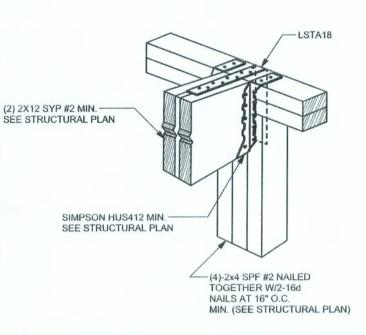


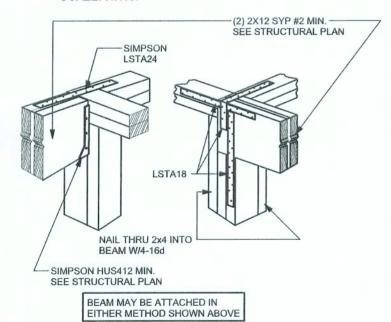
## EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

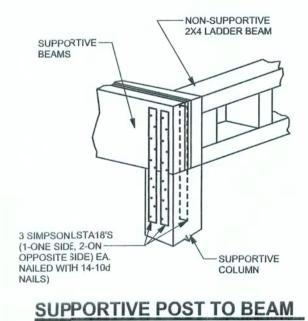
STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.



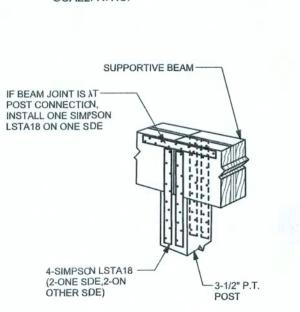
BEAM MID-WALL CONNECTION DETAIL SCALE: N.T.S.



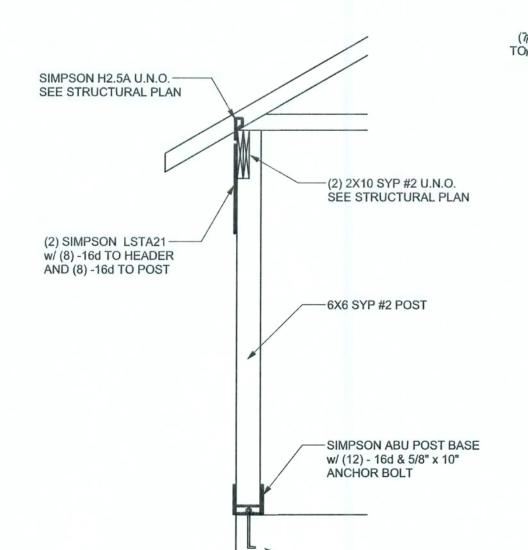
**BEAM CORNER CONNECTION. DETAIL** SCALE: N.T.S.



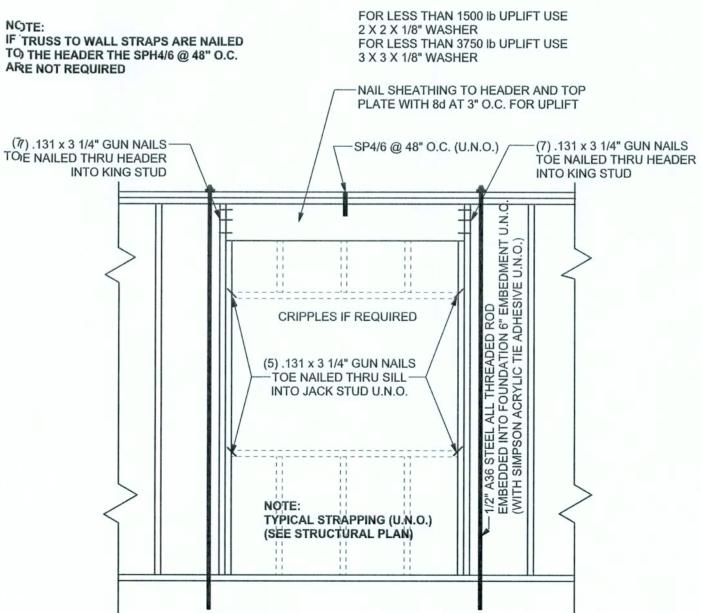
DETAIL FOR SINGLE BEAM SCALE: N.T.S.



SUPPORTIVE CENTER POST TO BEAM DETAIL



-SEE FOOTING DETAILS TYPICAL PORCH POST DETAIL



(1) 2X6 SPF #2 SILL UP TO 7'-6" U.N.O.

(2) 2X4 SPF #2 SILL UP TO 7'-8" U.N.O.

(1) 2X4 SPF #2 SILL UP TO 5'-1" U.N.O.

(FOR: 120 MPH, 10'-0" WALL HEIGHT U.N.O.)

TYPICAL 1 STORY HEADER STRAPING DETAIL

MSTA30, 10-10d (1700lb)

(5) NAILS EACH SIDE OF STUD

1/2" ANCHOR w/ 7" EMBEDMENT, SIMPSON AT ----

(MAY BE RECESSED BELOW FINISHED FLOOR)

SCALE: 1/2" = 1'-0"

(OR STRAP STUD TO HEADER 20-10d)

LTT20B, 10-16d (1750lb)

ALTERNATE WALL TIE CONNECTION WHERE

THREADED ROD CANNOT BE PLACED IN WALL

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			÷n
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATI
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED 12" EMBEDME
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED 12" EMBEDME
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED 12" EMBEDME
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED 12" EMBEDME
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2
< 1240	< 1065	SPH4			10-10d, 1 1/2
< 885	< 760	SP6			6-10d, 1 1/2
< 1240	< 1065	SPH6			10-10d, 1 1/2
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATI
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

#### ANCHOR TABLE

## **GRADE & SPECIES TABLE**

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

#### **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBC 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS

VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" × 0" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 48 \* DB (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MENBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPÉCIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

#### BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

#### ROOF SYSTEM DESIGN

BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2004, SECTION 1609 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

## **DESIGN DATA**

MEAN I	OSED SIMPL ROOF HEIGH PER HALF O AND UNOB	HT NOT E F HILL OF	XCEEDIN R ESCARF	G LEAST PMENT 60	HORIZON	YTAL P. B.	DIME 30FT	NSION IN EX	OR 60	) FT; ND >1
BUILDI	NG IS NOT I	N THE HIC	3H VELOC	CITY HURI	RICANE	ZONE				
BUILDI	NG IS NOT I	N THE WI	ND-BORN	E DEBRIS	REGION	V				
1.) BA	ASIC WIND S	PEED =	110 MPH							
2.) W	IND EXPOSI	JRE = B								
3.) W	IND IMPORT	ANCE FA	CTOR = 1	1.0						
4.) BL	JILDING CAT	EGORY =	: 11							
5.) RO	OOF ANGLE	= 10-45 D	EGREES							
6.) ME	EAN ROOF H	IEIGHT =	<30 FT							
7.) IN	TERNAL PR	ESSURE (	COEFFICI	ENT = N/A	(ENCLC	SED	BUILD	DING)	_	
8.) CC	OMPONENTS	S AND CL	ADDING [	DESIGN W	IND PRE	SSUF	RES [	TABLE	R301.	2(2)]
3	2	2 2 1 2	5		1 2	19.9 19.9	-21.8 -42.1	18.1 18.1	-18.1 -29.1	
3/	4	2	5			-		-	-	
		5 5			3 4	-	-42.1 -23.6	18.1	-29.1	
2	STEEL STEEL	***			5	-	-29.1	18.5	-22.6	
/	2/3				Doors	& Wind	dows	21.8	-29.1	
(3)	1				1.000	st Cas	5.0			
2	2	/3	5		8x7 Gar	5, 10		19.5	-21.3	
	1,	3	4 Z		16x7 Ga			18.5	-20.4	
		55	₹F							
DESIGN	LOADS									
FLOOR	40 PSF (A	LL OTHE	R DWELL	NG ROOM	MS)					
	30 PSF (S	LEEPING	ROOMS)							
	20 PSF (A	TTICS WI	TH STOR/	AGE)						
	10 PSF (A	TTICS WIT	THOUT ST	TORAGE,	<3:12)					
	40 PSF (D									
	00 DOF /F	<b>XTERIOR</b>	BALCONI	ES)						
ROOF	20 PSF (F	LAT OR <								
ROOF		LAT OR < :12 TO <12	2:12)							

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

SOFTPIXN

REVISIONS

PE No.53915, POB 868, Lake City, FL

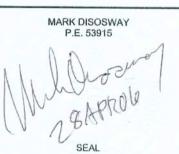
32056, 386-754-5419

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examined this plan, and that the applicable portions of the plan, relating to wind engineer comply with section R301.2.1, florida building code residential 2004, to the best of my

LIMITATION: This design is valid for one building, at specified location.



RICHARD KEEN

SPEC HOUSE

ADDRESS: Lot 5 Carolin Heights S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

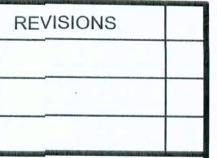
April 28, 2006 DRAWN BY: STRUCTURAL B Ben Sparks Ben Sparks

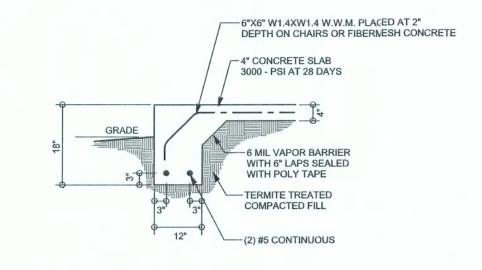
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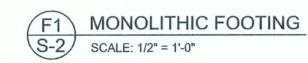
FINALS DATE: 27 / Apr / 06

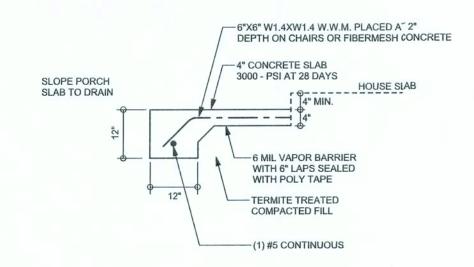
> JOB NUMBER: 604262 DRAWING NUMBER

> > OF 6 SHEETS

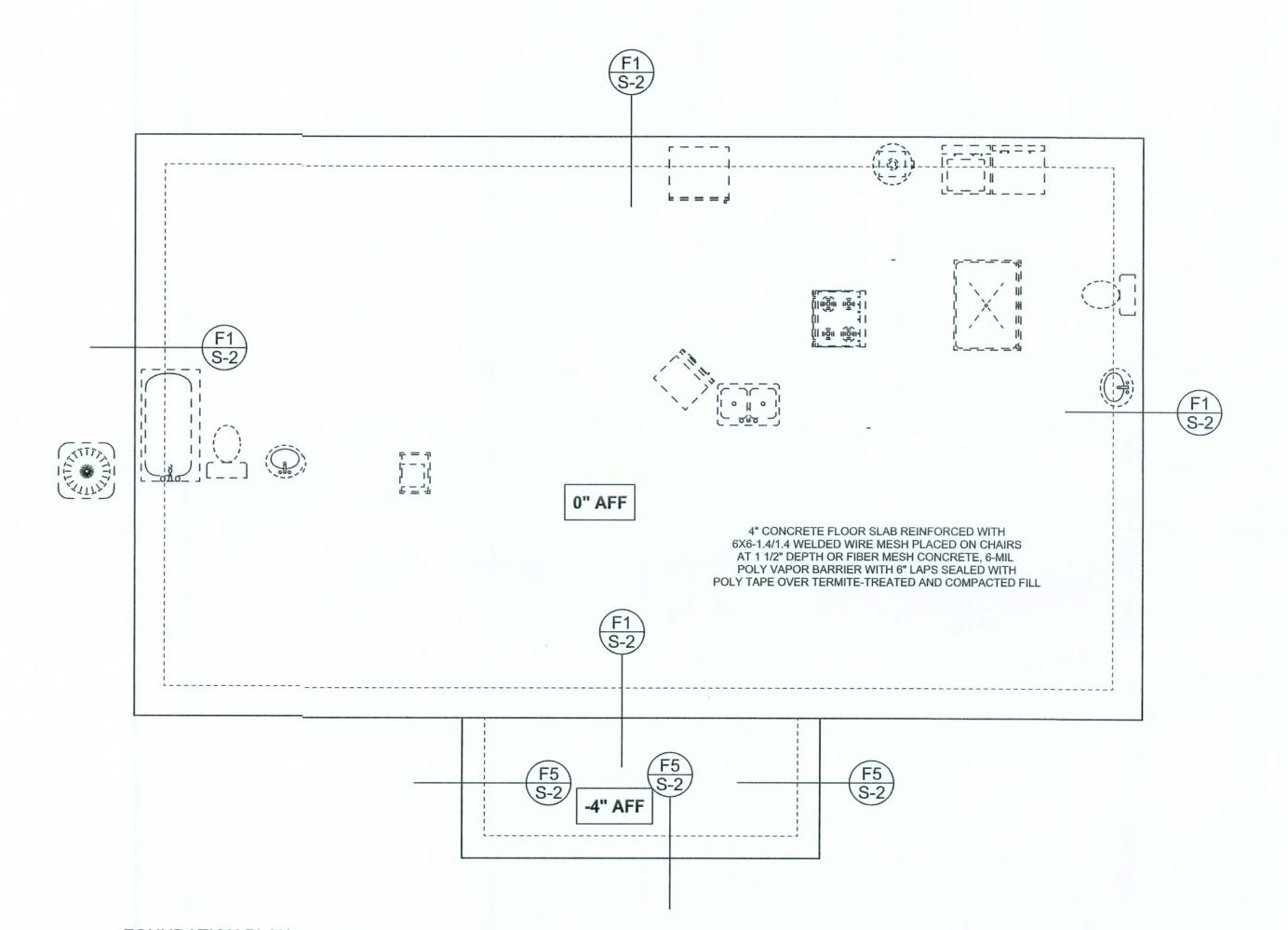








F5 PORCH FOO S-2 SCALE: 1/2" = 1'-0" PORCH FOOTING



FOUNDATION PLAN SCALE: 1/4" = 1'-0" DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCIHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 DIMENSIONS:
Stated dimensions supercede scaled dimensions Refer all questions to Mark Disosway, P.E. for resolution.
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MARK DISOSWAY P.E. 53915

RICHARD KEEN

SPEC HOUSE

ADDRESS: Lot 5 Carolin Heights S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: April 28, 2006 DRAWN BY: STRUCTURAL BY: Ben Sparks Ben Sparks

FINALS DATE: 27 / Apr / 06

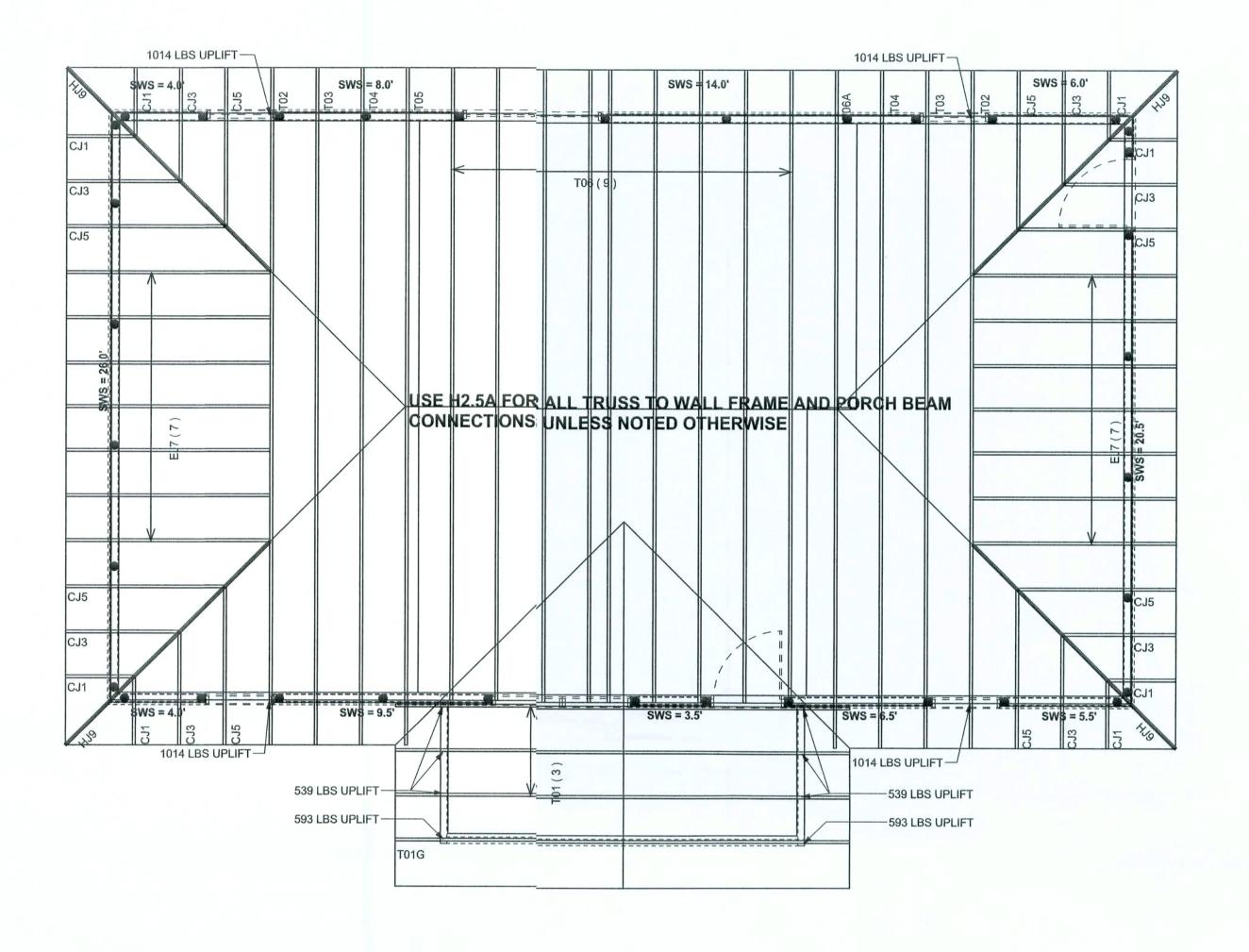
JOB NUMBER: 604262 DRAWING NUMBER

**S-2** 

OF 6 SHEETS

REVISIONS

SOFTPIA



STRUCTURAL PLAN SCALE: 1/4" = 1'-0"

## STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3
DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3, BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

## WALL LEGEND

SM2 = 0.0,	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOIR EXTERIOR
IBW \$0000001 = = = = 10000000	1ST FLOOIR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOIR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

## THREADED ROD LEGEND

•	INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD
<b>®</b>	INDICATES LOCATION OF: 2ND FLOOR 1/2" A307 ALL THREADED ROD

## TOTAL SHEAR WALL SEGMENTS

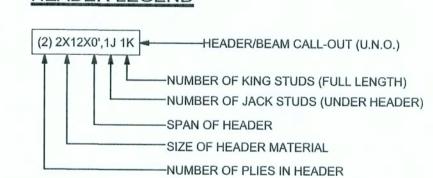
SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

REQUIRED ACTUAL

TRANSVERSE 35.0' 46.5'

LONGITUDINAL 15.0' 61.0'

# HEADER LEGEND



CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE (JOB #L16/997 13, LS.

WINDLOAD ENGINEER: Mark Disosway PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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MARK DISOSWAY
P.E. 53915

SEAL

RICHARD KEEN

SPEC HOUSE

ADDRESS: Lot 5 Carolin Heights S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE:
April 28, 2006

DRAWN BY: STRUCTURAL BY:
Ben Sparks Ben Sparks

FINALS DATE: 27 / Apr / 06

JOB NUMBER: 604262

DRAWING NUMBER

OF 6 SHEETS