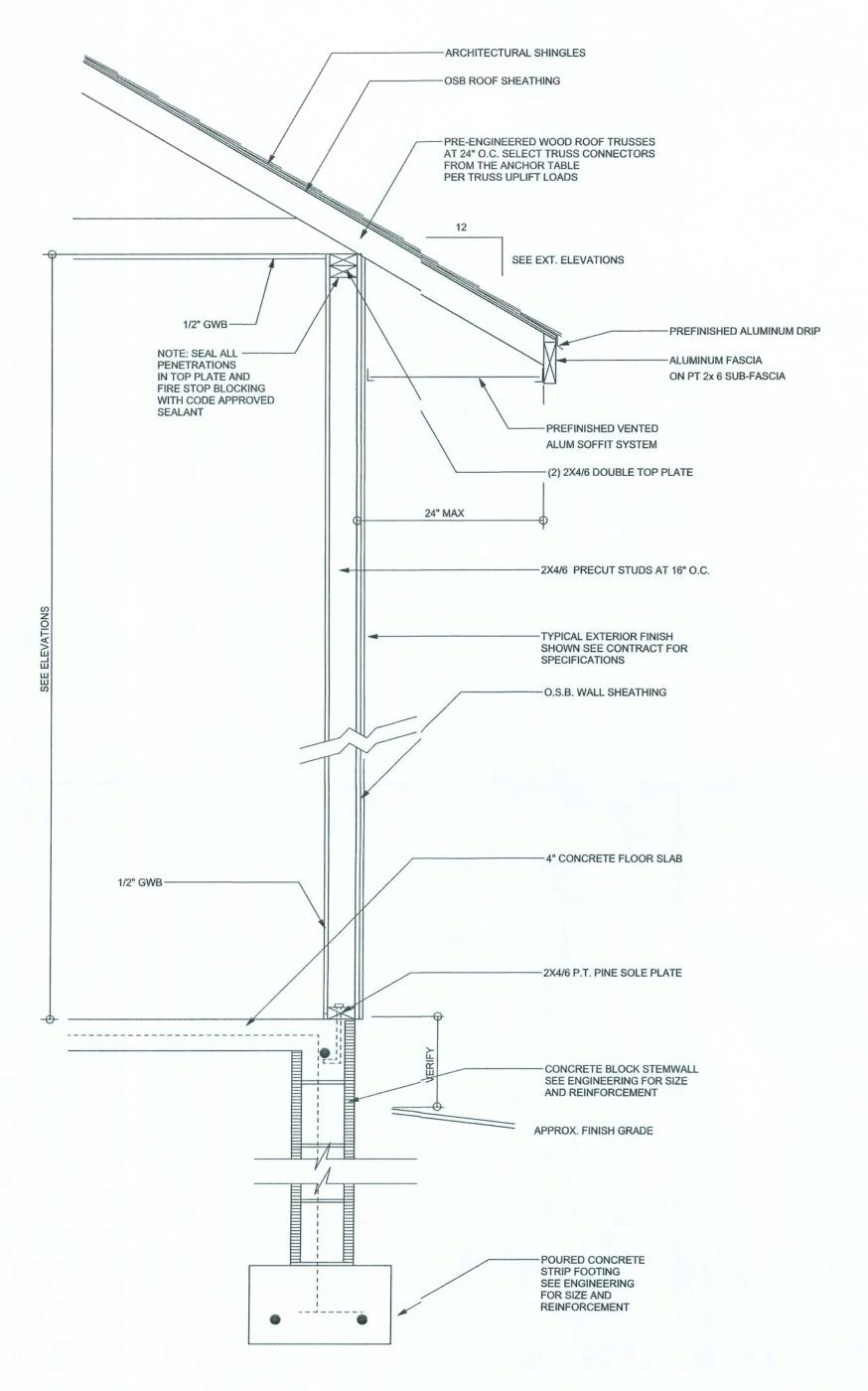
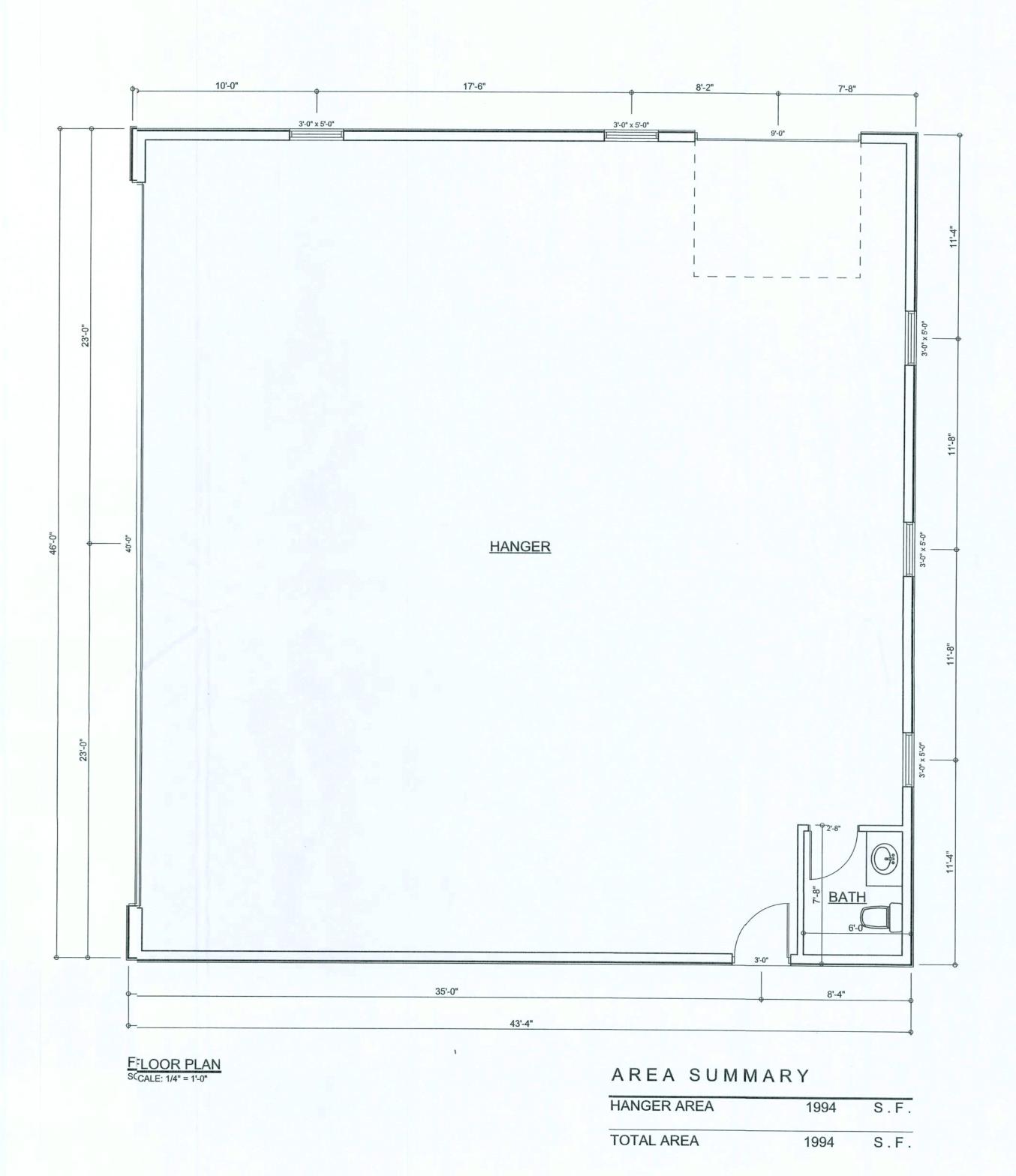


**REVISIONS** 

SOFTPIAN ARCHITECTULAL DESIGN SAFTWARE



TYPICAL DESIGN WALI SECTION NON - STRUCTURAL DATA



WINDLOAD ENGINE:R: Mark Disosway, PE No.53915, POB &8, Lake City, FL 32056, 386-754-541\$

DIMENSIONS:
Stated dimensions swercede scaled dimensions. Refer allquestions to Mark Disosway, P.E. or resolution.
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permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, plating to wind engineering comply with section 1509, florida building code 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified ocation.

MARK DISOSWAY P.E. 53915

### **Edgley Construction**

Arthur Hutchison & Ann Bormolini Hanger

ADDRESS: Lot 3 Southern Landings S/D Columbia County, Florida

Mark Disosway P.E. P.O. 3ox 868 Lake City, Florida 32056 Phone: (3&6) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: January 06, 2006

DRAWN BY: CHECKED BY: David Disosway

FINALS DATE: 6 / Jan / 06

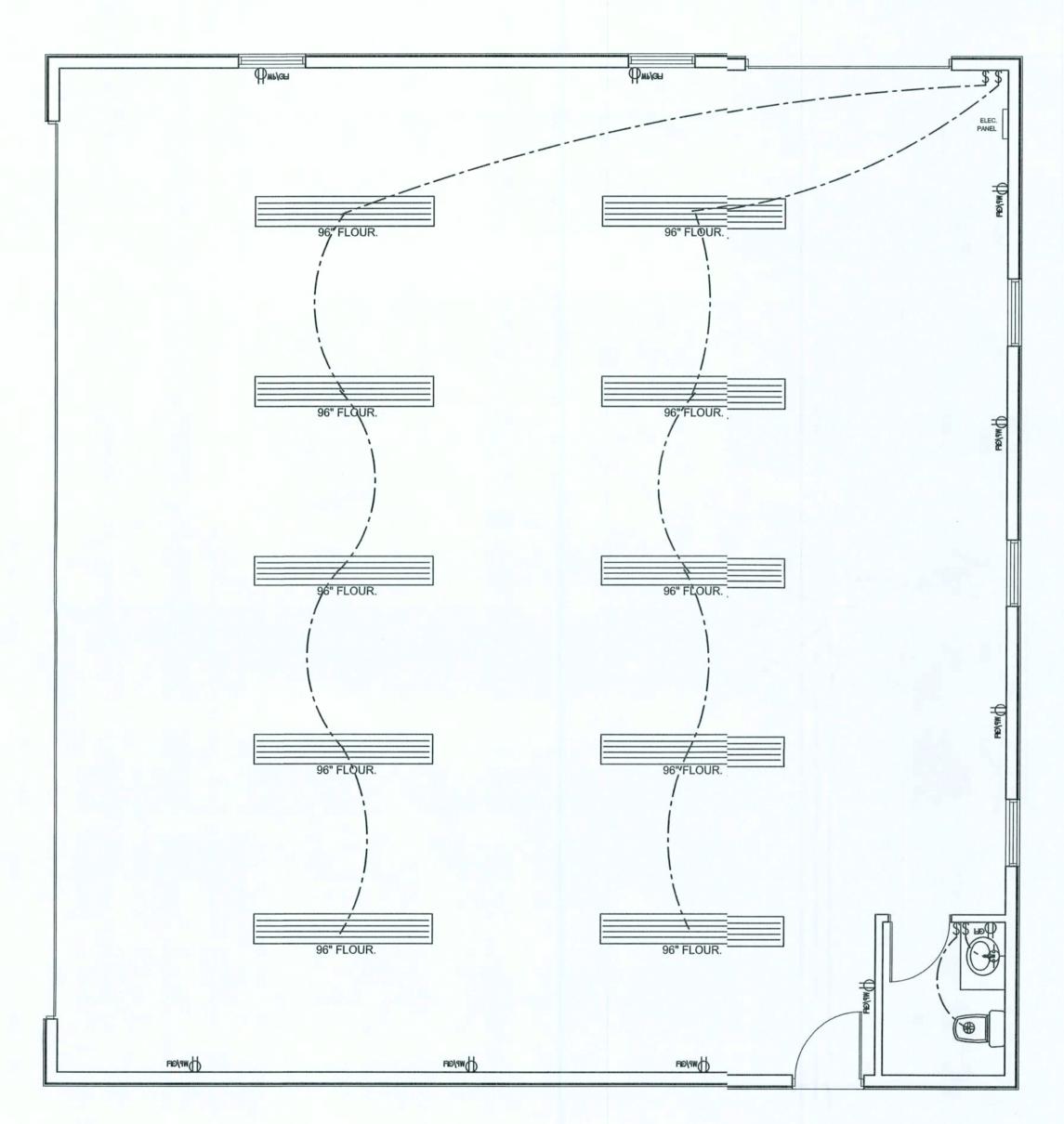
JOB NUMBER: 510191a

DRAWING NUMBER **A-2** 

OF 63HEETS

REVISIONS

SOFTPIAN ARCHIECTURAL DESIGN SOFTWARE



ELECTRICAL PLAN SCALE: 1/4" = 1'-0"

### ELECTRICAL PLAN NOTES

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4

  ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY
  BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL
  BE INTERLOCKED TOGETHER. INSTALL INSIDE AND
  NEAR ALL BEDROOMS.
- E -5
  TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE
  DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S
  DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE
  SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD)
  TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION

	ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)	
QP	DOUBLE SECURITY LIGHT	
	2X4 FLUORESCENT LIGHT FIXTURE	
0	RECESSED CAN LIGHT	
-♦-	BATH EXAUST FAN WITH LIGHT	
₩	BATH EXAUST FAN	
- <b>\( -</b>	LIGHT FIXTURE	
Ф	DUPLEX OUTLET	
₩	220v OUTLET	
Фан	GFI DUPLEX OUTLET	
•	SMOKE DETECTOR	
\$	WALL SWITCH	
\$3	3 WAY WALL SWITCH	
\$	4 WAY WALL SWITCH	
₩ <sub>P/GFI</sub>	WATER PROOF GFI OUTLET	
$\nabla$	PHONE JACK	
0	TELEVISION JACK	
更	GARAGE DOOR OPENER	
	WALL HEATER	

WINDLOAD ENSINEER: Mark Disosway, PE No.53915, PDB 868, Lake City, FL 32056, 386-7545419

DIMENSIONS:

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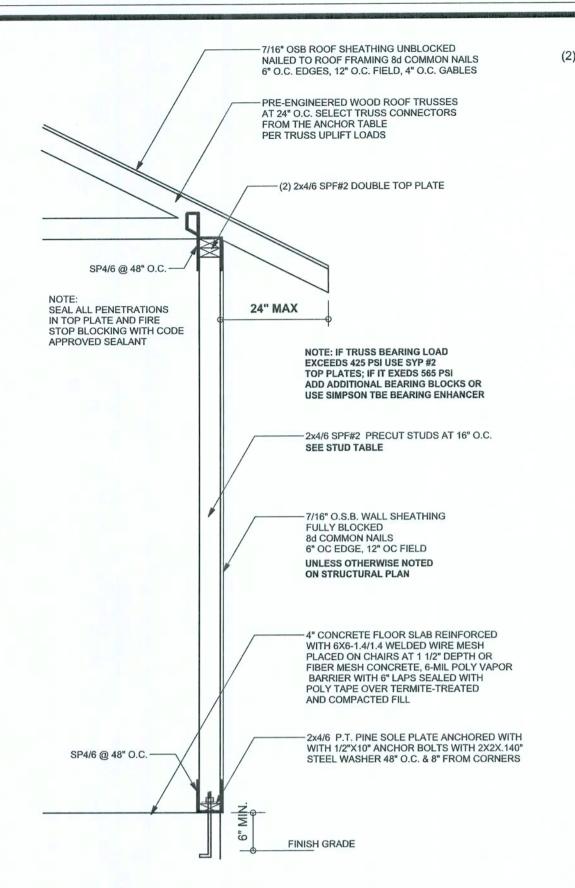
FINALS DATE:

JOE NUMBER: 510191a

DRAVING NUMBER

A-3

CF 6 SHEETS

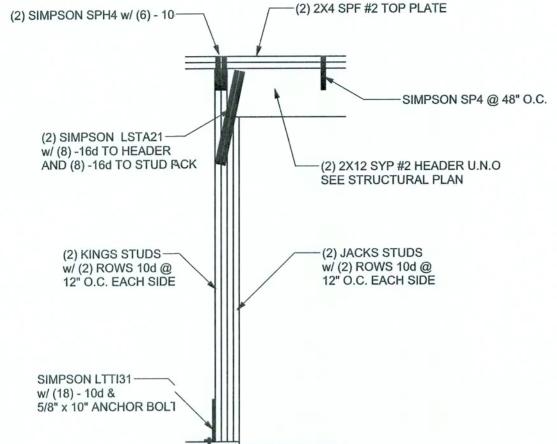


## EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

ONE STORY WALL SECTION

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WECM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.



TYPICAL GARAGE DOOR HEADER STRAPING DETAIL

2 X 4 CONT. LITERAL -

W/ 2-10d NAIL EA. TRUSS

5 COOLER NAILS AT 7 IN. O.C.-

DIAPHRAGM AS SECIFIED IN THE "WOOD FRAME CONSTRUCTION

TRUSS BOTTOM CHORD.

ALTERNATIVE TO:ALLOON FRAMING IS GYPSUM CEILING

MANUAL " (WECM) SEE WECM TABLE 3.13 AND FIGURE

TO PRESERVE STENGTH OF DIAPHRAGM.

-(4)-2x43PF #2 NAILED

TOGEHER W/2-16d

BEAM MID-WALL CONNECTION DETAIL

MIN. (SE STRUCTURAL PLAN)

—() 2X12 SYP #2 MIN. —— SE STRUCTURAL PLAN

3 6a SHONE ABO'E, PROTECT GYPSUM FROM MOISTURE

BRACING AT 'O.C.

1/2 GYP, BOARD Wbd-

(2) 2X12 SYP #2 MIN. -SEE STRUCTURAL PLAN

SIMPSON HUS412 MIN. -

SEE STRUCTURAL PLAN

SCALE: N.T.S.

LSTA24

NAIL THRU 2x4 INT BEAM W/4-16d

BEAM MAY BE ATTACHED IN

BEAM CORNER CONNECTION. DETAIL

-SIMPSON HUS412 MIN.

SCALE: N.T.S.

SEE STRUCTURAL PLAN

COOLER NAILS AT 3 IN. O.C.

-FOUNDATION SEE

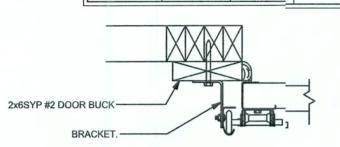
SEE FOOTING DETAILS

#### 2x6 SYP #2 GARAGE DOOR BUCK AATTACHMENT ATTACH GARAGE DOOR BUCK TO STUD PAGCK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4"1" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY I BE COUNTERSUNK. HORIZONTAL JAMBS DO NGOT TRANSFER LOAD. CENTER LAG SCREWS OFR

STAGGER 16d NAILS OR (2) ROWS OF .131 x<sub>x 3 1/4</sub>"

GN PER TABLE BELOW:

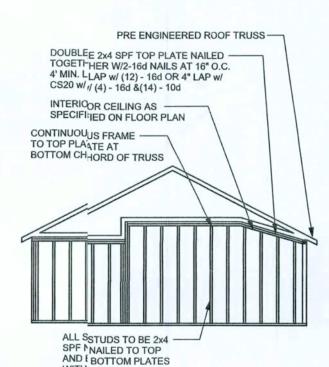
DOOR WIDTH	3/8" x 4" LAG	16d STAGGER (	(2) ROWS OF .131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



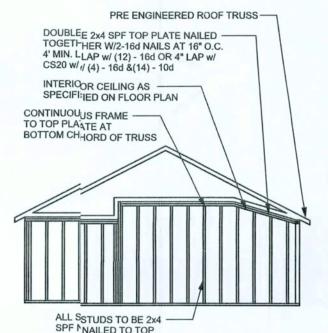
GARAGE DOOR BUCK INSTALLLATION DETAIL

#### **GRADE & SPECIES TABLE**

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



CONTINUOUS FRAME TO **CEIL ING DIAPHRAGM DETAIL** 



SCALE: 1 N.T.S.

-NAIL SHEATHING TO HEADER AND TOP

#### **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBC 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB:  $6" \times 6" \times 1.4 \times 1$ MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

	R AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH AR Y NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND IT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	RIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC 2004 FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE THE P	TINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU AN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL ENGINEER IMMEDIATELY.
	USS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS MENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS.

**ROOF SYSTEM DESIGN** 

BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBC 2004, SECTION 1606 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ANCHOR TABLE

MANUFACTURER'S ENGINEERING

< 420

< 455

< 360

< 455

< 415

< 600

< 950

< 745

< 1465

< 1465

< 990

< 1470

< 1470

< 1000

< 1450

< 2900

< 2050

< 3965

< 10980

< 10530

< 9250

< 435

< 455

< 825

< 825

< 885

< 1240

< 885

< 1240

< 1235

< 1235

< 1030

< 1705

< 1350

< 2310

< 2775

< 4175

< 1400

< 3335

< 2200

< 2300

< 2320

UPLIFT LBS. SYP UPLIFT LBS. SPF

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

< 245

< 265

< 235

< 320

< 365

< 535

< 820

< 565

< 1050

< 1050

< 850

< 655

< 1265

< 1265

< 860

< 1245

< 2490

< 1785

< 3330

< 6485

< 9035

< 9250

< 435

< 420

< 825

< 600

< 760

< 1065

< 760

< 1065

< 1165

< 1235

< 1030

< 1705

< 1305

< 2310

< 2570

< 3695

< 1400

< 3335

< 2200

< 2300

< 2320

**DESIGN DATA** 

TRUSS CONNECTOR\*

H5A

H2.5

H2.5A

H6

H14-1

H14-2

H10-1

H10-2

H16-1

H16-2

MTS24C

2 - HTS24

LGT2

EAVY GIRDER TIEDOWNS\*

HGT-3

HGT-4

STUD STRAP CONNECTOR

SSP SINGLE SILL PLATE

DSP DOUBLE TOP PLATE

DSP SINGLE SILL PLATE

SPH4

SPH6

LSTA18

LSTA21

CS20

CS16

STUD ANCHORS\*

LTTI31

HD2A

HTT16

PAHD42

HPAHD22

ABU44

ABU88

SSP DOUBLE TOP PLATE 3 -10d

TO PLATES TO RAFTER/TRUSS

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2"

12-8d, 1 1/2"

12-8d, 1 1/2"

8-8d, 1 1/2"

6-10d

2-10d, 1 1/2"

2-10d, 1 1/2"

7-10d 1 1/2"

12-10d 1 1/2

14 -16d

22 -10d

16 -10d

16 -10d

16 -10d

3-8d

4-8d

4-8d

4-8d

5-8d

5-8d

8-8d

5-10d, 1 1/2

13-8d

8-8d, 1 1/2"

6-10d

10-10d, 1 1/2

10-10d, 1 1/2

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

6 -10d

14-10d

16-10d

18-8d

28-8d

8-16d

18-10d, 1 1/2"

2-5/8" BOLTS

18 - 16d

16-16d

16-16d

12-16d

12-16d

18 - 16d

TO STUDS

TO STUDS

TO FOUNDATION

12" EMBEDMENT

-5/8" THREADED ROD

-5/8" THREADED ROD

12" EMBEDMENT

5/8" THREADED ROD

12" EMBEDMENT

TO STUDS

4 -10d

4 -10d

8 -10d

8 -10d

6-10d, 1 1/2"

10-10d, 1 1/2"

6-10d, 1 1/2"

10-10d, 1 1/2"

TO FOUNDATION

1/2" AB

1/2" AB

5/8" AB

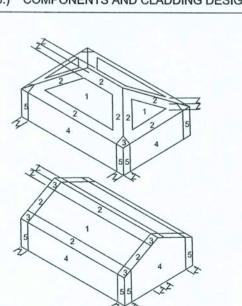
5/8" AB

1/2" AB

1/2" AB

2-5/8" AB

MEAN	LOSED SIMPLE DIAPHRAGM BUILDINGS V I ROOF HEIGHT NOT EXCEEDING LEAST I PPER HALF OF HILL OR ESCARPMENT 60	HORIZONTAL D	<b>IMENSION</b>	OR 60 FT; NO	TC
SLOP	E AND UNOBSTRUCTED UPWIND FOR 50	X HEIGHT OR 1	MILE WHIC	CHEVER IS L	ËSS
BUILD	DING IS NOT IN THE HIGH VELOCITY HURF	RICANE ZONE			
BUILD	DING IS NOT IN THE WIND-BORNE DEBRIS	REGION			
1.) E	BASIC WIND SPEED = 110 MPH				
2.) V	WIND EXPOSURE = B				
3.) V	WIND IMPORTANCE FACTOR = 1.0				
4.) E	BUILDING CATEGORY = II				
5.) F	ROOF ANGLE = 10-45 DEGREES				
6.) N	MEAN ROOF HEIGHT = <30 FT				
7.) II	NTERNAL PRESSURE COEFFICIENT = N/A	(ENCLOSED B	UILDING, 1	609)	
8.) (	COMPONENTS AND CLADDING DESIGN W	IND PRESSURI	ES (EBC TA	BI F 1609 B8	C)



DESIGN	LOADS
FLOOD	

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)	
	30 PSF (SLEEPING ROOMS)	
	30 PSF (ATTICS WITH STORAGE)	
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)	
ROOF	20 PSF (FLAT OR <4:12)	
	16 PSF (4:12 TO <12:12)	

12 PSF (12:12 AND GREATER) STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

SOIL BEARING CAPACITY 1000PSF NOT IN FLOOD ZONE (BUILDER TO VERIFY) REVISIONS

SOFTPLAN

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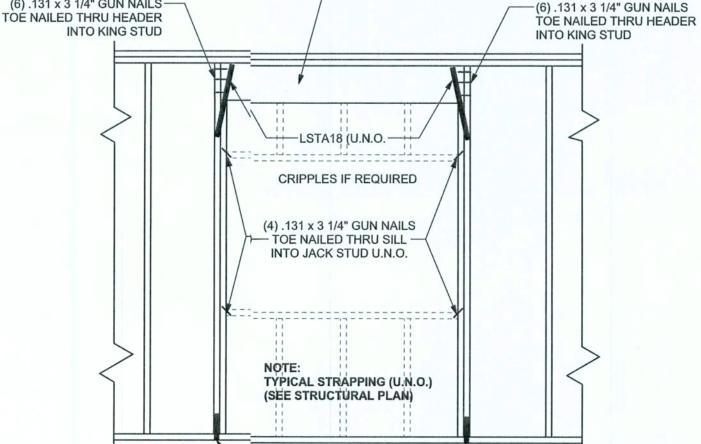
Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

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FINAL; DATE: 6 / Jan / 06

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> **S-1** OF 6 SHEETS



SUPPORTIVE BEAM -IF BEAM JOINT IS AT-POST CONNECTION, INSTALL ONE SIMPSON LSTA18 ON ONE SIDE 4-SIMPSON LSTA18 -(2-ONE SIDE,2-ON OTHER SIDE) -3-1/2" P.T

SUPPORTIVE POST TO BEAM

**DETAIL FOR SINGLE BEAM** 

SUPPORTIVE CENTER POST TO BEAM DETAIL

EA. BRACE W/4-10d NAILS

GYPSUM BOARD MAY BE REPLACED BY 29ga STEEL CEILING PANELS ATTACHED

BRACING SHOULD BE RUN AT 45 DEGREES FROM GABLE END TO SIDE WALLS AND

GYPSUM CEILING DIAPHRAGM

OPTION - GABLE END WALL

SCALE: NONE

ADD 6 - 16d NAILS AT EACH END OF THE RAT RUN INTO THE GABLE TRUSS AND INTO

THE SIDE WALL BLOCKING. . THE SUM OF DIAGONAL RAT RUNS AT 6'OC ALONG GABLE

AND 29ga DIAPHRAGM WILL RESIST 200 PLF OUT OF PLANE LATERAL LOAD AT THE GABLE

SUPPORTIVE -

3 SIMPSON LSTA18'S

OPPOSITE SIDE) EA.

NAILED WITH 14-10d

SCALE: N.T.S.

-GABLE END

- SIMPSON LSTA30 STRAP W/

10-8d NAILS IN 2 X 4 BRACE

& 10-8d NAILS IN ENDWALL STUD

TRUSS

- ENDWALL STUDS

NOTE: EXTEND RAT RUN BRACES GABLE

---- NON-SUPPORTIVE

2X4 LADDER BEAM

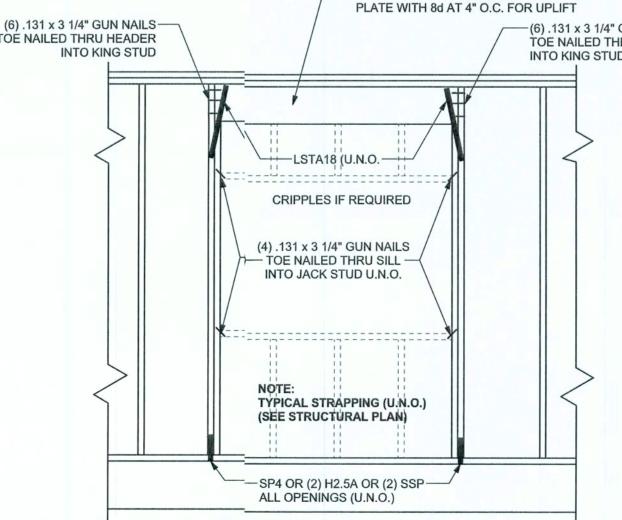
-SUPPORTIVE

TO 1/2 OF GABLE WIDTH FOR ROOFS

UP TO 7:12 AND 5/8 OF GABLE WIDTH

TO GABLE OR A LENGTH EQUAL

**OVER 7:12** 



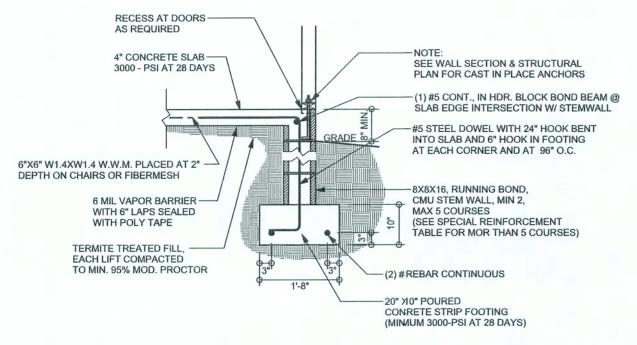
(1) 22X6 SPF #2 SILL UP TO 11'-0" U.N.O. (1) 2X4 SPF #2 SILL UP TO 7'-3" U.N.O. (FOR:: 110 MPH, 10'-0" WALL HIGHT U.N.O.) TYPICAL HEADER STRAPING DETAIL

#### **MASONRY NOTES:**

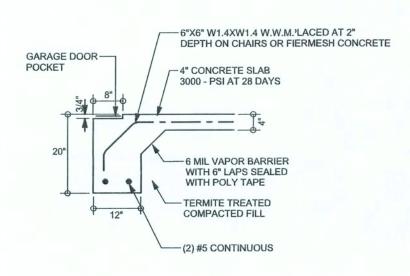
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALI CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

	ACI530.1-02 Section	Specific Requirements		
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi		
2.1	Mortar	ASTM C 270, Type N, UNO		
2.2	Grout	ASTM C 476, admixtures require approval		
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block		
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"		
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)		
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS		
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS		
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.		
3.3.E.7 Movement joints		Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.		

SOFTPIAN



F9 STEM WALL FOOTING
S-2 SCALE: 1/2" = 1'-0"

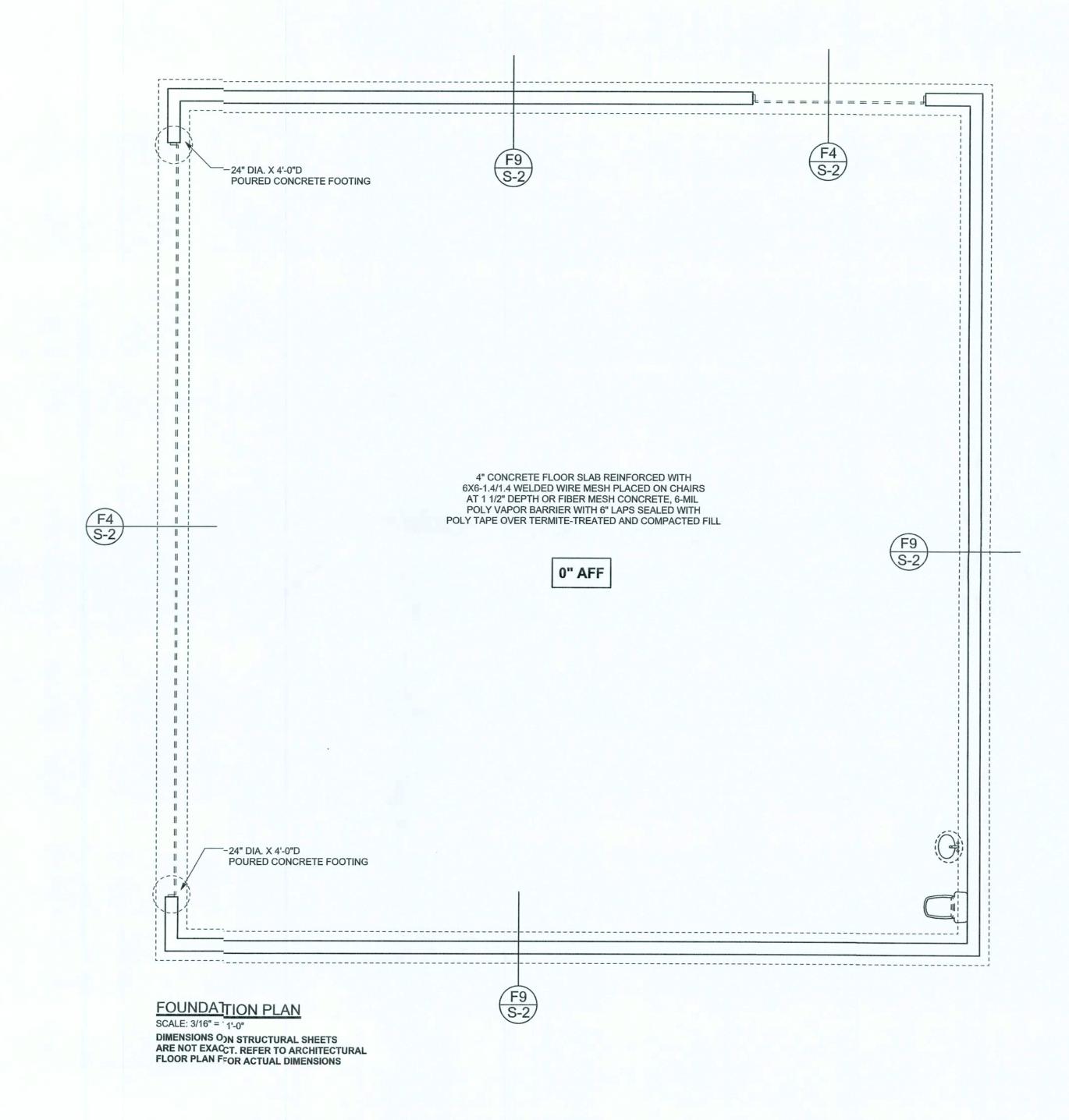


F4 GARAGE DOOR FOOTING
S-2 SCALE: 1/2" = 1'-0"

#### TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 1" into the reinforced slab at the top. The vertical steel is to be placed toward the tension sle of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall if the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizntal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU ray be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTIAL REINFORCEMENT FOR 2" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



WINDLCAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 36-754-5419

DIMENSONS: Stated dmensions supercede scaled dimensions. Refer all questions to Mark Diosway, P.E. for resolution. Do not poceed without clarification.

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CERTIFCATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply vith section 1609, florida building code 2004, to the best of my knowledge.

LIMITATON: This design is valid for one building, at specified location.

MARK DISOSWAY

#### **Edgley Construction**

Athur Hutchison & Ann Bormolini Hanger

ADDRESS: Let 3 Southern Landings S/D Columbia County, Florida

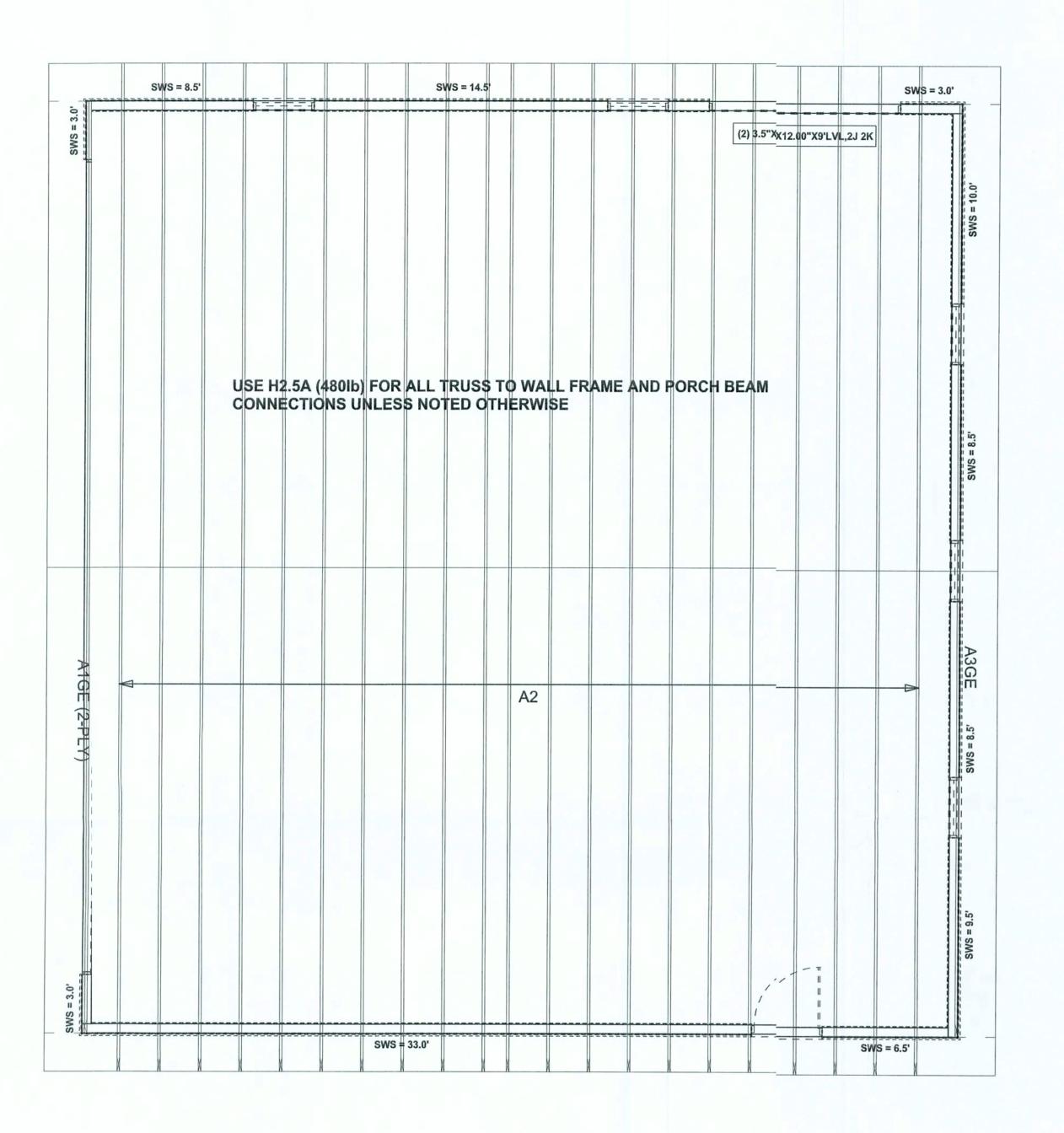
Nark Disosway P.E. P.O. Box 868 Lale City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: January 06, 2006 DRAVN BY: CHECKED BY: DavidDisosway

FINAIS DATE: 6 / Jan / 06

> JOB NUMBER: 510191a DRAWING NUMBER

> > OF 6 SHEETS



SEE TRUSS ENGINEERING -SHEATH BOTH SIDES w/ 7/16 OSB NAIL SHEATHING WITH 8d @ 3" O.C. EDGES 6" O.C. FIELD -ATTACH STEEL BEAM TO STUDS w/ 1/2" BOLTS @ 24' O.C. (TYP.) -6X6 STEEL I BEAM (3) 2X6 SYP #2 STUDS ---HANGER DOOR ATTACH DOOR TO I BEAM AND GIRDER TRUSS PER MANUFACTURES INSTRUCTIONS SIMPSON-HD8A w/ 7/8" X 16" AB & (3) 7/8" BOLTS TO STUD PACK (TYP.) (2) #5 X 3"--24" DIA. X 4'-0"D POURED CONCRETE FOOTING

GIRDER TRUSS ---

HANGER DOOR WALL DETAIL SCALE: 1/2" = 1'-0"

# SCALE: 1/4" = 1'-0"

#### S'RUCTURAL PLAN NOTES

- SI-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)
- ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SI-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

#### WALL LEGEND

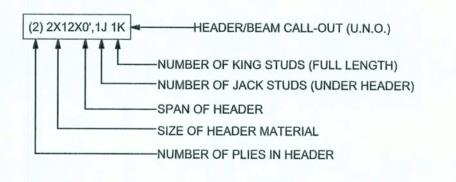
SM2 = 0.0,	1ST FLOOR EX (TERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NALLS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
SWS = 0.0'	2ND FLOOR EXCTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NALLS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
IBW	1ST FLOOR INTTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTTERIOR BEARING WALLS SEE DETAILS OON SHEET S-1

#### TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

OIS INDIONNES OF IENT WALL OF ONE		
	REQUIRED	ACTUAL
TRANSVERSE	26.5'	60.0'
LONGITUDINAL	23.5'	42.5'

## HEADER LEGEND



CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. W.B. HOWLAND JOB #3009 \**A**./

**REVISIONS** 

SOFTPI RCHITECTURAL DESIGN SOFTWARE

WINDLOAL ENGINEER: Mark Disosway, PE No.5395, POB 868, Lake City, FL 32056, 386754-5419

DIMENSIOIS: Stated dimensions supercede scaled

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MARK DISOSWAY P.E. 53915

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Edgley Construction

Arthur Hutchison & Ann Bormolini Hanger

ADDRESS: Lot : Southern Landings S/D Coumbia County, Florida

Mak Disosway P.E.
P.O. Box 868
LakeCity, Florida 32056
Phore: (386) 754 - 5419
Fax (386) 269 - 4871

PRINTED DATE:
lanuary 06, 2006

DRAWNBY: CHECKED BY:
David Dissway

FINALS DATE: 6 / Jan 06

> J0B NUMBER: 510191a

S-3

OF 6 SHEETS

