DATE 03/01/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000024170

APPLICANT NORA TERRY PHONE 754-5810

ADDRESS 291 SW SISTERS WELCOME RD LAKE CITY FL 32055

OWNER ELLIS & CAROLE NOLL PHONE

ADDRESS 784 NW ZACK DRIVE LAKE CITY FL 32055

CONTRACTOR BLAKE LUNDE PHONE 754-5810

LOCATION OF PROPERTY 90 W, R BROWN RD, L WINDING WAY, L EMERALD LAKES,
HOUSE ON NE CORNER OF ZACK AND EMERALD LAKES DR

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 131250.00

HEATED FLOOR AREA 2625.00 TOTAL AREA 3677.00 HEIGHT 17.06 STORIES 1

FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO.

PARCEL ID 28-3S-16-02372-633 SUBDIVISION ARBOR GREEN @ EMERALD LAKES

LOT 33 BLOCK PHASE 2 UNIT TOTAL ACRES 0.50

000000985		CBC1253408	Nora L. Terry	
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
PERMIT	06-0162-N	BLK	JTH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: FLOOR ONE FOOT ABOVE THE ROAD
NOC ON FILE

Check # or Cash 5004

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by | Foundation _____ date/app. by | Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by | Slab _____ date/app. by | Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by | Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by | Heat & Air Duct _____ date/app. by | Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by | C.O. Final _____ date/app. by | Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by | Pool _____ date/app. by

Reconnection _____ date/app. by | Pump pole _____ date/app. by | Utility Pole _____ date/app. by

M/H Pole _____ date/app. by | Travel Trailer _____ date/app. by | Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 660.00 CERTIFICATION FEE $ 18.39 SURCHARGE FEE $ 18.39

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 796.78

INSPECTORS OFFICE L. H. CLERKS OFFICE CN

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

ck# 5004

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0602-51 Date Received 2.15.06 By LT Permit # 985/24170

Application Approved by - Zoning Official BLK Date 28.02.06 Plans Examiner OKJTH Date 2-21-06

Flood Zone X Per Plat Development Permit N/A Zoning RSF-2 Land Use Plan Map Category RES. Low Den

Comments 867-1645

Applicants Name ~~Keith Blake Construction~~ Nora Terry Phone 754-5810

Address 10102 Oak Creek Lane, Knoxville, TN 37932

Owners Name Noll, Ellis & Carole Phone same as above

911 Address 784 NW Zack Dr, Lake City, FL 32055

Contractors Name Blake Construction Company Phone same as above

Address 291 SW Sisters Welcome Rd, Lake City, FL 32025

Fee Simple Owner Name & Address

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Tim Delbene

Mortgage Lenders Name & Address N/A

Circle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Progressive Energy

Property ID Number 28-3S-16-02372-633 Estimated Cost of Construction 185,000

Subdivision Name Arbor Greene@Emerald Lakes, Phs II Lot 33 Block — Unit — Phase —

Driving Directions Hwy 90 West, TR on Brown Rd, TL on Winding Way TL on Emerald Lakes Dr. House on NE Corner of Zack & Emerald Lakes Dr.

Type of Construction SFD Number of Existing Dwellings on Property 0

Total Acreage .5041 AC Lot Size .5041 AC Do you need a – Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 29' ✓ Side 17.8' ✓ Side 30.3 ✓ Rear 24.4 ✓

Total Building Height 17'6" Number of Stories 1 Heated Floor Area 2625 Roof Pitch 6/12

Porches 327 GARAGE 725 TOTAL 3677

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor)

STATE OF FLORIDA
COUNTY OF COLUMBIA

NORA L. TERRY
Notary Public - State of Florida
My Commission Expires Mar 24, 2009
Commission # DD 410803
Bonded By National Notary Assn.

Sworn to (or affirmed) and subscribed before me this 14th day of February 2006.

Personally known ✓ or Produced Identification

Contractor Signature

Contractors License Number CBC 1253408

Competency Card Number – N/A –

NOTARY STAMP/SEAL

Nora L. Terry

Notary Signature

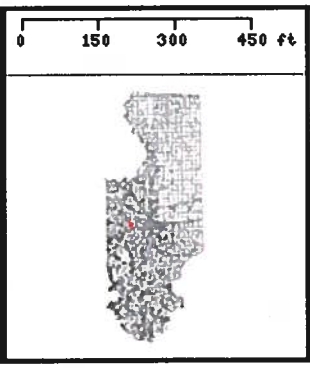
Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

PARCEL: 28-3S-16-02372-633 - VACANT (000000)

LOT 33 ARBOR GREENE AT EMERALD LAKES PHASE 2. ORB WD 998-2281, WD 1030-377.

Name:	NOLL ELLIS & CAROLE	LandVal	$18,275.00
Site:	ARBOR GREENE PHS 2	BldgVal	$0.00
Mail:	10102 OAK CREEK LANE	ApprVal	$18,275.00
	KNOXVILLE, TN 379321629	JustVal	$18,275.00
Sales	11/3/2004 $24,000.00V / Q	Assd	$18,275.00
Info	10/17/2003 $43,000.00V / Q	Exmpt	$0.00
		Taxable	$18,275.00



This information, GIS Map Updated: 8/3/2005, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

Columbia County Property Appraiser

DB Last Updated: 9/16/2005

2005 Proposed Values

Parcel: 28-3S-16-02372-633 Tax Record Property Card Interactive GIS Map Print

Search Result: 1 of 1

Owner & Property Info

Owner's Name	NOLL ELLIS & CAROLE
Site Address	ARBOR GREENE PHS 2
Mailing Address	10102 OAK CREEK LANE KNOXVILLE, TN 379321629
Brief Legal	LOT 33 ARBOR GREENE AT EMERALD LAKES PHASE 2. ORB WD 998-2281, WD 1030-377.

Use Desc. (code)	VACANT (000000)
Neighborhood	28316.05
Tax District	2
UD Codes	MKTA06
Market Area	06
Total Land Area	0.500 ACRES

Property & Assessment Values

Mkt Land Value	cnt: (1)	$18,275.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$18,275.00

Just Value	$18,275.00
Class Value	$0.00
Assessed Value	$18,275.00
Exempt Value	$0.00
Total Taxable Value	$18,275.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
11/3/2004	1030/377	WD	V	Q		$24,000.00
10/17/2003	998/2281	WD	V	Q		$43,000.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	1.000 LT - (.500AC)	1.00/1.00/1.00/1.00	$18,275.00	$18,275.00

Columbia County Property Appraiser DB Last Updated: 9/16/2005

1 of 1

Prepared by and
Return to:
Blake Construction Company
291 S.W. Sisters Welcome Road, Suite 102
Lake City, Florida 32055

Inst:2006003795 Date:02/15/2006 Time:15:48
DC,P.DeWitt Cason,Columbia County B:1074 P:689

TAX FOLIO NO.: 28-3S-16-02372-633

Notice of Commencement

STATE OF FLORIDA
COUNTY OF COLUMBIA

The undersigned hereby gives notice that the improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property: Lot 33, Arbor Greene of Emerald Lakes, Phase, II, a subdivision as recorded in plat book 7, page 32 of the public records of Columbia County, Florida.

2. General description of improvement: Construction of dwelling

3. Owner information:
   a. Name and address: Ellis and Carole Noll, 10102 Oak Creek Lane, Knoxville, Tennessee 37932
   b. Interest in property: Fee simple
   c. Name and address of fee simple title holder (if other than Owner):

4. Contractor: BLAKE CONSTRUCTION COMPANY, 291 S.W. Sisters Welcome Road, Suite 102, Lake City, Florida 32025

5. Surety: N/A
   a. Name and address: None
   b. Amount of bond: N/A

6. Lender: N/A

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by §713.13(1)(a)7, Florida Statutes: None

8. In addition to himself, Owner designates N/A to receive a copy of the Lienor's Notice as provided in §713.13(1)(b), Florida Statutes.

9. Expiration date of Notice of Commencement (the expiration date is 1 year from the date of recording unless a different date is specified:

By: Blake N. Lunde, II, President

The foregoing instrument was acknowledged before me this 14th day of February 2006 by Blake N. Lunde, II, who is personally known to me and who did not take an oath.

Notary Public

(SEAL)

NORA L. TERRY
Notary Public - State of Florida
My Commission Expires Mar 24, 2009
Commission # DD 410803
Bonded By National Notary Assn.

Notice of Treatment 11914

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: BAYA AVE
City Lake City Phone 752-1703

Site Location: Subdivision Emerald Lakes S/D / Arbor Greene
Lot # 33 Block# Permit # 24176
Address 784 NW Zack St.

Product used	Active Ingredient	% Concentration
☐ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☑ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☐ Soil ☑ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
Dwelling	3677	1003	5.5

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

5-10-06 Date | 0920 Time | F254 GUNNY Print Technician's Name

Remarks: _____

Applicator - White Permit File - Canary Permit Holder - Pink
10/05 ©

CLYATT WELL DRILLING, INC.

Established in 1971
Post Office Box 180
Worthington Springs, Florida 32697
Phone (386)496-2488 FAX (386)496-4640

WELL DESCRIPTION

DESCRIPTION DATE

CUSTOMER NAME AND ADDRESS

Blake Construction Company
of North Florida, Inc.
872 Southwest Jaguar Drive
Lake City, Florida 32025

DESCRIPTION OF WORK

4" Well and Pump

DESCRIPTION

4" Well

1 HP Submersible Pump

1-1/4" Galvanized Drop Pipe

14/3 Submersible Pump Wire

PC244 (220 Gallon Equivalent) Pressure Tank

4 X 1-1/4 Well Seal

Pressure Relief Valve

Controls & Fittings

Sales Tax

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Noll Residence**	Builder:	Blake Construction
Address:	Lot: 33, Sub: Arbor Greene P2, Plat: 7, Page 32	Permitting Office:	Columbia Co
City, State:	Lake City, FL 32055-	Permit Number:	
Owner:	Noll	Jurisdiction Number:	121000
Climate Zone:	North		

1. New construction or existing		New	
2. Single family or multi-family		Single family	
3. Number of units, if multi-family		1	
4. Number of Bedrooms		3	
5. Is this a worst case?		No	
6. Conditioned floor area (ft²)		2625 ft²	
7. Glass area & type	Single Pane	Double Pane	
a. Clear glass, default U-factor	0.0 ft²	295.0 ft²	
b. Default tint	0.0 ft²	0.0 ft²	
c. Labeled U or SHGC	0.0 ft²	0.0 ft²	
8. Floor types			
a. Slab-On-Grade Edge Insulation		R=0.0, 269.0(p) ft	
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior		R=13.0, 2083.0 ft²	
b. N/A			
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic		R=30.0, 2625.0 ft²	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior		Sup. R=6.0, 25.0 ft	
b. N/A			

12. Cooling systems		
a. Central Unit	Cap: 35.0 kBtu/hr	
	SEER: 12.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 35.0 kBtu/hr	
	HSPF: 7.90	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 30.0 gallons	
	EF: 0.90	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	PT, CF,	

Glass/Floor Area: 0.11

Total as-built points: 28338
Total base points: 36038

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Tim Delbene

DATE: 9/14/05

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 33, Sub: Arbor Greene P2, Plat: 7, Page 32, Lake City, FL, 32055** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	2625.0	20.04	9468.9	Double, Clear	N	2.0	5.0	9.0	19.20	0.87	150.5	
				Double, Clear	N	2.0	7.0	20.0	19.20	0.92	354.1	
				Double, Clear	N	2.0	7.0	15.0	19.20	0.92	265.6	
				Double, Clear	S	2.0	7.0	30.0	35.87	0.82	882.5	
				Double, Clear	S	12.0	10.0	36.0	35.87	0.49	636.3	
				Double, Clear	S	2.0	7.0	26.0	35.87	0.82	764.8	
				Double, Clear	E	2.0	7.0	15.0	42.06	0.89	559.0	
				Double, Clear	E	2.0	9.0	25.0	42.06	0.93	980.8	
				Double, Clear	E	12.0	10.0	48.0	42.06	0.47	947.3	
				Double, Clear	E	12.0	10.0	36.0	42.06	0.47	710.5	
				Double, Clear	W	2.0	7.0	32.0	38.52	0.89	1093.1	
				Double, Clear	W	2.0	4.0	3.0	38.52	0.73	84.4	
				As-Built Total:				**295.0**			**7428.7**	

WALL TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior	13.0	2083.0	1.50	3124.5
Exterior	2083.0	1.70	3541.1					
Base Total:	**2083.0**		**3541.1**	**As-Built Total:**		**2083.0**		**3124.5**

DOOR TYPES	Area X	BSPM	= Points	Type	Area X	SPM	= Points
Adjacent	21.0	2.40	50.4	Exterior Insulated	21.0	4.10	86.1
Exterior	42.0	6.10	256.2	Exterior Insulated	21.0	4.10	86.1
				Adjacent Insulated	21.0	1.60	33.6
Base Total:	**63.0**		**306.6**	**As-Built Total:**	**63.0**		**205.8**

CEILING TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM X SCM	= Points
Under Attic	2625.0	1.73	4541.3	Under Attic	30.0	2625.0	1.73 X 1.00	4541.3
Base Total:	**2625.0**		**4541.3**	**As-Built Total:**		**2625.0**		**4541.3**

FLOOR TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Slab	269.0(p)	-37.0	-9953.0	Slab-On-Grade Edge Insulation	0.0	269.0(p	-41.20	-11082.8
Raised	0.0	0.00	0.0					
Base Total:			**-9953.0**	**As-Built Total:**		**269.0**		**-11082.8**

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 33, Sub: Arbor Greene P2, Plat: 7, Page 32, Lake City, FL, 32055** PERMIT #:

BASE					AS-BUILT						
INFILTRATION	Area	X	BSPM	= Points		Area	X	SPM	=	Points	
	2625.0		10.21	26801.3		2625.0		10.21		26801.3	
Summer Base Points:				**34706.1**	**Summer As-Built Points:**						**31018.7**
Total Summer Points	X	System Multiplier	=	Cooling Points	Total Component	X	Cap Ratio	X	Duct Multiplier (DM x DSM x AHU)	X	System Multiplier	X	Credit Multiplier	=	Cooling Points
34706.1		**0.4266**		**14805.6**	31018.7 **31018.7**		1.000 **1.00**		(1.090 x 1.147 x 0.91) **1.138**		0.284 **0.284**		0.902 **0.902**		9058.5 **9058.5**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 33, Sub: Arbor Greene P2, Plat: 7, Page 32, Lake City, FL, 32055** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	2625.0	12.74	6019.6	Double, Clear	N	2.0	5.0	9.0	24.58	1.01	222.7	
				Double, Clear	N	2.0	7.0	20.0	24.58	1.00	493.2	
				Double, Clear	N	2.0	7.0	15.0	24.58	1.00	369.9	
				Double, Clear	S	2.0	7.0	30.0	13.30	1.17	467.1	
				Double, Clear	S	12.0	10.0	36.0	13.30	3.05	1459.4	
				Double, Clear	S	2.0	7.0	26.0	13.30	1.17	404.8	
				Double, Clear	E	2.0	7.0	15.0	18.79	1.05	294.7	
				Double, Clear	E	2.0	9.0	25.0	18.79	1.03	483.2	
				Double, Clear	E	12.0	10.0	48.0	18.79	1.34	1208.1	
				Double, Clear	E	12.0	10.0	36.0	18.79	1.34	906.1	
				Double, Clear	W	2.0	7.0	32.0	20.73	1.03	684.0	
				Double, Clear	W	2.0	4.0	3.0	20.73	1.08	67.4	
				As-Built Total:				**295.0**			**7060.7**	

WALL TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM	= Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior	13.0	2083.0	3.40	7082.2
Exterior	2083.0	3.70	7707.1					
Base Total:	**2083.0**		**7707.1**	**As-Built Total:**		**2083.0**		**7082.2**

DOOR TYPES	Area X	BWPM	= Points	Type	Area X	WPM	= Points
Adjacent	21.0	11.50	241.5	Exterior Insulated	21.0	8.40	176.4
Exterior	42.0	12.30	516.6	Exterior Insulated	21.0	8.40	176.4
				Adjacent Insulated	21.0	8.00	168.0
Base Total:	**63.0**		**758.1**	**As-Built Total:**	**63.0**		**520.8**

CEILING TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM X WCM	= Points
Under Attic	2625.0	2.05	5381.3	Under Attic	30.0	2625.0	2.05 X 1.00	5381.3
Base Total:	**2625.0**		**5381.3**	**As-Built Total:**		**2625.0**		**5381.3**

FLOOR TYPES	Area X	BWPM	= Points	Type	R-Value	Area X	WPM	= Points
Slab	269.0(p)	8.9	2394.1	Slab-On-Grade Edge Insulation	0.0	269.0(p	18.80	5057.2
Raised	0.0	0.00	0.0					
Base Total:			**2394.1**	**As-Built Total:**		**269.0**		**5057.2**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 33, Sub: Arbor Greene P2, Plat: 7, Page 32, Lake City, FL, 32055** PERMIT #:

BASE				AS-BUILT						
INFILTRATION	Area X	BWPM	= Points				Area X	WPM	=	Points
	2625.0	-0.59	-1548.7				2625.0	-0.59		-1548.7
Winter Base Points:			**20711.5**	**Winter As-Built Points:**						**23553.4**
Total Winter Points	X System Multiplier	=	Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Heating Points
20711.5	**0.6274**		**12994.4**	23553.4 **23553.4**	1.000 **1.00**	(1.069 x 1.169 x 0.93) **1.162**	0.432 **0.432**	0.950 **0.950**		11224.8 **11224.8**

FORM 600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 33, Sub: Arbor Greene P2, Plat: 7, Page 32, Lake City, FL, 32055** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	N/A
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

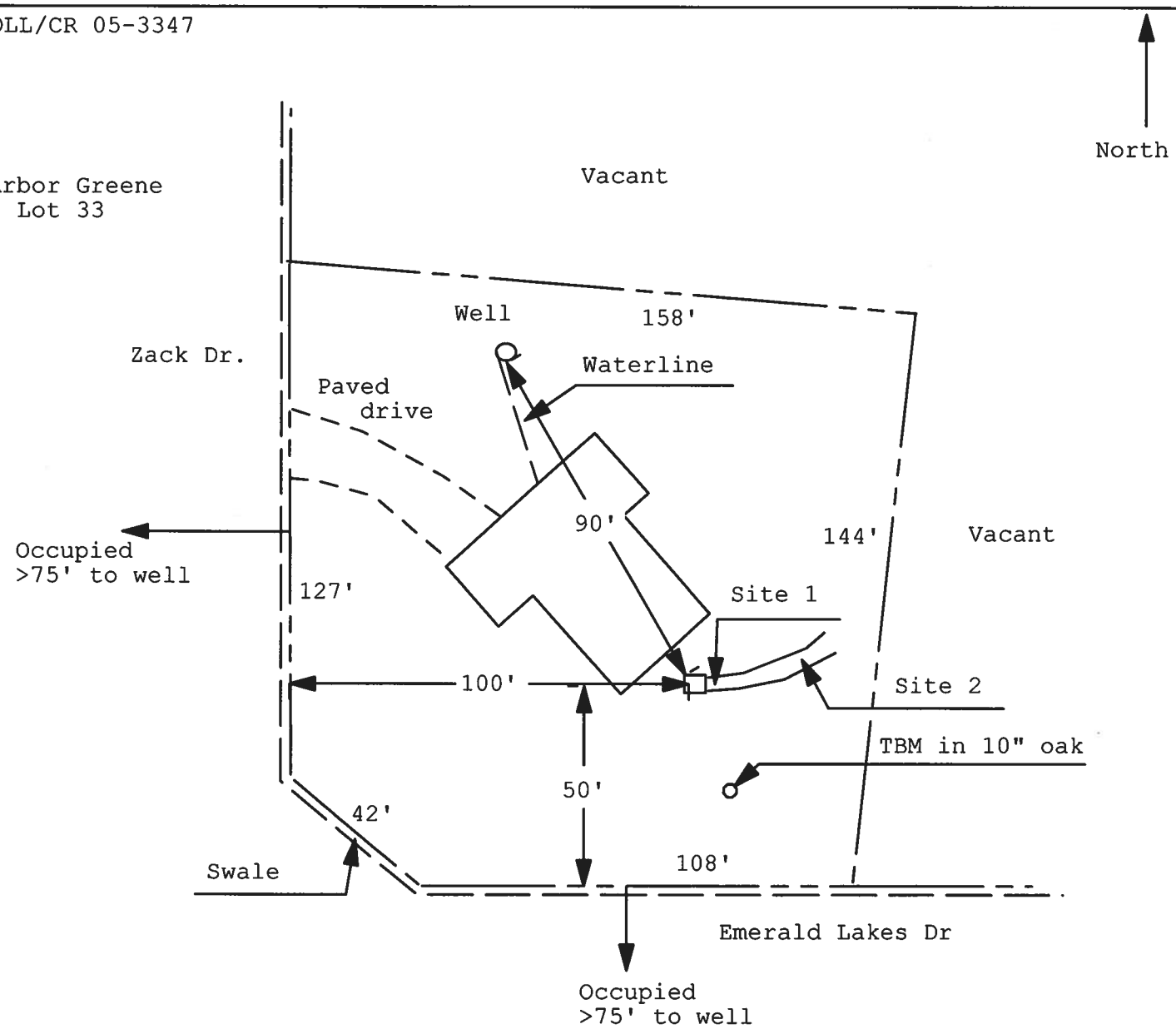
COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	N/A
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 06-0162N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

NOLL/CR 05-3347



1 inch = 40 feet

Site Plan Submitted By Paul Lloyd **Date** 2/8/06

Plan Approved ✓ **Not Approved** ____ **Date** 2-23-06

By [signature] Columbia **CPHU**

Notes: _____

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000000985

DATE 03/01/2006 PARCEL ID # 28-3S-16-02372-633

APPLICANT NORA TERRY PHONE 754-5810

ADDRESS 291 SW SISTERS WELCOME RD LAKE CITY FL 32025

OWNER ELLIS & CAROLE NOLL PHONE

ADDRESS 784 NW ZACK DRIVE LAKE CITY FL 32055

CONTRACTOR BLAKE LUNDE PHONE 754-5810

LOCATION OF PROPERTY 90 W, R BROWN RD, L WINDING WAY, L EMERALD LAKES,

HOUSE ON NE CORNER OF ZACK AND EMERALD LAKES DR

SUBDIVISION/LOT/BLOCK/PHASE/UNIT ARBOR GRN @EMERALD L 33 2

SIGNATURE Nora L Terry

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY, FLORIDA

COLUMBIA COUNTY BUILDING DEPARTMENT

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2001 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE MARCH 1, 2002

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 1606 OF THE FLORIDA BUILDING CODE 2001 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1606 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ------- 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional" details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 104.2.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** a) Plans or specifications must state compliance with FBC Section 1606 b) The following information must be shown as per section 1606.1.7 FBC a. Basic wind speed (MPH) b. Wind importance factor (I) and building category c. Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated d. The applicable internal pressure coefficient e. Components and Cladding. The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component and cladding materials not specifically designed by the registered design professional
☑	☐	**Elevations including:**
☑	☐	a) All sides
☑	☐	b) Roof pitch
☑	☐	c) Overhang dimensions and detail with attic ventilation
☑	☐	d) Location, size and height above roof of chimneys
☐	☐	e) Location and size of skylights
☑	☐	f) Building height
☑	☐	e) Number of stories

Floor Plan Including:

- ☑ ☐ a) Rooms labeled and dimensioned
- ☑ ☐ b) Shear walls
- ☑ ☐ c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed (egress windows in bedrooms to be shown)
- ☑ ☐ d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- ☐ N/A ☐ e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- ☑ ☐ f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan Including:

- ☑ ☐ a) Location of all load-bearing wall with required footings indicated as standard Or monolithic and dimensions and reinforcing
- ☑ ☐ b) All posts and/or column footing including size and reinforcing
- ☐ N/A ☐ c) Any special support required by soil analysis such as piling
- ☑ ☐ d) Location of any vertical steel

Roof System:

- ☑ ☐ a) Truss package including:
  1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
  2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- N/A ☐ b) Conventional Framing Layout including:
  1. Rafter size, species and spacing
  2. Attachment to wall and uplift
  3. Ridge beam sized and valley framing and support details
  4. Roof assembly (FBC 104.2.1 Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections Including:

- N/A ☐ a) Masonry wall
  1. All materials making up wall
  2. Block size and mortar type with size and spacing of reinforcement
  3. Lintel, tie-beam sizes and reinforcement
  4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
  5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
  6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
  7. Fire resistant construction (if required)
  8. Fireproofing requirements
  9. Shoe type of termite treatment (termicide or alternative method)
  10. Slab on grade
      a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
      b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
  11. Indicate where pressure treated wood will be placed
  12. Provide insulation R value for the following:
      a. Attic space
      b. Exterior wall cavity
      c. Crawl space (if applicable)

☑ ☐ **b) Wood frame wall**

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
    a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

N/A ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

N/A ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

N/A ☐ b) Floor joist size and spacing

N/A ☐ c) Girder size and spacing

N/A ☐ d) Attachment of joist to girder

N/A ☐ e) Wind load requirements where applicable

N/A ☐ **Plumbing Fixture layout**

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☑ ☐ b) Ceiling fans

☑ ☐ c) Smoke detectors

☑ ☐ d) Service panel and sub-panel size and location(s)

☑ ☐ e) Meter location with type of service entrance (overhead or underground)

☑ ☐ f) Appliances and HVAC equipment

☑ ☐ g) Arc Fault Circuits (AFCI) in bedrooms

HVAC Information

☑ ☐ a) Manual J sizing equipment or equivalent computation

☑ ☐ b) Exhaust fans in bathroom

☑ ☐ **Energy Calculations** (dimensions shall match plans)

☐ ☐ **Gas System** Type (LP or Natural) Location and BTU demand of equipment

N/A ☐ **Disclosure Statement for Owner Builders**

☑ ☐ ***Notice Of Commencement Required Before Any Inspections Will Be Done

☑ ☐ **Private Potable Water**

a) Size of pump motor

b) Size of pressure tank

c) Cycle stop valve if used

ELK Roofing Products Specifications – Tuscaloosa, AL

PRESTIQUE®
HIGH DEFINITION®

RAISED PROFILE®

Prestique Plus *High Definition* and Prestique Gallery Collection®

Product size	13¼"x 39⅜"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	11

50-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph, extended 110 mph***

Raised Profile

Product size	13¼"x 38⅝"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 70 mph.

Prestique I *High Definition*

Product size	13¼"x 39⅜"
Exposure	5⅝"
Pieces/Bundle	16
Bundles/Square	4/98.5 sq.ft.
Squares/Pallet	14

40-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph, extended 90 mph***

Prestique *High Definition*

Product size	13¼"x 38⅝"
Exposure	5⅝"
Pieces/Bundle	22
Bundles/Square	3/100 sq.ft.
Squares/Pallet	16

30-year limited warranty period: 5-7**years non-prorated coverage for shingles and application labor with prorated coverage for remainder of limited warranty period, plus an option for transferability*. 5-year limited wind warranty*. Wind Coverage: standard 80 mph.

HIP AND RIDGE SHINGLES

Seal-A-Ridge® w/*FLX*

Size: 12"x 12"
Exposure: 6⅝"
Pieces/Bundle: 45
Coverage: 4 Bundles = 100 linear feet

Vented RidgeCrest® w/*FLX*

Size: 13"x13¼"
Exposure: 9¼"
Pieces/Box: 26
Coverage: 5 boxes = 100 linear feet

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
19 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors (Check Availability): Antique Slate, Weatheredwood, Shakewood, Sablewood, Hickory, Barkwood, Forest Green, Wedgewood, Birchwood, Sandalwood.
Gallery Collection: Balsam Forest™, Weathered Sage™, Sienna Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge, and Prestique Starter Strip roofing products contain sealant which activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard* treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae.

All Prestique and Raised Profile shingles meet UL* Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 790); and ASTM Specifications D 3018, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles have approval from the Florida Building Code Commission, Metro-Dade County, ICBO, and Texas Department of Insurance.

*See actual limited warranty for conditions and limitations.
** Effective January 1, 2004, the seven year non-prorated Umbrella Coverage Period applies only when a full Elk Roof System is installed with the original installation of the Elk shingles, all in accordance with Elk's application instructions for such products. A full Elk roof system includes Elk Hip and Ridge shingles on all hips and ridges, Elk Starter Strip along all rake and eave edges, an Elk ventilation system, and Elk All-Climate Self-Adhering Underlayment in all valleys. Additionally, Elk All-Climate Self-Adhering Underlayment is required along the rake and eave edges of the roof in and north of the states of VA, KY, MO, KS, CO, UT, NV, & OR.
***For a limited Wind Warranty up to 110 mph for Prestique Gallery Collection, Prestique Plus, or 90 mph for Prestique I or Grandé, at least six (6) properly placed NAILS and Elk Starter Strip shingles are required. See application instructions printed on the shingle wrapper for additional requirements.

Specifications

Scope: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color). Hip and ridge type to be Elk Seal-A-Ridge with formula *FLX*.

All exposed metal surfaces (flashing, vents, etc.) to be painted with matching Elk roof accessory paint.

Preparation of Roof Deck: Roof deck to be dry, well-seasoned 1" x 6" (25.4mm x 152.4mm) boards; exterior-grade plywood (exposure 1 rated sheathing) at least 3/8" (9.525mm) thick conforming to the specifications of the American Plywood Association; 7/16" (11.074mm) oriented strandboard; or chipboard. Most fire retardant plywood decks are NOT approved substrates for Elk shingles. Consult Elk Field Service for application specifications over other decks and other slopes.

Materials: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater: apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For Low slopes[4" per foot (101.6/304.8mm) to a minimum of 2" per foot (50.8/304.8mm)], use two plies of underlayment overlapped a minimum of 19". Fasteners shall be of sufficient length and holding power for securing material as required by the application instructions printed on shingle wrapper.

For areas where algae is a problem, shingles shall be (name) with StainGuard treatment, as manufactured by the Elk Tuscaloosa plant. Hip and ridge type to be Seal-A-Ridge with formula *FLX* with StainGuard treatment.

Complete application instructions are published by Elk and printed on the back of every shingle bundle. All warranties are contingent upon the correct installation as shown on the instructions. These instructions are the minimum required to meet Elk application requirements. In some areas, building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements less than those contained in its application instructions.

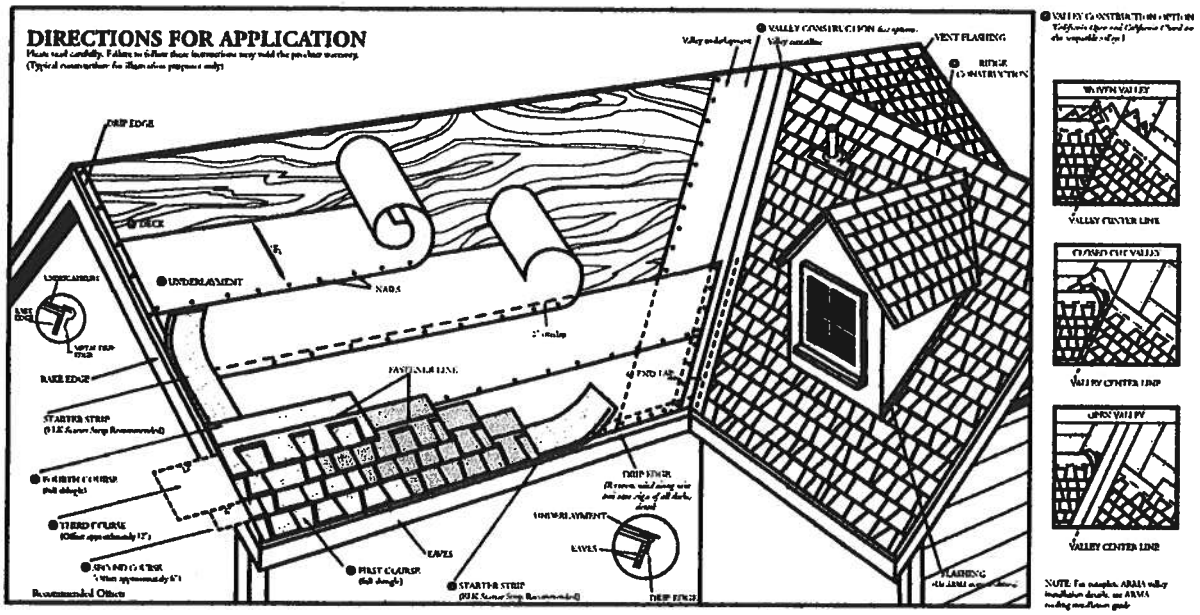
For specifications in CSI format, call 800.354.SPEC (7732) or e-mail specinfo@elkcorp.com.

Southeast & Atlantic Office: 800.945.5551

Corporate Headquarters: 800.354.7732

Plant Location: 800.945.5545

ELK The Premium Choice™
www.elkcorp.com



DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet Elk's application requirements. Your failure to follow these instructions may void the product warranty. In some areas, the building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements that are less than those printed here. Shingles should not be jammed tightly together. All attics should be properly ventilated. Note: It is not necessary to remove tape on back of shingle.

❶ DECK PREPARATION

Roof decks should be dry, well-seasoned 1" x 6" boards or exterior grade plywood minimum 3/8" thick and conform to the specifications of the American Plywood Association or 7/16" oriented strandboard, or 7/16" chipboard.

❷ UNDERLAYMENT

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt). Elk Versashield® or self adhering underlayment is also acceptable. Cover drip edge at eaves only.

For low slope(2/12 up to 4/12), completely cover the deck with two plies of underlayment overlapping a minimum of 19". Begin by fastening a 19" wide strip of underlayment placed along the eaves. Place a full 36" wide sheet over the starter, horizontally placed along the eaves and completely overlapping the starter strip.

EAVE FLASHING FOR ICE DAMS (ASK A ROOFING CONTRACTOR, REFER TO ARMA MANUAL OR CHECK LOCAL CODES)

For standard slope (4/12 to less than 21/12), use coated roll roofing of no less than 50 pounds over the felt underlayment extending from the eave edge to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

For low slope (2/12 up to 4/12), use a continuous layer of asphalt plastic cement between the two plies of underlayment from the eave edge up roof to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

Consult the Elk Technical Services Department for application specifications over other decks and other slopes.

❸ STARTER SHINGLE COURSE

USE AN ELK STARTER STRIP OR THE HEADLAP OF A STRIP SHINGLE WITH THE ADHESIVE STRIP POSITIONED AT THE EAVE EDGE. With at least 3" trimmed from the end of the first shingle, start at the rake edge overhanging the eave and rake edges 1/2" to 3/4". Fasten 2" from the lower edge and 1" from each side.

❹ FIRST COURSE

Start at rake and continue course with full shingles laid flush with the starter course. Shingles may be applied with a course alignment of 45° on the roof

❺ SECOND COURSE

Offset the second course of shingles with respect to the first by approximately 6". Other offsets are approved if greater than 4".

❻ THIRD COURSE

Offset the next course by 6" with respect to the second course, or consistent with the original offset.

❼ FOURTH COURSE

Start at the rake and continue with full shingles across roof.

FIFTH AND SUCCEEDING COURSES.

Repeat application as shown for second, third, and fourth courses. Do not rack shingles straight up the roof. Offsets may be adjusted around valleys and penetrations.

❽ VALLEY CONSTRUCTION

Open, woven and closed cut valleys are acceptable when applied by Asphalt Roofing Manufacturing Association (ARMA) recommended procedures. For metal valleys, use 36" wide vertical underlayment prior to applying metal flashing (secure edge with nails). No nails are to be within 6" of valley center.

❾ RIDGE CONSTRUCTION

For ridge construction Elk recommends Class "A" Z®Ridge or Seal-A-Ridge® with formula *FLX*™ or RidgeCrest™ with *FLX* (See ridge package for installation instructions). Vented RidgeCrest or 3-tab shingles are also approved.

FASTENERS

While nailing is the preferred method for Elk shingles, Elk will accept fastening methods according to the following instructions.

Using the fastener line as a reference, nail or staple the shingle in the double thickness common bond area. For shingles without a fastener line, nails or staples must be placed between and/or in the sealant dots.

NAILS: Corrosive resistant, 3/8" head, minimum 12-gauge roofing nails. Elk recommends 1-1/4" for new roofs and 1-1/2" for roof-overs. In cases where you are applying shingles to a roof that has an exposed overhang, for new roofs only, 3/4" ring shank nails are allowed to be used from the eave's edge to a point up the roof that is past the outside wall line. 1" ring shank nails allowed for re-roof.

STAPLES: Corrosive resistant, 16-gauge minimum, crown width minimum of 15/16". Note: An improperly adjusted staple gun can result in raised staples that can cause a fish-mouthed appearance and can prevent sealing.

Fasteners should be long enough to obtain 3/4" deck penetration or penetration through deck, whichever is less. This product meets the requirements of the IRC 2003 code when fastened with 4 nails.

MANSARD APPLICATIONS

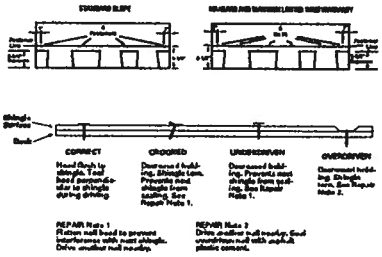
Correct fastening is critical to the performance of the roof. For slopes exceeding 60° (or 21/12) use six fasteners per shingle. Locate fasteners in the fastener area 1" from each side edge with the remaining four fasteners equally spaced along the length of the double thickness (laminated) area. Only fastening methods according to the above instructions are acceptable.

LIMITED WIND WARRANTY

- For a Limited Wind Warranty, all Prestique and Raised Profile™ shingles must be applied with 4 properly placed fasteners, or in the case of mansard applications, 6 properly placed fasteners per shingle.
- For a Limited Wind Warranty up to 110 MPH for Prestique Gallery Collection or Prestique Plus or 90 MPH for Prestique I, shingles must be applied with 6 properly placed NAILS per shingle. SHINGLES APPLIED WITH STAPLES WILL NOT QUALIFY FOR THIS ENHANCED LIMITED WIND WARRANTY. Also, Elk Starter Strip shingles must be applied at the eaves and rake edges to qualify Prestique Plus, Prestique Gallery Collection and Prestique I shingles for this enhanced Limited Wind Warranty. Under no circumstances should the Elk Shingles or the Elk Starter Strip overhang the eaves or rake edge more than 3/4 of an inch.

HELP STOP BLOW-OFFS AND CALL-BACKS

A minimum of four fasteners must be driven into the DOUBLE THICKNESS (laminated) area of the shingle. Nails or staples must be placed along – and through – the "fastener line" or on products without fastener lines, nail or staple between and in line with sealant dots. CAUTION: Do not use fastener line for shingle alignment.



Refer to local codes which in some areas may require specific application techniques beyond those Elk has specified.
All Prestique and Raised Profile shingles have a U.L.® Wind Resistance Rating when applied in accordance with these instructions using nails or staples on re-roofs as well as new construction.

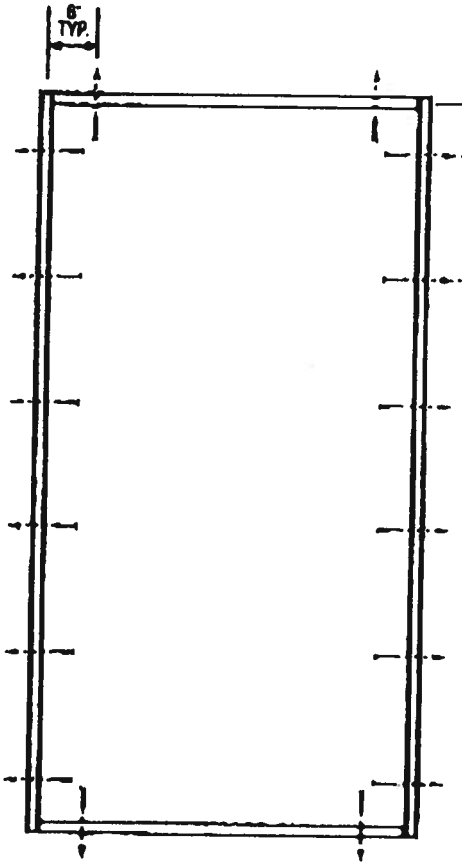
CAUTION TO WHOLESALER: Careless and improper storage or handling can harm fiberglass shingles. Keep these shingles completely covered, dry, reasonably cool, and protected from the weather. Do not store near various sources of heat. Do not store in direct sunlight until applied. DO NOT DOUBLE STACK. Systematically rotate all stock so that the material that has been stored the longest will be the first to be moved out.

ELK
The Premium Choice®
www.elkcorp.com

X
Unit

MID-WL-MA0001-02

SINGLE DOOR



Minimum Fastener Count

- 6 per vertical framing member
- 2 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Test Data Review Certificate #3026447A; #3026447B, #3026447C and COP/Test Report Validation Matrix #3026447A-001, 002, 003, #3026447B-001, 002, 003; #3026447C-001, 002, 003 provides additional information - available from the ITW/MM website (www.itwamm.com) the Masonite website (www.masonite.com) or the Masonite technical center

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 3146, 3186, 3241*, 3246, 3261* or 3265**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom.

*Based on required Design Pressure – see COP sheet for details.

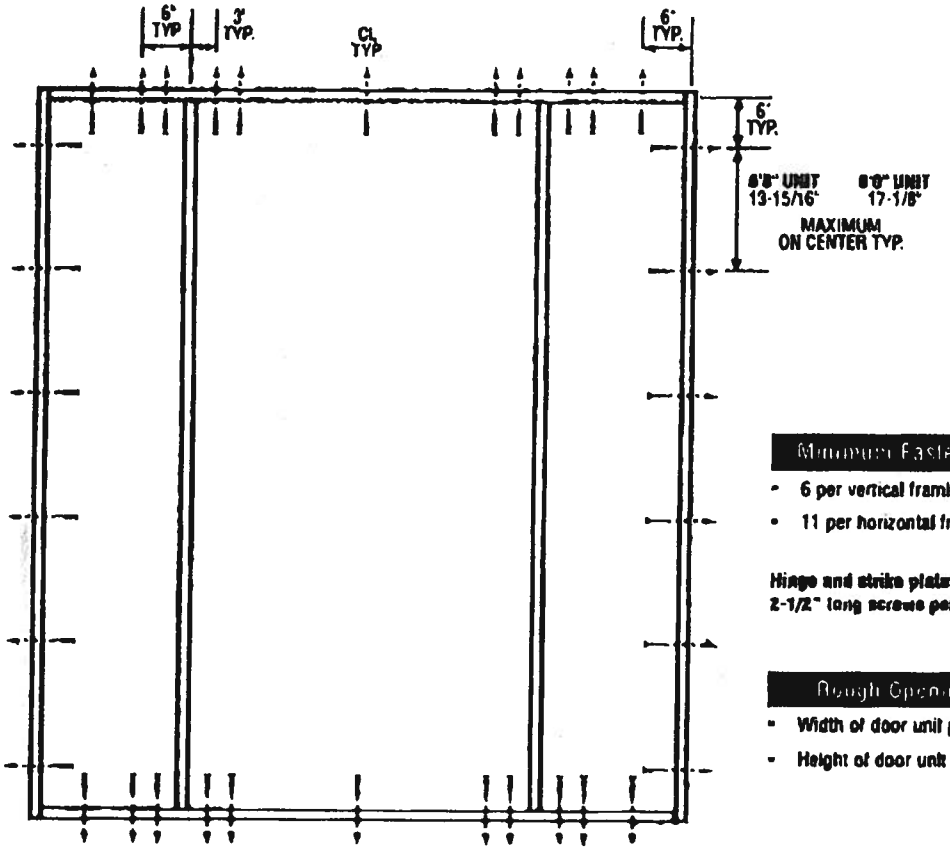
Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

OXO
Unit

MID-WL-MA0004-02

SINGLE DOOR WITH 2 SIDELITES



Minimum Fastener Count

- 6 per vertical framing member
- 11 per horizontal framing member

Hinge and strike plates require two 2-1/2" long screws per location.

Rough Opening (RO)

- Width of door unit plus 1/2"
- Height of door unit plus 1/4"

Test Data Review Certificate #3028447A; #3028447B; #3028447C and COP/Test Report Validation Matrix #3028447A-001, 002, 003; #3028447B-001, 002, 003, #3028447C-001, 002, 003 provides additional information - available from the ITS/WH website (www.ittswh.com), the Masonite website (www.masonite.com) or the Masonite technical center

Latching Hardware:

- Compliance requires that GRADE 3 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.
- **UNITS COVERED BY COP DOCUMENT 3244*, 3249, 3264* or 3269**
  Compliance requires that 8" GRADE 1 (ANSI/BHMA A156.16) surface bolts be installed on latch side of active door panel – (1) at top and (1) at bottom

*Based on required Design Pressure – see COP sheet for details.

Notes:

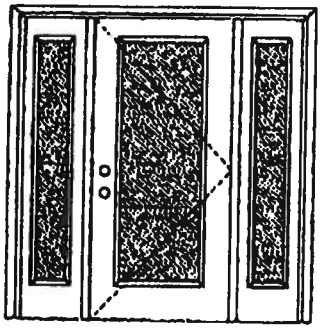
1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
2. The wood screw single shear design values come from Table 11.3A of ANSI/AF & PA NDS for southern pine lumber with a side member thickness of 1-1/4" and achievement of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and ELCO Dade Country approvals respectively, each with minimum 1-1/4" embedment.
3. Wood bucks by others, must be anchored properly to transfer loads to the structure.

OXO
Glazed Inswing Unit

COP-WL-JH4144-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.etlsenrte.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Single Door with 2 Sidelites
Maximum unit size = 9'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistance requirements for a specific building design and geographic location is determined by ASCE 7-98/02/05, state or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0004-02 or MAD-WL-MA0007-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0004-02.

APPROVED DOOR STYLES:

1/4 GLASS:


100 Series


133, 135 Series


138 Series


680 Series


822 Series

1/2 GLASS:


105 Series*


108, 160 Series*


129 Series*


200 Series*


12 RL, 23 RL, 24 RL Series*


107 Series*


108 Series


304 Series

*The glass kit may also be used in the following door styles: 5-panel, 6-panel with scroll; Eyebrow 6-panel; Eyebrow 5-panel with scroll

Johnson™ EntrySystems

June 17, 2004
Our continuing program of product improvement makes specifications, design and product subject to change without notice.

PREMDOOR Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

OXO
Glazed Inswing Unit

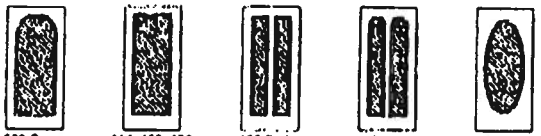
COP-WL-JH4144-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:

FULL GLASS:



APPROVED SIDELITE STYLES:

CERTIFIED TEST REPORTS:

NCTL 210-1097-7, 8, 9, 10, 11, 12; NCTL 210-1061-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 28-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab and sidelite panels glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Test Data Review Certificate #3028447A and COP/Test Report Validation Matrix #3028447A-001 provides additional information - available from the ITS/WII website (www.stlmasonite.com), the Masonite website (www.masonite.com) or the Masonite technical center

Johnson EntrySystems

June 17, 2002

Our continuing program of product improvement may change specifications, design and product detail subject to change without notice.

PREMDOOR Fiberglass Premium Quality Doors

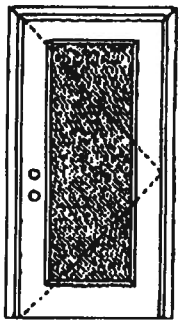
Exclusively from Masonite®
Masonite International Corporation

X
Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'0".

Test Data Review Certificate #3028447A and COPYTest Report Validation Match #3028447A-001 provides additional information - available from the ITS/WH website (www.etsewh.com), the Masonite website (www.masonite.com) or the Masonite technical center

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+40.5/-40.5
Limited water unless special threshold design is used

Large Missile Impact Resistance
Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location is determined by ASCE 7 national code or local building codes specify the edition required.

MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:
1/4 GLASS:


100 Series


133 135 Series


135 Series


680 Series


822 Series

1/2 GLASS:


105 Series*

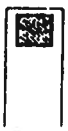

105, 180 Series*


129 Series*


200 Series*


12 R/L, 23 R/L, 24 R/L Series*


107 Series*


108 Series


304 Series

*This glass lite may also be used in the following door styles: 3-panel; 5-panel with scroll; Eyebrow 6-panel; Eyebrow 6-panel with scroll.

Johnson EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product data subject to change without notice.

PREMDOR Collection Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

X
Glazed Inswing Unit

COP-WL-JH4141-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:

3/4 GLASS:


404 Series


410 Series


450 Series

FULL GLASS:


109 Series


114, 120, 122 Series


152 Series


149 Series


300 Series

CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12, NCTL 210-1861-4, 5, 6, 10, 11, 12, NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip lite surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt L Balthazor

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Test Data Review Certificate #3028447A and COP/Test Report Validation Matrix #3028447A-001 provides additional information - available from the ITS/WTH website (www.citadelwtc.com), the Masonite website (www.masonite.com) or the Masonite technical center.

Johnson EntrySystems

June 17, 2004
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Collection

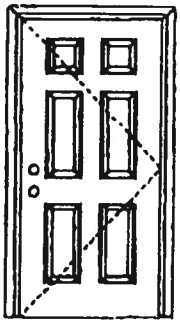
Exclusively from Masonite®
Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-JH4101-02

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Warnock Hersey

Test Data Review Certificate #3025447A and COP/Test Report Validation Matrix #3025447A-001 provides additional information - available from the ITS/www website (www.etsamko.com), the Masonite website (www.masonite.com) or the Masonite technical center

Note:
Units of other sizes are covered by this report as long as the panel used does not exceed 3'0" x 6'8".

Single Door
Maximum unit size = 3'0" x 6'8"

Design Pressure
+66.0/-66.0
Limited water unless special threshold design is used.

Large Missile Impact Resistance
Hurricane protective system (shutters) is NOT REQUIRED.

Actual design pressure and impact resistant requirements for a specific building design and geographic location to determined by ASCE 7-national, state or local building codes specify the edition required

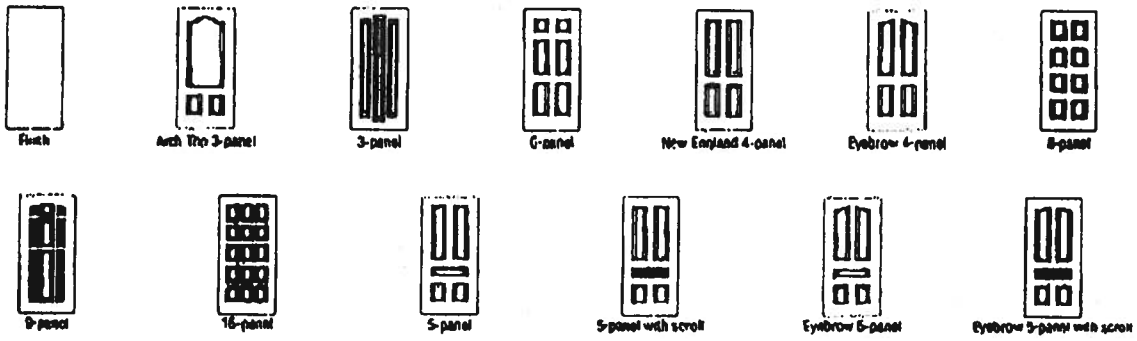
MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed – see MAD-WL-MA0001-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed – see MID-WL-MA0001-02.

APPROVED DOOR STYLES:



Johnson EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

PREMDOR Premium Quality Doors

Exclusively from Masonite®
Masonite International Corporation

X
Opaque Inswing Unit

COP-WL-JHJ101-02

WOOD-EDGE STEEL DOORS

CERTIFIED TEST REPORTS:

NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry O. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA201, PA202 and PA203.

Door panels constructed from 26-gauge 0.017" thick steel skins. Both stiles constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN ACCORDANCE WITH
MIAMI-DADE BCCO
PA201, PA202 & PA203

COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural tests and inspections).

Kurt L Balthy

State of Florida, Professional Engineer
Kurt Balthazor, P.E. – License Number 56533

Test Data Review Certificate #3026447A and COP/Test Report Validation Matrix #3026447A-001 provides additional information - available from the ITS/WH website (www.qualanod.com) the Masonite website (www.masonite.com) or the Masonite technical center

Johnson EntrySystems

June 17, 2002
Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

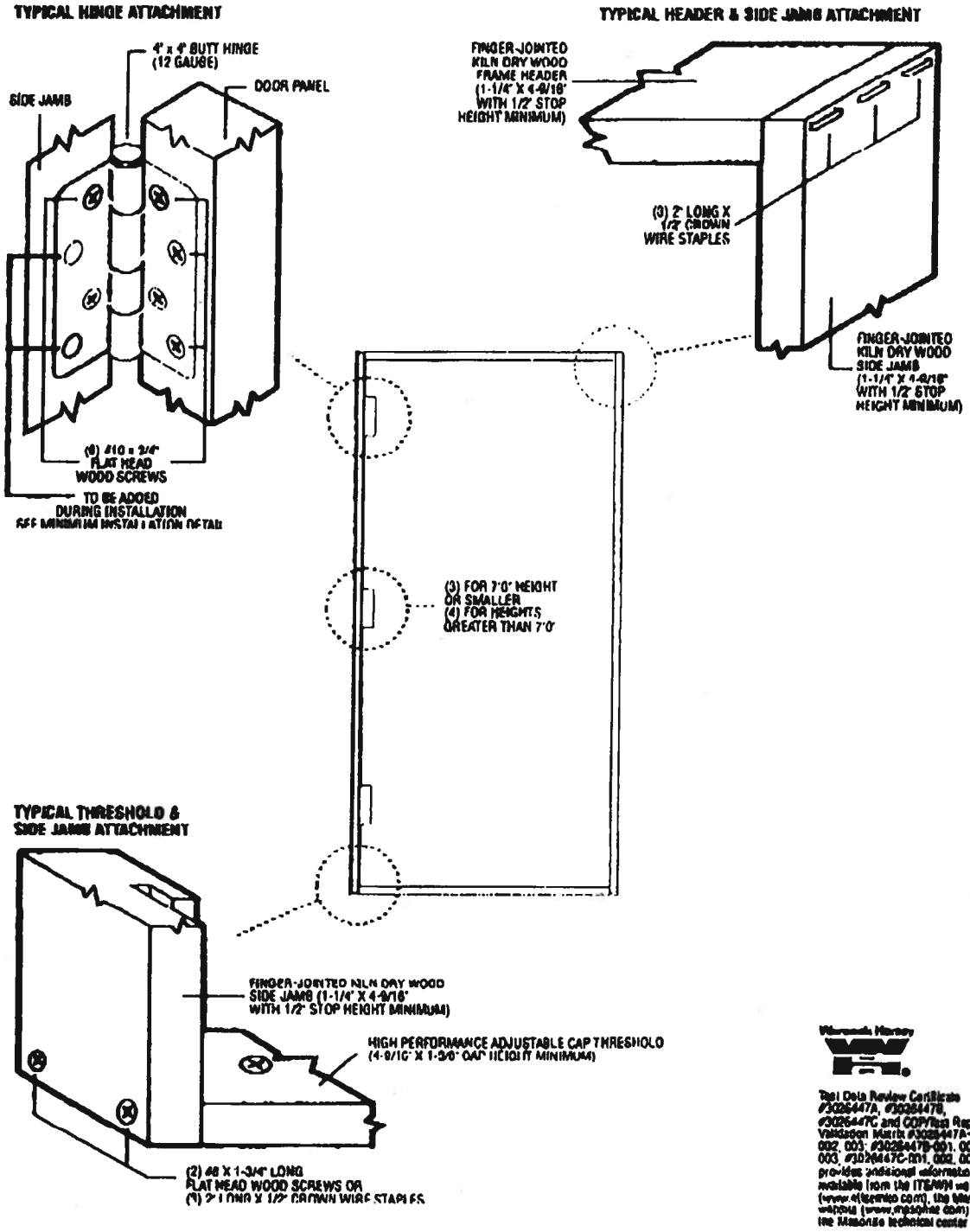
PREMDOR Collection

Exclusively from Masonite®
Masonite International Corporation

X
Unit

MAD-WL-MA0001-02

INSWING UNIT WITH SINGLE DOOR



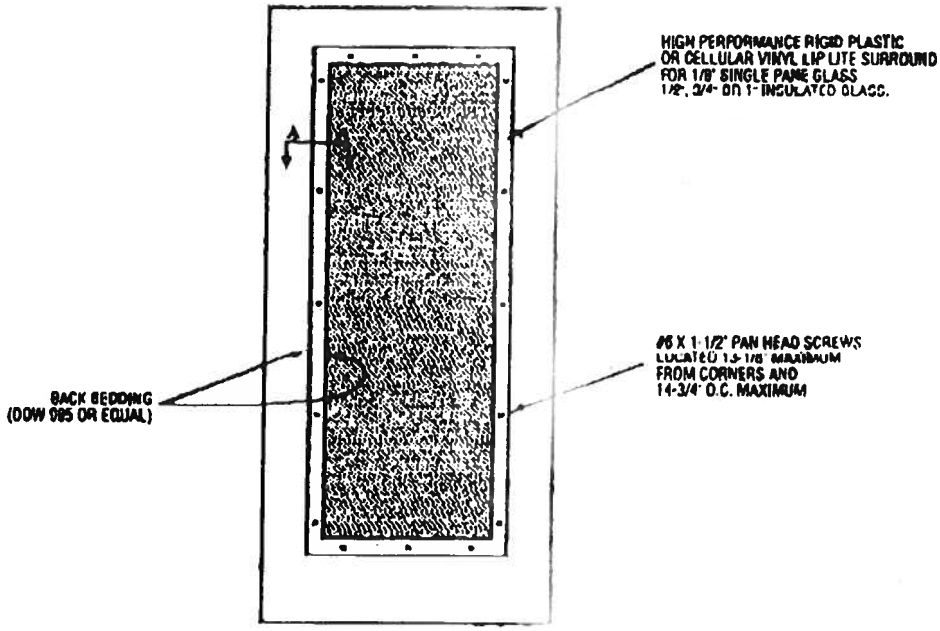
Test Data Review Certificate #3026447A, #3026447B, #3026447C and COPYRIGHT Report Validation Mark #3026447A-001, 002, 003; #3026447B-001, 002, 003, #3026447C-001, 002, 003 provides additional information - available from the ITEM/M website (www.itemm.com), the Masonite website (www.masonite.com) or the Masonite technical center

PREMDOOR Millennium Premium Quality Doors

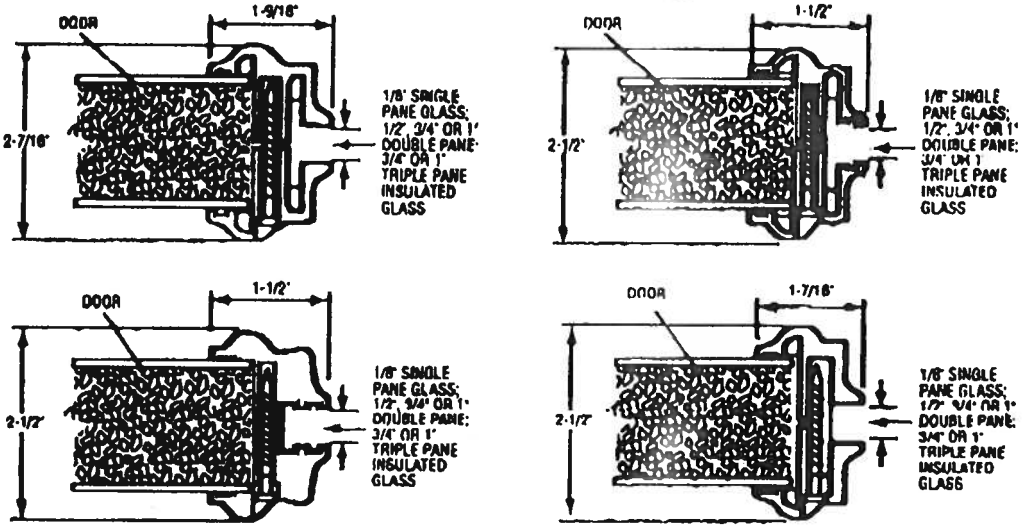
Masonite International Corporation

MAD-WL-MA0041 02

GLASS INSERT IN DOOR OR SIDELITE PANEL



SECTION A-A
TYPICAL RIGID PLASTIC LIP LITE SURROUND



*Glass inserts to be sub-listed by Intertek Testing Services/ETL Semko or approved validation service.

Test Data Review Certificate #30284478, #30284478, #30284478 and CQP/Test Report Validation Matrix #30284478-001, 002, 003; #30284478-001, 002, 003; #30284478-001, 002, 003 provides additional information - available from the ITS/ETL website (www.etlsemko.com), the Masonite website (www.masonite.com) or the Masonite technical center.

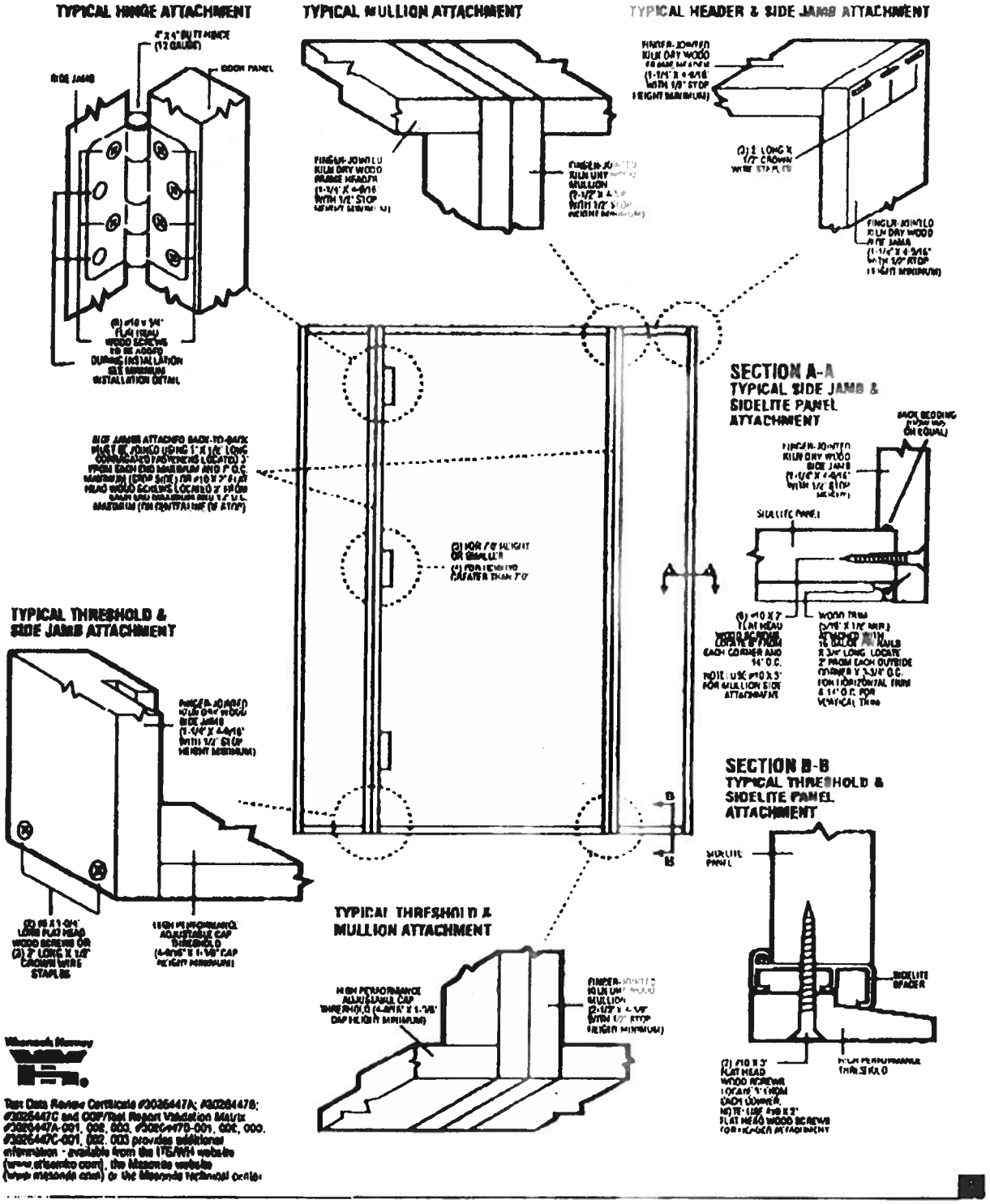
June 17, 2002
For continuing program of product improvement, unless specific revision, design and product detail subject to change without notice.

Exclusively from Masonite
Masonite International Corporation

OXO
Unit

MAD·WL·MA0007·02

INSWING UNIT WITH SINGLE DOOR & TWO SIDELITES (BOXED CONSTRUCTION)



Test Data Review Certificate #30264478; #30264478; #30264478C and COF/Test Report Validation Matrix #30264478A-001, 002, 003, #30264478D-001, 002, 003, #30264478C-001, 002, 003 provides additional information - available from the ITS/WH website (www.atsenter.com), the Masonite website (www.masonite.com) or the Masonite technical center.

June 17, 2002

Our continuing program of product improvement makes specifications, design and product detail subject to change without notice.

Exclusively from Masonite
Masonite International Corporation

CAPITOL
WINDOWS AND DOORS

- **Series** 165 Single Hung and **Fixed Windows**
- **Series** 650 Single Hung and **Fixed Windows**
- **Series 168 Horizontal Slider and Fixed Windows**
- **Series** 680 Horizontal Slider and **Fixed Windows**

NOTE: SEE INDIVIDUAL TEST REPORT(S) FOR DP RATINGS AND MAXIMUM ALLOWABLE SIZES.

INSTALLATION INSTRUCTIONS FOR "APPROVED FOR FLORIDA" ALUMINUM FIN WINDOWS

Capitol Windows & Doors appreciates your recent purchase of a maintenance free prime window, which will not rust, rot, mildew, or warp. This is a quality product that left our factory in good condition – proper handling and installation are just as important as good design and workmanship. Please follow these recommendations to allow this product to complete its function.

1. Handle units one at a time in the closed and locked position and take care not to scratch frame or glass or to bend the nailing fin. Place a continuous bead of caulk on the back side of nail fin (mounting flange).
2. Set unit plumb and square into opening and make sure that there is 3/16" ± 1/16" clearance around the frame. Fasten unit into opening in the closed and locked position, making sure that fasteners are screwed in straight in order to avoid twisting or bowing of the frame. Make sure that sill is straight and level. Check operation of unit frequently as fasteners are set.
3. Use # 8 sheet metal or wood screws with a minimum of 1" penetration into the framing (stud). Place first screws (two at each corner) 3" from end of fin. For positive and negative DPs (design pressures) up to 35, do not exceed 24" spacing of additional screws. For DPs from 35.1 to 50, do not exceed 18" spacing.
4. Caulk entire perimeter of fin to mounting surface joint and caulk over screw heads.
   Note: this step can be eliminated if 4" wide adhesive type flashing is used (sill 1st., jambs 2nd., head 3rd.).
5. Fill voids between frame and construction with loose batten type insulation or non-expanding aerosol foam specifically formulated for windows and doors to eliminate drafts. The use of expanding aerosol type insulating foam, which can bow the frame, waives all stated warranties.
6. Remove plaster, mortar, paint, and debris that has collected on the unit and make sure that sash/vent tracks and interlocks are also clean. Do not use abrasives, solvents, ammonia, vinegar, alkaline, or acid solutions for clean-up, especially with insulated glass units as their use could cause chemical breakdown of the glass seal. Take care not to scratch glass; scratches severely weaken glass and it could eventually break from thermal expansion and contraction. Clean units with water and mild detergent.

- CAUTION -

Capitol Windows & Doors or its representatives are unable to control and cannot assume responsibility for the selection and placement of their products in a building or structure in a manner required by laws, statutes, and/or building codes. The purchaser is solely responsible for knowledge of and adherence to the same. BetterBilt window products are not provided with safety glazing unless specifically ordered with such. Many laws and codes require safety glazing (tempered glass) near doors, bathtubs, and shower enclosures. Also be aware of other code requirements such as emergency egress and structural / energy performance.

Corporate Headquarters:
M.I. Home Products
650 West Market St.
Gratz, PA 17030-0370
(717) 365-3300

www.mihp.com

It 221
July 29, 2003

STEVEN M. URICH
CERTIFICATE
No. 57795
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Rev. 7-24-03

AAMA/NWWDA 101/I.S
TEST REPORT SUMM

Rendered to:

MI HOME PRODUCTS,

SERIES/MODEL: 650
TYPE: Aluminum Single Hun low

Title of Test	s
Rating	H- x 72
Overall Design Pressure	f
Operating Force	
Air Infiltration	0.1
Water Resistance	
Structural Test Pressure	+67.5 psf -70.8 psf
Deglazing	d
Forced Entry Resistance	0

Reference should be made to Report No. 01-41134.01 dated 2 for complete test specimen **description and data.**

For ARCHITECTURAL TESTING, INC.

Mark A. Hess

Mark A. Hess, Technician

MAH:nlb

REEVES
No. 15354
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

2002

Architectural

AAMA/NWWDA 101/I.S.2-97 T PORT

Rendered to:

MI HOME PRODUCTS,
650 West Market Stre
P.O. Box 370
Gratz, Pennsylvania 1703

Report No: 01- 134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was c ted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single window at their facility located in Elizabethville, Pennsylvania. The samples tested ssfully met the perf mance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in dance with AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4' 4-1/4" wide by 6' 0-3/8" high

Active Sash Size: 4' 1-3/4" wide by 3' 0-5/8" high

Daylight Opening Size: 3' 11-3/8" wide by 2' 9-1/2" hi

Screen Size: 4' 0-1/4" wide by 2' 11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized hick, sealed insulating glass constructed from two sheets of 1/8" thick, clear anneal and a metal reinforced butyl spacer system. The active sash was channel glazed ut a flexible vinyl wrap around gasket. The fixed lite was interior glazed against do ded adhesive foam tape and secured with PVC snap-in glazing beads.

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

REEVES NO. 10354 STATE OF FLORIDA PROFESSIONAL ENGINEER

P.E. 2002

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	
0.230" high by 0.270" backed polypile with center fin	1 Row	eting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	ash stiles
1/2" x 1/2" dust plug	4 Pieces	sh, top and botto
1/4" foam-filled vinyl bulb seal	1 Row	ash, bottom rail

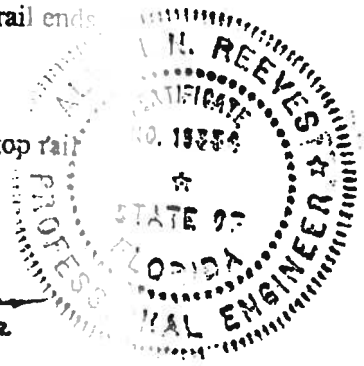
Frame Construction: The frame was constructed [illegible] led aluminum w[illegible]oped, butted, and sealed corners fastened with two #8 x 1" [illegible]rough the head an[illegible]l into each jamb screw boss. End caps were utilized on t[illegible]f the fixed meeti[illegible]l and secured with two 1-1/4" screws per cap. Meeting rail [illegible]ed to the frame uti[illegible]g two 1-1/4" screws.

Sash Construction: The sash was constructed of ex[illegible]luminum with cope[illegible]utted, and sealed corners fastened with two #8 x 1-1/2" scr[illegible]gh the rails into jamb screw boss.

Screen Construction: The screen was constructed fr[illegible]rmed aluminum wi[illegible]eyed corners. The fiberglass mesh was secured with a flexib[illegible].

Hardware:

Description	Quantity	
Metal cam lock with keeper		[illegible], active meeting rail [illegible] [illegible]djacent on fixed meet[illegible] rail
Plastic tilt latch	2	[illegible]sh, meeting rail ends
Metal tilt pin	2	sh, bottom rail ends
Balance assembly	2	[illegible]ch jamb
Screen plunger	2	rail ends on top rail



N. Reeves
2002

Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed in [illegible] 2 Spruce-Pine [illegible] test buck with #8 x 1-5/8" drywall screws every 8" on [illegible] d the nail fi[illegible]ane was used as a sealant under the nail fin and around th[illegible] perimeter.

Test Results:

The results are tabulated as follows

Paragraph	Title of Test - Test Method	
2.2.1.6.1	Operating Force	3[illegible]
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.3 c[illegible]

***Note #1:** The tested specimen meets the performa[illegible]s specified in A[illegible]M[illegible]DA 101/I.S. 2-97 for air infiltration.*

	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf	No [illegible]
2.1.4.1	Uniform Load Deflection (ASTM E [illegible] (Measurements reported were taken [illegible]e ing rail) (Loads were held for 33 seconds) @ 25.9 psf (positive) @ 34.7 psf (negative)	0[illegible] 0[illegible]

***Exceeds L/175 for deflection, but passes all other test [illegible]ments.*

2.1.4.2	Uniform Load Structural (ASTM E 3[illegible] (Measurements reported were taken o[illegible]ing rail) (Loads were held for 10 seconds) @ 38.9 psf (positive) @ 52.1 psf (negative)	0[illegible] 0.1[illegible]

[illegible] Reeves
2002

[illegible] REEVES ★ [illegible]ICATE [illegible] ★ [illegible]TE OF [illegible]ORIDA [illegible]AL ENGINEER ★

Test Specimen Description: (Continued)

Paragraph	Title of Test - Test Method	
2.2.1.6.2	Deglazing Test (ASTM E 987) In operating direction at 70 lbs	
	Meeting rail	0
	Bottom rail	0
	In remaining direction at 50 lbs	
	Left stile	0
	Right stile	0
	Forced Entry Resistance (ASTM	
	Type: A Grade: 10	
	Lock Manipulation Test	
	Tests A1 through A5	
	Test A7	
	Lock Manipulation Test	
Optional Performance		
4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 6.00 psf	No
	Uniform Load Deflection (ASTM (Measurements reported were taken g rail) (Loads were held for 33 seconds)	
	@ 45.0 psf (positive)	0
	@ 47.2 psf (negative)	0

**Exceeds L/175 for deflection, but passes all other requirements.*

	Uniform Load Structural (ASTM E (Measurements reported were taken g rail) (Loads were held for 10 seconds)	
	@ 67.5 psf (positive)	0.1
	@ 70.8 psf (negative)	0.

M. Reeves
2002

REEVES
19554
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Detailed drawings, representative samples of the test [illegible] **and a copy of** [illegible] **be retained by ATI for a period of four years. The** [illegible] **results were secu** [illegible] **the designated test methods and they indicate compliance** [illegible] **performance re** [illegible] **the above referenced specification. This report does** [illegible] **certification** [illegible] **uct, which may only be granted by the certification pro** [illegible] **tor.**

For ARCHITECTURAL TESTING, INC:

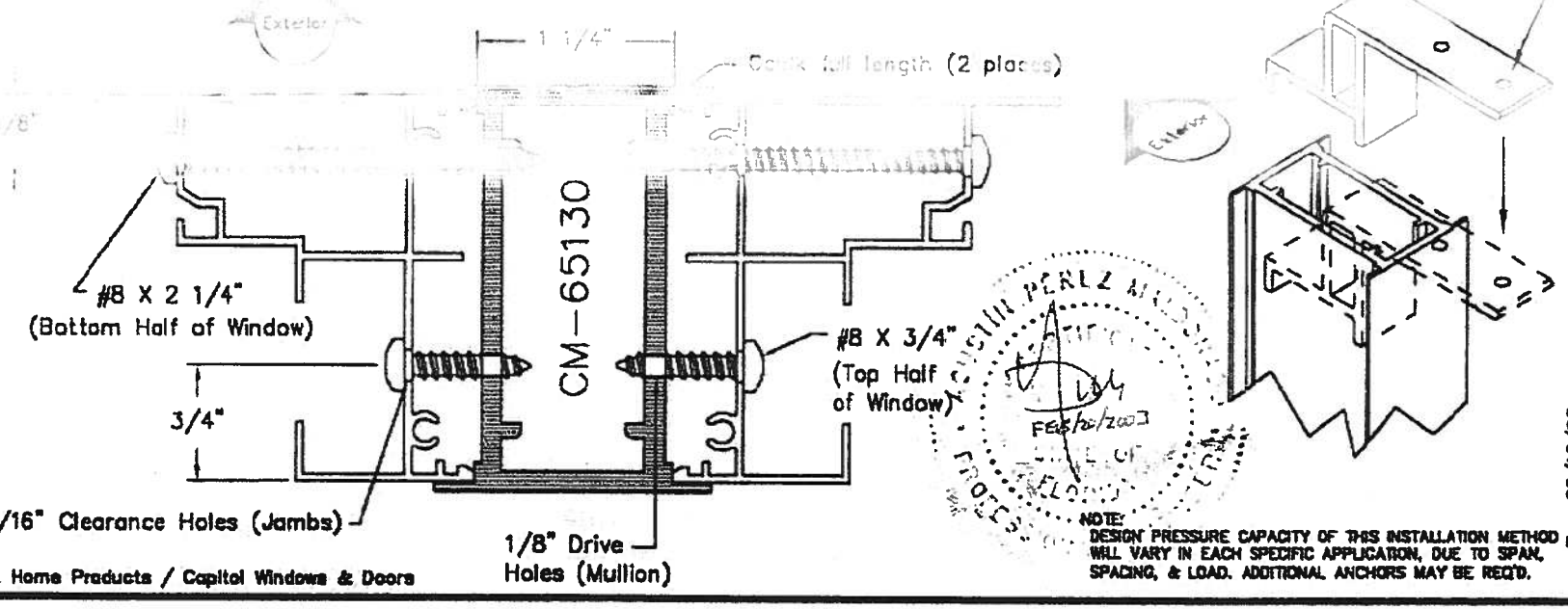
Mark A. Hess
Technician

[illegible] M. Reeves
[illegible] ceves, P.E.
[illegible] - Engineering Se
[illegible] 2002

MAH:nlb
01-41134.01

STRUCTURAL VERTICAL MULLION – NAIL FIN type 650 SH / PW

MULLION PART # CM-65130



M.I. Home Products / Capitol Windows & Doors

Rev. 02/18/03

650 S

L - FIN

MI HOME PRODUCTS

VERTICAL MULLION DESIGN L
FOR ALUMINUM TUBE MULLIO

WDW. WIDTH > MULLION SPAN V	19.125	36.000	53.125
36.000	576	450	450
37.375	528	402	402
48.000	291	198	186
50.625	245	164	150
60.000	142	91	77
63.000	122	77	64
72.000	80	49	39
72.250	79	48	39

CHART APPLIES ONLY TO EXTRUDED A 5130)
READ WINDOW WIDTH AND HEIGHT IN
DESIGN PRESSURE VALUES ON THIS C
WINDOW WIDTH DIMENSIONS REPRESE **OW IN A SI** **L WID**
THE OPENING.
DESIGN PRESSURE VALUES SHOWN ON **TED T** **E OPENI**
CAPACITIES APPLY TO ANY NUMBER O **IG, PR** **ON SPA**
ARE NOT EXCEEDED.
Dmax = L / 175
INSTALLATION OF MULLION: MULLION M **TE. C** **O**
ADEQUATELY TRANSFER LOAD TO THE **ER'S MU**

PREPARED BY:
PRODUCT TECHNOLOGY CORPORATIO
250 INTERNATIONAL PARKWAY
SUITE 250
HEATHROW, FLORIDA 32746
PHONE 407 805-0365 / FAX 407 805-0366

21, 02
02-0450

Division • 7252 Narcoo
Fax (407) 384-7751
www.ctlarch.com
@ctlarch.com

CTLA-744W
November 15, 20

STRUCTURAL

M HOME PRODUC
650 WEST MARKE
P.O. BOX 370 GRATZ

and Series: Model
x 80")

Class

Test Specification: AAMA/NWWDA
(PVC) Wood W

cificati

The extruded alum
and butted corner
SMS each corner

3.75"
(2) #8

Operation:

OXO as viewed fr

Two (2) fixed pan
78.5" One (1) astra
(1) fixed panel righ
each measured 45"
(1) #8x .75" Pan h

center,
measur
78.5" C
r const
orner.

Weatherstripping:

Quantity	Locati
Three (3) strips	Each F
	right ja
Two (2) strips	Active
Two (2) strips	Inactiv
One (1) strip	Thresh

Hardware & Location:

Quantity	
One (1)	
One (1)	
One (1)	
One (1)	

Page 2 of 3 M. I. Home
Report # CTL-773

Glazing: 5/32" Clea... glaz... glazing g...

Sealant: A silicone ... in fr... ck (interior, ... type frame co...

Weep System: Two (2) we... 50" fr...

Reinforcements: One (1) alu... left stile. Sec... alumin... L ... head secured ... pan

Additional Description:

Installation: Head: Fou... ... 2) para... n lef... 73", Sill: Seve... P.I., ... Mea... mb 5.7. ...70", 5 ... 5" Jambs: Eleh... ... were u... rows of fa... 5", 3...

Surface/Finish: Mill

Comments: Nominal 2 mi... gai... structural loa... t di... t results

Report No.	Title of Test	Te...
2.1.2	Air Infiltration @ 1.57 psf. This test specimen etc... Results reflected in two (	24 c... AA... W... -9
2.1.3/4.3	Water Resistance 5.0gph/ft	No E...
	WTP= 5.25 psf	o F...
	Unit tested with and wi...	
2.1.3/4.3	Water Resistance 5.0gph/ft	No E...
	WTP= 6 psf	... E...
	Unit tested with and wi...	

APPLIED
P. O. BOX
HIALEAH, FL 3301
TEL. (305)
FAX. (305)

CLIENT:
FILE NO.
JOB NO.
DATE:
SERIES:

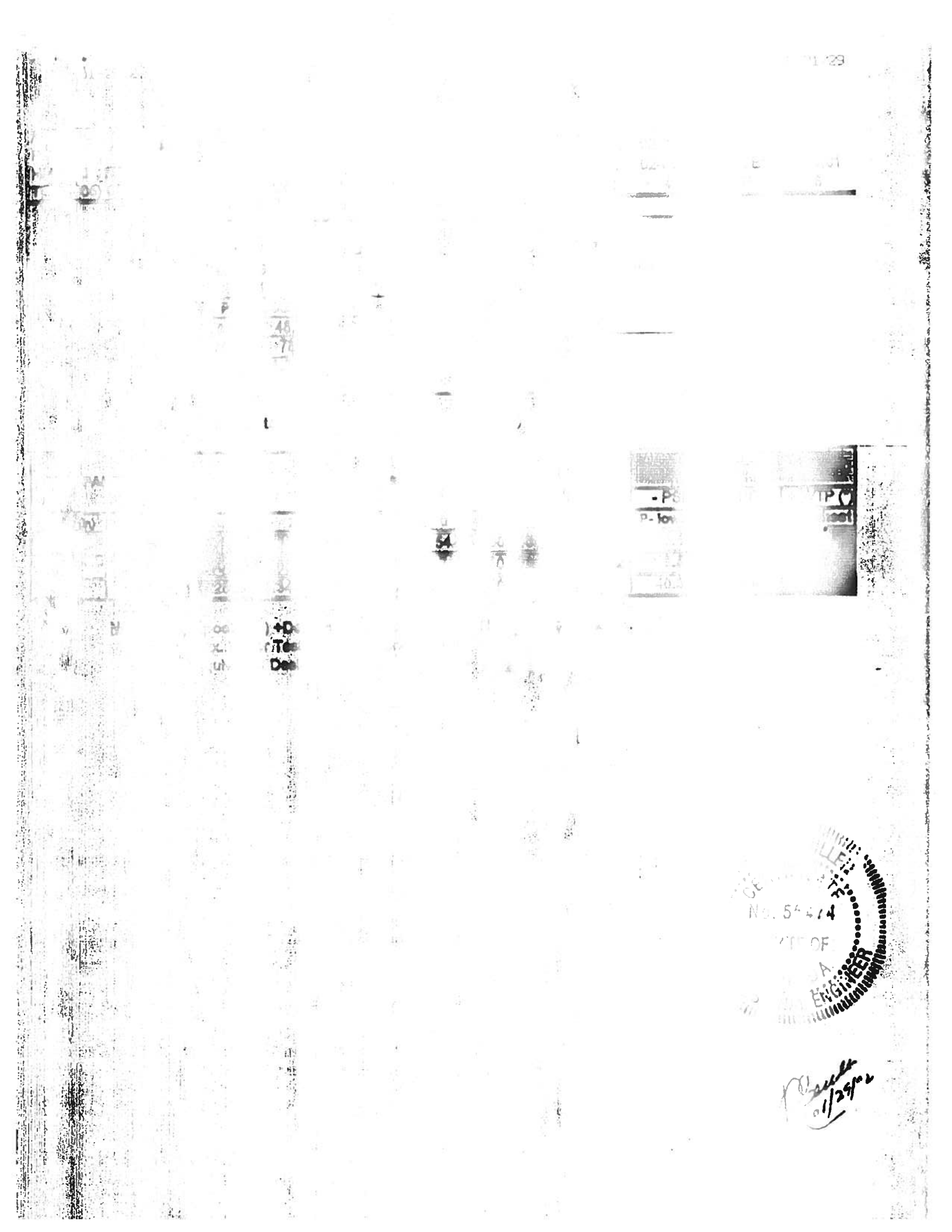
APPLIE N SEP
P. O. BOX
HIALEAH, F
TEL. (305) 8
FAX. (305) 8

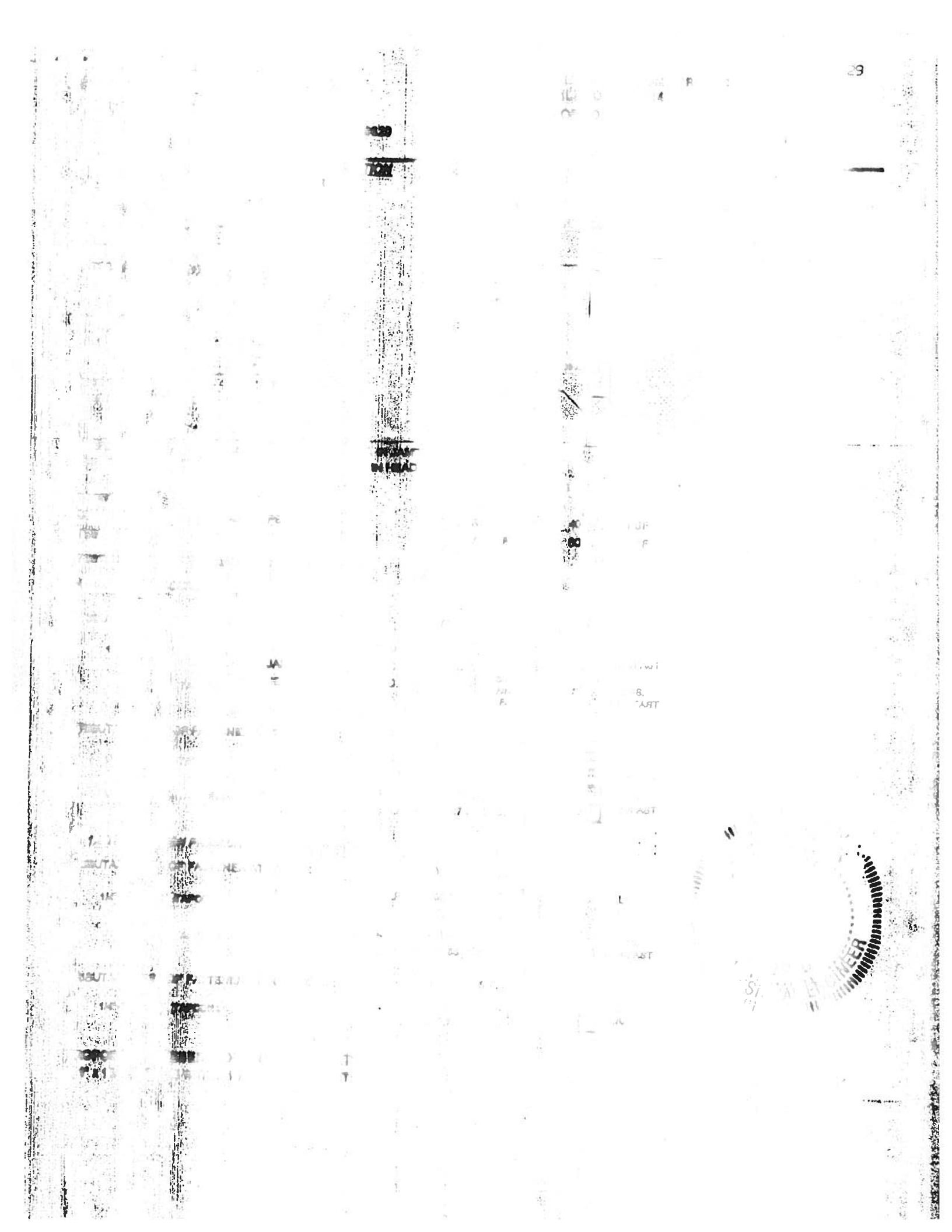
CLIEN S
FILE NO.
JOB NO.
DATE:
SERIE

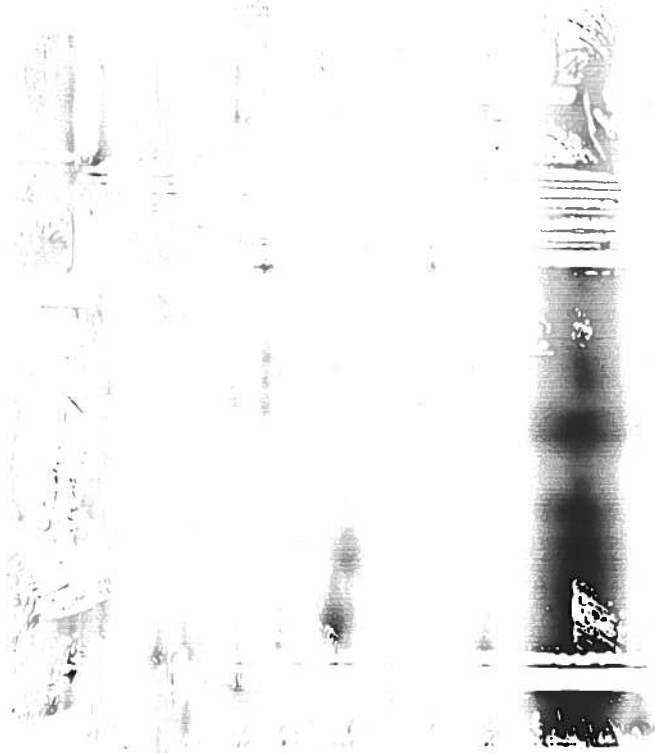
1.0

1.0


CERTIFICATE
5474
ENGINEER

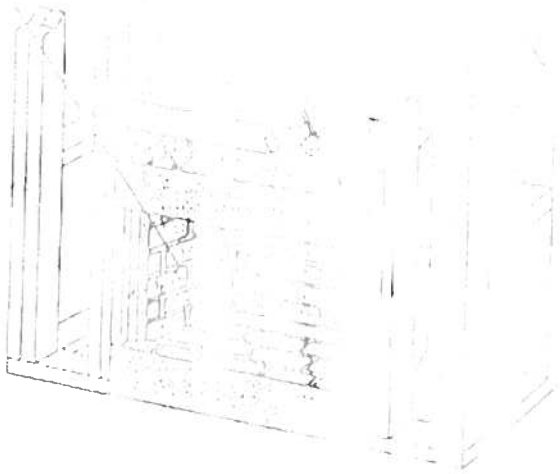






FRO

Real
interior



Distributed by:

DRAWING NUMBER

A10560

DATE 8-06-90

TO CONSTRUCT

BRACKET TO FOUR

ALL

PF

GIVEN TO

HURRICANE POSTS.

EVAR II

DATE	BY	DESCRIPTION

3. SECTION HEIGHTS OF 21.00" AND 19.50" ARE AVAILABLE AND MAY BE USED IN ANY COMBINATION TO ACHIEVE VARIOUS DOOR HEIGHTS.

4. WINDOWS MAY BE INSTALLED IN THE TOP SECTION. (AS TESTED WITH 1/8" DSB GLASS OR EQUIVALENT) OR IN THE SECTION IMMEDIATELY BELOW THE TOP SECTION.

TEK SCREWS.

8. QUANTITY OF SIDE LOCKS CAN BE 0,1, OR (2 AS TESTED).

△9 9. DROP IN TYPE OF INSULATION IS OPTIONAL

RIGHT-J LOAD AND EQUIPMENT SUMMARY
Entire House

Touchstone Heating and Air, Inc.
490 SE 3rd Ave., Lake Butler, FL 32054 Phone: 386-496-3467 Fax: 386-496-3147

Job: Ellis & Carole Noll
02/15/06

Project Information

For: Blake Construction
291 S.W. Sisters Welcome Road #102, Lake City, FL 32025
Phone: 386-754-5810 Fax: 386-719-6708

Notes:

Design Information

Weather: Gainesville, FL , US

Winter Design Conditions

Outside db	33	°F
Inside db	70	°F
Design TD	37	°F

Summer Design Conditions

Outside db	92	°F
Inside db	75	°F
Design TD	17	°F
Daily range	M	
Relative humidity	50	%
Moisture difference	52	gr/lb

Heating Summary

Building heat loss	37804	Btuh
Ventilation air	0	cfm
Ventilation air loss	0	Btuh
Design heat load	37804	Btuh

Sensible Cooling Equipment Load Sizing

Structure	35239	Btuh
Ventilation	935	Btuh
Design temperature swing	3.0	°F
Use mfg. data	n	
Rate/swing multiplier	0.97	
Total sens. equip. load	35089	Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	1

	Heating	Cooling
Area (ft²)	***2625***	***2625***
Volume (ft³)	***22313***	***22313***
Air changes/hour	***0.10***	***0.30***
Equiv. AVF (cfm)	37	112

Latent Cooling Equipment Load Sizing

Internal gains	2990	Btuh
Ventilation	1753	Btuh
Infiltration	3911	Btuh
Total latent equip. load	8654	Btuh
Total equipment load	43743	Btuh
Req. total capacity at 0.70% SHR	4.2	ton

Heating Equipment Summary

Make Trane
Trade E. XE 1200 Weathertron
TWP048C

Efficiency	8.3	HSPF
Heating input		
Heating output	45500	Btuh @ 47°F
Heating temp rise	25	°F
Actual heating fan	1667	cfm
Heating air flow factor	0.044	cfm/Btuh

Space thermostat

Cooling Equipment Summary

Make Trane
Trade E. XE 1200 Weathertron
TWP048C
TWE049E13

Efficiency	13.0	SEER
Sensible cooling	35000	Btuh
Latent cooling	15000	Btuh
Total cooling	50000	Btuh
Actual cooling fan	1667	cfm
Cooling air flow factor	0.047	cfm/Btuh
Load sensible heat ratio	81	%

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 7th Ed.

RIGHT-J LOAD AND EQUIPMENT SUMMARY
Entire House

Touchstone Heating and Air, Inc.
490 SE 3rd Ave., Lake Butler, FL 32054 Phone: 386-496-3467 Fax: 386-496-3147

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Actual cooling fan	1667	cfm
Cooling air flow factor	0.047	cfm/Btuh
Load sensible heat ratio	81	%

Bold/Italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 7th Ed.

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 28-3S-16-02372-633 **Building permit No.** 000024170

Use Classification SFD,UTILITY **Fire:** 5.92

Permit Holder BLAKE LUNDE **Waste:** 12.25

Owner of Building ELLIS & CAROLE NOLL **Total:** 18.17

Location: 784 NW ZACK DR, LAKE CITY, FL

Date: 09/28/2006

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

Project Information for:	L145800		
Builder:	BLAKE CONST.	Date:	1/11/2006
Lot:	LOT 31 ARBOR GREEN	Start Number:	**1329**
Subdivision:	N/A		
County or City:	COLUMBIA COUNTY		
Truss Page Count:	52		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	42	Wind Standard:	ASCE 7-02		
Floor (psf):	55	Wind Speed (mph):	110		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

LUNDE, BLAKE N II RR 0067618
Address: 2250 SW JAGUAR DR
LAKE CITY, FL. 32025 Designer: 73

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	111061329	1/11/2006	41	T24	111061369	1/11/2006
2	CJ3	111061330	1/11/2006	42	T25	111061370	1/11/2006
3	CJ5	111061331	1/11/2006	43	T26	111061371	1/11/2006
4	EJ5	111061332	1/11/2006	44	T27	111061372	1/11/2006
5	EJ7	111061333	1/11/2006	45	T28	111061373	1/11/2006
6	EJ7A	111061334	1/11/2006	46	T29	111061374	1/11/2006
7	EJ7B	111061335	1/11/2006	47	T30	111061375	1/11/2006
8	EJ7C	111061336	1/11/2006	48	T31	111061376	1/11/2006
9	EJ7T	111061337	1/11/2006	49	T32	111061377	1/11/2006
10	EJ7V	111061338	1/11/2006	50	T33	111061378	1/11/2006
11	EJ7VA	111061339	1/11/2006	51	T34	111061379	1/11/2006
12	HJ7	111061340	1/11/2006	52	T35	111061380	1/11/2006
13	HJ9	111061341	1/11/2006				
14	T01	111061342	1/11/2006				
15	T01G	111061343	1/11/2006				
16	T02	111061344	1/11/2006				
17	T03	111061345	1/11/2006				
18	T04	111061346	1/11/2006				
19	T05	111061347	1/11/2006				
20	T06	111061348	1/11/2006				
21	T06G	111061349	1/11/2006				
22	T07	111061350	1/11/2006				
23	T07A	111061351	1/11/2006				
24	T08	111061352	1/11/2006				
25	T08G	111061353	1/11/2006				
26	T09	111061354	1/11/2006				
27	T10	111061355	1/11/2006				
28	T11	111061356	1/11/2006				
29	T12	111061357	1/11/2006				
30	T13	111061358	1/11/2006				
31	T14	111061359	1/11/2006				
32	T15	111061360	1/11/2006				
33	T16	111061361	1/11/2006				
34	T17	111061362	1/11/2006				
35	T18	111061363	1/11/2006				
36	T19	111061364	1/11/2006				
37	T20	111061365	1/11/2006				
38	T21	111061366	1/11/2006				
39	T22	111061367	1/11/2006				
40	T23	111061368	1/11/2006				

JAN 1 1 2006

Licensee Details

Licensee Information

Name:	**LUNDE, BLAKE N II (Primary Name)**
	BLAKE CONSTRUCTION (DBA Name)
Main Address:	**872 SW JAGUAR DR**
	LAKE CITY Florida 32025
License Mailing:	
LicenseLocation:	**2250 SW JAGUAR DR**
	LAKE CITY FL 32025

License Information

License Type:	**Registered Residential Contractor**
Rank:	**Reg Residential**
License Number:	**RR0067618**
Status:	**Current,Active**
Licensure Date:	**03/13/2001**
Expires:	**08/31/2005**

View Related License Information

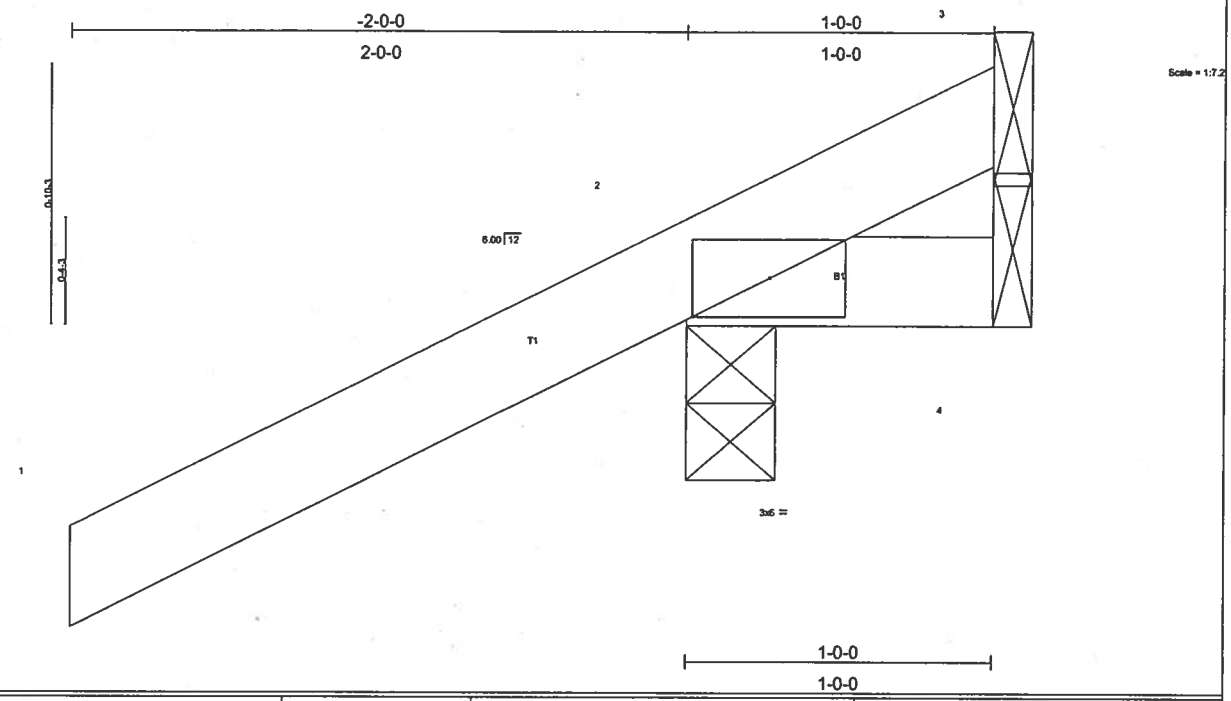
View License Complaint

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061329

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	CJ1	MONO TRUSS	12	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 08:34:48 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/0-3-8, 4=14/Mechanical, 3=-90/Mechanical
Max Horz 2=87(load case 5)
Max Uplift2=-274(load case 5), 3=-90(load case 1)
Max Grav 2=266(load case 1), 4=14(load case 1), 3=127(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/75
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 274 lb uplift at joint 2 and 90 lb uplift at joint 3.

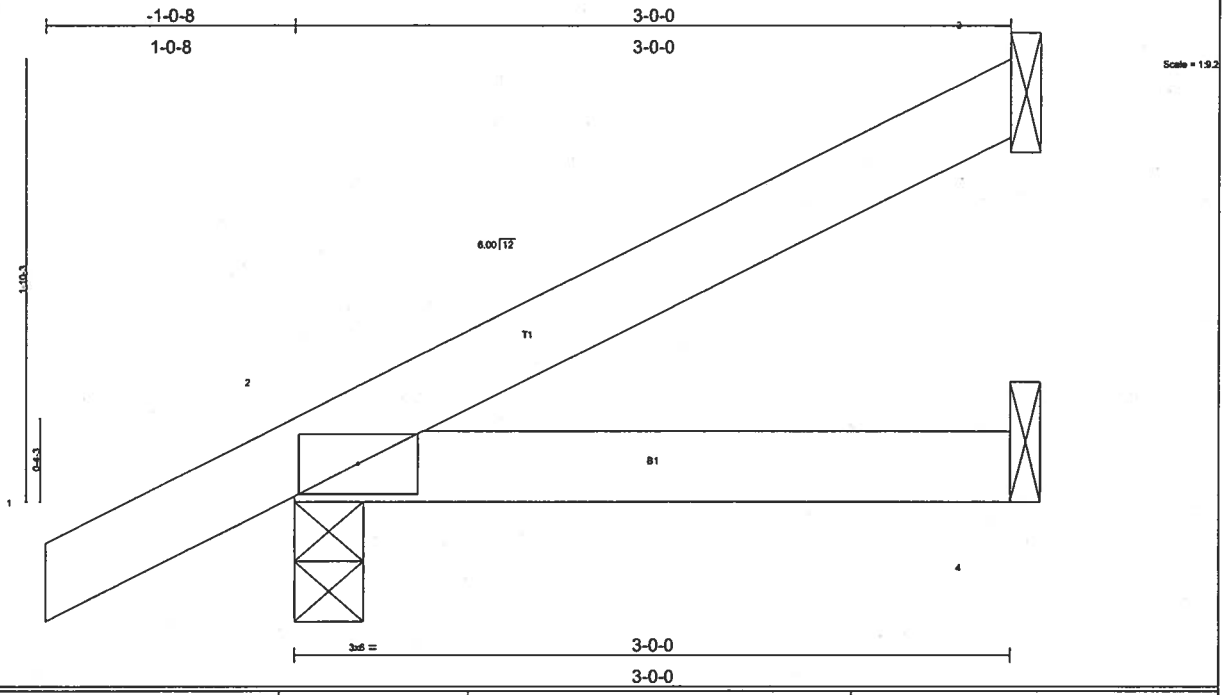
LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061330

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	CJ3	MONO TRUSS	12	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 08:35:07 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.09	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 11 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=62/Mechanical, 2=195/0-3-8, 4=42/Mechanical
Max Horz 2=101(load case 5)
Max Uplift3=-55(load case 5), 2=-109(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/24, 2-3=-55/22
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 55 lb uplift at joint 3 and 109 lb uplift at joint 2.

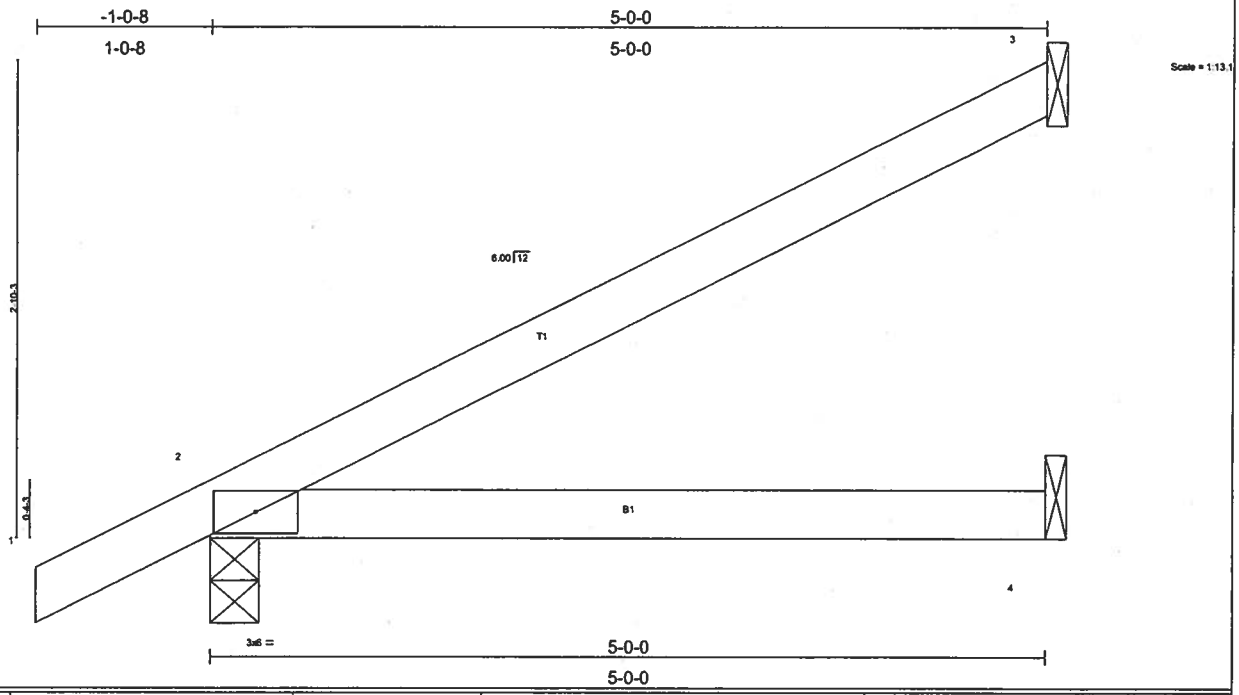
LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061331

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	CJ5	MONO TRUSS	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 08:35:28 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=121/Mechanical, 2=273/0-3-8, 4=72/Mechanical
Max Horz 2=147(load case 5)
Max Uplift3=-111(load case 5), 2=-122(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/24, 2-3=-102/44
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 111 lb uplift at joint 3 and 122 lb uplift at joint 2.

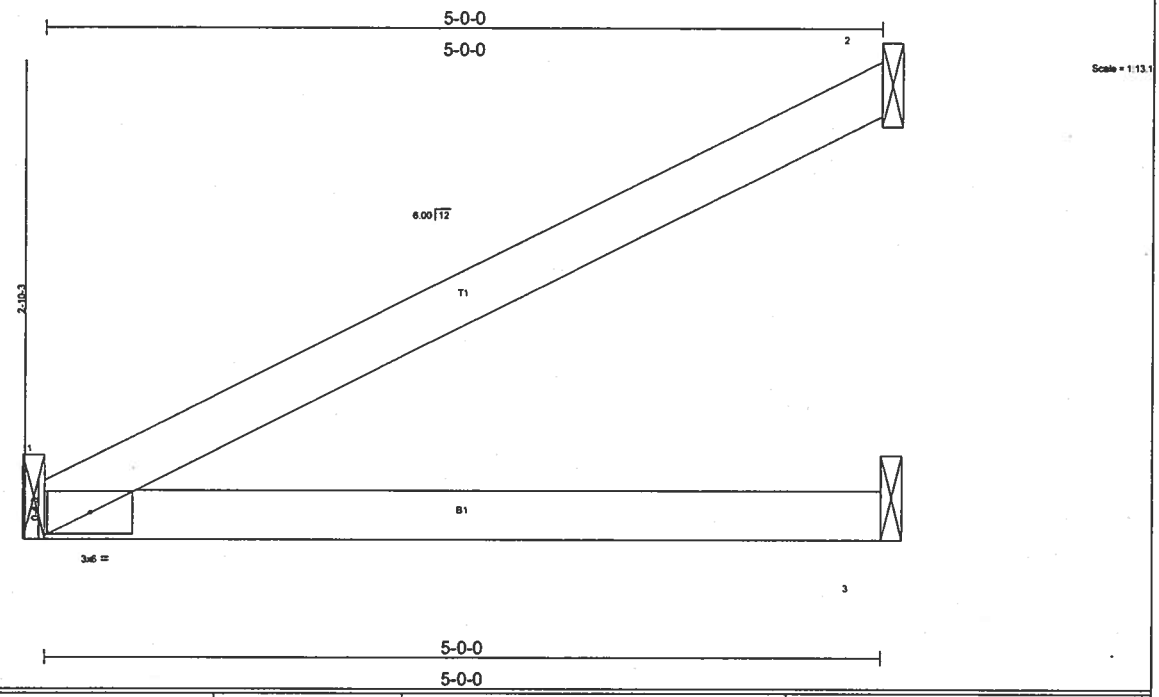
LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061332

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	EJ5	MONO TRUSS	5	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 08:35:46 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.03	1-3	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.05	1-3	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 16 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=205/Mechanical, 2=132/Mechanical, 3=73/Mechanical
Max Horz 1=116(load case 5)
Max Uplift1=-48(load case 5), 2=-122(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-105/48
BOT CHORD 1-3=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 1 and 122 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061333

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	EJ7	MONO TRUSS	27	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 08:36:03 2006 Page 1

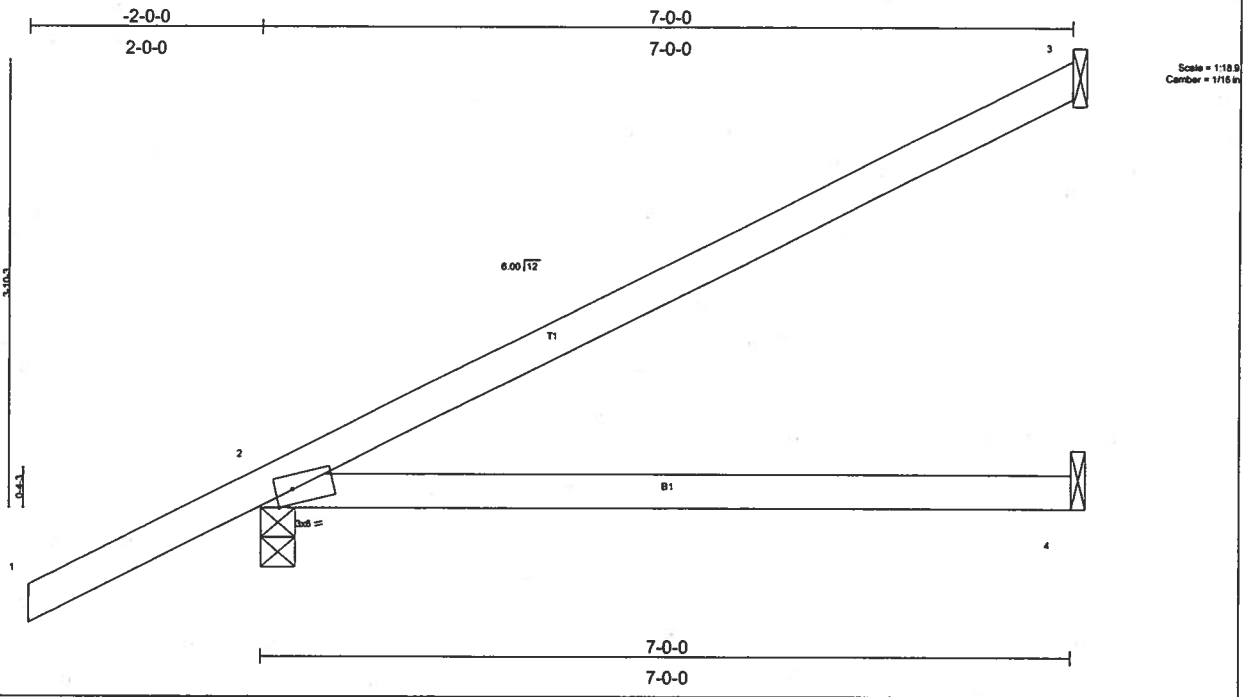


Plate Offsets (X,Y): [2:0-1-12,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.12	2-4	>664	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.21	2-4	>397	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=419/0-3-8, 4=104/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-134(load case 5), 2=-210(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-119/58
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 lb uplift at joint 3 and 210 lb uplift at joint 2.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061334

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	EJ7A	MONO TRUSS	1	1	Job Reference (optional)

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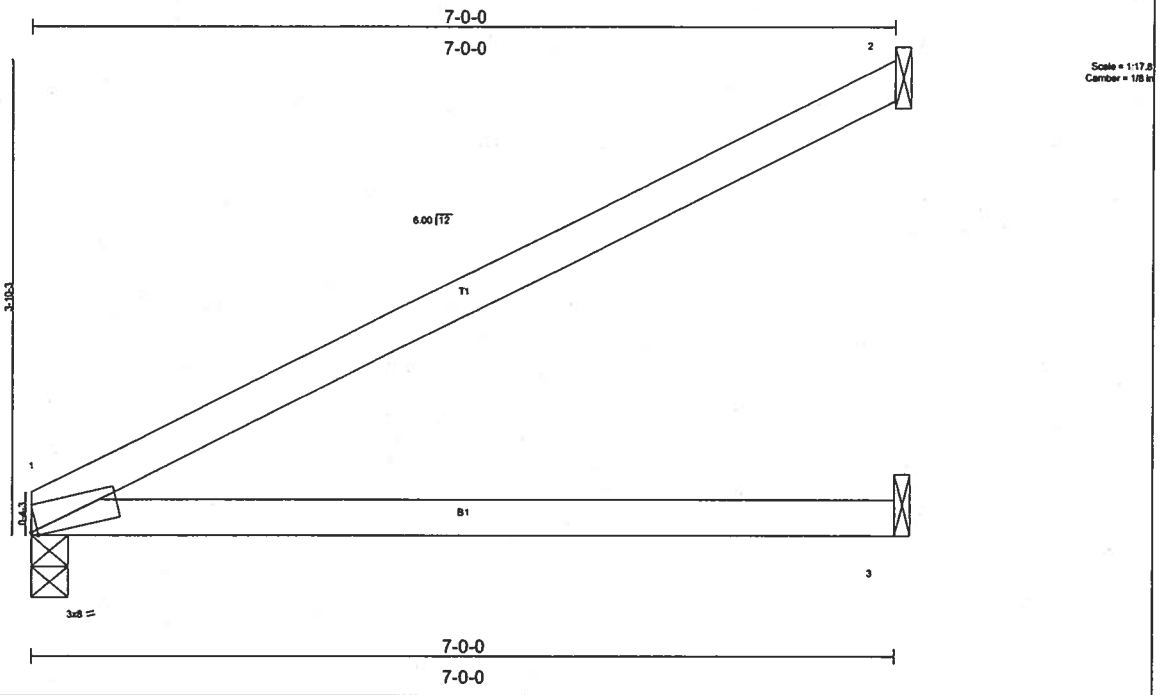

Plate Offsets (X,Y): [1:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.15	1-3	>539	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.25	1-3	>328	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 22 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=285/0-3-8, 2=171/Mechanical, 3=114/Mechanical
Max Horz 1=162(load case 5)
Max Uplift1=-66(load case 5), 2=-145(load case 5), 3=-5(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-126/62
BOT CHORD 1-3=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 66 lb uplift at joint 1, 145 lb uplift at joint 2 and 5 lb uplift at joint 3.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061335

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	EJ7B	MONO TRUSS	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:12:58 2006 Page 1

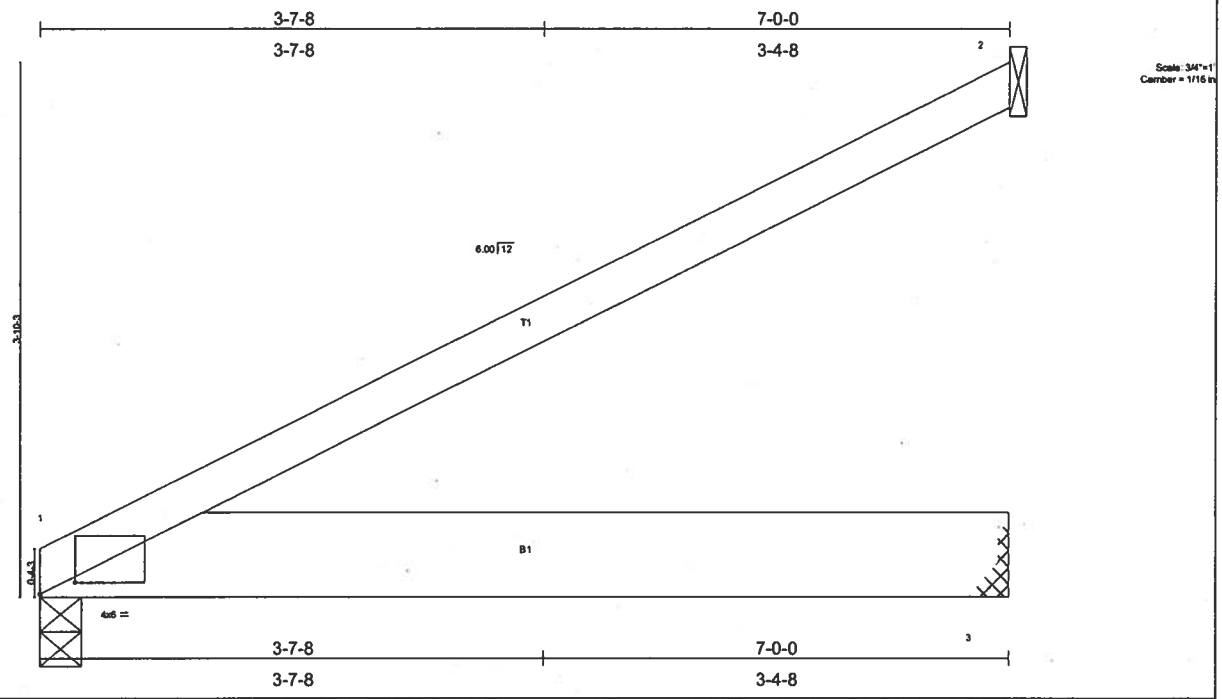

Plate Offsets (X,Y): [1:0-3-0,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.11	1-3	>728	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.18	1-3	>453	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-11 oc bracing.

REACTIONS (lb/size) 1=1214/0-3-8, 2=160/Mechanical, 3=1055/Mechanical
Max Horz 1=155(load case 4)
Max Uplift1=-421(load case 4), 2=-134(load case 4), 3=-364(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-92/54
BOT CHORD 1-3=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 421 lb uplift at joint 1, 134 lb uplift at joint 2 and 364 lb uplift at joint 3.
5) Girder carries tie-in span(s): 15-4-0 from 0-0-0 to 7-0-0
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 1-3=-307(F=-277)

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061336

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	EJ7C	MONO TRUSS	1	1	Job Reference (optional)

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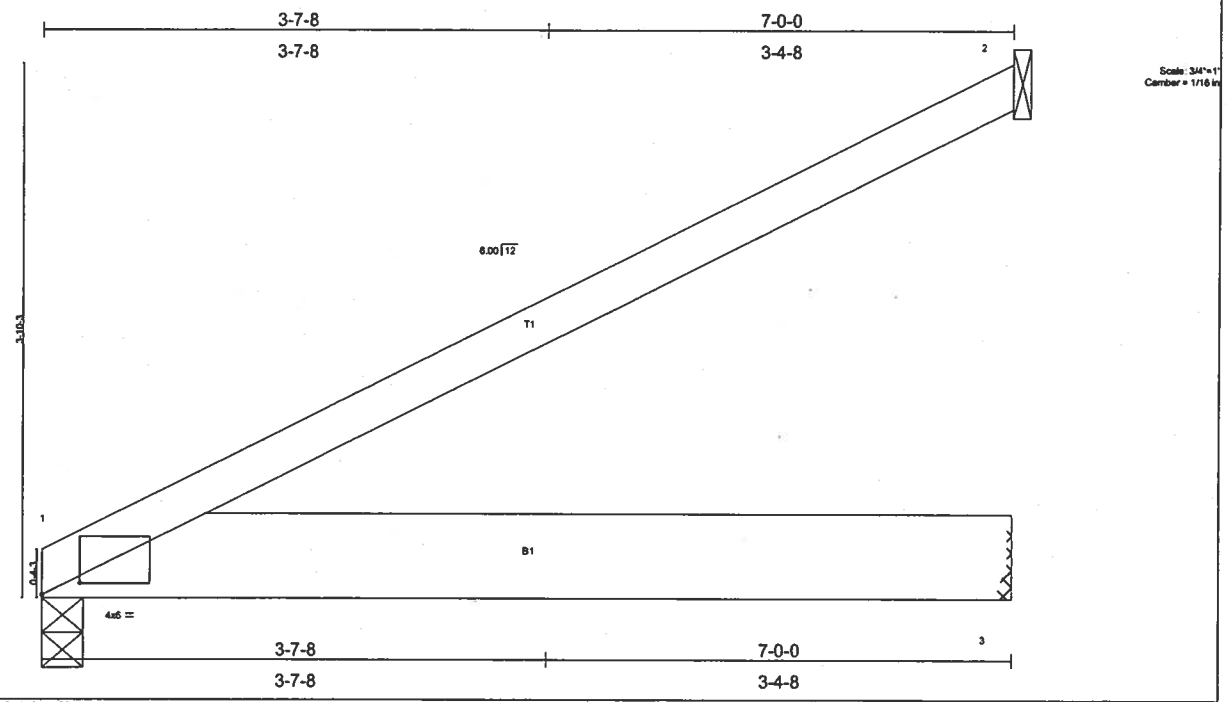

Plate Offsets (X,Y): [1:0-3-4,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.14	1-3	>579	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.90	Vert(TL)	-0.23	1-3	>361	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 3-9-0 oc bracing.

REACTIONS (lb/size) 1=1340/0-3-8, 2=165/Mechanical, 3=1175/Mechanical
Max Horz 1=155(load case 4)
Max Uplift1=-468(load case 4), 2=-136(load case 4), 3=-408(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-89/57
BOT CHORD 1-3=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 468 lb uplift at joint 1, 136 lb uplift at joint 2 and 408 lb uplift at joint 3.
5) Girder carries tie-in span(s): 16-11-0 from 0-0-0 to 7-0-0
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 1-3=-341(F=-311)

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
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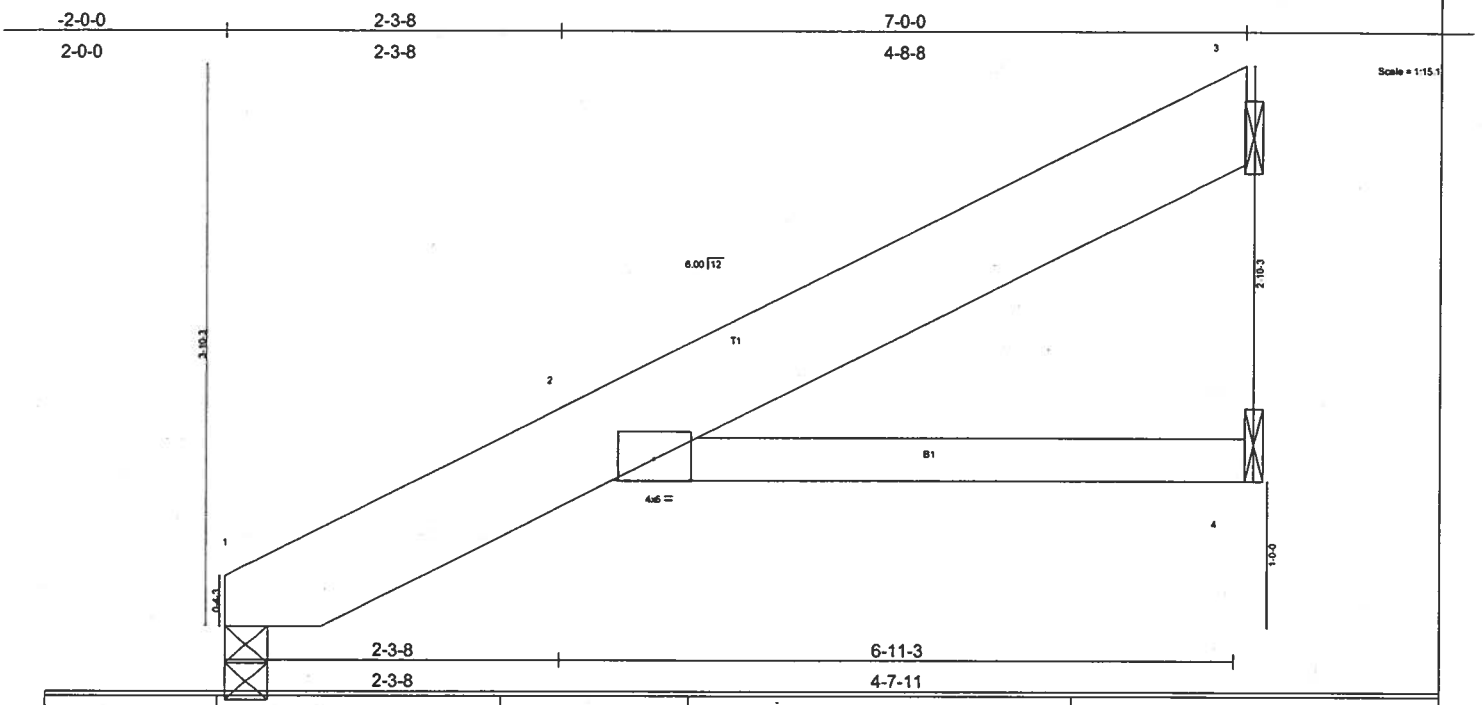
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061337

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	EJ7T	SPECIAL	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 08:38:14 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.11	Vert(LL)	0.04	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	-0.06	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 31 lb	

LUMBER
TOP CHORD 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=246/0-3-8, 3=212/Mechanical, 4=64/Mechanical
Max Horz 1=157(load case 5)
Max Uplift1=-67(load case 5), 3=-159(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-166/0, 2-3=-139/90
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 67 lb uplift at joint 1 and 159 lb uplift at joint 3.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061338

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	EJ7V	SPECIAL	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 08:38:49 2006 Page 1

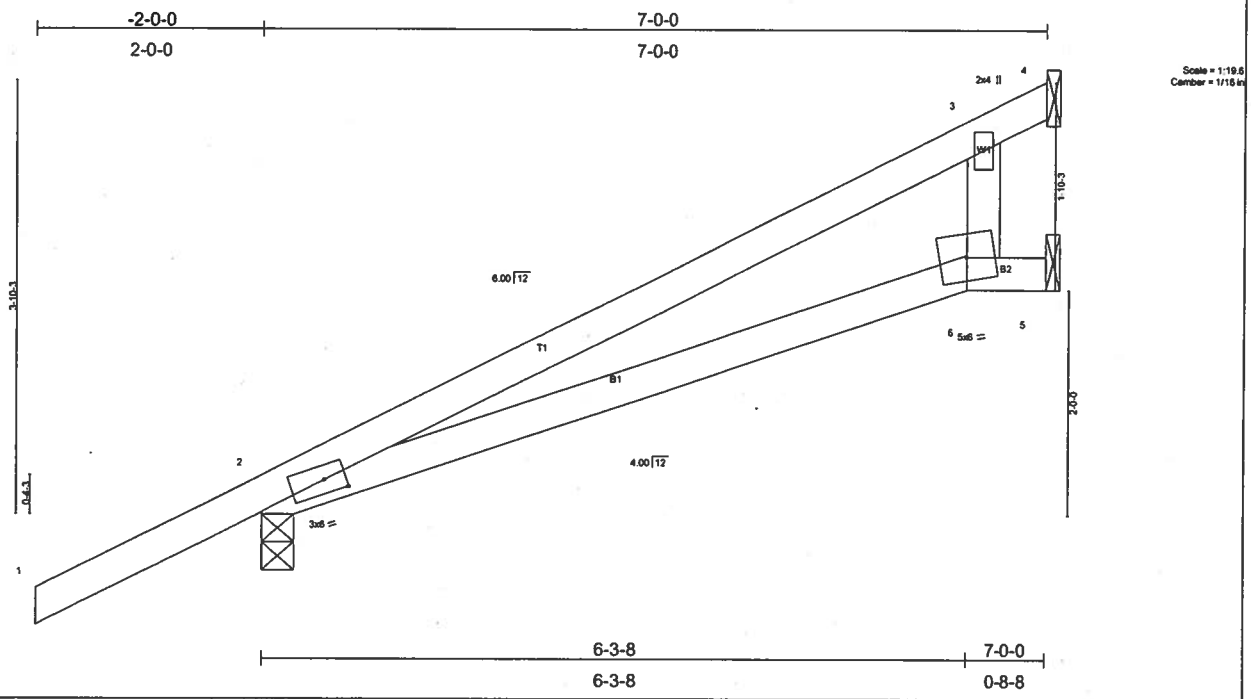


Plate Offsets (X,Y): [2:0-2-4,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.14	2-6	>563	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.22	2-6	>372	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 28 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=138/Mechanical, 2=419/0-3-8, 5=129/Mechanical
Max Horz 2=223(load case 5)
Max Uplift2=-209(load case 5), 5=-167(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-174/53, 3-4=0/61
BOT CHORD 2-6=-31/47, 5-6=-1/2
WEBS 3-6=-30/297

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 209 lb uplift at joint 2 and 167 lb uplift at joint 5.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061339

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	EJ7VA	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 08:39:56 2006 Page 1

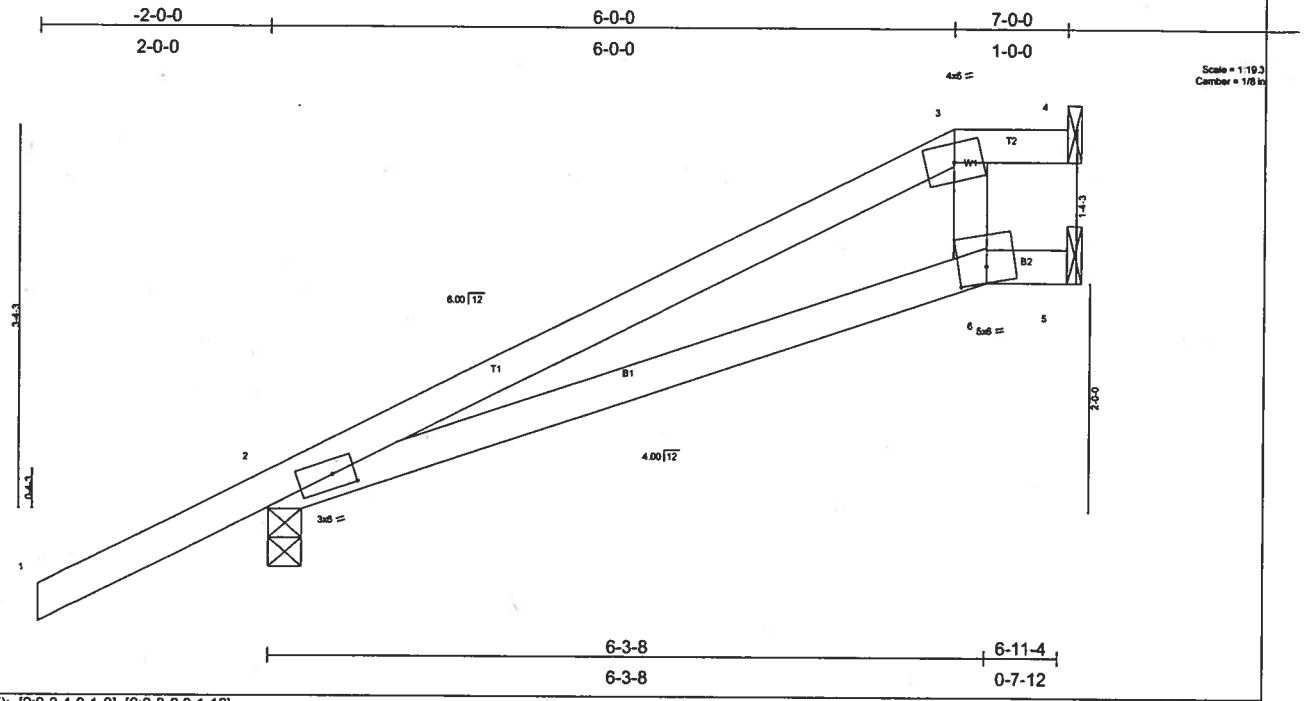


Plate Offsets (X,Y): [2:0-2-4,0-1-8], [6:0-3-0,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.15	2-6	>555	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.24	2-6	>338	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.04	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 27 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=20/Mechanical, 2=419/0-3-8, 5=247/Mechanical
Max Horz 2=202(load case 5)
Max Uplift2=-218(load case 5), 5=-165(load case 5)
Max Grav 4=70(load case 10), 2=419(load case 1), 5=247(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-128/9, 3-4=-0/1
BOT CHORD 2-6=-80/58, 5-6=-1/2
WEBS 3-6=-161/295

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 218 lb uplift at joint 2 and 165 lb uplift at joint 5.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061340

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	HJ7	MONO TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 08:40:45 2006 Page 1

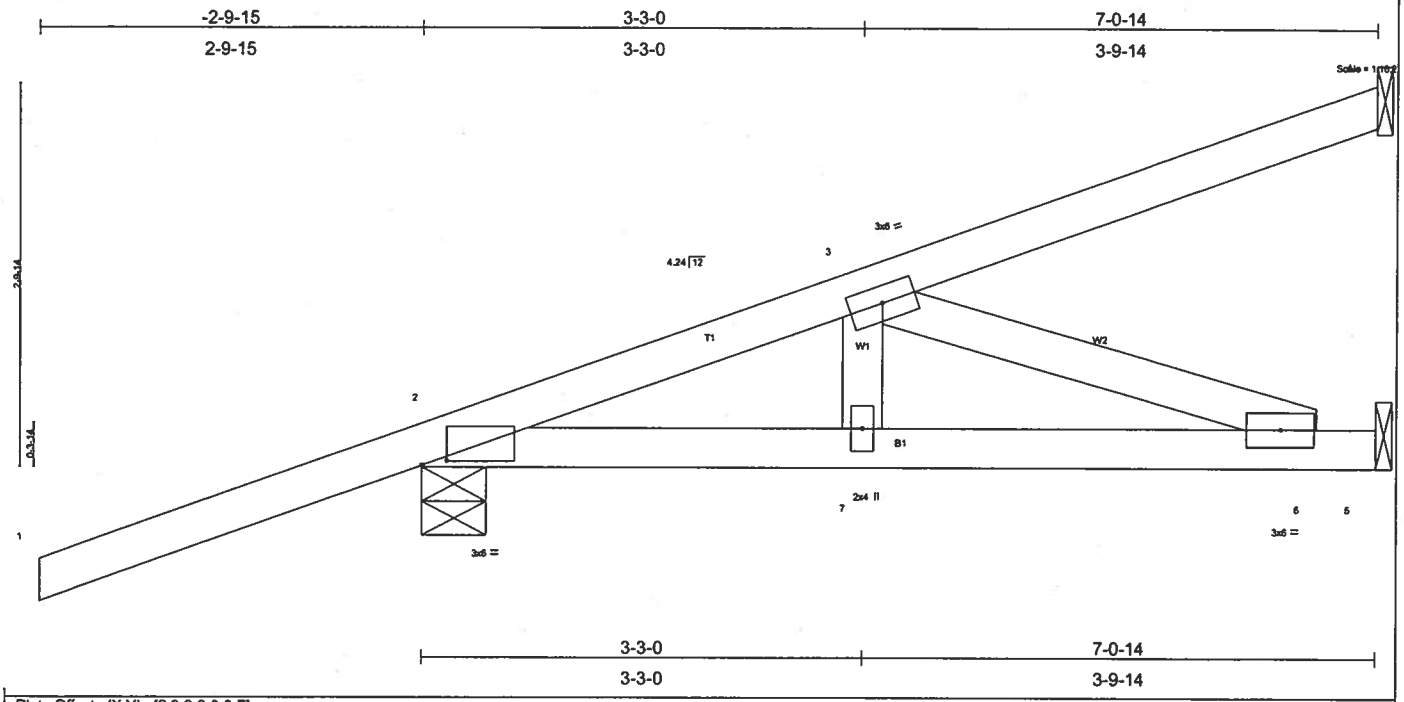


Plate Offsets (X,Y): [2:0-2-3,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.02	2-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.04	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.08	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=137/Mechanical, 2=378/0-5-11, 5=164/Mechanical
Max Horz 2=165(load case 2)
Max Uplift4=-115(load case 2), 2=-251(load case 2), 5=-5(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-309/0, 3-4=-52/34
BOT CHORD 2-7=-50/270, 6-7=-50/270, 5-6=0/0
WEBS 3-7=0/97, 3-6=-286/53

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 115 lb uplift at joint 4, 251 lb uplift at joint 2 and 5 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

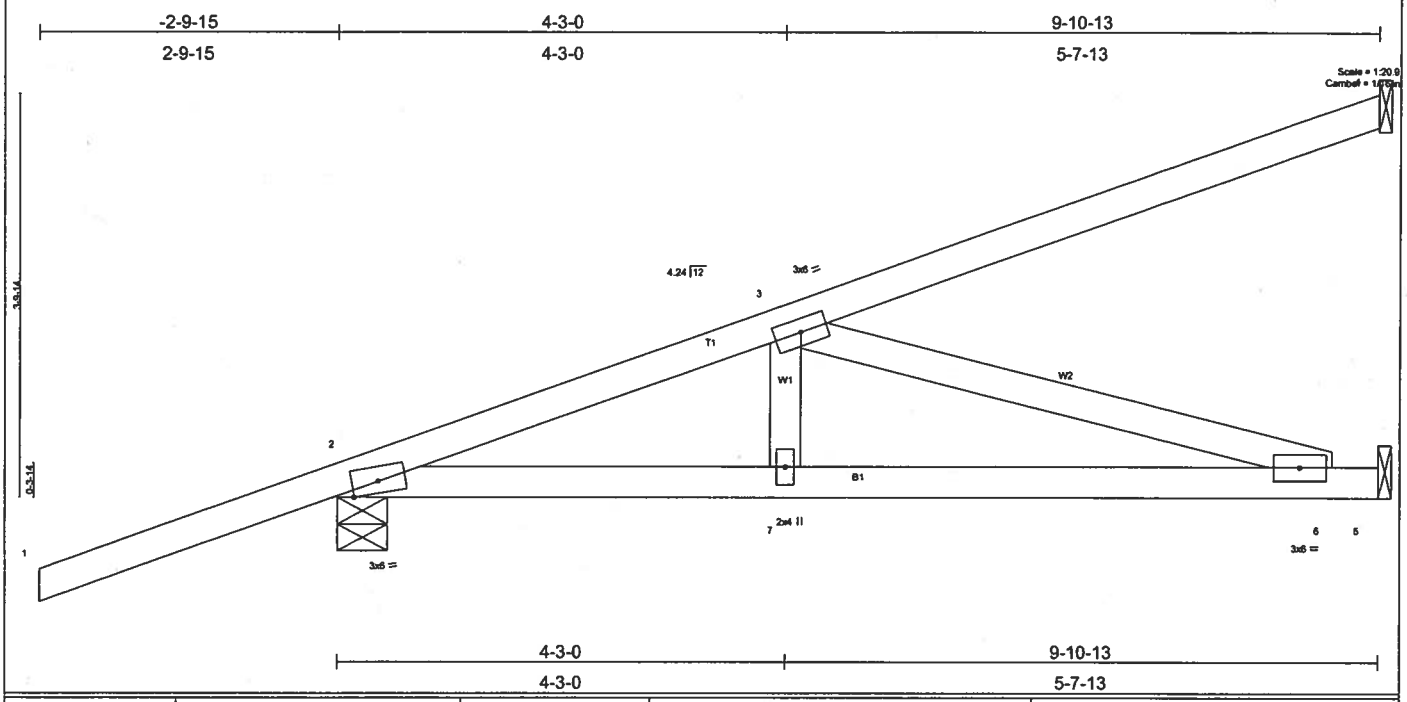
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-95(F=-21, B=-21), 2=0(F=15, B=15)-to-5=-53(F=-12, B=-12)

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061341

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	HJ9	MONO TRUSS	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 08:41:23 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.19	6-7	>622	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.47	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=270/Mechanical, 2=535/0-5-11, 5=374/Mechanical
Max Horz 2=269(load case 2)
Max Uplift4=-231(load case 2), 2=-281(load case 2), 5=-62(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-883/120, 3-4=-105/66
BOT CHORD 2-7=-308/817, 6-7=-308/817, 5-6=0/0
WEBS 3-7=0/188, 3-6=-851/321

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 4, 281 lb uplift at joint 2 and 62 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
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Dwg.#0111061342

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T01	COMMON	9	1	Job Reference (optional)

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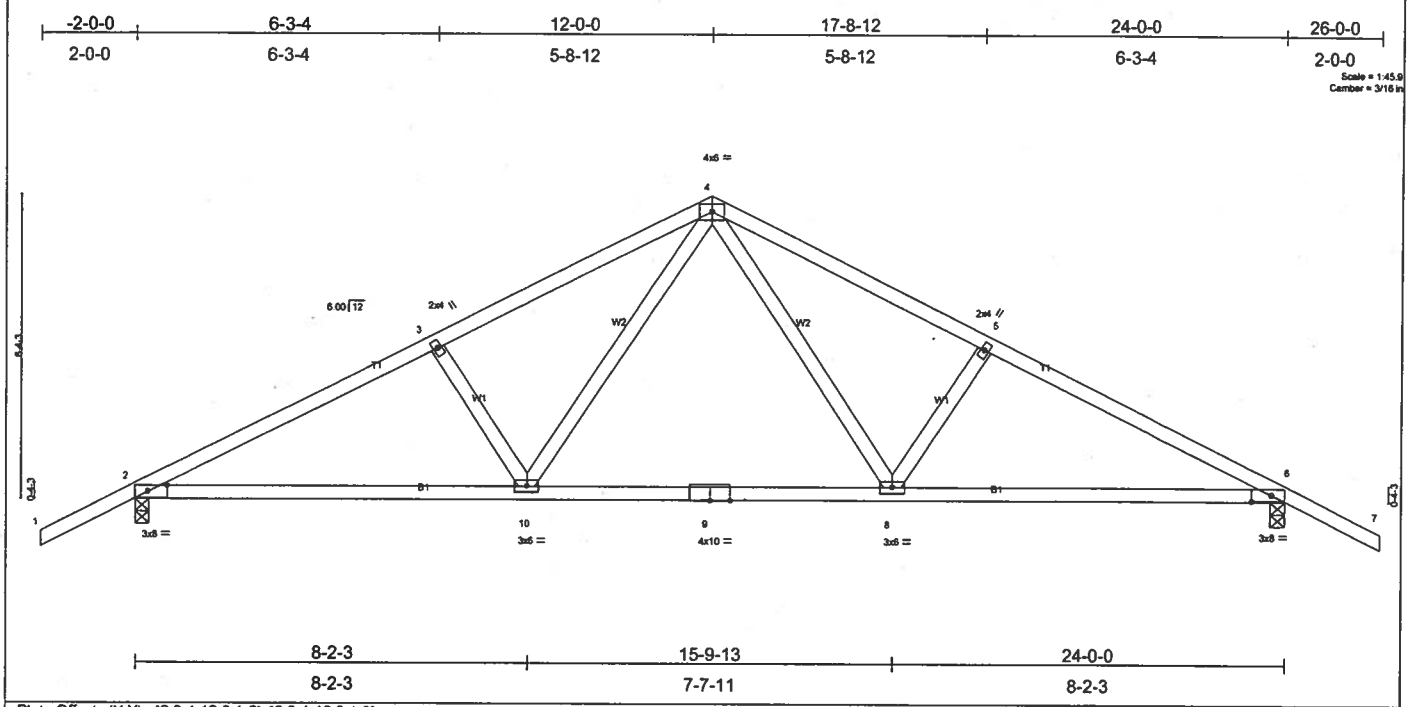

Plate Offsets (X,Y): [2:0-4-12,0-1-8], [6:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.28	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.99	Vert(TL)	-0.46	8-10	>620	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.28	Horz(TL)	0.06	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 113 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-10-12 oc bracing.

REACTIONS (lb/size) 2=1303/0-3-8, 6=1303/0-3-8
Max Horz 2=122(load case 5)
Max Uplift2=-521(load case 5), 6=-521(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2129/893, 3-4=-1954/889, 4-5=-1954/889, 5-6=-2129/893, 6-7=0/47
BOT CHORD 2-10=-618/1827, 9-10=-308/1251, 8-9=-308/1251, 6-8=-618/1827
WEBS 3-10=-283/264, 4-10=-314/828, 4-8=-314/828, 5-8=-283/264

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 521 lb uplift at joint 2 and 521 lb uplift at joint 6.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061343

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T01G	COMMON	1	1	Job Reference (optional)

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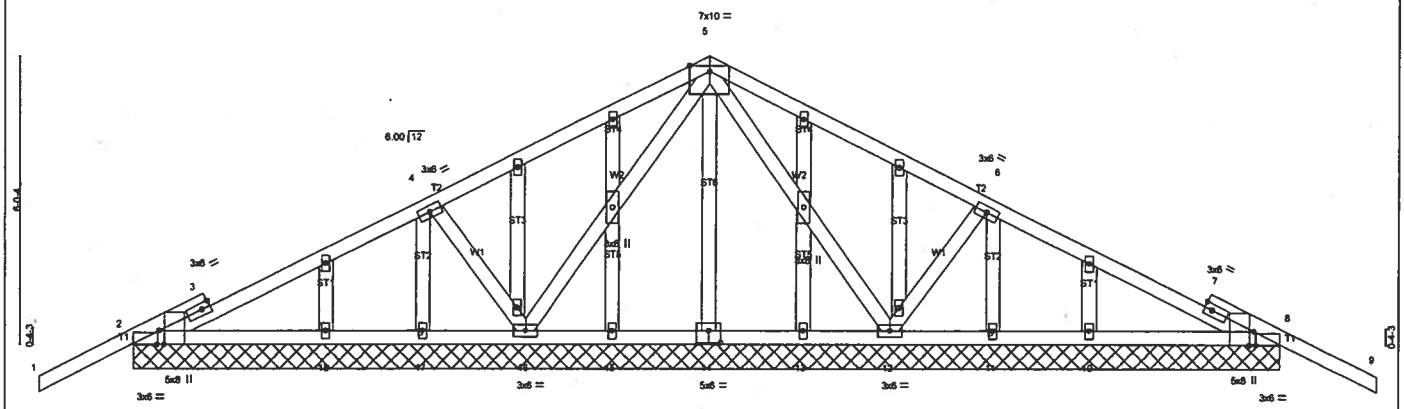
-2-0-0 | 6-3-4 | 12-0-0 | 17-8-12 | 24-0-0 | 26-0-0
2-0-0 | 6-3-4 | 5-8-12 | 5-8-12 | 6-3-4 | 2-0-0

Scale = 1:45.9



8-2-3 | 15-9-13 | 24-0-0
8-2-3 | 7-7-11 | 8-2-3

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [8:0-3-8,Edge], [8:0-0-8,Edge], [14:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.60	Vert(LL)	0.02	9	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.26	Vert(TL)	0.03	9	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.57	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 157 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=542/24-0-0, 8=542/24-0-0, 16=1117/24-0-0, 12=1117/24-0-0, 15=100/24-0-0, 17=8/24-0-0, 18=189/24-0-0, 13=100/24-0-0, 11=8/24-0-0, 10=189/24-0-0
Max Horz 2=118(load case 5)
Max Uplift2=-295(load case 5), 8=-311(load case 6), 16=-531(load case 5), 12=-513(load case 6), 18=-23(load case 6), 10=-21(load case 5)
Max Grav 2=556(load case 9), 8=556(load case 10), 16=1117(load case 1), 12=1117(load case 1), 15=101(load case 10), 17=31(load case 6), 18=189(load case 1), 13=101(load case 9), 11=30(load case 5), 10=189(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-15/99, 2-3=-63/65, 3-4=-81/271, 4-5=-178/664, 5-6=-156/664, 6-7=-75/271, 7-8=-53/65, 8-9=-15/99
BOT CHORD 2-18=-111/197, 17-18=-111/197, 16-17=-111/197, 15-16=-91/234, 14-15=-91/234, 13-14=-91/234, 12-13=-91/234, 11-12=-111/157, 10-11=-111/157, 8-10=-111/157
WEBS 4-16=-634/426, 5-16=-671/306, 5-12=-671/306, 6-12=-634/426

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 295 lb uplift at joint 2, 311 lb uplift at joint 8, 531 lb uplift at joint 16, 513 lb uplift at joint 12, 23 lb uplift at joint 18 and 21 lb uplift at joint 10.
8) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 8.
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-114(F=-60), 5-9=-114(F=-60), 2-8=-30

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061344

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T02	SPECIAL	1	1	Job Reference (optional)

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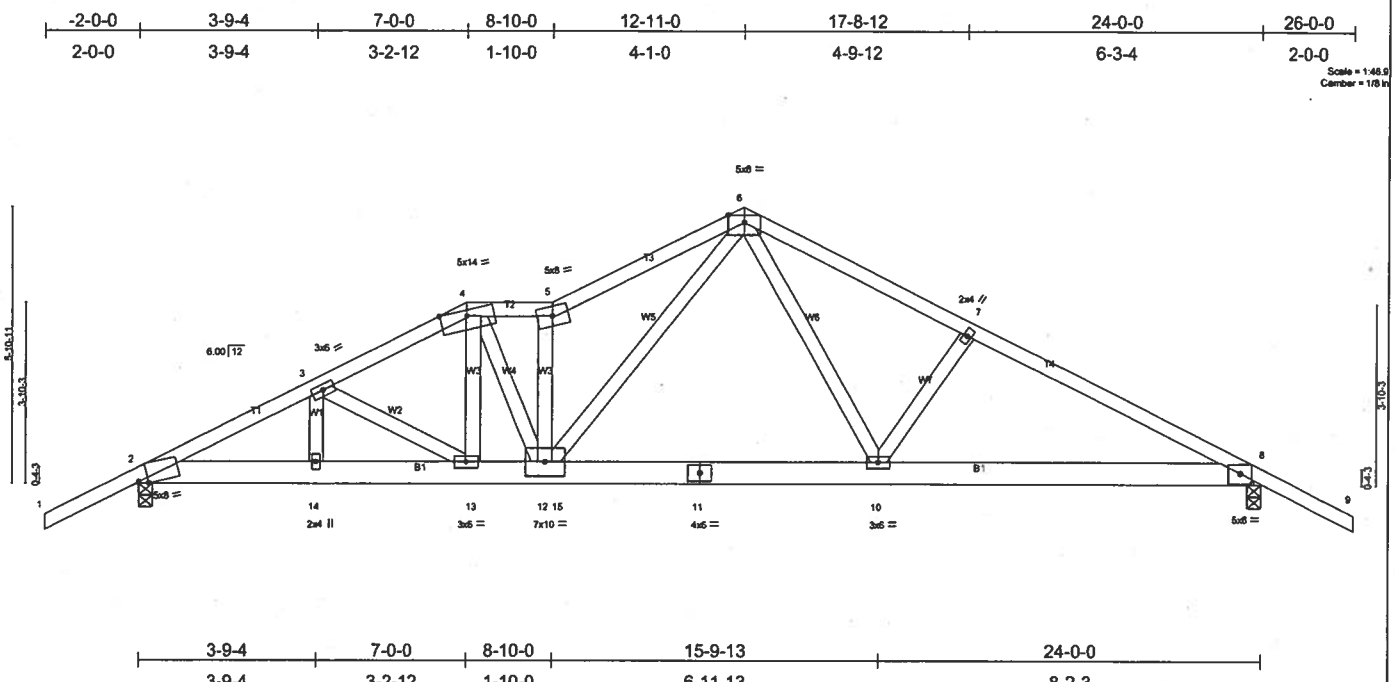

Plate Offsets (X,Y): [2:0-2-7,Edge], [8:0-3-0,0-2-9]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.21	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.33	10-12	>860	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.94	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 148 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-7-9 oc bracing.

REACTIONS (lb/size) 2=2139/0-3-8, 8=1644/0-3-8
Max Horz 2=-118(load case 5)
Max Uplift2=-845(load case 4), 8=-645(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-3978/1333, 3-4=-3852/1377, 4-5=-3810/1380, 5-6=-4363/1630, 6-7=-2710/942, 7-8=-2885/950, 8-9=0/51
BOT CHORD 2-14=-1185/3499, 13-14=-1185/3499, 12-13=-1167/3456, 12-15=-618/2111, 11-15=-618/2111, 10-11=-618/2111, 8-10=-742/2507
WEBS 3-14=-41/113, 3-13=-98/40, 4-13=-264/703, 4-12=-266/809, 5-12=-1983/805, 6-12=-1125/2923, 6-10=-172/572, 7-10=-254/234

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 845 lb uplift at joint 2 and 645 lb uplift at joint 8.
5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 400 lb down and 151 lb up at 7-0-0, and 1055 lb down and 398 lb up at 8-10-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-5=-91(F=-37), 5-6=-54, 6-9=-54, 2-13=-30, 13-15=-50(F=-20), 8-15=-30
Concentrated Loads (lb)
Vert: 13=-400(F) 12=-1055(F)

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
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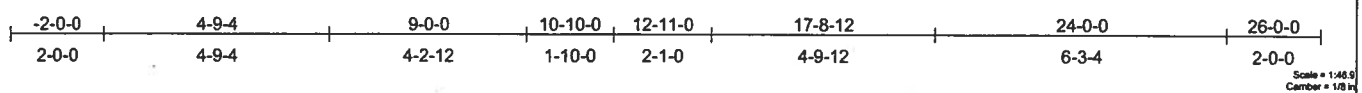
Dwg.#0111061345

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T03	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 08:51:15 2006 Page 1



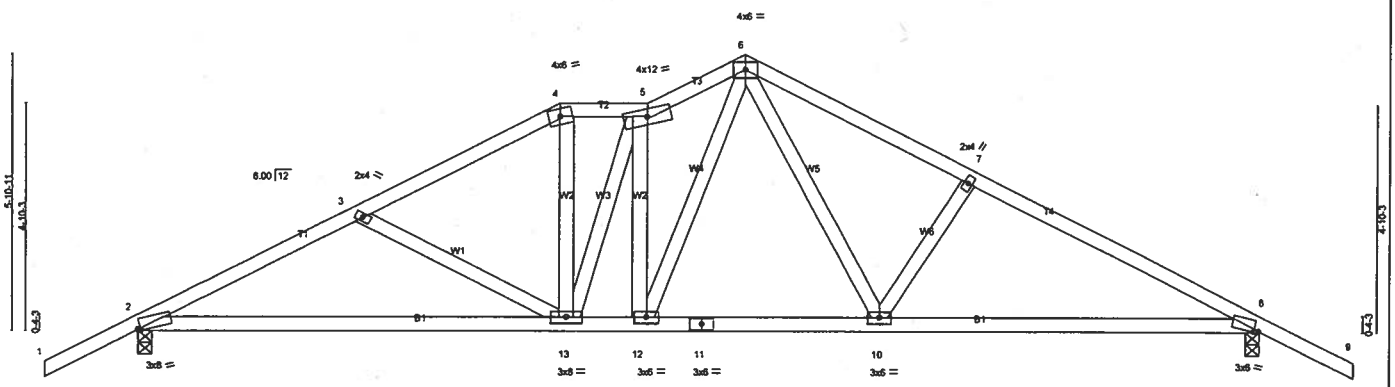


Plate Offsets (X,Y): [2:0-0-10,Edge], [8:0-1-5,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.17	2-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.28	2-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.24	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 132 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-0-4 oc bracing.

REACTIONS (lb/size) 2=1112/0-3-8, 8=1112/0-3-8
Max Horz 2=-116(load case 6)
Max Uplift2=-456(load case 5), 8=-444(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1690/716, 3-4=-1438/607, 4-5=-1248/596, 5-6=-1404/701, 6-7=-1498/681, 7-8=-1673/687, 8-9=0/47
BOT CHORD 2-13=-477/1468, 12-13=-306/1243, 11-12=-213/1039, 10-11=-213/1039, 8-10=-436/1426
WEBS 3-13=-270/217, 4-13=-68/388, 5-13=-30/103, 5-12=-628/220, 6-12=-274/623, 6-10=-186/534, 7-10=-279/257

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 456 lb uplift at joint 2 and 444 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

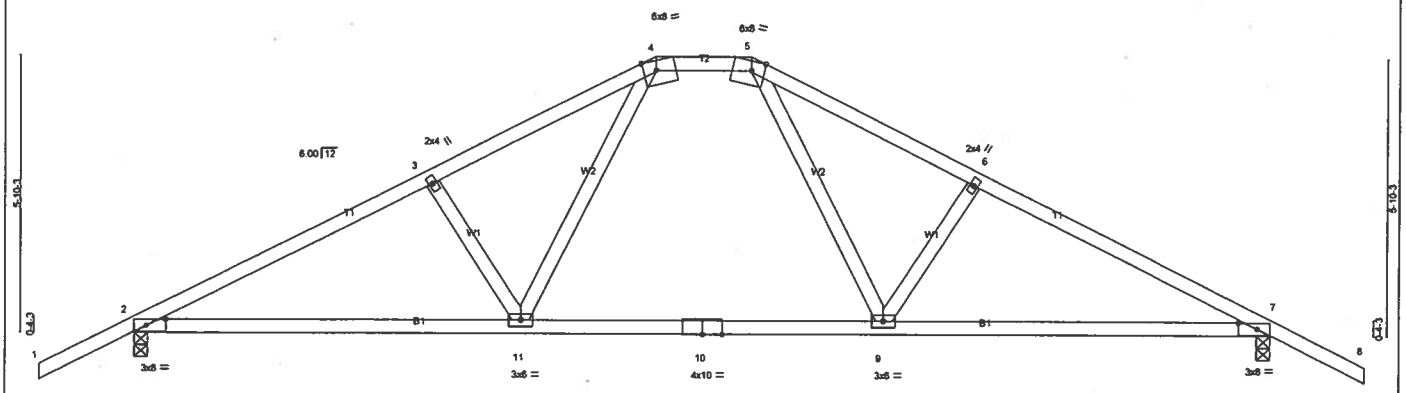
Dwg.#0111061346

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T04	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 08:57:48 2006 Page 1

-2-0-0	6-3-4	11-0-0	13-0-0	17-8-12	24-0-0	26-0-0
2-0-0	6-3-4	4-8-12	2-0-0	4-8-12	6-3-4	2-0-0

Scale = 1:46.4
Camber = 3/16 in



8-2-3	15-9-13	24-0-0
8-2-3	7-7-10	8-2-3

Plate Offsets (X,Y): [2:0-4-12,0-1-8], [4:0-3-3,Edge], [5:0-3-3,Edge], [7:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.28	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.99	Vert(TL)	-0.45	9-11	>631	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.25	Horz(TL)	0.06	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 110 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-11-11 oc bracing.

REACTIONS (lb/size) 2=1303/0-3-8, 7=1303/0-3-8
Max Horz 2=115(load case 5)
Max Uplift2=-515(load case 5), 7=-515(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2126/881, 3-4=-1952/875, 4-5=-1341/701, 5-6=-1952/875, 6-7=-2126/881, 7-8=0/47
BOT CHORD 2-11=-607/1824, 10-11=-340/1341, 9-10=-340/1341, 7-9=-607/1824
WEBS 3-11=-292/254, 4-11=-288/785, 5-9=-288/785, 6-9=-292/254

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 515 lb uplift at joint 2 and 515 lb uplift at joint 7.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

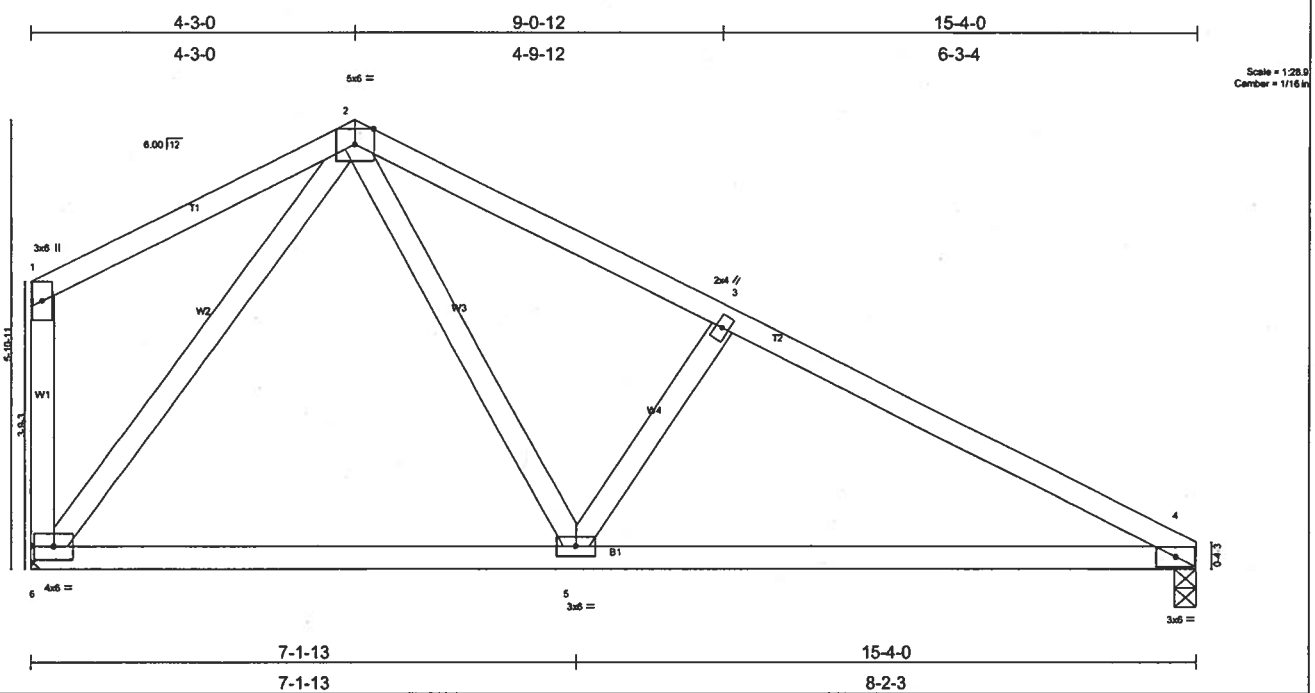
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-5=-54, 5-8=-54, 2-11=-30, 9-11=-80(F=-50), 7-9=-30

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061347

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T05	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 08:53:08 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.91	Vert(LL)	-0.14	5-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.80	Vert(TL)	-0.23	5-6	>797	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 77 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-7-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=713/0-3-8, 6=900/Mechanical
Max Horz 6=-191(load case 6)
Max Uplift4=-234(load case 6), 6=-318(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-170/104, 2-3=-960/479, 3-4=-1123/489, 1-6=-179/128
BOT CHORD 5-6=-38/436, 4-5=-344/954
WEBS 2-5=-339/793, 3-5=-301/298, 2-6=-572/301

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 234 lb uplift at joint 4 and 318 lb uplift at joint 6.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-4=-54, 5-6=-80(F=-50), 4-5=-30

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

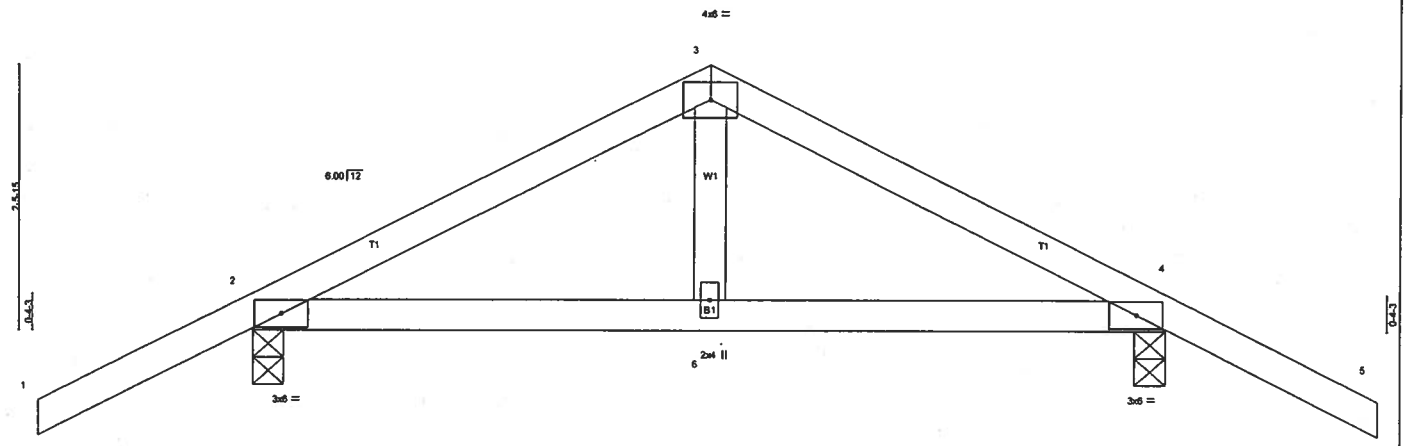
Dwg.#0111061348

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T06	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 08:59:34 2006 Page 1

-2-0-0 | 4-3-8 | 8-7-0 | 10-7-0
2-0-0 | 4-3-8 | 4-3-8 | 2-0-0

Scale = 1:20.6



4-3-8 | 8-7-0
4-3-8 | 4-3-8

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.02	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.03	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.12	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 37 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=700/0-3-8, 4=700/0-3-8
Max Horz 2=-68(load case 5)
Max Uplift2=-336(load case 4), 4=-336(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-720/199, 3-4=-720/199, 4-5=0/47
BOT CHORD 2-6=-111/596, 4-6=-111/596
WEBS 3-6=-60/388

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 336 lb uplift at joint 2 and 336 lb uplift at joint 4.
4) Girder carries tie-in span(s): 5-0-0 from 0-0-0 to 8-7-0
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-54, 2-4=-87(B=-57)

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

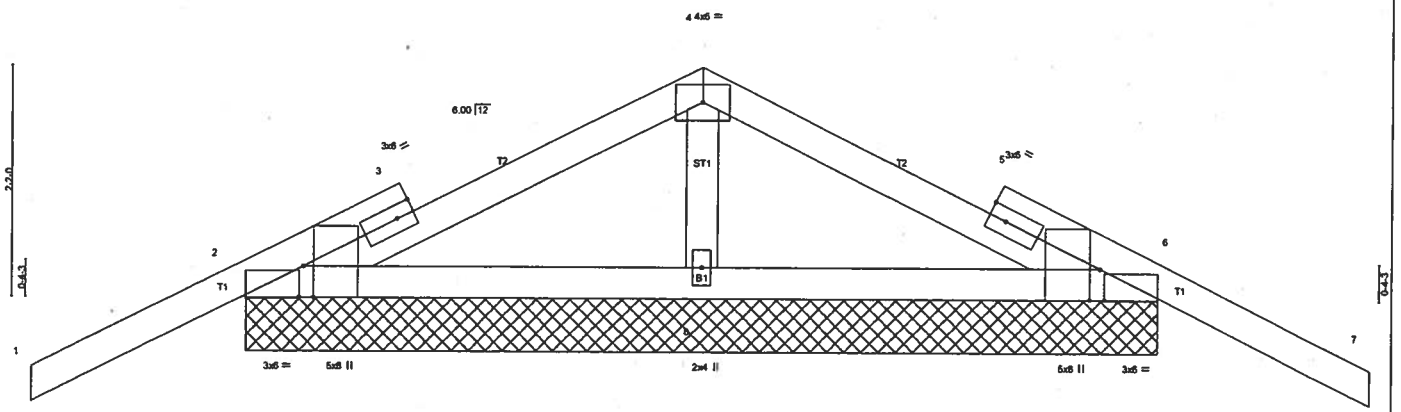
Dwg.#0111061349

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T06G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 08:59:17 2006 Page 1

-2-0-0 | 4-3-8 | 8-7-0 | 10-7-0
2-0-0 | 4-3-8 | 4-3-8 | 2-0-0

Scale = 1:20.6



8-7-0
8-7-0

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [6:0-3-8,Edge], [6:0-0-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.03	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.09	Vert(TL)	-0.04	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.10	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 39 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 8-7-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=489/8-7-0, 6=489/8-7-0, 8=714/8-7-0
Max Horz 2=-64(load case 6)
Max Uplift2=-292(load case 5), 6=-302(load case 6), 8=-185(load case 5)
Max Grav 2=502(load case 9), 6=502(load case 10), 8=714(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-22/99, 2-3=-24/77, 3-4=-38/238, 4-5=-30/238, 5-6=-13/78, 6-7=-22/99
BOT CHORD 2-8=-124/141, 6-8=-124/141
WEBS 4-8=-568/312

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2, 302 lb uplift at joint 6 and 185 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-114(F=-60), 4-7=-114(F=-60), 2-6=-30

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

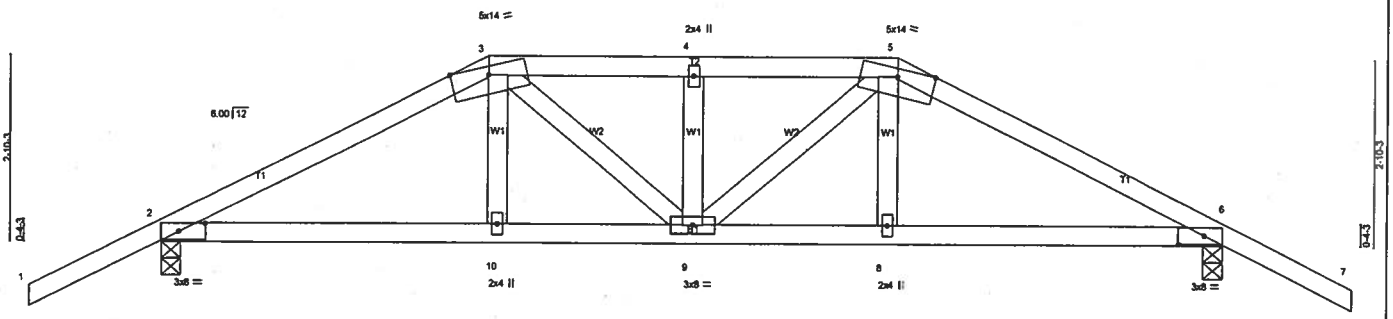
Dwg.#0111061350

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T07	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:00:33 2006 Page 1

-2-0-0 | 5-0-0 | 8-1-8 | 11-3-0 | 16-3-0 | 18-3-0
2-0-0 | 5-0-0 | 3-1-8 | 3-1-8 | 5-0-0 | 2-0-0

Scale = 1:33.6



5-0-0 | 8-1-8 | 11-3-0 | 16-3-0
5-0-0 | 3-1-8 | 3-1-8 | 5-0-0

Plate Offsets (X,Y): [2:0-4-12,0-1-8], [6:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.07	9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.11	9	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.13	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 78 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-1-5 oc bracing.

REACTIONS (lb/size) 2=1206/0-3-8, 6=1206/0-3-8
Max Horz 2=-73(load case 5)
Max Uplift2=-542(load case 4), 6=-542(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1963/728, 3-4=-1943/783, 4-5=-1943/783, 5-6=-1963/728, 6-7=0/47
BOT CHORD 2-10=-606/1685, 9-10=-611/1708, 8-9=-584/1708, 6-8=-579/1685
WEBS 3-10=-92/402, 3-9=-206/368, 4-9=-268/234, 5-9=-206/368, 5-8=-91/402

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 542 lb uplift at joint 2 and 542 lb uplift at joint 6.
5) Girder carries hip end with 5-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 126 lb up at 11-3-0, and 245 lb down and 126 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-91(F=-37), 5-7=-54, 2-10=-30, 8-10=-50(F=-20), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-245(F) 8=-245(F)

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

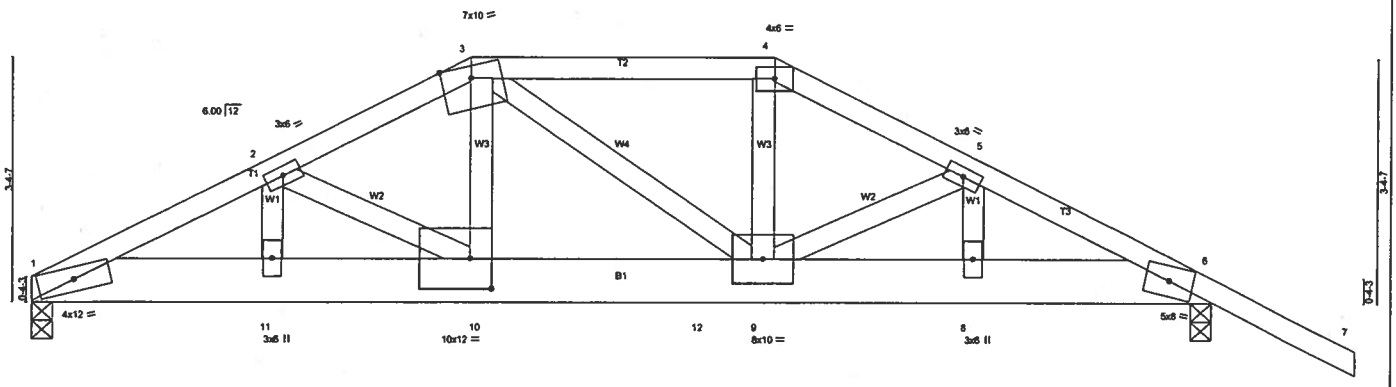
Dwg.#0111061351

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T07A	HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 10:33:14 2006 Page 1

3-3-8	6-0-8	10-2-8	12-11-8	16-3-0	18-3-0
3-3-8	2-9-0	4-2-0	2-9-0	3-3-8	2-0-0

Scale = 1:30.3
Camber = 1/16 in



3-3-8	6-0-8	9-3-0	10-2-8	12-11-8	16-3-0
3-3-8	2-9-0	3-2-8	0-11-8	2-9-0	3-3-8

Plate Offsets (X,Y): [10:0-3-8,0-5-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.13	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.21	9-10	>916	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.71	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 211 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=6300/0-3-8, 6=3912/0-3-8
Max Horz 1=-113(load case 5)
Max Uplift1=-2330(load case 4), 6=-1513(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-10609/3906, 2-3=-9111/3364, 3-4=-7599/2807, 4-5=-8318/3043, 5-6=-7447/2653, 6-7=0/54
BOT CHORD 1-11=-3483/9508, 10-11=-3483/9508, 10-12=-2999/8359, 9-12=-2999/8359, 8-9=-2324/6653, 6-8=-2324/6653
WEBS 2-11=-501/1190, 2-10=-1555/656, 3-10=-1622/4399, 3-9=-1005/558, 4-9=-1277/3513, 5-9=-421/951, 5-8=-996/453

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2330 lb uplift at joint 1 and 1513 lb uplift at joint 6.
7) Girder carries tie-in span(s): 36-8-8 from 0-0-0 to 9-3-0
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2176 lb down and 822 lb up at 9-3-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

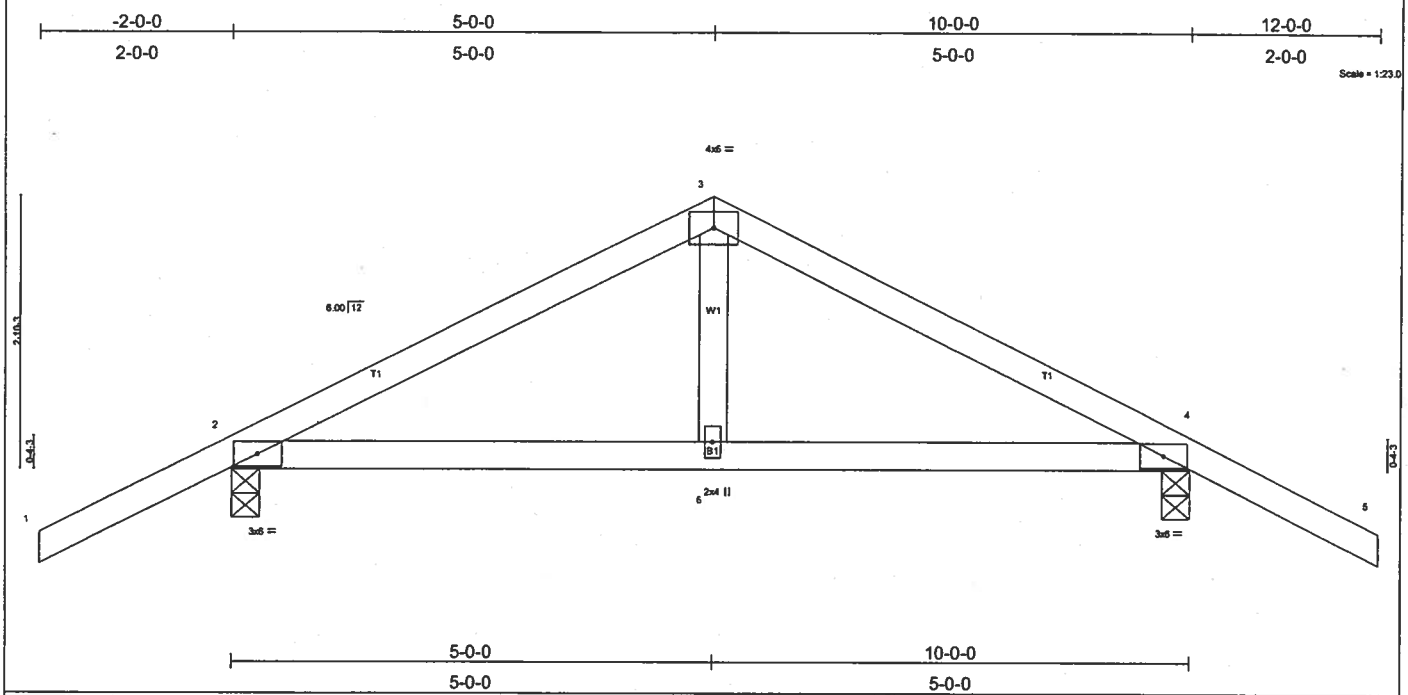
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-54, 4-7=-54, 1-12=-753(F=-723), 6-12=-30
Concentrated Loads (lb)
Vert: 12=-2176(F)

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061352

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T08	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:02:32 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.03	4-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.15	Vert(TL)	0.03	4-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 42 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=524/0-3-8, 4=524/0-3-8
Max Horz 2=-73(load case 6)
Max Uplift2=-386(load case 5), 4=-386(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-519/579, 3-4=-519/579, 4-5=0/47
BOT CHORD 2-6=-353/408, 4-6=-353/408
WEBS 3-6=-272/158

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 386 lb uplift at joint 2 and 386 lb uplift at joint 4.

LOAD CASE(S) Standard

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061353

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T08G	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:02:56 2006 Page 1

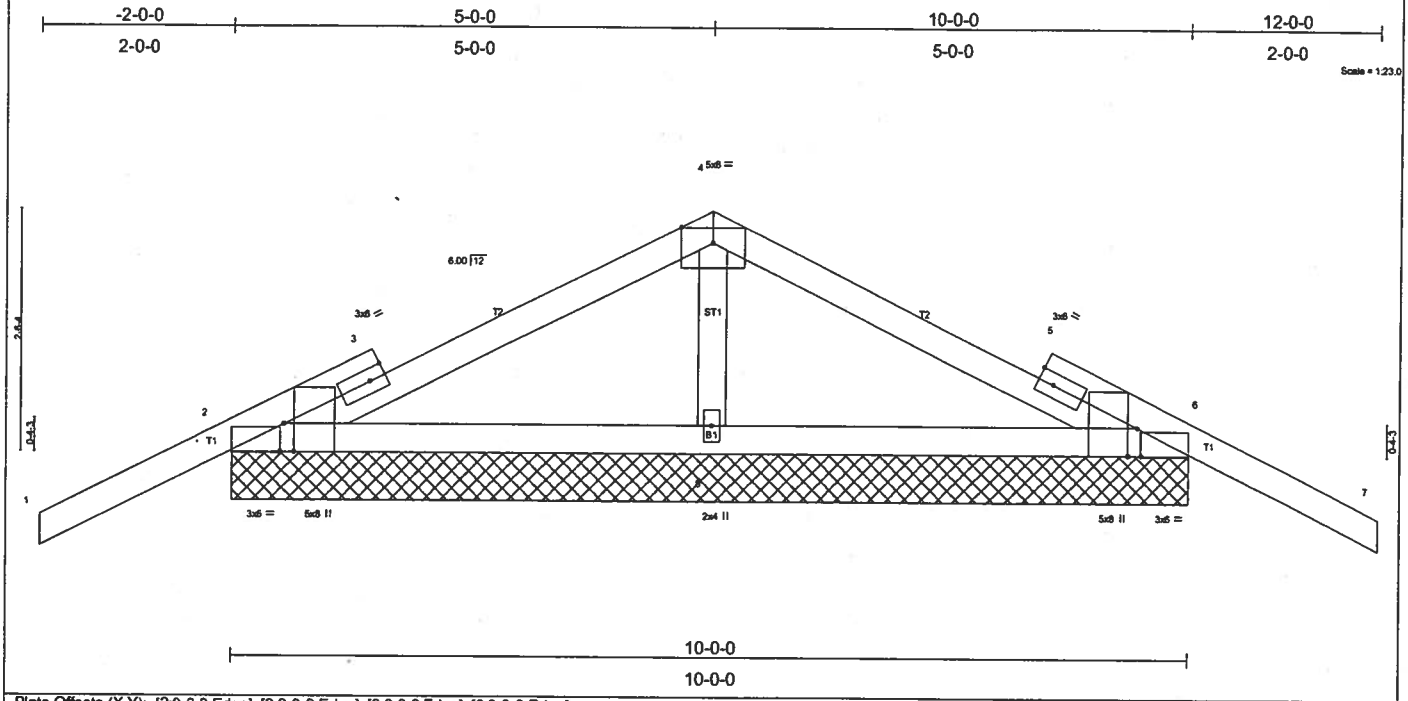

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [6:0-3-8,Edge], [6:0-0-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.01	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.18	Vert(TL)	-0.02	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.14	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 44 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=469/10-0-0, 6=469/10-0-0, 8=958/10-0-0
Max Horz 2=-69(load case 6)
Max Uplift2=-279(load case 5), 6=-289(load case 6), 8=-279(load case 5)
Max Grav 2=492(load case 9), 6=492(load case 10), 8=958(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-21/99, 2-3=-78/184, 3-4=-97/382, 4-5=-97/382, 5-6=-67/185, 6-7=-21/99
BOT CHORD 2-8=-234/211, 6-8=-234/211
WEBS 4-8=-775/430

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 279 lb uplift at joint 2, 289 lb uplift at joint 6 and 279 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-114(F=-60), 4-7=-114(F=-60), 2-6=-30

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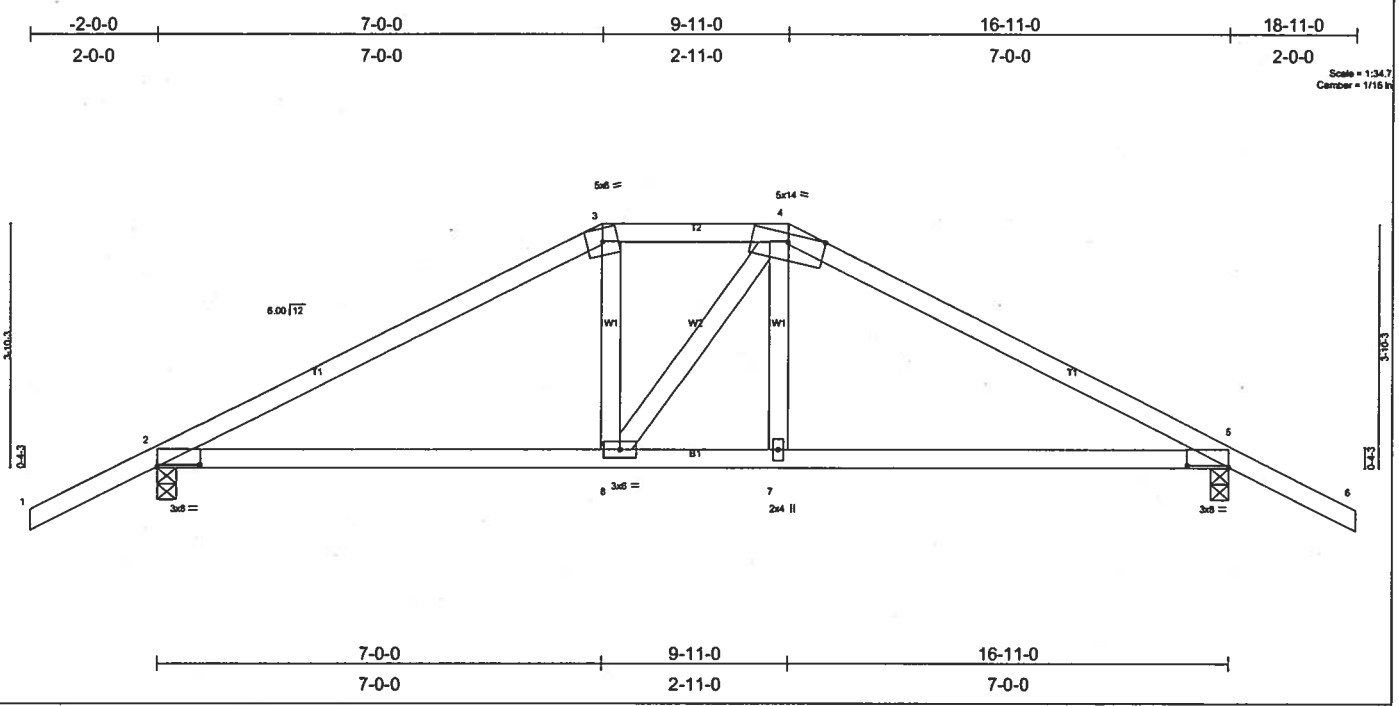
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061354

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
TEMP	T09	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:05:38 2006 Page 1



7-0-0	9-11-0	16-11-0
7-0-0	2-11-0	7-0-0

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [5:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.13	5-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.21	5-7	>941	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.27	Horz(TL)	0.06	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 76 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-10-9 oc bracing.

REACTIONS (lb/size) 2=1492/0-3-8, 5=1492/0-3-8
Max Horz 2=87(load case 4)
Max Uplift2=-698(load case 4), 5=-698(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2495/1026, 3-4=-2175/986, 4-5=-2492/1025, 5-6=0/47
BOT CHORD 2-8=-839/2145, 7-8=-821/2172, 5-7=-811/2142
WEBS 3-8=-287/832, 4-8=-127/137, 4-7=-252/746

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 698 lb uplift at joint 2 and 698 lb uplift at joint 5.
5) Girder carries hip end with 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 9-11-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-8=-30, 7-8=-65(F=-35), 5-7=-30
Concentrated Loads (lb)
Vert: 8=-539(F) 7=-539(F)

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

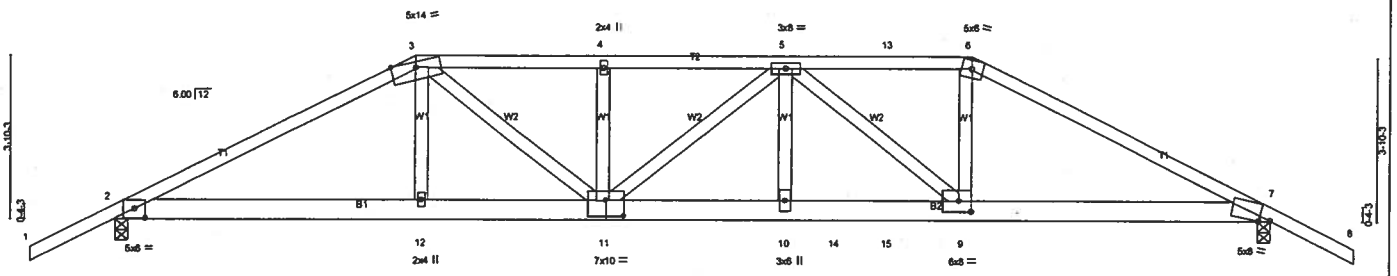
Dwg.#0111061355

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T10	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:10:56 2006 Page 1

-2-0-0	7-0-0	11-5-4	15-8-12	20-2-0	27-2-0	29-2-0
2-0-0	7-0-0	4-5-4	4-3-8	4-5-4	7-0-0	2-0-0

Scale = 1:51.7
Camber = 1/8 in



7-0-0	11-5-4	15-8-12	16-11-0	20-2-0	27-2-0
7-0-0	4-5-4	4-3-8	1-2-4	3-3-0	7-0-0

Plate Offsets (X,Y): [2:0-3-0,0-2-9], [7:0-2-15,0-0-14], [9:0-3-8,0-3-0], [11:0-5-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	-0.26	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	-0.41	9-10	>784	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.60	Horz(TL)	0.09	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 154 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-7-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-3-14 oc bracing.

REACTIONS (lb/size) 2=1823/0-3-8, 7=2352/0-3-8
Max Horz 2=-89(load case 5)
Max Uplift2=-662(load case 4), 7=-861(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-3274/1039, 3-4=-3960/1370, 4-5=-3957/1368, 5-13=-3988/1389, 6-13=-3989/1389, 6-7=-4479/1493, 7-8=0/51
BOT CHORD 2-12=-883/2847, 11-12=-882/2857, 10-11=-1596/4807, 10-14=-1596/4807, 14-15=-1596/4807, 9-15=-1596/4807, 7-9=-1248/3921
WEBS 3-12=0/253, 3-11=-606/1497, 4-11=-232/168, 5-11=-1136/437, 5-10=-375/1141, 5-9=-1154/479, 6-9=-517/1639

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 662 lb uplift at joint 2 and 861 lb uplift at joint 7.
5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 400 lb down and 151 lb up at 20-2-0, and 1175 lb down and 444 lb up at 16-11-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-13=-54, 6-13=-91(F=-37), 6-8=-54, 2-15=-30, 9-15=-50(F=-20), 7-9=-30
Concentrated Loads (lb)
Vert: 9=-400(F) 14=-1175(F)

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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061356

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T11	COMMON	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:11:19 2006 Page 1

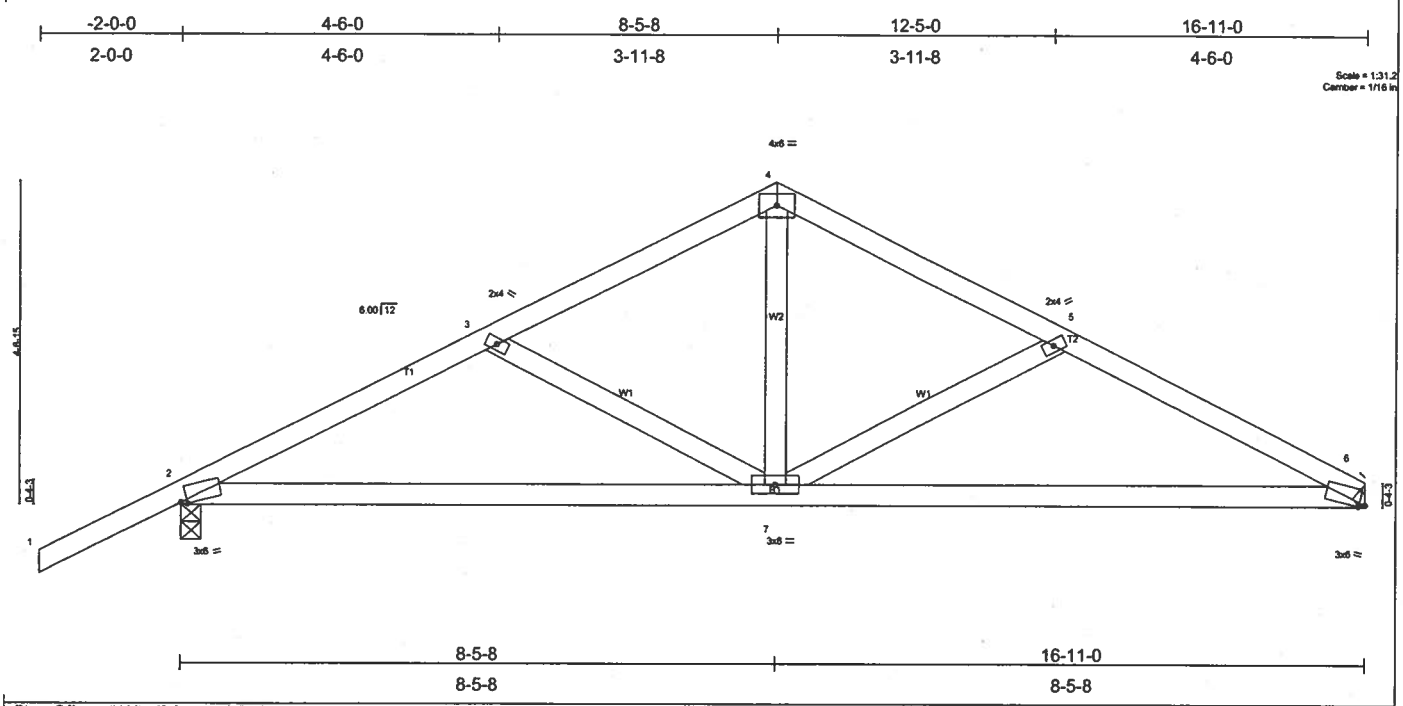

Plate Offsets (X,Y): [2:0-1-1,0-0-7], [6:0-1-1,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.19	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.17	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 76 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-7-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-11-10 oc bracing.

REACTIONS (lb/size) 6=691/Mechanical, 2=822/0-3-8
Max Horz 2=122(load case 5)
Max Uplift6=-224(load case 6), 2=-357(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1097/480, 3-4=-866/384, 4-5=-868/388, 5-6=-1117/513
BOT CHORD 2-7=-347/940, 6-7=-392/968
WEBS 3-7=-258/195, 4-7=-152/517, 5-7=-289/247

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 224 lb uplift at joint 6 and 357 lb uplift at joint 2.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
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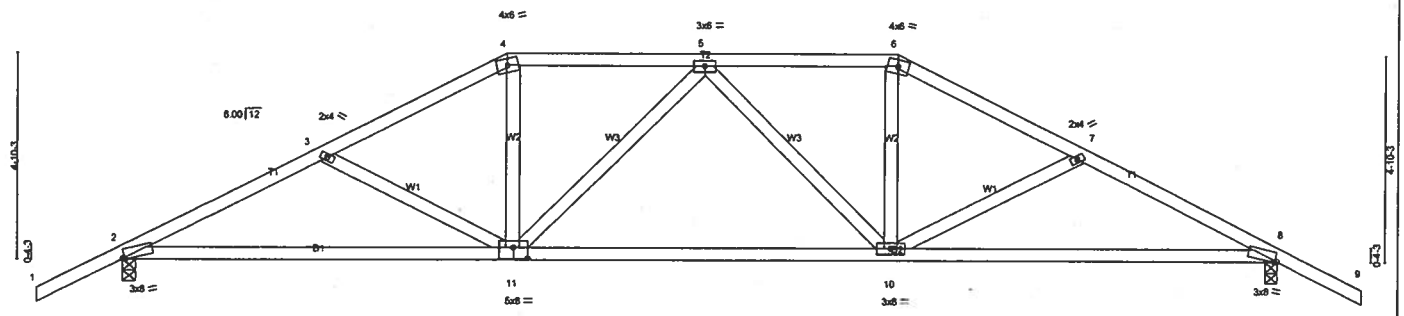
Dwg.#0111061357

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T12	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:15:44 2006 Page 1

-2-0-0	4-9-4	9-0-0	13-7-0	18-2-0	22-4-12	27-2-0	29-2-0
2-0-0	4-9-4	4-2-12	4-7-0	4-7-0	4-2-12	4-9-4	2-0-0

Scale = 1:51.7
Camber = 1/8 in



9-0-0	18-2-0	27-2-0
9-0-0	9-2-0	9-0-0

Plate Offsets (X,Y): [2:0-0-10,Edge], [8:0-0-10,Edge], [11:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.17	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.29	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.19	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 136 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-3-12 oc bracing.

REACTIONS (lb/size) 2=1245/0-3-8, 8=1245/0-3-8
Max Horz 2=-101(load case 6)
Max Uplift2=-461(load case 5), 8=-461(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1976/814, 3-4=-1733/707, 4-5=-1521/689, 5-6=-1519/688, 6-7=-1734/708, 7-8=-1976/814, 8-9=0/47
BOT CHORD 2-11=-562/1719, 10-11=-456/1636, 8-10=-562/1719
WEBS 3-11=-249/211, 4-11=-115/507, 5-11=-256/167, 5-10=-258/167, 6-10=-116/507, 7-10=-248/210

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 461 lb uplift at joint 2 and 461 lb uplift at joint 8.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

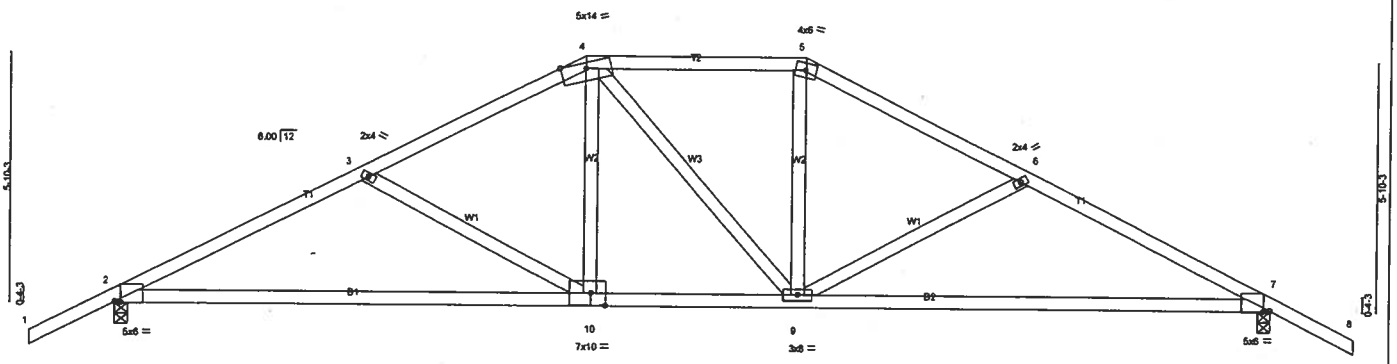
Dwg.#0111061358

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T13	HIP	1	1	
Builders FirstSource, Lake City, Fl 32055					Job Reference (optional)

6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:16:41 2006 Page 1

-2-0-0	5-9-4	11-0-0	16-2-0	21-4-12	27-2-0	29-2-0
2-0-0	5-9-4	5-2-12	5-2-0	5-2-12	5-9-4	2-0-0

Scale = 1:51.7
Camber = 1/4 in



5-9-4	11-0-0	16-2-0	21-4-12	27-2-0
5-9-4	5-2-12	5-2-0	5-2-12	5-9-4

Plate Offsets (X,Y): [2:0-1-11,Edge], [7:0-1-11,Edge], [10:0-4-1,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.38	2-10	>844	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.66	2-10	>491	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.23	Horz(TL)	0.07	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 135 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-4-1 oc bracing.

REACTIONS (lb/size) 2=1245/0-3-8, 7=1245/0-3-8
Max Horz 2=-115(load case 6)
Max Uplift2=-476(load case 5), 7=-476(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1899/825, 3-4=-1573/678, 4-5=-1356/667, 5-6=-1575/679, 6-7=-1899/825, 7-8=0/47
BOT CHORD 2-10=-561/1654, 9-10=-306/1356, 7-9=-561/1654
WEBS 3-10=-353/292, 4-10=-89/407, 4-9=-128/129, 5-9=-89/406, 6-9=-352/292

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 476 lb uplift at joint 2 and 476 lb uplift at joint 7.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

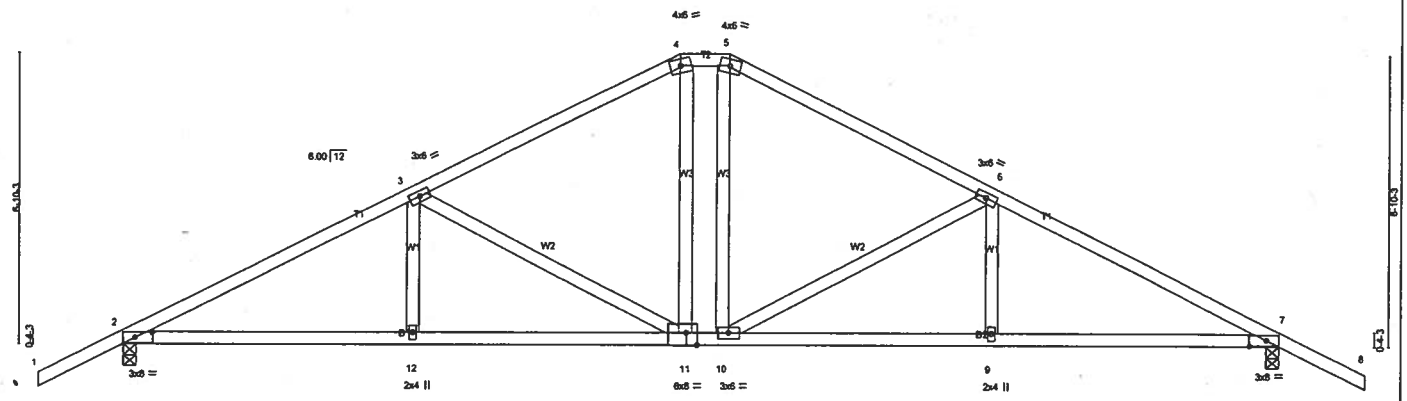
Dwg.#0111061359

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T14	HIP	1	1	
					Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:17:12 2006 Page 1

-2-0-0 | 6-9-4 | 13-0-0 | 14-2-0 | 20-4-12 | 27-2-0 | 29-2-0
2-0-0 | 6-9-4 | 6-2-12 | 1-2-0 | 6-2-12 | 6-9-4 | 2-0-0

Scale = 1:51.7
Camber = 1/16 in



6-9-4 | 13-0-0 | 14-2-0 | 20-4-12 | 27-2-0
6-9-4 | 6-2-12 | 1-2-0 | 6-2-12 | 6-9-4

Plate Offsets (X,Y): [2:0-4-12,0-1-8], [7:0-4-12,0-1-8], [11:0-2-15,0-3-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.12	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.20	11-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.59	Horz(TL)	0.07	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 141 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-7-12 oc bracing.

REACTIONS (lb/size) 2=1245/0-3-8, 7=1245/0-3-8
Max Horz 2=-129(load case 6)
Max Uplift2=-488(load case 5), 7=-488(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1991/800, 3-4=-1377/653, 4-5=-1169/653, 5-6=-1379/653, 6-7=-1991/800, 7-8=0/47
BOT CHORD 2-12=-532/1702, 11-12=-532/1702, 10-11=-241/1169, 9-10=-532/1702, 7-9=-532/1702
WEBS 3-12=0/232, 3-11=-645/334, 4-11=-139/400, 5-10=-140/399, 6-10=-644/334, 6-9=0/231

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 488 lb uplift at joint 2 and 488 lb uplift at joint 7.

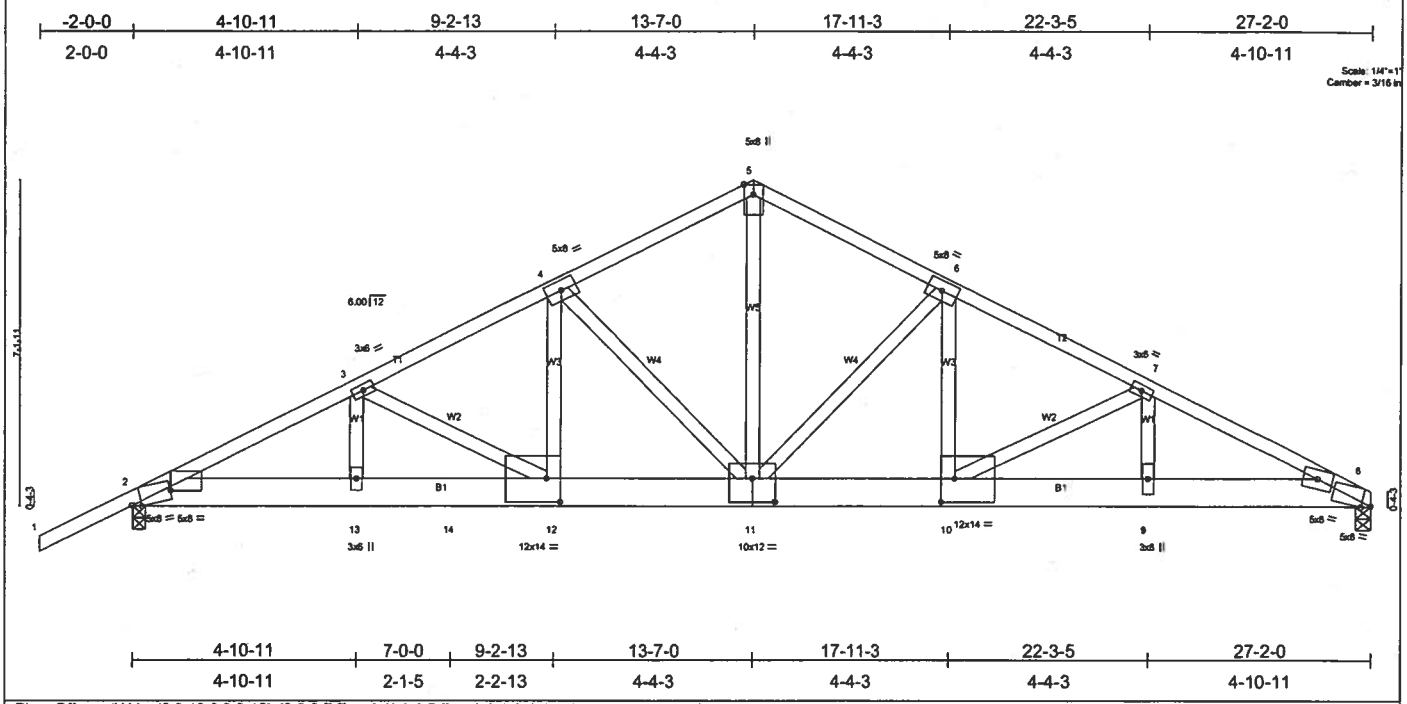
LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061360

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T15	COMMON	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:39:57 2006 Page 1



4-10-11	7-0-0	9-2-13	13-7-0	17-11-3	22-3-5	27-2-0
4-10-11	2-1-5	2-2-13	4-4-3	4-4-3	4-4-3	4-10-11

Plate Offsets (X,Y): [2:0-10-0,0-3-15], [2:0-2-7,Edge], [8:0-2-7,Edge], [10:0-3-9,0-6-0], [11:0-6-0,0-6-0], [12:0-3-8,0-6-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.86	Vert(LL)	-0.34	12-13	>936	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.55	12-13	>585	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.86	Horz(TL)	0.12	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 381 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except* T2 2 X 4 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except* W5 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-5-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=9184/0-4-0, 2=7664/0-3-8
Max Horz 2=166(load case 4)
Max Uplift8=-3409(load case 5), 2=-2916(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-16379/5996, 3-4=-14578/5410, 4-5=-10766/4056, 5-6=-10765/4046, 6-7=-14004/5212, 7-8=-17032/6306
BOT CHORD 2-13=-5409/14642, 13-14=-5409/14642, 12-14=-5409/14642, 11-12=-4770/13006, 10-11=-4500/12490, 9-10=-5596/15241, 8-9=-5596/15241
WEBS 3-13=-437/1384, 3-12=-1862/725, 4-12=-1835/5012, 4-11=-4900/1913, 5-11=-3455/9288, 6-11=-4159/1644, 6-10=-1551/4228, 7-10=-3126/1246, 7-9=-890/2499

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 3 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 3409 lb uplift at joint 8 and 2916 lb uplift at joint 2.
6) Girder carries tie-in span(s): 30-2-8 from 7-0-0 to 27-2-0
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2752 lb down and 1039 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-8=-54, 2-14=-30, 8-14=-616(F=-586)
Concentrated Loads (lb)
Vert: 14=-2752(F)

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061361

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T16	SPECIAL	1	1	
Builders FirstSource, Lake City, Fl 32055					Job Reference (optional)

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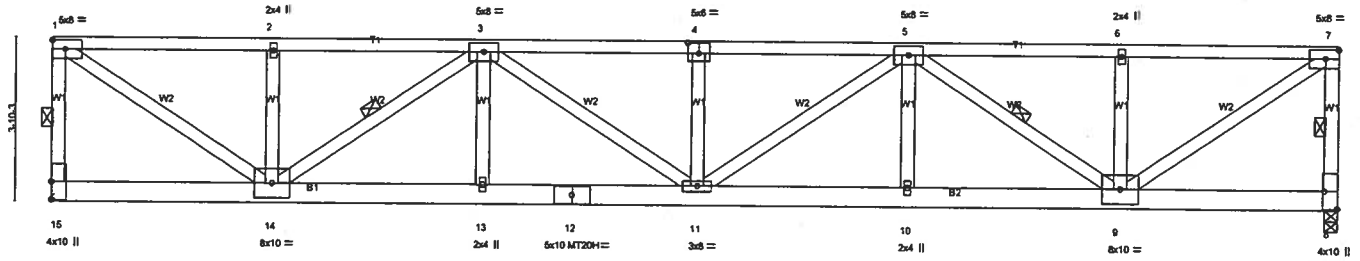
5-1-9	10-1-7	15-1-4	20-1-1	25-0-15	30-2-8
5-1-9	4-11-13	4-11-13	4-11-13	4-11-13	5-1-9

Scale = 1:51.5
Camber = 3/16 in



5-1-9	10-1-7	15-1-4	20-1-1	25-0-15	30-2-8
5-1-9	4-11-13	4-11-13	4-11-13	4-11-13	5-1-9

Plate Offsets (X,Y): [4:0-3-0,0-3-0], [8:Edge,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.89	Vert(LL)	-0.36	11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.57	11	>629	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.79	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 198 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2, W2 2 X 4 SYP No.2, W2 2 X 4 SYP No.2, W2 2 X 4 SYP No.2
W2 2 X 4 SYP No.2, W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-1-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-11-0 oc bracing.
WEBS 1 Row at midpt 1-15, 7-8, 3-14, 5-9

REACTIONS (lb/size) 15=2752/Mechanical, 8=2752/0-3-8
Max Uplift15=-1039(load case 2), 8=-1039(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-15=-2556/999, 1-2=-3361/1267, 2-3=-3361/1267, 3-4=-5981/2257, 4-5=-5981/2257, 5-6=-3361/1267, 6-7=-3361/1267, 7-8=-2556/999
BOT CHORD 14-15=-45/105, 13-14=-2026/5364, 12-13=-2026/5364, 11-12=-2026/5364, 10-11=-2026/5364, 9-10=-2026/5364, 8-9=-45/105
WEBS 1-14=-1489/3968, 2-14=-594/316, 3-14=-2440/925, 3-13=-58/369, 3-11=-282/752, 4-11=-576/304, 5-11=-282/752, 5-10=-58/369, 5-9=-2440/925, 6-9=-594/316, 7-9=-1489/3968

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1039 lb uplift at joint 15 and 1039 lb uplift at joint 8.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-7=-118(F=-64), 8-15=-66(F=-36)

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061362

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T17	SPECIAL	1	1	Job Reference (optional)

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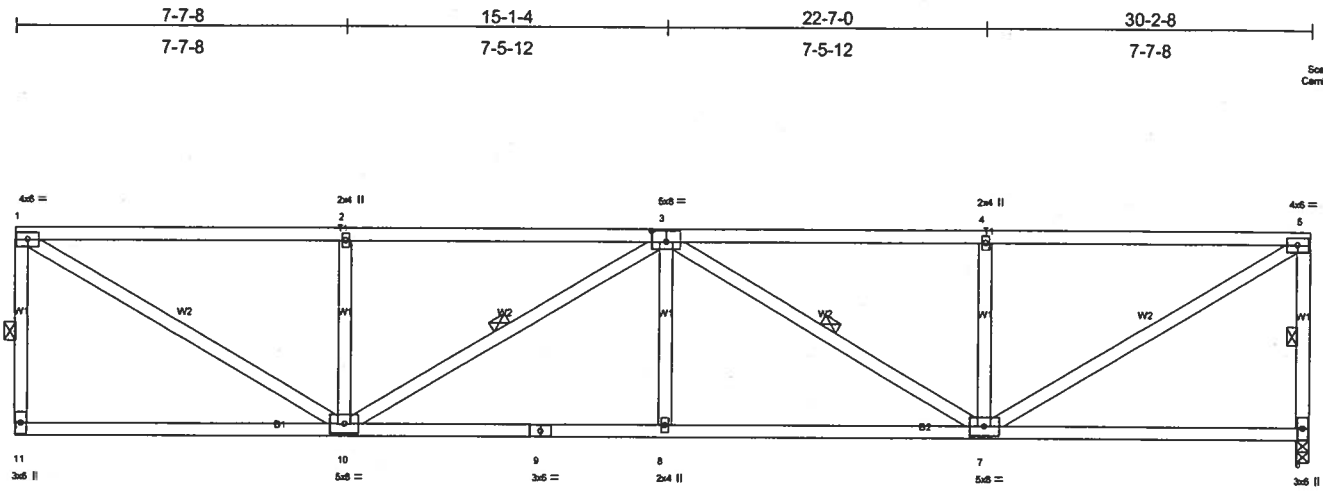

Plate Offsets (X,Y): [3:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	-0.17	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.28	8-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.95	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 171 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-11 oc bracing.
WEBS 1 Row at midpt 1-11, 5-6, 3-10, 3-7

REACTIONS (lb/size) 11=1256/Mechanical, 6=1256/0-3-8
Max Uplift11=-474(load case 3), 6=-474(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-11=-1144/506, 1-2=-1633/638, 2-3=-1633/638, 3-4=-1633/638, 4-5=-1633/638, 5-6=-1144/506
BOT CHORD 10-11=-27/68, 9-10=-837/2143, 8-9=-837/2143, 7-8=-837/2143, 6-7=-27/68
WEBS 1-10=-716/1832, 2-10=-418/306, 3-10=-596/233, 3-8=0/224, 3-7=-596/233, 4-7=-418/306, 5-7=-716/1832

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 474 lb uplift at joint 11 and 474 lb uplift at joint 6.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061363

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T18	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:40:32 2006 Page 1

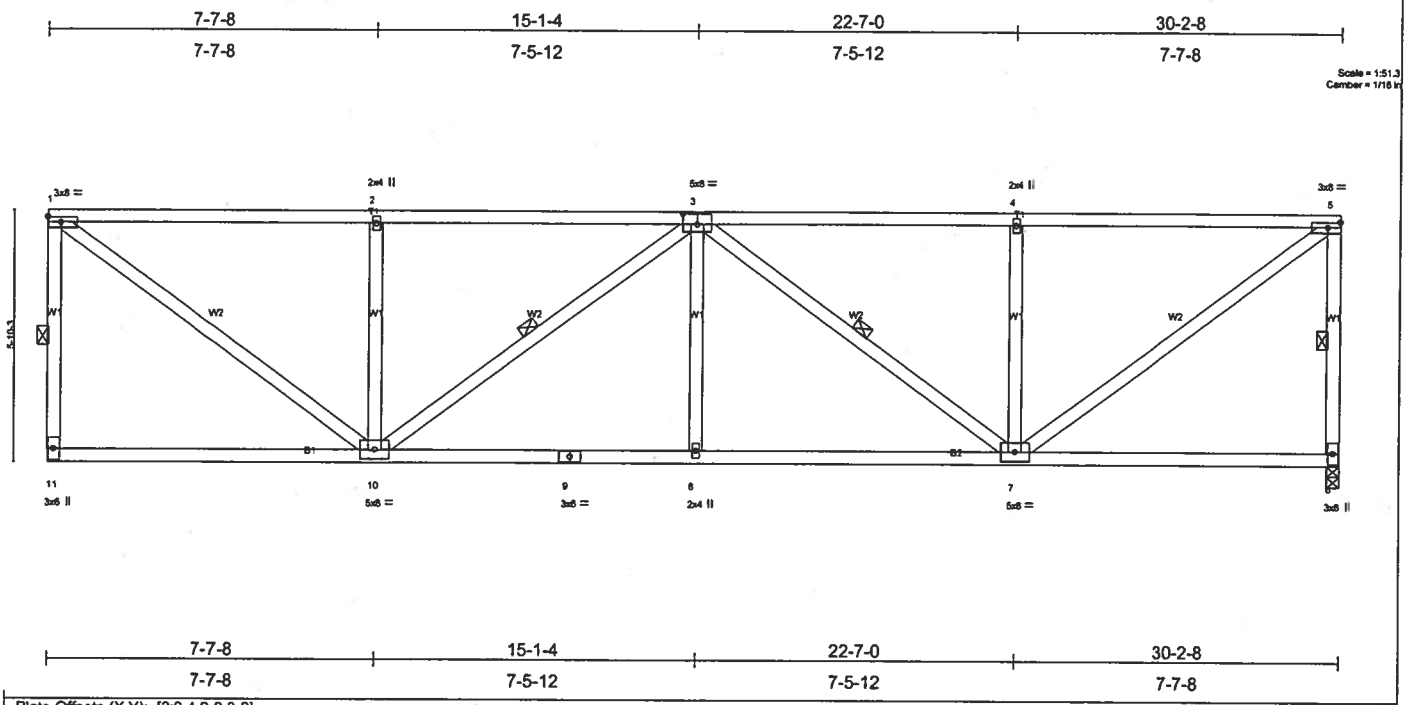

Plate Offsets (X,Y): [3:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.14	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.22	8-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.94	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 182 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-7-3 oc bracing.
WEBS 1 Row at midpt 1-11, 5-6, 3-10, 3-7

REACTIONS (lb/size) 11=1256/Mechanical, 6=1256/0-3-8
Max Uplift11=-474(load case 3), 6=-474(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-11=-1146/506, 1-2=-1342/524, 2-3=-1342/524, 3-4=-1342/524, 4-5=-1342/524, 5-6=-1146/506
BOT CHORD 10-11=-20/52, 9-10=-687/1757, 8-9=-687/1757, 7-8=-687/1757, 6-7=-20/52
WEBS 1-10=-627/1606, 2-10=-422/308, 3-10=-518/203, 3-8=0/223, 3-7=-518/203, 4-7=-422/308, 5-7=-627/1606

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 474 lb uplift at joint 11 and 474 lb uplift at joint 6.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061364

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T19	SPECIAL	1	1	Job Reference (optional)

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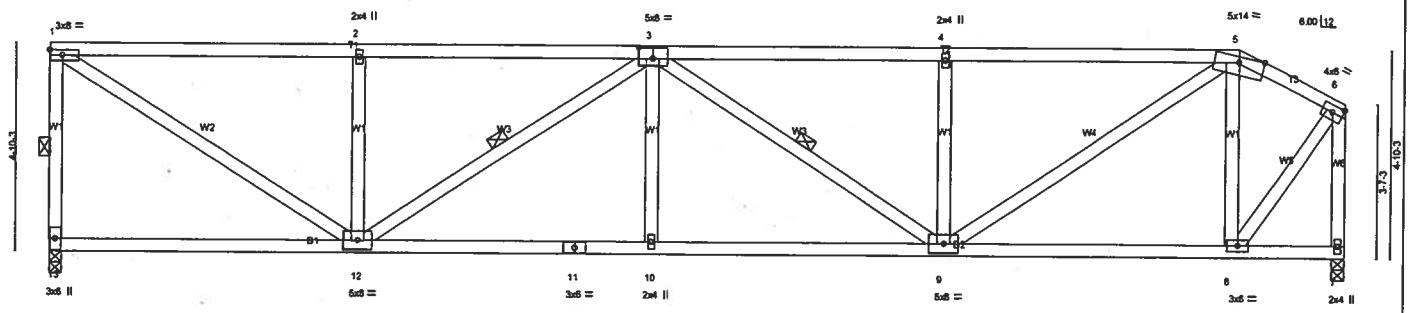
7-1-12	14-0-0	20-10-4	27-8-8	30-2-8
7-1-12	6-10-4	6-10-4	6-10-4	2-6-0

Scale = 1:51.3
Camber = 1/8 in



7-1-12	14-0-0	20-10-4	27-8-8	30-2-8
7-1-12	6-10-4	6-10-4	6-10-4	2-6-0

Plate Offsets (X,Y): [3:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.16	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.26	9-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.85	Horz(TL)	0.06	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 179 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-1-1 oc bracing.
WEBS 1 Row at midpt 1-13, 3-12, 3-9

REACTIONS (lb/size) 13=1256/0-3-8, 7=1256/0-3-8
Max Horz 13=-55(load case 6)
Max Uplift13=-477(load case 3), 7=-429(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-13=-1151/513, 1-2=-1553/619, 2-3=-1553/619, 3-4=-1830/753, 4-5=-1830/753, 5-6=-760/305, 6-7=-1251/489
BOT CHORD 12-13=-6/60, 11-12=-793/2118, 10-11=-793/2118, 9-10=-793/2118, 8-9=-246/657, 7-8=-3/3
WEBS 1-12=-710/1781, 2-12=-388/283, 3-12=-679/281, 3-10=0/204, 3-9=-346/132, 4-9=-379/273, 5-9=-536/1407, 5-8=-748/380, 6-8=-429/1138

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 477 lb uplift at joint 13 and 429 lb uplift at joint 7.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

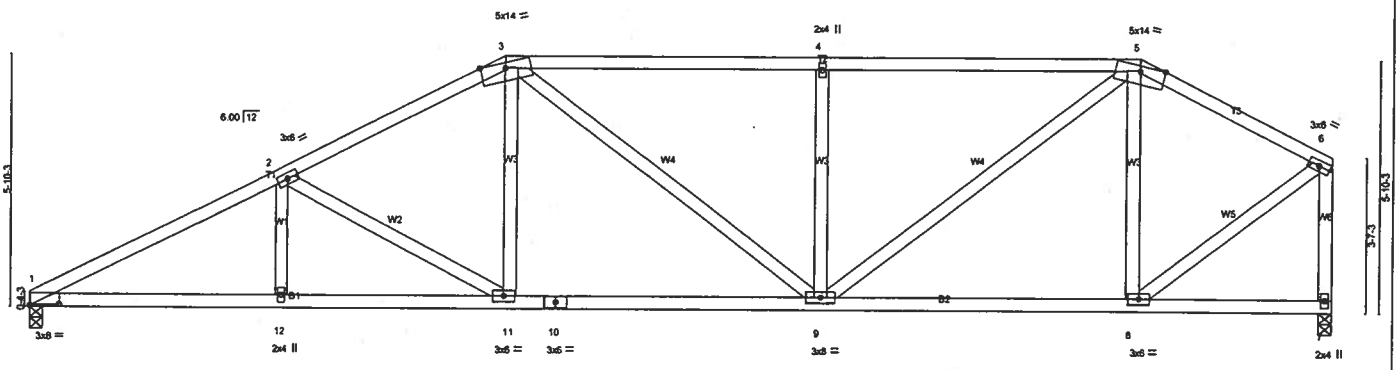
Dwg.#0111061365

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T20	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:42:02 2006 Page 1

5-9-4	11-0-0	18-4-4	25-8-8	30-2-8
5-9-4	5-2-12	7-4-4	7-4-4	4-6-0

Scale = 1:51.0
Camber = 1/8 in



5-9-4	11-0-0	18-4-4	25-8-8	30-2-8
5-9-4	5-2-12	7-4-4	7-4-4	4-6-0

Plate Offsets (X,Y): [1:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.15	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.25	9-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.07	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-4-8 oc bracing.

REACTIONS (lb/size) 1=1256/0-3-8, 7=1256/0-3-8
Max Horz 1=186(load case 5)
Max Uplift1=-376(load case 5), 7=-336(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2350/952, 2-3=-1858/808, 3-4=-1684/786, 4-5=-1684/786, 5-6=-1054/457, 6-7=-1204/529
BOT CHORD 1-12=-935/2029, 11-12=-935/2029, 10-11=-672/1620, 9-10=-672/1620, 8-9=-348/896, 7-8=-15/20
WEBS 2-12=0/180, 2-11=-479/302, 3-11=-105/417, 3-9=-141/221, 4-9=-424/300, 5-9=-409/1015, 5-8=-479/296, 6-8=-426/1110

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 376 lb uplift at joint 1 and 336 lb uplift at joint 7.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

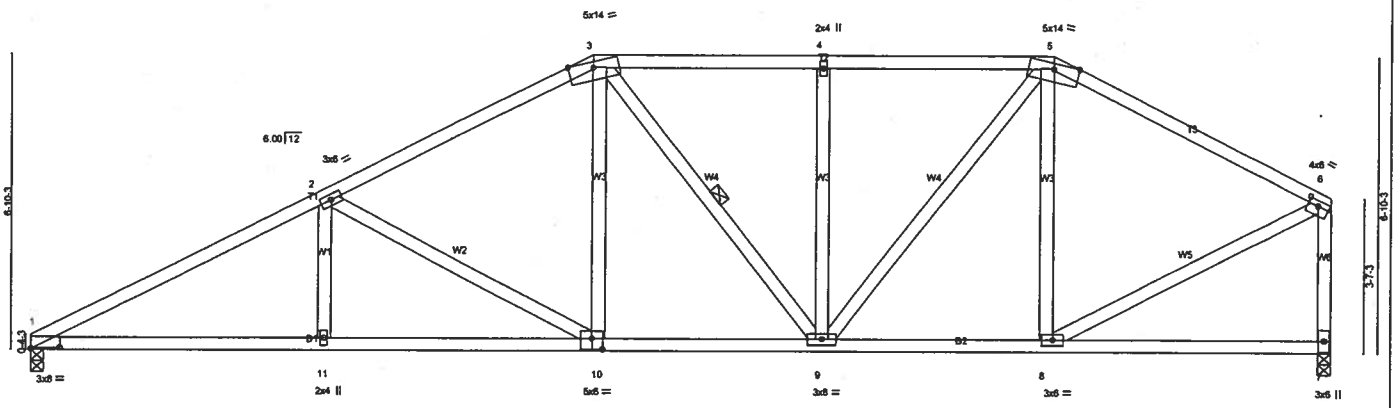
Dwg.#0111061366

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T21	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:42:14 2006 Page 1

6-9-4	13-0-0	18-4-4	23-8-8	30-2-8
6-9-4	6-2-12	5-4-4	5-4-4	6-6-0

Scale = 1:51.0
Camber = 1/16 in



6-9-4	13-0-0	18-4-4	23-8-8	30-2-8
6-9-4	6-2-12	5-4-4	5-4-4	6-6-0

Plate Offsets (X,Y): [1:0-8-0,0-0-6], [6:0-3-0,0-1-8], [10:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.13	1-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.21	1-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.58	Horz(TL)	0.06	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 175 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-4-2 oc bracing.
WEBS 1 Row at midpt 3-9

REACTIONS (lb/size) 1=1256/0-3-8, 7=1256/0-3-8
Max Horz 1=200(load case 5)
Max Uplift1=-390(load case 5), 7=-343(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2313/946, 2-3=-1683/759, 3-4=-1390/706, 4-5=-1390/706, 5-6=-1204/536, 6-7=-1156/544
BOT CHORD 1-11=-919/1991, 10-11=-919/1991, 9-10=-584/1443, 8-9=-385/1002, 7-8=-44/75
WEBS 2-11=0/231, 2-10=-637/384, 3-10=-141/471, 3-9=-112/100, 4-9=-290/209, 5-9=-251/673, 5-8=-304/225, 6-8=-389/1052

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 390 lb uplift at joint 1 and 343 lb uplift at joint 7.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

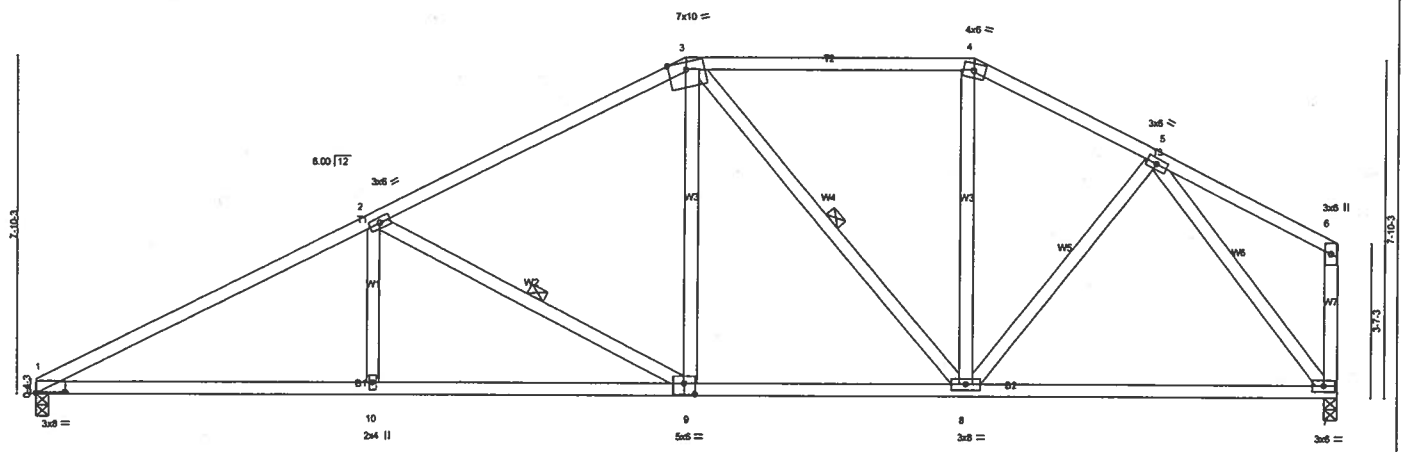
Dwg.#0111061367

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T22	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:42:21 2006 Page 1

7-9-4	15-0-0	21-8-8	25-9-12	30-2-8
7-9-4	7-2-12	6-8-8	4-1-4	4-4-12

Scale = 1:51.0
Camber = 1/8 in



7-9-4	15-0-0	21-8-8	30-2-8
7-9-4	7-2-12	6-8-8	8-6-0

Plate Offsets (X,Y): [1:0-8-0,0-0-6], [9:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.18	1-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.30	1-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.99	Horz(TL)	0.07	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 172 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-3-8 oc bracing.
WEBS 1 Row at midpt 2-9, 3-8

REACTIONS (lb/size) 1=1256/0-3-8, 7=1256/0-3-8
Max Horz 1=214(load case 5)
Max Uplift1=-402(load case 5), 7=-361(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2262/935, 2-3=-1520/714, 3-4=-1051/595, 4-5=-1210/610, 5-6=-139/73, 6-7=-162/111
BOT CHORD 1-10=-897/1939, 9-10=-897/1939, 8-9=-504/1287, 7-8=-355/814
WEBS 2-10=0/272, 2-9=-751/449, 3-9=-167/536, 3-8=-427/191, 4-8=-63/223, 5-8=-103/429, 5-7=-1218/570

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 402 lb uplift at joint 1 and 361 lb uplift at joint 7.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

Dwg.#0111061368

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
TEMP	T23	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:45:56 2006 Page 1

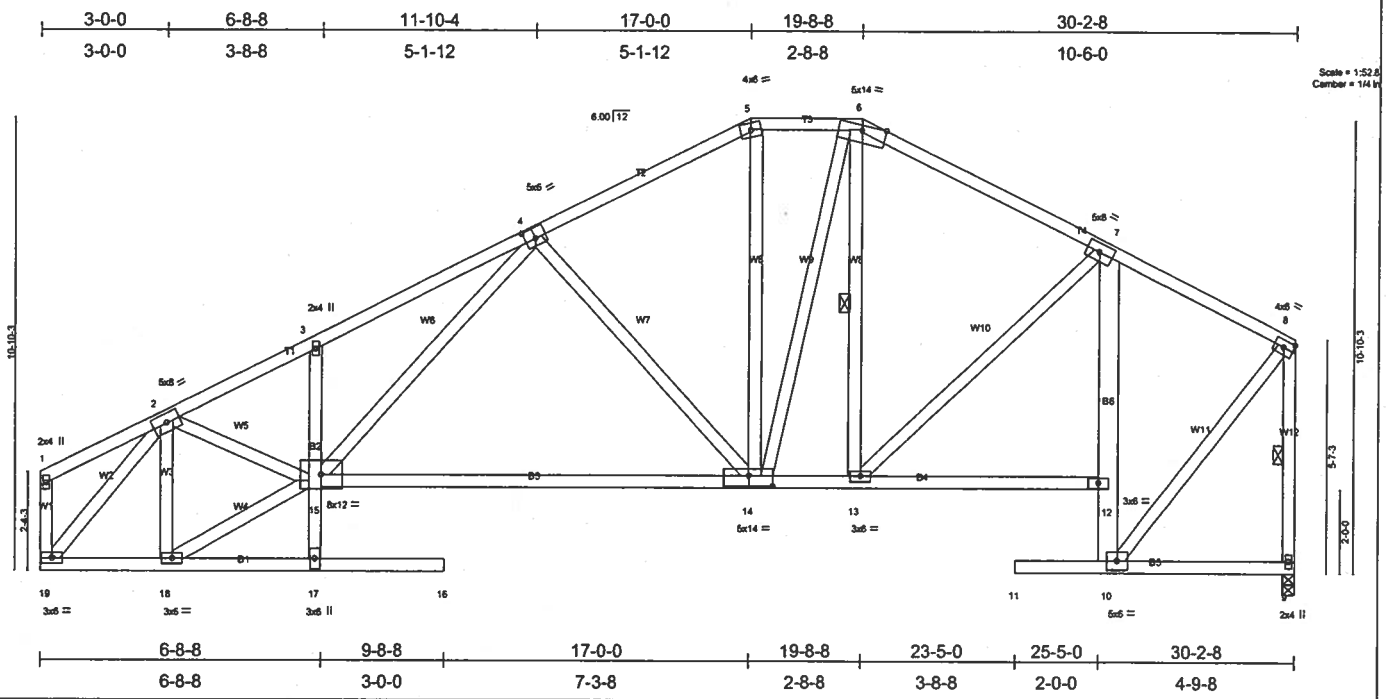


Plate Offsets (X,Y): [4:0-3-0,0-3-0], [14:0-7-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.33	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.86	Vert(TL)	-0.56	14-15	>646	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.72	Horz(TL)	0.37	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 241 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B6 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-13, 8-9

REACTIONS (lb/size) 9=1336/0-3-8, 19=1338/Mechanical
Max Horz 19=223(load case 5)
Max Uplift9=-362(load case 6), 19=-380(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-92/69, 2-3=-2454/990, 3-4=-2529/1122, 4-5=-1388/690, 5-6=-1180/669, 6-7=-1287/646, 7-8=-861/376, 1-19=-109/86, 8-9=-1354/579
BOT CHORD 18-19=-523/991, 17-18=-110/0, 16-17=0/0, 15-17=0/171, 3-15=-255/244, 14-15=-691/1601, 13-14=-372/1076, 12-13=-361/950, 10-12=-632/330, 7-12=-576/368, 10-11=0/0, 9-10=-7/11
WEBS 4-14=-638/433, 5-14=-159/386, 6-14=-153/460, 6-13=-52/69, 7-13=-76/233, 8-10=-421/1100, 2-18=-554/302, 2-15=-444/1269, 15-18=-542/1244, 4-15=-369/924, 2-19=-1500/535

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 362 lb uplift at joint 9 and 380 lb uplift at joint 19.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

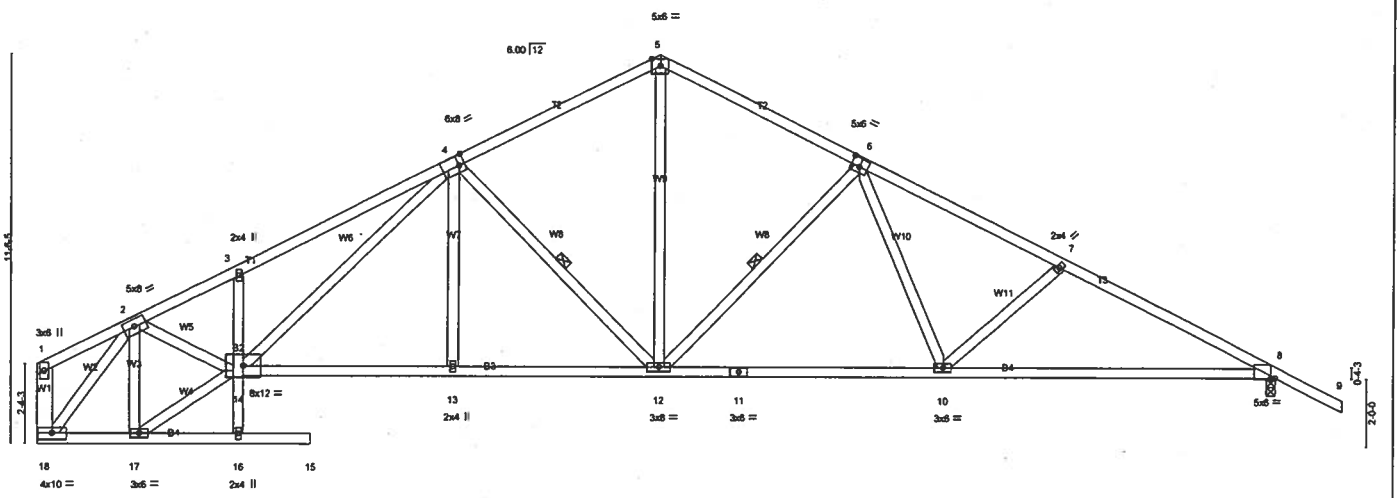
Dwg.#0111061369

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T24	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:51:08 2006 Page 1

3-0-0	6-0-5	12-5-0	18-4-4	24-3-8	30-2-12	36-8-8	38-8-8
3-0-0	3-0-5	6-4-11	5-11-4	5-11-4	5-11-4	6-5-12	2-0-0

Scale = 1:65.2
Camber = 3/16 in



6-0-5	8-0-5	18-4-4	26-9-7	36-8-8
6-0-5	2-0-0	10-3-15	8-5-3	9-11-1

Plate Offsets (X,Y): [4:0-2-0,Edge], [6:0-3-0,0-3-0], [8:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.29	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.48	8-10	>902	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.83	Horz(TL)	0.19	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 230 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D, B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-8-5 oc bracing.
WEBS 1 Row at midpt 4-12, 6-12

REACTIONS (lb/size) 8=1645/0-3-8, 18=1523/Mechanical
Max Horz 18=-196(load case 6)
Max Uplift8=-610(load case 6), 18=-489(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-131/91, 2-3=-2757/1173, 3-4=-2873/1353, 4-5=-1782/915, 5-6=-1780/913, 6-7=-2531/1121, 7-8=-2781/1203, 8-9=0/47, 1-18=-144/108
BOT CHORD 17-18=-423/1065, 16-17=-36/39, 15-16=0/0, 14-16=-4/63, 3-14=-302/299, 13-14=-624/2022, 12-13=-624/2020, 11-12=-623/1997, 10-11=-623/1997, 8-10=-893/2427
WEBS 4-14=-409/674, 4-12=-725/415, 5-12=-555/1201, 6-12=-691/414, 6-10=-144/548, 7-10=-314/293, 2-14=-557/1539, 14-17=-466/1230, 2-17=-643/299, 2-18=-1640/683, 4-13=0/191

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 610 lb uplift at joint 8 and 489 lb uplift at joint 18.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ. FL 33549

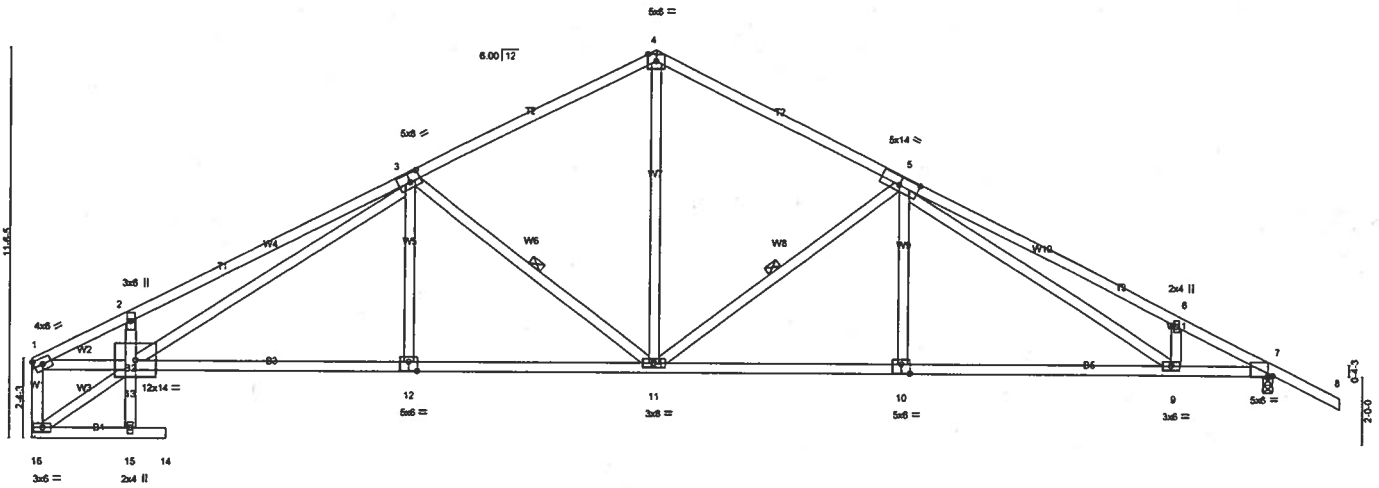
Dwg.#0111061370

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
J-145800	T25	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:54:59 2006 Page 1

3-0-0	11-1-7	18-4-4	25-7-1	30-2-12	36-8-8	38-8-8
3-0-0	8-1-7	7-2-13	7-2-13	4-7-11	6-5-12	2-0-0

Scale = 1:65.0
Camber = 3/16 in



3-0-0	3-10-11	11-1-7	20-0-5	33-8-8	36-8-8
3-0-0	0-10-11	7-2-13	8-10-14	13-8-3	3-0-0

Plate Offsets (X,Y): [3:0-3-12,0-3-0], [5:0-7-0,0-3-0], [7:0-1-11,Edge], [10:0-3-0,0-3-4], [12:0-3-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.77	Vert(LL)	-0.27	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.72	Vert(TL)	-0.45	9-10	>973	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.91	Horz(TL)	0.29	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 221 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-1-0 oc bracing.
WEBS 1 Row at midpt 3-11, 5-11

REACTIONS (lb/size) 16=1554/Mechanical, 7=1652/0-3-8
Max Horz 16=-195(load case 6)
Max Uplift16=-485(load case 5), 7=-611(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2975/1128, 2-3=-3306/1446, 3-4=-1820/910, 4-5=-1821/911, 5-6=-3091/1379, 6-7=-3061/1209, 7-8=0/47, 1-16=-1500/572
BOT CHORD 15-16=-54/50, 14-15=0/0, 13-15=0/72, 2-13=-400/393, 12-13=-707/2188, 11-12=-707/2191, 10-11=-709/2176, 9-10=-709/2179, 7-9=-968/2683
WEBS 3-13=-540/850, 3-12=0/253, 3-11=-832/458, 4-11=-510/1175, 5-11=-813/461, 5-9=-313/635, 6-9=-227/305, 13-16=-83/272, 1-13=-981/2600, 5-10=0/260

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 485 lb uplift at joint 16 and 611 lb uplift at joint 7.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B LUTZ, FL 33549

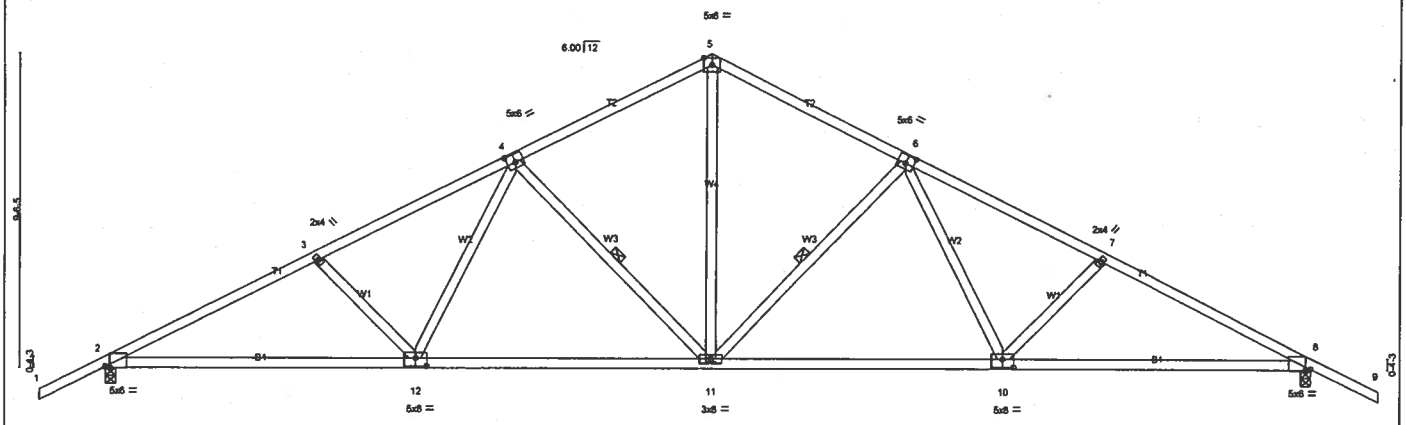
Dwg.#0111061371

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T26	COMMON	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 09:55:59 2006 Page 1

-2-0-0	6-5-12	12-5-0	18-4-4	24-3-8	30-2-12	36-8-8	38-8-8
2-0-0	6-5-12	5-11-4	5-11-4	5-11-4	5-11-4	6-5-12	2-0-0

Scale = 1:66.8
Camber = 3/16 in



0-1-14	9-5-6	18-4-4	27-3-2	36-8-8
0-1-14	9-3-8	8-10-14	8-10-14	9-5-6

Plate Offsets (X,Y): [2:0-1-11,Edge], [4:0-3-0,0-3-0], [6:0-3-0,0-3-0], [8:0-1-11,Edge], [10:0-4-0,0-3-0], [12:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.26	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.43	8-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.82	Horz(TL)	0.13	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 193 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-8-5 oc bracing.
WEBS 1 Row at midpt 4-11, 6-11

REACTIONS (lb/size) 2=1645/0-3-8, 8=1645/0-3-8
Max Horz 2=-167(load case 6)
Max Uplift2=-621(load case 5), 8=-621(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2792/1202, 3-4=-2567/1142, 4-5=-1788/914, 5-6=-1788/914, 6-7=-2567/1142, 7-8=-2792/1202, 8-9=0/47
BOT CHORD 2-12=-892/2434, 11-12=-625/1991, 10-11=-625/1991, 8-10=-892/2434
WEBS 3-12=-301/287, 4-12=-161/562, 4-11=-672/415, 5-11=-550/1199, 6-11=-672/415, 6-10=-161/562, 7-10=-301/287

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 621 lb uplift at joint 2 and 621 lb uplift at joint 8.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B. LUTZ, FL 33549

Dwg.#0111061372

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T27	SPECIAL	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 10:24:33 2006 Page 1

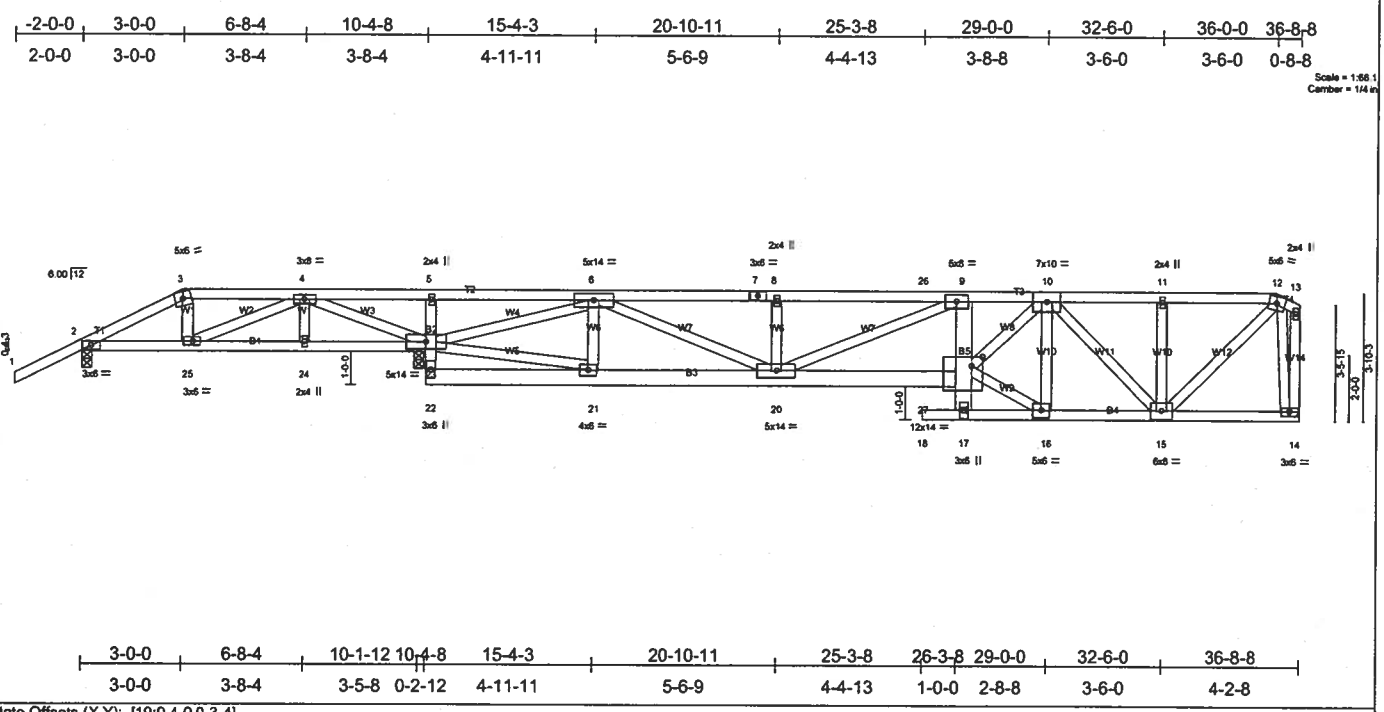


Plate Offsets (X,Y): [19:0-4-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.83	Vert(LL)	-0.36	18	>857	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.96	Vert(TL)	-0.58	18	>535	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.90	Horz(TL)	0.10	14	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 463 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 6 SYP No.1D, B5 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W8 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-8-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=350/0-3-8, 14=3243/Mechanical, 23=5109/0-3-8
Max Horz 2=143(load case 4)
Max Uplift2=-292(load case 4), 14=-1439(load case 2), 23=-2257(load case 3)
Max Grav 2=350(load case 8), 14=3244(load case 9), 23=5109(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-29/120, 3-4=0/90, 4-5=-1908/4311, 5-6=-1843/4162, 6-7=-8092/3511, 7-8=-8092/3511, 8-26=-8092/3511, 9-26=-8092/3511, 9-10=-10525/4699, 10-11=-3326/1506, 11-12=-3326/1506, 12-13=-207/60, 13-14=-278/71
BOT CHORD 2-25=-66/0, 24-25=-1455/573, 23-24=-1455/573, 22-23=-35/181, 5-23=-597/306, 21-22=-107/86, 20-21=-1259/3135, 20-27=-5138/11530, 19-27=-5199/11658, 17-19=-5/237, 9-19=-483/1155, 17-18=0/0, 16-17=-328/724, 15-16=-2433/5465, 14-15=-246/532
WEBS 3-25=-342/136, 4-25=-662/1507, 4-24=-125/248, 4-23=-3097/1486, 21-23=-1367/3300, 6-23=-7663/3278, 6-21=-502/318, 6-20=-2439/5428, 8-20=-758/387, 9-20=-3749/1806, 16-19=-2434/5482, 10-19=-2941/6652, 10-16=-2643/1293, 10-15=-3049/1363, 11-15=-429/352, 12-15=-1755/3983, 12-14=-2847/1399

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 6 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2, 1439 lb uplift at joint 14 and 2257 lb uplift at joint 23.
8) Girder carries tie-in span(s): 3-0-12 from 10-4-8 to 25-3-8; 6-0-2 from 10-4-8 to 25-3-8; 4-7-13 from 0-0-0 to 10-4-8; 4-3-8 from 0-0-0 to 10-4-8
9) Girder carries hip end with 0-0-0 right side setback, 25-3-8 left side setback, and 7-0-0 end setback.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1947 lb down and 1003 lb up at 25-3-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

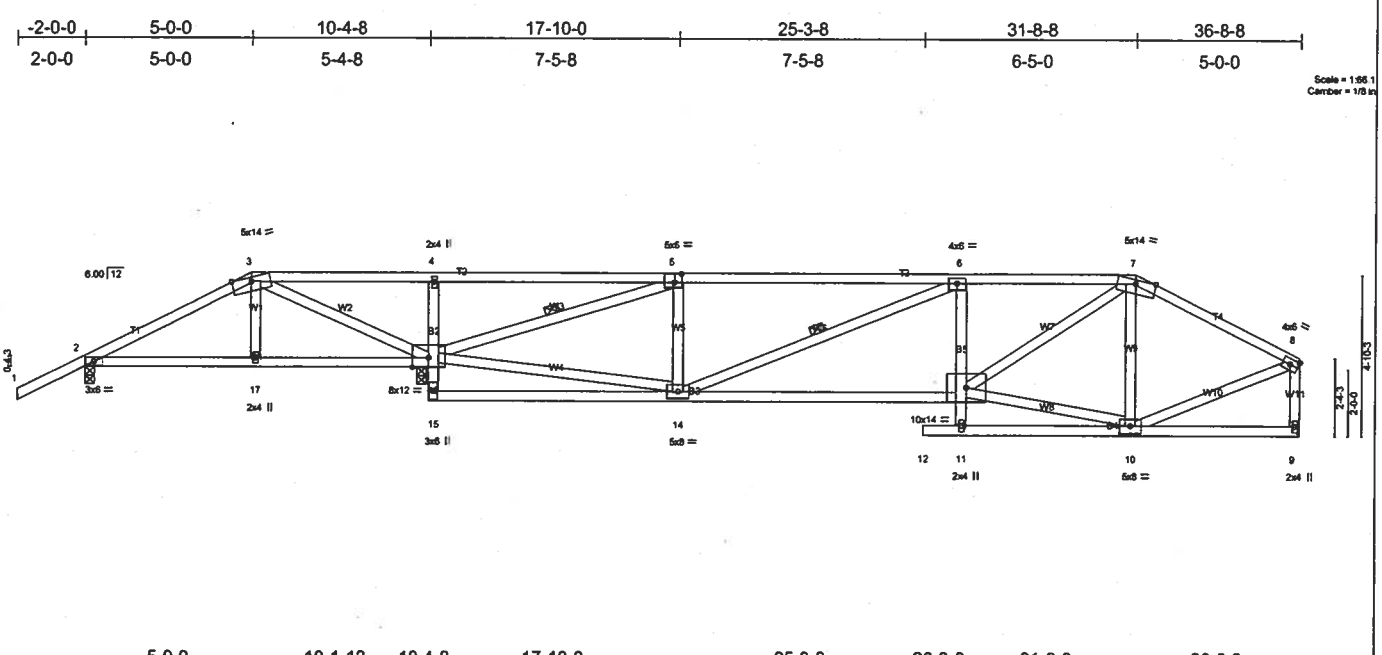
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-97(F=-43), 3-5=-97(F=-43), 5-26=-133(F=-79), 12-26=-118(F=-64), 12-13=-118(F=-64), 2-23=-80(F=-50), 22-27=-47(F=-17), 19-27=-65(F=-35), 17-18=-65(F=-35), 14-17=-65(F=-35)
Concentrated Loads (lb)
Vert: 27=-1947(F)

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061373

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T28	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 10:23:38 2006 Page 1



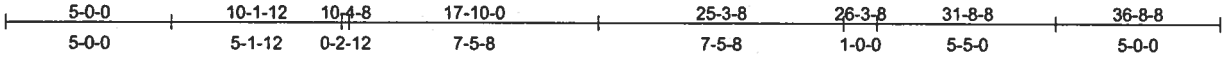


Plate Offsets (X,Y): [5:0-2-8,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.20	13-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.34	13-14	>923	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.06	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 211 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
7-9-10 oc bracing: 13-14.
WEBS 1 Row at midpt 5-16, 6-14

REACTIONS (lb/size) 2=361/0-3-8, 16=1815/0-3-8, 9=1033/Mechanical
Max Horz 2=126(load case 5)
Max Uplift2=-346(load case 5), 16=-791(load case 4), 9=-295(load case 3)
Max Grav 2=365(load case 9), 16=1815(load case 1), 9=1034(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-174/257, 3-4=-279/723, 4-5=-283/724, 5-6=-1346/530, 6-7=-1879/744, 7-8=-1100/431, 8-9=-957/404
BOT CHORD 2-17=-199/102, 16-17=-210/111, 15-16=0/105, 4-16=-358/247, 14-15=0/92, 13-14=-659/1969, 11-13=0/115, 6-13=-166/206, 11-12=0/0, 10-11=-72/43, 9-10=-46/76
WEBS 3-17=-182/161, 3-16=-908/629, 14-16=-503/1257, 5-16=-2179/864, 5-14=0/293, 6-14=-673/273, 10-13=-242/898, 7-13=-401/1160, 7-10=-415/211, 8-10=-293/929

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 346 lb uplift at joint 2, 791 lb uplift at joint 16 and 295 lb uplift at joint 9.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061374

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T29	SPECIAL	1	1	Job Reference (optional)

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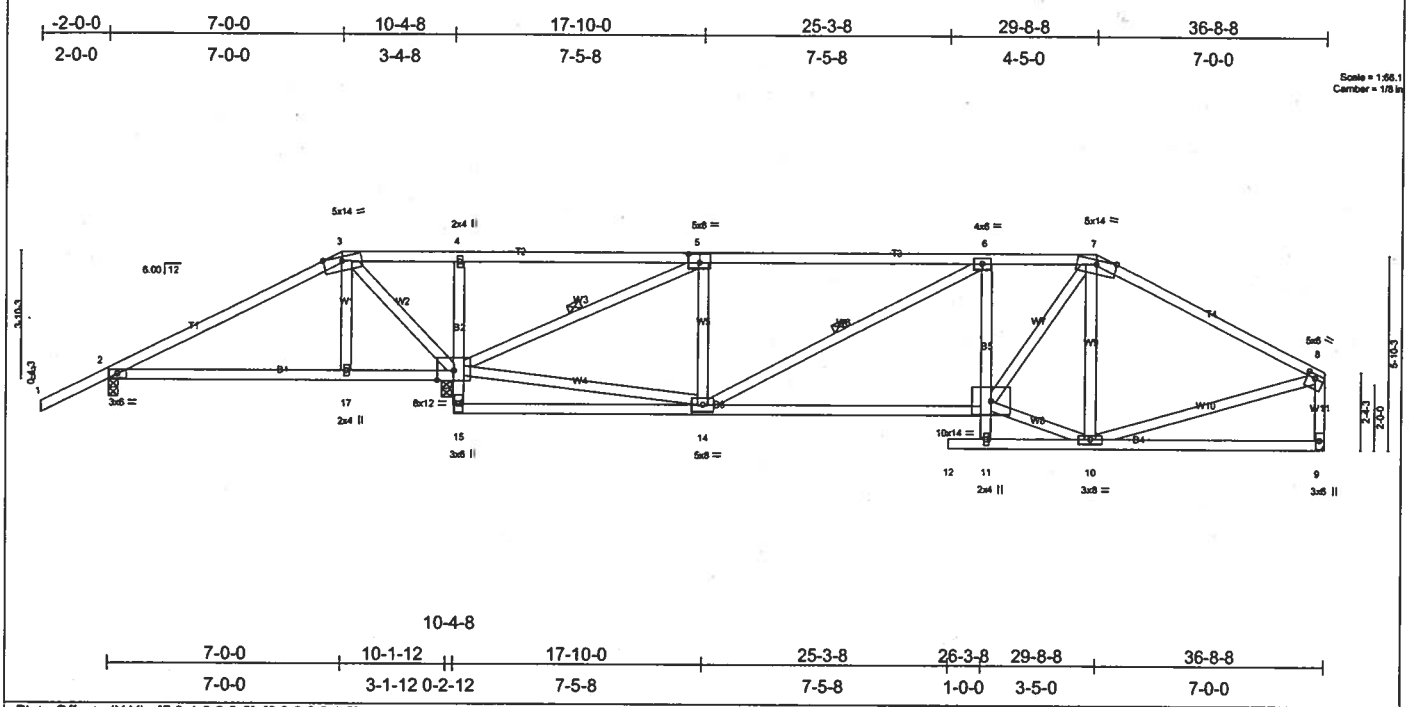

Plate Offsets (X,Y): [5:0-4-0,0-3-0], [8:0-3-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	0.13	2-17	>972	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.26	13-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.59	Horz(TL)	0.04	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 218 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-16, 6-14

REACTIONS (lb/size) 2=334/0-3-8, 16=1852/0-3-8, 9=1023/Mechanical
Max Horz 2=115(load case 5)
Max Uplift2=-345(load case 5), 16=-766(load case 4), 9=-282(load case 6)
Max Grav 2=344(load case 9), 16=1852(load case 1), 9=1027(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-37/229, 3-4=-248/653, 4-5=-219/622, 5-6=-1026/447, 6-7=-1442/609, 7-8=-1160/457, 8-9=-915/412
BOT CHORD 2-17=-179/92, 16-17=-170/87, 15-16=0/104, 4-16=-287/201, 14-15=0/129, 13-14=-469/1510, 11-13=-2/58, 6-13=-144/197, 11-12=0/0, 10-11=-86/0, 9-10=-95/164
WEBS 3-17=-246/224, 3-16=-822/637, 14-16=-318/896, 5-16=-1808/743, 5-14=0/349, 6-14=-552/222, 10-13=-233/1054, 7-13=-289/858, 7-10=-450/203, 8-10=-223/831

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 345 lb uplift at joint 2, 766 lb uplift at joint 16 and 282 lb uplift at joint 9.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0111061375

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T30	SPECIAL	1	1	Job Reference (optional)

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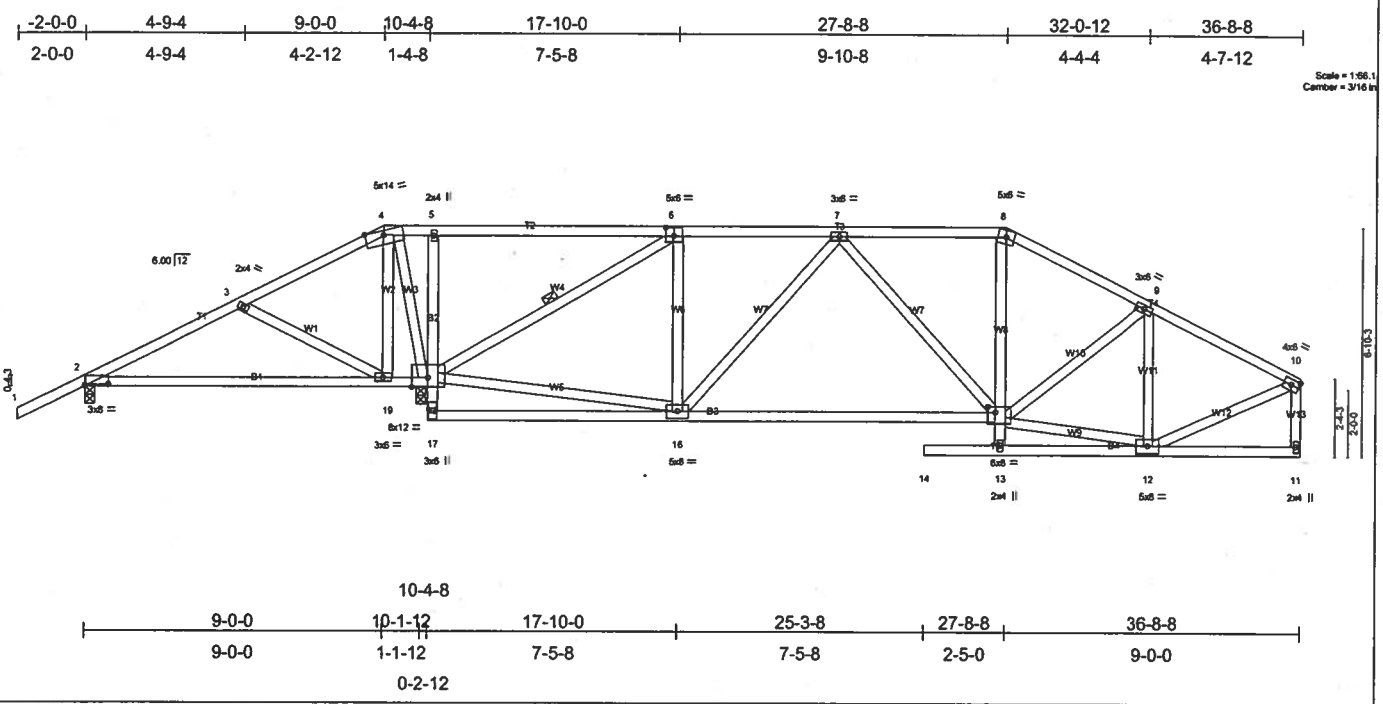


Plate Offsets (X,Y): [2:0-8-4,0-0-10], [6:0-3-0,0-3-0], [15:0-2-8,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	0.22	2-19	>560	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	-0.51	15-16	>610	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.03	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 238 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-6-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 2-19,18-19.
WEBS 1 Row at midpt 6-18

REACTIONS (lb/size) 2=313/0-3-8, 18=1892/0-3-8, 11=1033/Mechanical
Max Horz 2=130(load case 5)
Max Uplift2=-329(load case 5), 18=-718(load case 4), 11=-291(load case 6)
Max Grav 2=335(load case 9), 18=1892(load case 1), 11=1046(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-42/250, 3-4=-91/409, 4-5=-176/538, 5-6=-161/521, 6-7=-852/367, 7-8=-1116/515, 8-9=-1277/519, 9-10=-1072/414, 10-11=-972/399
BOT CHORD 2-19=-193/103, 18-19=-311/207, 17-18=0/85, 5-18=-375/251, 16-17=0/107, 15-16=-298/1093, 13-14=0/0, 12-13=-3/41, 11-12=-37/70
WEBS 3-19=-328/321, 4-19=-508/476, 4-18=-716/617, 16-18=-198/740, 6-18=-1558/623, 6-16=-88/498, 7-16=-383/229, 13-15=0/150, 8-15=-36/288, 9-15=-88/277, 10-12=-299/928, 7-15=-48/112, 9-12=-463/226, 12-15=-312/889

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 329 lb uplift at joint 2, 718 lb uplift at joint 18 and 291 lb uplift at joint 11.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

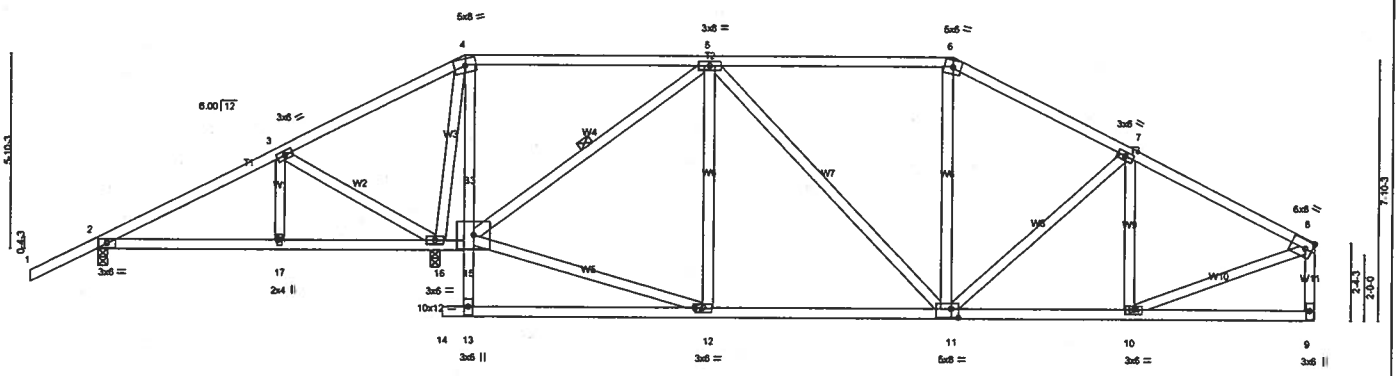
Dwg.#0111061376

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
la145800	T31	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 10:23:17 2006 Page 1

-2-0-0	5-5-8	11-0-0	18-4-4	25-8-8	31-0-12	36-8-8
2-0-0	5-5-8	5-6-8	7-4-4	7-4-4	5-4-4	5-7-12

Scale = 1:66.1



5-5-8	10-1-12	11-0-0	18-4-4	25-8-8	31-0-12	36-8-8
5-5-8	4-8-4	0-10-4	7-4-4	7-4-4	5-4-4	5-7-12

Plate Offsets (X,Y): [11:0-2-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.49	Vert(LL)	-0.08	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.14	11-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.02	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 236 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-9-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 2-17,16-17.
WEBS 1 Row at midpt 5-15
JOINTS 1 Brace at Jt(s): 15

REACTIONS (lb/size) 2=363/0-3-8, 16=1804/0-3-8, 9=1031/Mechanical
Max Horz 2=143(load case 5)
Max Uplift2=-320(load case 5), 16=-645(load case 5), 9=-319(load case 6)
Max Grav 2=384(load case 9), 16=1804(load case 1), 9=1045(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-187/174, 3-4=-184/446, 4-5=-16/157, 5-6=-887/502, 6-7=-1051/494, 7-8=-1136/462, 8-9=-957/422
BOT CHORD 2-17=-111/111, 16-17=-111/111, 15-16=-271/341, 13-15=0/128, 4-15=-106/587, 13-14=0/0, 12-13=-5/102, 11-12=-169/730, 10-11=-336/956, 9-10=-55/101
WEBS 3-17=-177/148, 3-16=-472/509, 12-15=-178/661, 5-15=-1061/518, 5-12=0/99, 5-11=-144/247, 6-11=0/98, 7-11=-98/157, 7-10=-182/147, 8-10=-301/916, 4-16=-1107/449

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 320 lb uplift at joint 2, 645 lb uplift at joint 16 and 319 lb uplift at joint 9.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

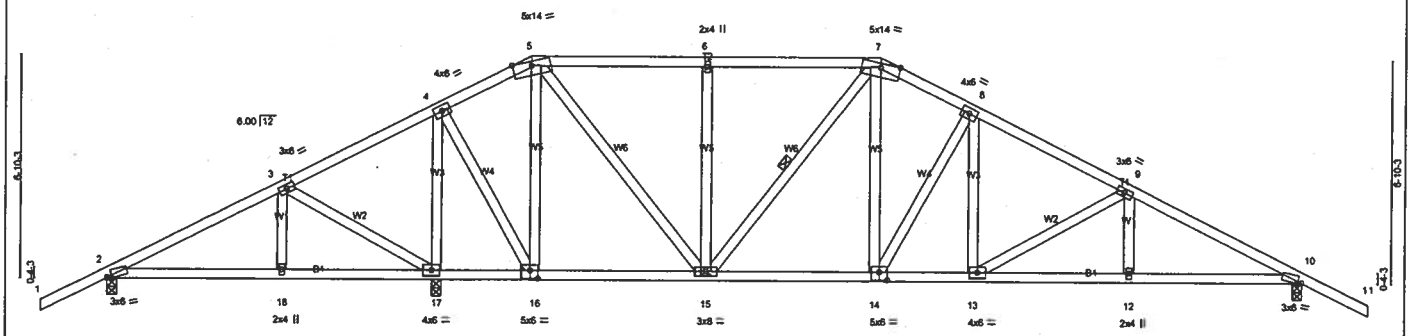
Dwg.#0111061377

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
J145800	T32	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 10:23:09 2006 Page 1

-2-0-0	5-4-2	10-1-12	13-0-0	18-4-4	23-8-8	26-6-12	31-4-6	36-8-8	38-8-8
2-0-0	5-4-2	4-9-10	2-10-4	5-4-4	5-4-4	2-10-4	4-9-10	5-4-2	2-0-0

Scale = 1:67.5



5-4-2	10-1-12	13-0-0	18-4-4	23-8-8	26-6-12	31-4-6	36-8-8
5-4-2	4-9-10	2-10-4	5-4-4	5-4-4	2-10-4	4-9-10	5-4-2

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [10:0-1-9,0-0-7], [14:0-3-0,0-3-0], [16:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.09	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.14	12-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.70	Horz(TL)	0.04	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 226 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 7-15

REACTIONS (lb/size) 2=305/0-3-8, 17=1848/0-3-8, 10=1138/0-3-8
Max Horz 2=129(load case 5)
Max Uplift2=-317(load case 5), 17=-669(load case 5), 10=-463(load case 6)
Max Grav 2=357(load case 9), 17=1848(load case 1), 10=1144(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-141/208, 3-4=-164/556, 4-5=-204/178, 5-6=-746/433, 6-7=-746/433, 7-8=-1093/554, 8-9=-1385/605, 9-10=-1831/689, 10-11=0/47
BOT CHORD 2-18=-165/77, 17-18=-165/77, 16-17=-450/420, 15-16=0/232, 14-15=-139/961, 13-14=-253/1183, 12-13=-450/1568, 10-12=-450/1568
WEBS 3-18=-183/161, 3-17=-482/503, 4-17=-1488/642, 4-16=-388/1158, 5-16=-891/385, 5-15=-400/967, 6-15=-312/222, 7-15=-348/154, 7-14=-210/518, 8-14=-461/273, 8-13=-96/326, 9-13=-442/227, 9-12=0/161

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 317 lb uplift at joint 2, 669 lb uplift at joint 17 and 463 lb uplift at joint 10.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

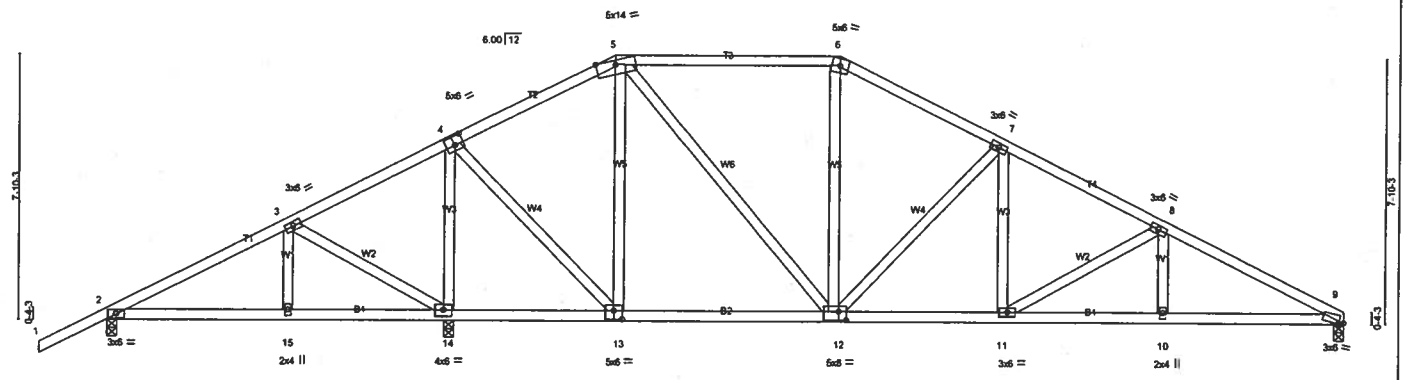
Dwg.#0111061378

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T33	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 10:25:15 2006 Page 1

-2-0-0	5-4-2	10-1-12	15-0-0	21-8-8	26-6-12	31-4-6	36-8-8
2-0-0	5-4-2	4-9-10	4-10-4	6-8-8	4-10-4	4-9-10	5-4-2

Scale = 1:65.1



5-4-2	10-1-12	15-0-0	21-8-8	26-6-12	31-4-6	36-8-8
5-4-2	4-9-10	4-10-4	6-8-8	4-10-4	4-9-10	5-4-2

Plate Offsets (X,Y): [4:0-3-0,0-3-0], [9:0-1-9,0-0-7], [12:0-2-12,0-3-0], [13:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.08	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.14	12-13	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.04	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 211 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-5-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 9=1018/0-3-8, 2=305/0-3-8, 14=1852/0-3-8
Max Horz 2=168(load case 5)
Max Uplift9=-341(load case 6), 2=-308(load case 5), 14=-703(load case 5)
Max Grav 9=1018(load case 1), 2=368(load case 9), 14=1852(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-163/202, 3-4=-209/555, 4-5=-463/276, 5-6=-810/503, 6-7=-956/499, 7-8=-1395/628, 8-9=-1864/761
BOT CHORD 2-15=-160/90, 14-15=-160/90, 13-14=-423/369, 12-13=-36/356, 11-12=-352/1192, 10-11=-601/1603, 9-10=-601/1603
WEBS 3-15=-188/158, 3-14=-479/503, 4-14=-1475/707, 4-13=-415/1146, 5-13=-651/350, 5-12=-314/699, 6-12=0/77, 7-12=-555/339, 7-11=-109/360, 8-11=-472/285, 8-10=0/175

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 341 lb uplift at joint 9, 308 lb uplift at joint 2 and 703 lb uplift at joint 14.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

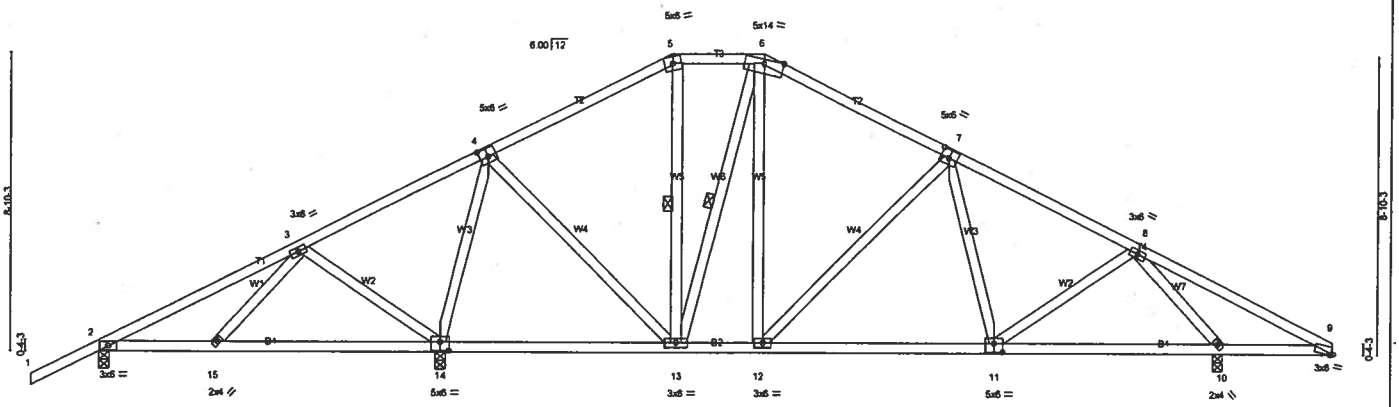
Dwg.#0111061379

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
L145800	T34	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 10:27:37 2006 Page 1

-2-0-0	6-0-5	11-6-3	17-0-0	19-8-8	25-2-5	30-8-3	36-8-8
2-0-0	6-0-5	5-5-13	5-5-13	2-8-8	5-5-13	5-5-13	6-0-5

Scale = 1:65.2



3-5-12	10-1-12	17-0-0	19-8-8	26-6-12	33-2-12	36-8-8
3-5-12	6-8-0	6-10-4	2-8-8	6-10-4	6-8-0	3-5-12

Plate Offsets (X,Y): [4:0-3-0,0-3-0], [7:0-3-0,0-3-0], [9:0-0-13,Edge], [11:0-3-0,0-3-0], [14:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.10	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.12	11-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.02	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 222 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-13, 6-13

REACTIONS (lb/size) 2=397/0-3-8, 14=1578/0-3-8, 10=1212/0-3-8
Max Horz 2=182(load case 5)
Max Uplift2=-312(load case 5), 14=-684(load case 5), 10=-501(load case 6)
Max Grav 2=422(load case 9), 14=1578(load case 1), 10=1212(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-358/307, 3-4=-157/366, 4-5=-561/322, 5-6=-437/344, 6-7=-663/376, 7-8=-950/401, 8-9=-314/368
BOT CHORD 2-15=-197/255, 14-15=-84/111, 13-14=-21/220, 12-13=0/536, 11-12=-136/773, 10-11=-135/582, 9-10=-262/342
WEBS 3-15=-284/222, 3-14=-404/429, 4-14=-1148/600, 4-13=-184/641, 5-13=-75/111, 6-13=-400/183, 6-12=-174/382, 7-12=-351/252, 7-11=0/100, 8-11=0/252, 8-10=-1301/736

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 312 lb uplift at joint 2, 684 lb uplift at joint 14 and 501 lb uplift at joint 10.

LOAD CASE(S) Standard

JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

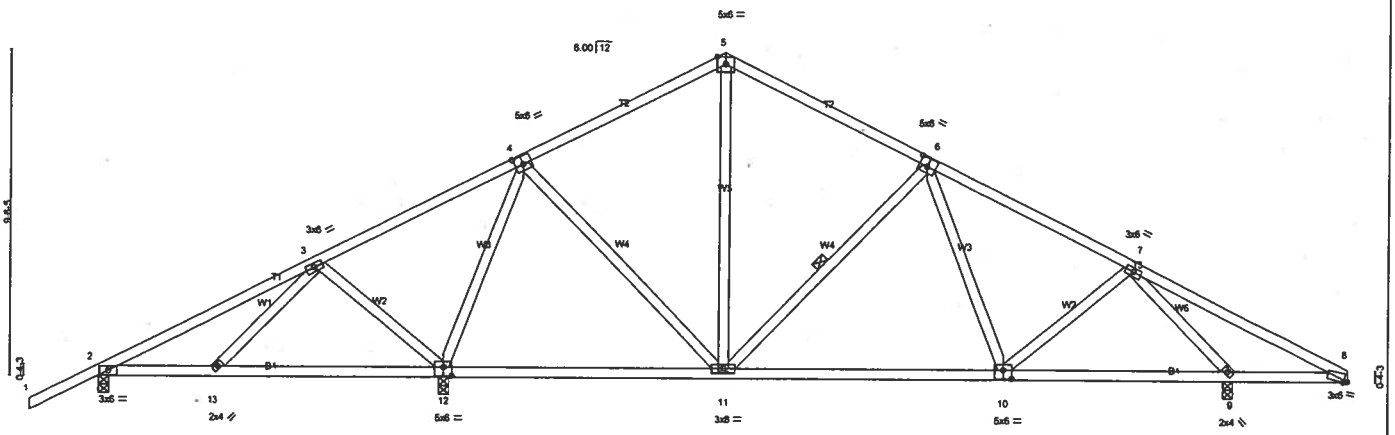
Dwg.#0111061380

Job	Truss	Truss Type	Qty	Ply	BLAKE-NOLL RES.
TEMP	T35	COMMON	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Jan 10 10:29:55 2006 Page 1

-2-0-0	6-5-12	12-5-0	18-4-4	24-3-8	30-2-12	36-8-8
2-0-0	6-5-12	5-11-4	5-11-4	5-11-4	5-11-4	6-5-12

Scale: 3/16"=1'



3-5-12	10-1-12	18-4-4	26-6-12	33-2-12	36-8-8
3-5-12	6-8-0	8-2-8	8-2-8	6-8-0	3-5-12

Plate Offsets (X,Y): [4:0-3-0,0-3-0], [6:0-3-0,0-3-0], [8:0-1-5,0-0-7], [10:0-3-0,0-3-0], [12:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.11	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.15	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.92	Horz(TL)	0.02	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 203 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-11

REACTIONS (lb/size) 2=380/0-3-8, 12=1603/0-3-8, 9=1204/0-3-8
Max Horz 2=191(load case 5)
Max Uplift2=-304(load case 5), 12=-705(load case 5), 9=-501(load case 6)
Max Grav 2=411(load case 9), 12=1603(load case 1), 9=1204(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-328/287, 3-4=-176/418, 4-5=-590/353, 5-6=-589/353, 6-7=-938/411, 7-8=-320/386
BOT CHORD 2-13=-202/224, 12-13=-100/123, 11-12=0/195, 10-11=-118/731, 9-10=-166/646, 8-9=-272/352
WEBS 3-13=-312/237, 3-12=-412/439, 4-12=-1169/614, 4-11=-137/550, 5-11=-61/180, 6-11=-409/294, 6-10=-7/135, 7-10=0/165, 7-9=-1340/757

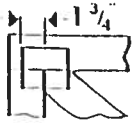
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 304 lb uplift at joint 2, 705 lb uplift at joint 12 and 501 lb uplift at joint 9.

LOAD CASE(S) Standard

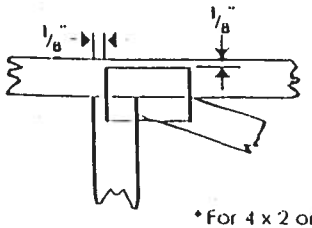
JANUARY 11, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION



* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seal.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.

* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

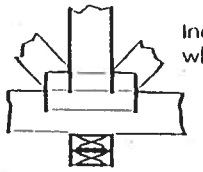
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

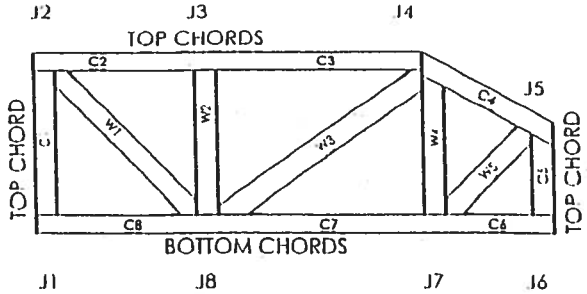
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

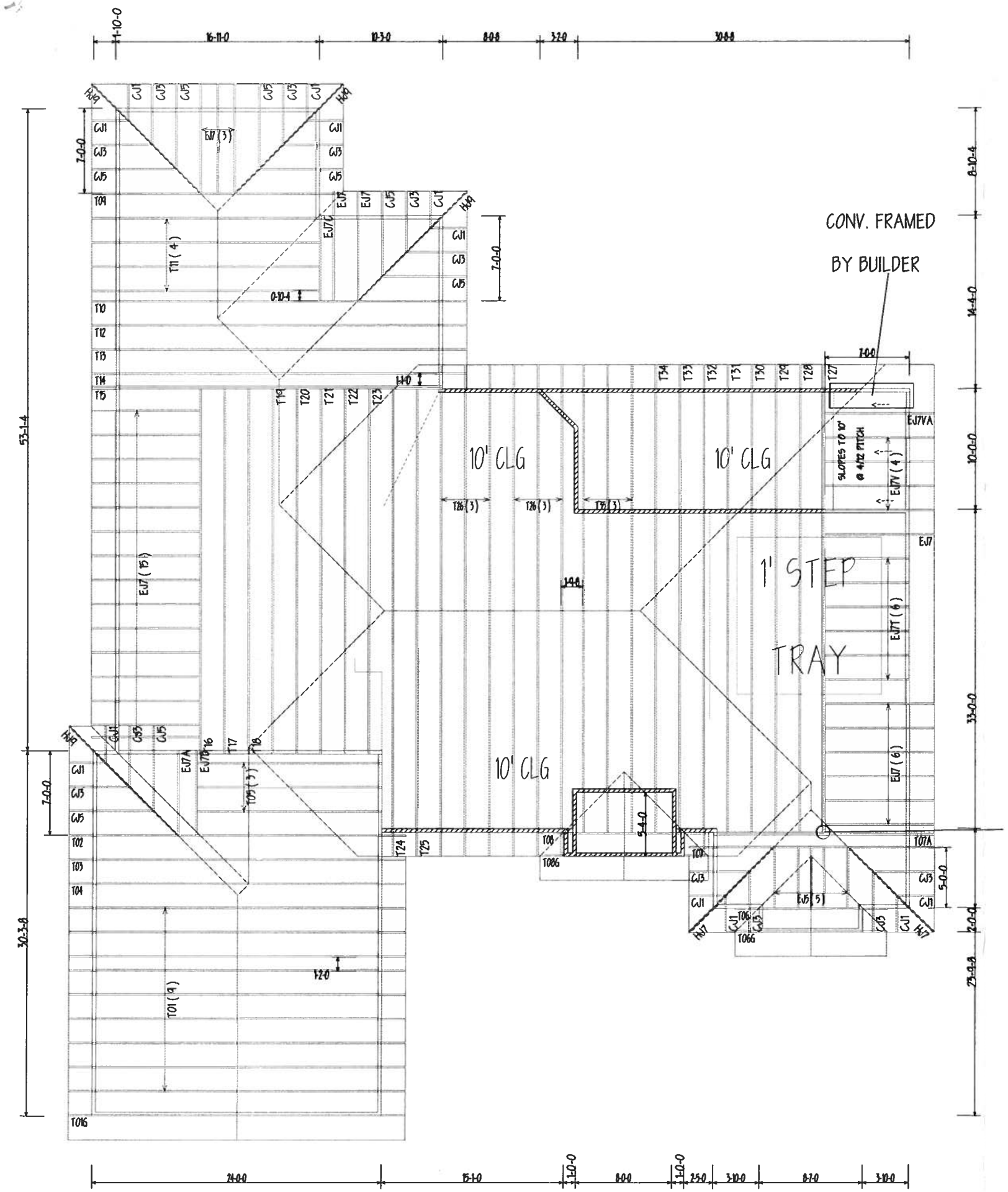
BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-H
HER	561

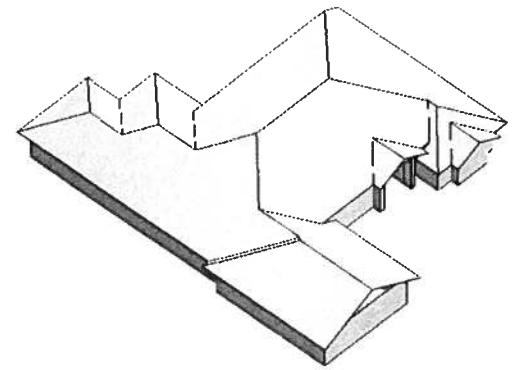
MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at ¼ panel length (± 6" from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.





6/12 PITCH
2' OH

BEARING HEIGHT SCHEDULE

8'-0"

10'-0"

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V10S FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date _____

Approved by: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: BLAKE CONST.		
LEGAL ADDRESS: NOLL RES.		
MODEL: CUSTOM		REVISION: SCALE: NTS
DATE: 01-10-06	DRAWN BY: JRD	JOB #: L145800