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Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 21-5236
Job Description: Fort White Park-Lot 32	
Address: Live Oak, FL	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res	IntelliVIEW Version: 20.01.01A
	JRef #: 1X372150004
Wind Standard: ASCE 7-16	Wind Speed (mph): 130
Building Type: Closed	Design Loading (psf): 40.00

This package contains general notes pages, 22 truss drawing(s) and 3 detail(s).

Item	Drawing Number	Truss
1	056.21.0751.56210	A1
3	056.21.0752.00647	A11
5	056.21.0752.06767	A13
7	056.21.0752.10180	A15
9	056.21.0752.15907	A3
11	056.21.0752.20690	A5
13	056.21.0752.24430	JC4
15	056.21.0752.26893	JE7
17	056.21.0752.29263	JE8A
19	056.21.0752.32530	PBA1
21	056.21.0752.40293	PBA3
23	A14015ENC160118	
25	PB160160118	

Item	Drawing Number	Truss
2	056.21.0751.58807	A10
4	056.21.0752.04667	A12
6	056.21.0752.08203	A14
8	056.21.0752.13600	A2
10	056.21.0752.18517	A4
12	056.21.0752.23047	JC2
14	056.21.0752.25583	JC6
16	056.21.0752.28070	JE8
18	056.21.0752.30480	JH11
20	056.21.0752.34743	PBA2
22	056.21.0752.44697	PBA4
24	GBLLETIN0118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

lc = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

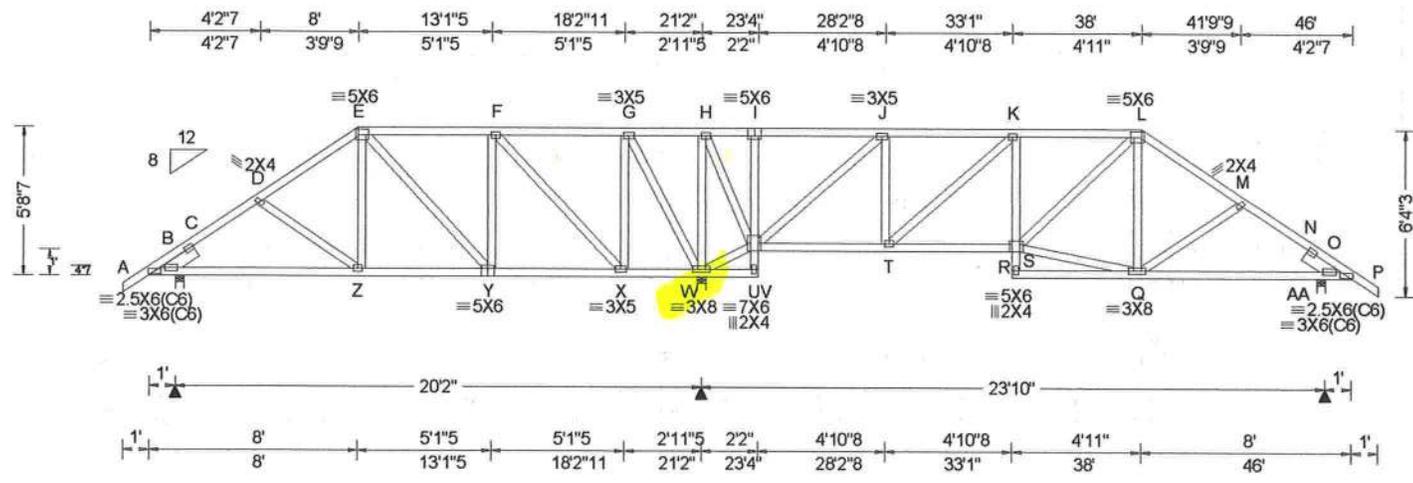
Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcindustry.com.

2 Complete Trusses Required



Loading Criteria (psf)

TCLL: 20.00
 TC DL: 10.00
 BCLL: 0.00
 BCDL: 10.00
 Des Ld: 40.00
 NCBCLL: 0.00
 Soffit: 0.00
 Load Duration: 1.25
 Spacing: 24.0"

Wind Criteria

Wind Std: ASCE 7-16
 Speed: 130 mph
 Enclosure: Closed
 Risk Category: II
 EXP: C Kzt: NA
 Mean Height: 15.00 ft
 TC DL: 5.0 psf
 BCDL: 5.0 psf
 MWFRS Parallel Dist: 0 to h/2
 C&C Dist a: 4.60 ft
 Loc. from endwall: not in 6.50 ft
 GCpi: 0.18
 Wind Duration: 1.25

Snow Criteria (Pg,Pf in PSF)

Pg: NA Ct: NA CAT: NA
 Pf: NA Ce: NA
 Lu: NA Cs: NA
 Snow Duration: NA

Building Code:
 FBC 7th Ed. 2020 Res.
 TPI Std: 2014
 Rep Fac: No
 FT/RT:20(0)/0(0)
 Plate Type(s):
 WAVE

Defl/CSI Criteria

PP Deflection in loc L/defl L/#
 VERT(LL): 0.052 Q 999 240
 VERT(CL): 0.109 Q 999 240
 HORZ(LL): 0.044 N - -
 HORZ(TL): 0.093 N - -
 Creep Factor: 2.0
 Max TC CSI: 0.907
 Max BC CSI: 0.612
 Max Web CSI: 0.696

VIEW Ver: 20.01.01A.0724.11

Maximum Reactions (lbs)

Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
B	1086	-	-	-	/235	-
W	5349	-	-	-	/1499	-
AA	1514	-	-	-	/305	-

Wind reactions based on MWFRS
 B Brg Width = 4.0 Min Req = 1.5
 W Brg Width = 4.0 Min Req = 2.8
 AA Brg Width = 4.0 Min Req = 1.5
 Bearings B, W, & AA are a rigid surface.
 Members not listed have forces less than 375#
Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
C - D	131 -536	I - J	994 -287
D - E	118 -502	K - L	163 -802
F - G	620 -149	L - M	170 -820
G - H	1271 -347	M - N	180 -829
H - I	994 -287		

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.500'
 Rt Slider: 2x6 SP 2400f-2.0E; block length = 1.500'

Wind

Wind loads and reactions based on MWFRS.
 Left and right cantilevers are exposed to wind
 Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - Z	378 -87	T - R	801 -163
Z - Y	405 -93	Q - O	596 -123
X - W	163 -672		

Nailnote

Nail Schedule: 0.131"x3", min. nails
 Top Chord: 1 Row @12.00" o.c.
 Bot Chord: 1 Row @12.00" o.c.
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

Additional Notes

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
 The overall height of this truss including bracing is 5-8-7.

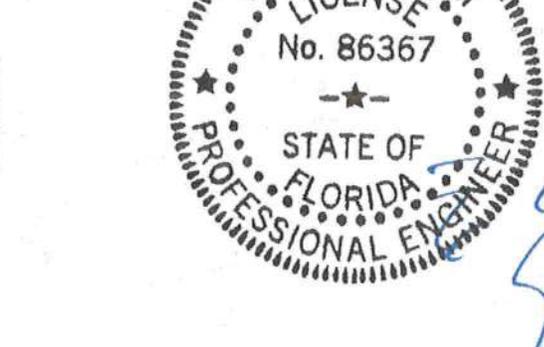
Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
B - C	144 -443	W - U	393 -1430
E - Y	54 -379	H - U	675 -138
Y - F	544 -107	U - J	373 -1525
F - X	299 -1046	J - T	736 -151
X - G	971 -265	T - K	217 -742
G - W	401 -1305	R - Q	670 -132
W - H	160 -670	N - O	177 -632

Special Loads

(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC: From 64 plf at -1.00 to 64 plf at 8.00
 TC: From 32 plf at 8.00 to 32 plf at 38.00
 TC: From 64 plf at 38.00 to 64 plf at 47.00
 BC: From 20 plf at 0.00 to 20 plf at 8.03
 BC: From 10 plf at 8.03 to 10 plf at 37.97
 BC: From 20 plf at 37.97 to 20 plf at 46.00
 TC: 112 lb Conc. Load at 8.06,10.06,12.06,14.06,16.06,18.06,20.06,22.06,33.94,35.94,37.94
 TC: 141 lb Conc. Load at 23.94,25.94,27.94,29.94,31.94
 BC: 453 lb Conc. Load at 8.03,37.97
 BC: 170 lb Conc. Load at 10.06,12.06,14.06,16.06,18.06,20.06,22.06,33.94,35.94
 BC: 148 lb Conc. Load at 23.94,25.94,27.94,29.94,31.94



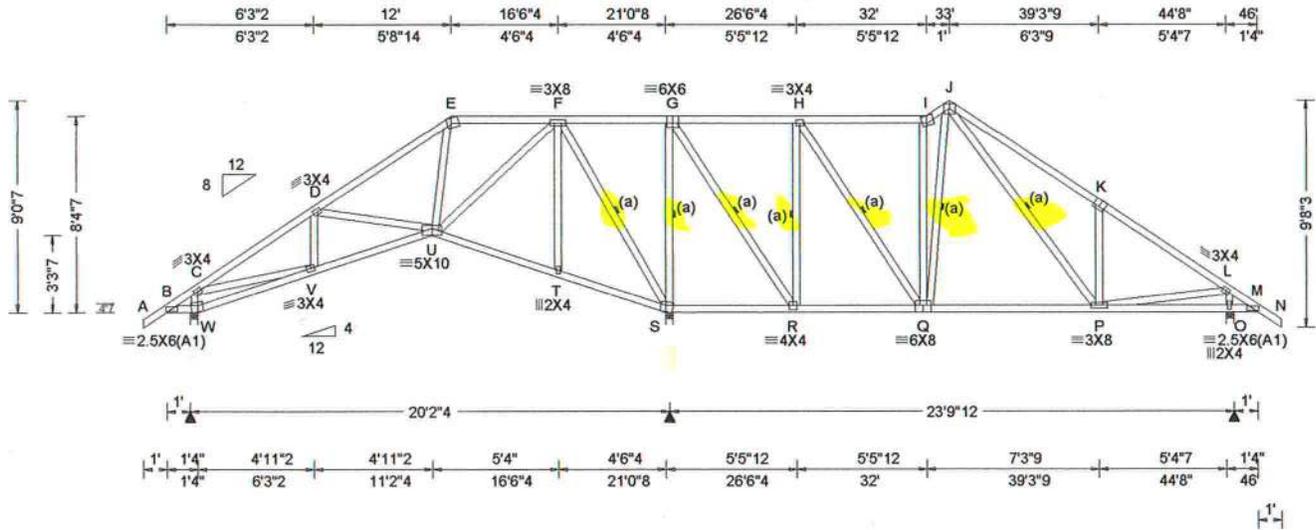
FL REG# 278, Yoonhwak Kim, FL PE #86367
 02/25/2021

Plating Notes

All plates are 3X4 except as noted.

****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
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 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinet.org; SBCA: sbcaindustry.com; ICC: iccsafe.org; AWC: awc.org





Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.60 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.25	Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/0(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.041 I 999 240 VERT(CL): 0.084 I 999 240 HORZ(LL): 0.017 O - - HORZ(TL): 0.036 S - - Creep Factor: 2.0 Max TC CSI: 0.499 Max BC CSI: 0.585 Max Web CSI: 0.816 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL W 753 /- /- /494 /14 /296 S 2629 /- /- /1345 /265 /- O 1117 /- /- /728 /86 /- Wind reactions based on MWFRS W Brg Width = 4.0 Min Req = 1.5 S Brg Width = 3.5 Min Req = 3.1 O Brg Width = 4.0 Min Req = 1.5 Bearings W, S, & O are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.					
				Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;	C - D 186 -922 I - J 360 -695 D - E 151 -505 J - K 443 -1211 F - G 732 -39 K - L 265 -1193 H - I 310 -614				

Bracing (a) Continuous lateral restraint equally spaced on member. Or 1x4 "T" reinforcement. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" oc.	Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. V - U 775 -308 Q - P 593 0 S - R 275 -678
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Plating Notes All plates are 5X6 except as noted.	Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. C - W 217 -631 R - H 351 -984 C - V 799 -149 H - Q 733 -96 D - U 204 -411 Q - I 280 -468 U - F 716 -150 J - P 584 -211 F - S 250 -985 P - K 253 -390 S - G 484 -1504 P - L 882 -160 G - R 1389 -349 L - O 311 -1053
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Loading
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left and right cantilevers are exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
 The overall height of this truss excluding overhang is 9'-0.7".



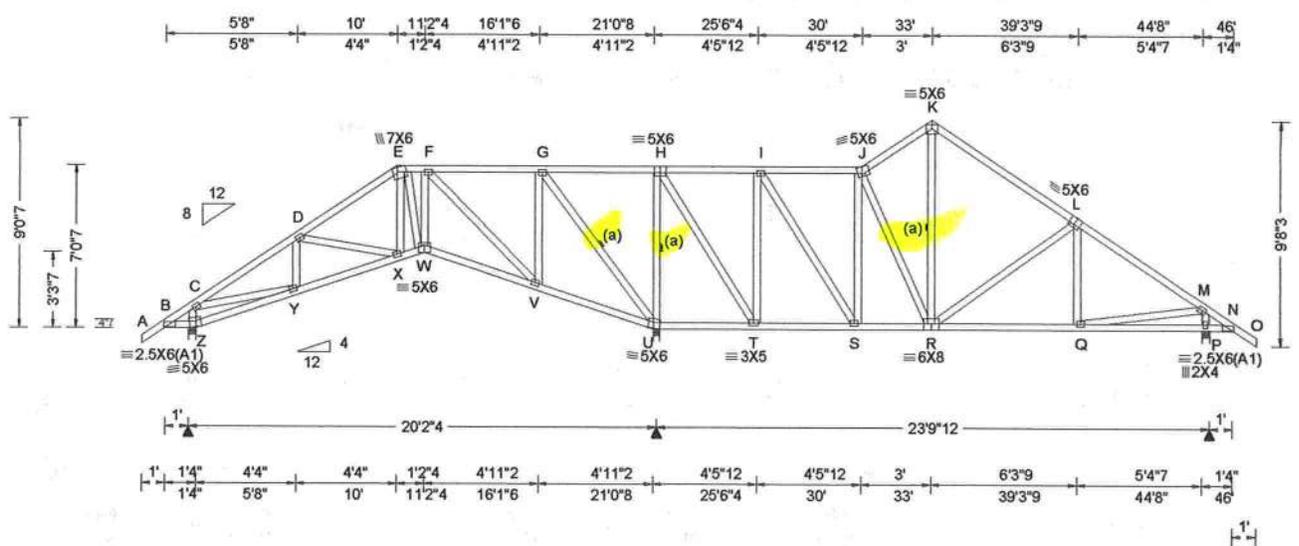
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 4.60 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.25	Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/0(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.029 X 999 240 VERT(CL): 0.066 X 999 240 HORZ(LL): 0.024 P - - HORZ(TL): 0.054 U - - Creep Factor: 2.0 Max TC CSI: 0.541 Max BC CSI: 0.434 Max Web CSI: 0.828 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>Z</td> <td>781</td> <td>-</td> <td>-</td> <td>/493</td> <td>/13</td> <td>/296</td> </tr> <tr> <td>U</td> <td>2346</td> <td>-</td> <td>-</td> <td>/1311</td> <td>/158</td> <td>-</td> </tr> <tr> <td>P</td> <td>1012</td> <td>-</td> <td>-</td> <td>/714</td> <td>/49</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS Z Brg Width = 4.0 Min Req = 1.5 U Brg Width = 3.5 Min Req = 2.8 P Brg Width = 4.0 Min Req = 1.5 Bearings Z, U, & P are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> <th>Chords</th> <th>Tens.</th> <th>Comp.</th> </tr> </thead> <tbody> <tr> <td>C - D</td> <td>198</td> <td>-954</td> <td>I - J</td> <td>264</td> <td>-457</td> </tr> <tr> <td>D - E</td> <td>198</td> <td>-685</td> <td>J - K</td> <td>294</td> <td>-608</td> </tr> <tr> <td>E - F</td> <td>206</td> <td>-497</td> <td>K - L</td> <td>272</td> <td>-692</td> </tr> <tr> <td>G - H</td> <td>812</td> <td>-65</td> <td>L - M</td> <td>231</td> <td>-1012</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	Z	781	-	-	/493	/13	/296	U	2346	-	-	/1311	/158	-	P	1012	-	-	/714	/49	-	Chords	Tens.	Comp.	Chords	Tens.	Comp.	C - D	198	-954	I - J	264	-457	D - E	198	-685	J - K	294	-608	E - F	206	-497	K - L	272	-692	G - H	812	-65	L - M	231	-1012
Loc	Gravity			Non-Gravity																																																																
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Z	781	-	-	/493	/13	/296																																																														
U	2346	-	-	/1311	/158	-																																																														
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C - D	198	-954	I - J	264	-457																																																															
D - E	198	-685	J - K	294	-608																																																															
E - F	206	-497	K - L	272	-692																																																															
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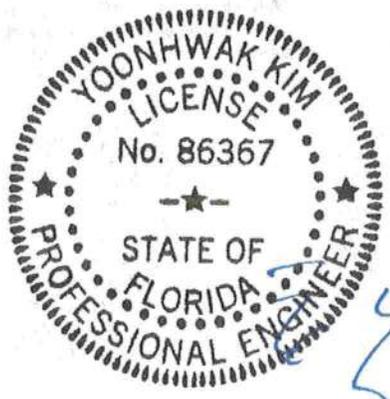
Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member. Or 1x4 "T" reinforcement. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" oc.

Plating Notes
 All plates are 3X4 except as noted.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left and right cantilevers are exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
 The overall height of this truss excluding overhang is 9-0-7.

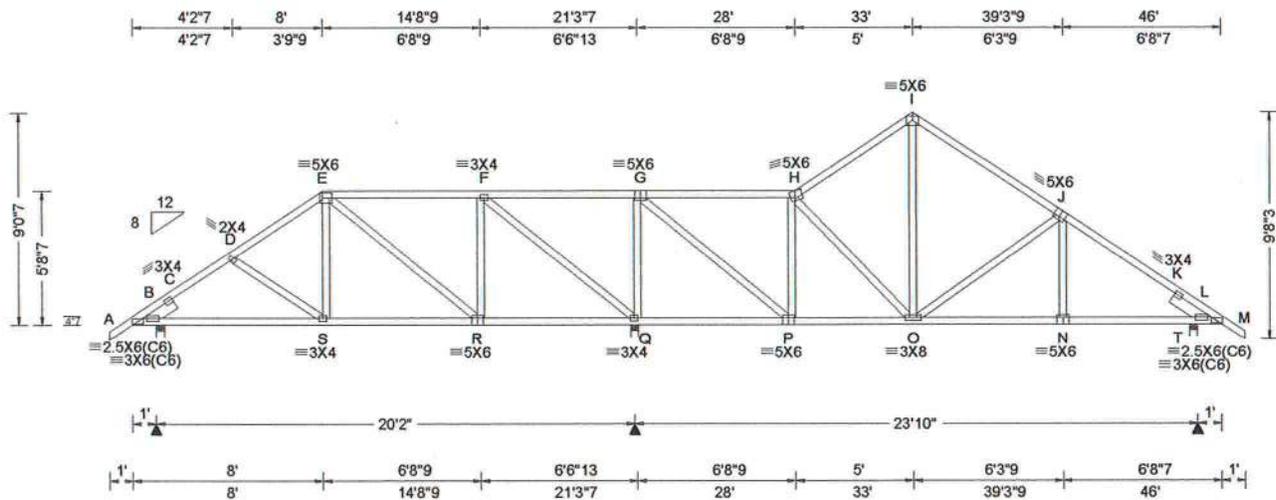


FL REG# 278, Yoonhwak Kim, FL PE #86367
 02/25/2021

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→ 2 Complete Trusses Required



Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.60 ft Loc. from endwall: not in 13.00 ft GCpi: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/0(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.040 S 999 240 VERT(CL): 0.087 S 999 240 HORZ(LL): -0.028 C - - HORZ(TL): 0.062 C - - Creep Factor: 2.0 Max TC CSI: 0.819 Max BC CSI: 0.701 Max Web CSI: 0.874 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>1351</td> <td>-</td> <td>-</td> <td>-</td> <td>1313</td> <td>-</td> </tr> <tr> <td>Q</td> <td>3705</td> <td>-</td> <td>-</td> <td>-</td> <td>1886</td> <td>-</td> </tr> <tr> <td>T</td> <td>822</td> <td>-</td> <td>-</td> <td>-</td> <td>1108</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 Q Brg Width = 4.0 Min Req = 1.8 T Brg Width = 4.0 Min Req = 1.5 Bearings B, Q, & T are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>C - D</td> <td>186 -721</td> <td>F - G</td> <td>795 -192</td> </tr> <tr> <td>D - E</td> <td>179 -709</td> <td></td> <td></td> </tr> </tbody> </table> Maximum Bot Chord Forces Per Ply (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - S</td> <td>512 -126</td> <td>Q - P</td> <td>181 -752</td> </tr> <tr> <td>S - R</td> <td>586 -148</td> <td></td> <td></td> </tr> </tbody> </table> Maximum Web Forces Per Ply (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>185 -583</td> <td>Q - G</td> <td>205 -794</td> </tr> <tr> <td>R - F</td> <td>566 -107</td> <td>G - P</td> <td>774 -137</td> </tr> <tr> <td>F - Q</td> <td>400 -1351</td> <td>P - H</td> <td>122 -470</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	1351	-	-	-	1313	-	Q	3705	-	-	-	1886	-	T	822	-	-	-	1108	-	Chords	Tens.Comp.	Chords	Tens. Comp.	C - D	186 -721	F - G	795 -192	D - E	179 -709			Chords	Tens.Comp.	Chords	Tens. Comp.	B - S	512 -126	Q - P	181 -752	S - R	586 -148			Webs	Tens.Comp.	Webs	Tens. Comp.	B - C	185 -583	Q - G	205 -794	R - F	566 -107	G - P	774 -137	F - Q	400 -1351	P - H	122 -470
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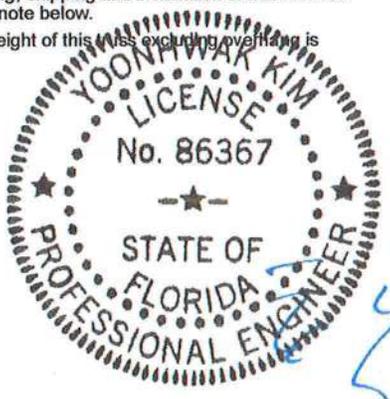
Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.500'
Rt Slider: 2x6 SP 2400f-2.0E; block length = 1.848'

Nailnote
Nail Schedule: 0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
---(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC: From 64 plf at -1.00 to 64 plf at 8.00
TC: From 32 plf at 8.00 to 32 plf at 21.43
TC: From 64 plf at 21.43 to 64 plf at 47.00
BC: From 20 plf at 0.00 to 20 plf at 8.03
BC: From 10 plf at 8.03 to 10 plf at 20.06
BC: From 20 plf at 20.06 to 20 plf at 46.00
TC: 112 lb Conc. Load at 8.06,10.06,12.06,14.06,16.06,18.06
TC: 200 lb Conc. Load at 20.06
BC: 453 lb Conc. Load at 8.03
BC: 170 lb Conc. Load at 10.06,12.06,14.06,16.06,18.06
BC: 132 lb Conc. Load at 20.06

Wind
Wind loads and reactions based on MWFRS.
Left and right cantilevers are exposed to wind
Wind loading based on both gable and hip roof types.

Additional Notes
WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
The overall height of this truss excluding overhang is 9'-0".



FL REG# 278, Yoonhwak Kim, FL PE #86367
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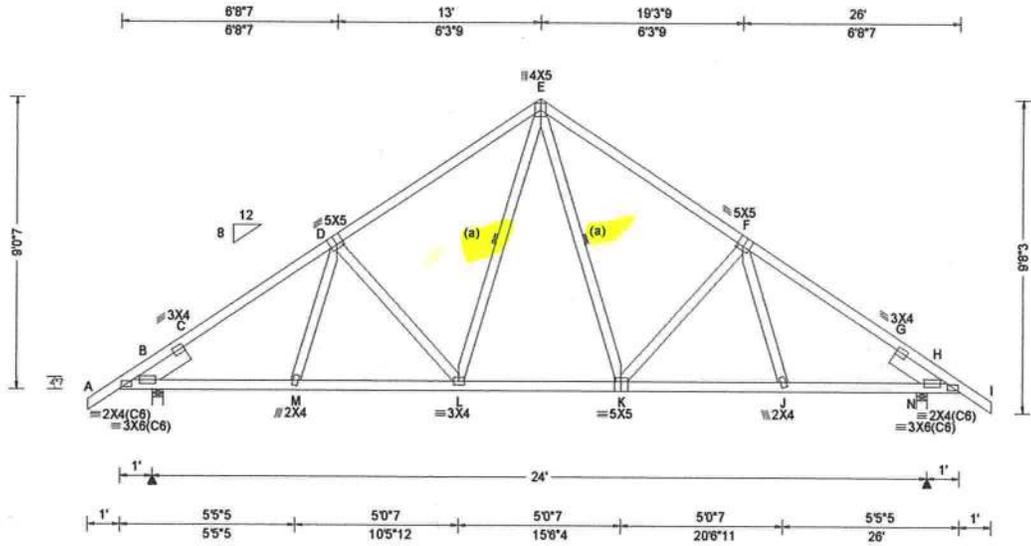
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6750 Forum Drive
 Suite 305
 Orlando FL, 32821



Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/0(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.090 L 999 240 VERT(CL): 0.177 L 999 240 HORZ(LL): 0.070 G - - HORZ(TL): 0.139 G - - Creep Factor: 2.0 Max TC CSI: 0.801 Max BC CSI: 0.528 Max Web CSI: 0.139 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>1213</td> <td>-</td> <td>-</td> <td>/727</td> <td>/181</td> <td>/275</td> </tr> <tr> <td>N</td> <td>1213</td> <td>-</td> <td>-</td> <td>/727</td> <td>/181</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 N Brg Width = 4.0 Min Req = 1.5 Bearings B & N are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>0 -621</td> <td>E - F</td> <td>411 -1120</td> </tr> <tr> <td>C - D</td> <td>338 -1251</td> <td>F - G</td> <td>338 -1251</td> </tr> <tr> <td>D - E</td> <td>411 -1121</td> <td>G - H</td> <td>0 -621</td> </tr> </tbody> </table> </p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	1213	-	-	/727	/181	/275	N	1213	-	-	/727	/181	-	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	0 -621	E - F	411 -1120	C - D	338 -1251	F - G	338 -1251	D - E	411 -1121	G - H	0 -621
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP M-31;
 Webs: 2x4 SP #3;
 Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.835'
 Rt Slider: 2x6 SP 2400f-2.0E; block length = 1.835'

Bracing
 (a) Continuous lateral restraint equally spaced on member. Or 1x4 "T" reinforcement. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

Loading
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left and right cantilevers are exposed to wind
 Wind loading based on both gable and hip roof types.

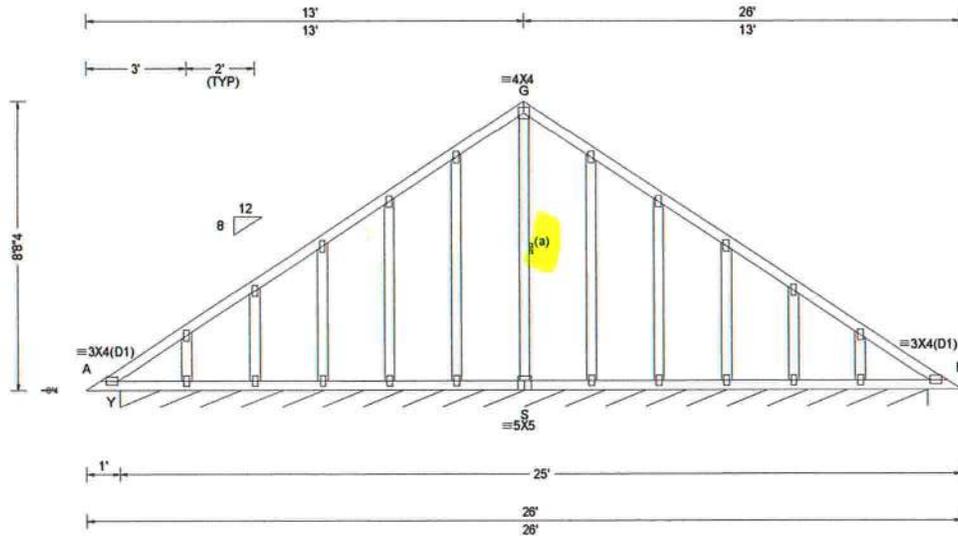
Additional Notes
 The overall height of this truss excluding overhang is 9-0-7.



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Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member. Or 1x4 "T" reinforcement. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.

Plating Notes

All plates are 2X4 except as noted.

Wind

Wind loads based on MWFRS with additional C&C member design.

Left and right cantilevers are exposed to wind

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

The overall height of this truss excluding overhang is 8-8-4.



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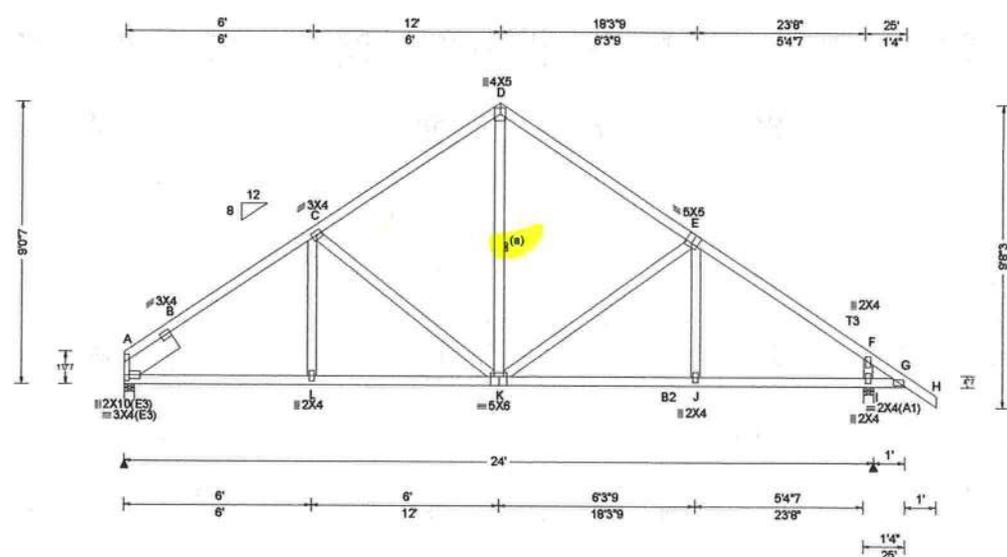
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Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.25	Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/0(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.084 E 999 240 VERT(CL): 0.175 E 999 240 HORZ(LL): 0.061 F - - HORZ(TL): 0.128 F - - Creep Factor: 2.0 Max TC CSI: 0.525 Max BC CSI: 0.545 Max Web CSI: 0.382 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>998</td> <td>-</td> <td>-</td> <td>/582</td> <td>/155</td> <td>/254</td> </tr> <tr> <td>I</td> <td>1171</td> <td>-</td> <td>-</td> <td>/734</td> <td>/184</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS A Brg Width = 4.0 Min Req = 1.5 I Brg Width = 4.0 Min Req = 1.5 Bearings A & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)</p> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>232 - 1354</td> <td>D - E</td> <td>274 - 959</td> </tr> <tr> <td>B - C</td> <td>255 - 1277</td> <td>E - F</td> <td>241 - 1249</td> </tr> <tr> <td>C - D</td> <td>279 - 957</td> <td>F - G</td> <td>103 - 986</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	998	-	-	/582	/155	/254	I	1171	-	-	/734	/184	-	Chords	Tens.Comp.	Chords	Tens. Comp.	A - B	232 - 1354	D - E	274 - 959	B - C	255 - 1277	E - F	241 - 1249	C - D	279 - 957	F - G	103 - 986
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Lumber
 Top chord: 2x4 SP #2; T3 2x4 SP M-31;
 Bot chord: 2x4 SP #2; B2 2x4 SP M-31;
 Webs: 2x4 SP #3;
 Lt Slider: 2x8 SP #2; block length = 1.979'

Bracing
 (a) Continuous lateral restraint equally spaced on member. Or 1x4 "T" reinforcement. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" oc.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Right cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

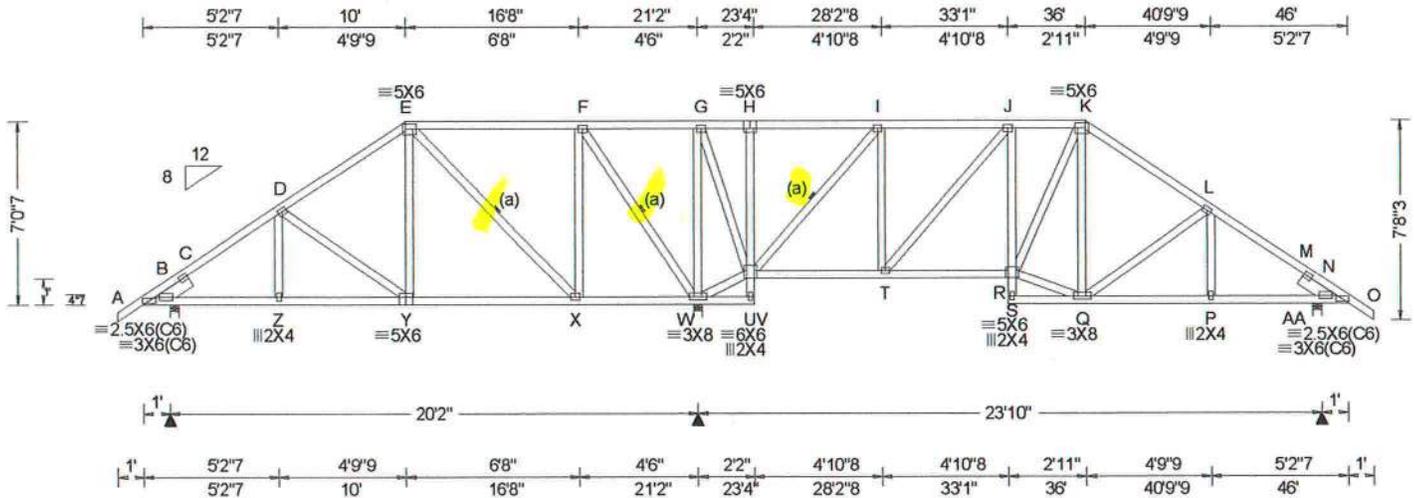
Additional Notes
 The overall height of this truss excluding overhang is 9-0-7.



FL REG# 278, Yoonhwak Kim, FL PE #86367
 02/25/2021

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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 4.60 ft Loc. from endwall: not in 6.50 ft GCpi: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/0(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.049 L 999 240 VERT(CL): 0.109 L 999 240 HORZ(LL): 0.044 M - - HORZ(TL): 0.096 M - - Creep Factor: 2.0 Max TC CSI: 0.789 Max BC CSI: 0.823 Max Web CSI: 0.724 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 785 /- /- /482 /117 /236 W 2594 /- /- /1303 /414 /- AA 894 /- /- /625 /152 /- Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 W Brg Width = 4.0 Min Req = 2.7 AA Brg Width = 4.0 Min Req = 1.5 Bearings B, W, & AA are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.					
				C - D 154 -649 H - I 935 -110 D - E 196 -488 J - K 304 -491 E - F 434 0 K - L 296 -640 F - G 1144 -166 L - M 242 -806 G - H 936 -110					

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.500'
 Rt Slider: 2x6 SP 2400f-2.0E; block length = 1.500'

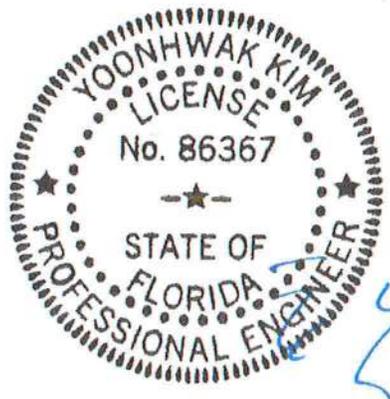
Bracing
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Plating Notes
 All plates are 3X4 except as noted.

Loading
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left and right cantilevers are exposed to wind
 Wind loading based on both gable and hip roof types.

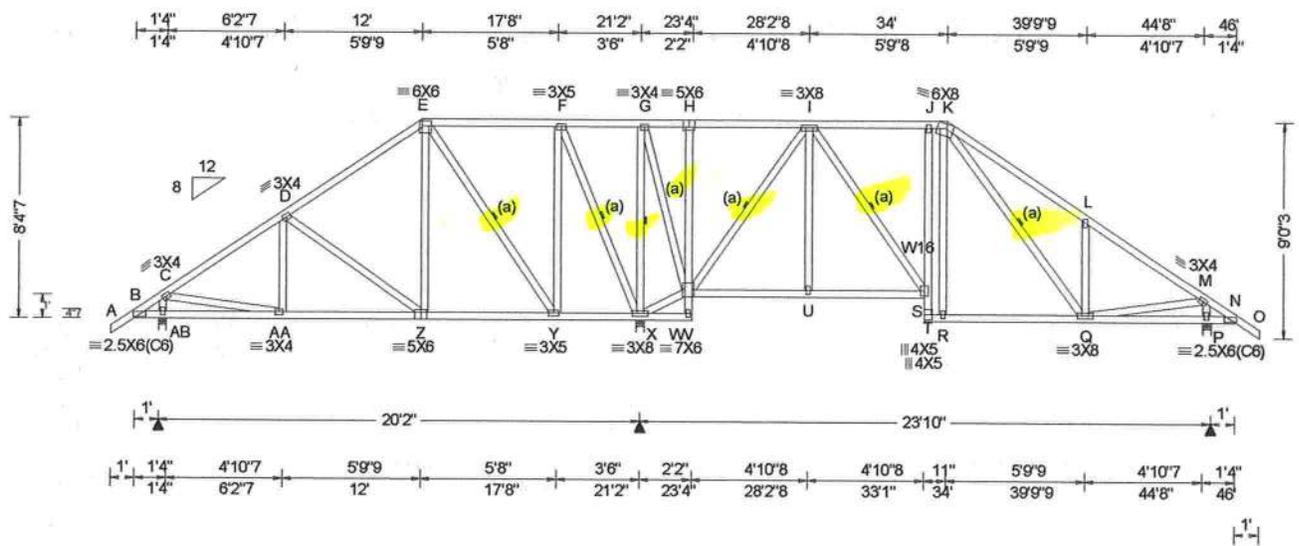
Additional Notes
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.
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Loading Criteria (psf)

TCLL: 20.00
 TCDL: 10.00
 BCLL: 0.00
 BCDL: 10.00
 Des Ld: 40.00
 NCBCLL: 10.00
 Soffit: 0.00
 Load Duration: 1.25
 Spacing: 24.0"

Wind Criteria

Wind Std: ASCE 7-16
 Speed: 130 mph
 Enclosure: Closed
 Risk Category: II
 EXP: C Kzt: NA
 Mean Height: 15.00 ft
 TCDL: 5.0 psf
 BCDL: 5.0 psf
 MWFRS Parallel Dist: h/2 to h
 C&C Dist a: 4.60 ft
 Loc. from endwall: not in 13.00 ft
 GCp1: 0.18
 Wind Duration: 1.25

Snow Criteria (Pg, Pf in PSF)

Pg: NA Ct: NA CAT: NA
 Pf: NA Ce: NA
 Lu: NA Cs: NA
 Snow Duration: NA

Building Code:
 FBC 7th Ed. 2020 Res.
 TPI Std: 2014
 Rep Fac: Yes
 FT/RT: 20(0)/0(0)
 Plate Type(s):
 WAVE

Defl/CSI Criteria

PP Deflection in loc L/defl L/#
 VERT(LL): 0.084 R 999 240
 VERT(CL): 0.163 R 999 240
 HORZ(LL): 0.071 Q - -
 HORZ(TL): 0.136 P - -
 Creep Factor: 2.0
 Max TC CSI: 0.621
 Max BC CSI: 0.472
 Max Web CSI: 0.886

VIEW Ver: 20.01.01A.0724.11

Maximum Reactions (lbs)

Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
AB	764	-	-	/470	/163	/276
X	2928	-	-	/1364	/322	-
P	897	-	-	/663	/190	-

Wind reactions based on MWFRS
 AB Brg Width = 4.0 Min Req = 1.5
 X Brg Width = 4.0 Min Req = 3.1
 P Brg Width = 4.0 Min Req = 1.5
 Bearings AB, X, & P are a rigid surface.
 Members not listed have forces less than 375#
Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
C - D	221 -646	H - I	924 0
E - F	724 0	K - L	436 -906
F - G	1161 -5	L - M	272 -871
G - H	925 0		

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3; W16 2x4 SP M-31;

Bracing
 (a) Continuous lateral restraint equally spaced on member. Or 1x4 "T" reinforcement, 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" oc.

Plating Notes
 All plates are 2X4 except as noted.

Loading
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left and right cantilevers are exposed to wind
 Wind loading based on both gable and hip roof types.

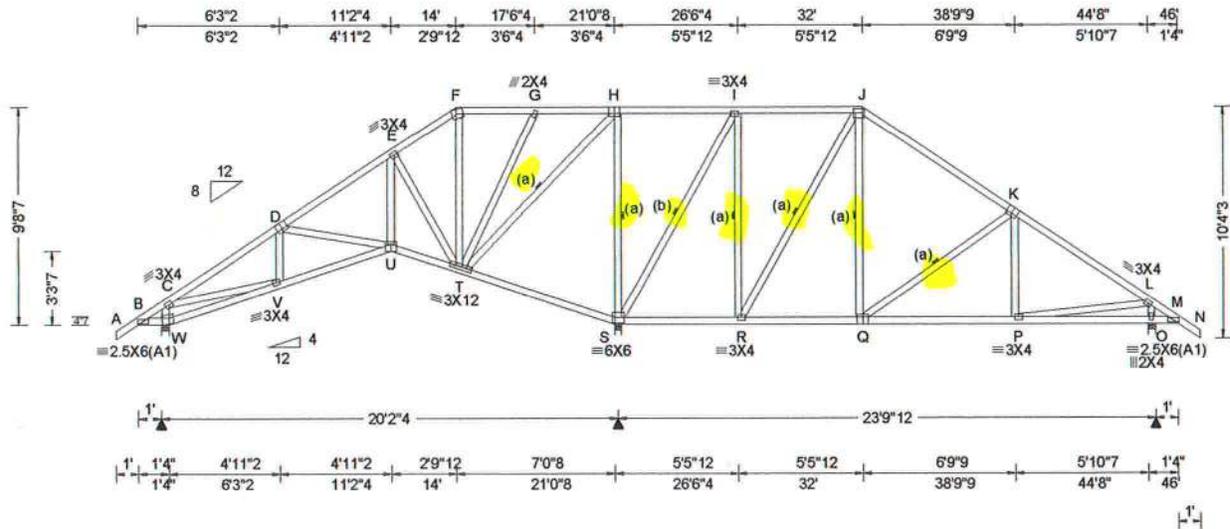
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member. Or 1x4 "T" reinforcement. 80% length of web member. Same species & SRB grade or better, attached with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" oc.
 (b) Continuous lateral restraint equally spaced on member. Or 2x4 "T" reinforcement. 80% length of web member. Same species & grade or better, attached with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

Plating Notes
 All plates are 5X6 except as noted.

Loading
 Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Wind
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 Left and right cantilevers are exposed to wind
 Wind loading based on both gable and hip roof types.

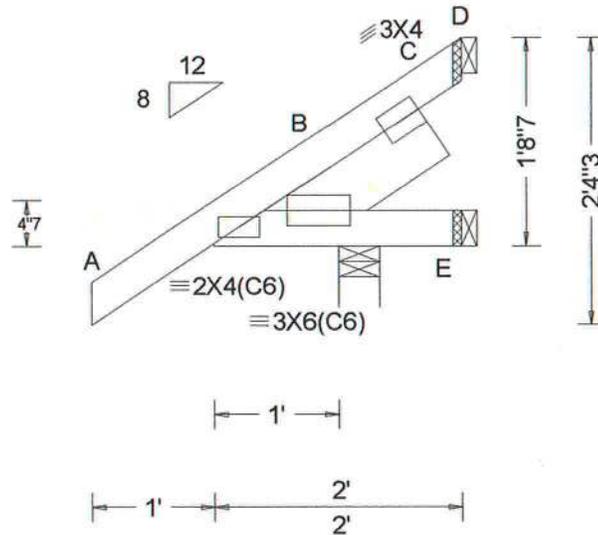
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Loading Criteria (psf) TCELL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/0(0) Plate Type(s): WAVE	Def/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.005 E 999 240 VERT(CL): 0.011 E 999 240 HORZ(LL): -0.004 C - - HORZ(TL): 0.009 C - - Creep Factor: 2.0 Max TC CSI: 0.175 Max BC CSI: 0.145 Max Web CSI: 0.024 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL					
				B 394 /- /- /341 /65 /64 E - /-49 /- /21 /47 /- D - /-130 /- /37 /113 /- Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 E Brg Width = 1.5 Min Req = - D Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#					

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Rt Slider: 2x6 SP 2400f-2.0E; block length = 1.50'

Wind

Wind loads based on MWFRS with additional C&C member design.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes

The overall height of this truss excluding overhang is 1-8-7.

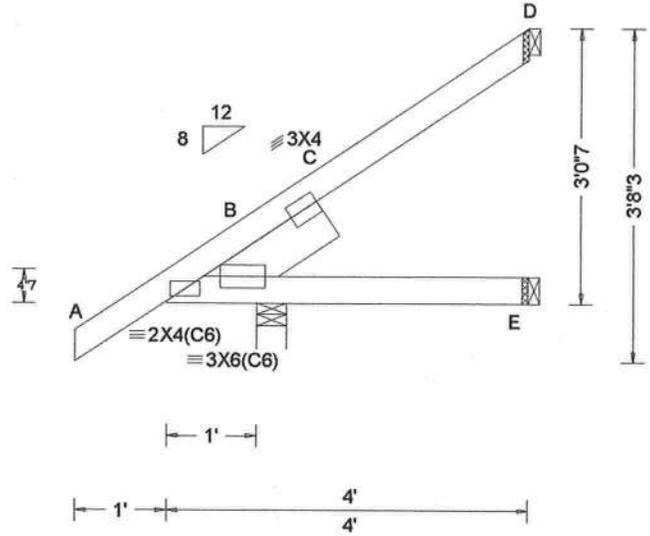


FL REG# 278, Yoonhwak Kim, FL PE #86367
 02/25/2021

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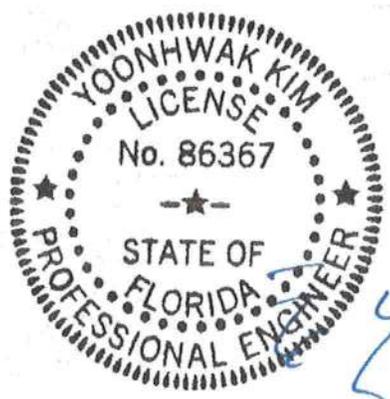


Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/0(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.009 E 999 240 VERT(CL): 0.023 E 613 240 HORZ(LL): -0.009 C - - HORZ(TL): 0.021 C - - Creep Factor: 2.0 Max TC CSI: 0.192 Max BC CSI: 0.176 Max Web CSI: 0.018 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>339</td> <td>/-</td> <td>/-</td> <td>/262</td> <td>/11</td> <td>/112</td> </tr> <tr> <td>E</td> <td>42</td> <td>/-</td> <td>/-</td> <td>/31</td> <td>/3</td> <td>/-</td> </tr> <tr> <td>D</td> <td>69</td> <td>/-</td> <td>/-</td> <td>/38</td> <td>/54</td> <td>/-</td> </tr> </tbody> </table> Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 E Brg Width = 1.5 Min Req = - D Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	339	/-	/-	/262	/11	/112	E	42	/-	/-	/31	/3	/-	D	69	/-	/-	/38	/54	/-
Loc	Gravity			Non-Gravity																																		
	R+	/R-	/Rh	/Rw	/U	/RL																																
B	339	/-	/-	/262	/11	/112																																
E	42	/-	/-	/31	/3	/-																																
D	69	/-	/-	/38	/54	/-																																

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.50'

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

Additional Notes
 The overall height of this truss excluding overhang is 3-0-7.



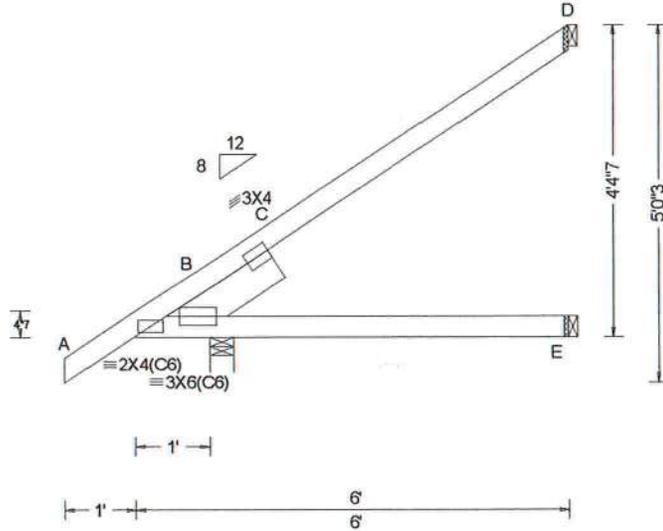
FL REG# 278, Yoonhwak Kim, FL PE #86367
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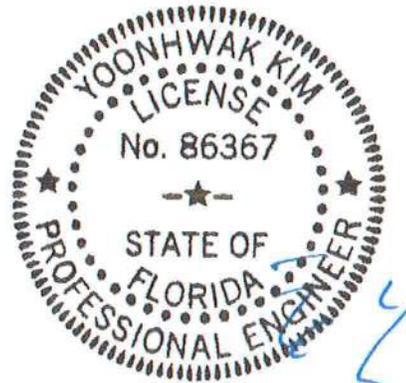


Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.18 Wind Duration: 1.25	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/0(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.032 E 431 240 VERT(CL): -0.048 E 999 240 HORZ(LL): -0.035 C - - HORZ(TL): 0.052 C - - Creep Factor: 2.0 Max TC CSI: 0.540 Max BC CSI: 0.493 Max Web CSI: 0.033 VIEW Ver: 20.01.01A.0724.11	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 399 /- /- /293 /2 /159 E 86 /- /- /52 /- /- D 133 /- /- /92 /88 /- Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 E Brg Width = 1.5 Min Req = - D Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 237 -491 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. B - C 403 -374

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.681'

Wind
Wind loads based on MWFRS with additional C&C member design.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

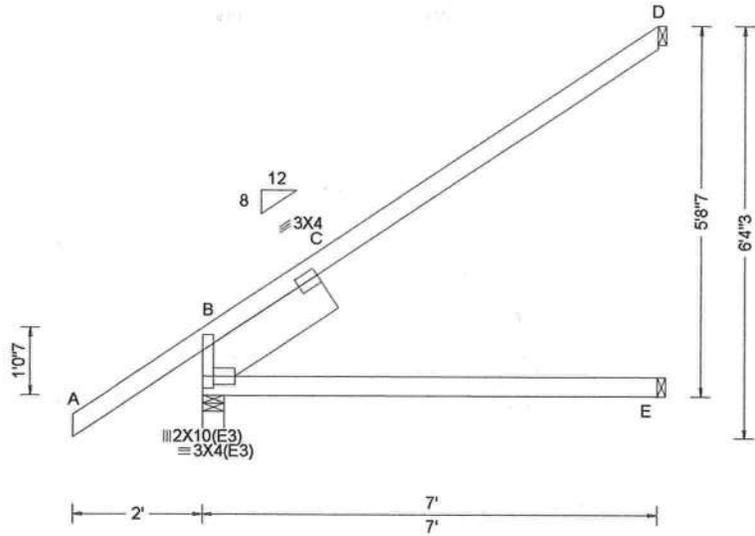
Additional Notes
The overall height of this truss excluding overhang is 4-4-7.



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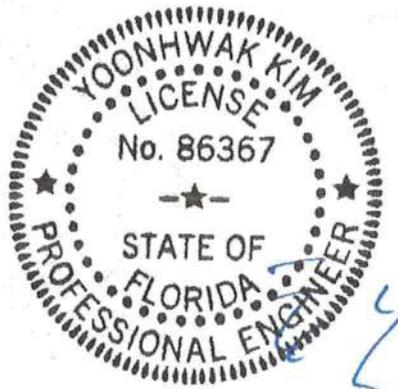


Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.25	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/0(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.097 C - - HORZ(TL): 0.193 C - - Creep Factor: 2.0 Max TC CSI: 0.433 Max BC CSI: 0.543 Max Web CSI: 0.064 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 440 /- /- /310 /- /145 E 132 /- /- /73 /- /- D 200 /- /- /149 /86 /- Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 E Brg Width = 1.5 Min Req = - D Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 279 -474
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Lumber
Top chord: 2x4 SP M-31;
Bot chord: 2x4 SP #2;
Lt Slider: 2x8 SP #2; block length = 2.323'

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes
The overall height of this truss excluding overhang is 5-8-7.



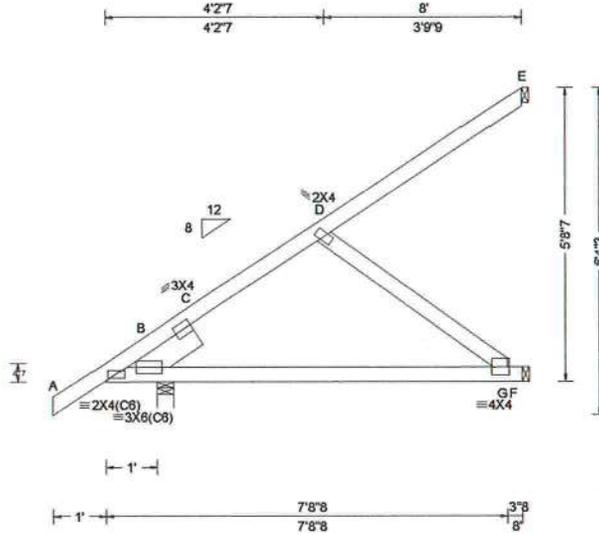
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ALPINE
AN ITW COMPANY
6750 Forum Drive
Suite 305
Orlando FL, 32821



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.25	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/0(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.011 G 999 240 VERT(CL): 0.026 G 999 240 HORZ(LL): -0.010 C - - HORZ(TL): 0.021 C - - Creep Factor: 2.0 Max TC CSI: 0.257 Max BC CSI: 0.451 Max Web CSI: 0.102 VIEW Ver: 20.01.01A.0724.11	Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 473 /- /- /337 /- /206 F 170 /- /0 /135 /54 /0 E 112 /- /- /78 /61 /- Wind reactions based on MWFRS B Brg Width = 4.0 Min Req = 1.5 F Brg Width = 1.5 Min Req = - E Brg Width = 1.5 Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.500'

Wind
Wind loads based on MWFRS with additional C&C member design.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

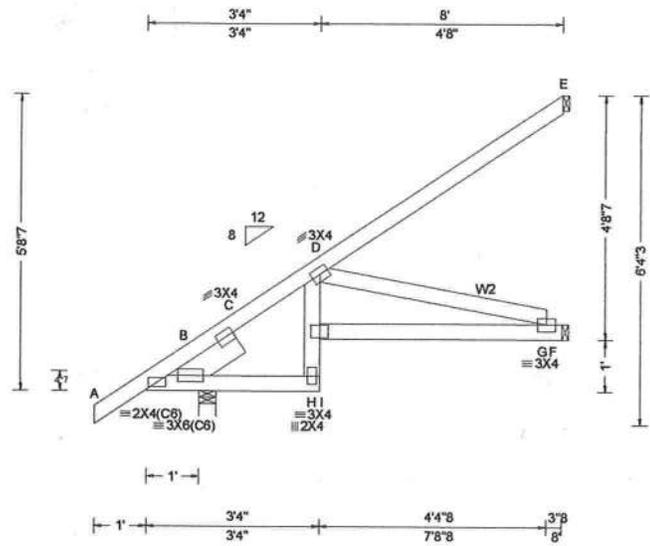
Additional Notes
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Loc	Gravity			Non-Gravity																																								
	R+	/R-	/Rh	/Rw	/U	/RL																																						
B	473	/-	/-	/337	/-	/149																																						
F	148	/-	/0	/111	/15	/0																																						
E	141	/-	/-	/103	/51	/-																																						
H - G	345	-430																																										
D - G	445	-358																																										

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #2; W2 2x4 SP #3;
 Lt Slider: 2x6 SP 2400F-2.0E; block length = 1.500'

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

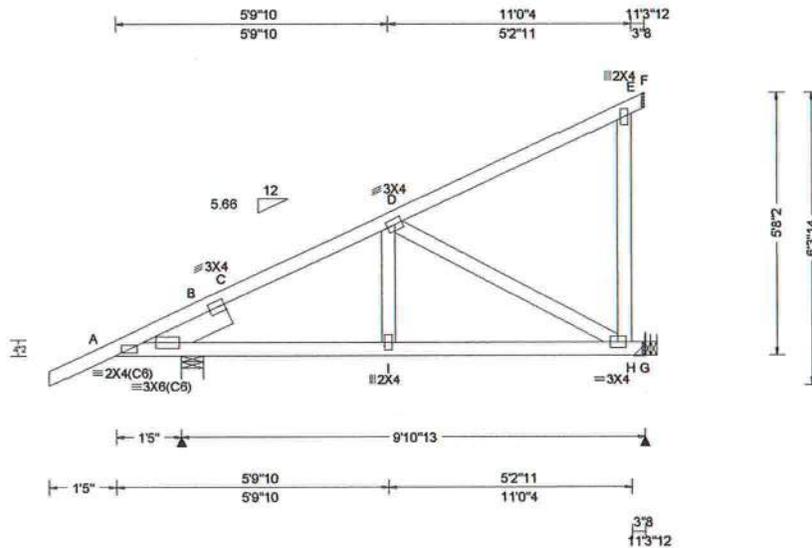
Additional Notes
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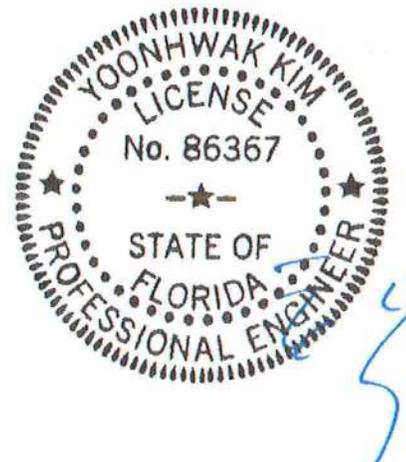


Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCcpi: 0.18 Wind Duration: 1.25	Snow Criteria (Pg, Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/0(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.021 I 961 240 VERT(CL): 0.067 I 296 240 HORZ(LL): -0.014 C - - HORZ(TL): 0.046 C - - Creep Factor: 2.0 Max TC CSI: 0.368 Max BC CSI: 0.444 Max Web CSI: 0.176 VIEW Ver: 20.01.01A.0724.11	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>450</td> <td>/-</td> <td>/-</td> <td>/-</td> <td>/8</td> <td>/-</td> </tr> <tr> <td>G</td> <td>284</td> <td>/-</td> <td>/-</td> <td>/13</td> <td>/-</td> <td>/-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Width = 5.7 Min Req = 1.5 G Brg Width = - Min Req = - Bearing B is a rigid surface. Members not listed have forces less than 375#</p>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	450	/-	/-	/-	/8	/-	G	284	/-	/-	/13	/-	/-
Loc	Gravity			Non-Gravity																											
	R+	/R-	/Rh	/Rw	/U	/RL																									
B	450	/-	/-	/-	/8	/-																									
G	284	/-	/-	/13	/-	/-																									

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Lt Slider: 2x6 SP 2400f-2.0E; block length = 1.945'

Wind
 Wind loads and reactions based on MWFRS.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.

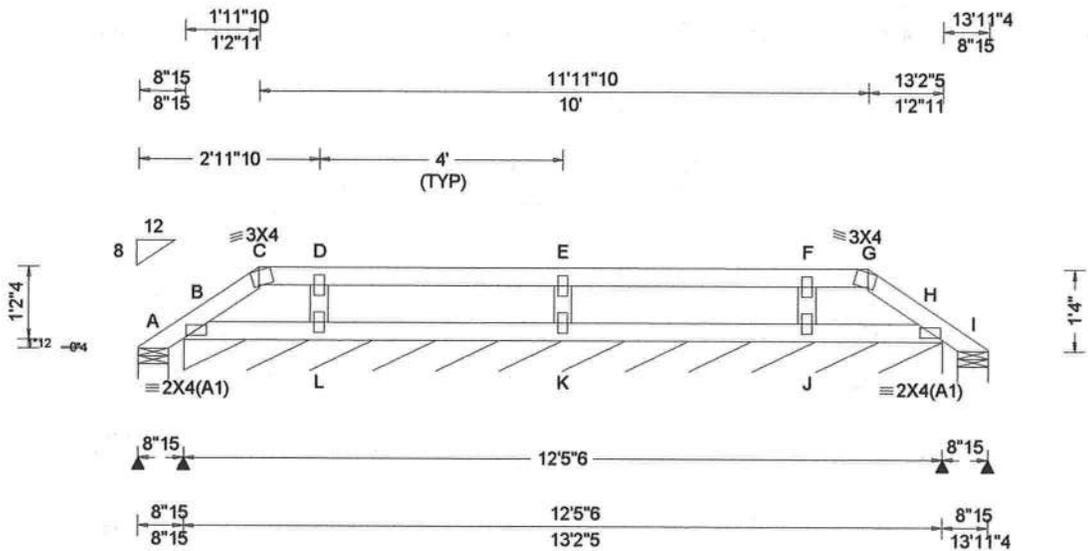
Additional Notes
 The overall height of this truss excluding overhang is 5-8-2.



FL REG# 278, Yoonhwak Kim, FL PE #86367
 02/25/2021

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=-PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 19.80 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.25	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/0(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): -0.001 C 999 240 VERT(CL): -0.002 C 999 240 HORZ(LL): 0.000 J - - HORZ(TL): 0.001 C - - Creep Factor: 2.0 Max TC CSI: 0.340 Max BC CSI: 0.150 Max Web CSI: 0.148 VIEW Ver: 20.01.01A.0724.11	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 18 /- /- /25 /21 /33 B* 87 /- /- /45 /18 /- I 18 /- /- /7 /8 /- L /-258 K /-344 J /-257 Wind reactions based on MWFRS A Brg Width = 5.9 Min Req = 1.5 B Brg Width = 149 Min Req = - I Brg Width = 5.9 Min Req = 1.5 Bearings A, B, & I are a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. E - K 388 -275

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes
All plates are 2X4 except as noted.

Purlins
In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

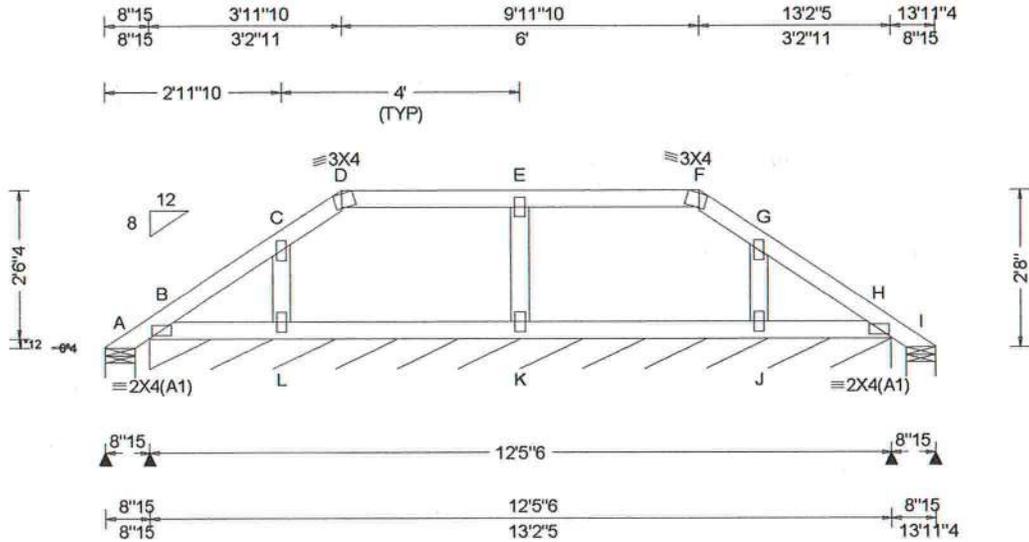
Additional Notes
Refer to DWG PB160160118 for piggyback details.
The overall height of this truss excluding overhang is 1-4-0.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 20.47 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.25	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/0(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.004 D 999 240 VERT(CL): 0.012 D 999 240 HORZ(LL): 0.002 D - - HORZ(TL): 0.007 D - - Creep Factor: 2.0 Max TC CSI: 0.238 Max BC CSI: 0.160 Max Web CSI: 0.123 VIEW Ver: 20.01.01A.0724.11	Gravity Loc R+ / R- / Rh / Rw / U / RL A 21 /- /- /45 /38 /72 B* 86 /- /- /48 /12 /- I 21 /- /- /7 /14 /- B /-134 K /-280 H /-122 Non-Gravity Wind reactions based on MWFRS A Brg Width = 5.9 Min Req = 1.5 B Brg Width = 149 Min Req = - I Brg Width = 5.9 Min Req = 1.5 Bearings A, B, & I are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Purlins

In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

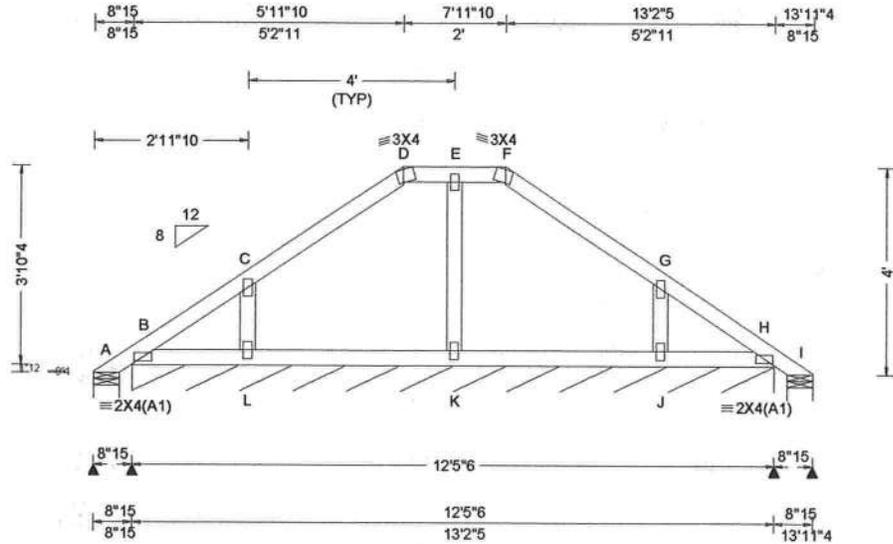
Refer to DWG PB160160118 for piggyback details.
The overall height of this truss excluding overhang is 2-8-0.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0"	Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 21.13 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.25	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/0(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.005 D 999 240 VERT(CL): 0.016 D 999 240 HORZ(LL): 0.004 F - - HORZ(TL): 0.009 D - - Creep Factor: 2.0 Max TC CSI: 0.168 Max BC CSI: 0.160 Max Web CSI: 0.067 VIEW Ver: 20.01.01A.0724.11	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 26 /- /- /65 /56 /111 B* 85 /- /- /50 /6 /- I 26 /- /- /13 /4 /- L /-140 J /-140 Wind reactions based on MWFRS A Brg Width = 5.9 Min Req = 1.5 B Brg Width = 149 Min Req = - I Brg Width = 5.9 Min Req = 1.5 Bearings A, B, & I are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Purlins

In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

Wind

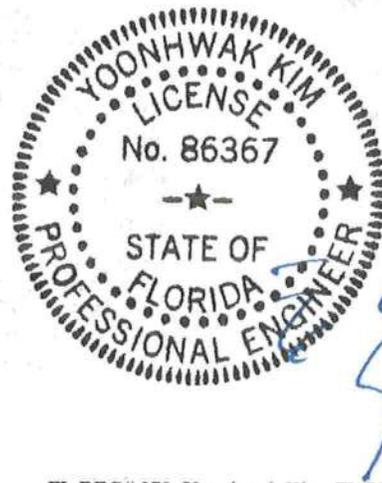
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

Refer to DWG PB160160118 for piggyback details.

The overall height of this truss excluding overhang is 4-0-0.



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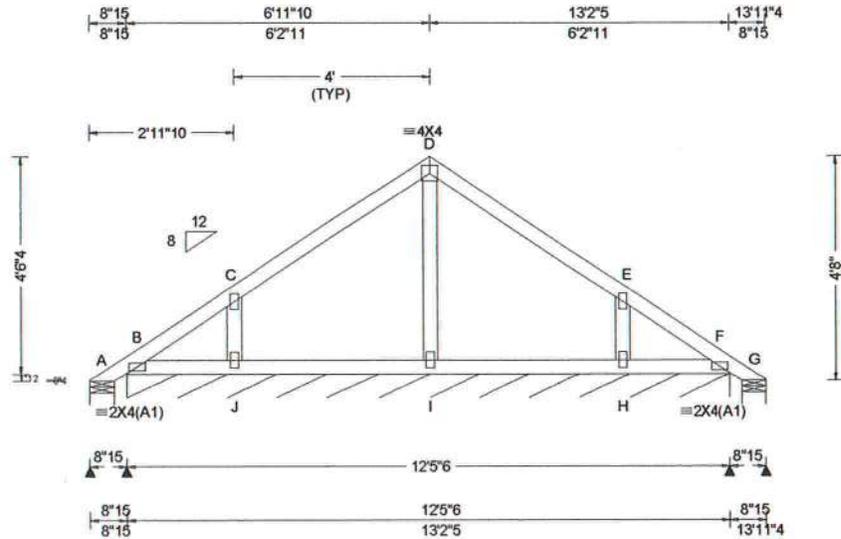
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Loc	Gravity			Non-Gravity																																																
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A	29	/-	/-	/79	/69	/130																																														
B*	85	/-	/-	/51	/5	/-																																														
G	29	/-	/-	/16	/8	/-																																														
J			/-189																																																	
H			/-189																																																	

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Purlins

In lieu of rigid ceiling use purlins to brace BC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

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The overall height of this truss excluding overhang is 4-8-0.



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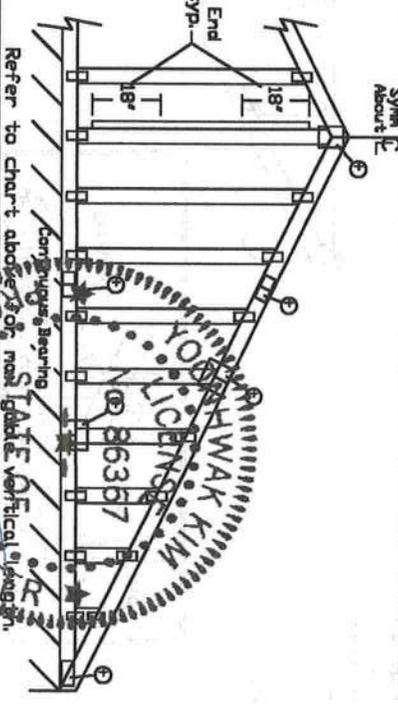
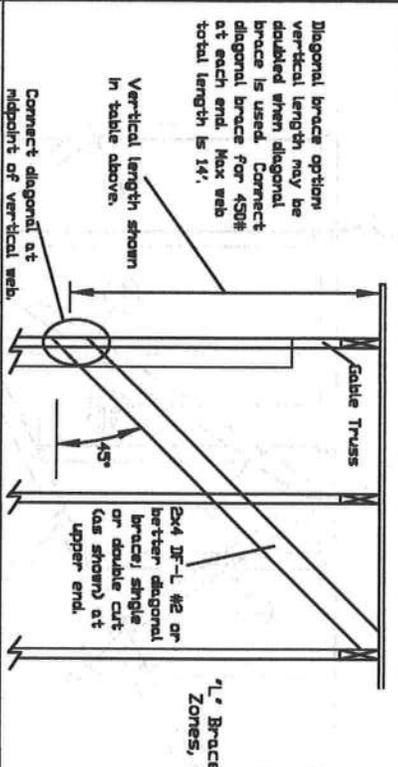
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Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dm, 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
 Dm, 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
 Dm, 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Gable Vertical Spacing	Brace Species	Grade	No Braces	12" o.c.		16" o.c.		24" o.c.					
				Group A	Group B	Group A	Group B	Group A	Group B				
24"	SPF	#1 / #2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
		#3	4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		Stud	4' 1"	6' 7"	7' 0"	8' 5"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
24"	HF	#1	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		#2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
24"	DFL	Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 2"	11' 0"	11' 10"	14' 0"	14' 0"
		#1 / #2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
16"	SPF	Standard	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#1	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
16"	DFL	Standard	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"
		#1 / #2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
12"	SPF	Standard	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#1	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
12"	DFL	Standard	5' 3"	8' 5"	9' 0"	10' 9"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"
		#1	5' 3"	8' 5"	9' 0"	10' 9"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	8' 5"	9' 0"	10' 9"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"



Gable Vertical Plate Sizes	
Vertical Length	No Sillae
Less than 4' 0"	1x4 or 2x3
Greater than 4' 0"	3x4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

Bracing Group Species and Grades	
Group A1 Spruce-Pine-Fir #1 / #2 Standard #3 Stud	Hem-Fir #2 Stud #3 Standard
Douglas Fir-Larch #3 Stud Standard	Southern Pine #3 Stud Standard
Group B1 Hem-Fir #1 & 2-Stud #1	Southern Pine #1 #2

2x4 Braces shall be SRB (Stress-Rated Board), #1 or #2. No. 1 Pine use only Industrial SS or Industrial 4S (Stress-Rated Boards). Group B values may be used with these grades.

Gable Truss Detail Notes:
 Wind Load deflection criterion is L/240.
 Provide uplift connections for SS p/lf over continuous bearing G p/lf TC Dead Load.
 Gable end supports load from 4' 0" out-tookers with 2' 0" overhang, or 12" plywood overhang.

514 Earth City Expressway
Suite 242
Earth City, MO 63045

Transmitting engineer name in building, handling, shipping, handling and bracing. Refer to and follow the latest edition of ASCE 7-16. Gable Truss Design shall be in accordance with the provisions prior to performing these functions. Installers shall provide temporary bracing per ASCE 7-16. Unless noted otherwise, top chord shall have properly structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of ends of members shall be in accordance with ASCE 7-16. Locations shown for permanent lateral restraint of ends of members shall be in accordance with ASCE 7-16. Locations shown for permanent lateral restraint of ends of members shall be in accordance with ASCE 7-16. Locations shown for permanent lateral restraint of ends of members shall be in accordance with ASCE 7-16.

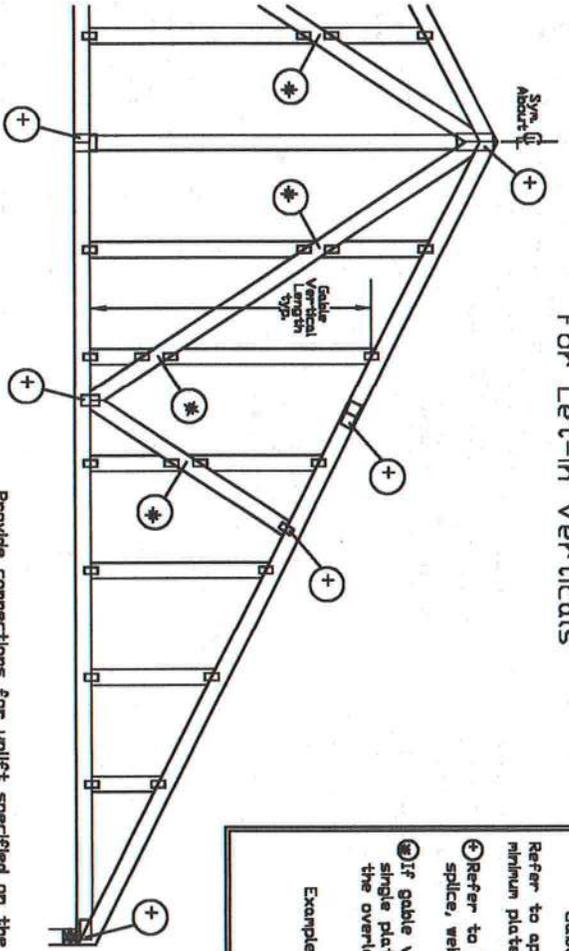
Alpine, a division of TWS Building Components Group Inc. shall not be responsible for any deviation from the design shown. The suitability and use of this drawing is the responsibility of the professional engineering firm. For more information see the job's general notes page and these web sites: www.alpine.com and www.tws.com.
 ALPINE 25/202#278, Yoonhwak Kim, FL PE #86367

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0'

REF	ASCE7-16-GAB14015
DATE	01/26/2018
DRWG	A14015ENC160118

Gable Detail For Let-in Verticals



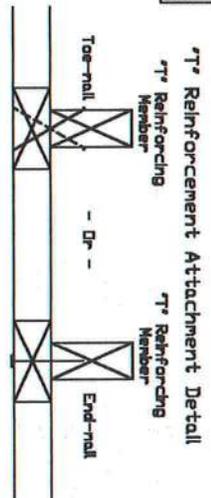
Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊕ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

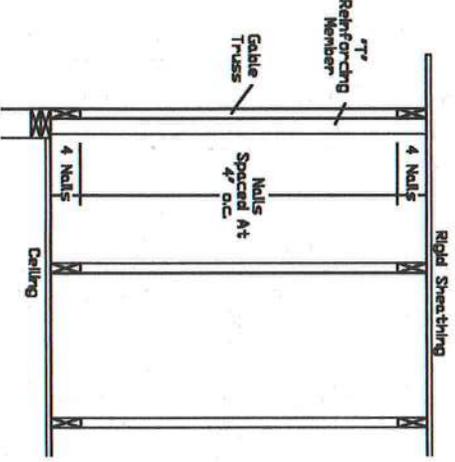
Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Min. Size	"T" Increase %
2x4	30 %
2x6	20 %

Example:
 ASCE 7-10 Wind Speed = 120 mph
 Mean Roof Height = 30 ft, Kzt = 1.00
 Gable Vertical = 24' o.c. SP #3
 "T" Reinforcing Member Size = 2x4
 "T" Brace Increase (from Above) = 30% = 1.30
 (1) 2x4 "L" Brace Length = 8' 7"
 Maximum "T" Reinforced Gable Vertical Length = 130 x 8' 7" = 11' 2"



This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.



WARNING: READ AND FOLLOW ALL NOTES ON THIS DRAWING.

IMPORTANT: FABRICATE THIS DRAWING TO ALL CONNECTIONS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Before to and after erection, the truss shall be inspected by the fabricator and the installer for safety. Failure to follow these instructions may result in injury or death. The fabricator and the installer shall be responsible for performing these duties. Trusses shall provide temporary bracing per ASCE. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per ASCE sections 10.2.7 or 10.2.8, as applicable. Apply plates to each face of truss and position as shown above and on the "Light Details", unless noted otherwise. Refer to drawings 104H-7 for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in accordance with ASCE 10.2.7, 10.2.8 or for handling, shipping, or erection of the truss. The fabricator and the installer shall be responsible for the design, engineering, responsibility and use of the drawing. The fabricator and the installer shall be responsible for any structure in the responsibility of the building designer per ASCE 10.2.7, 10.2.8.

For more information see the job's general notes page and these web sites: www.alpine.com, www.itw.com, www.asce.org

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Suite 242
Earth City, MO 63045



REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118
MAX. TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX. SPACING	24.0"

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kz-t=1.00

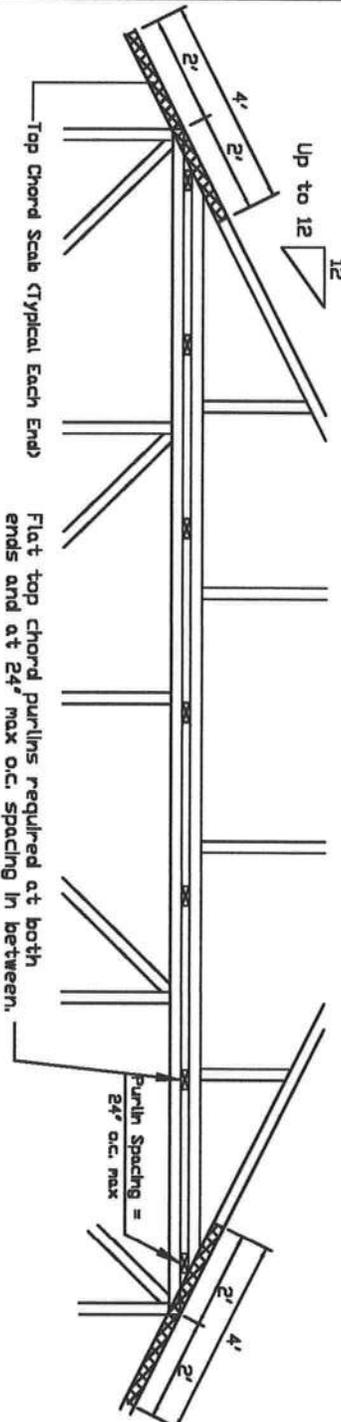
160 mph Wind, 3000 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp C, Wind DL = 5.0 psf (min), Kz-t=1.0.
 Dr 140 mph wind, 3000 ft Mean Hgt, ASCE 7-16, Enclosed Bldg, located anywhere in roof, Exp D, Wind DL = 5.0 psf (min), Kz-t=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

See Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" O.C. OR LESS

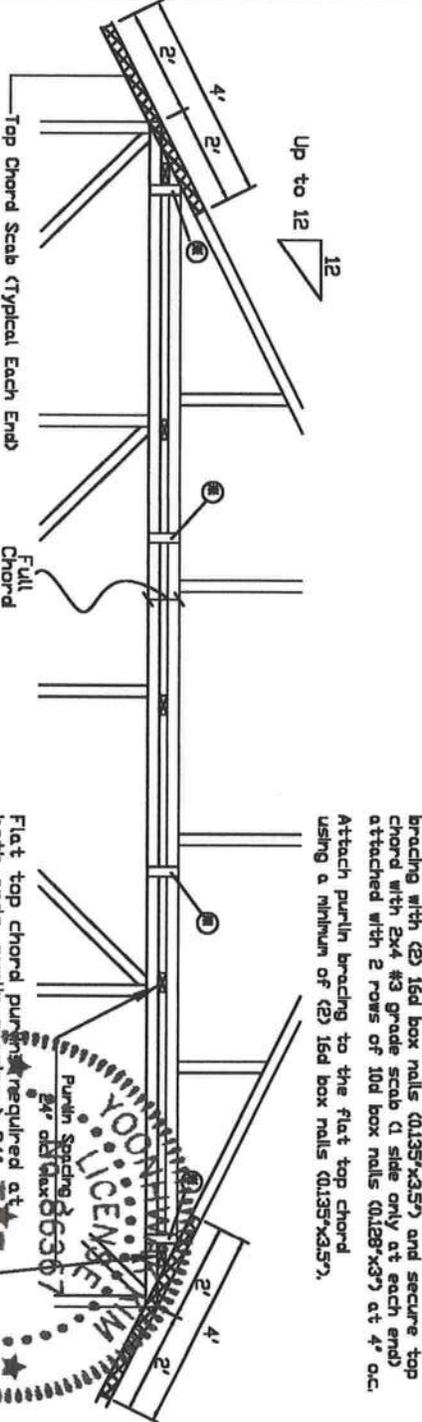


Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3x8 Trulox plate attached with (3) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (2) 2BPB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" O.C.



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

Flat top chord purlin bracing required at both ends, purlin spacing > 24' o.c.



<p>■ In addition, provide connection with one of the following methods:</p> <p>Trulox: Use 3x8 Trulox plates for 2x4 chord member, and 3x10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.</p> <p>APA Rated Gussset: 8"x8"x7/16" (min) APA rated sheathing gusssets (each face). Attach @ 8' o.c. with (8) 5d common (0.113"x2") nails per gussset, (4) in cap bottom chord and (4) in base truss top chord. Gusssets may be staggered 4' o.c. front to back faces.</p> <p>2x4 Vertical Scabs: 2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.</p> <p>2BPB Wave Piggyback Plate: One 2BPB wave piggyback plate to each face @ 8' o.c. Attach with (4) piggyback truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.</p>	
REF	PIGGYBACK
DATE	01/02/2018
DRWG	PB160160118

13723 Riverport Drive
 Suite 200
 Maryland Heights, MO 63043

Trusses require extreme care in fabricating, handling, shipping, handling and bracing. Refer to and follow the latest edition of ASCE Guiding Concrete Safety Information by ITW and ASCE for safety practices prior to performing these functions. Installers shall provide temporary bracing per ASCE. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of web shall have bracing installed per ASCE sections 22.37 or 22.40, as applicable. Apply plates to each face of all bracing members. The intent is to provide lateral restraint, unless noted otherwise.

Refer to drawings 160A-2 for standard plate specifications.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ASCE/ITW 1 or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing the drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ASCE/ITW 1. See 2.

For more information see the Job's general notes page and those with drawings.

ALPINE engineers.com | 1795 s.w. 10th ave. | Suite 200 | Ft. Lauderdale, FL 33309 | Phone: 954.279.4000 | Fax: 954.279.4001

SPACING	24.0'
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