb/size) 7=236/0-3-8, 2=633/0-3-8, 8=785/0-3-8
Max Horz 2=92(LC 11)
Max Uplift 2=-53(LC 12)
Max Grav 7=264(LC 22), 2=633(LC 1), 8=785(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-723/188, 3-4=-585/205, 5-7=-286/79
BOT CHORD 2-9=-114/601
WEBS 3-9=-294/182, 4-9=-98/462, 4-8=-469/112, 5-8=-282/177

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.
- 6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
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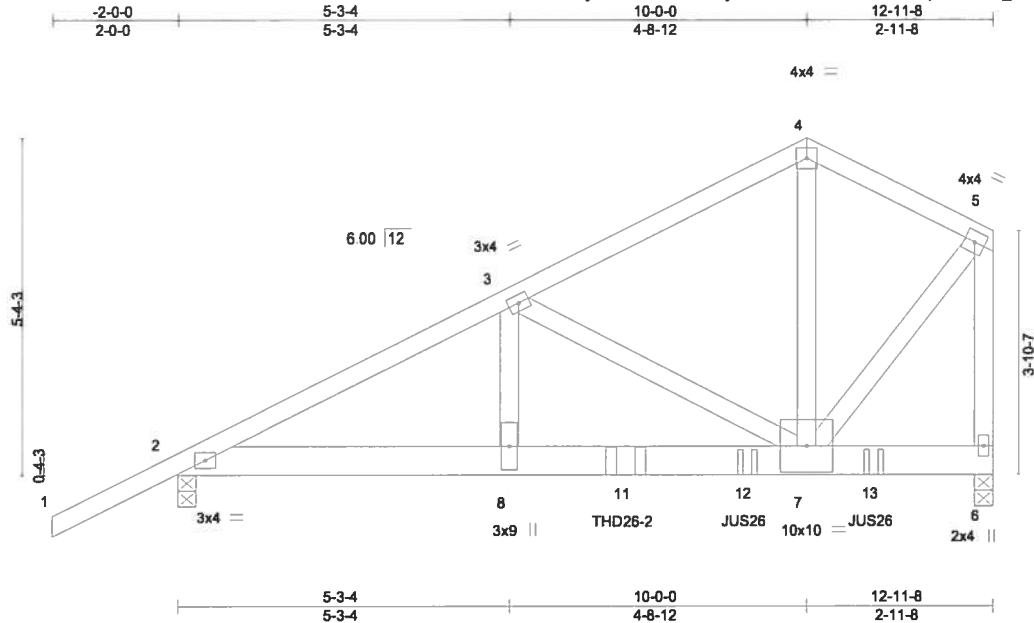
6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581587
Aragon	D5GIR	Common Girder	1	2	Job Reference (optional)	

Mayo Truss Company, Inc., Mayo, FL - 32066,

8.240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:45 2019 Page 1

ID:31mchhOHyrVRSWwdKMvL?uyMA?e-HcZG8h7G6R5cVMqxhWEU2v_eHdxNJcLpPRuVcdyLzFW



Scale = 1/34.9

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.20	Vert(LL)	-0.07	7-8	>999	240	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.69	Vert(CT)	-0.14	7-8	>999	180	
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.37	Horz(CT)	0.02	6	n/a	n/a	
BCDL 10.0	Code FBC2017/TPI2014		Matrix-MS						
								Weight: 168 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP SS
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 5-10-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=2172/0-3-8, 6=3553/0-3-8
Max Horz 2=144(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

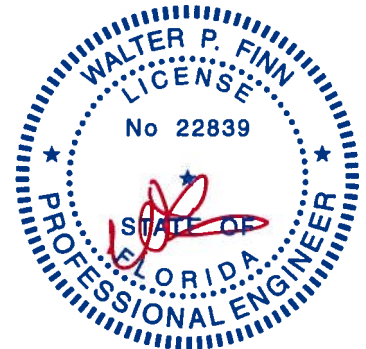
TOP CHORD 2-3=-4561/0, 3-4=-2334/0, 4-5=-2301/0, 5-6=-3634/0
BOT CHORD 2-8=0/4022, 7-8=0/4022
WEBS 3-8=0/1826, 3-7=-2272/0, 4-7=0/1820, 5-7=0/3254

NOTES-

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-4-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Use USP THD26-2 (With 18-16d nails into Girder & 12-10d nails into Truss) or equivalent at 7-1-8 from the left end to connect truss(es) to back face of bottom chord.
- Use USP JUS26 (With 4-10d nails into Girder & 4-10d nails into Truss) or equivalent spaced at 2-0-0 oc max. starting at 9-0-12 from the left end to 11-0-12 to connect truss(es) to back face of bottom chord.
- Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

- Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-60, 4-5=-60, 2-6=-20
Concentrated Loads (lb)
Vert: 11=-2390(B) 12=-1093(B) 13=-1098(B)



Walter P. Finn PE No.22839
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6904 Parke East Blvd. Tampa FL 33610
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6904 Parke East Blvd.
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Job	Truss	Truss Type	Qty	Ply	Aragon	T18581588
Aragon	J1	Jack-Open	25	1	Job Reference (optional)	

Mayo Truss Company, Inc., Mayo, FL - 32066

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:47 2019 Page 1
ID:31mchhOHyrVRSWwdKMvL7uyMA7e-D7h0YM8Xe2LKkg_KoxHy7K3v_Qg?nba5slNchVyLzfU

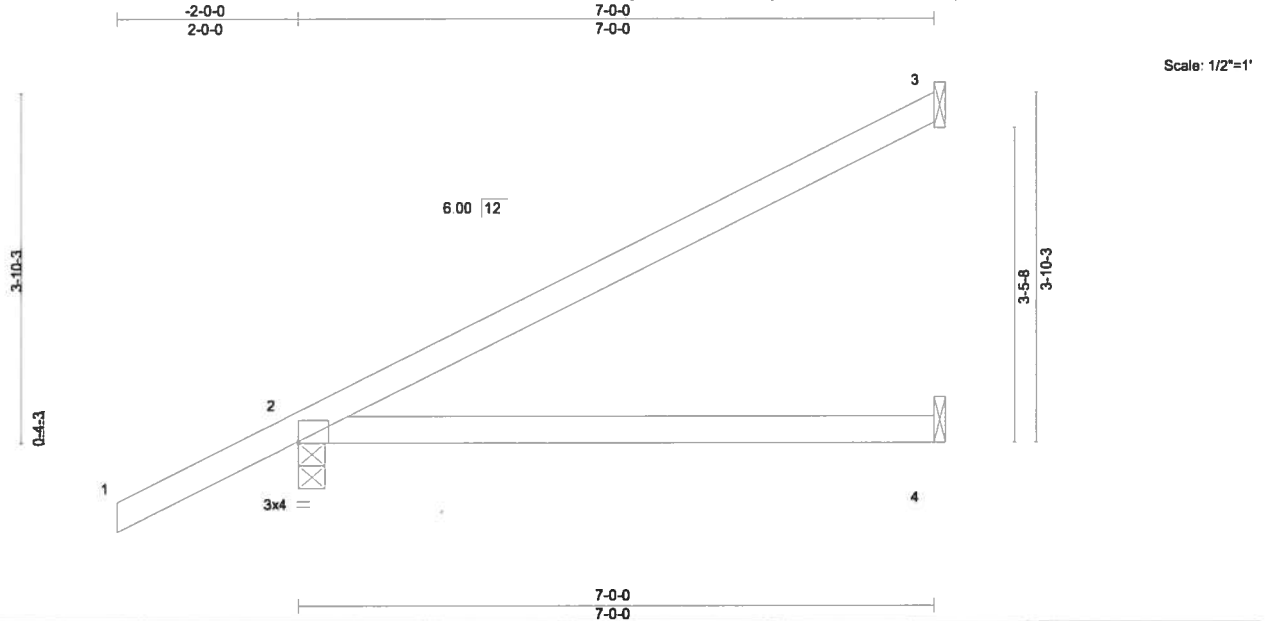


Plate Offsets (X,Y)- [2:Edge,0-0-4]

LOADING (psf)	SPACING-		CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	2-0-0	TC 0.57	Vert(LL)	-0.09	4-7	>969	240	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.49	Vert(CT)	-0.20	4-7	>408	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(CT)	0.00	2	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-AS						Weight: 26 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS. (lb/size) 3=181/Mechanical, 2=415/0-3-8, 4=79/Mechanical
Max Horz 2=120(LC 12)
Max Uplift 3=42(LC 12), 2=37(LC 12)
Max Grav 3=181(LC 1), 2=415(LC 1), 4=123(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.
- 6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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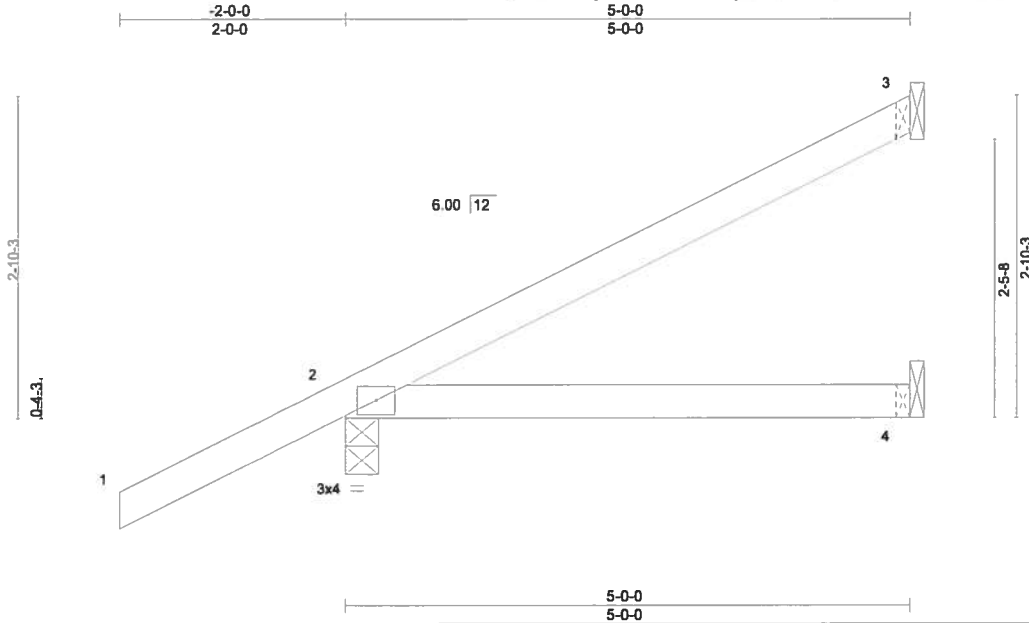
6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581589
Aragon	J2	Jack-Open	4	1	Job Reference (optional)	

Mayo Truss Company, Inc., Mayo, FL - 32066,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:48 2019 Page 1

ID:31mchhOHyrVRSWwdKMvL?uyMA7e-hBFPmi99PMUBMqYWMfoBgXc8dq4NW2qF5P79DxyLzTf



Scale = 1:19.5

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.26	Vert(LL)	-0.02	4-7	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.23	Vert(CT)	-0.05	4-7	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-AS						Weight: 19 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS. (lb/size) 3=121/Mechanical, 2=342/0-3-8, 4=53/Mechanical
Max Horz 2=96(LC 12)
Max Uplift 3=26(LC 12), 2=47(LC 12)
Max Grav 3=121(LC 1), 2=342(LC 1), 4=86(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; End., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.
- 6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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Date:

November 6, 2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

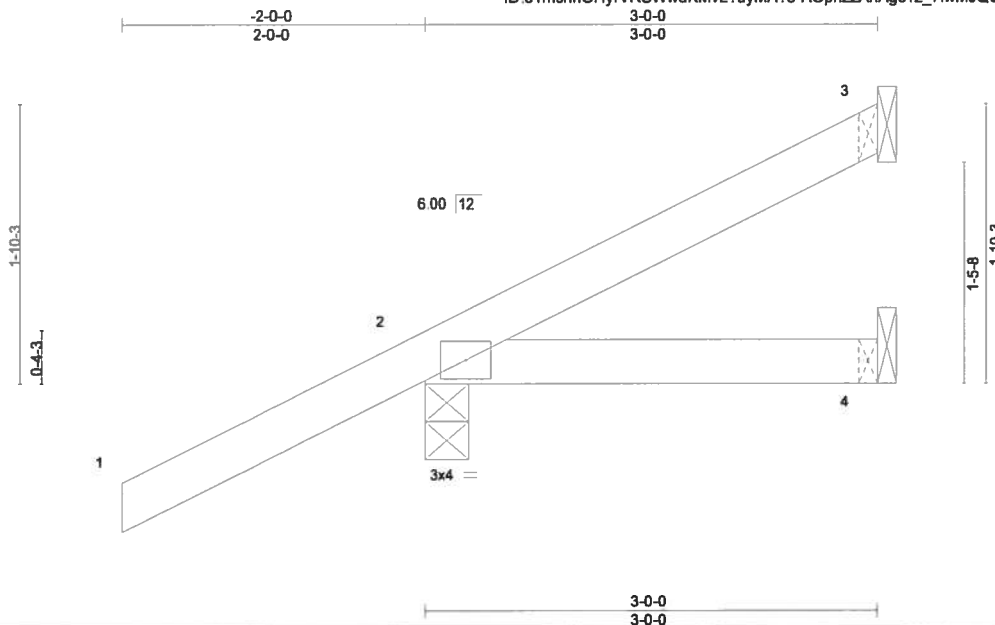


6904 Parke East Blvd.
Tampa, FL 36610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581590
Aragon	J3	Jack-Open	4	1	Job Reference (optional)	

Mayo Truss Company, Inc., Mayo, FL - 32066,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:49 2019 Page 1
ID:31mchhOHyrVRSWwdKMvL?uyMA?e-AOpnz2AnAgc1z_7iwmJQCIBJHESCFV4OK3sjiOyLzfS



Scale = 1:14.6

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	In (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.26	Vert(LL)	-0.00	4-7	>999	240	MT20
TCDL 10.0	Lumber DOL	1.25	BC 0.06	Vert(CT)	-0.01	4-7	>999	180	244/190
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(CT)	0.00	3	n/a	n/a	
BCDL 10.0	Code FBC2017/TPI2014		Matrix-MP						
								Weight: 13 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

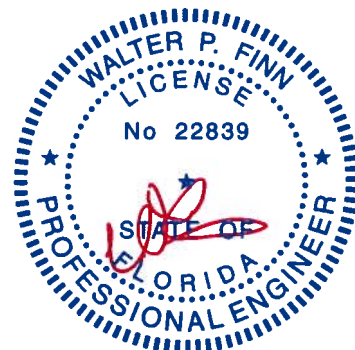
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=57/Mechanical, 2=278/0-3-8, 4=20/Mechanical
Max Horz 2=72(LC 12)
Max Uplift 3=8(LC 12), 2=63(LC 12)
Max Grav 3=57(LC 1), 2=278(LC 1), 4=47(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

November 6, 2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

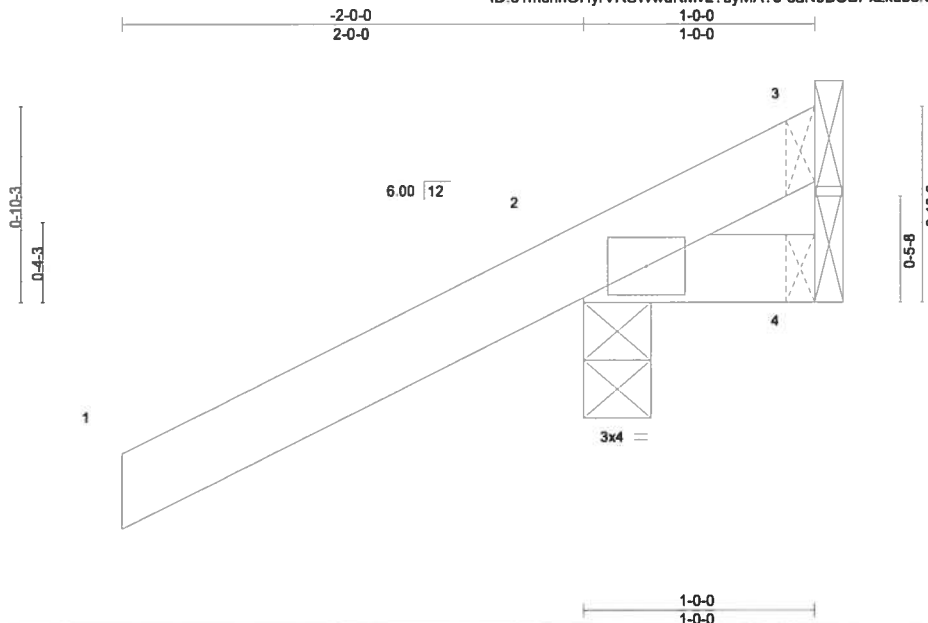


6904 Parke East Blvd.
Tampa, FL 33610

Job	Truss	Truss Type	Qty	Ply	Aragon	T18581591
Aragon	J4	Jack-Open	4	1	Job Reference (optional)	

Mayo Truss Company, Inc., Mayo, FL - 32066,

8 240 s Jul 14 2019 MiTek Industries, Inc. Wed Nov 6 07:24:50 2019 Page 1
ID:31mchhOHyrVRSWwdKMvL7uyMA7e-eaNSBOBPxzkub8ivT3qflyhU1eoa_yKYZjcGHqyLzfR



Scale = 1:9.5

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.26	Vert(LL)	0.00	7	>999	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC 0.05	Vert(CT)	0.00	7	>999		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(CT)	-0.00	4	n/a		
BCDL 10.0	Code FBC2017/TPI2014		Matrix-MP					Weight: 7 lb	FT = 0%

LUMBER-

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=29/Mechanical, 2=281/0-3-8, 4=53/Mechanical
Max Horz 2=48(LC 12)
Max Uplift 3=29(LC 1), 2=113(LC 12), 4=53(LC 1)
Max Grav 3=24(LC 12), 2=281(LC 1), 4=39(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=113.



Walter P. Finn PE No.22839
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

November 6, 2019

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.

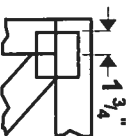
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



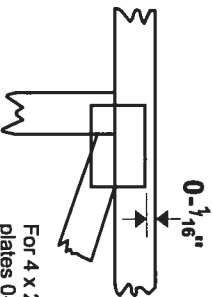
6904 Parke East Blvd.
Tampa, FL 33610

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

— This symbol indicates the required direction of slots in connector plates.

* Plate location details available in **MITek 20/20 software** or upon request.

PLATE SIZE

4 X 4

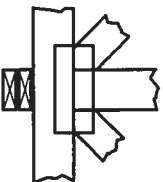
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or L bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TP1: National Design Specification for Metal Plate Connected Wood Truss Construction.

DSB-89:

Design Standard for Bracing.

BCSI:

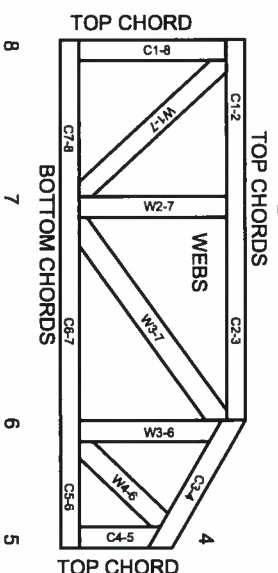
Building Component Safety Information, Guide to Good Practice for Handling.

Installing & Bracing of Metal Plate

Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
 ESR-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 section 6.3. These truss designs rely on lumber values established by others.

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MITek Engineering Reference Sheet: MIL-7473 rev. 10/03/2015



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor l bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and warps at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.

Residential System Sizing Calculation

Summary

Marvin Aragon
316 SW Newport Lane
Lake City, FL 32024

Project Title:
Aragon Res

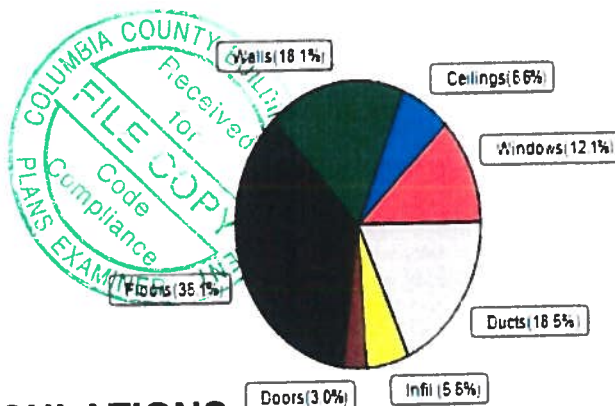
10/23/2019

Location for weather data: Gainesville, FL - Defaults: Latitude(29.7) Altitude(152 ft.) Temp Range(M)			
Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(51gr.)			
Winter design temperature(TMY3 99%)	30 F	Summer design temperature(TMY3 99%)	94 F
Winter setpoint	70 F	Summer setpoint	75 F
Winter temperature difference	40 F	Summer temperature difference	19 F
Total heating load calculation	23271 Btuh	Total cooling load calculation	16706 Btuh
Submitted heating capacity	% of calc Btuh	Submitted cooling capacity	% of calc Btuh
Total (Electric Heat Pump)	100.0 23271	Sensible (SHR = 0.70)	85.8 11694
Heat Pump + Auxiliary(0.0kW)	100.0 23271	Latent	163.2 5012
		Total (Electric Heat Pump)	100.0 16706

WINTER CALCULATIONS

Winter Heating Load (for 1450 sqft)

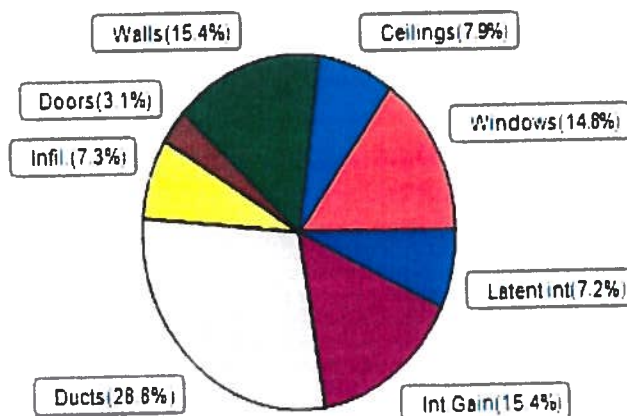
Load component		Load	
Window total	195 sqft	2806	Btuh
Wall total	1189 sqft	4223	Btuh
Door total	38 sqft	695	Btuh
Ceiling total	1523 sqft	1546	Btuh
Floor total	1450 sqft	8394	Btuh
Infiltration	30 cfm	1295	Btuh
Duct loss		4312	Btuh
Subtotal		23271	Btuh
Ventilation	0 cfm	0	Btuh
TOTAL HEAT LOSS		23271	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 1450 sqft)

Load component		Load	
Window total	195 sqft	2478	Btuh
Wall total	1189 sqft	2567	Btuh
Door total	38 sqft	521	Btuh
Ceiling total	1523 sqft	1314	Btuh
Floor total		0	Btuh
Infiltration	22 cfm	461	Btuh
Internal gain		2580	Btuh
Duct gain		3712	Btuh
Sens. Ventilation	0 cfm	0	Btuh
Blower Load		0	Btuh
Total sensible gain		13634	Btuh
Latent gain(ducts)		1106	Btuh
Latent gain(infiltration)		766	Btuh
Latent gain(ventilation)		0	Btuh
Latent gain(internal/occupants/other)		1200	Btuh
Total latent gain		3072	Btuh
TOTAL HEAT GAIN		16706	Btuh



8th Edition

EnergyGauge® System Sizing

PREPARED BY: _____

DATE: _____

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Marvin Aragon
316 SW Newport Lane
Lake City, FL 32024

Project Title:
Aragon Res
Building Type: User

10/23/2019

Reference City: Gainesville, FL (Defaults) Winter Temperature Difference: 40.0 F (TMY3 99%)

Component Loads for Whole House								
Window	Panes/Type	Frame	U	Orientation	Area(sqft)	X	HTM=	Load
1	2, NFRC 0.25	Vinyl	0.36	S	30.0		14.4	432 Btuh
2	2, NFRC 0.25	Vinyl	0.36	S	15.0		14.4	216 Btuh
3	2, NFRC 0.25	Vinyl	0.36	E	6.0		14.4	86 Btuh
4	2, NFRC 0.25	Vinyl	0.36	N	30.0		14.4	432 Btuh
5	2, NFRC 0.25	Vinyl	0.36	N	60.0		14.4	864 Btuh
6	2, NFRC 0.25	Vinyl	0.36	N	35.6		14.4	512 Btuh
7	2, NFRC 0.25	Vinyl	0.36	N	15.0		14.4	216 Btuh
8	2, NFRC 0.25	Vinyl	0.36	W	3.3		14.4	48 Btuh
Window Total					194.9(sqft)			2806 Btuh
Walls	Type	Ornt.	Ueff.	R-Value (Cav/Sh)	Area	X	HTM=	Load
1	Frame - Wood	- Ext	(0.089)	13.0/0.0	74		3.55	263 Btuh
2	Frame - Wood	- Ext	(0.089)	13.0/0.0	44		3.55	156 Btuh
3	Frame - Wood	- Ext	(0.089)	13.0/0.0	28		3.55	99 Btuh
4	Frame - Wood	- Ext	(0.089)	13.0/0.0	44		3.55	156 Btuh
5	Frame - Wood	- Ext	(0.089)	13.0/0.0	72		3.55	254 Btuh
6	Frame - Wood	- Adj	(0.089)	13.0/0.0	56		3.55	197 Btuh
7	Frame - Wood	- Adj	(0.089)	13.0/0.0	160		3.55	568 Btuh
8	Frame - Wood	- Ext	(0.089)	13.0/0.0	190		3.55	675 Btuh
9	Frame - Wood	- Ext	(0.089)	13.0/0.0	83		3.55	296 Btuh
10	Frame - Wood	- Ext	(0.089)	13.0/0.0	96		3.55	342 Btuh
11	Frame - Wood	- Ext	(0.089)	13.0/0.0	77		3.55	273 Btuh
12	Frame - Wood	- Ext	(0.089)	13.0/0.0	265		3.55	942 Btuh
Wall Total					1189(sqft)			4223 Btuh
Doors	Type	Storm	Ueff.		Area	X	HTM=	Load
1	Insulated - Exterior, n		(0.460)		20		18.4	368 Btuh
2	Insulated - Garage, n		(0.460)		18		18.4	327 Btuh
Door Total					38(sqft)			695Btuh
Ceilings	Type/Color/Surface		Ueff.	R-Value	Area	X	HTM=	Load
1	Vented Attic/L/Shing		(0.025)	38.0/0.0	1523		1.0	1546 Btuh
Ceiling Total					1523(sqft)			1546Btuh
Floors	Type		Ueff.	R-Value	Size	X	HTM=	Load
1	Slab On Grade		(1.180)	0.0	177.8 ft(perim.)		47.2	8394 Btuh
Floor Total					1450 sqft			8394 Btuh
	Envelope Subtotal:							17663 Btuh
Infiltration	Type	Wholehouse	ACH	Volume(cuft)	Wall Ratio	CFM=		Load
	Natural		0.15	11600	1.00	29.6		1295 Btuh
Duct load	Average sealed, R6.0, Supply(Att), Return(Att) (DLM of 0.227)							4312 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Marvin Aragon
316 SW Newport Lane
Lake City, FL 32024

Project Title:
Aragon Res
Building Type: User

10/23/2019

All Zones	Sensible Subtotal All Zones	23271 Btuh
------------------	------------------------------------	-------------------

WHOLE HOUSE TOTALS

Totals for Heating	Subtotal Sensible Heat Loss Ventilation Sensible Heat Loss Total Heat Loss	23271 Btuh 0 Btuh 23271 Btuh
---------------------------	--	------------------------------------

EQUIPMENT

1. Electric Heat Pump	#	23271 Btuh
-----------------------	---	------------

Key: Window types - NFRC (Requires U-Factor and Shading coefficient(SHGC) of glass as numerical values)
or - Glass as 'Clear' or 'Tint' (Uses U-Factor and SHGC defaults)
U - (Window U-Factor)
HTM - (ManualJ Heat Transfer Multiplier)



Version 8

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Marvin Aragon
316 SW Newport Lane
Lake City, FL 32024

Project Title:
Aragon Res

10/23/2019

Reference City: Gainesville, FL

Temperature Difference: 19.0F(TMY3 99%) Humidity difference: 51gr.

Component Loads for Whole House

Window	Type*						Overhang		Window Area(sqft)			HTM		Load	
	Panes	SHGC	U	InSh	IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2 NFRC	0.25, 0.36	No	No	S		8.0ft.	0.5ft.	30.0	30.0	0.0	12	14	363	Btuh
2	2 NFRC	0.25, 0.36	No	No	S		1.5ft.	0.5ft.	15.0	15.0	0.0	12	14	181	Btuh
3	2 NFRC	0.25, 0.36	No	No	E		1.5ft.	0.5ft.	6.0	1.5	4.5	12	31	158	Btuh
4	2 NFRC	0.25, 0.36	No	No	N		1.5ft.	0.5ft.	30.0	0.0	30.0	12	12	363	Btuh
5	2 NFRC	0.25, 0.36	No	No	N		13.5f	0.5ft.	60.0	0.0	60.0	12	12	726	Btuh
6	2 NFRC	0.25, 0.36	No	No	N		13.5f	0.5ft.	35.6	0.0	35.6	12	12	430	Btuh
7	2 NFRC	0.25, 0.36	No	No	N		1.5ft.	0.5ft.	15.0	0.0	15.0	12	12	181	Btuh
8	2 NFRC	0.25, 0.36	No	No	W		1.5ft.	0.5ft.	3.3	1.5	1.8	12	31	75	Btuh
Window Total									195 (sqft)					2478 Btuh	
Walls	Type							U-Value	R-Value	Area(sqft)	HTM		Load		
								Cav/Sheath							
1	Frame - Wood - Ext						0.09	13.0/0.0	74.0	2.3		167 Btuh			
2	Frame - Wood - Ext						0.09	13.0/0.0	44.0	2.3		100 Btuh			
3	Frame - Wood - Ext						0.09	13.0/0.0	28.0	2.3		63 Btuh			
4	Frame - Wood - Ext						0.09	13.0/0.0	44.0	2.3		100 Btuh			
5	Frame - Wood - Ext						0.09	13.0/0.0	71.7	2.3		162 Btuh			
6	Frame - Wood - Adj						0.09	13.0/0.0	55.6	1.7		94 Btuh			
7	Frame - Wood - Adj						0.09	13.0/0.0	160.0	1.7		270 Btuh			
8	Frame - Wood - Ext						0.09	13.0/0.0	190.0	2.3		430 Btuh			
9	Frame - Wood - Ext						0.09	13.0/0.0	83.3	2.3		189 Btuh			
10	Frame - Wood - Ext						0.09	13.0/0.0	96.4	2.3		218 Btuh			
11	Frame - Wood - Ext						0.09	13.0/0.0	77.0	2.3		174 Btuh			
12	Frame - Wood - Ext						0.09	13.0/0.0	265.3	2.3		601 Btuh			
Wall Total									1189 (sqft)					2567 Btuh	
Doors	Type									Area (sqft)	HTM		Load		
	1 Insulated - Exterior								20.0	13.8		276 Btuh			
	2 Insulated - Garage								17.8	13.8		245 Btuh			
Door Total									38 (sqft)					521 Btuh	
Ceilings	Type/Color/Surface						U-Value	R-Value	Area(sqft)	HTM		Load			
	1 Vented AtticLight/Shingle/RB						0.025	38.0/0.0	1522.5	0.86		1314 Btuh			
Ceiling Total									1523 (sqft)					1314 Btuh	
Floors	Type									Size	HTM		Load		
	1 Slab On Grade							0.0	1450 (ft-perimeter)	0.0		0 Btuh			
Floor Total									1450.0 (sqft)					0 Btuh	
Envelope Subtotal:														6880 Btuh	

Manual J Summer Calculations

Residential Load - Component Details (continued)

Marvin Aragon
316 SW Newport Lane
Lake City, FL 32024

Project Title:
Aragon Res

Climate:FL_GAINESVILLE_REGIONAL_A

10/23/2019

Infiltration	Type Natural	Average ACH 0.11	Volume(cuft) 11600	Wall Ratio 1	CFM= 22.2	Load 461 Btuh
Internal gain		Occupants 6	Btuh/occupant X 230	Appliance +	1200	Load 2580 Btuh
	Sensible Envelope Load:					9922 Btuh
Duct load	Average sealed,Supply(R6.0-Attic), Return(R6.0-Attic)				(DGM of 0.374)	3712 Btuh
	Sensible Load All Zones					13634 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Marvin Aragon
316 SW Newport Lane
Lake City, FL 32024

Project Title:
Aragon Res

Climate: FL_GAINESVILLE_REGIONAL_A

10/23/2019

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	Sensible Envelope Load All Zones	9922 Btuh
	Sensible Duct Load	3712 Btuh
	Total Sensible Zone Loads	13634 Btuh
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	13634 Btuh
	Latent infiltration gain (for 51 gr. humidity difference)	766 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	1106 Btuh
	Latent occupant gain (6.0 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	3072 Btuh
	TOTAL GAIN	16706 Btuh

EQUIPMENT

1. Central Unit	#	16706 Btuh
-----------------	---	------------

*Key: Window types (Panels - Number and type of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value)
(U - Window U-Factor)
(InSh - Interior shading device: none(No), Blinds(B), Draperies(D) or Roller Shades(R))
- For Blinds: Assume medium color, half closed
For Draperies: Assume medium weave, half closed
For Roller shades: Assume translucent, half closed
(IS - Insect screen: none(N), Full(F) or Half(½))
(Ornt - compass orientation)



Version 8

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Business and Professional Regulation - Residential Performance Method

Project Name: Aragon Res
 Street: 316 SW Newport Lane
 City, State, Zip: Lake City, FL, 32024
 Owner: Marvin Aragon
 Design Location: FL, Gainesville

Builder Name:
 Permit Office: Columbia County
 Permit Number:
 Jurisdiction:
 County: Columbia (Florida Climate Zone 2)

1. New construction or existing New (From Plans)
 2. Single family or multiple family Single-family
 3. Number of units, if multiple family 1
 4. Number of Bedrooms 3
 5. Is this a worst case? No
 6. Conditioned floor area above grade (ft²) 1450
 Conditioned floor area below grade (ft²) 0

7. Windows (194.9 sqft.) Description Area
 a. U-Factor: Dbl, U=0.36 194.89 ft²
 SHGC: SHGC=0.25
 b. U-Factor: N/A ft²
 SHGC:
 c. U-Factor: N/A ft²
 SHGC:
 d. U-Factor: N/A ft²
 SHGC:
 Area Weighted Average Overhang Depth: 8.384 ft.
 Area Weighted Average SHGC: 0.250

8. Floor Types (1450.0 sqft.) Insulation Area
 a. Slab-On-Grade Edge Insulation R=0.0 1450.00 ft²
 b. N/A R= ft²
 c. N/A R= ft²

9. Wall Types (1422.0 sqft.) Insulation Area
 a. Frame - Wood, Exterior R=13.0 1188.70 ft²
 b. Frame - Wood, Adjacent R=13.0 233.33 ft²
 c. N/A R= ft²
 d. N/A R= ft²
 10. Ceiling Types (1522.5 sqft.) Insulation Area
 a. Under Attic (Vented) R=38.0 1522.50 ft²
 b. N/A R= ft²
 c. N/A R= ft²
 11. Ducts R ft²
 a. Sup: Attic, Ret: Attic, AH: Garage 6 362.5

12. Cooling systems kBtu/hr Efficiency
 a. Central Unit 16.7 SEER: 14.00

13. Heating systems kBtu/hr Efficiency
 a. Electric Heat Pump 23.8 HSPF: 8.20

14. Hot water systems
 a. Electric Cap: 50 gallons
 b. Conservation features EF: 0.920
 None

15. Credits CV, Pstat

Glass/Floor Area: 0.134

Total Proposed Modified Loads: 40.38

Total Baseline Loads: 41.88

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: 10/13/2019
 DATE: 10/13/2019

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____
 DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____
 DATE: _____



- Compliance requires certification by the air handler unit manufacturer that the air handler enclosure qualifies as certified factory-sealed in accordance with R403.3.2.1.

- Compliance requires an Air Barrier and Insulation Inspection Checklist in accordance with R402.4.1.1 and this project requires an envelope leakage test report with envelope leakage no greater than 5.00 ACH50 (R402.4.1.2).

INPUT SUMMARY CHECKLIST REPORT

PROJECT

Title:	Aragon Res	Bedrooms:	3	Address Type:	Street Address
Building Type:	User	Conditioned Area:	1450	Lot #	
Owner Name:	Marvin Aragon	Total Stories:	1	Block/Subdivision:	
# of Units:	1	Worst Case:	No	PlatBook:	
Builder Name:		Rotate Angle:	0	Street:	316 SW Newport Lane
Permit Office:	Columbia County	Cross Ventilation:	Yes	County:	Columbia
Jurisdiction:		Whole House Fan:	No	City, State, Zip:	Lake City , FL , 32024
Family Type:	Single-family				
New/Existing:	New (From Plans)				
Comment:					

CLIMATE

✓	Design Location	TMY Site	Design Temp 97.5 %	2.5 %	Int Design Temp Winter	Summer	Heating Degree Days	Design Moisture	Daily Temp Range
_____	FL, Gainesville	FL_GAINESVILLE_REGI	32	92	70	75	1305.5	51	Medium

BLOCKS

Number	Name	Area	Volume
1	Block1	1450	11600

SPACES

Number	Name	Area	Volume	Kitchen	Occupants	Bedrooms	Infil ID	Finished	Cooled	Heated
1	Main	1450	11600	Yes	6	3	1	Yes	Yes	Yes

FLOORS

✓	#	Floor Type	Space	Perimeter	R-Value	Area		Tile	Wood	Carpet
_____	1	Slab-On-Grade Edge Insulation	Main	177.83 ft	0	1450 ft²	----	0	0	1

ROOF

✓	#	Type	Materials	Roof Area	Gable Area	Roof Color	Rad Barr	Solar Absor.	SA Tested	Emitt Tested	Emitt Tested	Deck Insul.	Pitch (deg)
_____	1	Hip	Composition shingles	1622 ft²	0 ft²	Medium	Y	0.96	No	0.9	No	0	26.6

ATTIC

✓	#	Type	Ventilation	Vent Ratio (1 in)	Area	RBS	IRCC
_____	1	Full attic	Vented	300	1450 ft²	Y	N

CEILING

✓	#	Ceiling Type	Space	R-Value	Ins Type	Area	Framing Frac	Truss Type
_____	1	Under Attic (Vented)	Main	38	Double Batt	1522.5 ft²	0.11	Wood

INPUT SUMMARY CHECKLIST REPORT

WALLS

✓ #	Omt	Adjacent To	Wall Type	Space	Cavity R-Value	Width Ft	In	Height Ft	In	Area	Sheathing R-Value	Framing Fraction	Solar Absor.	Below Grade%
1	S	Exterior	Frame - Wood	Main	13	13		8		104.0 ft²		0.23	0.75	0
2	E	Exterior	Frame - Wood	Main	13	5	6	8		44.0 ft²		0.23	0.75	0
3	S	Exterior	Frame - Wood	Main	13	6		8		48.0 ft²		0.23	0.75	0
4	W	Exterior	Frame - Wood	Main	13	5	6	8		44.0 ft²		0.23	0.75	0
5	S	Exterior	Frame - Wood	Main	13	10	10	8		86.7 ft²		0.23	0.75	0
6	E	Garage	Frame - Wood	Main	13	9	2	8		73.3 ft²		0.23	0.75	0
7	S	Garage	Frame - Wood	Main	13	20		8		160.0 ft²		0.23	0.75	0
8	E	Exterior	Frame - Wood	Main	13	24	6	8		196.0 ft²		0.23	0.75	0
9	N	Exterior	Frame - Wood	Main	13	14	2	8		113.3 ft²		0.23	0.75	0
10	N	Exterior	Frame - Wood	Main	13	24		8		192.0 ft²		0.23	0.75	0
11	N	Exterior	Frame - Wood	Main	13	11	6	8		92.0 ft²		0.23	0.75	0
12	W	Exterior	Frame - Wood	Main	13	33	7	8		268.7 ft²		0.23	0.75	0

DOORS

✓ #	Omt	Door Type	Space	Storms	U-Value	Width Ft	In	Height Ft	In	Area
1	S	Insulated	Main	None	.46	3		6	8	20 ft²
2	E	Insulated	Main	None	.46	2	8	6	8	17.8 ft²

WINDOWS

Orientation shown is the entered, Proposed orientation.

✓ #	Omt	Wall ID	Frame	Panes	NFRC	U-Factor	SHGC	Imp	Area	Overhang Depth	Separation	Int Shade	Screening
1	S	1	Vinyl	Low-E Double	Yes	0.36	0.25	N	30.0 ft²	8 ft 0 in	0 ft 6 in	None	None
2	S	5	Vinyl	Low-E Double	Yes	0.36	0.25	N	15.0 ft²	1 ft 6 in	0 ft 6 in	None	None
3	E	8	Vinyl	Low-E Double	Yes	0.36	0.25	N	6.0 ft²	1 ft 6 in	0 ft 6 in	None	None
4	N	9	Vinyl	Low-E Double	Yes	0.36	0.25	N	30.0 ft²	1 ft 6 in	0 ft 6 in	None	None
5	N	10	Vinyl	Low-E Double	Yes	0.36	0.25	N	60.0 ft²	13 ft 6 in	0 ft 6 in	None	None
6	N	10	Vinyl	Low-E Double	Yes	0.36	0.25	N	35.6 ft²	13 ft 6 in	0 ft 6 in	None	None
7	N	11	Vinyl	Low-E Double	Yes	0.36	0.25	N	15.0 ft²	1 ft 6 in	0 ft 6 in	None	None
8	W	12	Vinyl	Low-E Double	Yes	0.36	0.25	N	3.3 ft²	1 ft 6 in	0 ft 6 in	None	None

GARAGE

✓ #	Floor Area	Ceiling Area	Exposed Wall Perimeter	Avg. Wall Height	Exposed Wall Insulation
1	400 ft²	400 ft²	50.2 ft	8 ft	1

INFILTRATION

#	Scope	Method	SLA	CFM 50	ELA	EqLA	ACH	ACH 50
1	Wholehouse	Proposed ACH(50)	.000254	966.7	53.07	99.8	.0956	5

INPUT SUMMARY CHECKLIST REPORT

HEATING SYSTEM

<input checked="" type="checkbox"/>	#	System Type	Subtype	Efficiency	Capacity	Block	Ducts
<input type="checkbox"/>	1	Electric Heat Pump/	None	HSPF:8.2	23.27 kBtu/hr	1	sys#1

COOLING SYSTEM

<input checked="" type="checkbox"/>	#	System Type	Subtype	Efficiency	Capacity	Air Flow	SHR	Block	Ducts
<input type="checkbox"/>	1	Central Unit/	None	SEER: 14	16.71 kBtu/hr	510 cfm	0.7	1	sys#1

HOT WATER SYSTEM

<input checked="" type="checkbox"/>	#	System Type	SubType	Location	EF	Cap	Use	SetPnt	Conservation
<input type="checkbox"/>	1	Electric	None	Garage	0.92	50 gal	40 gal	120 deg	None

SOLAR HOT WATER SYSTEM

<input checked="" type="checkbox"/>	FSEC Cert #	Company Name	System Model #	Collector Model #	Collector Area	Storage Volume	FEF
<input type="checkbox"/>	None	None			ft ²		

DUCTS

<input checked="" type="checkbox"/>	#	--- Supply --- Location	R-Value	Area	--- Return --- Location	Area	Leakage Type	Air Handler	CFM 25 TOT	CFM25 OUT	QN	RLF	HVAC # Heat	Cool
<input type="checkbox"/>	1	Attic	6	362.5 ft ²	Attic	72.5 ft ²	Default Leakage	Garage	(Default) c	(Default) c			1	1

TEMPERATURES

Programable Thermostat: Y

Ceiling Fans:

Cooling	<input checked="" type="checkbox"/> Jan	<input checked="" type="checkbox"/> Feb	<input checked="" type="checkbox"/> Mar	<input type="checkbox"/> Apr	<input type="checkbox"/> May	<input checked="" type="checkbox"/> Jun	<input checked="" type="checkbox"/> Jul	<input checked="" type="checkbox"/> Aug	<input checked="" type="checkbox"/> Sep	<input type="checkbox"/> Oct	<input type="checkbox"/> Nov	<input checked="" type="checkbox"/> Dec
Heating	<input checked="" type="checkbox"/> Jan	<input checked="" type="checkbox"/> Feb	<input checked="" type="checkbox"/> Mar	<input type="checkbox"/> Apr	<input type="checkbox"/> May	<input type="checkbox"/> Jun	<input type="checkbox"/> Jul	<input type="checkbox"/> Aug	<input type="checkbox"/> Sep	<input checked="" type="checkbox"/> Oct	<input checked="" type="checkbox"/> Nov	<input type="checkbox"/> Dec
Venting	<input type="checkbox"/> Jan	<input type="checkbox"/> Feb	<input type="checkbox"/> Mar	<input type="checkbox"/> Apr	<input type="checkbox"/> May	<input type="checkbox"/> Jun	<input type="checkbox"/> Jul	<input type="checkbox"/> Aug	<input type="checkbox"/> Sep	<input checked="" type="checkbox"/> Oct	<input checked="" type="checkbox"/> Nov	<input type="checkbox"/> Dec

Thermostat Schedule: HERS 2006 Reference

Hours

Schedule Type		1	2	3	4	5	6	7	8	9	10	11	12
Cooling (WD)	AM	78	78	78	78	78	78	78	78	80	80	80	80
	PM	80	80	78	78	78	78	78	78	78	78	78	78
Cooling (WEH)	AM	78	78	78	78	78	78	78	78	78	78	78	78
	PM	78	78	78	78	78	78	78	78	78	78	78	78
Heating (WD)	AM	66	66	66	66	66	68	68	68	68	68	68	68
	PM	68	68	68	68	68	68	68	68	68	68	66	66
Heating (WEH)	AM	66	66	66	66	66	68	68	68	68	68	68	68
	PM	68	68	68	68	68	68	68	68	68	68	66	66

MASS

Mass Type	Area	Thickness	Furniture Fraction	Space
Default(8 lbs/sq.ft.	0 ft ²	0 ft	0.3	Main

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD**ESTIMATED ENERGY PERFORMANCE INDEX* =96****The lower the Energy Performance Index, the more efficient the home.**

1. New home or, addition	1. <u>New (From Plans)</u>	12. Ducts, location & insulation level
2. Single-family or multiple-family	2. <u>Single-family</u>	a) Supply ducts R <u>6.0</u>
3. No. of units (if multiple-family)	3. <u>1</u>	b) Return ducts R <u>6.0</u>
4. Number of bedrooms	4. <u>3</u>	c) AHU location <u>Garage</u>
5. Is this a worst case? (yes/no)	5. <u>No</u>	13. Cooling system: Capacity <u>16.7</u>
6. Conditioned floor area (sq. ft.)	6. <u>1450</u>	a) Split system SEER <u> </u>
7. Windows, type and area		b) Single package SEER <u> </u>
a) U-factor:(weighted average)	7a. <u>0.360</u>	c) Ground/water source SEER/COP <u> </u>
b) Solar Heat Gain Coefficient (SHGC)	7b. <u>0.250</u>	d) Room unit/PTAC EER <u> </u>
c) Area	7c. <u>194.9</u>	e) Other <u>14.0</u>
8. Skylights		14. Heating system: Capacity <u>23.3</u>
a) U-factor:(weighted average)	8a. <u>NA</u>	a) Split system heat pump HSPF <u> </u>
b) Solar Heat Gain Coefficient (SHGC)	8b. <u>NA</u>	b) Single package heat pump HSPF <u> </u>
9. Floor type, insulation level:		c) Electric resistance COP <u> </u>
a) Slab-on-grade (R-value)	9a. <u>0.0</u>	d) Gas furnace, natural gas AFUE <u> </u>
b) Wood, raised (R-value)	9b. <u> </u>	e) Gas furnace, LPG AFUE <u> </u>
c) Concrete, raised (R-value)	9c. <u> </u>	f) Other <u>8.20</u>
10. Wall type and insulation:		15. Water heating system
A. Exterior:		a) Electric resistance EF <u>0.92</u>
1. Wood frame (Insulation R-value)	10A1. <u>13.0</u>	b) Gas fired, natural gas EF <u> </u>
2. Masonry (Insulation R-value)	10A2. <u> </u>	c) Gas fired, LPG EF <u> </u>
B. Adjacent:		d) Solar system with tank EF <u> </u>
1. Wood frame (Insulation R-value)	10B1. <u>13.0</u>	e) Dedicated heat pump with tank EF <u> </u>
2. Masonry (Insulation R-value)	10B2. <u> </u>	f) Heat recovery unit HeatRec% <u> </u>
11. Ceiling type and insulation level		g) Other <u> </u>
a) Under attic	11a. <u>38.0</u>	16. HVAC credits claimed (Performance Method)
b) Single assembly	11b. <u> </u>	a) Ceiling fans <u> </u>
c) Knee walls/skylight walls	11c. <u> </u>	b) Cross ventilation <u>Yes</u>
d) Radiant barrier installed	11d. <u>Yes</u>	c) Whole house fan <u>No</u>
		d) Multizone cooling credit <u> </u>
		e) Multizone heating credit <u> </u>
		f) Programmable thermostat <u>Yes</u>

*Label required by Section R303.1.3 of the Florida Building Code, Energy Conservation, if not DEFAULT.

I certify that this home has complied with the Florida Building Code, Energy Conservation, through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL display card will be completed based on installed code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: 316 SW Newport Lane City/FL Zip: Lake City, FL 32024

Envelope Leakage Test Report (Blower Door Test)

Residential Prescriptive, Performance or ERI Method Compliance

2017 Florida Building Code, Energy Conservation, 6th Edition

Jurisdiction:

Permit #:

Job Information

Builder:

Community:

Lot: NA

Address: 316 SW Newport Lane

City: Lake City

State: FL

Zip: 32024

Air Leakage Test Results *Passing results must meet either the Performance, Prescriptive, or ERI Method*



PRESCRIPTIVE METHOD-The building or dwelling unit shall be tested and verified as having an air leakage rate of not exceeding 7 air changes per hour at a pressure of 0.2 inch w.g. (50 Pascals) in Climate Zones 1 and 2.



PERFORMANCE or ERI METHOD-The building or dwelling unit shall be tested and verified as having an air leakage rate of not exceeding the selected ACH(50) value, as shown on Form R405-2017 (Performance) or R406-2017 (ERI), section labeled as infiltration, sub-section ACH50.

ACH(50) specified on Form R405-2017-Energy Calc (Performance) or R406-2017 (ERI):

5.000

$$\frac{\text{CFM}(50)}{\text{Building Volume}} \times 60 \div 11600 = \text{ACH}(50)$$



PASS



When ACH(50) is less than 3, Mechanical Ventilation installation must be verified by building department.

Method for calculating building volume:



Retrieved from architectural plans



Code software calculated



Field measured and calculated

R402.4.1.2 Testing. Testing shall be conducted in accordance with ANSI/RESNET/ICC 380 and reported at a pressure of 0.2 inch w.g. (50 Pascals). Testing shall be conducted by either individuals as defined in Section 553.993(5) or *(Florida Statutes)* or individuals licensed as set forth in Section 489.105(3)(f), (g), or (i) or an approved third party. A written report of the results of the test shall be signed by the party conducting the test and provided to the *code official*. Testing shall be performed at any time after creation of all penetrations of the *building thermal envelope*.

During testing:

1. Exterior windows and doors, fireplace and stove doors shall be closed, but not sealed, beyond the intended weatherstripping or other infiltration control measures.
2. Dampers including exhaust, intake, makeup air, back draft and flue dampers shall be closed, but not sealed beyond intended infiltration control measures.
3. Interior doors, if installed at the time of the test, shall be open.
4. Exterior doors for continuous ventilation systems and heat recovery ventilators shall be closed and sealed.
5. Heating and cooling systems, if installed at the time of the test, shall be turned off.
6. Supply and return registers, if installed at the time of the test, shall be fully open.

Testing Company

Company Name: _____ Phone: _____

I hereby verify that the above Air Leakage results are in accordance with the 2017 6th Edition Florida Building Code Energy Conservation requirements according to the compliance method selected above.

Signature of Tester: _____ Date of Test: _____

Printed Name of Tester: _____

License/Certification #: _____ Issuing Authority: _____