

DATE 03/24/2008

Columbia County Building Permit

PERMIT
000026866

This Permit Must Be Prominently Posted on Premises During Construction

APPLICANT DONNY WILLIAMS PHONE 755-0764
 ADDRESS 541 SW AIRPARK GLEN LAKE CITY FL 32025
 OWNER JAMES RICHARDSON PHONE 755-5779
 ADDRESS 692 SW ARLINGTON BLVD LAKE CITY FL 32025
 CONTRACTOR DONNY WILLIAMS PHONE 755-0764

LOCATION OF PROPERTY 90W, TL ON SISTERS WELCOME RD, TL ON MIDTOWN PLACE, TR ON WATERFORD CT, TL ARLINGTON BLVD, 2ND LOT ON LEFT

TYPE DEVELOPMENT COMM. METAL BLDG ESTIMATED COST OF CONSTRUCTION 75000.00

HEATED FLOOR AREA 502.00 TOTAL AREA 2400.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS ROOF PITCH 4/12 FLOOR SLAB

LAND USE & ZONING CI MAX. HEIGHT

Minimum Set Back Requirments: STREET-FRONT 5.00 REAR 15.00 SIDE 5.00

NO. EX.D.U. 1 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 31-3S-17-06262-012 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES 0.57

 CGC004692
 Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor

EXISTING X08-020 BK JH N

Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD, NOC ON FILE

VARIANCE 197, REDUCTION OF FRONT SETBACKS TO 5 FEET

Check # or Cash 14020

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power Foundation Monolithic
date/app. by date/app. by date/app. by

Under slab rough-in plumbing Slab Sheathing/Nailing
date/app. by date/app. by date/app. by

Framing Rough-in plumbing above slab and below wood floor
date/app. by date/app. by

Electrical rough-in Heat & Air Duct Peri. beam (Lintel)
date/app. by date/app. by date/app. by

Permanent power C.O. Final Culvert
date/app. by date/app. by date/app. by

M/H tie downs, blocking, electricity and plumbing Pool
date/app. by date/app. by

Reconnection Pump pole Utility Pole
date/app. by date/app. by date/app. by

M/H Pole Travel Trailer Re-roof
date/app. by date/app. by date/app. by

BUILDING PERMIT FEE \$ 375.00 CERTIFICATION FEE \$ 12.00 SURCHARGE FEE \$ 12.00

MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$

FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ **TOTAL FEE** 474.00

INSPECTORS OFFICE L. Sobow CLERKS OFFICE CA

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGRESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

REPORT OF SUBSURFACE EXPLORATION

**Richardson Aluminum-New Building
Arlington Boulevard
Lake City, Columbia County, Florida
CTI Project No. 08-00141-01**

- Prepared For -
Donnie Williams Construction
541 SW Airpark Glen
Lake City, Florida 32025

- Prepared by -
Cal-Tech Testing, Inc.
P.O. Box 1625
Lake City, Florida 32056-1625

February 4, 2008

FILE COPY



Cal-Tech Testing, Inc.

- Engineering
- Geotechnical
- Environmental

P.O. Box 1625 • Lake City, FL 32056

4784 Rosselle Street • Jacksonville, FL 32254

2230 Greensboro Highway • Quincy, FL 32351

Tel. (386) 755-3633 • Fax (386) 752-5456

Tel. (904) 381-8901 • Fax (904) 381-8902

Tel. (850) 442-3495 • Fax (850) 442-4008

LABORATORIES

February 4, 2008

Donnie Williams Construction

5414784 West U.S. Highway 90

Lake City, Florida 32055

Attention: Mr. Donnie Williams

Subject: Report of Subsurface Exploration
Proposed Richardson Aluminum New Building
Arlington Boulevard
Lake City, Columbia County, Florida
CTI Project No. 08-00141-01

Dear Mr. Williams:

Cal-Tech Testing, Inc. (CTI) has completed the subsurface exploration for the proposed Richardson Aluminum new building. Authorization to this work was verbally provided by you on February 28, 2008.

The following report presents the results of our field exploration and testing, an evaluation of the subsurface conditions with respect to available project characteristics, and recommendations to aid in the design and construction of the proposed building.

We have enjoyed assisting you on this project and look forward to serving as your geotechnical and construction materials testing consultant for the remainder of this and future projects. Should you have any questions concerning this report, please contact our office at 386-755-3633.

Sincerely,
CAL-TECH TESTING, INC.

David B. Brown
Executive Vice President

Nabil O. Hmeidi, P.E.
Senior Geotechnical Engineer
Licensed, Florida No. 57842

Distribution: File (1 copy)
Addressee (2 bound copies)

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ATTACMENTS

- Vicinity Map (1 page)
- Field Exploration Plan (1 page)
- Soil Boring Logs (2 pages)
- Subsurface Diagram (1 page)
- Unified Soil Classification System Chart (1 page)
- Key To Test Data (1 page)

1.0 PROJECT INFORMATION

The purpose of this exploration was to develop information concerning the site and subsurface conditions in order to evaluate site preparation requirements and foundation support recommendations for the proposed building. The subject site is located on the west side of SW Arlington Boulevard approximately ¼ mile north of Atlantic Coastline Road in Lake City, Columbia County, Florida. This report briefly describes our field activities and presents our findings.

It is our understanding the proposed building will have an approximate footprint of 2,400 SF and will be used as a warehouse/office space. The building will be one-story and constructed of structural steel with Concrete Masonry Unit (CMU)/or metal stud framed walls supported on a conventional shallow foundation system. Field testing related to drainage or pavement design is beyond the scope of this exploration.

Detailed structural information has not been provided; however, we anticipate individual column loads will not exceed 50 kips. We have assumed that soil-supported ground floor loads (dead load plus live load) in the proposed building will not exceed 200 psf. We have not been provided finished floor elevation for the proposed structures; however, We assume that less than two feet of earthwork fill will be required to achieve desired grade.

2.0 FIELD EXPLORATION

The subsurface conditions at the subject site were explored by drilling two (2) Standard Penetration Test (SPT) borings extending to a depth of 15 feet below the existing ground surface. The SPT borings were performed at the approximate locations shown on the attached Field Exploration Plan. These locations were determined in the field and measured by tape and turning approximate right angles from existing features. Therefore, the borings location should be considered only as accurate as the means and methods by which they were obtained.

Sampling and penetration procedures of the SPT borings were accomplished in general accordance with ASTM D-1586, "*Penetration Test and Split-Barrel Sampling of Soils*", using a power rotary drill rig. The standard penetration tests were performed by driving a standard 1-3/8" I.D. and 2" O.D. split spoon sampler with a 140 pound hammer falling 30 inches. The number of hammer blows required to drive the sampler a total of 18 inches, in 6 inch increments, were recorded. The penetration resistance or "N" value is the summation of the last two 6 inch increments and is illustrated on the attached boring logs adjacent to their corresponding sample depths. The penetration resistance is used as an index to derive soil parameters from various empirical correlations. The borings were performed using a **BK-51 (manual hammer)**.

The attached Generalized Subsurface Profile graphically illustrates penetration resistances, groundwater levels, and soil descriptions. It must be noted the stratification lines and depth designations indicated on the boring records represent approximate boundaries between soil types. In some instances, the transition between these soil types may be gradual. When reviewing the boring records, it should be understood that soil conditions may vary away from the boring locations.

3.0 SITE AND SUBSURFACE CONDITIONS

3.1 Site Conditions

The existing site conditions were observed by our personnel during our field program. At the time of our visit, the ground surface was grass-covered and was relatively level.

3.2 Area Geology/Sinkholes Potential

A review of the site geology indicates the subject project is underlain by Undifferentiated Quaternary Sediments (**Qu**) of the Pleistocene and Holocene epochs. These sediments consist of siliciclastics, organics and freshwater carbonates. The siliciclastics are light gray, tan, brown to dark, unconsolidated to poorly consolidated, clean to clayey, silty, fossiliferous, variably organic-bearing sands to blue green to olive green, poorly to moderately consolidated, sandy, silty, clays. Freshwater carbonates "*marls*" are buff colored to tan, unconsolidated to poorly consolidated, fossiliferous (mollusks) carbonate muds containing organics.

We note that limestone in this area consists of carbonate rock and its weathered residuum. In **Columbia** County, Florida and the surrounding areas, the limestone is marked by solution features (sinkholes) associated with *karst* terrains. Sinkholes are primarily caused by an advanced state of internal soil erosion or raveling action, which under certain circumstances can lead to ground subsidences. This internal soil erosion is a very slow process by which soil particle usually migrate under the influence of a hydraulic gradient to underlying Karsted and/or fractured limestone formation. A review of the Sinkhole Database issued by the Florida Geological Survey indicates a number of sinkhole occurrences within a 1½-mile radius of the subject site (database reference No. 29-022 & 29-505). It should be noted that only reported sinkholes are documented in this database.

Our site observation and results of the test borings did not reveal presence of active sinkholes within the explored areas. Therefore, it is our opinion the proposed development on this site will have no greater risk of damage due to sinkhole activity than the development of structures in other areas within the immediate vicinity of the subject site.

3.3 Subsurface Conditions

A representation of the subsurface conditions encountered in the explored areas is shown on the attached Generalized Subsurface Profile. Visual classification of the site soils indicates the soil profile as disclosed by SPT borings B-1 and B-2 initially consisted of about 12 inches of grayish brown silty fine sand with some organics. This surficial cover is underlain by about 4 to 5 feet of loose, gray to light brown, silty fine sand (SP-SM). This stratum is underlain by about 5½ feet of loose to medium dense, reddish tan and light gray, mottled, clayey fine sand (SC). Beneath this stratum to the borings termination depths, the soil profile consisted of about 4 to 5 feet of medium dense, light gray, fine sand to clayey fine sand (SP-SC). The borings were terminated at a depth of 15 feet below the existing ground surface.

3.4 Groundwater

At the time of completion of drilling, the groundwater was encountered in all SPT borings at depths ranging from about 12 to 13 feet below the existing ground surface. We note that due to the relatively short time frame of the field exploration, the groundwater may not have had sufficient time to stabilize. For a true groundwater level reading, piezometers may be required. In any event, fluctuation in groundwater levels should be expected due to seasonal climatic changes, construction activity, rainfall variations, surface water runoff, and other site-specific factors. Since groundwater level variations are anticipated, design drawings and specifications should accommodate such possibilities and construction planning should be based on the assumption that variations will occur.

4.0 RECOMMENDATIONS FOR FOUNDATION DESIGN & SITE PREPARATION

The recommendations presented in this report are based upon available project information, anticipated loading conditions, and data obtained during our field program. If the structural information is incorrect or the location of the structure changes, please contact this office so our recommendations may be reviewed and/or revised. Discovery of any site or subsurface condition during construction, which deviates from the data collected during this exploration, should be reported to us for evaluation. We note that assessment of site environmental conditions or presence of pollutants was beyond the scope of this exploration.

4.1 General

Based on our evaluation of the encountered subsoils, anticipated loading conditions and our past experience with similar projects, it is our professional opinion the subject site can be made suitable for the support of the proposed development.

4.2 Foundation Support

Our site observation indicated the presence of loose soils within the upper 12 to 18 inches of the existing ground surface. With the exception of the topsoil, the majority of the site soils are considered suitable for use as structural fill, however, they loose soils are not considered acceptable for the support of the proposed buildings and pavement sections in their current conditions. To improve the density of these soils, the upper 18 inches of the site soils (after removal of topsoil) within the buildings and pavement areas (including 5 feet outside the perimeter of the building) should be recompacted as indicated herein.

Provided the foundation and site soils are prepared in accordance with the guidelines presented in this report, it is our opinion the proposed structure may be supported on a conventional shallow foundation system. The shallow foundation may be designed for an allowable bearing pressure of 2,000 pounds per square foot (psf) or less supported on **recompacted** soils or newly placed structural fill.

In using net pressures, the weight of the footing and backfill over the footing need not be considered. Hence, only loads applied at or above final grade need to be used for dimensioning footings. However, wall bearing footings should be designed with a minimum width of 18 inches, while the individual column footings should have minimum dimensions of 2 feet by 2 feet.

4.3 Settlement Analyses

Actual magnitude of settlement that will occur beneath foundations will depend upon variations within the subsurface soil profile, actual structural loading conditions, embedment depth of the footings, actual thickness of compacted fill or cut, and the quality of the earthwork operations. Assuming the foundation related site work and foundation design is completed in accordance with the enclosed recommendations, we estimate the total settlement of the structure will be on the order of 1 inch or less. Differential settlements (between adjacent columns or along the length of a continuous wall footing) should be approximately one-half of the total settlement. This settlement is primarily the result of elastic compression of the upper looser sands, and should occur almost immediately following the application of the structural dead load during construction.

4.4 Uplift Resistance

Under wind loading conditions, the foundations will likely be subjected to considerable uplift forces. In order to resist these uplift forces, it may be necessary to increase the footing size (thus increasing the dead weight) or lower the footing to mobilize additional soil weight above the footing. Uplift resistance from the soil may be evaluated as the weight of the soil directly above the footing, plus the shearing resistance along the vertical face of the soil prism. Alternately, the available soil uplift resistance may be calculated as the weight of the soil prism defined by the diagonal line drawn from the top of the footing to the ground surface at an angle of 30 degrees with the vertical. We recommend that a total unit weight of 100 pcf (compacted to 95% of the

modified Proctor maximum dry density) be used for well-compacted, suitable fill. Should the bottom of any structure be below the stabilized seasonal-high groundwater level, these structures must be properly designed to resist the resulting uplift forces due to hydrostatic pressures.

4.5 Lateral Resistance

Lateral loads created by wind loads may be resisted by the passive pressure of the soil acting against the side of the individual footings and/or the friction developed between the base of the foundation system and the underlying soils. For compacted backfill and/or in-situ material, the passive pressure may be taken as an equivalent to the pressure exerted by a fluid weighing 330 pcf for above the ground-water table and 113 pcf below the water level. A coefficient of friction equal to 0.4 may be used for calculating the frictional resistance at the base of the shallow footings. The resistance values discussed herein are based on the assumption that the foundations can withstand horizontal movements on the order of ¼ inch. Lateral resistance determined in accordance with the recommendations provided herein should be considered the total available resistance. Consequently, the design should include a minimum factor of safety of 1.5.

4.6 Lateral Earth Pressures

In general, retaining walls are subject to "at-rest" or "active" pressures. Retaining walls that are restrained at the top will be subject to "at-rest" pressures due to their restricted movement. These "at-rest" pressures may be calculated as the equivalent pressure exerted by a fluid density of 50 pcf. Where walls are not restrained at the top and thus allowed sufficient movement to mobilize "active" pressures, an equivalent fluid density of 33 pcf should be used in the design.

These values may be used only for walls above the groundwater table. Therefore, the presence of any groundwater due to surface water intrusion should be handled with the use of a drainage layer behind the walls with a collection pipe discharging accumulated water away from the walls. If this is not practical, then the hydrostatic pressure due to water should be included in the design of the walls.

4.7 Drainage Considerations

Adequate drainage should be provided at the site in order to minimize increase in moisture content of the foundation soils. Excessive moisture can significantly reduce the soil's bearing capacity and contribute to foundation settlement. For the protection of the foundation soils, we recommend that the ground water surface be sloped away from all proposed structures.

4.8 Floor Slab

Exposed subgrade should be properly recompacted and proofrolled with a fully-loaded, tandem-axle dump-truck or similar pneumatic-tired equipment. Provided the recompaction and proofrolling operations do not indicate significant deflecting or pumping of the existing subgrade, the floor slab may be designed as a slab-on-grade. Any soft or loose soils found during the proofrolling procedure should be undercut and replaced with suitable, well-compacted, engineered fill.

All floor slabs should be supported on at least 4 inches of relatively clean granular material, such as sand, sand and gravel, or crushed stone. This is to help distribute concentrated loads and equalize moisture beneath the slab. This granular material should have 100 percent passing the 1½ -inch sieve and a maximum of 10 percent passing the No. 200 sieve. A vapor retarder may be installed on top of the subgrade to reduce dampness of the surface of the floor slabs. The vapor retarder should consist of a minimum 6-mil thickness overlapping (unsealed) sheets of plastic. In addition, properly constructed jointing will alleviate the potential for cracking and allow for some differential movement.

Based upon the soil conditions encountered at the subject site, the anticipated fill placement, and the recommended site preparation operations presented in this report, an estimated modulus of vertical subgrade reaction (k) for the slab bearing soils of 175 pounds per square inch per inch of vertical deflection (pci) may be used.

4.9 Exposed Subgrade

All vegetation, topsoil, and other organic matters should be removed from the building and pavement areas. Following this operation, the exposed soils in the buildings and pavement areas should be compacted with overlapping passes of a relatively heavy weight drum roller (operating in static mode) having a total operating static weight (weight of fuel and water included) of at least 10 tons and a drum diameter of 5 feet. All exposed surfaces should be compacted to a minimum of 95 percent of the modified Proctor maximum dry density (ASTM D-1557) to a depth of at least 12 inches below the compacted surface.

4.10 Structural Fill/Backfill

Structural fill should be placed in thin loose lifts not exceeding 12 inches in thickness and compacted with a heavy roller as described above. For walk-behind equipment, a maximum loose lift thickness of 6 inches is recommended. Each lift should be thoroughly compacted with the drum roller to provide densities equivalent to at least 95 percent of the modified Proctor maximum dry density (ASTM D-1557). Structural fill should consist of an inorganic, non-plastic, granular soil containing less than 10 percent material passing the No. 200 mesh sieve (relatively clean sand with a Unified Soil Classification of SP or SP-SM).

4.11 Pavement Subgrade Consideration

Pavement subgrades should be compacted to a minimum depth of 12 inches to at least 98 percent of the modified Proctor maximum dry density (ASTM D-1557). Any fill utilized to elevate the pavement areas to final subgrade elevation should consist of relatively clean fine sands (inorganic, non-expansive/non-plastic sands containing less than 10 percent, by weight, of fines). Pavement subgrade should be uniformly compacted to a minimum density of 95 percent of the soil's modified Proctor maximum dry density (ASTM D1557).

Laboratory tests should be performed on all off-site structural fill to be used to elevate proposed pavement areas to confirm that these soils meet the minimum requirements and can achieve the desired LBR values. Where subgrade stabilization is necessary, we recommend stabilization be used, as specified by the Florida Department of Transportation (FDOT) "Standard Specifications for Road and Bridge Construction," 2007 Edition, Section 160. To avoid rutting, traffic should not be allowed on pavement subgrade prior to placement and compaction of the base course materials.

5.0 REPORT LIMITATIONS

This report has been prepared for the exclusive use of the **Donnie Williams Construction of Lake City, Florida**, for the specific application to the project discussed herein. Our conclusions and recommendations have been rendered using generally accepted standards of geotechnical engineering practice in the State of Florida. No other warranty is expressed or implied. **CTI** is not responsible for the interpretations, conclusions, opinions, or recommendations of others based on the data contained herein. We note that the assessment of environmental conditions for the presence of pollutants in the soil, rock, or groundwater at the site was beyond the scope of the exploration. Field observations, monitoring, and quality assurance testing during earthwork and foundation installation are an extension of the geotechnical design. We recommend that the owner retain these services and that **CTI** be allowed to continue our involvement in the project through these phases of construction.

ATTACHMENTS



CAL-TECH TESTING, INC.
 P.O. Box 1625
 Lake City, Florida 32056-1625
 Phone: (386) 755-3633
 Fax: (386) 752-5456

Vicinity Map
 Subsurface Exploration
 Richardson Aluminum New Building
 Lake City, Columbia County, Florida
 Cal-Tech Project No. 08-00141-01

Figure 1



CAL-TECH TESTING, INC.
 3309 SW SR 247
 Lake City, Florida 32024
 Telephone: (386) 755-3633
 Fax: (386) 752-5456

CLIENT Donnie Williams Construction PROJECT NAME Richardson Aluminum Building
 PROJECT NUMBER 08-00141-01 PROJECT LOCATION Arlington Blvd., Lake City, Columbia County, Florida
 DATE STARTED 02/29/08 COMPLETED 02/29/08 GROUND ELEVATION 0 ft HOLE SIZE _____
 DRILLING CONTRACTOR Cal-Tech Testing, Inc. GROUND WATER LEVELS:
 DRILLING METHOD Continuous Flight Auger AT TIME OF DRILLING ---
 LOGGED BY N.H. CHECKED BY N.H. ▼ AT END OF DRILLING 14.00 ft / Elev -14.00 ft
 NOTES _____ AFTER DRILLING ---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	▲ SPT N VALUE ▲			
								20	40	60	80
								PL	MC	LL	
								20	40	60	80
								☐ FINES CONTENT (%) ☐			
								20	40	60	80
0.0		Grayish brown, silty fine SAND, some organics (TOPSOIL)									
		LOOSE, gray to light brown, silty fine SAND (SP-SM)									
2.5			SPT 1	100	4-4-5 (9)						
			SPT 2	100	3-3-3 (6)						
5.0			SPT 3	100	3-3-4 (7)						
		LOOSE to MEDIUM DENSE, reddish tan and light gray, mottled, clayey fine sand (SC)	SPT 4	100	3-3-4 (7)						
7.5			SPT 5	100	4-5-6 (11)						
			SPT 6	100	5-7-8 (15)						
10.0											
		MEDIUM DENSE, light gray, fine sand to clayey fine sand (SP-SC)									
12.5			SPT 7	100	3-5-7 (12)						
15.0											

GEOTECH BH PLOTS - GINT STD US LAB GDT - 03/04/08 12:20 - NCALTECHSERVER\ALL LAKE CITY PROJECTS\2008\08-00141-01\08-00141-01.GPJ

Bottom of borehole at 15.0 feet.



CAL-TECH TESTING, INC.
 3309 SW SR 247
 Lake City, Florida 32024
 Telephone: (386) 755-3633
 Fax: (386) 752-5456

BORING NUMBER B-2
 PAGE 1 OF 1

CLIENT Donnie Williams Construction **PROJECT NAME** Richardson Aluminum Building
PROJECT NUMBER 08-00141-01 **PROJECT LOCATION** Arlington Blvd., Lake City, Columbia County, Florida
DATE STARTED 02/29/08 **COMPLETED** 02/29/08 **GROUND ELEVATION** 0 ft **HOLE SIZE** _____
DRILLING CONTRACTOR Cal-Tech Testing, Inc. **GROUND WATER LEVELS:**
DRILLING METHOD Continuous Flight Auger **AT TIME OF DRILLING** ---
LOGGED BY N.H. **CHECKED BY** N.H. **AT END OF DRILLING** 13.50 ft / Elev -13.50 ft
NOTES _____ **AFTER DRILLING** ---

GEOTECH BH PLOTS - GINT STD US LAB.GDT - 03/04/08 12:20 - \\CALTECHSERVER\ALL LAKE CITY PROJECTS\2008\08-00141-01\108-00141-01.GPJ

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	▲ SPT N VALUE ▲			
								20	40	60	80
								PL	MC	LL	
								20	40	60	80
								☐ FINES CONTENT (%) ☐			
								20	40	60	80
0.0		Grayish brown, silty fine SAND, some organics (TOPSOIL)									
		LOOSE, gray to light brown, silty fine SAND (SP-SM)									
2.5			SPT 1	100	2-2-3 (5)						
			SPT 2	100	3-4-3 (7)						
5.0		LOOSE to MEDIUM DENSE, reddish tan and light gray, mottled, clayey fine sand (SC)	SPT 3	100	2-3-4 (7)						
			SPT 4	100	4-7-7 (14)						
7.5			SPT 5	100	5-6-6 (12)						
			SPT 6	100	6-8-9 (17)						
10.0		MEDIUM DENSE, light gray, fine sand to clayey fine sand (SP-SC)									
12.5											
15.0			SPT 7	100	5-6-8 (14)						

Bottom of borehole at 15.0 feet.

CAL-TECH TESTING, INC.
 3309 SW SR 247
 Lake City, Florida 32024
 Telephone: (386) 755-3633
 Fax: (386) 752-5456



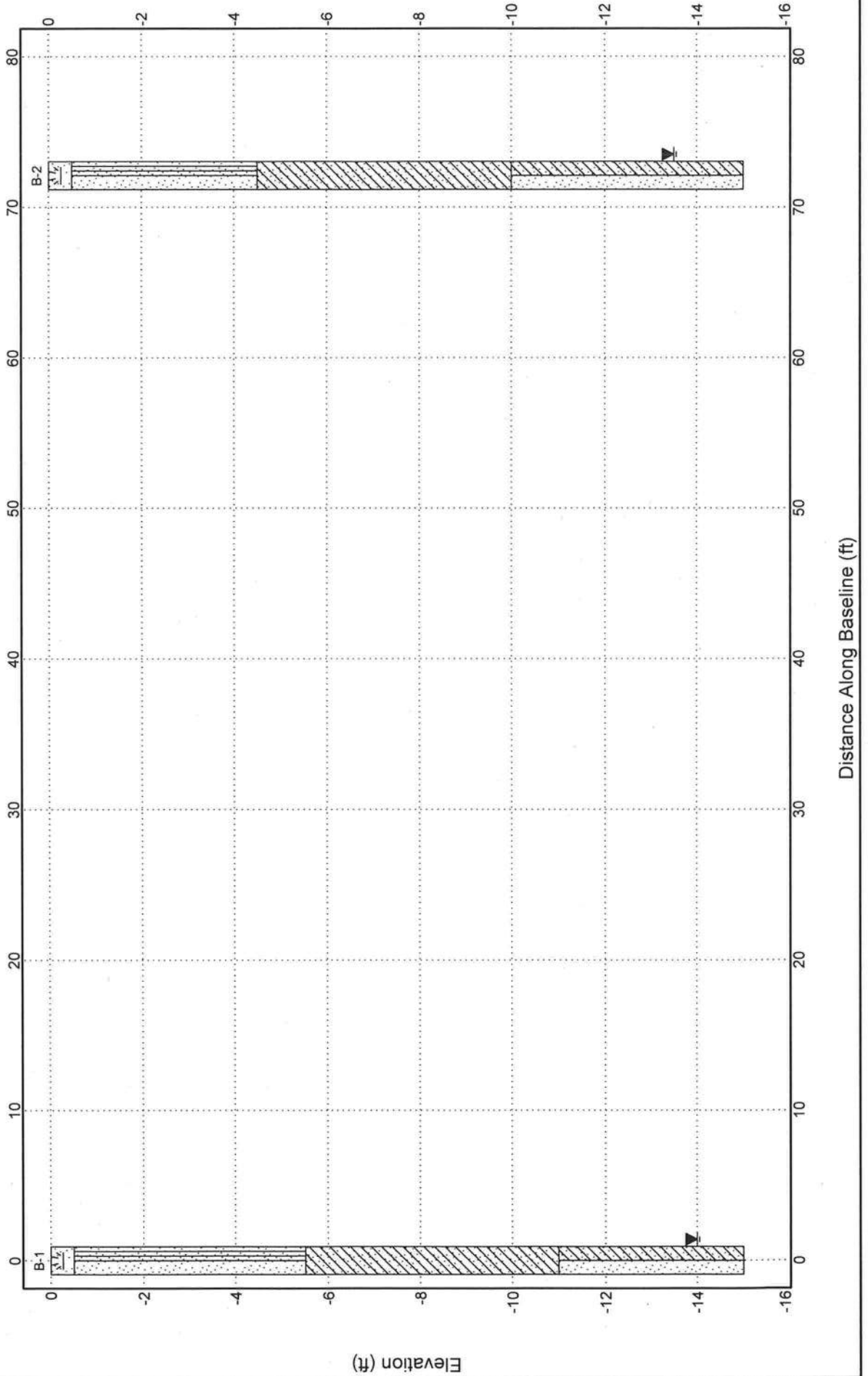
SUBSURFACE DIAGRAM

CLIENT Donnie Williams Construction

PROJECT NAME Richardson Aluminum Building

PROJECT NUMBER 08-00141-01

PROJECT LOCATION Arlington Blvd., Lake City, Columbia County, Florida



UNIFIED SOIL CLASSIFICATION SYSTEM

ASTM DESIGNATION D-2487

MAJOR DIVISIONS		GROUP SYMBOL	TYPICAL NAMES	LABORATORY CLASSIFICATION CRITERIA		
COARSE GRAINED SOILS (More than half of the material is larger than No. 200 sieve)	Gravels (more than half of the coarse fraction is larger than No. 4 sieve)	Clean gravels	GW	Well-graded gravels, gravel-sand mixtures, little or no fines.	$C_u = \frac{D_{60}}{D_{10}} > 4 ; 1 < C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}} < 3$	
		Gravel with fines	GP	Poorly graded gravels, gravel-sand mixture, little or no fines.		
		Sands (more than half of the coarse fraction is smaller than No. 4 sieve)	Clean sands	GM	Silty gravels, gravel-sand-silt mixtures.	Determine percentage of sand and gravel from grain size curve Depending on percentage of fines (fraction smaller than No. 200 Sieve size), coarse grained soils are classified as follows: Less than 5% GW, GP, SW, SP More than 12% ... GM, GC, SM, SC 5 to 12% Borderline cases requiring dual symbols
			Sands with fine	GC	Clayey gravels, gravel-sand-clay mixtures.	
			Clean sands	SW	Well-graded sands, gravelly sands, little or no fines.	
			Sands with fine	SP	Poorly graded sands, gravelly sands, little or no fines.	
	FINE GRAINED SOILS (More than half of the material is finer than No. 200 sieve)	Silts and Clays (LL less than 50)	ML	Inorganic silts, very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity.	PLASTICITY CHART 1. Plot intersection of PI as determined by the Atterberg Limits tests. 2. Points plotted above the A-Line indicate clay soils. 3. Points plotted below the A-Line indicate silt.	
			CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clay.		
			OL	Organic silts and organic silty clays of low plasticity.		
		Silts and Clays (LL greater than 50)	MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts.		
CH			Inorganic clays of high plasticity, fat clay.			
OH			Organic clays of medium to high plasticity, organic silts.			
Highly Organic Soils		Pt	Peat and other highly organic soils.			

CAL-TECH TESTING, INC.
 P.O. Box 1625
 Lake City, Florida 32056-1625
 Phone: 386-755-3633 Fax: 386-752-5456

5% Max. Passing the U.S. No. 200 Sieve SP
 5% - 12% Passing the U.S. No. 200 Sieve SM-SP
 12% - 50% Passing the U.S. No. 200 Sieve SM/SC

KEY TO TEST DATA

STANDARD PENETRATION TEST:-

Soil sampling and penetration testing is performed in accordance with ASTM D-1586. The standard penetration resistance ("N") is the number of blows of a 140-pound hammer falling 30 inches to drive a 2-inch O.D., 1.4-inch I.D. split spoon sampler one foot.

ROCK CORE DRILLING:-

Rock sampling and core drilling is performed in accordance with ASTM D-2113. The rock quality designation percentage (RQD) is determined by summing only pieces of core that are at least 4 inches long, and dividing by the "run" length.

Relation of RQD and In-situ Rock Quality	
RQD (%)	Rock Quality
90 - 100	Excellent
75 - 90	Good
50 - 75	Fair
25 - 50	Poor
0 - 25	Very Poor

RELATIVE DENSITY:-

SANDS:

Very loose - less than 4 blows/ft.
Loose - 5 to 10 blows/ft.
Medium - 11 to 30 blows/ft.
Dense - 31 to 50 blows/ft.
Very dense - over 50 blows/ft.

SILTS AND CLAYS:

Very soft - less than 2 blows/ft.
Soft - 3 to 4 blows/ft.
Medium stiff - 5 to 8 blows/ft.
Stiff - 9 to 15 blows/ft.
Very stiff - 16 to 30 blows/ft.
Hard - 31 to 50 blows/ft.
Very hard - over 50 blows/ft.

ROCKS:

Soft - Rock core crumbles when handled.
Medium - Can break core with hands.
Moderately hard - Thin edges of rock core can be broken with fingers.
Hard - Thin edges of core can not be broken with fingers.
Very hard - Can not be scratched with knife.

GROUNDWATER:- Water levels shown on boring logs are taken immediately upon completion of boring, and are intended for general information. The apparent level may have been altered by the drilling process. Groundwater levels, if desired, can be monitored over a long time interval.

Columbia County Building Permit Application

CK# 14020

For Office Use On/IV Application # 0801-134 Date Received 1/28 By JW Permit # 26866
 Zoning Official BLK Date 19.03.08 Flood Zone X FEMA Map # N/A Zoning CI
 Land Use Com. Elevation N/A MFE above RL River N/A Plans Examiner OKJH Date 3-18-08
 Comments VARIANCE 0197 reduction of Front setback to 5 feet.
 NOC EH Deed or PA Site Plan State Road Info Parent Parcel # _____
 Tri-Dev Permit # _____ n In Floodway ri Letter of Authorization from Contractor
 Unincorporated area Incorporated area Town of Fort White Town of Fort White Compliance letter

Septic Permit No. 08-020 Fox _____

Name Authorized Person Signing Permit DONNY WILLIAMS Phone 386-755-0764

Address 541 SW AIRPARK GLEN, LAKE CITY, FL 32025

Owners Name JAMES & ANDREA RICHARDSON Phone 386-755-5779

911 Address 692 SW ARLINGTON BLVD., LAKE CITY, FL 32025

Contractors Name DONNY WILLIAMS Phone 386-755-0764

Address 541 SW AIRPARK GLEN, LAKE CITY, FL 32025

Fee Simple Owner Name & Address 692 SW ARLINGTON BLVD.

Bonding Co. Name & Address NA

Architect/Engineer Name & Address HARRY V. WHIDDON, 2195 OLD QUITMAN RD., ADEL, GA.

Mortgage Lenders Name & Address NA

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee/Valley Elec. - Progress Energy

Property ID Number 31-3S-17-06262-012 Estimated Cost of Construction 75,000.00
~~8762,000.00~~

Subdivision Name NA Lot _____ Block _____ Unit _____ Phase _____

Driving Directions US #90 WEST TO CR 341, RIGHT TO FIRST ROAD TO LEFT FOLLOW TO ARLINGTON BLVD THEN LEFT 150 YDS. TO JOB
TL on midtown PL, TR Waterford Ct, TL Arlington Blvd, 150yds on right, or 2nd lot

Number of Existing Dwellings on Property 1 METAL BLD

Construction of METAL BLD. Total Acreage .573 Lot Size NA

Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Total Building Height 1

Actual Distance of Structure from Property Lines - Front 42' Side 10' Side 24' Rear 28'

Number of Stories 1 Heated Floor Area 502sf Total Floor Area 2400sf RoofPitch 4/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction,

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Vince Richardson

Owners Signature

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Donald E. Smith

Contractor's Signature (Permitee)

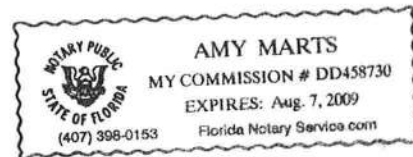
Contractor's License Number CBC-004692
Columbia County
Competency Card Number _____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 28 day of Jan 2008
Personally known or Produced Identification _____

Amy Marts

State of Florida Notary Signature (For the Contractor)

SEAL:



NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 31-3s-17-06262-012

1. Description of property: (legal description of the property and street address or 911 address)

COMM SW COR OF NE1/4 OF SW1/4, RUN E 286.94 FT TO E R/W OF A CO RD, S 29 DEG W ALONG R/W 145.52 FT
FOR POB, CONT S 29 DEG W ALONG R/W 250 FT, S 60 DEG E 100 FT, N 29
DEG E 250 FT, N 60 DEG W 100 FT TO POB. ORB 950-2449, CORR DEED 955-2704,

2. General description of improvement: OFFICE/WAREHOUSE

Inst:200812001697 Date:1/28/2008 Time:11:21 AM
DC, P. DeWitt Cason, Columbia County Page 1 of 1

3. Owner Name & Address JAMES & ANDERA RICHARDSON

692 SW ARLINGTON BLVD, LAKE CITY, FL 32025 Interest in Property OWNERS

4. Name & Address of Fee Simple Owner (if other than owner): SAME AS ABOVE

5. Contractor Name DONNY WILLAMS CONSTRUCTION LLC Phone Number 755-0764

Address 541 SW AIRPARK GLEN, LAKE CITY, FL 32055

6. Surety Holders Name NONE Phone Number _____

Address _____

Amount of Bond _____

7. Lender Name NONE Phone Number _____

Address _____

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:

Name JAMES & ANDERA RICHARDSON Phone Number 755-5779

Address 692 SW ARLINGTON BLVD, LAKE CITY, FL 32025

9. In addition to himself / herself the owner designates None of _____
_____ to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) -

(a) 7. Phone Number of the designee NA

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording,
(Unless a different date is specified) _____

NOTICE AS PER CHAPTER 713, Florida Statutes:

The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Vince Richardson
Signature of Owner

Sworn to (or affirmed) and subscribed before
day of January 28, 2008



Amy Marts
Signature of Notary



Columbia County, Florida Planning & Zoning Department

Review of Building Permit for compliance with
County's Comprehensive Plan and
Land Development Regulations

To: Donny Williams

Fax: 386.755.0764

From : Brian L. Kepner, County Planner

Fax: 386.758.2160

Number of Pages : 1

Date : 18 March 2008

RE: Building Permit Application 0801-134, Richardson

Dear Donny:

I am finishing up the review of the above referenced building permit application. Your revised site plan shows two (2) handicap parking spaces on the west side of the proposed building. Is there an existing handicap parking space on the property? If so, then no additional handicap spaces would be required for the property because the total number of parking spaces need has not exceeded twenty-five (25) spaces. Nothing says you cannot have more. If you desire them to be handicap parking spaces the required size is thirteen (13) feet in width and twenty (20) feet in length. Your revised site plan shows the handicap spaces at ten (10) feet wide and nineteen (19) feet long.

If you have any questions concerning this matter, please do not hesitate to contact me at 386.758.1007.

Sincerely,

Brian L. Kepner
Land Development Regulation Administrator,
County Planner

Confidentiality Notice: This facsimile transmission is confidential and is intended only for the review of the party to whom it is addressed. It may contain proprietary and/or privileged information protected by law. If you are not the intended recipient, you may not use, copy or distribute this facsimile message or its attachments. If you have received this transmission in error, please immediately telephone the sender above to arrange for its return.

District No. 1 - Ronald Williams
District No. 2 - Dewey Weaver
District No. 3 - George Skinner
District No. 4 - Stephen E. Bailey
District No. 5 - Elizabeth Porter



BOARD OF COUNTY COMMISSIONERS • COLUMBIA COUNTY

18 March 2008

TO: File
FROM: Land Development Regulation Administrator
SUBJECT: BP 08-1 (Williams/Richardson)

Concurrency Assessment Concerning a Building Permit

The following assessment is provided for the purpose of a binding concurrency determination regarding the demand and residual capacities for public facilities required to be addressed within the Concurrency Management System. This assessment serves as a binding concurrency determination, but does not ensure that facilities, which are not owned, operated or permitted by the County will be available to the property at the time development occurs.

BP 08-1, an application by Donny Williams, as agent for James and Andrea Richardson, for building permit approval for general office and warehouse use located in a COMMERCIAL INTENSIVE (CI) zoning district in accordance with a site plan and submitted as part of building permit application 0801-134 dated January 28, 2008 to be located on property described, as follows:

A parcel of land lying within Section 31, Township 3 South, Range 17 East, Columbia County, Florida. Being more particularly described, as follows: Commence at the Southwest corner of the Northeast $\frac{1}{4}$ of the Southwest $\frac{1}{4}$ of said Section 31; thence North $88^{\circ}07'51''$ East along the South line of said Northeast $\frac{1}{4}$ of the Southwest $\frac{1}{4}$ of said Section 31 a distance of 286.94 feet to the East right-of-way line of Southwest Arlington Boulevard; thence South $29^{\circ}36'25''$ West along said East right-of-way line of Southwest Arlington Boulevard a distance of 145.52 feet to the Point of Beginning; thence continue South $29^{\circ}36'25''$ West still along said East right-of-way line of Southwest Arlington Boulevard a distance 250.00 feet; thence South $60^{\circ}38'16''$ East 100.00 feet; thence North $29^{\circ}36'25''$ East 250.00 feet; thence North $60^{\circ}38'16''$ West 100.00 feet to the Point of Beginning.

Containing 0.57 acre, more or less.

BOARD MEETS FIRST THURSDAY AT 7:00 P.M.
AND THIRD THURSDAY AT 7:00 P.M.