Columbia County Building Permit PERMIT This Permit Expires One Year From the Date of Issue 000025005 **PHONE** 397-0545 TRENT GIEBEIG APPLICANT FL32024 SE HOLLY TERR LAKE CITY ADDRESS 697 **PHONE** 752-0791 OWNER PETER & HOLLY GIEBEIG FL 32024 SW VANN COURT LAKE CITY **ADDRESS** 194 397-0545 **PHONE** CONTRACTOR TRENT GIEBEIG CONSTRUCTION 247S, TR ON MAYFAIR, TR ON VANN COURT, 4TH ON LEFT LOCATION OF PROPERTY ESTIMATED COST OF CONSTRUCTION 146950.00 TYPE DEVELOPMENT SFD, UTILITY TOTAL AREA 3806.00 **HEIGHT STORIES** HEATED FLOOR AREA 2939.00 FLOOR SLAB **ROOF PITCH** 6/12 **FOUNDATION** CONC WALLS FRAMED MAX. HEIGHT 21 RSF-2 LAND USE & ZONING **SIDE** 10.00 STREET-FRONT 25.00 15.00 Minimum Set Back Requirments: NO. EX.D.U. FLOOD ZONE DEVELOPMENT PERMIT NO. 11-4S-16-02911-312 **SUBDIVISION MAYFAIR** PARCEL ID PHASE UNIT LOT 12 BLOCK 000001210 RR28281153 Culvert Permit No. **Culvert Waiver** Contractor's License Number Applicant/Owner/Q htractor **CULVERT** 06-0770-N BK LU & Zoning checked by Approved for Issuance New Resident **Driveway Connection** Septic Tank Number COMMENTS: ONE FOOT ABOVE THE ROAD, NOC ON FILE Check # or Cash FOR BUILDING & ZONING DEPARTMENT ONLY (footer/Slab) Temporary Power Monolithic date/app. by date/app. by Under slab rough-in plumbing Sheathing/Nailing date/app. by date/app. by date/app. by Framing Rough-in plumbing above slab and below wood floor date/app. by date/app. by Electrical rough-in Heat & Air Duct Peri. beam (Lintel) date/app. by date/app. by date/app. by Permanent power C.O. Final Culvert date/app. by date/app. by date/app. by M/H tie downs, blocking, electricity and plumbing date/app. by date/app. by Reconnection **Utility Pole** Pump pole date/app. by date/app. by date/app. by M/H Pole Travel Trailer Re-roof date/app. by date/app. by date/app. by **BUILDING PERMIT FEE \$** 735.00 **CERTIFICATION FEE \$** 19.03 **SURCHARGE FEE \$** MISC. FEES \$ 0.00 **ZONING CERT. FEE \$** 50.00 FIRE FEE \$ 0.00 WASTE FEE \$

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

FLOOD ZONE FEE \$ 25.00

FLOOD DEVELOPMENT FEE \$

INSPECTORS OFFICE

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

CULVERT FEE \$ 25.00

CLERKS OFFICE

TOTAL FEE

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

ATS# 15948

Prepared by: Michael H. Harrell Abstract & Title Services, Inc. 283 NW Cole Terrace Lake City, FL 32055

Warranty Deed

Individual to Individual

THIS WARRANTY DEED made the 8th day of September, 2006 by

Peter W. Giebeig, A Single Person

Inst:2006021429 Date:09/08/2006 Time:14:01

Doc Stamp-Deed : 402.50

hereinafter called the grantor, to

. DC,P. DeWitt Cason, Columbia County 8:1085 P:887

Peter W. Giebelg, Jr., and his wife, Holly L. Glebelg

whose post office address is: 128 SW Tarragon Glen, Lake City, FL 32024 hereinafter called the grantee:

(Wherever used herein the terms "grantor" and "grante" include all the parties to this instrument and the helrs, legal representatives and assigns of individuals, and the successors and assigns of corporation)

Witnesseth: That the grantor, for and in consideration of the sum of \$10.00 and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys, and confirms unto the grantee, all that certain land situate in COLUMBIA County, FLORIDA, viz: Parcel ID# P/O R02914-003

Lot 12, Mayfair Unit 3, a subdivision according to the plat thereof filed in Plat Book 8, Pages 84-85, of the Public Records of Columbia County, Florida.

TOGETHER with all tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

TO HAVE AND TO HOLD, the same in fee simple fore er.

AND the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to said and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2005.

IN WITNESS WHEREOF, the said grantor has signed and sealed these presents the day and year first above written.

Signed, sealed and delivered in our presence:

Printed Name

Loui W

LUN (7.

Printed Name

Peter W. Giebeig

STATE OF FLORIDA COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before nie this 8th day of September, 2006 by Peter W. Giebelg, A Single Person personally known to me or, if not personally known to me, who produced for identification and who did not take an oath.

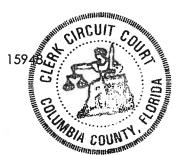
(SEAL)

DORIS M DRAKE
MY COMMISSION & DD397517
EUFIRES: Apr. 5, 2010
(407) 288-6103 Flates Noticy Gurdoncom

35

My Commission Expires:

Received Time Sep. 13. 11:11AM



STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing
is a true copy of the original filed in this office.
P. DEWLY CASON, CLERK OF COURTS

Date 9/8/06

This Instrument Prepared By:
Michael H. Harrell
Abstract & Title Services, Inc.
283 NW Cole Terrace
Lake City, Florida 32055

Inst:2006021431 Date:09/08/2006 Time:14:01

DC,P.DeWitt Cason,Columbia County B:1095 P:981

NOTICE OF COMMENCEMENT

TO WHOM IT MAY CONCERN:

The undersigned hereby give notice that improvements will be made to certain real property and in accordance with Chapter 713.13, Florida Statues, the following is provided in this Notice of Commencement:

- 1. <u>Description of Property</u>: Lot 12, Mayfair Unit 3, a subdivision according to the plat thereof filed in Plat Book 8, Pages 84-85, of the Public Records of Columbia County, Florida.
- 2. General Description of Improvement: Construction of Dwelling
- 3. Owner Information:
 - a. Name and Address: Peter W. Giebeig, Jr. and his wife, Holly L. Giebeig, 126 SW Tarragon Glen, Lake City, Fl 32024
 - b. Interest in property: Fee Simple
 - c. Name and address of fee simple title holder (if other than Owner): NONE
- 4. <u>Contractor (name and address:</u> Trent Giebeig Construction, Inc., 462 SW Fairlington Court, Lake City, Fl 32025
- 5. Surety:

a. Name and Address: N/A

b. Amount of Bond: N/A

 LENDER: First Federal Savings Bank of Florida 4705 West US Highway 90 PO Box 2029

Lake City, FL 32056

- 7. Persons within the State of Florida designated by Owner upon whom notices of other documents may be served as provided in Section 713.13(1)(a)7., Florida Statutes: NONE
- 8. In addition to himself, Owner designates PAULA HACKER, of FIRST FEDERAL SAVINGS BANK OF FLORIDA at 4705 WEST US HIGHWAY 90 / PO BOX 2029, LAKE CITY, FL 32056, to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b) Florida Statutes.
- 8. Expiration date of Notice of Commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

*Owner is used for singular or plural as context requires.

Signed, sealed and delivered in the presence:

A D =

muc burey

STATE OF FLORIDA

COUNTY OF COLUMBIA

Peter W. Giebeig, Jr.

Holly L. Giebeig

Before me, personally appeared Peter W. Giebeig, Jr., and his wife, Holly L. Giebeig to me known to be the person(s) described in and who executed the foregoing instrument, and they acknowledged to and before me that they executed said instrument for the purpose therein

Witness my hand and official seal this 8th day of September, 2006.

(SEAL)

DORIS M DRAKE
MY COMMISSION # DD537517
EXPIRES: Apr. 5, 2010
30153 Florida Notary Service.com

MOTARY PUBLIC

My Commission Expires:

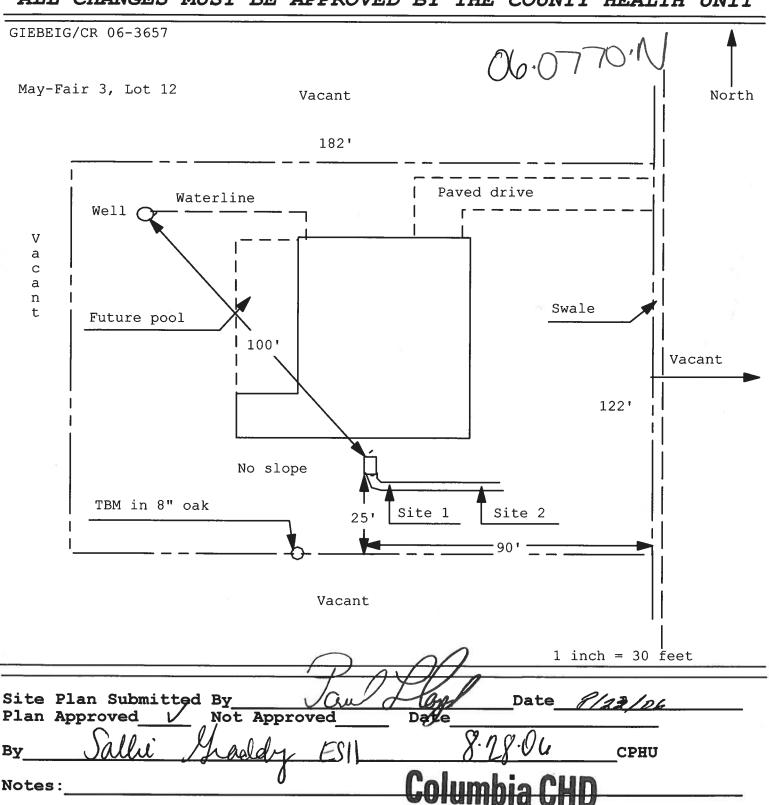
Columbia County Building Permit Application

	·
For Office Use Only Application # 0609 32 Date Re	aceived 9-13-4 By 4 Permit # 1210/ 7.5005
Application Approved by - Zoning Official BUK Date	12.07.06 Plans Examiner DK JTH Date 9-20-06
Flood Zone Development Permit W/A Zoning	188F - 2 Land Use Plan Man Category 8 1
Comments SITE PLAN ON PLANS BALEEDS	CORRECT I OFF
T (1.1.)	20
Applicants Name Trent Giebeig Construction	Phone 397-0345
Address 697 SE Holly Terrace	Lake City FL
Owners Name Peter + Holly Greberg	Phone 752-0791
911 Address 194 SW Vann CT L	ate City FL 32024
Contractors Name Trent Giebeig Const	nuction Phone 397-0545
Address 697 SE Holly Terrace	1110116
Fee Simple Owner Name & Address	
Bonding Co. Name & Address	
Architect/Engineer Name & Address Fremain De	Sign Grown
Mortgage Lenders Name & Address First Federa	
Circle the correct power company - FL Power & Light - Cla	V Flac - Suwannoe Velley Flac - Brown -
Property ID Number	
Subdivision Name May fair Com THE	Estimated Cost of Construction
Driving Directions 247 South right	into Maufair
take 3rd right Van	
	1. 00 1611
Type of Construction <u>Frame 570</u>	Number of Existing Dwellings on Property
Total Acreage 15/ Lot Size 51 Do you need a Cuh	vert Permit or Cubert Water as II
Actual Distance of Structure from Property Lines - Front 34	
21121	0020 Kedi 15
	101AL 3. 806
Application is hereby made to obtain a permit to do work and installation has commenced prior to the issuence of a negative	nstallations as indicated. I certify that no work or
installation has commenced prior to the issuance of a permit at all laws regulating construction in this jurisdiction.	nd that all work be performed to meet the standards of
OWNERS AFFIDAVIT: I hereby certify that all the foregoing info	rmation is accurate and all work will be done in
complication with all applicable laws and regulating construction	n and zoning.
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU IN: LENDER OR ATTORNEY BEFORE: PECORDING YOUR NOTICE.	I END TO OPTAIN PINAMOUND COMMAN
LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE	OF COMMENCEMENT,
	of follow
Owner Builder or Agent (Including Contractor)	Contractor Signature On 2001
STATE OF FLORIDA	Contractors License Number (K NON) 11523
COUNTY OF COLUMBIA	Competency Card Number 5754 NOTARY STAMP/SEAL
Sworn to (or affirmed) and subscribed before me	ELAINE K. TOLAR
this 13 th day of Sept 20 06.	MY COMMISSION # DD 436381 EXPRES October 3-2009
Personally known X or Produced Identification	Notary Signature

. - 1 n. 1/0/9.00 = . 0 77 no

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan Permit Application Number:

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



LYNCH WELL DRILLING, INC.

173 SW Tusienuggee Ave Lake City, FL. 32025 Phone 386-752-6677 Fax 386-752-1477

Lar	ic City, I D. 52025		* 2.10
P	hone 386-752-6677 Fax 386-752-1477	m.	ayfour Lot 12
Building Permit #	Owner's Name In	nt Giel	Jayfair Lot 12 unit 3 Fi.
Well Depth Ft. Casing	DepthFt. Wat	er Level	Ft.
Casing Size 4 inch Steel Pump I	nstallation: <u>Deep \</u>	Vell Submersib	le
Pump Make Qormotor	Pump Model <u>S2C</u>	1.100	HP
System Pressure (PSI)On _	30 of 50	Average Press	ure <u>40</u>
Pumping System GPM at average pre	essure and pumping lev	el <u>20</u>	_(GPM)
Tank Installation: Bladder/Galvaniz Model PC 244 Size 81	<u>ed</u> Make <u>(M. Q.V.)</u> —	langer	
Tank Draw-down per cycle at system	pressure	5.1	gailons
I HEREBY VERTIFY THAT THIS INSTALLED AS PER THE ABOV			BEEN
<u>Binda Newco</u> mb	-	Linda Newcor Print Name	4
2609 License Number		Date	3·06

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Project Name: Address: City, State: Owner: Climate Zone: Dr. Giebeig Giebeig North		Builder: Permitting Office: Cour Permit Number: 2500 Jurisdiction Number: 22	5
 New construction or existing Single family or multi-family Number of units, if multi-family Number of Bedrooms Is this a worst case? Conditioned floor area (ft²) Glass area & type Clear glass, default U-factor Default tint Labeled U or SHGC Floor types Slab-On-Grade Edge Insulation N/A N/A N/A N/A N/A Ciling types Under Attic N/A N/A N/A Toucts Sup: Unc. Ret: Unc. AH: Interior N/A 	New Single family	 12. Cooling systems a. Central Unit b. N/A c. N/A 13. Heating systems a. Electric Heat Pump b. N/A c. N/A 14. Hot water systems a. Electric Resistance b. N/A c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) 15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating) 	Cap: 60.0 kBtu/hr SEER: 13.00 Cap: 60.0 kBtu/hr HSPF: 8.00 Cap: 50.0 gallons EF: 0.90 MZ-C, PT, CF,
Glass/Floor Area:	0.07 Total as-built p	points: 27639 PASS	
I hereby certify that the plans and	d specifications covered	Review of the plans and	

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY:

DATE:

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code.
Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



BUILDING OFFICIAL:	
DATE:	H

SUMMER CALCULATIONS

ADDRESS: ,,,	PERMIT #:	

BASE		AS-BUILT		
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area	•	Overhang Prnt Len Hgt Area	SPM X SOF	= Points
.18 2939.0 20.04 1060	1.6 Double, Clear	E 1.5 6.0 50.0	42.06 0.91	1919.6
	Double, Clear	S 1.5 5.0 12.0	35.87 0.81	347.3
×	Double, Clear	W 1.5 6.0 100.0	38.52 0.91	3518.6
	Double, Clear	N 1.5 6.0 45.0	19.20 0.94	811.0
	Double, Clear	N 1.5 2.0 5.0	19.20 0.76	72.6
	As-Built Total:	212.0		6669.2
WALL TYPES Area X BSPM = Po	pints Type	R-Value Are	a X SPM =	Points
Adjacent 0.0 0.00	0.0 Frame, Wood, Exterior	13.0 2046.4	1.50	3069.6
Exterior 2046.4 1.70 3	178.9			
Base Total: 2046.4 3	178.9 As-Built Total:	2046.4		3069.6
DOOR TYPES Area X BSPM = Po	pints Type	Are	a X SPM =	Points
Adjacent 0.0 0.00	0.0 Exterior Wood	68.0	6.10	414.8
Exterior 95.9 6.10	584.9 Exterior Wood	27.9	6.10	170.1
Base Total: 95.9	584.9 As-Built Total:	95.9		584.9
CEILING TYPES Area X BSPM = Po	pints Type	R-Value Area X	SPM X SCM =	Points
Under Attic 2939.0 1.73 5	084.5 Under Attic	30.0 3232.9	1.73 X 1.00	5592.9
Base Total: 2939.0 5	84.5 As-Built Total:	3232.9		5592.9
FLOOR TYPES Area X BSPM = Pe	oints Type	R-Value Are	a X SPM =	Points
Slab 255.8(p) -37.0 -9	64.6 Slab-On-Grade Edge Insulation	0.0 255.8(p	-41.20	-10539.0
Raised 0.0 0.00	0.0			
Base Total: -9	64.6 As-Built Total:	255.8		-10539.0
INFILTRATION Area X BSPM = Pe	nints	Are	a X SPM =	Points
2939.0 10.21 30	07.2	2939	0.0 10.21	30007.2

SUMMER CALCULATIONS

ADDRESS: ,,,	PERMIT #:

	BASE		AS-BUILT			
Summer Bas	se Points:	40292.4	Summer As-Built Points:	35384.8		
Total Summer Points	X System Multiplier	= Cooling Points	Total X Cap X Duct X System X Credit Component Ratio Multiplier Multiplier Multiplie (DM x DSM x AHU)	•		
40292.4	0.4266	17188.7	35384.8 1.000 (1.090 x 1.147 x 0.91) 0.263 0.857 35384.8 1.00 1.138 0.263 0.857	9061.7 9061.7		

WINTER CALCULATIONS

ADDRESS: ,,,	PERMIT #:

BASE				AS-	BUI	LT					
GLASS TYPES .18 X Conditioned X BWPI Floor Area	M = Points	Type/SC C		erhang Len	Hgt	Area X	WF	PM :	x w	/OF	= Points
.18 2939.0 12.7	4 6739.7	Double, Clear	Е	1.5	6.0	50.0	18.	79	1.0	04	973.0
		Double, Clear	S	1.5	5.0	12.0	13.	30	1.3	20	191.0
		Double, Clear	W	1.5	6.0	100.0	20.	73	1.0	02	2121.5
5		Double, Clear	N	1.5	6.0	45.0	24.			00	1108.6
		Double, Clear	N	1.5	2.0	5.0	24.	58	1.0	01	124.7
		As-Built Total:				212.0					4518.9
WALL TYPES Area X BV	VPM = Points	Туре		R-	√alue	Area	Х	WF	PM	=	Points
	0.00 0.0 3.70 7571.7	Frame, Wood, Exterior			13.0	2046.4		3.4	0		6957.8
Base Total: 2046.4	7571.7	As-Built Total:				2046.4					6957.8
DOOR TYPES Area X BV	VPM = Points	Туре				Area	Х	WF	M	=	Points
Adjacent 0.0 0	0.00	Exterior Wood				68.0		12.3	0		836.4
Exterior 95.9 12	2.30 1179.3	Exterior Wood				27.9		12.3	0		342.9
Base Total: 95.9	1179.3	As-Built Total:				95.9					1179.3
CEILING TYPES Area X BV	VPM = Points	Туре	R	R-Value	Ar	ea X W	PM	χv	VCM	=	Points
Under Attic 2939.0 2	2.05 6024.9	Under Attic		;	30.0	3232.9	2.05	X 1.0	0		6627.4
Base Total: 2939.0	6024.9	As-Built Total:				3232.9					6627.4
FLOOR TYPES Area X BV	VPM = Points	Туре		R-	Value	Area	Χ	WF	PM	=	Points
Slab 255.8(p)	8.9 2276.6	Slab-On-Grade Edge Insulation	n		0.0	255.8(p		18.8	0		4809.0
	0.00					*					
Base Total:	2276.6	As-Built Total:				255.8					4809.0
INFILTRATION Area X BV	VPM = Points					Area	Х	WF	PM	=	Points
2939.0	0.59 -1734.0					2939.0	0	-0.	59		-1734.0

WINTER CALCULATIONS

	 -1 I
ADDRESS: , , ,	 PERMIT #:

BASE			AS-BUILT			
Winter Base	Points:	22058.3	Winter As-Built Points:	22358.4		
Total Winter / Points	X System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit Component Ratio Multiplier Multiplier Multiplier (DM x DSM x AHU)	•		
22058.3	0.6274	13839.4	22358.4 1.000 (1.069 x 1.169 x 0.93) 0.426 0.950 22358.4 1.00 1.162 0.426 0.950	10522.1 10522.1		

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: ,,,	PERMIT #:

BASE			AS-BUILT										
WATER HEA Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	x	Tank X Ratio	Multiplier	X Credit Multiplie		otal
3		2746.00		8238.0	50.0	0.90	3		1.00	2684.98	1.00	80	054.9
		-			As-Built To	otal:						80)54.9

CODE COMPLIANCE STATUS							
BASE	AS-BUILT						
Cooling + Heating + Hot Water = Total Points Points Points Points	Cooling + Heating + Hot Water = Total Points Points Points Points						
17189 13839 8238 39266	9062 10522 8055 27639						

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS:	DEDMIT #
ADDRESS.,,,	PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit	
		breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools	
		must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems 610.1 All ducts, fittings, mechanical equipment and plenum chambers shall be mecha attached, sealed, insulated, and installed in accordance with the criteria of Section 1.		All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 88.5

The higher the score, the more efficient the home.

beig		

				• • • • •	•		
1.	New construction or existing		New _	12.	Cooling systems		
2.	Single family or multi-family		Single family	_ a.	Central Unit	Cap: 60.0 kBtu/hr	
3.	Number of units, if multi-family		. 1 _			SEER: 13.00	100000
4.	Number of Bedrooms		3 _	_ b.	N/A		
5.	Is this a worst case?		Yes _	_			100
6.	Conditioned floor area (ft²)		2939 ft²	c.	N/A		
7.	Glass area & type	Single Pane	Double Pane				
. a.	Clear - single pane	0.0 ft²	212.0 ft² _	13.	Heating systems		
b.	Clear - double pane	0.0 ft²	0.0 ft²	_ a.	Electric Heat Pump	Cap: 60.0 kBtu/hr	
c.	Tint/other SHGC - single pane	0.0 ft ²	0.0 ft ²	_	·	HSPF: 8.00	
d.	Tint/other SHGC - double pane		-	_ b.	N/A		7
8.	Floor types					8	
a.	Slab-On-Grade Edge Insulation	R=	0.0, 255.8(p) ft _	С.	N/A		
	N/A		1 " =	_			
c.	N/A			14.	Hot water systems		
9.	Wall types				Electric Resistance	Cap: 50.0 gallons	
a.	Frame, Wood, Exterior	R=1	3.0, 2046.4 ft ²			EF: 0.90	
b.	N/A		_	_ b.	N/A		
c.	N/A			_			
d.	N/A		_	- с.	Conservation credits		
e.	N/A		_	_	(HR-Heat recovery, Solar		
10.	Ceiling types				DHP-Dedicated heat pump)		
	Under Attic	R=3	30.0, 3232.9 ft ²	15.	HVAC credits	MZ-C, PT, CF,	
b.	N/A		•	-	(CF-Ceiling fan, CV-Cross ventilation,		
c.	N/A		1		HF-Whole house fan.		
11.	Ducts				PT-Programmable Thermostat,		
a.	Sup: Unc. Ret: Unc. AH: Interior	Sup.	R=6.0, 73.8 ft		MZ-C-Multizone cooling,		
	N/A	•	_	70	MZ-H-Multizone heating)		
	rtify that this home has complic						
	struction through the above en					OF THE STATE	.
m a	nis home before final inspection	n. Otnerwise	, a new EPL Dis	spiay Car	a will be completed		B

based on installed Code compliant features.

Builder Signature: Address of New Home: City/FL Zip: _____



*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTMdesignation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction,

Residential System Sizing Calculation

Summary Project Title:

Giebeig

Dr. Giebeig

Code Only **Professional Version** Climate: North

Location for weather data: Gainesville - User customized: Latitude(29) Temp Range(M)

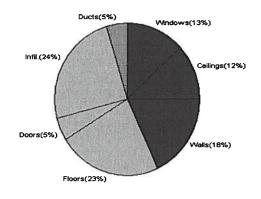
8/21/2006

Humidity data: Interior RH (50%) Outdoor wet bulb (78F) Humidity difference(51gr.)							
Winter design temperature	31	F	Summer design temperature	98	F		
Winter setpoint	70	F	Summer setpoint	75	F		
Winter temperature difference	39	F	Summer temperature difference	23	F		
Total heating load calculation	34997	Btuh	Total cooling load calculation	35400	Btuh		
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh		
Total (Electric Heat Pump)	171.4	60000	Sensible (SHR = 0.5)	106.9	30000		
Heat Pump + Auxiliary(0.0kW)	171.4	60000	Latent	408.9	30000		
			Total (Electric Heat Pump)	169.5	60000		

WINTER CALCULATIONS

Winter Heating Load (for 2939 sqft)

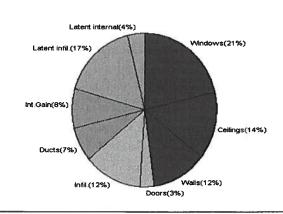
Load component			Load	
Window total	212	sqft	4558	Btuh
Wall total	2046	sqft	6344	Btuh
Door total	96	sqft	1720	Btuh
Ceiling total	3233	sqft	4203	Btuh
Floor total	256	ft	8083	Btuh
Infiltration	196	cfm	8422	Btuh
Subtotal			33330	Btuh
Duct loss			1667	Btuh
TOTAL HEAT LOSS		_	34997	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 2939 sqft)

Load component			Load	
Window total	212	sqft	7566	Btuh
Wall total	2046	sqft	4379	Btuh
Door total	96	sqft	1177	Btuh
Ceiling total	3233	sqft	5043	Btuh
Floor total			0	Btuh
Infiltration	172	cfm	4346	Btuh
Internal gain			3000	Btuh
Subtotal(sensible)			25512	Btuh
Duct gain			2551	Btuh
Total sensible gain			28063	Btuh
Latent gain(infiltration)			5957	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			7337	Btuh
TOTAL HEAT GAIN			35400	Btuh



EnergyGauge® System Sizing based on ACCA Manual J. PREPARED BY: DATE:

EnergyGauge® FLRCPB v3.30

System Sizing Calculations - Winter

Residential Load - Component Details

Giebeig

Project Title: Dr. Giebeig

Code Only Professional Version

Climate: North

Reference City: Gainesville (User customized) Winter Temperature Difference: 39.0 F

8/21/2006

Window	Panes/SHGC/Frame/U	Orientation	n Area X	HTM=	Load
1	2, Clear, Wood, DEF	N	50.0	21.5	1075 Btuh
2 3	2, Clear, Wood, DEF	Ε	12.0	21.5	258 Btuh
3	2, Clear, Wood, DEF	S	100.0	21.5	2150 Btuh
4	2, Clear, Wood, DEF	W	45.0	21.5	968 Btuh
5	2, Clear, Wood, DEF	W	5.0	21.5	108 Btuh
]				
	Window Total		212		4558 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Exterior	13.0	2046	3.1	6344 Btuh
	Wall Total		2046		6344 Btuh
Doors	Туре		Area X	HTM=	Load
1	Wood - Exter		68	17.9	1220 Btuh
2	Wood - Exter		28	17.9	500 Btuh
	Door Total		96		1720Btuh
Ceilings	Туре	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	3233	1.3	4203 Btuh
	.				
	Ceiling Total		3233		4203Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	255.8 ft(p)	31.6	8083 Btuh
	 				
1.634 41	Floor Total		256	·	8083 Btuh
Infiltration	Туре	ACH X	Building Volume	CFM=	Load
	Natural	0.40	29390(sqft)	196	8422 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			196	8422 Btuh

	Subtotal	33330 Btuh
Totals for Heating	Duct Loss(using duct multiplier of 0.05)	1667 Btuh
	Total Btuh Loss	34997 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)

(Frame types - metal, wood or insulated metal)

(U - Window U-Factor or 'DEF' for default)

(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Summer

Residential Load - Component Details

Giebeig

Project Title: Dr. Giebeig

Code Only Professional Version

Climate: North

Reference City: Gainesville (User customized)

Summer Temperature Difference: 23.0 F

8/21/2006

	Туре	Over	hang	Win	dow Are	a(sqft)	Н	TM	Load	
Window	Panes/SHGC/U/InSh/ExSh Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2, Clear, DEF, N, N N	1.5	6	50.0	0.0	50.0	24	24	1200	Btuh
2	2, Clear, DEF, N, N E	1.5	5	12.0	0.0	12.0	24	74	888	Btuh
3	2, Clear, DEF, N, N S	1.5	6	100.0	100.0	0.0	24	39	2400	Btuh
4	2, Clear, DEF, N, N W	1.5	6	45.0	9.3	35.7	24	74	2863	Btuh
5	2, Clear, DEF, N, N W	1.5	2	5.0	3.1	1.9	24	74	214	Btuh
	Window Total			212					7566	Btuh
Walls	Туре	R-	Value		-	Area		MTH	Load	
1	Frame - Exterior		13.0		2	046.4		2.1	4379	Btuh
	Wall Total			_	20	046.4			4379	Btuh
Doors	Туре				-	Area		HTM	Load	
1	Wood - Exter					68.0		12.3	835	Btuh
2	Wood - Exter					27.9		12.3	342	Btuh
	Door Total				9	95.9			1177	Btuh
Ceilings	Type/Color	R-\	/alue		-	Area		НТМ	Load	
1	Under Attic/Dark		30.0		3	232.9		1.6	5043	Btuh
	Ceiling Total				32	232.9			5043	Btuh
Floors	Туре	R-\	/alue			Size		НТМ	Load	
1	Slab-On-Grade Edge Insulation		0.0		2	255.8 ft(p)		0.0	0	Btuh
	Floor Total				2	55.8			0	Btuh
Infiltration	Туре	Α	CH			lume		CFM=	Load	
	Natural		0.35		2	9390		171.8	4346	Btuh
	Mechanical							0	0	Btuh
	Infiltration Total							172	4346	Btuh

Internal	Occupants	Btuh/occupant	Appliance	Load
gain	6	X 300 +	1200	3000 Btuh

	Subtotal	25512	Btuh
Totals for Cooling	Duct gain(using duct multiplier of 0.10)	2551	Btuh
	Total sensible gain	28063	Btuh
	Latent infiltration gain (for 51 gr. humidity difference)	5957	Btuh
	Latent occupant gain (6 people @ 230 Btuh per person)	1380	Btuh
	Latent other gain	0	Btuh
	TOTAL GAIN	35400	Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint) (U - Window U-Factor or 'DEF' for default)

(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))

(ExSh - Exterior shading device: none(N) or numerical value)

(Ornt - compass orientation)



ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 480/680/880 Drop-in PRODUCT TYPE: Aluminum Horizontal Sliding Window (XO-Fin)

	Results		
Title	Test Specimen #1	Test Specimen #2	
Rating	HS-C30 71 x 71	HS-C40 71 x 59	
Operating Force	11 lbf max.	14 lbf max.	
Air Infiltration	0.11 cfm/ft ²	0.09 cfm/ft^2	
Water Resistance Test Pressure	5.3 psf	6.0 psf	
Uniform Load Deflection Test Pressure	± 30.0 psf	+ 45.0 psf -47.2 psf	
Uniform Structural Load Test Pressure	± 45.0 psf	+ 67.5 psf -70.8 psf	
Forced Entry Resistance	Grade 10	Grade 10	

Reference should be made to ATI Report Identification No. 01-47320.03 for complete test specimen description and data_{130 Derry Court}

York, PA 17402-9405 phone: 717.764.7700

fax: 717.764.4129 www.archtest.com



ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC. P.O. Box 370 650 West Market Street Gratz, Pennsylvania 17030-0370

ATI Report Identification No.: 01-47320.03

Test Dates: 10/07/03 Through: 10/08/03

And: 12/01/03 And: 12/15/03 And: 03/17/04

Report Date: 04/16/04 Expiration Date: 10/07/07

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on two Series/Model 480/680/880 Drop-in, aluminum horizontal sliding windows at MI Home Products, Inc. test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1: HS-C30 71 x 71; Test Specimen #2: HS-C40 71 x 59. Test specimen description and results are reported herein.

Test Specification: The test specimens were evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.

Test Specimen Description:

Series/Model: 480/680/880 Drop-in

Product Type: Aluminum Horizontal Sliding Window (XO Fin)

Test Specimen #1: HS-C30 71 x 71

Overall Size: 5' 11-7/16" wide by 5' 11" high

Active Sash Size: 2' 11-5/8" wide by 5' 8-3/8" high

Fixed Daylight Opening Size: 2' 8-3/16" wide by 5' 5-5/8" high

Screen Size: 2' 10" wide by 5' 6-1/2" high

130 Derry Court York, PA 17402-9405 phone: 717.764.7700 fax: 717.764.4129 www.archtest.com



Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.250" high by 0.187" backed polypile with center fin	1 Row	Active sash top and bottom rails and fixed meeting rail interlock
0.250" high by 0.187" backed polypile with center fin	2 Rows	Jamb stile

Test Specimen #2: HS-C40 71 x 59

Overall Size: 5' 11-3/8" wide by 4' 11-1/8" high

Active Sash Size: 2' 11-5/8" wide by 4' 8-1/4" high

Fixed Daylight Opening Size: 2' 8-1/4" wide by 4' 5-7/8" high

Screen Size: 2' 10-1/4" wide by 4' 7-1/8" high

Weatherstripping:

Description	Quantity	Location
0.310" high by 0.187" backed polypile with center fin	1 Row	Active sash top and bottom rails
0.250" high by 0.187" backed polypile with center fin	1 Rows	Fixed meeting rail interlock
0.310" high by 0.187" backed polypile with center fin	2 Rows	Jamb stile
0.550" high by 1" by 1" backed polypile pad	1 Pad	Corner of bottom rail and locking stile



Test Specimen Description: (Continued)

The following descriptions apply to all specimens.

Finish: All aluminum was white.

Glazing Details: The window utilized 5/8" thick sealed insulating glass constructed from two sheets of 1/8" thick clear annealed glass and a Swiggle spacer system. The lites were interior glazed onto double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Frame Construction: The frame was constructed of thermally broken extruded aluminum. The corners were secured utilizing three $\#8 \times 1$ " screws per corner through the jambs into the head and sill screw bosses. End caps were utilized on the ends of the fixed meeting rails and secured with two $\#8 \times 3/4$ " screws per cap. The meeting rails were then secured to the frame with two $\#8 \times 3/4$ " screws.

Sash Construction: The sash was constructed of thermally broken extruded aluminum. The corners were secured utilizing one #8 x 1" screw per corner through the head and sill into the jambs screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible vinyl spline.

Hardware:

Description	Quantity	Location
Cam lock	1	One midspan of active panel with integral lock keeper on fixed meeting stile
Roller assembly	2	One each end of bottom rail
Screen constant force spring	2	5" from rails on screen stiles
Screen lift handles	2	5" from rails on screen stiles

Drainage:

<u>Description</u>	Quantity	Location
1-1/4" long by 1/4" wide weepslot with cover	2	3-1/2" from jambs on sill face
1/2" long by 1/8" wide weepslot	2	2" from jambs on sill track

Reinforcement: No reinforcement was utilized.

Installation: The window was installed into a #2 Spruce-Pine-Fir wood buck. The window was secured utilizing #8 x 1-5/8" drywall screws located in corners and 12" on center around nail-fin perimeter. Silicone was utilized around the exterior perimeter.



Test Results:

The results are tabulated as follows:

<u>Paragraph</u>	Title of Test - Test Method	Results	Allowed
Test Specimen	<u>1</u> #1: HS-C30 71 x 71		
2.2.2.5.1	Operating Force	11 lbf	25 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.11 cfm/ft ²	0.3 cfm/ft ² max.
Note #1: ANSI/AAMA/N	The tested specimen meets th WWDA 101/I.S. 2-97 for air infiltrat	he performance ion.	levels specified in
2.1.3	Water Resistance per ASTM E 547 (with and without screen) 4.50 psf	-00 No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTI (Deflections reported were taken of (Loads were held for 52 seconds) 30.0 psf (positive) 30.0 psf (negative)		See Note #2 See Note #2
101/I.S.2-97 fo	Uniform Load Deflection test is n r this product designation. The defl ampliance and information only.	ot requirement of A lection data is reco	ANSI/AAMA/NWWDA rded in this report for
2.1.4.2	Uniform Load Structural per ASTN (Permanent sets reported were take (Loads were held for 10 seconds) 45.0 psf (positive) 45.0 psf (negative)		0.26" max. 0.26" max.
2.2.2.5.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Handle stile Lock stile	0.13"/25% 0.19"/38%	0.50"/100% 0.50"/100%
	In remaining direction - 50 lbs		
	Top rail Bottom rail	0.09"/19% 0.06"/13%	0.50"/100% 0.50"/100%



Test Results: (Continued)

LINE ARCHIV							
Paragraph	Title of Test - Test Method	Results	Allowed				
Test Specimen #1: HS-C30 71 x 71 (Continued)							
2.1.8	Forced Entry Resistance per ASTM F 588						
Type: A	Grade: 10						
	Lock Manipulation Test	No entry	No entry				
	Test A1 thru A5	No entry	No entry				
	Test A7	No entry	No entry				
	Lock Manipulation Test	No entry	No entry				
Optional Perfo	rmance						
4.3	Water Resistance per ASTM E 54 (with and without screen) 5.3 psf	7-00 No leakage	No leakage				
Test Specimer	1 #2 : HS-C40 71 x 59						
2.2.2.5.1	Operating Force	14 lbf	25 lbf max.				
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.09 cfm/ft ²	0.3 cfm/ft ² max.				
Note #1: ANSI/AAMA/N	The tested specimen meets WWDA 101/I.S. 2-97 for air infiltra	the performance ation.	levels specified i	in			
2.1.3	Water Resistance per ASTM E 54 (with and without screen)	7-00					
	4.50 psf	No leakage	No leakage				
2.1.4.1 Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)							
	30.0 psf (positive) 30.0 psf (negative)	0.62" 0.51"	See Note #2 See Note #2				
2.1.4.2	Uniform Load Structural per AST (Permanent sets reported were tak (Loads were held for 10 seconds)		tile)				
	45.0 psf (positive)	0.03"	0.21" max.				
	45.0 psf (negative)	0.04"	0.21" max.				



Test Results: (Continued)

<u>Paragraph</u>	Title of Test - Test Method	Results	Allowed				
Test Specimen	Test Specimen #2: HS-C40 71 x 59 (Continued)						
2.2.2.5.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs						
	Handle stile Lock stile	0.13"/25% 0.13"/25%	0.50"/100% 0.50"/100%				
	In remaining direction - 50 lbs						
	Top rail Bottom rail	0.03"/6% 0.03"/6%	0.50"/100% 0.50"/100%				
2.1.8	Forced Entry Resistance per ASTN	MF 588					
	Type: A	Grade: 10					
	Lock Manipulation Test	No entry	No entry				
	Test A1 thru A5	No entry	No entry				
	Test A7	No entry	No entry				
	Lock Manipulation Test	No entry	No entry				
Optional Perfo	ormance						
4.3	Water Resistance per ASTM E 54	7-00					
	(with and without screen) 6.0 psf	No leakage	No leakage				
4.4.1	Uniform Load Deflection per AST (Deflections reported were taken of (Loads were held for 52 seconds)	CM E 330 on the meeting stile)					
	45.0 psf (positive) 47.2 psf (negative)	0.62" 0.54"	See Note #2 See Note #2				
4.4.2	Uniform Load Structural per AST (Permanent sets reported were take	M E 330 en on the meeting stile	e)				
	(Loads were held for 10 seconds) 67.5 psf (positive) 70.8 psf (negative)	0.04" 0.08"	0.21" max. 0.21" max.				

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without approval of Architectural Testing.

For ARCHITECTURAL TESTING, INC.

Digitally Signed by: Eric Westphal

Eric Westphal Technician

EW:dme 01-47320.03 Digitally Signed by: Steven M. Urich

Steven M. Urich, P. E. Senior Project Engineer

St 221



BUILDING CODE COMPLIANCE OFFICE (BCCO) PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA METRO-DADE FLAGLER BUILDING 140 WEST FLAGLER STREET, SUITE 1603 MIAMI, FLORIDA 33130-1563 (305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Ceco Door Products 9159 Telecom Drive Milan, TN 38358

in Swing

Scope:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: The Ceco Series Single Flush / Embossed Inswing Commercial Steel Doors - Impact APPROVAL DOCUMENT: Drawing No RD0728, titled "3-0 x 7-0, Series Regent, Omega, Imperial, Versa door", prepared by manufacturer, sheets 1 through 9 of 9 dated 05/22/02 and latest revised on 10-10-02, bearing the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

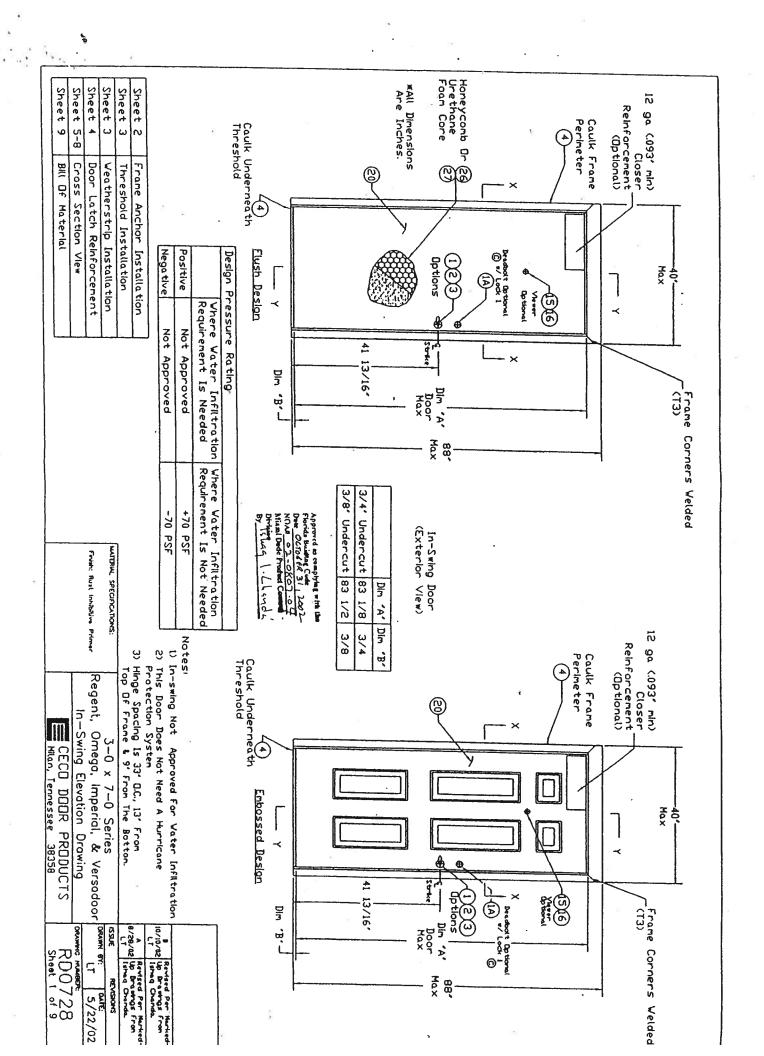
This NOA consists of this page 1 as well as approval document mentioned above.

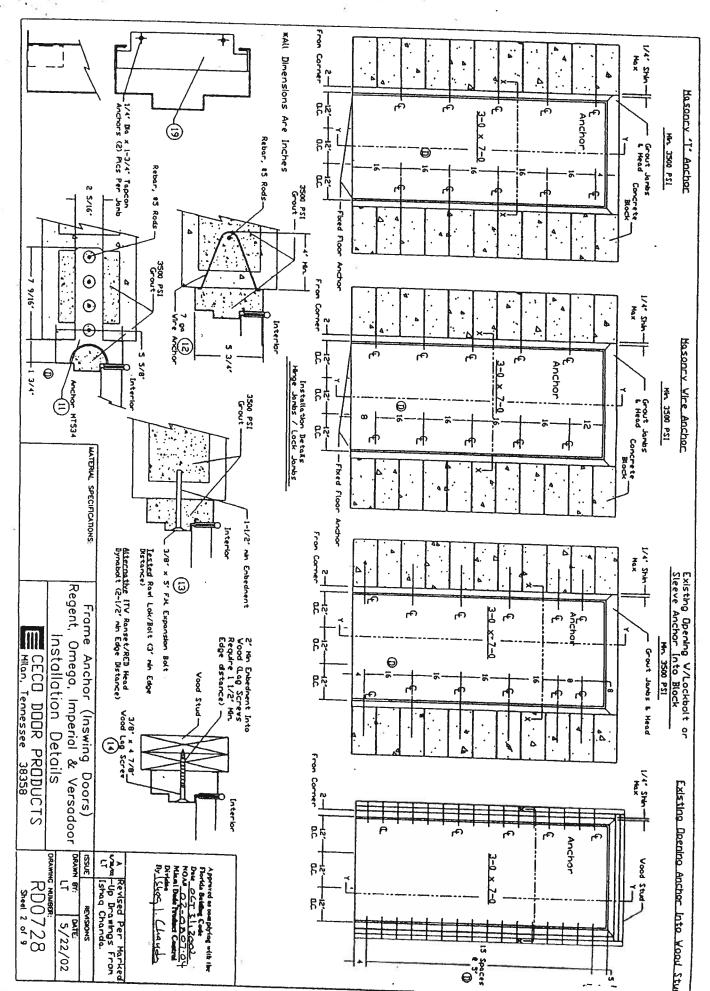
The submitted documentation was reviewed by Ishaq I. Chanda, P.E.

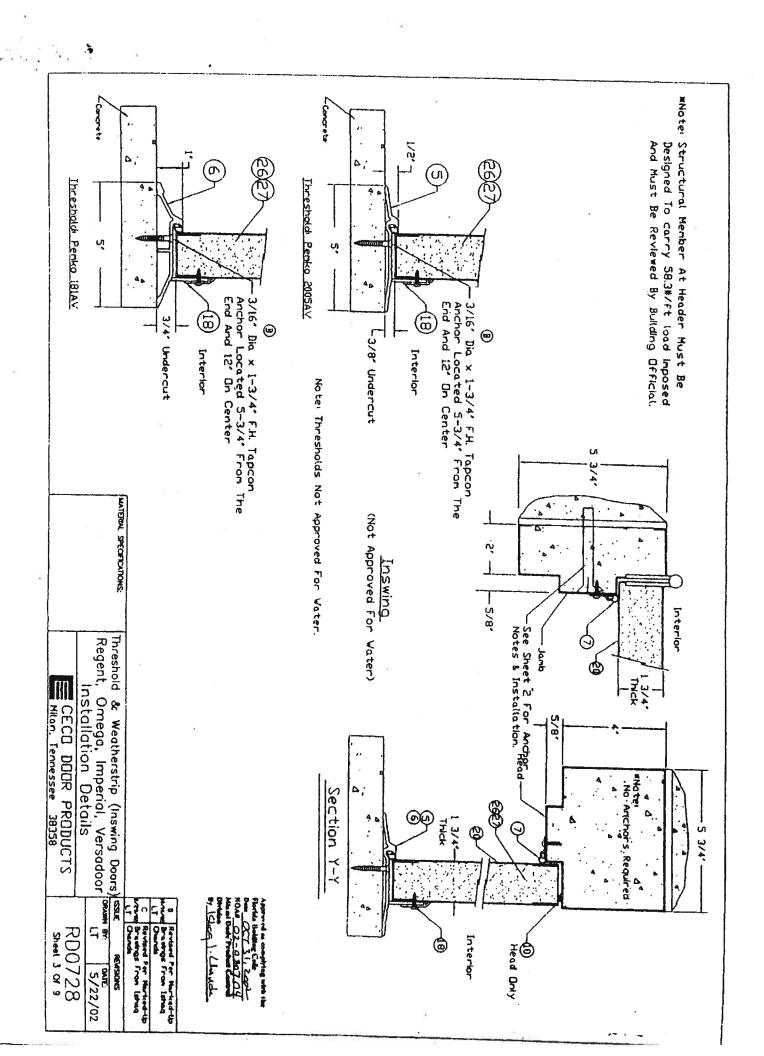


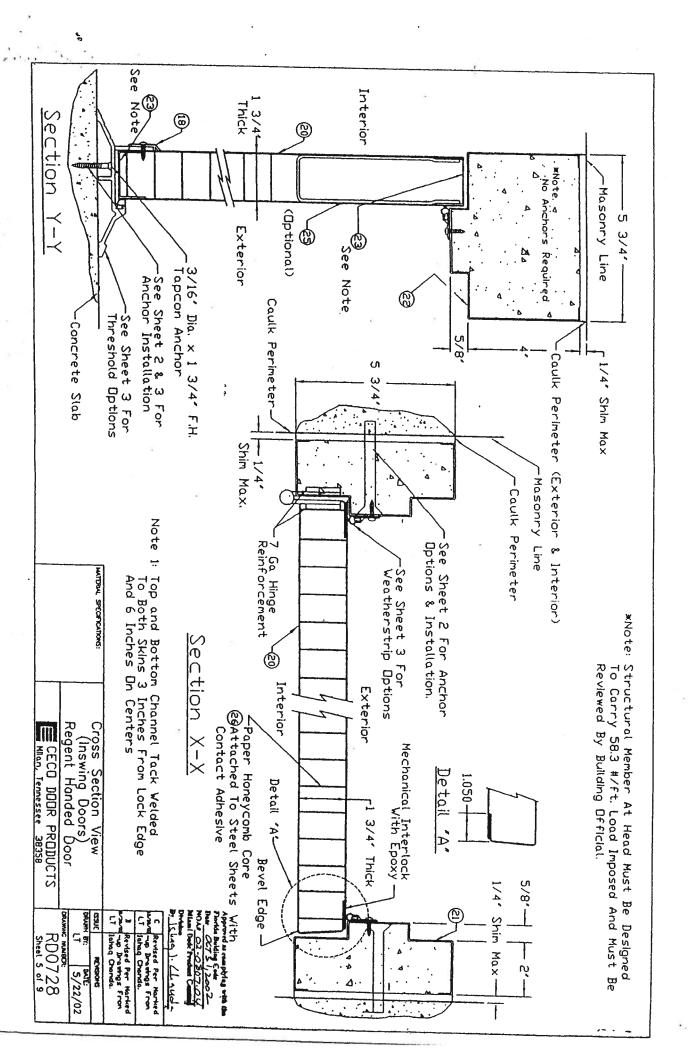
NOA No 02-0807.04 Expiration Date: October 31, 2007 Approval Date: October 31, 2002

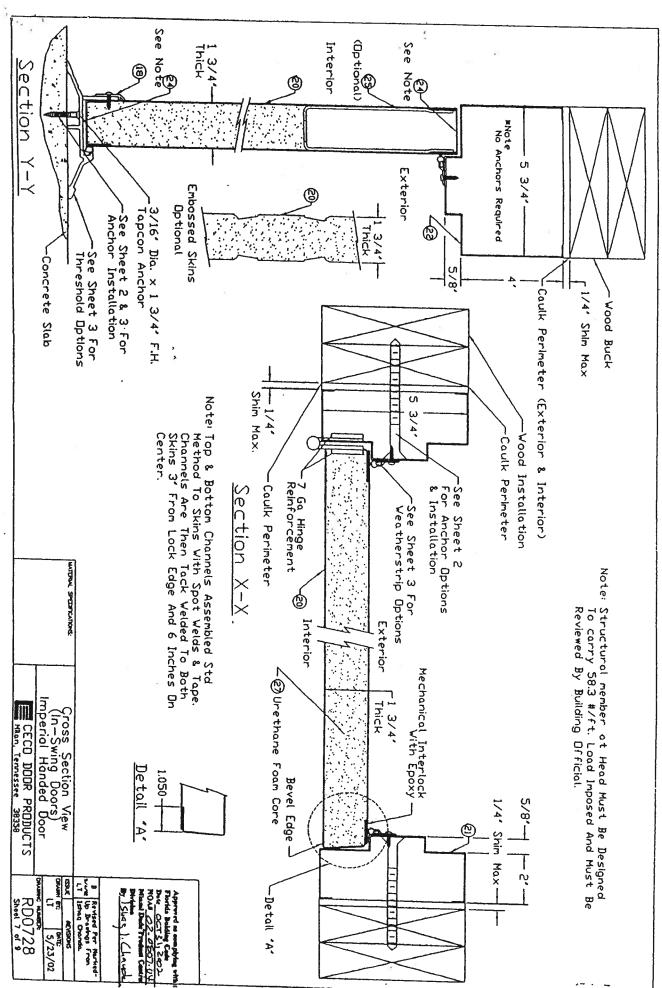
Page 1

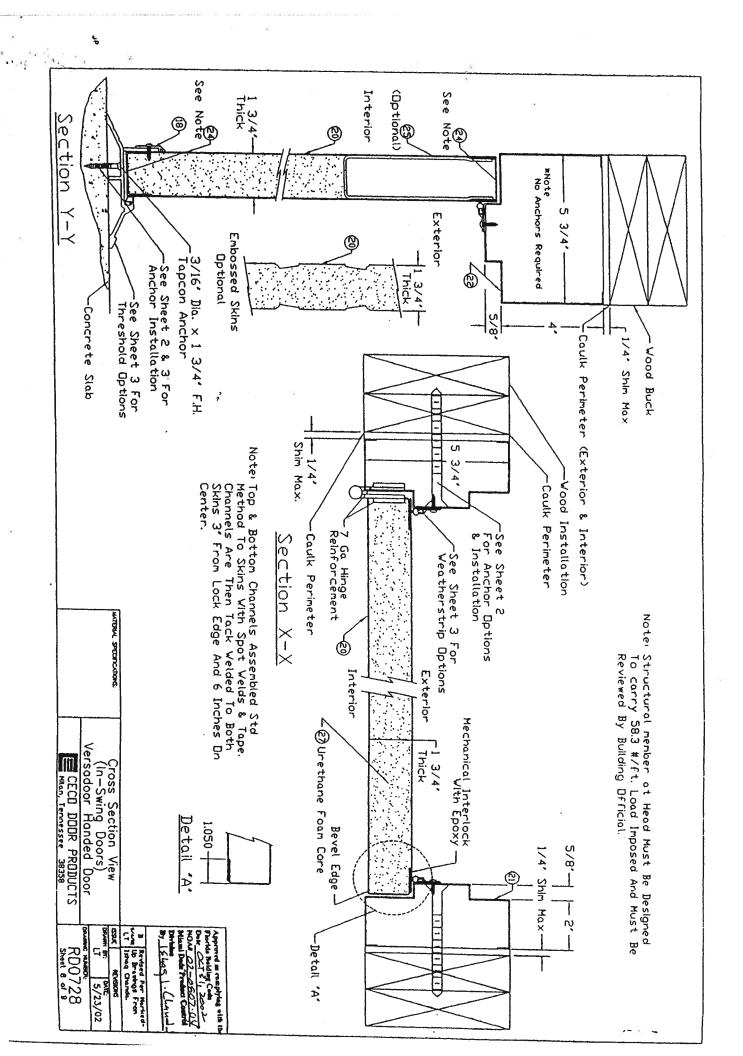












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		Honeycomb Core	Inforcement (Optional)	Taped To Top Skin, Tack Welded To Both	ack Welded To Bot	Door Channels, Spot Velded To Bottom Skin	A60 Galy Conforming To ASTM A653	1	OALLAS	or		Drip Cap, Top	Or	Viewer		טר		Frame Anchor	ers	٥- <	Hinge (Ball Bearing)	Veatherstrip	inreshold .	COUR	rtise Lock	ا حدا	Deadbolt (Dptional) (D)
MATERIAL SPECIFICATIONS:		n-Impregnated Kraft Pape	12 Ga (1093' MIN) CS Type B	B (History Yield Strand	nnercial Steel Type B (Minimum Yield Stren	A60 Galv Conforming To AST	B (Minimum Yield Stren	Commercial Steel Type B (Minimum Yield Strength 30,000psi)	merclat S	locr Anchor	Penko	Penko	MAG Security	HOOD FOR ACTION		Expansion Bolt	Wire, Relaxed Dimension 9' x 8'	Masonary Tee (RD0057)		Equal (Attached w/ (8) #12-24 x 1/2 HS	or Equal (Attached #/ (B) #12-24 x 1/2 HS Par	Pomico	Penko	Dow Corning	Saflok	Saflok	
× 7-0 s		\ I	12 00 (103' min × 1' × 1-3/4	-	si) 16 ga (.053' min) x 1' x 1-3/4' x 1'	_	4' Fore 5-3/4' Denth Min	si) 2' Face, 5-3/4' Depth Min. (RD0033)	16 Ga (.053° nIn)	16 ga (.053° mln) galvanized Steel	315 ×	346	8794-0	× 4-5/8′	/8' x 5' F.H.	3/8' x 5' F.H. Rawl Lok/Bolt	7 ' min) Galv Steel Wire	16 ga (.053' nin) Galv Steel Evelo = 304-1	(S88)	1	303AV3684	18)AV36		899 Silicone Glazing Sealant	MI SECTION	Presion St 2500	AL53PJI

CECO DOOR PRODUCTS

RD0728



MIAMI-DADE COUNTY, FLORIDA METRO-DADE FLAGLER BUILDING 140 WEST FLAGLER STREET, SUITE 1603 MIAMI, FLORIDA 33130-1563 (305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Ceco Door Products 9159 Telecom Drive Milan, TN 38358

outswing

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: Series "Regent" & "Omega" 18 ga. 3°-7° Outswing Commercial Steel Door

APPROVAL DOCUMENT: Drawing No. RD0087, titled "3-0 x 7-0 Series", sheets 1 through 7 of 7, dated 5/30/97 with revision C dated 2/24/00, prepared by the manufacturer, bearing the Miami-Dade County Product Control Renewal stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

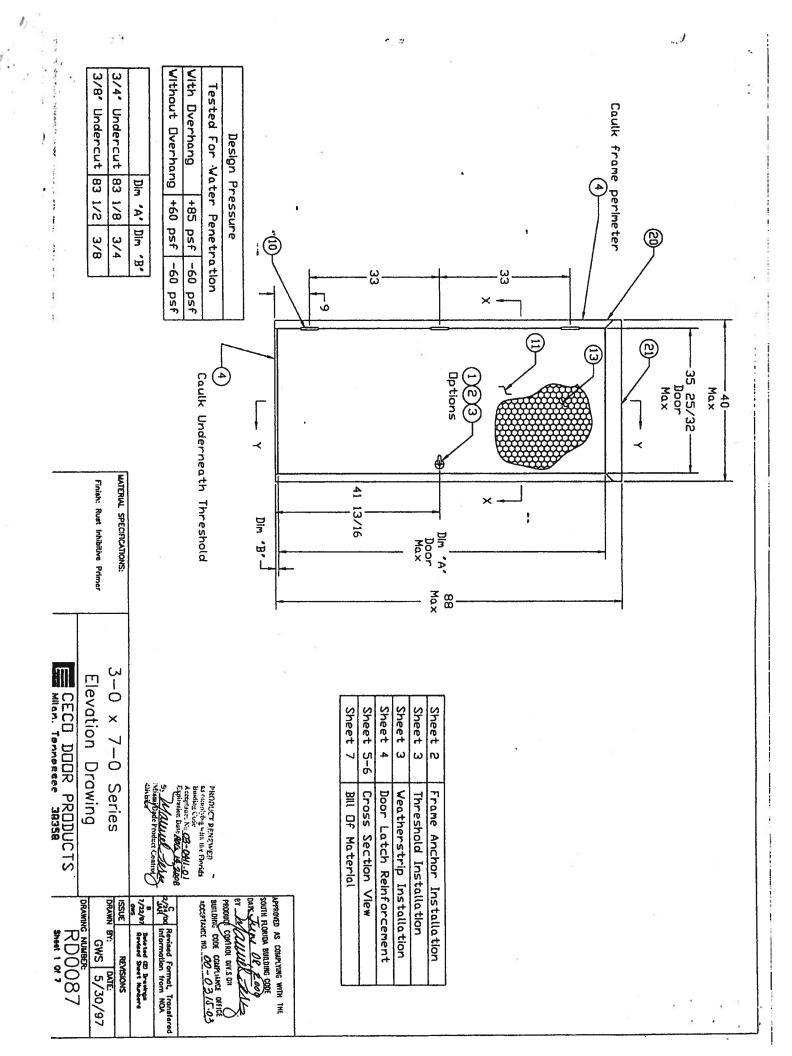
ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

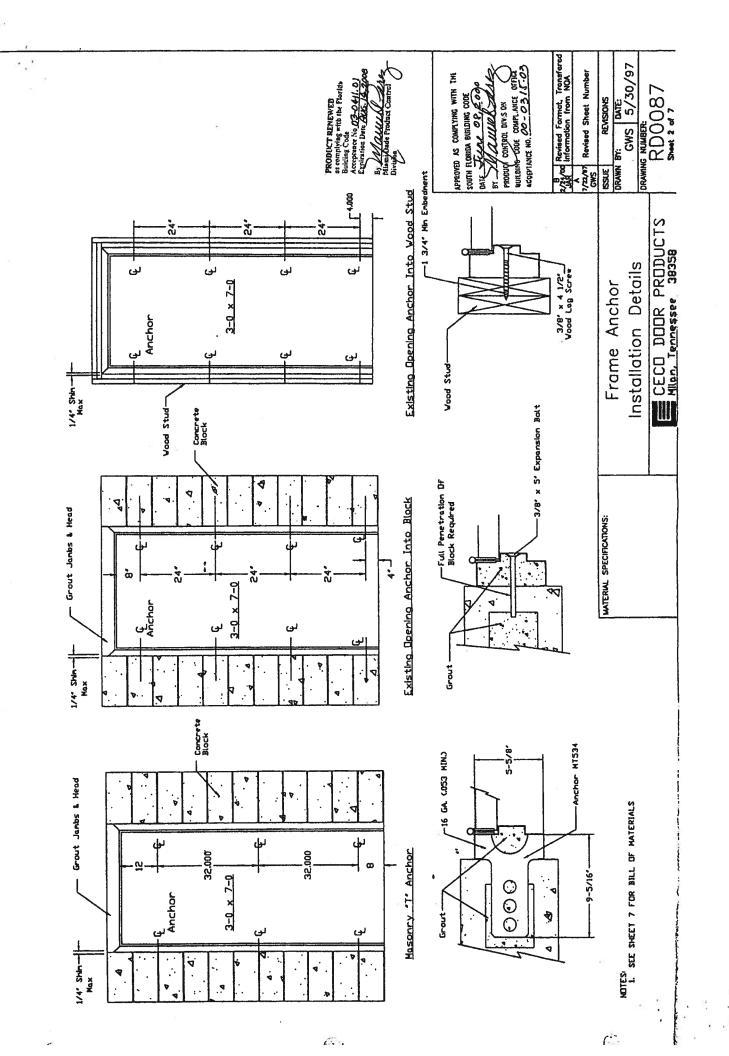
INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

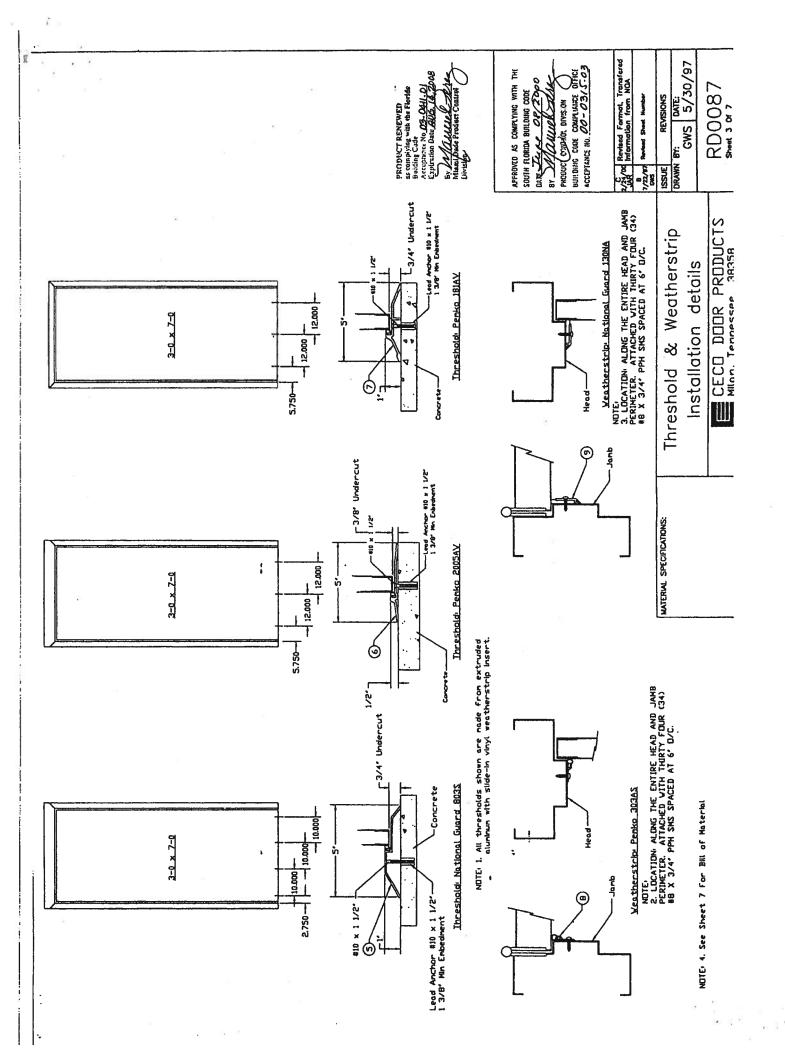
This NOA renews NOA # 00-0315.03 and consists of this page 1 as well as approval document mentioned above. The submitted documentation was reviewed by Manuel Perez, P.E.

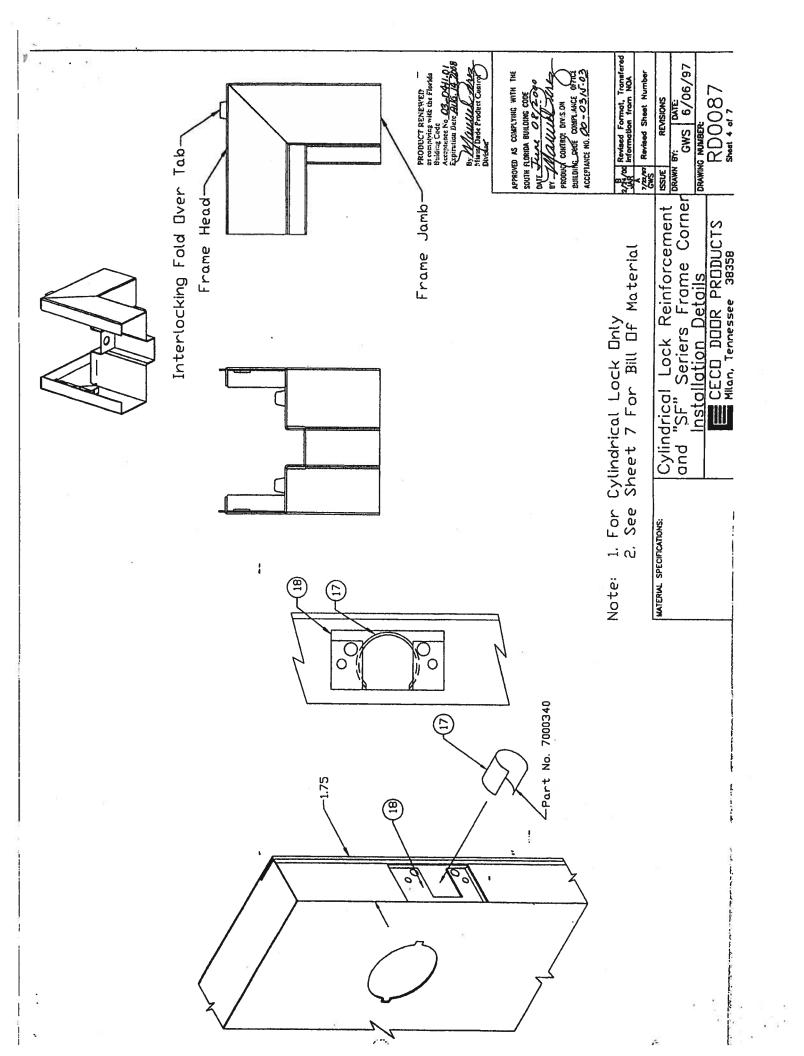


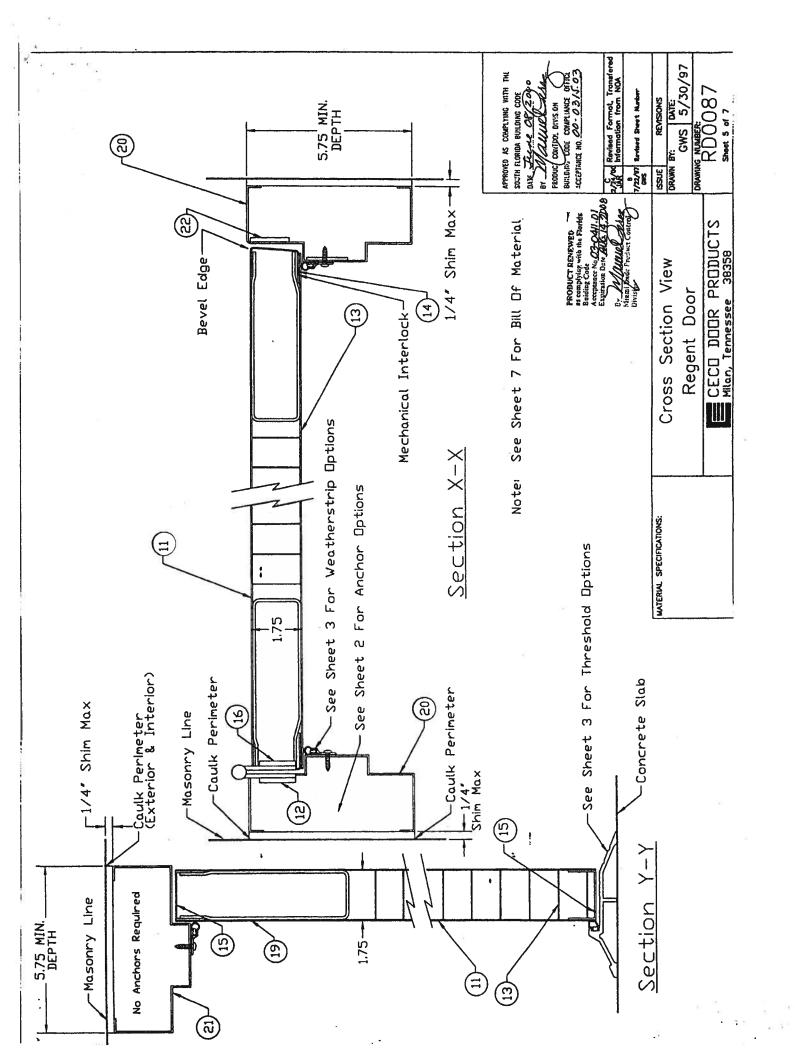
NOA No 03-0411.01 Expiration Date August 14, 2008 Approval Date: May 15, 2003 Page 1

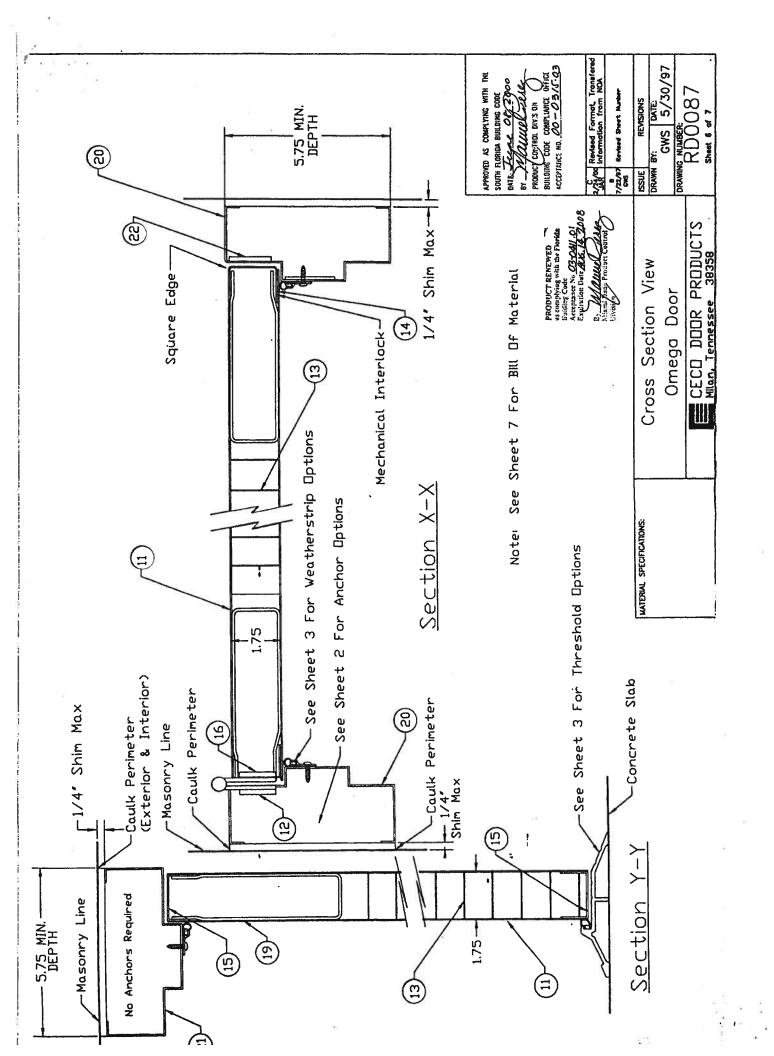












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SIZE											18 GAUGE (.042" MIN. THICK)	1-1/4" X 9" X 7 GA.	1-1/8' CELL		1' X 1-3/4' X 1' X 16 GA (453" MIN)	1-1/4" X 9" X 7 GA.	DIS' THICK X 1.313 INSIDE DIAMETER	16 GA.	ie GA, CUPS-7	2' FACE, 5-3/4' DEPTH MIN.	2" FACE, 5-3/4" DEPTH MIN.	1-1/8' X 2-1/2' X 12 GA.
MATERIAL				GE SILICONE HOUSEHOLD SEALANT							CONNERCIAL QUALITY COLD ROLLED STEEL (MINIMUM YEILD STR. OF Fy=36,000 psd	STEEL	PHENDLIC RESIN-IMPREGNATED KRAFT PAPER				28 GA. GALV.	\$ 1 E E L	STEEL	16 GA. (053" MIN) STEEL COMMERCIAL QUALITY COLD ROLLED STEEL (MINIMUM YEILD STR. OF Fy=40,000 psD	16 GA. (053" MIN) STEEL COLLED STEEL (MINIMUM YEILD STR. OF Fy=40,000 psd	STEEL
DESCRIPTION	SCHLAGE SERIES ALSOPD GRADE 2, LATCH LOCK, SINGLE LEVER OR KNDB OPERATED	HARKS SERIES 170AB GRADE 2, LATCH LDCK, INSIDE/DUTSIDE LEYER OPERATED	YALE SERIES AUS3070 GRADE 2 LATCH LOCK, SINGLE LEVER OR KNOB OPERATED.	CAULK FOR INSTALLATION AND VEATHERSTRIP ADAPTER SCREVS FRANE PERIMETER (INSIDE & COT) AND FRANE SILL CORNERS	NATIONAL GUARD #803S	PENKO #2005AV	PENKO #191AV	VEATHERSTRIP ADAPTER VITH A SILICON (TH) BULB INSERT	NATIONAL GUARD #130NA 1-1/4" VIDE X 0.188" SURFACE APPLIED EXTRUDED ALLMINUM VEATHERSTRIP ADAPT. VITH A FOAM INSERT	HAGAR BBI279, 4-1/2° X 4-1/2° X .0134° THICK SYEEL HINGE EACH ATTACHED VITH EIGHT #12-24 X 1/2° FH MS	FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A568	HINGE REINFORCING PLATE, PLATE SPOT VELDED TO FRAME JAMB AT EACH HINGE LOCATION	CORE. FULL HONEYCOMB CORE PERMANENTLY BONDED TO THE INSIDE OF EACH FACE SKIN VITH NON-FLAMMABLE ADHESIVE	DENFLEX 3500 STRUCTURAL ADHESIVE EPOXY	ROLL FORMED STEEL CHANNEL ON THE TOP AND BOTTOM OF THE DOOR SPOT VELDED TO EXTERIOR AND GLUED TO INTERIOR SKIN	DODR HINGE REINFORCEMENT	DOOR LATCH REINFORCEMENT, STEEL "C" RING	DOOR LOCK REINFORCEMENT	DOOR CLOSER REINFORCEMENT, ROLLED FORM CHANNELS TACK VELDED TO DOOR END CHANNELS	SERIES 'SF', FRAME JAMB, DOUBLE RABBET PROFILE FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A653	SERIES 'SF', FRANE HEAD, DOUBLE RABBET PROFILE FACE SHEET' CONFORMING TO ASTM A366 AND ASTM-A653	JAMB LOCK STRIKE REINFORCING PLATE
YTU	5		-	-	-	_		- ROS	1 R0V	6		(N	-	-		3		-	-	2	-	-
N L	J	જ	3	+	5	9		80	6	OI	=	12	13	7	ž.	16	4	18	19		12	25

SOUTH FLORIDA BUNDUMG CODE

DETL. LIGHT OF THE TANK

PRODUC CORPSCE DIVS ON

BUILDING CODE COMPLANCE OFFICE

CCEPTANCE NO. 00 - 0 3 14 - 0 3 APPROVED AS COMPLYING WITH THE PRODUCT RENEWER
As complying wife the Florids
Bailing Civic

 $3-0 \times 7-0$ Series

MATERIAL SPECIFICATIONS:

Bill Of Materials

CECO DOOR PRODUCTS

2/24/00 Revised Formet, Transfered
1/24/00 Information from NOA
1/22/37 Revised Sheet Number
1/22/37 Revised Formation NOA
1/22/37



January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

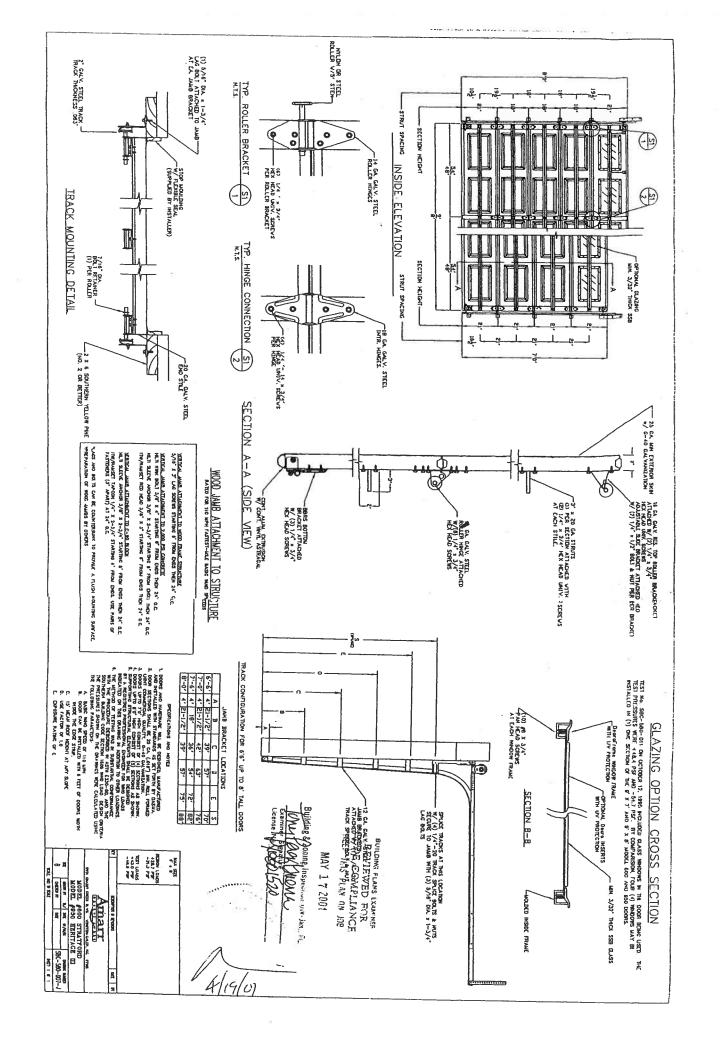
Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Builing Code TAS 100 for wind driven rain.

- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-461.

TAMKO Roofing Products, Inc.



RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
- 2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE -----110 MPH
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERA	L REQUIREME	NTS: Two (2) complete sets of plans containing the following:
Applicant	Plans Examine	r
	o o	All drawings must be clear, concise and drawn to scale ("Optional" details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans. Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
		Site Plan including: a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
		 Wind-load Engineering Summary, calculations and any details required Plans or specifications must state compliance with FBC Section 1609. The following information must be shown as per section 1603.1.4 FBC a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7. c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated. d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient. e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design
7		professional.
		Elevations including:
E.	0	a) All sides b) Roof pitch
R C	D D	,
	u	c) Overhang dimensions and detail with attic ventilation

	0	d) Location, size and height above roof of chimneys.
	0	e) Location and size of skylights
	0	f) Building height
w /		e) Number of stories
		Floor Plan including:
		a) Rooms labeled and dimensioned.
12/		b) Shear walls identified.
	0	c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (see attach forms).
		d) Show safety glazing of glass, where required by code.
G/		e) Identify egress windows in bedrooms, and size.
6		f) Fireplace (gas vented), (gas non-vented) or wood burning with
		hearth, (Please circle applicable type).
	0	g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
B		h) Must show and identify accessibility requirements (accessible bathroom)
		Foundation Plan including:
B	0	a) Location of all load-bearing wall with required footings indicated as standard
/		or monolithic and dimensions and reinforcing.
8		b) All posts and/or column footing including size and reinforcing
		c) Any special support required by soil analysis such as piling
	D	d) Location of any vertical steel.
		Roof System:
8		a) Truss package including:
		1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
		2. Roof assembly (FBC 106.1.1.2)Roofing system, materials,
		manufacturer, fastening requirements and product evaluation with
/		wind resistance rating)
		b) Conventional Framing Layout including:
		1. Rafter size, species and spacing
		2. Attachment to wall and uplift
		3. Ridge beam sized and valley framing and support details
		4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials,
		manufacturer, fastening requirements and product evaluation with
/		wind resistance rating)
		Wall Sections including:
		a) Masonry wall
		1. All materials making up wall
		Block size and mortar type with size and spacing of reinforcement
		3. Lintel, tie-beam sizes and reinforcement
		4. Gable ends with rake beams showing reinforcement or gable truss
		and wall bracing details
		All required connectors with uplift rating and required number and
		size of fasteners for continuous tie from roof to foundation shall b
		designed by a Windload engineer using the engineered roof truss
		nime

- 6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
- 7. Fire resistant construction (if required)
- 8. Fireproofing requirements
- 9. Shoe type of termite treatment (termiticide or alternative method)
- 10. Slab on grade
 - a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
 - b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
- 11. Indicate where pressure treated wood will be placed
- 12. Provide insulation R value for the following:

4/		
19	0	b) Wood frame wall
		1. All materials making up wall
		2. Size and species of studs
		3. Sheathing size, type and nailing schedule
		4. Headers sized
		5. Gable end showing balloon framing detail or gable truss and wall
		hinge bracing detail
		6. All required fasteners for continuous tie from roof to foundation
		(ITUSS anchors, straps, anchor bolts and washers) shall be designed
		by a Windload engineer using the engineered roof truss plans.
		7. Roof assembly shown here or on roof system detail (FBC
		106.1.1.2) Roofing system, materials, manufacturer, fastening
		requirements and product evaluation with wind resistance rating)
		8. Fire resistant construction (if applicable)
		9. Fireproofing requirements
		10. Show type of termite treatment (termiticide or alternative method)
		11. Slab on grade
		a. Vapor retarder (6Mil. Polyethylene with joints lapped 6
		inches and sealed b. Must show control joints, synthetic fiber reinforcement or
		welded wire fabric reinforcement and supports
		12. Indicate where pressure treated wood will be placed
		13. Provide insulation R value for the following:
		a. Attic space
		b. Exterior wall cavity
		c. Crawl space (if applicable)
		c) Metal frame wall and roof (designed, signed and sealed by Florida Prof.
		Engineer or Architect)
		Floor Framing System:
		a) Floor truss package including layout and details, signed and sealed by Florida
		Registered Professional Engineer
		b) Floor joist size and spacing
		c) Girder size and spacing
		d) Attachment of joist to girder
		e) Wind load requirements where applicable
		Plumbing Fixture layout
		Electrical layout including:
B		a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
		b) Ceiling fans
0/		c) Smoke detectors
E		d) Service panel and sub-panel size and location(s)
1		e) Meter location with type of service entrance (overhead or underground)
P		f) Appliances and HVAC equipment
R/	0	g) Arc Fault Circuits (AFCI) in bedrooms
	Ö	h) Exhaust fans in bathroom
	_	HVAC information
B		a) Energy Calculations (dimensions shall match plans)
		h) Manual I sizing equipment or occident across series
<u> </u>	0	b) Manual J sizing equipment or equivalent computation
		Cas System Type (LP or Natural) Location and BTU demand of equipment
		Disclosure Statement for Owner Builders
G		*** Notice Of Commencement Required Before Any Inspections Will Be Done
U		Private Potable Water

a. Attic spaceb. Exterior wall cavityc. Crawl space (if applicable)

- a) Size of pump motor
- b) Size of pressure tank
- c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

- 1. <u>Building Permit Application:</u> A current Building Permit Application form is to be completed and submitted for all residential projects.
- 2. Parcel Number: The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.
- 3. Environmental Health Permit or Sewer Tap Approval: A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued.

 (386) 758-1058 (Toilet facilities shall be provided for construction workers)
- 4. <u>City Approval:</u> If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321
- 5. Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.
 A development permit will also be required. Development permit cost is \$50.00
- 6. <u>Driveway Connection:</u> If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial. <u>If the project is to be located on a F.D.O.T. maintained road, than an F.D.O.T. access permit is required.</u>
- 7. <u>911 Address:</u> If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed colling. A your floride building approved products are listed colling.

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWNGING			
B. SLIDING			
C. SECTIONAL/ROLL UP			
D. OTHER			
2. WINDOWS	Capital		
A SINGLE/DOUBLE HUNG			
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION		•	
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING			
B. SOFFITS			
C. STOREFRONTS	•——		
D. GLASS BLOCK			
E. OTHER	brick		
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES	30 ur Hre		
B. NON-STRUCT METAL	• /		
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS			
B. WOOD ANCHORS			
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR			
ENVELOPE PRODUCTS			
Α.			

The products listed below did not demonstrate product a products, the following information must be available to t characteristics which the product was tested and certified requirements. Further, I understand these products may	the inspector on the jobsite; 1) copy of the product app d to comply with, 3) copy of the applicable manufacture	roval, 2) performance
	APPLICANT SIGNATURE	DATE



161 N.W. Madison St., Suite 102 Lake City, Florida 32055

Tel: 386-758-4209 Fax: 386-758-4290

Wednesday, December 13,2006

Columbia County Building Dept. Lake City, FL. 32055

RE: PERMIT #25005

To Whom It May Concern:

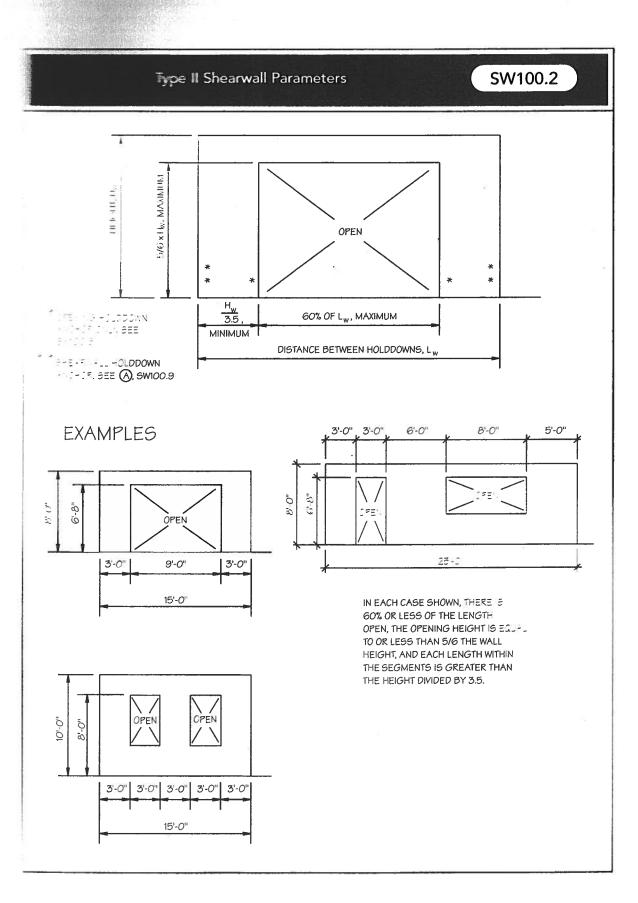
The required shearwall hold downs shall be ½" all thread rod. The locations shall be on the ends of the Type II shearwall. If a Type II shearwall contains an opening of 3 feet or less and is sheathed with Windstorm o.s.b. and nailed according to the detailed fastener pattern, then all thread shall not be required on each side of the opening. If the opening is larger then 3 feet, then a ½" all thread rod shall be placed on each end of the opening. If you have any questions, please call me at (386) 758-4209.

Sincerely,

William H. Freeman, P.E.

President

Certificate of Authorization # 00008701



New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.

Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise. # 25005 Section 1: General Information (Treating Company Information) Company Name: Aspen Pent Control Inc. Company Address: State State State State ___ Company Phone No. _____ FHA/VA Case No. (if any) _____ Section 2: Builder Information Company Name: ______ Company Phone No. _____ Section 3: Property Information Basement Inside ☐ Crawl Other ___ Approximate Depth of Footing: Outside ______ / Z Section 4: Treatment Information Date(s) of Treatment(s) 10-4-04 EPA Registration No. 7969 - 210 Approximate Final Mix Solution % __________________________________ Approximate Size of Treatment Area: Sq. ft. 345-9 Linear ft. 309 Linear ft. of Masonry Voids Approximate Total Gallons of Solution Applied ____ Was treatment completed on exterior? Yes No. Yes Yes Service Agreement Available? ☐ No Upon Zamplotio. Note: Some state laws require service agreements to be issued. This form does not preempt state law. Comments Trusted Austling Only Name of Applicator(s) 57 State law) F104376 The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010. 1012; 31 U.S.C. 3729, 3802)

Authorized Signature ____



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COLUMBIA COUNTY, FLORIDA

tment of Building and Zoning

and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code. This Certificate of Occupancy is issued to the below named permit holder for the building

Parcel Number 11-4S-16-02911-312

Building permit No. 000025005

Use Classification SFD, UTILITY

Fire: 39.06

Waste: 117.25

Permit Holder TRENT GIEBEIG CONSTRUCTION

Owner of Building PETER & HOLLY GIEBEIG

156.31

Total:



Location: 194 SW VANN COURT(MAYFAIR, LOT 12)

Date: 03/05/2007

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)



Project Information for:

L208402

Builder:

GIEBEIG HOMES

Lot:

12

Subdivision:

MAYFAIR COLUMBIA

County: Truss Count:

Design Program: MiTek 20/20 6.2 **Building Code:**

FBC2004/TPI2002

Truss Design Load Information: Gravity:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:
BRIAN TRENT GIEBEIG Florida License No. RR282811523

Address: 462 SW FAIRLINGTON CT, LAKE CITY,, FL

Truss Design Engineer:Lawrence A. Paine, PE Florida P.E. License No. 21475

Company: Builders FirstSource - Florida, LLC Address: 6550 Roosevelt Blvd. Jacksonville, FL 32244

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Lawerence A. Paine, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

Truss ID

T19

T20

T21

T22

T23

T24

T25

T26

T27

Date

8/24/06

8/24/06

8/24/06

8/24/06

8/24/06

8/24/06

8/24/06

8/24/06

8/24/06

Drwg. #

J1711013

J1711014

J1711015

J1711016

J1711017

J1711018

J1711019

J1711020

J1711021

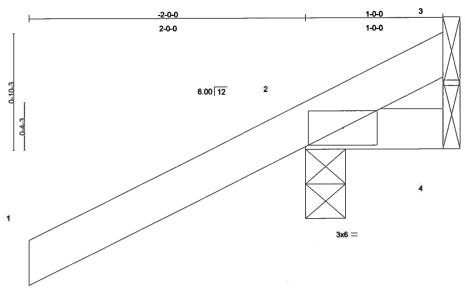
No.	Drwg. #	Truss ID	Date	No.
1	J1710985	CJ1	8/24/06	29
2	J1710986	CJ3	8/24/06	30
3	J1710987	CJ5	8/24/06	31
4	J1710988	EJ3	8/24/06	32
5	J1710989	EJ7	8/24/06	33
6	J1710990	EJ7A	8/24/06	34
7	J1710991	HJ4	8/24/06	35
8	J1710992	HJ9	8/24/06	36
9	J1710993	PB01	8/24/06	37
10	J1710994	PB02	8/24/06	
11	J1710995	PB03	8/24/06	i
12	J1710996	T01	8/24/06	
13	J1710997	T02	8/24/06	
14	J1710998	T03	8/24/06	
15	J1710999	T04	8/24/06	
16	J1711000	T05	8/24/06	
17	J1711001	T06	8/24/06	
18	J1711002	T07	8/24/06	
19	J1711003	T08	8/24/06	
20	J1711004	T09	8/24/06	
21	J1711005	T10	8/24/06	
22	J1711006	T11	8/24/06	
23	J1711007	T12	8/24/06	7
24	J1711008	T13	8/24/06	
25	J1711009	T14	8/24/06	
26	J1711010	T15	8/24/06	1
27	J1711011	T16	8/24/06	
28	J1711012	T18	8/24/06	



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Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
1 000 100	0.14	1401	40			J1710985
L208402	CJ1	JACK	18	'	Job Reference (optional)	

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1-0-0 1-0-0

TCLL 20.0 Plate TCDL 7.0 Lumb BCLL 10.0 Rep	CING 2-0-0 s Increase 1.25 oer Increase 1.25 Stress Incr YES FBC2004/TPI2002	CSI TC BC WB (Mat	0.28 0.01 0.00 rix)	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.00 -0.00 0.00	(loc) 2 2 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 7 lb	GRIP 244/190
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LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **BRACING**

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 1-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

(lb/size) 2=266/0-3-8, 4=14/Mechanical, 3=-90/Mechanical **REACTIONS**

Max Horz 2=87(load case 5)

Max Uplift 2=-286(load case 5), 4=-9(load case 3), 3=-90(load case 1)

Max Grav 2=266(load case 1), 4=14(load case 1), 3=127(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-69/75

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.14

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 Ib uplift at joint 2, 9 lb uplift at joint 4 and 90 lb uplift at joint 3.

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244

LOAD CASE(S) Standard

August 24,2006

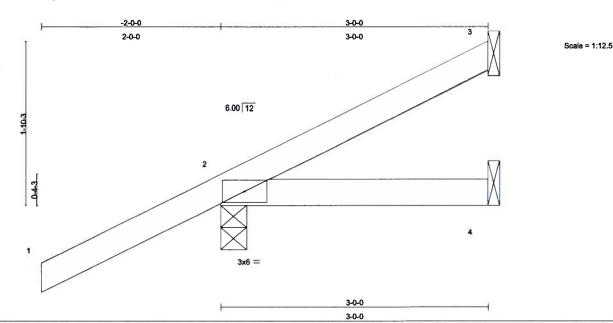
Scale: 1.5"=1"

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek conner. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult BCS-II or HIB-91 Handling Installing and Bracing Recommendation authority Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
						J1710986
L208402	CJ3	JACK	14	1		
	<u> </u>				Job Reference (optional)	

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LOADING TCLL TCDL BCLL	20.0 7.0 10.0	SPACING Plates Increase Lumber Increase Rep Stress Incr	2-0-0 1.25 1.25 YES	CSI TC BC WB	0.29 0.06 0.00	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.00 -0.01 -0.00	(loc) 2-4 2-4 3	I/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 13 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **BRACING TOP CHORD**

Structural wood sheathing directly applied or

3-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical

Max Horz 2=132(load case 5)

Max Uplift 3=-28(load case 6), 2=-203(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-57/7

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.13

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb Truss Design Engineer: Lawrence A. Paine, PE uplift at joint 3 and 203 lb uplift at joint 2.

Florida PE No. 21475

Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

August 24,2006

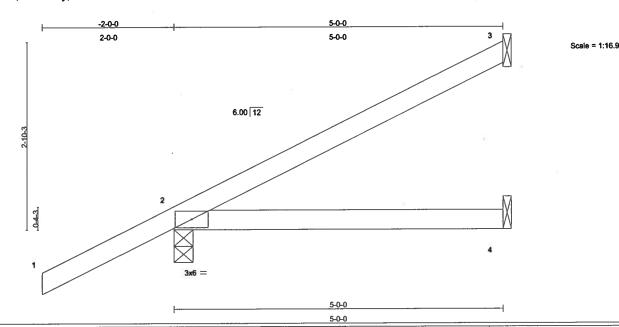
LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	J1710987
L208402	CJ5	JACK	14	1		317 10907
					Job Reference (optional)	

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LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004/TPI2002	CSI TC 0.29 BC 0.16 WB 0.00 (Matrix)	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.03 -0.05 -0.00	(loc) 2-4 2-4 3	I/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 19 lb	GRIP 244/190
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LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **BRACING**

TOP CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical

Max Horz 2=178(load case 5)

Max Uplift 3=-87(load case 5), 2=-199(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-88/36

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.14

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb Truss Design Engineer: Lawrence A. Paine, PE uplift at joint 3 and 199 lb uplift at joint 2.

Florida PE No. 21475 Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

LOAD CASE(S) Standard

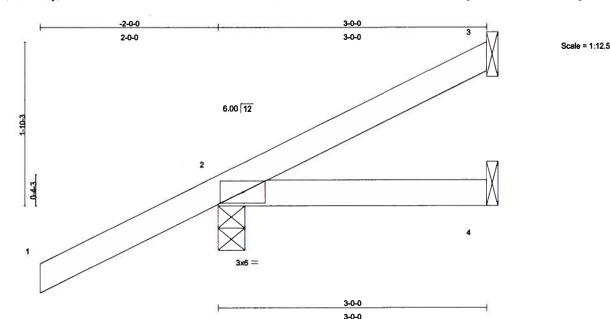
August 24,2006

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Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
				-		J1710988
L208402	EJ3	JACK	3	1		
			-		Job Reference (optional)	

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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 13 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical

Max Horz 2=132(load case 5)

Max Uplift 3=-28(load case 6), 2=-238(load case 5), 4=-27(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-57/7

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.13

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb Truss Design Engineer: Lawrence A. Paine, PE uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4.

Florida PE No. 21475 Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

LOAD CASE(S) Standard

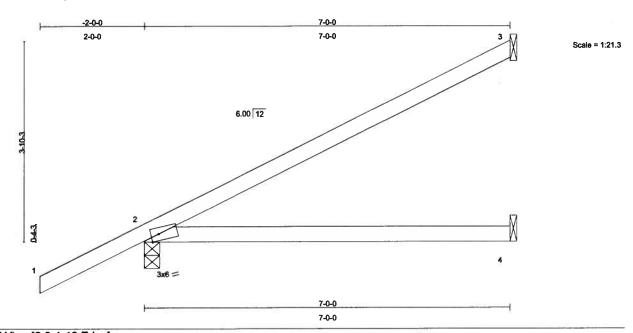
August 24,2006

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connex Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and /or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult 8CS-I or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
	F 17	MONO TRUCO	40			J1710989
L208402	EJ7	MONO TRUSS	42	1	Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.12	2-4	>664	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.21	2-4	>397	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	2002	(Mat	rix)	, ,					Weight: 26 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=419/0-3-8, 4=104/Mechanical

Max Horz 2=224(load case 5)

Max Uplift 3=-134(load case 5), 2=-210(load case 5)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD **BOT CHORD**

1-2=0/47, 2-3=-119/58 2-4=0/0

JOINT STRESS INDEX

2 = 0.77

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 Ib uplift at joint 3 and 210 lb uplift at joint 2.

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244

LOAD CASE(S) Standard

August 24,2006

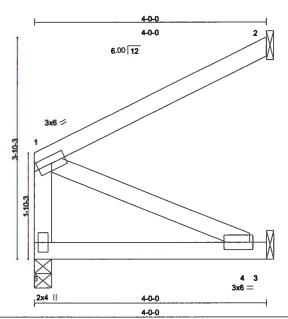
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Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
						J1710990
L208402	EJ7A	MONO TRUSS	5	1		
					Job Reference (optional)	

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Scale = 1:19.2

BCLL 10.0 Rep Stress Incr YES WB 0.04 Horz(TL) -0.00 2 n/a n/a	10.0	20. 7. 10.	20.0 7.0 10.0					DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.01 -0.02 -0.00	(loc) 4-5 4-5 2	I/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 20 lb	GRIP 244/190
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LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 **BRACING**

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=102/Mechanical, 5=159/0-3-8, 3=57/Mechanical

Max Horz 5=87(load case 5)

Max Uplift 2=-93(load case 5), 5=-1(load case 5), 3=-26(load case 5)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-79/37, 1-5=-102/23

BOT CHORD

4-5=-131/0, 3-4=0/0

WEBS

1-4=0/144

JOINT STRESS INDEX

1 = 0.06, 4 = 0.04 and 5 = 0.04

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 93 lb Florida PE No. 21475 uplift at joint 2, 1 lb uplift at joint 5 and 26 lb uplift at joint 3.

Truss Design Engineer: Lawrence A. Paine, PE Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

LOAD CASE(S) Standard

August 24,2006

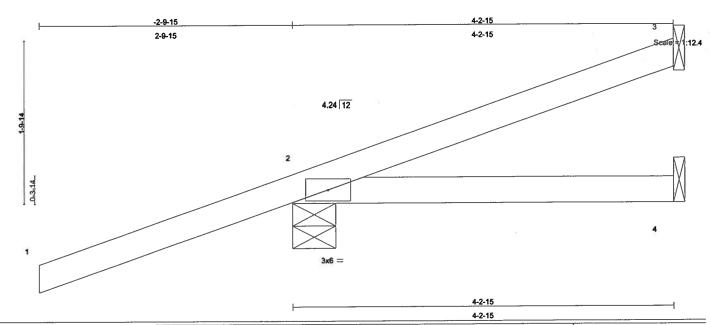
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Ply GIEBEIG HOMES - LOT 12 MAYFAIR Truss Truss Type Qty Job J1710991 JACK 1 L208402 HJ4 Job Reference (optional)

Builders FirstSource, Lake City, FI 32055

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LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 5.0	Plates Increase 1 Lumber Increase 1	-0-0 1.25 1.25 NO 002	CSI TC BC WB (Matr	0.54 0.07 0.00 ix)	DEFL Vert(LL) Vert(TL) Horz(TL)	in 0.02 0.01 -0.00	(loc) 2-4 2-4 3	I/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 18 lb	GRIP 244/190
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LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **BRACING**

TOP CHORD

Structural wood sheathing directly applied or

4-2-15 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 3=13/Mechanical, 2=293/0-5-11, 4=42/Mechanical

Max Horz 2=98(load case 2)

Max Uplift 3=-5(load case 4), 2=-305(load case 2), 4=-41(load case 2)

Max Grav 3=33(load case 5), 2=293(load case 1), 4=42(load case 1)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/50, 2-3=-38/11

BOT CHORD

2-4=0/0

JOINT STRESS INDEX

2 = 0.11

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.

2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 5 lb uplift at joint 3, 305 lb uplift at joint 2 and 41 lb uplift at joint 4.

6550 Roosevelt Blvd. Jacksonville, FL 32244

Florida PE No. 21475

Truss Design Engineer: Lawrence A. Paine, PE

4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or backBuilders FirstSource - Florida, LLC (B).

August 24,2006

Continued on page 2

warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
						J1710991
L208402	HJ4	JACK	2	1		
					Job Reference (optional)	

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LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf) Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-3=-57(F=-2, B=-2), 2=0(F=15, B=15)-to-4=-32(F=-1, B=-1)

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244



•					
Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR
					J1710992
L208402	HJ9	MONO TRUSS	7	1	**
					Job Reference (optional)
Builders FirstSou	rce Lake City Fl	32055 6.:	200 s Jul 13 2005 N	MiTek In	ndustries, Inc. Thu Aug 24 10:20:47 2006 Page 1

9-10-13 -2-9-15 2-9-15 4-3-0 5-7-13 4.24 12 3x6 = 0-3-14 7 2x4 | 4-3-0 9-10-13 4-3-0 5-7-13 2-0-0 CSI DEFL I/defl L/d **PLATES GRIP** LOADING (psf) **SPACING** in (loc) 244/190 >999 240 MT20 TCLL 20.0 Plates Increase 1.25 TC 0.62 Vert(LL) -0.11 6-7 BC 0.61 Vert(TL) -0.18 6-7 >623 180 Lumber Increase 1.25 TCDL 7.0 **BCLL** 10.0 Rep Stress Incr NO **WB** 0.47 Horz(TL) 0.01 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 45 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3 WEBS

BRACING

TOP CHORD

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

Structural wood sheathing directly applied or

bracing.

REACTIONS (lb/size) 4=270/Mechanical, 2=535/0-5-11, 5=374/Mechanical

Max Horz 2=269(load case 2)

Max Uplift 4=-232(load case 2), 2=-281(load case 2), 5=-62(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/50, 2-3=-884/120, 3-4=-105/66

BOT CHORD WEBS

2-7=-308/818, 6-7=-308/818, 5-6=0/0 3-7=0/188, 3-6=-852/321

JOINT STRESS INDEX

2 = 0.74, 3 = 0.22, 6 = 0.23 and 7 = 0.13

NOTES

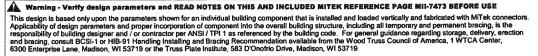
- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 232 Ib uplift at joint 4, 281 Ib uplift at joint 2 and 62 Ib uplift at joint 5.

4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or backFlorida PE No. 21475 (B).

Truss Design Engineer: Lawrence A. Paine, PE

Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244

Continued on page 2





Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR
					J171099
L208402	HJ9	MONO TRUSS	7	1	
				_	Job Reference (optional)

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LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

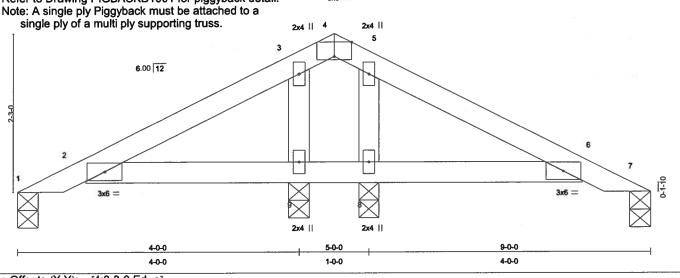
Vert: 1-2=-54 Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244



GIEBEIG HOMES - LOT 12 MAYFAIR Job Truss Truss Type Qty Ply J1710993 **PB01 PIGGYBACK** L208402 Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 10:20:47 2006 Page 1 Builders FirstSource, Lake City, FI 32055 9-0-0 4-6-0 4-6-0 Scale = 1:15.8 Refer to Drawing PIGBACKB1001 for piggyback detail. 3x6 =



LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	0.01	2-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	6-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2004/Ti	PI2002	(Mat	rix)	, ,					Weight: 29 lb	

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

LUMBER

WEBS

2 X 4 SYP No.3

BRACING TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=40/0-3-8, 7=40/0-3-8, 9=310/0-3-8, 8=310/0-3-8

Max Horz 1=32(load case 4)

Max Uplift 1=-12(load case 5), 7=-23(load case 6), 9=-151(load case 5), 8=-140(load

Max Grav 1=59(load case 9), 7=59(load case 10), 9=351(load case 9), 8=351(load case 10)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-27/38, 2-3=-165/281, 3-4=-67/188, 4-5=-67/188, 5-6=-165/281, 6-7=-22/15

BOT CHORD 2-9=-210/215, 8-9=-210/215, 6-8=-210/215

WEBS 3-9=-245/191, 5-8=-245/191

JOINT STRESS INDEX

2 = 0.21, 3 = 0.10, 4 = 0.08, 5 = 0.10, 6 = 0.21, 8 = 0.11 and 9 = 0.11

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; ExpTruss Design Engineer: Lawrence A. Paine, PE B: enclosed: MWFRS gable end zone and C-C Exterior(2) zone: Lumber DOI =1.60 plate grip.

Florida PE No. 21475 B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions 6550 Roosevelt Blvd. Jacksonville, FL 32244 specified.

Builders FirstSource - Florida, LLC

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▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-I1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
						J1710993
L208402	PB01	PIGGYBACK	7	1		
		· · · · · · · · · · · · · · · · · · ·			Job Reference (optional)	

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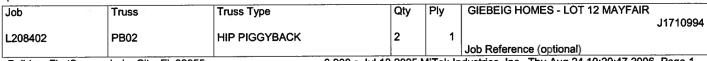
NOTES

- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 12 lb uplift at joint 1, 23 lb uplift at joint 7, 151 lb uplift at joint 9 and 140 lb uplift at joint 8.

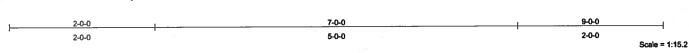
LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244

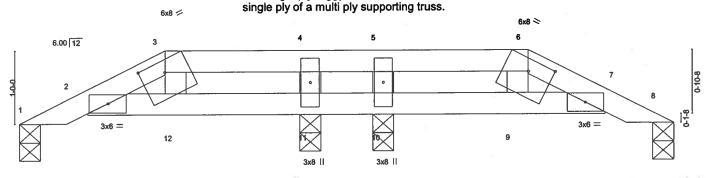




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Refer to Drawing PIGBACKB1001 for piggyback detail. Note: A single ply Piggyback must be attached to a



2-1-12	4-0-0	5-0-0	6-10-4	9-0-0	
2-1-12	1-10-4	1-0-0	1-10-4	2-1-12	
Plate Offsets (X,Y): [3:0-4-0,0-1-15],	[6:0-4-0,0-1-15]			444	

LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	0.01	9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.12	Vert(TL)	-0.02	12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)	, ,					Weight: 26 lb	

			_	_
	М	в		

D

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3 **WFBS**

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 1=138/0-3-8, 8=138/0-3-8, 11=212/0-3-8, 10=212/0-3-8

Max Horz 1=-14(load case 3)

Max Uplift 1=-49(load case 5), 8=-51(load case 6), 11=-87(load case 4), 10=-84(load

case 3)

Max Grav 1=138(load case 1), 8=138(load case 1), 11=242(load case 9), 10=242(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-55/44, 2-3=-128/87, 3-4=-110/91, 4-5=-110/92, 5-6=-110/91, 6-7=-128/87, TOP CHORD

2-12=-41/111, 11-12=-30/110, 10-11=-30/110, 9-10=-30/110, 7-9=-41/111 **BOT CHORD**

3-12=-5/42, 6-9=-5/42, 4-11=-133/110, 5-10=-133/110 **WEBS**

JOINT STRESS INDEX

2 = 0.29, 3 = 0.09, 4 = 0.00, 5 = 0.00, 6 = 0.09, 7 = 0.29, 9 = 0.00, 10 = 0.07, 11 = 0.07 and 12 = 0.00

NOTES

1) Unbalanced roof live loads have been considered for this design.

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475

Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

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Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	4
						J1710994
L208402	PB02	HIP PIGGYBACK	2	1		
			<u> </u>	1	Job Reference (optional)	

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NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 1, 51 lb uplift at joint 8, 87 lb uplift at joint 11 and 84 lb uplift at joint 10.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244



Ply **GIEBEIG HOMES - LOT 12 MAYFAIR** Job Truss Truss Type Qty J1710995 HIP PIGGYBACK L208402 **PB03** Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 10:20:48 2006 Page 1 Builders FirstSource, Lake City, Fl 32055 9-0-0 1-0-0 4-0-0 Scale = 1:15.2 Refer to Drawing PIGBACKB1001 for piggyback detail. 4x6 = 4x6 = Note: A single ply Piggyback must be attached to a single ply of a multi ply supporting truss. 3 6.00 12 3x6 = 2x4 || 4-1-12 9-0-0 0-1-12 LOADING (psf) **SPACING** 2-0-0 CSI DEF (loc) I/defi L/d **PLATES GRIP** in 0.13 TC 0.01 5-7 >999 240 MT20 244/190 TCLL 20.0 Plates Increase 1.25 Vert(LL) 1.25 BC 0.09 Vert(TL) -0.01 >999 180 TCDL 7.0 Lumber Increase 0.00 **BCLL** 10.0 Rep Stress Incr YES **WB** 0.05 Horz(TL) n/a n/a Weight: 29 lb **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) **BRACING LUMBER** TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD 2 X 4 SYP No.2 Rigid ceiling directly applied or 6-0-0 oc 2 X 4 SYP No.3 **BOT CHORD WEBS** bracing. (lb/size) 1=41/0-3-8, 6=41/0-3-8, 8=309/0-3-8, 7=309/0-3-8 REACTIONS Max Horz 1=-28(load case 3) Max Uplift 1=-18(load case 5), 6=-28(load case 6), 8=-146(load case 5), 7=-136(load Max Grav 1=62(load case 9), 6=62(load case 10), 8=366(load case 9), 7=366(load case 10) FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-23/35, 2-3=-147/282, 3-4=-89/234, 4-5=-147/282, 5-6=-23/18 TOP CHORD

BOT CHORD 2-8=-212/198, 7-8=-234/215, 5-7=-212/198

WEBS 3-8=-248/185, 4-7=-248/185

JOINT STRESS INDEX

2 = 0.22, 3 = 0.35, 4 = 0.35, 5 = 0.22, 7 = 0.10 and 8 = 0.10

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475

Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
						J1710995
L208402	PB03	HIP PIGGYBACK	2	1		
					Job Reference (optional)	

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NOTES

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

- 5) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 1, 28 lb uplift at joint 6, 146 lb uplift at joint 8 and 136 lb uplift at joint 7.

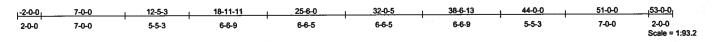
LOAD CASE(S) Standard

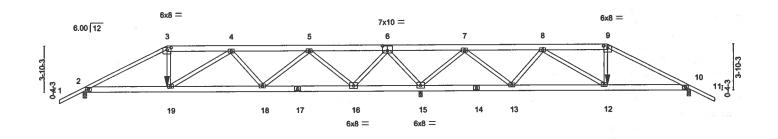
Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	J1710996
L208402	T01	HIP	1	2	Job Reference (optional)	

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	7-0-0	15-0-5		22-8-4		20-3-12	33-11-11	44-4	~	31-0-0	
,	7-0-0	8-0-5		7-7-15	88 6	5-7-8	7-7-15	8-0	-5	7-0-0	
Plate Off	sets (X,Y)	: [3:0-4-0,0-1-15], [6:0-5-0,0-	4-8], [9:0	0-4-0,0-1	-15]					
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.Ó	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.10 18-19	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.27	Vert(TL)	-0.16 18-19	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.03 10	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)	, ,				Weight: 629 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except*

3-6 2 X 6 SYP No.1D, 6-9 2 X 6 SYP No.1D

BOT CHORD 2 X 6 SYP No.1D

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing, Except:

6-0-0 oc bracing: 15-16,13-15.

REACTIONS (lb/size) 2=1983/0-3-8, 15=5908/0-3-8, 10=1327/0-3-8

Max Horz 2=-89(load case 5)

Max Uplift 2=-880(load case 4), 15=-2606(load case 3), 10=-639(load case 5) Max Grav 2=1989(load case 8), 15=5908(load case 1), 10=1337(load case 9)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/51, 2-3=-3706/1548, 3-4=-3295/1439, 4-5=-3278/1418, 5-6=-89/65,

6-7=-1863/4233, 7-8=-425/197, 8-9=-1972/888, 9-10=-2259/916, 10-11=0/51

2-19=-1343/3244, 18-19=-1659/3657, 17-18=-983/2064, 16-17=-983/2064, **BOT CHORD**

15-16=-1766/748, 14-15=-1550/631, 13-14=-1550/631, 12-13=-585/1300,

10-12=-735/1949

3-19=-414/1199, 4-19=-548/425, 4-18=-632/523, 5-18=-472/1609, 5-16=-2684/138 PE No. 21475

. 6-16=-1078/2839, 6-15=-3862/1903, 7-15=-3633/1794, 7-13=-882/2547,

8-13=-1450/880, 8-12=-250/814, 9-12=-123/552

Truss Design Engineer: Lawrence A. Paine, PE

Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

JOINT STRESS INDEX

WEBS

2 = 0.57, 3 = 0.55, 4 = 0.26, 5 = 0.65, 6 = 0.50, 7 = 0.65, 8 = 0.26, 9 = 0.55, 10 = 0.57, 12 = 0.27, 13 = 0.52, 14 = 0.33, 15 = 0.520.40, 16 = 0.40, 17 = 0.33, 18 = 0.52 and 19 = 0.27

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
						J1710996
L208402	T01	HIP	1	2	lab Defending (antique)	
					Job Reference (optional)	

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NOTES

- 2-ply truss to be connected together with 0.131"x3" Nails as follows:
 Top chords connected as follows: 2 X 4 1 row at 0-9-0 oc, 2 X 6 2 rows at 0-9-0 oc.

 Bottom chords connected as follows: 2 X 6 2 rows at 0-9-0 oc.
 Webs connected as follows: 2 X 4 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 5) Provide adequate drainage to prevent water ponding.
- 6) All plates are 4x6 MT20 unless otherwise indicated.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 880 lb uplift at joint 2, 2606 lb uplift at joint 15 and 639 lb uplift at joint 10.
- 9) Girder carries hip end with 7-0-0 end setback.

LOAD CASE(S) Standard

1) Regular: Lumber increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-9=-118(F=-64), 9-11=-54, 2-19=-30, 12-19=-65(F=-35), 10-12=-30

Concentrated Loads (lb)

Vert: 19=-539(F) 12=-539(F)

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR
					J1710997
L208402	T02	HIP	1	1	
					Job Reference (optional)
Builders FirstSo	urce, Lake City, FI	32055	6.200 s Jul 13 2005	MiTek Ir	ndustries, Inc. Thu Aug 24 10:20:49 2006 Page 1

5-6-10

31-0-10

5-6-10

37-7-5

6-6-11

42-0-0

4-4-12

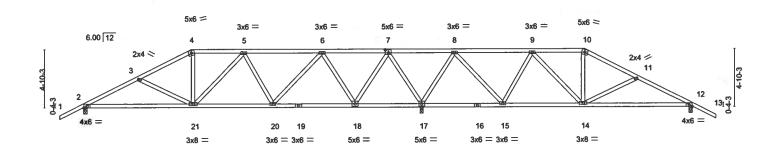
4-4-1

51-0-0

4-7-15

53-0-0

2-0-0 Scale = 1:93.2



	9	HO-O 15-10-	2	22-8-4		28-3-12	35-1-14	-	42-0-0		51-0-0	4
	9	LO-0 6-10-2	2	6-10-2	1.5	5-7-8	6-10-2		6-10-2	8.	9-0-0	
Plate Of	fsets (X,Y)): [7:0-3-0,0-3-0]				-						
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.16	2-21	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.28	2-21	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.81	Horz(TL)	0.04	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)	, ,					Weight: 269	lb
LUMBER	R	•				BRACING					,	
TOP CH	ORD 2X	(4 SYP No.2				TOP CHO	RD	Structu	ıral wood	sheathir	ng directly applic	ed or
BOT CH	ORD 2X	(4 SYP No.2						4-8-7	oc purlins	3.		
WEBS	2 X	(4 SYP No.3				вот сно	RD	Rigid of bracing	-	ectly app	olied or 6-0-0 oc	

REACTIONS (lb/size) 2=1062/0-3-8, 17=2663/0-3-8, 12=766/0-3-8

4-4-12

6-6-11

Max Horz 2=-101(load case 6)

Max Uplift 2=-421(load case 5), 17=-879(load case 4), 12=-369(load case 6) Max Grav 2=1076(load case 9), 17=2663(load case 1), 12=786(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1618/687, 3-4=-1359/571, 4-5=-1175/564, 5-6=-1007/501,

6-7=-47/136, 7-8=-421/1479, 8-9=-9/125, 9-10=-601/341, 10-11=-728/325,

11-12=-1001/446, 12-13=0/47

BOT CHORD 2-21=-452/1406, 20-21=-381/1173, 19-20=-206/554, 18-19=-206/554,

17-18=-666/391, 16-17=-787/428, 15-16=-787/428, 14-15=-57/331, 12-14=-239/860

WEBS 3-21=-272/222, 4-21=-73/366, 5-21=-86/152, 5-20=-367/203, 6-20=-194/696,

6-18=-1001/457, 7-18=-413/1222, 7-17=-1548/646, 8-17=-1344/588,

8-15=-343/1086, 9-15=-702/331, 9-14=-106/424, 10-14=0/88, 11-14=-293/231

Truss Design Engineer: Lawrence A. Paine, PE

Florida PE No. 21475

Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

JOINT STRESS INDEX

-2-0-01

2-0-0

4-7-15

2 = 0.72, 3 = 0.33, 4 = 0.28, 5 = 0.43, 6 = 0.68, 7 = 0.62, 8 = 0.68, 9 = 0.43, 10 = 0.28, 11 = 0.33, 12 = 0.72, 14 = 0.56, 15 = 0.68, 16 = 0.29, 17 = 0.46, 18 = 0.46, 19 = 0.29, 20 = 0.68 and 21 = 0.56

Continued on page 2



	Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
ì							J1710997
	L208402	T02	HIP	1	1		
						Job Reference (optional)	

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NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 421 lb uplift at joint 2, 879 lb uplift at joint 17 and 369 lb uplift at joint 12.

LOAD CASE(S) Standard

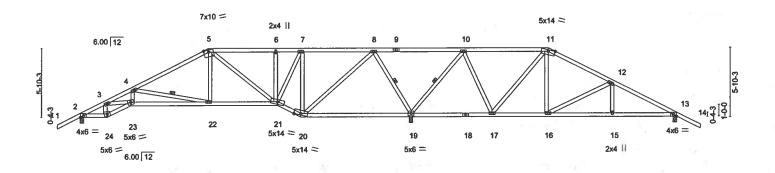
Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
	2		١.	١.,		J1710998
L208402	T03	SPECIAL	1	1	Joh Deference (autional)	
	<u> </u>				Job Reference (optional)	

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	2-3-8 4-3-8	11-0-0	16-10-8	18-10-8	28-3-12		35-2-11	4	0-0-0	45-5-11	51-0-0	
	2-3-8 2-0-0	6-8-8	5-10-8	2-0-0	9-5-4		6-10-15		1-9-5	5-5-11	5-6-5	
LOADII	NG (psf)	SPACING	2-0-	o cs	I	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.Ó	Plates Increase	1.2	5 TC	0.64	Vert(LL)	-0.19	22-23	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.2	5 BC	0.60	Vert(TL)	-0.31	22-23	>999	180		
BCLL	10.0	Rep Stress Incr	YES	s WE	0.77	Horz(TL)	0.10	13	n/a	n/a		
BCDL	5.0	Code FBC2004/	TPI2002	! (M:	atrix)	, ,					Weight: 287 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WFBS	2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or

3-8-8 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing, Except: 7-4-4 oc bracing: 22-23

6-0-0 oc bracing: 19-20,17-19.

WEBS

1 Row at midpt

4-22, 8-19, 10-19

REACTIONS (lb/size) 2=970/0-3-8, 19=2869/0-3-8, 13=652/0-3-8

Max Horz 2=-115(load case 6)

Max Uplift 2=-408(load case 5), 19=-873(load case 4), 13=-368(load case 6) Max Grav 2=991(load case 9), 19=2869(load case 1), 13=713(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1497/503, 3-4=-2722/978, 4-5=-1233/511, 5-6=-637/373,

6-7=-637/374, 7-8=-327/282, 8-9=-467/1672, 9-10=-467/1672, 10-11=-68/418,

11-12=-386/232, 12-13=-899/334, 13-14=0/47

BOT CHORD 2-24=-323/1262, 23-24=-333/1346, 22-23=-732/2357, 21-22=-277/1055,

20-21=-118/309, 19-20=-861/463, 18-19=-702/412, 17-18=-702/412, 16-17=-20/300

, 15-16=-159/740, 13-15=-159/740

WEBS 3-24=-583/180, 3-23=-469/1251, 4-23=-90/672, 4-22=-1340/570, 5-22=-70/475,

5-21=-565/209, 6-21=-226/160, 7-21=-255/740, 7-20=-865/367, 8-20=-515/1500,

8-19=-1616/726, 10-19=-1543/662, 10-17=-238/852, 11-17=-830/332,

11-16=-90/375, 12-16=-506/259, 12-15=0/182

Truss Design Engineer: Lawrence A. Paine, PE

Florida PE No. 21475

Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

August 24,2006

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR
					J1710998
L208402	T03	SPECIAL	1	1	
					Job Reference (optional)

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JOINT STRESS INDEX

2 = 0.52, 3 = 0.60, 4 = 0.48, 5 = 0.68, 6 = 0.33, 7 = 0.62, 8 = 0.90, 9 = 0.62, 10 = 0.72, 11 = 0.86, 12 = 0.39, 13 = 0.33, 15 = 0.33, 16 = 0.34, 17 = 0.72, 18 = 0.16, 19 = 0.31, 20 = 0.52, 21 = 0.39, 22 = 0.37, 23 = 0.72 and 24 = 0.33

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are 3x6 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 408 lb uplift at joint 2, 873 lb uplift at joint 19 and 368 lb uplift at joint 13.

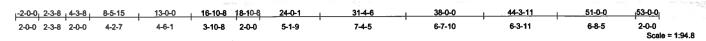
LOAD CASE(S) Standard

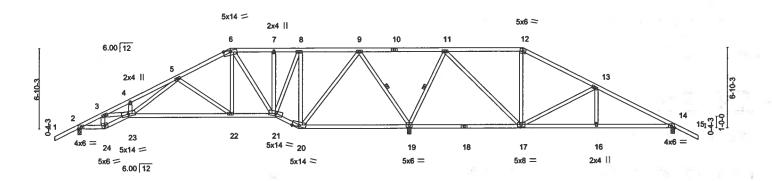
Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	14740000
L208402	T04	SPECIAL	1	1		J1710999
					Job Reference (optional)	

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2-3-8 4-3-8	13-0-0	16-10-8	18-10-8	28-3-12		38-0-0		44-	3-11	51-0-0	
2-3-8 2-0-0	8-8-8	3-10-8	2-0-0	9-5-4		9-8-4		6-3	<u>-11</u>	6-8-5	
G (psf)	SPACING	2-0-0	csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
20.0	Plates Increase	1.25	5 TC	0.57	Vert(LL)	-0.24 2	2-23	>999	240	MT20	244/190
7.0	Lumber Increase	1.25	5 BC	0.47	Vert(TL)	-0.40 2	2-23	>845	180		
10.0	Rep Stress Incr	YES	s wb	0.64	Horz(TL)	0.08	19	n/a	n/a		
5.0	Code FBC2004/T	PI2002	(Mat	trix)	, ,					Weight: 299 lb	
	2-3-8 2-0-0 G (psf) 20.0 7.0 10.0	2.3-8 2-0-0 8-8-8 G (psf) SPACING 20.0 Plates Increase 10.0 Rep Stress Incr	G (psf) SPACING 2-0-0 20.0 Plates Increase 1.25 7.0 Lumber Increase 1.25 10.0 Rep Stress Incr YES	2-3-8 2-0-0 8-8-8 3-10-8 2-0-0 G (psf) SPACING 2-0-0 CSI 20.0 Plates Increase 1.25 TC 7.0 Lumber Increase 1.25 BC 10.0 Rep Stress Incr YES WB	Company SPACING 2-0-0 CSI 20.0 Plates Increase 1.25 TC 0.57 7.0 Lumber Increase 1.25 BC 0.47 10.0 Rep Stress Incr YES WB 0.64	G (psf) SPACING 2-0-0 CSI DEFL 20.0 Plates Increase 1.25 TC 0.57 Vert(LL) 7.0 Lumber Increase 1.25 BC 0.47 Vert(TL) 10.0 Rep Stress Incr YES WB 0.64 Horz(TL)	G (psf) SPACING 2-0-0 CSI DEFL in 20.0 Plates Increase 1.25 TC 0.57 Vert(LL) -0.24 2 7.0 Lumber Increase 1.25 BC 0.47 Vert(TL) -0.40 2 10.0 Rep Stress Incr YES WB 0.64 Horz(TL) 0.08	Company SPACING 2-0-0 CSI DEFL in (loc) 20.0 Plates Increase 1.25 TC 0.57 Vert(LL) -0.24 22-23 7.0 Lumber Increase 1.25 BC 0.47 Vert(TL) -0.40 22-23 10.0 Rep Stress Incr YES WB 0.64 Horz(TL) 0.08 19	G (psf) SPACING 2-0-0 CSI DEFL in (loc) l/defl 20.0 Plates Increase 1.25 TC 0.57 Vert(LL) -0.24 22-23 >999 7.0 Lumber Increase 1.25 BC 0.47 Vert(TL) -0.40 22-23 >845 10.0 Rep Stress Incr YES WB 0.64 Horz(TL) 0.08 19 n/a	Company SPACING 2-0-0 CSI DEFL in (loc) l/defl L/d 20.0 Plates Increase 1.25 TC 0.57 Vert(LL) -0.24 22-23 >999 240 7.0 Lumber Increase 1.25 BC 0.47 Vert(TL) -0.40 22-23 >845 180 10.0 Rep Stress Incr YES WB 0.64 Horz(TL) 0.08 19 n/a n/a	G (psf) SPACING 2-0-0 CSI DEFL in (loc) l/defl L/d PLATES 20.0 Plates Increase 1.25 TC 0.57 Vert(LL) -0.24 22-23 >999 240 MT20 7.0 Lumber Increase 1.25 BC 0.47 Vert(TL) -0.40 22-23 >845 180 10.0 Rep Stress Incr YES WB 0.64 Horz(TL) 0.08 19 n/a n/a

LUMBEK	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WERS	2 X 4 SYP No 3

BRACING TOP CHORD

Structural wood sheathing directly applied or

3-8-13 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing, Except:

6-0-0 oc bracing: 19-20,17-19.

WEBS

1 Row at midpt

9-19, 11-19

REACTIONS (lb/size) 2=949/0-3-8, 19=2916/0-3-8, 14=626/0-3-8

Max Horz 2=129(load case 5)

Max Uplift 2=-414(load case 5), 19=-805(load case 4), 14=-378(load case 6)

Max Grav 2=978(load case 9), 19=2916(load case 1), 14=708(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1472/526, 3-4=-2588/882, 4-5=-2585/976, 5-6=-905/441,

6-7=-466/345, 7-8=-466/345, 8-9=-232/274, 9-10=-368/1499, 10-11=-368/1499,

11-12=-120/240, 12-13=-220/229, 13-14=-826/347, 14-15=0/47

BOT CHORD 2-24=-360/1252, 23-24=-365/1336, 22-23=-353/1251, 21-22=-188/774,

20-21=-92/204, 19-20=-617/393, 18-19=-908/510, 17-18=-908/510, 16-17=-157/667

, 14-16=-157/667

3-24=-607/189, 3-23=-330/1120, 4-23=-128/175, 5-23=-392/1338, 5-22=-604/359,

6-22=-164/609, 6-21=-585/184, 7-21=-165/111, 8-21=-209/659, 8-20=-778/314,

9-20=-414/1244, 9-19=-1606/703, 11-19=-1399/639, 11-17=-428/1229,

12-17=-349/191, 13-17=-616/348, 13-16=0/199

Truss Design Engineer: Lawrence A. Paine, PE

Florida PE No. 21475

Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

JOINT STRESS INDEX

WEBS

2 = 0.51, 3 = 0.53, 4 = 0.33, 5 = 0.83, 6 = 0.38, 7 = 0.33, 8 = 0.58, 9 = 0.87, 10 = 0.75, 11 = 0.76, 12 = 0.69, 13 = 0.39, 14 = 0.390.31, 16 = 0.33, 17 = 0.57, 18 = 0.45, 19 = 0.30, 20 = 0.51, 21 = 0.36, 22 = 0.39, 23 = 0.50 and 24 = 0.38

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
						J1710999
L208402	T04	SPECIAL	1	1	lab Deference (actional)	
					Job Reference (optional)	

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NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are 3x6 MT20 unless otherwise indicated.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 414 lb uplift at joint 2, 805 lb uplift at joint 19 and 378 lb uplift at joint 14.

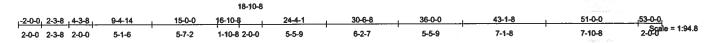
LOAD CASE(S) Standard

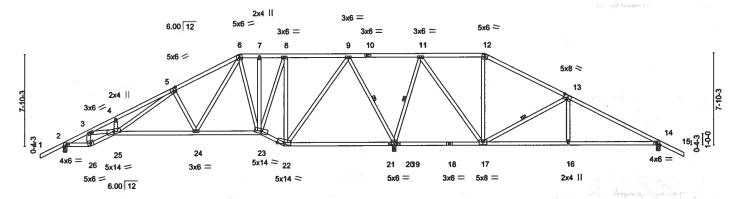
Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
L208402	T05	SPECIAL	1	1	J1711	000
					Job Reference (optional)	

6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 10:20:51 2006 Page 1





2-3-8	4-3-8	11-3-8	16-10-8	18-10-8	28-3-12	36-0-0	43-1-8	51-0-0
2-3-8	2-0-0	7-0-0	5-7-0	2-0-0	9-5-4	7-8-4	7-1-8	7-10-8

Plate Of	fsets (X,Y)): [5:0-3-0,0-3-0], [1:	3:0-4-0,0-3	3-0]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.19	24-25	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.44	Vert(TL)	-0.30	24-25	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.82	Horz(TL)	0.08	20	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	212002	(Mat	rix)	' '					Weight: 316 lb	

LL	JMB	ER
----	-----	----

TOP CHORD 2 X 4 SYP No.2

WEBS

BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-9-7 oc purlins.

BOT CHORD Rigid ceiling directly applied or 5-9-1 oc

bracing.

WEBS 1 Row at midpt 9-20, 11-20, 13-17

REACTIONS (lb/size) 2=920/0-3-8, 20=2982/0-3-8, 14=589/0-3-8

Max Horz 2=143(load case 5)

Max Uplift 2=-411(load case 5), 20=-743(load case 5), 14=-380(load case 6)

Max Grav 2=957(load case 9), 20=2982(load case 1), 14=699(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/47, 2-3=-1447/504, 3-4=-2444/869, 4-5=-2455/979, 5-6=-1054/544. **TOP CHORD**

6-7=-343/308, 7-8=-343/308, 8-9=-156/263, 9-10=-311/1409, 10-11=-311/1409,

11-12=-39/408, 12-13=-83/469, 13-14=-749/331, 14-15=0/47

2-26=-364/1226, 25-26=-369/1296, 24-25=-318/1126, 23-24=-124/481, **BOT CHORD**

22-23=-61/241, 21-22=-687/459, 20-21=-687/459, 19-20=-1015/598,

18-19=-1015/598, 17-18=-1015/598, 16-17=-128/587, 14-16=-128/590

3-26=-533/195, 3-25=-346/1012, 4-25=-164/204, 5-25=-458/1306, 5-24=-534/370,

6-24=-335/804, 6-23=-578/290, 7-23=-54/32, 8-23=-179/615, 8-22=-810/314, 9-22=-441/1282, 9-20=-1550/677, 11-20=-1393/622, 11-17=-471/1251,

12-17=-462/227, 13-17=-755/421, 13-16=0/260

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475

Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

August 24,2006

Continued on page 2

WEBS



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
						J1711000
L208402	T05	SPECIAL	1	1		
	101				Job Reference (optional)	

6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 10:20:51 2006 Page 2

JOINT STRESS INDEX

2 = 0.50, 3 = 0.48, 4 = 0.33, 5 = 0.57, 6 = 0.40, 7 = 0.33, 8 = 0.56, 9 = 0.93, 10 = 0.52, 11 = 0.90, 12 = 0.69, 13 = 0.70, 14 = 0.29, 16 = 0.33, 17 = 0.60, 18 = 0.21, 19 = 0.00, 19 = 0.00, 20 = 0.34, 20 = 0.00, 21 = 0.00,

NOTES

- 1) 2 X 4 SYP No.2 bearing block 12" long at jt. 20 attached to front face with 2 rows of 0.131"x3" Nails spaced 3" o.c. 8 Total fasteners. Bearing Fc perp is assumed to be 565 psi.
- 2) Unbalanced roof live loads have been considered for this design.
- 3) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 411 lb uplift at joint 2, 743 lb uplift at joint 20 and 380 lb uplift at joint 14.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244



Qty Ply **GIEBEIG HOMES - LOT 12 MAYFAIR** Job Truss Truss Type J1711001 T06 SPECIAL L208402 Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 10:20:51 2006 Page 1 Builders FirstSource, Lake City, FI 32055 39-10-13 45-6-9 51-0-0 53-0-0 26-3-10 10-6-11 17-0-0 18-10-6 -2-0-0, 2-3-8 4-3-8 7-5-2 7-8-6 5-10-13 5-7-11 5-5-7 2-0-0 1-10-8 2-0-0 2-3-8 2-0-0 6-3-2 6-5-5 Scale = 1:94.8 5x6 = 6.00 12 7x10 = 4x6 = 6 8 5x8 / 3x6 <> 10 5x6 > 11 22 21 4x6 = 14 19 1817 16 15 24 5x8 5x6 = 3x6 = 3x6 = 3x6 = 5x14 = 6x8 = 6.00 12 3x6 = 34-0-0 42-6-15 51-0-0 28-3-12 16-10-8 18-10-8 5-8-4 8-6-15 8-5-1 6-3-13 2-0-0 9-5-4 2-3-8 2-0-0 6-3-2 [5:0-3-8,0-3-4], [6:0-2-3,Edge], [11:0-3-0,0-3-0], [21:0-4-8,0-0-12] Plate Offsets (X,Y): **GRIP PLATES** L/d **SPACING** 2-0-0 CSI **DEFL** in (loc) I/defl LOADING (psf) 240 MT20 244/190 >999 -0.17 18-20 20.0 TC 0.61 Vert(LL) TCLL Plates Increase 1.25 -0.29 18-20 >999 180 BC 1.25 0.62 Vert(TL) TCDL 7.0 Lumber Increase **WB** 0.89 Horz(TL) 0.09 18 n/a n/a 10.0 Rep Stress Incr YES **BCLL** Weight: 315 lb **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) **BRACING** LUMBER Structural wood sheathing directly applied or TOP CHORD 2 X 4 SYP No.2 TOP CHORD 3-11-7 oc purlins. BOT CHORD 2 X 4 SYP No.2 **BOT CHORD** Rigid ceiling directly applied or 5-11-12 oc 2 X 4 SYP No.3 **WEBS** bracing. 7-20, 8-20, 8-18, 9-18 1 Row at midpt **WEBS** , 10-15

(lb/size) 2=889/0-3-8, 18=3051/0-3-10 (0-3-8 + bearing block), 12=551/0-3-8 REACTIONS

Max Horz 2=-157(load case 6)

Max Uplift 2=-404(load case 5), 18=-798(load case 5), 12=-375(load case 6) Max Grav 2=938(load case 9), 18=3051(load case 1), 12=686(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1400/467, 3-4=-2479/879, 4-5=-1163/486, 5-6=-385/261,

6-7=-269/303, 7-8=-107/257, 8-9=-273/1345, 9-10=-124/670, 10-11=-558/309,

11-12=-792/369, 12-13=0/47

2-24=-352/1179, 23-24=-362/1255, 22-23=-730/2141, 21-22=-259/996, **BOT CHORD**

20-21=-67/298, 19-20=-978/616, 18-19=-978/616, 17-18=-564/447, 16-17=-564/447

, 15-16=-564/447, 14-15=-352/206, 12-14=-188/653

3-24=-535/201, 3-23=-416/1105, 4-23=-106/619, 4-22=-1179/482, 5-22=-61/463,

5-21=-875/451, 6-21=-125/100, 7-21=-250/648, 7-20=-860/334, 8-20=-548/1492, Truss Design Engineer: Lawrence A. Paine, PE

8-18=-1605/742, 9-18=-1502/612, 9-15=-228/618, 10-15=-633/394, 10-14=-132/539 No. 21475

, 11-14=-301/267

Builders FirstSource - Florida, LL:C

6550 Roosevelt Blvd. Jacksonville, FL 32244

August 24,2006

Continued on page 2

WEBS



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
						J1711001
L208402	T06	SPECIAL	1	1		
	1 m	A 100 A			Job Reference (optional)	

6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 10:20:51 2006 Page 2

JOINT STRESS INDEX

2 = 0.49, 3 = 0.53, 4 = 0.45, 5 = 0.45, 6 = 0.48, 7 = 0.62, 8 = 0.67, 9 = 0.81, 10 = 0.40, 11 = 0.37, 12 = 0.41, 14 = 0.45, 15 = 0.39, 16 = 0.23, 17 = 0.00, 17 = 0.00, 18 = 0.33, 18 = 0.00, 19 = 0.00, 19 = 0.00, 20 = 0.58, 21 = 0.40, 22 = 0.34, 23 = 0.66 and 24 = 0.31

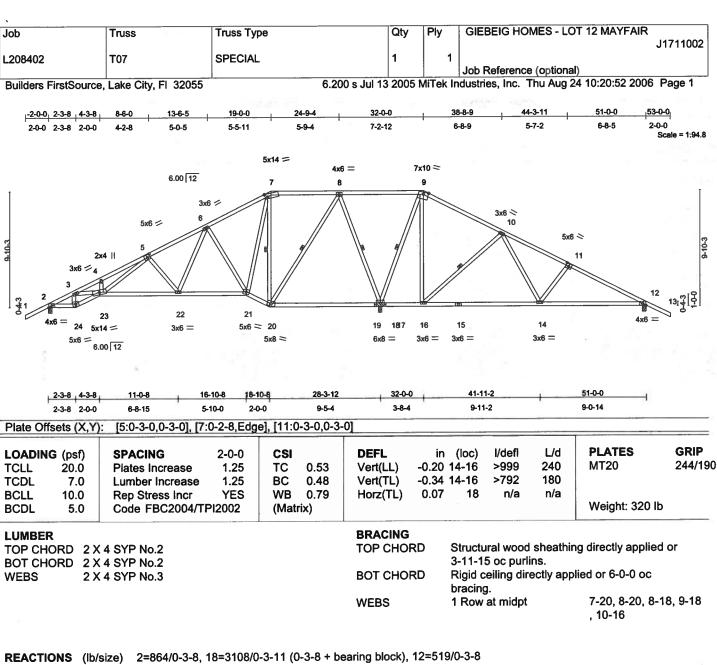
NOTES

- 1) 2 X 4 SYP No.2 bearing block 12" long at jt. 18 attached to front face with 2 rows of 0.131"x3" Nails spaced 3" o.c. 8 Total fasteners. Bearing Fc perp is assumed to be 565 psi.
- 2) Unbalanced roof live loads have been considered for this design.
- 3) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 404 lb uplift at joint 2, 798 lb uplift at joint 18 and 375 lb uplift at joint 12.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244





Max Horz 2=-171(load case 6)

Max Uplift 2=-398(load case 5), 18=-846(load case 5), 12=-371(load case 6) Max Grav 2=920(load case 9), 18=3108(load case 1), 12=677(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/47, 2-3=-1375/461, 3-4=-2297/782, 4-5=-2297/876, 5-6=-975/462. TOP CHORD

6-7=-292/274, 7-8=-57/259, 8-9=-241/1290, 9-10=-186/923, 10-11=-504/317,

11-12=-720/333, 12-13=0/47

BOT CHORD 2-24=-370/1165, 23-24=-377/1232, 22-23=-361/1127, 21-22=-133/559,

20-21=-128/343, 19-20=-741/559, 18-19=-741/559, 17-18=-738/570,

16-17=-738/570, 15-16=-483/248, 14-15=-483/248, 12-14=-205/577

3-24=-506/203, 3-23=-337/935, 4-23=-138/176, 5-23=-412/1164, 5-22=-502/319,

6-22=-234/676, 6-21=-702/423, 7-21=-249/636, 7-20=-811/330, 8-20=-484/1312, 8-18=-1578/700, 9-18=-1531/645, 9-16=-257/662, 10-16=-713/450, 10-14=-189/64 Plorida PE No. 21475

, 11-14=-319/290

Truss Design Engineer: Lawrence A. Paine, PE

Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

August 24,2006

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WEBS



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
					6	J1711002
L208402	T07	SPECIAL	1	1		
					Job Reference (optional)	

6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 10:20:52 2006 Page 2

JOINT STRESS INDEX

2 = 0.48, 3 = 0.45, 4 = 0.33, 5 = 0.36, 6 = 0.39, 7 = 0.82, 8 = 0.68, 9 = 0.95, 10 = 0.43, 11 = 0.46, 12 = 0.36, 14 = 0.52, 15 = 0.47, 16 = 0.42, 17 = 0.00, 17 = 0.00, 18 = 0.28, 18 = 0.00, 19 = 0.00, 19 = 0.00, 20 = 0.55, 21 = 0.26, 22 = 0.58, 23 = 0.39 and 24 = 0.31

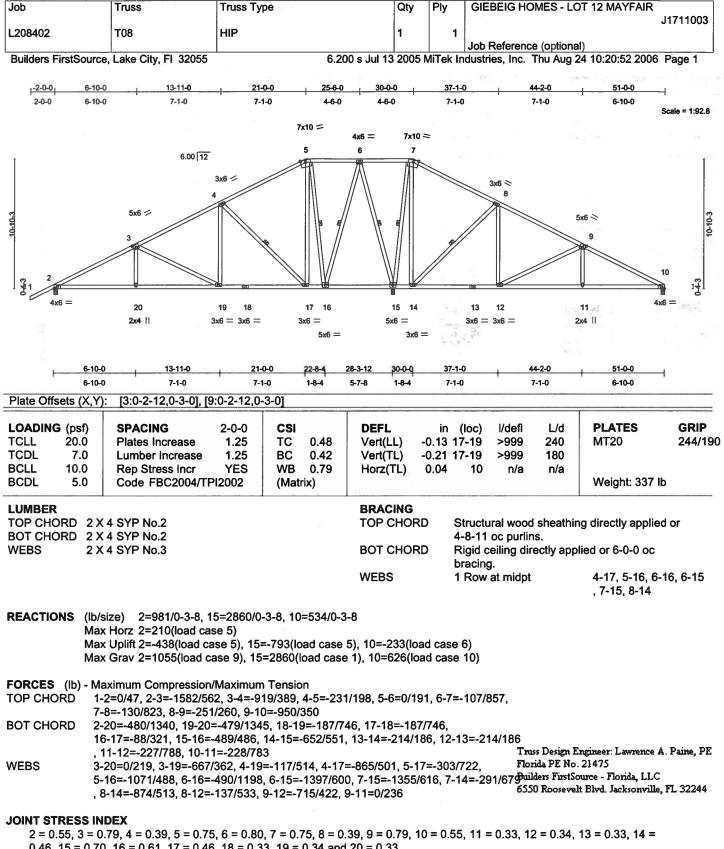
NOTES

- 1) 2 X 4 SYP No.2 bearing block 12" long at jt. 18 attached to front face with 2 rows of 0.131"x3" Nails spaced 3" o.c. 8 Total fasteners. Bearing Fc perp is assumed to be 565 psi.
- 2) Unbalanced roof live loads have been considered for this design.
- 3) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 398 lb uplift at joint 2, 846 lb uplift at joint 18 and 371 lb uplift at joint 12.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244





0.46, 15 = 0.70, 16 = 0.61, 17 = 0.46, 18 = 0.33, 19 = 0.34 and 20 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
						J1711003
L208402	T08	HIP	1	1	Job Reference (optional)	

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NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

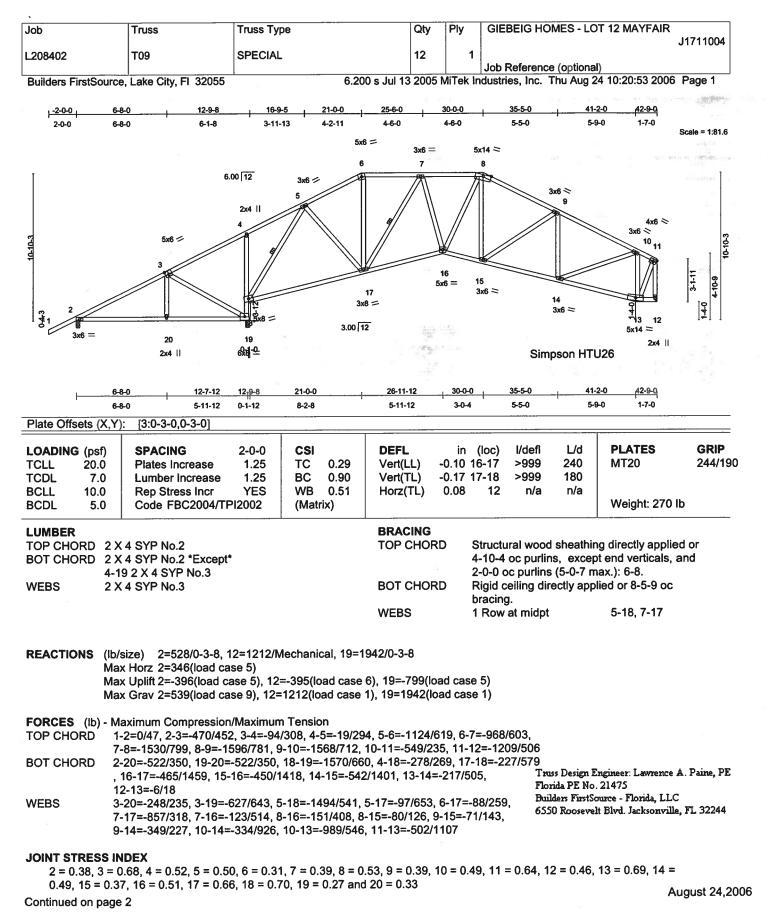
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 438 lb uplift at joint 2, 793 lb uplift at joint 15 and 233 lb uplift at joint 10.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244





Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCS-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
						J1711004
L208402	T09	SPECIAL	12	1	92	1
	1				Job Reference (optional)	

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NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

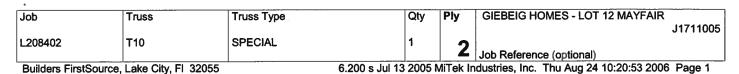
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

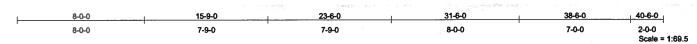
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 396 lb uplift at joint 2, 395 lb uplift at joint 12 and 799 lb uplift at joint 19.

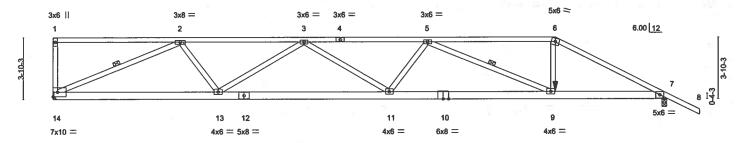
LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244









Simpson HGUS26-2

		10-4-12		10-8-8		1.	10-4-12		*	7-0-0	
Plate Of	fsets (X,Y)	: [7:0-3-0,0-2-9]								_	
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in (loc)	l/defi	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.97	Vert(LL)	-0.47 11-13	>975	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.76 11-13	>606	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.98	Horz(TL)	0.14 7	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	212002	(Mat	rix)					Weight: 432 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 6 SYP No.1D
WEBS	2 X 4 SYP No.3

BRACING TOP CHORD

OP CHORD Structural wood sheathing directly applied,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 8-9-15 oc

bracing.

WEBS 1 Row at midpt

2-14, 5-9

REACTIONS (lb/size) 14=3527/Mechanical, 7=3432/0-3-8

Max Horz 14=-228(load case 5)

Max Uplift 14=-1591(load case 2), 7=-1425(load case 5)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-14=-455/357, 1-2=-279/137, 2-3=-7723/3335, 3-4=-9817/4289, 4-5=-9817/4289,

5-6=-6249/2750, 6-7=-6948/2976, 7-8=0/51

BOT CHORD 13-14=-2888/6475, 12-13=-4247/9461, 11-12=-4247/9461, 10-11=-4224/9486,

9-10=-4224/9486, 7-9=-2592/6147

WEBS 2-14=-6774/3127, 2-13=-596/2198, 3-13=-2072/1232, 3-11=0/423, 5-11=0/582,

5-9=-3540/1794, 6-9=-872/2420

JOINT STRESS INDEX

1 = 0.74, 2 = 0.82, 3 = 0.34, 4 = 0.81, 5 = 0.50, 6 = 0.79, 7 = 0.80, 9 = 0.54, 10 = 0.94, 11 = 0.27, 12 = 0.90, 13 = 0.52 and 14 = 0.55

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
						J1711005
L208402	T10	SPECIAL	1	2		
					Job Reference (optional)	

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NOTES

- 2-ply truss to be connected together with 0.131"x3" Nails as follows: Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1591 lb uplift at joint 14 and 1425 lb uplift at joint 7.
- 7) Girder carries hip end with 7-0-0 right side setback, 0-0-0 left side setback, and 7-0-0 end setback.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

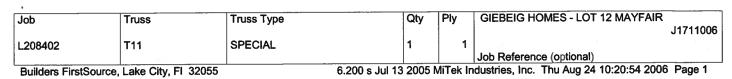
Vert: 1-6=-118(F=-64), 6-8=-54, 9-14=-65(F=-35), 7-9=-30

Concentrated Loads (lb)

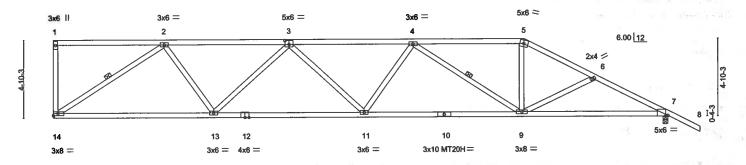
Vert: 9=-539(F)

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244



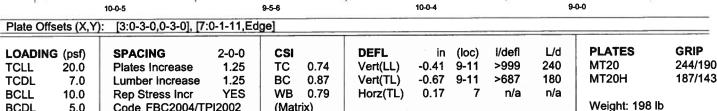






Simpson HTU26

10-0-5



BCLL BCDL	10.0 5.0	Rep Stress Incr YES Code FBC2004/TPI2002	(Matrix)	Horz(IL) 0.1	, ,	n/a	n/a	Weight: 198 lb
	ORD 2	K 4 SYP No.2 K 4 SYP No.2		BRACING TOP CHORD				ng directly applied or end verticals.

19-5-12

BOT CHORD 2 X 4 SYP No.3

Rigid ceiling directly applied or 6-1-3 oc bracing.

29-6-0

WEBS 1 Row at midpt 2-14, 4-9

38-6-0

REACTIONS (lb/size) 14=1601/Mechanical, 7=1724/0-3-8

Max Horz 14=-272(load case 6)

Max Uplift 14=-594(load case 3), 7=-560(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-14=-173/116, 1-2=-79/14, 2-3=-2594/1002, 3-4=-3368/1335, 4-5=-2489/1039, **TOP CHORD**

5-6=-2796/1088, 6-7=-3002/1182, 7-8=0/47

13-14=-648/2000, 12-13=-1058/3203, 11-12=-1058/3203, 10-11=-1067/3282, **BOT CHORD**

9-10=-1067/3282, 7-9=-887/2622

2-14=-2311/986, 2-13=-301/1064, 3-13=-846/453, 3-11=-13/228, 4-11=0/154, **WEBS**

4-9=-954/482, 5-9=-241/908, 6-9=-181/189

JOINT STRESS INDEX

WEBS

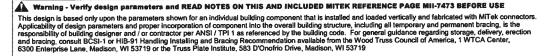
1 = 0.46, 2 = 0.76, 3 = 0.69, 4 = 0.40, 5 = 0.61, 6 = 0.33, 7 = 0.73, 9 = 0.56, 10 = 0.89, 11 = 0.40, 12 = 0.88, 13 = 0.76 and 14

= 0.62

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

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Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
						J1711006
L208402	T11	SPECIAL	1	1	727 NUN	
	l			L	Job Reference (optional)	

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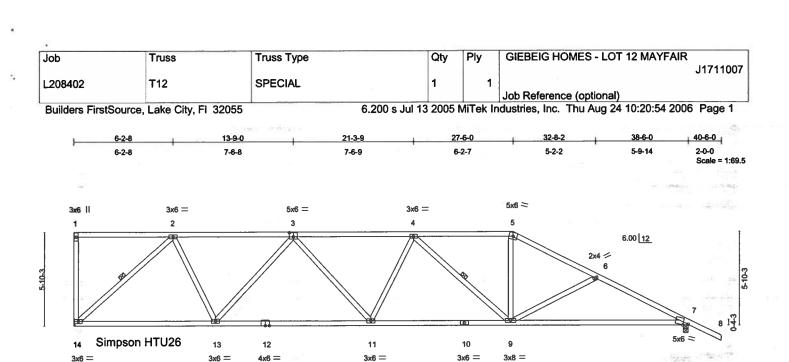
NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) All plates are MT20 plates unless otherwise indicated.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 594 lb uplift at joint 14 and 560 lb uplift at joint 7.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244





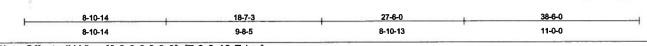


Plate Of	fsets (X,Y): [3:0-3-0,0-3-0], [7:	0-3-13,Ed	gej								
LOADIN	\• <i>,</i>	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.41	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.97	Vert(TL)	-0.69	7-9	>665	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.87	Horz(TL)	0.15	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	212002	(Mat	rix)						Weight: 207 lb	

L	U	M	В	E	I	₹		
	_		_			_	_	

3x6

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

BRACING

BOT CHORD

WEBS

TOP CHORD

Structural wood sheathing directly applied or 3-1-7 oc purlins, except end verticals.

Rigid ceiling directly applied or 2-2-0 oc

bracing.

1 Row at midpt

2-14, 4-9

REACTIONS (lb/size) 14=1601/Mechanical, 7=1724/0-3-8

3x6 =

4x6 =

Max Horz 14=-318(load case 6)

Max Uplift 14=-589(load case 3), 7=-575(load case 6)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-14=-146/99, 1-2=-42/8, 2-3=-1966/771, 3-4=-2756/1123, 4-5=-2319/996,

5-6=-2635/1039, 6-7=-2943/1181, 7-8=0/47

BOT CHORD 13-14=-429/1501, 12-13=-773/2544, 11-12=-773/2544, 10-11=-815/2747,

9-10=-815/2747, 7-9=-878/2578

WEBS 2-14=-1978/843, 2-13=-323/1065, 3-13=-881/469, 3-11=-68/322, 4-11=0/112,

4-9=-581/346, 5-9=-226/853, 6-9=-318/281

JOINT STRESS INDEX

1 = 0.31, 2 = 0.87, 3 = 0.65, 4 = 0.45, 5 = 0.52, 6 = 0.33, 7 = 0.87, 9 = 0.56, 10 = 0.89, 11 = 0.45, 12 = 0.86, 13 = 0.87 and 14 = 0.45, 12 = 0.45, 12 = 0.45, 12 = 0.86, 13 = 0.87= 0.70

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
		#1				J1711007
L208402	T12	SPECIAL	1	1		
	2.53				Job Reference (optional)	

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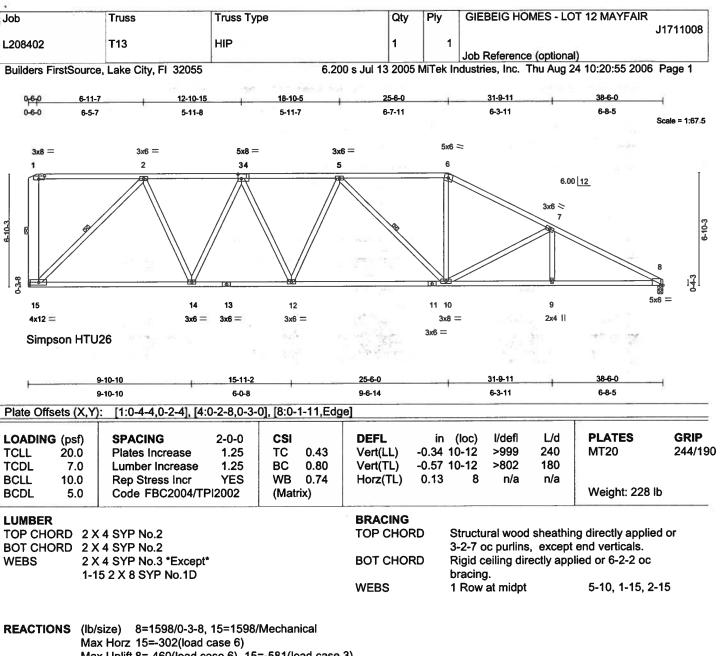
NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 589 lb uplift at joint 14 and 575 lb uplift at joint 7.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244





Max Uplift 8=-460(load case 6), 15=-581(load case 3)

FORCES (Ib) - Maximum Compression/Maximum Tension

1-2=-74/15, 2-3=-1757/723, 3-4=-2258/951, 4-5=-2258/951, 5-6=-2143/972, TOP CHORD

6-7=-2456/1007, 7-8=-3047/1197, 1-15=-186/130

14-15=-374/1347, 13-14=-579/2079, 12-13=-579/2079, 11-12=-698/2327, **BOT CHORD**

10-11=-698/2327, 9-10=-971/2644, 8-9=-971/2644

2-14=-342/1004, 3-14=-769/396, 3-12=-149/426, 5-12=-170/197, 5-10=-261/239, **WEBS**

6-10=-182/713, 7-10=-583/381, 7-9=0/201, 2-15=-1788/798

JOINT STRESS INDEX

1 = 0.96, 2 = 0.84, 3 = 0.00, 4 = 0.76, 5 = 0.46, 6 = 0.63, 7 = 0.39, 8 = 0.72, 9 = 0.33, 10 = 0.56, 11 = 0.90, 12 = 0.46, 13 = 0.46, 13 = 0.46, 14 = 0.46, 15 = 0.

0.71, 14 = 0.84 and 15 = 0.37

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244

August 24,2006

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
}						J1711008
L208402	T13	HIP	1	1		
				L	Job Reference (optional)	

6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 10:20:55 2006 Page 2

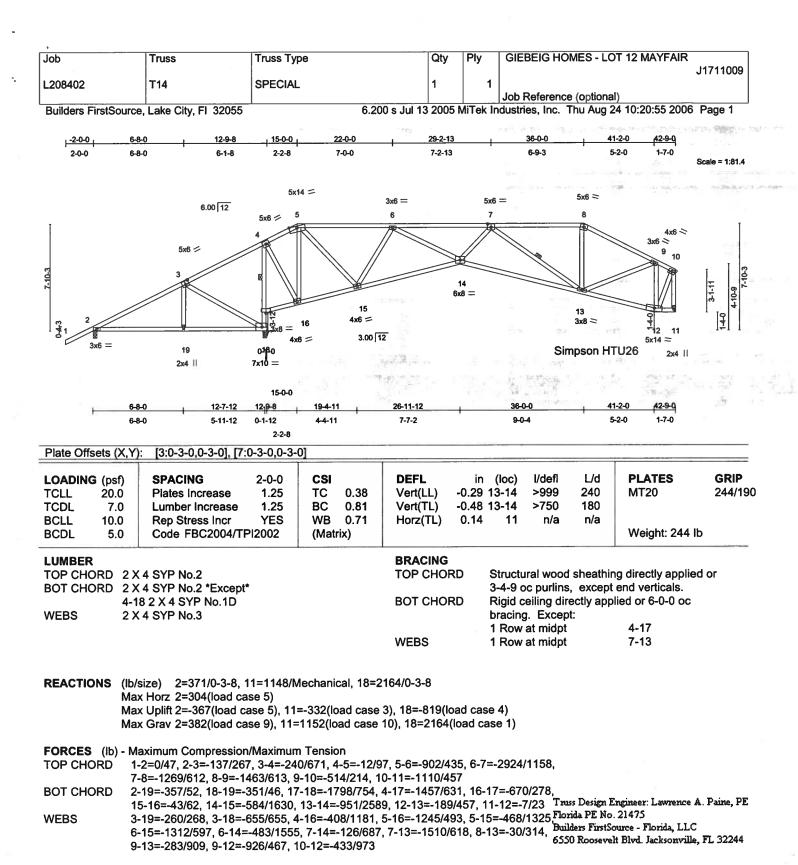
NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 460 lb uplift at joint 8 and 581 lb uplift at joint 15.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244





JOINT STRESS INDEX

2 = 0.34, 3 = 0.69, 4 = 0.59, 5 = 0.72, 6 = 0.87, 7 = 0.54, 8 = 0.55, 9 = 0.50, 10 = 0.57, 11 = 0.49, 12 = 0.60, 13 = 0.81, 14 = 0.77, 15 = 0.64, 16 = 0.69, 17 = 0.96, 18 = 0.25 and 19 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
						J1711009
L208402	T14	SPECIAL	1	1	lob Reference (antional)	
					Job Reference (optional)	

6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 10:20:55 2006 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.Zpsf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.

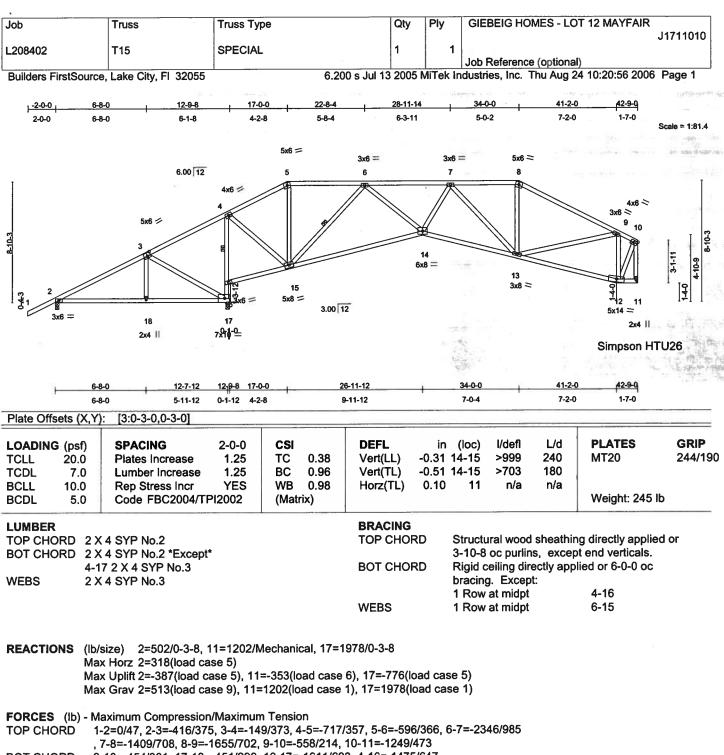
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 367 lb uplift at joint 2, 332 lb uplift at joint 11 and 819 lb uplift at joint 18.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244





BOT CHORD 2-18=-454/301, 17-18=-451/299, 16-17=-1611/693, 4-16=-1475/647,

15-16=-338/145, 14-15=-566/1618, 13-14=-751/2178, 12-13=-235/551, 11-12=-5/1 Truss Design Engineer: Lawrence A. Paine, PE

3-18=-254/251, 3-17=-637/647, 4-15=-366/1193, 5-15=0/97, 6-15=-1407/591, Florida PE No. 21475

6-14=-293/1069, 7-14=-99/547, 7-13=-1064/413, 8-13=-75/412, 9-13=-271/914, 6550 Roosevelt Blvd. Jacksonville, FL 32244

9-12=-1023/612, 10-12=-558/1223

JOINT STRESS INDEX

WEBS

2 = 0.36, 3 = 0.67, 4 = 0.57, 5 = 0.31, 6 = 0.62, 7 = 0.43, 8 = 0.62, 9 = 0.46, 10 = 0.71, 11 = 0.44, 12 = 0.76, 13 = 0.79, 14 = 0.80, 15 = 0.52, 16 = 0.55, 17 = 0.22 and 18 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
					J17110	10
L208402	T15	SPECIAL	1	1		
		Table 1/11 VS		7.0	Job Reference (optional)	

6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 10:20:56 2006 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

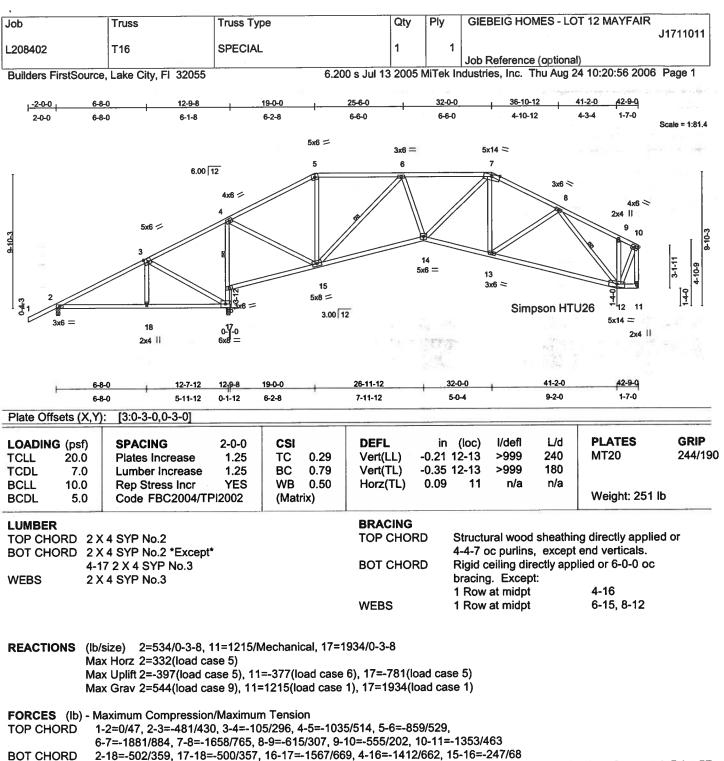
3) Provide adequate drainage to prevent water ponding.

- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 387 lb uplift at joint 2, 353 lb uplift at joint 11 and 776 lb uplift at joint 17.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244





, 14-15=-606/1784, 13-14=-488/1493, 12-13=-505/1246, 11-12=-10/9

3-18=-250/240, 3-17=-621/635, 4-15=-358/1207, 5-15=-10/165, 6-15=-1251/468,

6-14=-102/568, 7-14=-237/647, 7-13=-71/127, 8-13=-109/380, 8-12=-1109/497,

9-12=-196/196, 10-12=-418/1215

Truss Design Engineer: Lawrence A. Paine, PE

Florida PE No. 21475

Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

JOINT STRESS INDEX

WEBS

2 = 0.36, 3 = 0.66, 4 = 0.48, 5 = 0.55, 6 = 0.40, 7 = 0.76, 8 = 0.39, 9 = 0.33, 10 = 0.71, 11 = 0.48, 12 = 0.75, 13 = 0.37, 14 = 0.83, 15 = 0.51, 16 = 0.62, 17 = 0.27 and 18 = 0.33

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR
					J1711011
L208402	T16	SPECIAL	1	1	
					Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 10:20:56 2006 Page 2

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

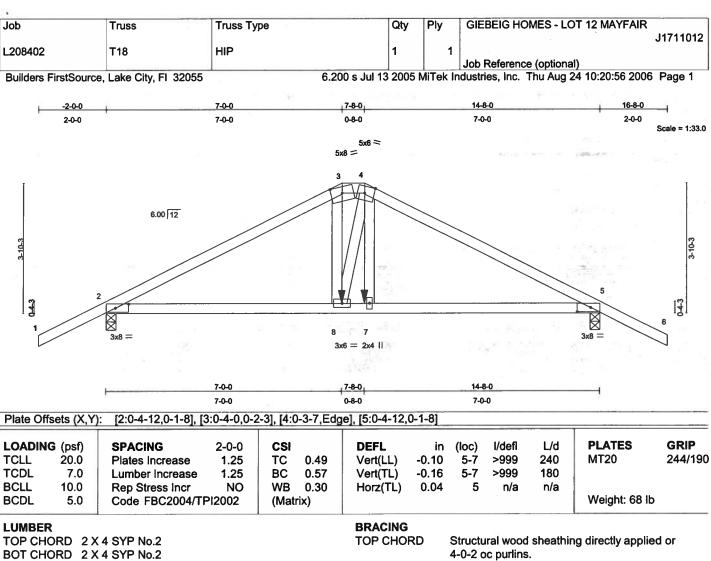
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 397 lb uplift at joint 2, 377 lb uplift at joint 11 and 781 lb uplift at joint 17.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244





WEBS 2 X 4 SYP No.3

BOT CHORD Rigid ceiling directly applied or 7-7-7 oc

bracing.

REACTIONS (lb/size) 2=1297/0-3-8, 5=1297/0-3-8

Max Horz 2=87(load case 4)

Max Uplift 2=-616(load case 4), 5=-616(load case 5)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/47, 2-3=-2095/860, 3-4=-1765/819, 4-5=-2086/856, 5-6=0/47

BOT CHORD

2-8=-686/1792, 7-8=-657/1754, 5-7=-668/1782

WEBS

3-8=-385/922, 4-8=-244/338, 4-7=-273/682

JOINT STRESS INDEX

2 = 0.76, 3 = 0.86, 4 = 0.72, 5 = 0.76, 7 = 0.48 and 8 = 0.58

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 616 Ib uplift at joint 2 and 616 lb uplift at joint 5.
- 6) Girder carries hip end with 7-0-0 end setback.

Continued on page 2

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475

Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
5				}		J1711012
L208402	T18	HIP	1	1		
					Job Reference (optional)	

6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 10:20:57 2006 Page 2

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-8=-30, 7-8=-65(F=-35), 5-7=-30

Concentrated Loads (lb)

Vert: 8=-539(F) 7=-539(F)

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244



Qty Ply **GIEBEIG HOMES - LOT 12 MAYFAIR** Job Truss Truss Type J1711013 L208402 T19 COMMON 5 Job Reference (optional) Builders FirstSource, Lake City, Fl 32055 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 10:20:57 2006 Page 1 7-4-0 14-8-0 16-8-0 -2-0-0 7-4-0 7-4-0 2-0-0 Scale = 1:31.1 5x6 = 6.00 12 \aleph 3x6 = 3x6 = 2x4 || 14-8-0 7-4-0 7-4-0 7-4-0 LOADING (psf) 2-0-0 L/d **PLATES** GRIP **SPACING** CSI **DEFL** in (loc) I/defl Vert(LL) -0.094-6 >999 240 **MT20** 244/190 TCLL 20.0 Plates Increase 1.25 TC 0.37 7.0 BC 0.42 Vert(TL) -0.14 4-6 >999 180 TCDL Lumber Increase 1.25 **BCLL** 10.0 Rep Stress Incr YES **WB** 0.08 Horz(TL) 0.02 4 n/a n/a Weight: 58 lb Code FBC2004/TPI2002 **BCDL** 5.0 (Matrix) **BRACING** LUMBER Structural wood sheathing directly applied or TOP CHORD 2 X 4 SYP No.2 TOP CHORD 6-0-0 oc purlins. BOT CHORD 2 X 4 SYP No.2 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc **WEBS** 2 X 4 SYP No.3

bracing.

REACTIONS (lb/size) 2=720/0-3-8, 4=720/0-3-8

Max Horz 2=90(load case 5)

Max Uplift 2=-324(load case 5), 4=-324(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/47, 2-3=-840/330, 3-4=-840/330, 4-5=0/47

BOT CHORD

2-6=-119/677, 4-6=-119/677

WEBS

3-6=0/265

JOINT STRESS INDEX

2 = 0.53, 3 = 0.70, 4 = 0.53 and 6 = 0.19

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 324 lb uplift at joint 2 and 324 lb uplift at joint 4.

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475

Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244

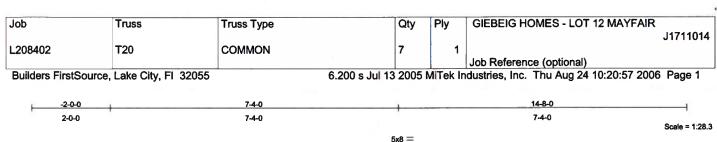
August 24,2006

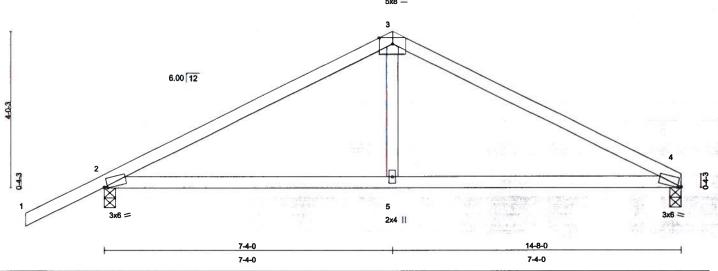
LOAD CASE(S) Standard

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building structure, including all temporary and permanent bracing, is the responsibility of building structure, including all temporary and permanent bracing, is the responsibility of building structure, including all temporary and permanent bracing, is the responsibility of building structure, including all temporary and permanent bracing, is the responsibility of building structure, including all temporary and permanent bracing, and including structure, including all temporary and permanent bracing, and including structure, including all temporary and permanent bracing, and including structure, including all temporary and permanent bracing, and including all temporary and permanent bracing, and including all temporary and permanent bracing, and including all temporary and p







LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.Ó	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.12	`4-Ś	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.49	Vert(TL)	-0.18	4-5	>963	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	212002	(Mat	rix)	` '					Weight: 55 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 4=595/0-3-8, 2=728/0-3-8

Max Horz 2=114(load case 5)

Max Uplift 4=-193(load case 6), 2=-328(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD **BOT CHORD**

1-2=0/47, 2-3=-868/375, 3-4=-864/368 2-5=-224/702, 4-5=-224/702

WEBS

3-5=0/278

JOINT STRESS INDEX

2 = 0.84, 3 = 0.86, 4 = 0.84 and 5 = 0.20

NOTES

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 Ib uplift at joint 4 and 328 lb uplift at joint 2.

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475

Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

August 24,2006

Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



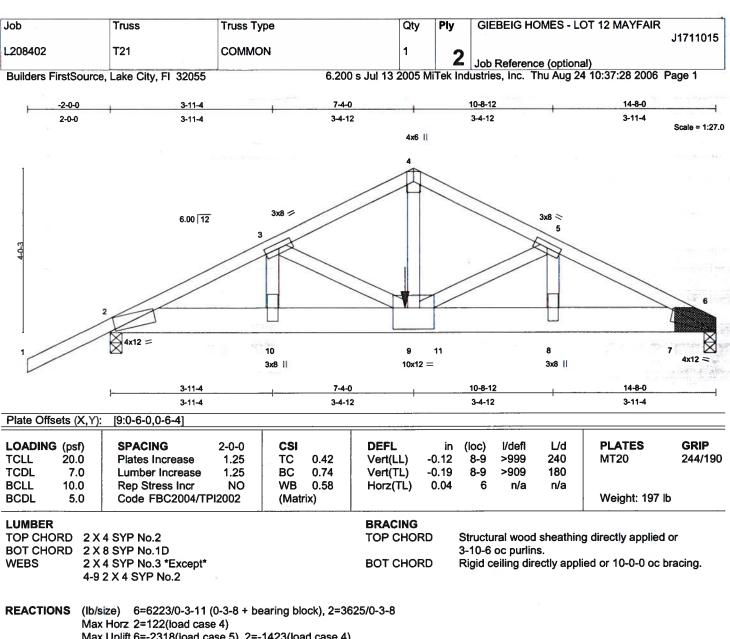
Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
			<u> </u>	١.	,	J1711014
L208402	T20	COMMON	7	1		1
					Job Reference (optional)	

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LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244





Max Uplift 6=-2318(load case 5), 2=-1423(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/54, 2-3=-7096/2572, 3-4=-7348/2756, 4-5=-7345/2744, 5-6=-10289/3819

BOT CHORD 2-10=-2284/6325, 9-10=-2284/6325, 9-11=-3383/9211, 8-11=-3383/9211, 7-8=-3383/9211,

6-7=-3383/9211

3-10=-441/260, 3-9=-181/389, 4-9=-2322/6278, 5-9=-3038/1199, 5-8=-942/2539

JOINT STRESS INDEX

2 = 0.76, 3 = 0.75, 4 = 0.72, 5 = 0.75, 6 = 0.76, 6 = 0.00, 6 = 0.00, 7 = 0.00, 7 = 0.00, 7 = 0.00, 8 = 0.41, 9 = 0.77 and 10 = 0.41

WEBS

1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc, Except member 9-4 2 X 4 - 1 row at 0-5-0 oc. Truss Design Engineer: Lawrence A. Paine, PE 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the Florida PE No. 21475

LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (Fibilders FirstSource - Florida, LLC or (B), unless otherwise indicated. 6550 Roosevelt Blvd. Jacksonville, FL 32244

August 24,2006

Continued on page 2





Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
L208402	T21	COMMON	1		41	J1711015
L200402	121		<u> </u>	2	Job Reference (optional)	

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NOTES

- 3) 2 X 8 SYP No.2 bearing block 12" long at jt. 6 attached to each face with 4 rows of 0.131"x3" Nails spaced 3" o.c. 16 Total fasteners per block. Bearing Fc perp is assumed to be 565 psi.
- 4) Unbalanced roof live loads have been considered for this design.
- 5) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2318 lb uplift at joint 6 and 1423 lb uplift at joint 2.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-6=-54, 2-11=-30, 6-11=-797(F=-767)

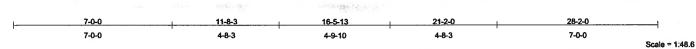
Concentrated Loads (lb) Vert: 9=-3527(F)

> Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244





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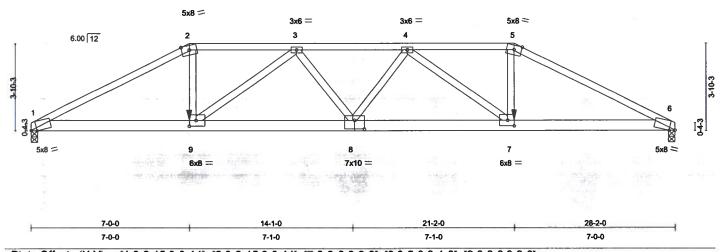


Plate Of	ffsets (X,Y): [1:0-2-15,0-0-14],	[6:0-2-15,	0-0-14],	[7:0-3-8	,0-3-0], [8:0-5	-0,0-4-8], [9:0-3	3-8,0-3-0		T	
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.Ó	Plates Increase	1.25	TC	0.71	Vert(LL)	-0.31	`7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	-0.50	7-8	>669	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.81	Horz(TL)	0.12	6	n/a	n/a	>	
BCDL	5.0	Code FBC2004/TI	212002	(Mat	rix)						Weight: 147 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 6 SYP No.1D

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

2-3-12 oc purlins.

BOT CHORD Rigid ceiling directly applied or 5-6-2 oc

bracing.

REACTIONS (lb/size) 1=2405/0-3-8, 6=2405/0-3-8

Max Horz 1=-50(load case 2)

Max Uplift 1=-967(load case 4), 6=-967(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-4874/2013, 2-3=-4350/1863, 3-4=-5539/2342, 4-5=-4350/1863,

5-6=-4874/2013

BOT CHORD 1-9=-1790/4279, 8-9=-2334/5375, 7-8=-2316/5375, 6-7=-1739/4279

WEBS 2-9=-642/1719, 3-9=-1401/729, 3-8=0/329, 4-8=0/329, 4-7=-1401/729,

5-7=-642/1719

JOINT STRESS INDEX

1 = 0.91, 2 = 0.90, 3 = 0.40, 4 = 0.40, 5 = 0.90, 6 = 0.91, 7 = 0.46, 8 = 0.91 and 9 = 0.46

NOTES

1) Unbalanced roof live loads have been considered for this design.

Truss Design Engineer: Lawrence A. Paine, PE 2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exproxida PE No. 21475 B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60. Builders FirstSource - Florida, LLC

Provide adequate drainage to prevent water ponding.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

6550 Roosevelt Blvd. Jacksonville, FL 32244 August 24,2006

warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek conne Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erec and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	I
1 209402	 T22	HIP	4	4		J1711016
L208402	122	nie			Job Reference (optional)	

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NOTES

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 967 lb uplift at joint 1 and 967 lb uplift at joint 6.
- 6) Girder carries hip end with 7-0-0 end setback.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54, 2-5=-118(F=-64), 5-6=-54, 1-9=-30, 7-9=-65(F=-35), 6-7=-30

Concentrated Loads (lb)

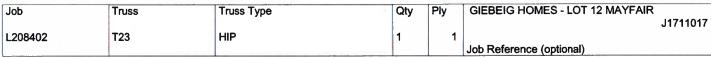
Vert: 9=-539(F) 7=-539(F)

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244

August 24,2006

A. Chia cally piece of the





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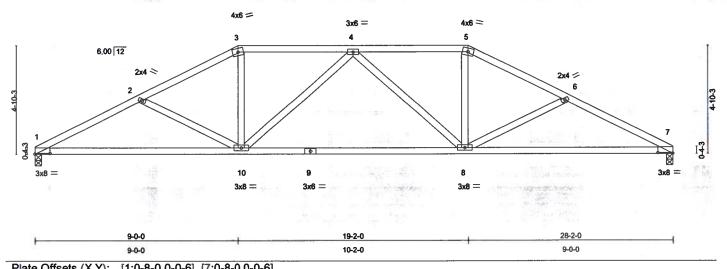


Plate Of	isels (A, t): [1:0-6-0,0-0-6], [7:	U-0-U,U-U-	0]	-			·				
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.22	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.36	8-10	>918	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.24	Horz(TL)	0.08	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)	, ,					Weight: 133 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-11-8 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 7-2-3 oc

bracing.

REACTIONS

(lb/size) 1=1171/0-3-8, 7=1171/0-3-8

Max Horz 1=-66(load case 3)

Max Uplift 1=-346(load case 5), 7=-346(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2121/930, 2-3=-1867/798, 3-4=-1639/767, 4-5=-1639/767, 5-6=-1867/798,

6-7=-2121/930

BOT CHORD 1-10=-759/1855, 9-10=-638/1788, 8-9=-638/1788, 7-8=-759/1855

WEBS 2-10=-267/257, 3-10=-159/560, 4-10=-299/191, 4-8=-299/190, 5-8=-159/560,

6-8=-267/257

JOINT STRESS INDEX

1 = 0.64, 2 = 0.33, 3 = 0.49, 4 = 0.35, 5 = 0.49, 6 = 0.33, 7 = 0.64, 8 = 0.56, 9 = 0.72 and 10 = 0.56

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions Florida PE No. 21475

Truss Design Engineer: Lawrence A. Paine, PE Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

3) Provide adequate drainage to prevent water ponding.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
						J1711017
L208402	T23	HIP	1	1		}
					Job Reference (optional)	

6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 10:20:58 2006 Page 2

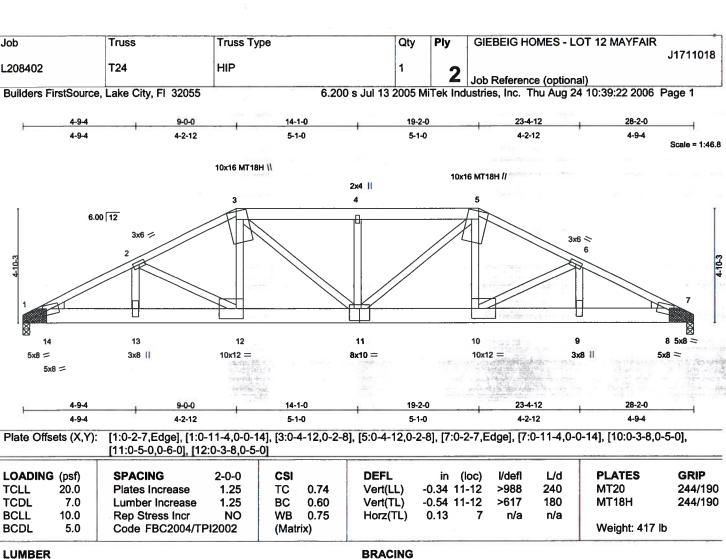
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 346 lb uplift at joint 1 and 346 lb uplift at joint 7.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244





LUMBER

TOP CHORD 2 X 4 SYP No.1D *Except*

3-5 2 X 6 SYP No.1D

2 X 8 SYP 2400F 2.0E BOT CHORD

WEBS 2 X 4 SYP No.3

WEDGE

WEBS

Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

TOP CHORD

Structural wood sheathing directly applied or 2-9-1

oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=9439/0-5-9 (0-3-8 + bearing block), 7=9439/0-5-9 (0-3-8 + bearing block)

Max Horz 1=-63(load case 2)

Max Uplift 1=-3468(load case 4), 7=-3468(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-17569/6427, 2-3=-14881/5454, 3-4=-15371/5632, 4-5=-15365/5629,

5-6=-14863/5448, 6-7=-17576/6431

BOT CHORD 1-14=-5772/15732, 13-14=-5772/15732, 12-13=-5772/15732, 11-12=-4880/13497,

10-11=-4810/13479, 9-10=-5714/15738, 8-9=-5714/15738, 7-8=-5714/15738

2-13=-783/2225, 2-12=-2703/1093, 3-12=-1721/4702, 3-11=-979/2552, 4-11=-333/227, 5-11=-994/2600, 5-10=-1697/4631, 6-10=-2727/1102, 6-9=-791/2247

Florida PE No. 21475

Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

Truss Design Engineer: Lawrence A. Paine, PE

JOINT STRESS INDEX

1 = 0.88, 1 = 0.00, 1 = 0.00, 1 = 0.88, 2 = 0.83, 3 = 0.86, 4 = 0.34, 5 = 0.86, 6 = 0.83, 7 = 0.88, 7 = 0.00, 7 = 0.00, 7 = 0.88, 8 = 0.00, 8= 0.00, 8 = 0.00, 9 = 0.37, 10 = 0.47, 11 = 0.94, 12 = 0.47, 13 = 0.36, 14 = 0.00, 14 = 0.00 and 14 = 0.00

August 24,2006

Continued on page 2

▲ Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek conner Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erect and bracing, consult 8CSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	744040
L208402	T24	HIP	1		J1/	711018
2200.02	\ <u>-</u> .				Job Reference (optional)	

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NOTES

- 2-ply truss to be connected together with 0.131"x3" Nails as follows: Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc. Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) 2 X 8 SYP No.2 bearing block 12" long at jt. 1 attached to each face with 4 rows of 0.131"x3" Nails spaced 3" o.c. 16 Total fasteners per block. Bearing Fc perp is assumed to be 565 psi.
- 4) 2 X 8 SYP No.2 bearing block 12" long at jt. 7 attached to each face with 4 rows of 0.131"x3" Nails spaced 3" o.c. 16 Total fasteners per block. Bearing Fc perp is assumed to be 565 psi.
- 5) Unbalanced roof live loads have been considered for this design.
- 6) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are MT20 plates unless otherwise indicated.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3468 lb uplift at joint 1 and 3468 lb uplift at joint 7.

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

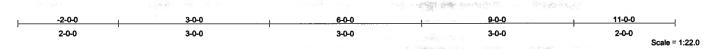
Vert: 1-3=-54, 3-5=-54, 5-7=-54, 1-7=-623(F=-593)

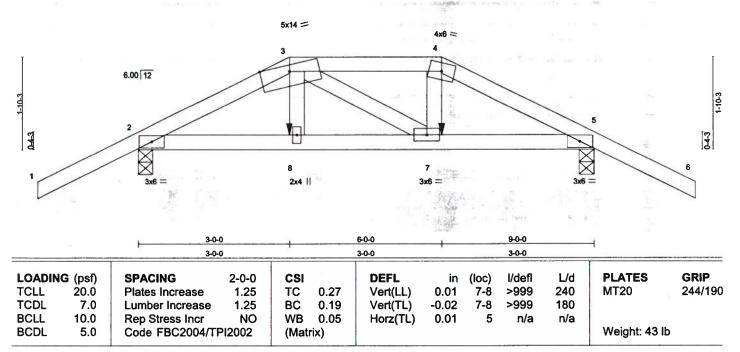
Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR
1.000.400	TOE				J1711019
L208402	T25	HIP	1	1	Job Reference (optional)

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LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 2=566/0-3-8, 5=566/0-3-8

Max Horz 2=59(load case 4)

Max Uplift 2=-395(load case 4), 5=-395(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-665/358, 3-4=-561/339, 4-5=-666/358, 5-6=0/47

BOT CHORD 2-8=-281/547, 7-8=-288/560, 5-7=-268/548

WEBS 3-8=-90/155, 3-7=-39/39, 4-7=-87/161

JOINT STRESS INDEX

2 = 0.53, 3 = 0.15, 4 = 0.16, 5 = 0.53, 7 = 0.10 and 8 = 0.11

NOTES

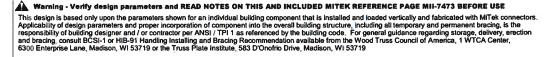
- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 395 lb uplift at joint 2 and 395 lb uplift at joint 5.
- 6) Girder carries hip end with 3-0-0 end setback.

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475

Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

Continued on page 2





Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
1 000 400	TOF	LUB			J171	11019
L208402	T25	HIP	T.	'	Job Reference (optional)	

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NOTES

7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-64(F=-10), 4-6=-54, 2-8=-30, 7-8=-35(F=-5), 5-7=-30

Concentrated Loads (lb)

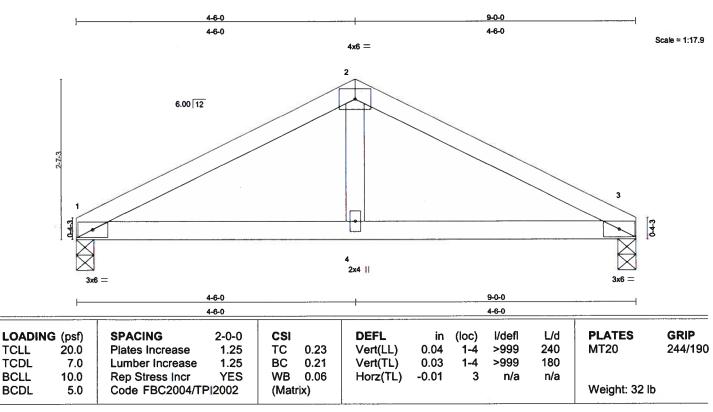
Vert: 8=-63(F) 7=-63(F)

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR
					J1711020
L208402	T26	COMMON	1	1	
					Job Reference (optional)

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LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 8-7-8 oc

bracing.

REACTIONS (lb/size) 1=366/0-3-8, 3=366/0-3-8

Max Horz 1=-33(load case 3)

Max Uplift 1=-228(load case 5), 3=-228(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-528/636, 2-3=-528/636 BOT CHORD 1-4=-498/427, 3-4=-498/427

WEBS 2-4=-305/173

JOINT STRESS INDEX

1 = 0.28, 2 = 0.60, 3 = 0.28 and 4 = 0.12

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 228 lb uplift at joint 1 and 228 lb uplift at joint 3.

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475

Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244

August 24,2006

LOAD CASE(S) Standard

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, eraction and bracing, consult BCS-1 or HIB-91 Handling installing and Bracing Recommendation authority Mood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Ply GIEBEIG HOMES - LOT 12 MAYFAIR Truss Type Qty Job Truss J1711021 L208402 **T27** COMMON Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055

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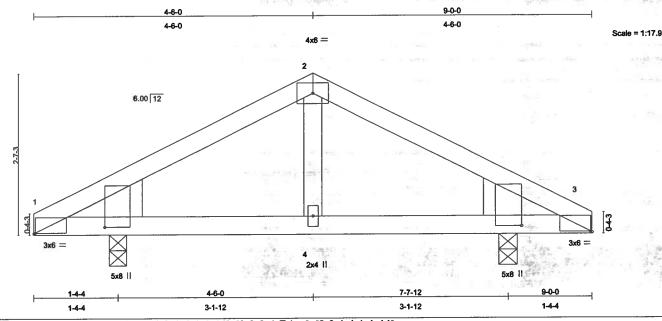


Plate Off	sets (X,Y	: [1:0-0-4,Edge], [1:	0-1-4,1-1-	11], [3:0)-U-4,Edg	je], [3:0-1-4,1	-1-11]				T	
LOADING TCLL TCDL BCLL BCDL	G (psf) 20.0 7.0 10.0 5.0	SPACING Plates Increase Lumber Increase Rep Stress Incr Code FBC2004/TI	2-0-0 1.25 1.25 YES PI2002	CSI TC BC WB (Mat	0.55 0.72 0.06 rix)	DEFL Vert(LL) Vert(TL) Horz(TL)	in 0.04 0.03 -0.01	(loc) 3-4 3-4 3	I/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 39 lb	GRIP 244/190

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

WEDGE

Left: 2 X 8 SYP No.1D, Right: 2 X 8 SYP No.1D

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 8-7-8 oc

bracing.

REACTIONS (lb/size) 1=366/0-3-8, 3=366/0-3-8

Max Horz 1=33(load case 4)

Max Uplift 1=-228(load case 5), 3=-228(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-528/636, 2-3=-528/636

BOT CHORD

1-4=-498/427, 3-4=-498/427

WEBS

2-4=-305/173

JOINT STRESS INDEX

1 = 0.28, 1 = 0.08, 2 = 0.60, 3 = 0.28, 3 = 0.08 and 4 = 0.12

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp.

B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever left and right exposed

Florida PE No. 21475 porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Builders FirstSource - Florida, LLC

6550 Roosevelt Blvd. Jacksonville, FL 32244

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

August 24,2006

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	GIEBEIG HOMES - LOT 12 MAYFAIR	
					,	J1711021
L208402	T27	COMMON	1	1		
					Job Reference (optional)	

6.200 s Jul 13 2005 MiTek Industries, Inc. Thu Aug 24 10:21:03 2006 Page 2

NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 228 lb uplift at joint 1 and 228 lb uplift at joint 3.

LOAD CASE(S) Standard

Truss Design Engineer: Lawrence A. Paine, PE Florida PE No. 21475 Builders FirstSource - Florida, LLC 6550 Roosevelt Blvd. Jacksonville, FL 32244

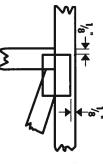


Symbols

PLATE LOCATION AND ORIENTATION



 Center plate on joint unless dimensions indicate otherwise. securely seat. plates to both sides of truss and Dimensions are in inches. Apply



For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

⋍

8

J7

76

'n

Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.



 This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

4 × 4

to slots. dimension is the length paralle perpendicular to slots. Second The first dimension is the width

LATERAL BRACING



continuous lateral bracing. Indicates location of required

BEARING



which bearings (supports) occur. Indicates location of joints at

TOP CHORD 72 OP CHORDS ၽ

JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA CBO 3907, 4922 96-31, 96-67

9667, 9432A

SBCCI

WISC/DILHR

960022-W, 970036-N

NER R

561

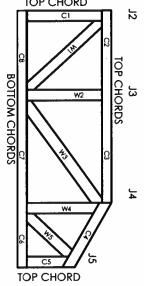


MiTek Engineering Reference Sheet: MII-7473

0

Numbering System

General Safety Notes



Ņ Damage or Personal Injury Failure to Follow Could Cause Property Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.

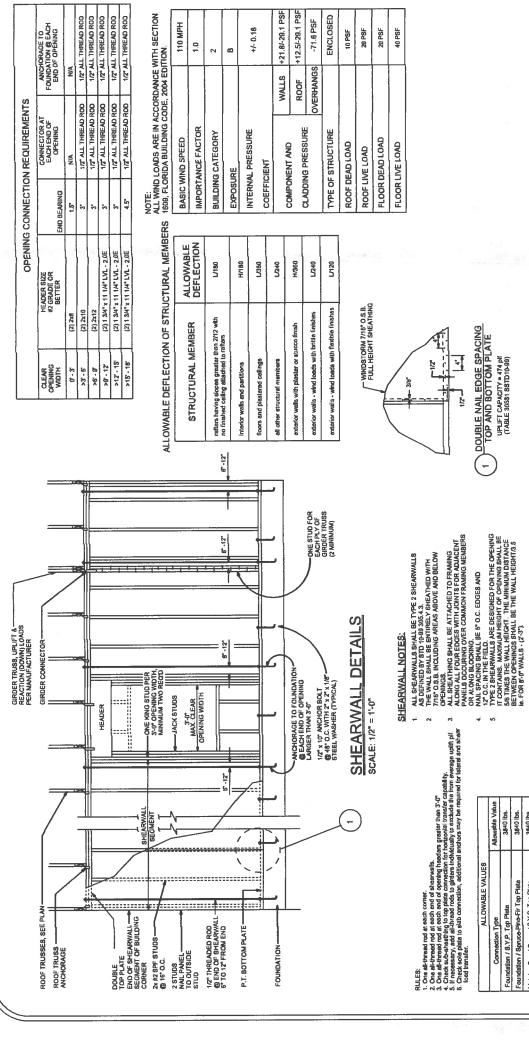
- Cut members to bear tightly against each
- ω
- Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- Unless otherwise noted, locate chord splices at 1/2 panel length (± 6" from adjacent joint.)

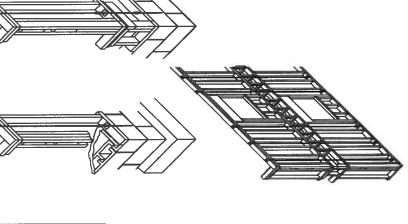
4.

- ٥. Unless expressly noted, this design is not applicable for use with fire retardant or
- Camber is a non-structural consideration and preservative treated lumber.
- 7. is the responsibility of truss fabricator. General practice is to camber for dead load deflection
- ထ Plate type, size and location dimensions shown indicate minimum plating requirements.
- .9 grade specified. Lumber shall be of the species and size, and in all respects, equal to or better than the
- Top chords must be sheathed or purlins provided at spacing shown on design
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 12. Anchorage and / or load transferring others unless shown. connections to trusses are the responsibility of
- Do not overload roof or floor trusses with stacks of construction materials.
- 14. Do not cut or after truss member or plate engineer without prior approval of a professional
- Care should be exercised in handling. 1993 MiTek® Holdings, Inc. erection and installation of trusses.

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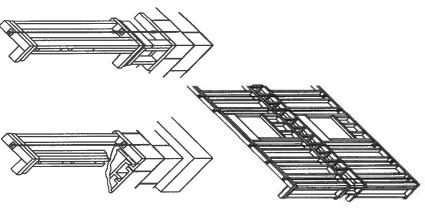
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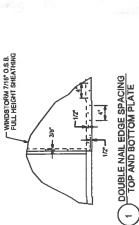




CONNECTION DETAIL

ALL THREAD ROD





16d TOE NALS EACH END

OPENING WIDTH UP TO 6'-0" > 6' TO 9'-0" > 9' TO 12'-0"

Lintel or Bond Beam / S.Y.P. Top Plate Lintel or Bond Beam / Spruce-Pine-Fir Top Plate

Placement at slab level:

SILL PLATES (1) 2x4 OR (1) 2x6 (3) 2x4 OR (1) 2x6 (5) 2x4 OR (2) 2x6

UPLIFT CAPACITY = 474 plf (TABLE 30581 8STD10-99)

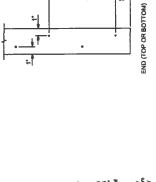
6021-857(985)

LAKE CITY, FL.

161 N.W. MADISON STREET SUITE #102

Design Group,

Lteeman



COUPLER (optional) - 1/2" r 1-1/2" zhr plata mat conform to ASTM AAR and A307 standards

Intermediate Coupler Connections
When usufing the rougher care should be taken to ensure full and
when usufing the not coupler. It is a seally activeted by titnesting the
coupler all the way only the not, then standing the two roots end to end,
then threading the coupler back over the rod joint so each rod is
halfway into the coupler.

Retro-fits In the case of an all thread rod misplacement, the rod may be epoxied into the concrete.

-RDD - 1/2" el-Broad rod must be zinc plased and conform to ASTM A38 and A307 standards

Comes

When presenting the sall-thread rod at a building comer, the rod
should be placed 8 to 12 inches away from the corner, so it does not set
under the corner framing members. When a self-thread rod is specified at a
building comer, it may be placed on either side of the corner.

Header ends

When preseding the all-thread rod at a header end, the rod
should be placed 8 to 12 inches away from the header end so it does not
should be placed 8 to 12 inches away from the header end so it does not
fall under the stud pack framing members.

Top connections

Top connections

I richts of the terming pack. A rud and 3X3 wesher shall be made within
the top lattes and ignitized decureby.

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Side plate to eithe corrections that the state of the state with the rins able three state states shall be connected to the state with the connection. As three states the states the design specifications, documents. As three forces shall be placed as per time design specifications. As thread rocks with a nut and washer at the sode plate will quelify as a sole plate over accordance to the rock of equal or other entires intermediate of the althroad cod locations to quelify the specified special proqueriements.

System Tightening:
On multiple alony applications, the all-thread rod system shall be rechecked
On multiple alony applications, the all-thread rod proper remain just before the walls are ventered. This will allow the
all-thread rod system to compensate for the buildings dead load compression

GIRDER COLUMN DETAIL SCALE: 1/2" = 1·0"

S.

-6	"Z	OTTOM)
•		END (TOP OR BOTTOM)

