

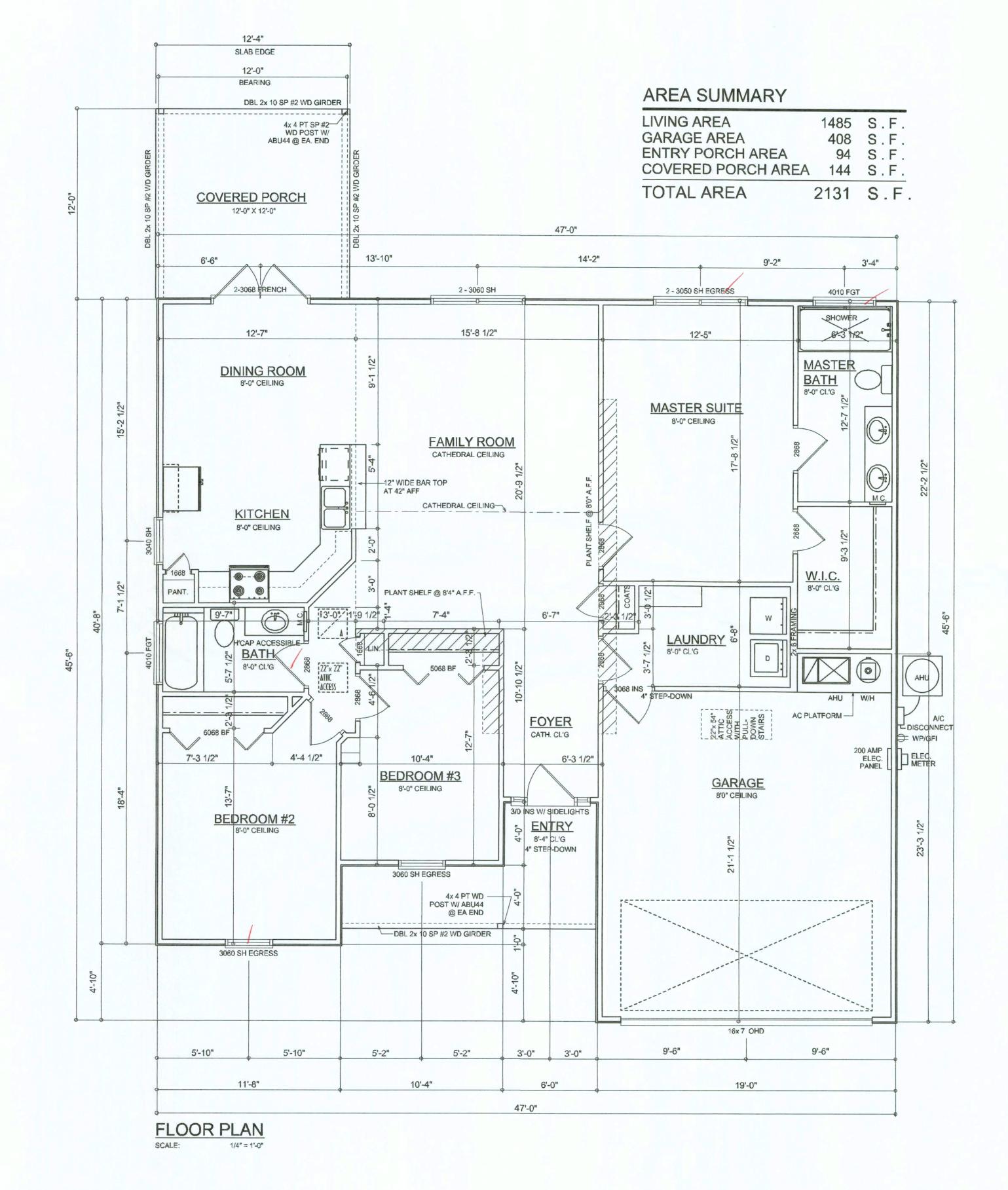
THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE

PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICHONE CONDUCTOR

SHALL BE USED AS AN EQUIPMENT GROUND.

INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVEAS A DISCONNECT MEANS.

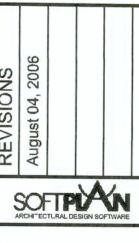
CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB



Garage fire separations shall comply with the following:

1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath halbitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gy/psum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 13/8 inches (34.9) mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted.

- Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.
- 3. A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.
- 4. When installing an attic access and/or pull-down stair unit in the garage, devise shall have a minimum 20 min. fire rating.

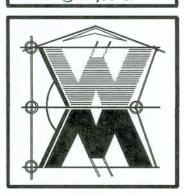


NC ARON SIMQUE HOMES, INC.



JOINT VENTURED WITH

OWILLIAM MYERS DE.SIGN P.O. BOX 1513 LAKE CITY, FL 32056 (386) 758-8406 will@willmyers.net



JOB NUMBER 060805

SHEET NUMBER

PRIOR TO THE CONSTRUCTION OF THE FOUNDATION, THE CONTRACTOR SHALL COORDINATE ANY INTERIOR BEARING LOCATION CONDITIONS PER THE TRUSS ENGINEERED SHOP DRAWINGS WITH THE FOUNDATION PLAN, ANY INTERIOR BEARING LOCATIONS OR ANY POINT LOADS OF 4.0 K OR GREATER SHALL BE SUPPORTED VIA A MODIFIED FOUNDATION PLAN TAKING THESE LOADS INTO CONSIDERATION. THE CONTRACTOR SHALL MAKE THE ENGINEERED TRUSS SHOP DRAWINGS AVAILABLE TO THE ARCHITECT FOR THE PURPOSE OF RENDERING SUCH MODIFICATIONS PRIOR TO POURING ANY CONCRETE.

> 12"x 12" TURNED DN SLAB EDGE, REINF'D W/ (1) #5 CONT.-12'-0" 47'-0" 4" STEP-DN 8" CMU STEM WALL ON ------PROVIDE STEMWALL REINFORCING AS PER POURED CONCRETE FOOTING, DETAILS A & B, THIS SHEET, AT 48" O.C. ALONG REFER TO SCHEDULE FOR SIZE ALL EXTERIOR WALLS AND ALL CORNERS -PROVIDE STEMWALL REINF'G PROVIDE A CONTINIOUS BOND BEAM AS PER PER A/A.4 \$ B/A.4 9 48" O.C. THE DETAILS A & B/A.4 W/I *5 REBAR, CONT: ALL AROUND TOP OF STEMWALL. PROVIDE A 20"XI2" X CONTINIOUS CONCRETE FOOTING W/ 2 *5 REBAR, BOTTOM 4 1 *3 REBAR, TRANSVERSE OR WIRE CHAIRS, @ 48" O.C. UNDER ALL PERIMETER WALLS OF HOUSE. 4" SMOOTH STEELED TROULLED CONC. SLAB, W/ FIBERMESH REINFORCING, OVER 6 MIL PLASTIC SHEETING, ON CLEAN, WELL COMPACTED SAND FILL, TERMITE TREATED LAP EDGES OF 6 MIL VAPOR BARRIER MIN. 6" -SEAL ALL JOINTS, TEALRS AND PIPING PENETRATIONS WITH DUCT TAPE 16"X12" X CONT. CONCRETE FOOTING W/ 2 *5 REBAR, BOTTOM @ GARAGE 4 PORCH STEM WALLS 4" STEP-DOWN 4 - *5 REBAR * 6" O.C. X 8'-0" LONG AT MID-DEPTH OF SLAB - 2 LOC -18'-8" 4" STEP-DOWN SLOPE TO 12"x 12" TURNED DN SLAB EDGE, REINF'D W/ (1) #5 CONT. 3/4" D. RECESS -AT GARAGE DR. 10'-4" 47'-0"

ALL EXTERIOR WALLS ARE 2X4 STUDS W/ 1/2" THICK CDX PLYWD, SHEATHING (4")

THE DESIGN WIND SPEED FOR THIS PROJECT IS 110 MPH PER 2004 FBC 1606 AND LOCAL JURISDICTION REQUIREMENTS

ADDED FILL SHALL BE APPLIED IN 8" LIFTS -EA. LIFT SHALL BE CONPACTED TO 95% DRY COMPACTION PER THE "MODIFIED PROCTOR" METHOD.

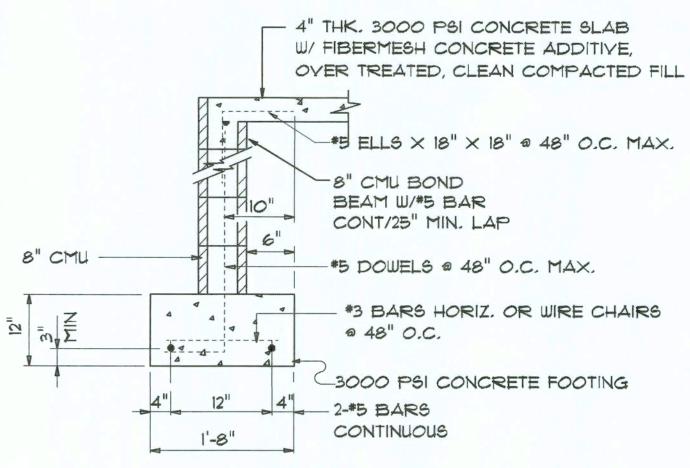
PLUMBING CONTRACTOR SHALL PREPARE "AS-BUILT" SHOP DRAWINGS INDICATING ALL PLUMBING WORK, INCLUDING ALL PLUMBING LINE LOCATIONS AND RISER DIAGRAM - CONT'R SHALL PROVIDE I COPY OF AS-BUILT DWGS TO OWNER AND I COPY TO THE PERMIT ISSUING AUTHORITY.

FOUNDATION PLAN

H.V.A.C. CONTRACTOR SHALL PREPARE "AS-BUILT" SHOP DRAWINGS INDICATING ALL H.V.A.C. WORK, INCLUDING ALL DUCTWORK LOC., SIZES, LINES, EQUIPMENT SCH. & BALANCING REPORT - CONT'R SHALL PROVIDE I COPY OF AS-BUILT DWGS TO OWNER & I COPY TO THE PERMIT ISSUING AUTHORITY.

CONCRETE / MASONRY / METALS GENERAL NOTES:

- I. DESIGN SOIL BEARING PRESSURE: 1000 PSF.
- 2. EXPANSIVE SOILS: WHERE DIRECTED BY THE SOILS ENGINEER, SOIL AUGMENTATION PER THE SOILS ENGINEER'S SPECIFICATIONS SHALL BE IMPLEMENTED PRIOR TO PLACING ANY FOUNDATIONS - TESTS AS SPECIFIED SHALL BE PREFORMED TO DETERMINE THE SUITABILITY OF THE SUB-GRADE TO SUPPORT THE DESIGN LOADS.
- 3. CLEAN SAND FILL OVER STRIPPED AND COMPACTED EXISTING GD. SHALL BE PLACED IN 12" LIFTS, BOTH SUB-SOIL AND FILL COMPAC-TION SHALL BE NOT LESS THAN 98% AS MEASURED BY A MODIFIED PROCTOR TEST AT THE RATE OF ONE TEST FOR EACH 1500 SF OF BUILDING PAD AREA, OR FRACTION THEREOF, FOR EACH 12" LIFT.
- 4. REINFORCING STEEL SHALL BE GRADE 60 AND MEET THE REQUIRE-MENTS OF ASTM AGIS, ALL BENDS SHALL BE MADE COLD.
- 5. WELDED WIRE MESH SLAB REINFORCING SHALL MEET THE REQUIRE-MENTS OF ASTM A185 - MIN. YEILD STRESS = 85 KSI.
- 6. CONCRETE SHALL BE STANDARD MIX F'c = 3000 PSI FOR ALL FTGS, SLABS, COLUMNS AND BEAMS OR SHALL BE STANDARD PUMP MIX F'C = 3000 PSI. STRENGTH SHALL BE ATTAINED WITHIN 28 DAYS OF PLACE-MENT. MIXING, PLACING AND FINISHING SHALL BE AS PER ACI STANDARDS.
- 7. CONCRETE BLOCK SHALL BE AS PER MANUFACTURER'S PRODUCT GUIDE FOR ASTM C-90 REQUIREMENTS WITH MEDIUM SURFACE FINISH -F'm = 1500 PSI.
- 8. MORTAR SHALL BE TYPE "M" OR "N" FOR ALL MASONRY UNITS.
- 9. STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 STANDARDS FOR STRENGTH, BOLTS SHALL BE ASTM A307 / GRADE I OR A325, AS PER PLAN REQUIREMENTS.
- 10. WELDS SHALL BE AS PER "AMERICAN WELDING SOCIETY" STANDARDS FOR STRUCTURAL STEEL APPLICATIONS.
- 11. 2X4 P/T WOOD SILL, CONT., ALL AROUND, W/ 5/8"~ A.B. W/ 3" SQ. X I/4" PLATE WASHERS WITHIN 6" FROM EACH CORNER, EA. WAY, & WITHIN 6" FROM ALL WALL OPENINGS / ENDS - 1/2"~ A.B. W/ 2" SQ. WASHERS ALONG EACH RUN @ 48" O.C., MAX. - ALL ANCHOR BOLTS SHALL HAVE A MINIMUM OF 8" EMBEDMENT INTO THE CONCRETE.



SCALE: 3/4" = 1'-0

SOFTPIAN

JOINT VENTURED WITH

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JOB NUMBER 060805

SHEET NUMBER

PROJECT COORDINATION REQUIREMENTS

NOTICE

THESE PLANS ARE DRAWN FOR AVERAGE SITE CONDITIONS AND COMPLIANCE WITH APPLICABLE CODES IN LAKE CITY, FL AT THE TIME THEY ARE DRAWN. DUE TO VARYING STATE, LOCAL, AND NATIONAL CODES RULES AND REGULATIONS, N.P.GEISLER, ARCHITCT CANNOT WARRANT COMPLIANCE WITH ALL APPLICABLE STATE, LOCAL, AND NATIONAL CODES IN YOUR AREA OR WITH YOUR PARTICULAR SI'E CONDITIONS, IT IS THE RESPONSIBILITY OF THE PURCHASER AND/OR BUILDER TO SEE THAT THE STRUC'URE IS BUILT IN STRICT COMPLIANCE WITH ALL GOVERNING MUNICIPAL CODES (CITY, COUNTY, STATE, AND FEDERAL), IF YOUR CITY OR STATE REQUIRES AN ENGINEER'S SEAL FOR THE SITE/CIVIL PORTIONS OF THE WORK, YOU WILL NEED TO HAVE THAT DONE LOCALLY BY A QUALIFIED, LICENCED PROFESSIONAL ENGINEER.

THE CONTRACTOR SHALL COORDINATE THE TRUSS TO TRUSS ANCHOR REQUIREMENTS WITH THE TRUSS ENGINEERING SHOP DRAWINGS. SOME OF THE TRUSS TO TRUSS CONNECTIONS WILL REQUIRE ANCHOR STRAPS IN ADDITION TO TYPICAL NAILING. ANCHOR DEVICES SHALL BE REQUIRED FOR ALL JOINTS WITH AN UPLIFT OR GRAVITY LOAD OF 100 LBS OR GREATER.

TRUSSES BEARING ON INTERIOR PARTITIONS WHERE UPLIFT LOADS ARE PRESENT SHALL REQUIRE ANCHORS OF EQUAL OR GREATER LOAD CAPACITY THAN THAT INDICATED BY THE TRUSS SHOP DRAWINGS. THE UPLIFT ANCHOR SYSTEM SHALL BE CONTINUOUS TO THE FOUNDATION.

SHOP DUG COORDINATION: THE TRUSS ANCHOR STRAPS AS INDICATED IN THE CONSTRUCTION DOCUMENTS ARE SUGGESTED STRAPS AND THAT THE TRUSS ENGINEERED SHOP DRAWING LOADS TAKE PRECEDENCE OVER THAT INDICATED IN THE CONSTRUCTION DOCUMENTS.

THE UPLIFT LOADS INDICATED FOR EACH TRUSS IN THE ENGINEERED TRUSS SHOP DRAWINGS MAY BE MATCHED TO STANDARD PRODUCT UPLIFT RATINGS FOR COMPARABLE UPLIFT CONNECTORS, AND THAT THE PRODUCTS THAT PROVIDE EQUAL OR GREATER UPLIFT RESISTANCE FOR THE LISTED LOADS MAY BE USED IN LIEU OF THOSE INDICATED IN THE CONSTRUCTION DOCUMENTS OR AS APPROVED BY THE BUILDING OFFICIAL.

> CONSTRUCT EXTERIOR WALLS W/ 2 TOP PLATES # 1 SILL -PLATE, 2X4 STUDS . 16" O.C., & "SIMPSON" SP2/SP1 STUD/PLATE CONNECTORS @ 32" O.C. - SHEATH WALL W/ 1/16" OSB, APPLIED W/ 8d COMMON NAILS @ 4" O.C. ALONG EDGES & 8" O.C. ALONG INTERMEDIATE SUPPORTS

SHEATH ROOF W/ 1/2" CDX PLYWOOD PLACED

W/ LONG DIMENSION PERPENDICULAR TO THE ROOF TRUSSES, SECURE TO FRAMING W/ 8d

PROJECT IS 110 MPH PER 2004 FBC 1606

AND LOCAL JURISDICTION REQUIREMENTS

NAILS - AS PER DETAIL ON SHEET SD.4

THE DESIGN WIND SPEEL FOR THIS

FASTEN TOP PLATE WITH 16d NAILS AT -12" O.C., TYPICAL T.O.

> ANCHOR ALL TRUSSES WITH "SEMCO" -HDPT2 STRAPS # 6 - 10" NAILS

> > 2X4 SUB-FASCIA, TYPICAL @ ALL -TRUSS EAVES & GABLE ENDS

> > > SCALE: 1/4" = 1'-0"

ANCHOR GIRDER TRUSS(ES) TO HEADER

ALL EXTERIOR WALLS ARE 2X4 STUDS W/

1/2" THICK CDX PLYWD. SHEATHING (4")

WITH 2 "SIMPSON" LGT(2, 3 OR 4),

ANCHOR HEADER TO KING STUDS W/

2 "SIMPSON" ST22 EA. END - TYP., T.O.

NOTE

ROOF PLAN NOTES

R-1 SEE EXTERIOR ELEVATIONS FOR ROOF PITCH

R-2 ALL OVERHANG 18" UNLESS OTHERWISE NOTED

PROVIDE ATTIC YENTILATION IN AC-CORDANCE WITH SCHEDULE ON SD.3

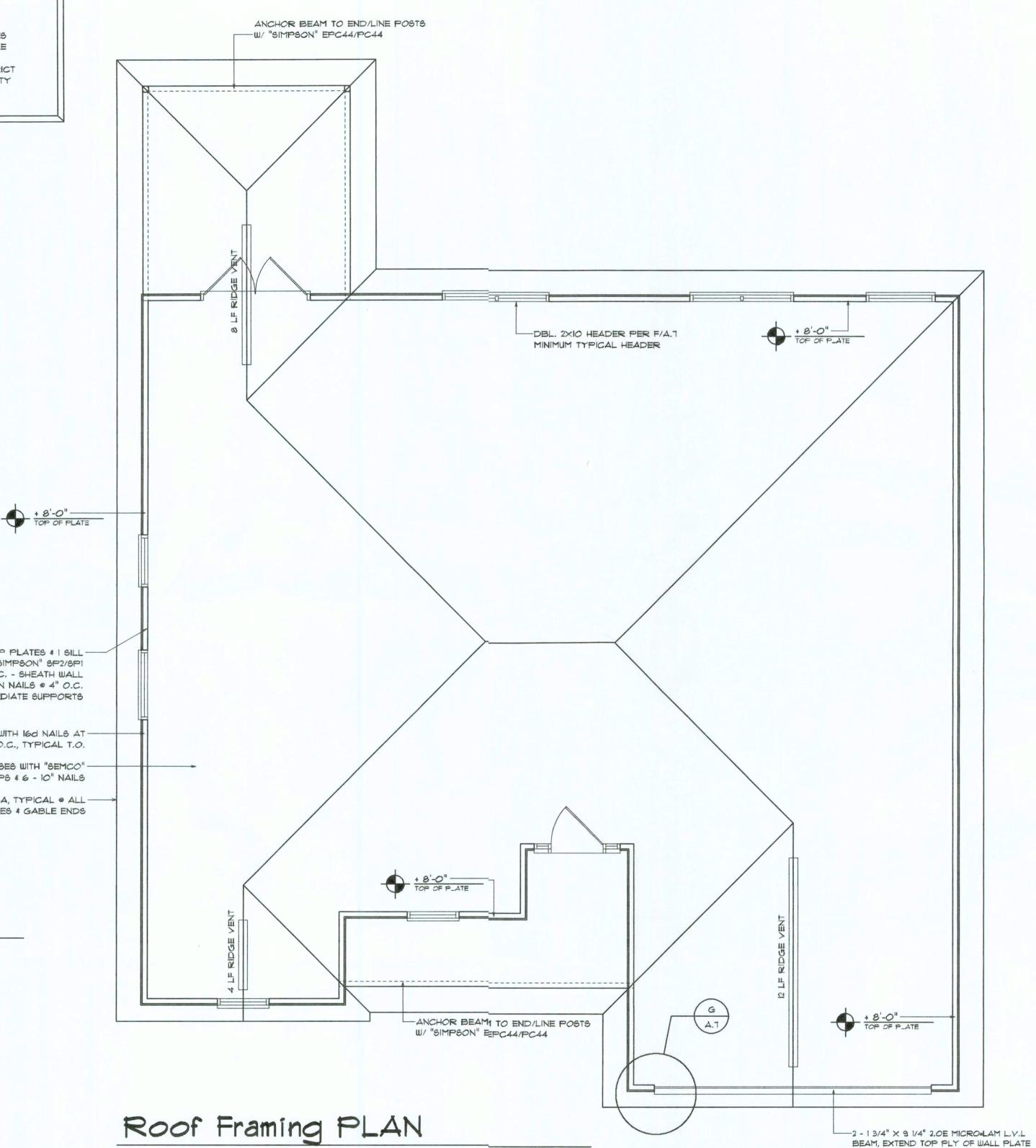
SEE EXTERIOR ELEVATIONS AND FLOOR PLANS TO VERIFY PLATE AND HEEL HEIGHTS

R-5 MOVE ALL VENTS AND OTHER ROOF PENETRATIONS TO REAR

ALL PENETRATIONS OF THE TOP PLATE OF ALL LOAD BEARING WALLS SHALL BE SEALED WITH FIRE RETARDANT CAULKING, INCLUDING WIRING, PLUMBING OR OTHER SUCH PENETRATIONS WALLS OVER 8'-O" TALL SHALL HAVE CONTINUOUS BLOCKING TO LIMIT CAVITY HEIGHT TO 8'-O". PENETRATIONS THROUGH SUCH BLOCKING SHALL BE TREATED IN THE SAME MANNER AS TOP PLATES, NOTED ABOVE

GENERAL TRUSS NOTES:

- 1. TRUSSES SHALL BE DESIGNED BY A LICENSED ENGINEER, AND IN ACCORDANCE WITH THE REQUIREMENTS OF THE "NATIONAL FOREST PRODUCTS ASSOCIATION" MANUAL FOR "STRESS RATED LUMBER AND IT'S CONNECTIONS", LATEST Ed., ALONG W/ THE "TRUSS PLATE INSTITUTE" SUGGESTED GUIDELINES FOR TEMPORARY AND PERMANENT BRACING, AND HANDLING OF TRUSSES. TRUSS SHOP DRAWINGS SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, DETS, 4 TRUSS TO TRUSS CONNECTIONS.
- 2. TRUSS SHOP DRAWINGS SHALL BE SIGNED & SEALED BY THE DESIGNING ENGINEER.
- 3. FOLLOWING DEVELOPMENT OF TRUSS SHOP DRAWINGS, ADJUSTMENTS TO THE ANCHOR REQUIRMENTS MAY BE REQUIRED DEPENDING ON THE ENGINEERED GRAVITY AND WIND UPLIFT REQUIREMENTS OF TRUSSES OR GIRDERS. THE CONTRACTOR SHALL MAKE AVAILABLE A COMPLETE SET OF TRUSS SHOP DRAWINGS TO THE ARCHITECT FOR THE PURPOSE OF REVIEW OF LOADS IMPOSED ON THE BALANCE OF THE STRUCTURE. ANY SUCH REQUIRED CHANGE SHALL BE INCORPORATED INTO THE CONSTRUCTION OF THIS



REFER TO THE WINDOW/IDOOR HEADER

MINIMUM SIZE HEADERS AND ALTERNATES

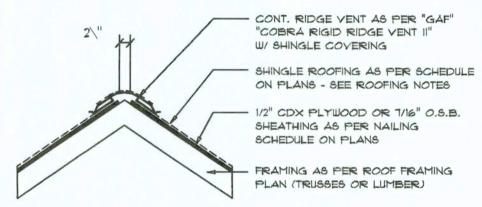
SCHEDULE ON SHEET SD),4 FOR ALL

MINIMUM SIZE ALLOWABLE IS 2-2×10.

WOOD STRUCTURAL NOTES

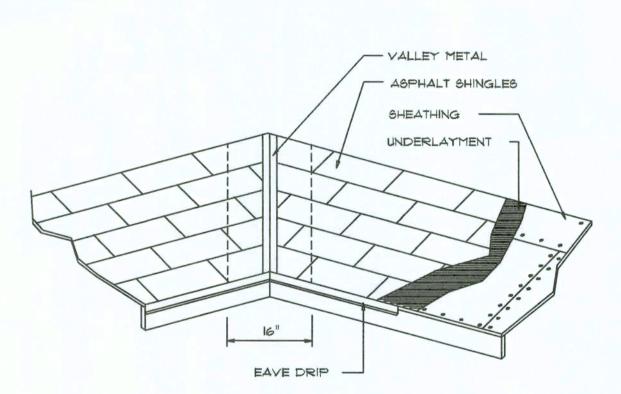
- I. TEMPORARY BRACING OF THE STRUCTURE DURING ERECTION, REQUIRED FOR SAFE AND STABLE CONSTRUCTION, SHALL BE THE SOLE RESPON-SIBILITY OF THE CONTRACTOR SO ENGAGED, TEMPORARY & PERMANENT BRACING OF ROOF TRUSSES SHALL BE AS PER THE STANDARD GUIDE-LINES OF THE "TRUSS PLATE INSTITUTE".
- 2. ALL TRUSSES SHALL BE DESIGNED BY A LICENSED PROFESSIONAL ENGINEER & SHALL BE SIGNED AND SEALED BY SAME, TRUSS DESIGN SHALL INCLUDE PLACEMENT PLANS, TRUSS DETAILS, TRUSS TO TRUSS CONNECTIONS & THE STANDARD SPECIFICATIONS & RECOMMENDATIONS OF INSTALLATION OF THE "TRUSS PLATE INSTITUTE",
- 3. WOOD STUDS IN EXTERIOR WALLS & INTERIOR BEARING WALLS SHALL BE NOT LESS THAN Nr.2 HEM-FIR OR BETTER.
- 4. CONNECTORS FOR WOOD FRAMING SHALL BE GALVANIZED METAL OR BLACK METAL AS MANUFACTURED OR AS CALLED FOR IN THE PLANS AND BE OF A DESIGN SUITABLE FOR THE LOADS AND USE INTENDED. REFER TO THE JOINT REINFORCEMENT SCHEDULE FOR PRINCIPLE CON-NECTIONS.

AREA OF ATTIC	REQ'D L.F. OF VENT	NET FREE AREA OF INTAKE
1600 SF	20 LF	410 5Q.IN.
1900 SF	24 LF	490 SQ.IN.
2200 SF	28 LF	570 SQ.IN.
2500 SF	32 LF	650 SQ.IN.
2800 SF	36 LF	730 8Q.IN.
3100 SF	40 LF	820 SQ.IN.
3600 SF	44 LF	900 SQ.IN.



Ridge Vent DETAIL

MIAMI/DADE PRODUCT APPROVAL REPORT: *98-0713.05



	TALS for FLAS ESS REQUIREMENTS	HING/ROOF	ING
MATERIAL	MINIMUM THICKNESS (in)	GAGE	WEIGHT
COPPER			16
ALUMINUM	0.024		
STAINLESS STEEL		28	
GALVANIZED STEEL	PTIO.0	26 (ZINC COATED G90)	
ZINC ALLOY LEAD PAINTED TERNE	0.027		40 20

Roofing/Flashing DETS.

FULL LENGTH, LAP MIN. 32" TO ADJOINING

WALL, ASSEMBLE W/ 16d NAILS @ 12" O.C.,

STAGGERED TOP 4 BOTTOM OF BEAM,

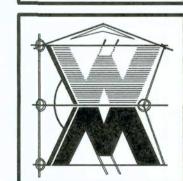
VALLEY FLASHING





JOINT VENTURED WITH

@WILLIAM MYERS DESIGN P.O. BOX 1513 LAKE CITY, FL 32056 (386) 758-8406 will@willmyers.net



JOB NUMBER 060805

SHEET NUMBER

OF 6 SHEETS

SCALE: NONE

FLORIDA BUILDING CODE

Compliance Summary

TYPE OF CONSTRUCTION

Roof: Hip Construction, Wood Trusses @ 24" O

Ualls: 2x4 Wood Studs @ 16" O.C. Floor: 4" Thk. Concrete Slab W/ Fibermesh Concrete Additive Foundation: Continuous Footer/Stem Wall

ROOF DECKING

Naterial: 1/2" CD Plywood or 7/16" O.S.B.

theet Size: 48"x96" Sheets Perpendicular to Roof Framing Fasteners: 8d Common Nalls per schedule on sheet A.7

SHEARWALLS

Naterial: 1/2" CD Plywood or 7/16" O.S.B.

thest Size: 48"x96" Sheets Placed Vertical Fasteners: 8d Common Nails # 4" O.C. Edges # 8" O.C. Interior Pragetrut: Double Top Plate (S.Y.P.) W/16d Nails @ 12" O.C.

liall Stude: 2x4 Hem Fir Stude @ 16" O.C.

HURRICANE UPLIFT CONNECTORS

Trues Anchors: SEMCO HDPT2 & Ea. Trues End (Typ. U.O.N.) Wall Sheathing Nailing is Adequate - 8d @ 4" O.C. Top & Bot. Anchor Bolts: 1/2" A307 Bolts # 48" O.C. - 1st Bolt 6" from corner

Corner Hold-down Device: (I) HD5a a each corner Porch Column Base Connector: Simpson ABU44/ABU66 € each column Porch Column to Beam Connector: Simpson EPC44/PC44 @ each column

FOOTINGS AND FOUNDATIONS

Footing: 20"x12" Cont. W/2-*5 Bars Cont. \$ 1-*3 Transverse @ 24" O.C. Stemwall: 8" C.M.U. W/1-#5 Vertical Dowel @ 48" O.C.

BASIC WIND SPEED:	110 MPH
WIND IMPORTANCE FACTOR (I):	I = 1.00
BUILDING CATAGORY:	CATAGORY II
WIND EXPOSURE:	"a"
INTERNAL PRESSURE COEFFICIENT:	+/- 0.18
MWFR6 PER TABLE 1606.2A (FBC 2001)	ROOF: - 23.1 PSF
DESIGN WIND PRESSURES:	WALLS: + 26.6 PSF
	EAVES: - 32.3 PSF
COMPONENTS & CLADING PER TABLES	OP'NG6: + 21.8 / - 29.1 PSF
1606.2B # 1606.2C (FBC 2001)	EAVES: - 68.3 PSF
DESIGN WIND PRESSURES:	ROOF: + 19.9 / - 25.5 PSI

TERMITE PROTECTION NOTES:

SOIL CHEMICAL BARRIER METHOD:

I. A PERMANENT SIGN WHICH IDENTIFIES THE TERMITE TREATMENT PROVIDER AND NEED FOR REINSPECTION AND TREATMENT CONTRACT RENEWAL SHALL BE PROVIDED, THE SIGN SHALL BE POSTED NEAR THE WATER HEATER OR ELECTRIC PANEL, FBC 104.2.6

2. CONDENSATE AND ROOF DOWNSPOUTS SHALL DISCHARGE AT LEAST 1'-O" AWAY FROM BUILDING SIDE WALLS. FBC 1503.4.4

3. IRRIGATION/SPRINKLER SYSTEMS INCLUDING ALL RISERS AND SPRAY HEADS SHALL NOT BE INSTALLED WITHIN I'-O" FROM BUILDING SIDE WALLS. FBC 1503,4,4

4, TO PROVIDE FOR INSPECTION FOR TERMITE INFESTATION, BETWEEN WALL COVERINGS AND FINAL EARTH GRADE SHALL NOT BE LESS THAN 6". EXCEPTION: PAINT AND DECORATIVE CEMENTIOUS FINISH LESS THAN 5/8" THICK ADHERED DIRECTLY TO THE FOUNDATION WALL. FBC 1403.1.6

5, NITIAL TREATMENT SHALL BE DONE AFTER ALL EXCAVATION AND BACKFILL IS COMPLETE. FBC 1816.1.1

6. SOIL DISTURBED AFTER THE INITIAL TREATMENT SHALL BE RETREATED INCLIDING SPACES BOXED OR FORMED. FBC 1816.1.2

T. BOXED AREAS IN CONCRETE FLOOR FOR SUBSEQUENT INSTALLATION OF RAPS, ETC., SHALL BE MADE WITH PERMANENT METAL OR PLASTIC FOR1S, PERMANENT FORMS MUST BE OF A SIZE AND DEPTH THAT WILL ELIMNATE THE DISTURBANCE OF SOIL AFTER THE INITIAL TREATMENT. FBC 1816.1.3

8. MNIMUM 6 MIL VAPOR RETARDER MUST BE INSTALLED TO PROTECT AGAINST RAINFALL DILUTION. IF RAINFALL OCCURS BEFORE VAPOR RET-ARCER PLACEMENT, RETREATMENT IS REQUIRED. FBC 1816.1.4

9. CONCRETE OVERPOUR AND MORTAR ALONG THE FOUNDATION PERIMETER MUS' BE REMOVED BEFORE EXTERIOR SOIL TREATMENT. FBC 1816.1.5 10. SOIL TREATMENT MUST BE APPLIED UNDER ALL EXTERIOR CONCRETE

OR &RADE WITHIN I'-O" OF THE STRUCTURE SIDEWALLS. FBC 1816.1.6 II. AN EXTERIOR VERTICAL CHEMICAL BARRIER MUST BE INSTALLED AFTER CONSTRUCTION IS COMPLETE INCLUDING LANDSCAPING AND IRRIGATION. ANY SOIL DISTURBED AFTER THE VERTICAL BARRIER IS APPLIED, SHALL

BE RETREATED. FBC 1816.1.6 12. AL BUILDINGS ARE REQUIRED TO HAVE PER-CONSTRUCTION TREATMENT. FBC1816.1.7

13. A CERTIFICATE OF COMPLIANCE MUST BE ISSUED TO THE BUILDING DEPART-MEN' BY # LICENSED PEST CONTROL COMPANY BEFORE A CERTIFICATE OF OCCIPANCY WILL BE ISSUED. THE CERTIFICATE OF COMPLIANCE SHALL STATE: "THEBUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SIBTERRANEAN TERMITES, THE TREATMENT IS IN ACCORDANCE WITH THE RULES AND LAWS OF THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONS-

14. AFTER ALL WORK IS COMPLETED, LOOSE WOOD AND FILL MUST BE REMOVED FROM BELOW AND WITHIN 1'-O" OF THE BUILDING. THIS INCLUDES ALL GRADE STAKES, TUB TRAP BOXES, FORMS, SHORING OR OTHER CELLULOSE CONTAINING MATERIAL FBC 2303.1.3

15. N) WOOD, VEGETATION, STUMPS, CARDBOARD, TRASH, ETC., SHALL BE BURIED WITHN 15'-O" OF ANY BUILDING OR PROPOSED BUILDING. FBC 2303.1.4

FRAMING ANCHOR SCHEDULE

APPLICATION MANUF'R/MODEL CAP. TRUSS TO WALL: SEMCO HDPT2, W/ 6 - 10d NAILS GIRDER TRUSS TO POST/HEADER: SIMPSON LGT, W/ 28 - 16d NAILS HEADER TO KING STUD(S): SIMPSON ST22 1370# PLATE TO STUD: SIMPSON SP2 065# SIMPSON SPI STUD TO SILL: 585# PORCH BEAM TO POST SIMPSON PC44/EPC44 1700# PORCH POST TO FND .: SIMPSON ABU44 2200# MISC. JOINTS SIMPSON A34 315*/240*

ALL ANCHORS SHALL BE SECURED W/ NAILS AS PRESCRIBED BY THE MANUFACTURER FOR MAXIMUM JOINT STRENGTH, UNLESS NOTED OTHERWISE.

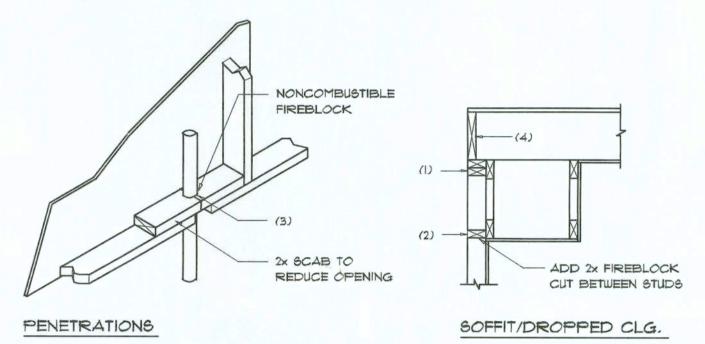
REFER TO THE INCLUDED STRUCTURAL DETAILS FOR ADDITIONAL ANCHORS/ JOINT REINFORCEMENT AND FASTENERS.

ALL UNLISTED JOINTS IN THE LOAD PATH SHALL BE REINFORCED WITH SIMPSON A34 FRAMING ANCHORS, TYPICAL T.O.

"SEMCO" PRODUCT APPROVAL:

MIAMI/DADE COUNTY REPORT *95-0818.15

"SIMPSON" PRODUCT APPROVALS: MIAMI/DADE COUNTY REPORT *97-0107.05, *96-1126.11, *99-0623.04 SBCC1 NER-443, NER-393



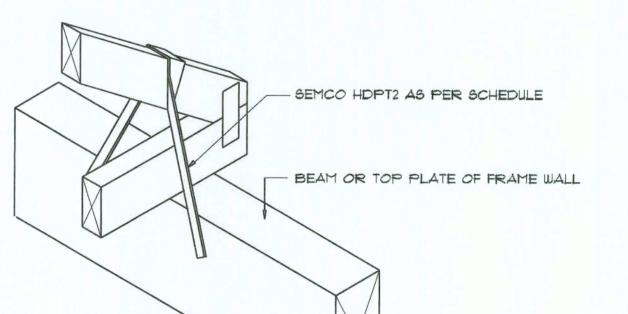
FIREBLOCKING NOTES:

FIREBLOCKING SHALL BE INSTALLED IN WOOD FRAME CONSTRUCTION IN THE FOLLOWING LOCATIONS:

- I. IN CONCEALED SPACES OF STUD WALLS AND PARTITIONS INCLUDING FURRED SPACES AT CEILING AND FLOOR LEVELS.
- 2. AT ALL INTERCONNECTIONS BETWEEN CONCEALED VERTICAL AND HORIZONTAL SPACES SUCH AS OCCUR AT SOFFITS, DROP CEILINGS, COVE CEILINGS, ETC.
- 3. AT OPENINGS AROUND VENTS, PIPES, DUCTS, CHIMNEYS AND FIREPLACES AT CEILING AND FLOOR LEVELS WITH "PYROPANEL MULTIFLEX SEALANT"
- 4. AT ALL INTERCONNECTIONS BETWEEN CONCEALED VERTICAL STUD WALL OR PARTITION SPACES AND CONCEALED SPACES CREATED BY AN ASSEMBLY OF FLOOR JOISTS, FIREBLOCKING SHALL BE PROVIDED FOR THE FULL DEPTH OF THE JOISTS AT THE ENDS AND OVER THE SUPPORTS.

Fire Stopping DETAILS

SCALE: NONE



SEMCO HDPT2

TRUSS TO WOOD BEAM SCALE: 1/2" = 1'-0"



General Roofing NOTES:

ASPHALT SHINGLES SHALL BE FASTENED TO SOLIDLY SHEATHED DECKS.

ASPHALT SHINGLES SHALL BE USED ONLY ON ROOF SLOPES OF 2:12 OR GREATER. FOR ROOF SLOPES FROM 2:12 TO 4:12, DBL. UNDERLAYMENT IS REQUIRED.

UNLESS OTHERWISE NOTED, UNDERLAYMENT SHALL CONFORM W/ ASTM D 226, TYPE I, OR ASTM D 4869, TYPE I.

SELF-ADHERING POLYMER MODIFIED BITUMEN SHEET: SELF ADHERING POLYMER MODIFIED BITUMEN SHALL COMPLY W/ ASTM D 1970.

ASPHALT SHINGLES:

ASPHALT SHINGLES SHALL HAVE SELF SEAL STRIPS OR BE INTERLOCKING, AND COMPLY WITH ASTM D 225 OR ASTM D 3462.

FASTENERS FOR ASPHALT SHINGLES SHALL BE GALVANIZED, STAINLESS STEEL, ALUMINUM OR COPPER ROOFING NAILS, MINIMUM 12 GAUGE SHANK WITH A MINIMUM 3/8 INCH DIAMETER HEAD, OF A LENGTH TO PENETRATE THROUGH THE ROOFING MATERIAL AND A MINIMUM 3/4" INTO THE ROOF SHEATHING.

WHERE THE SHEATHING IS LESS THAN 3/4" THICK, THE NAILS SHALL PENETRATE

THROUGH THE SHEATHING.

ASPHALT SHINGLES SHALL BE SECURED TO THE ROOF WITH NOT LESS THAN FOUR FASTENERS PER STRIP SHINGLE OR TWO FASTENERS PER INDIVIDUAL SHINGLE. WHERE ROOFS LOCATED IN BASIC WIND SPEED OF 110 MPH OR GREATER, SPECIAL METHODS OF FASTENING ARE REQUIRED. UNLESS OTHERWISE NOTED, ATTACHMENT OF ASPHALT SHINGLES SHALL CONFORM WITH ASTM D 3161 OR M-DC PA 107-95.

UNDERLAYMENT APPLICATION:

FOR ROOF SLOPES FORM 2:12 TO 4:12, UNDERLAYMENT SHALL BE A MINIMUM OF TWO LAYERS APPLIED AS FOLLOWS:

I. STARTING AT THE EAVE, A 19 INCH STRIP OF UNDERLAYMENT SHALL BE APPLIED PARALLEL WITH THE EAVE AND FASTENED SUFFICIENTLY TO STAY IN PLACE.

2. STARTING AT THE EAVE, 36 INCH WIDE STRIPS OF UNDERLAYMENT FELT SHALL BE APPLIED OVERLAPPING SUCCESSIVE SHEETS 19 INCHES AND FASTENED SUFFICIENTLY TO STAY IN PLACE.

FOR ROOF SLOPED 4:12 AND GREATER, UNDERLAYMENT SHALL BE A MINIMUM OF ONE LAYER OF UNDERLAYMENT FELT APPLIED AS FOLLOWS: STARTING AT THE EAVE, UNDERLAYMENT SHALL BE APPLIED SHINGLE FASHION PARALLEL TO THE EAVE, LAPPED 2 INCHES, AND FASTENED SUFFICIENTLY TO STAY IN PLACE.

BASE AND CAP FLASHINGS:

BASE AND CAP FLASHING SHALL BE INSTALLED IN ACCORDANCE W/ MFGR'S INSTALLATION INSTRUCTIONS. BASE FLASHING SHALL BE OF EITHER CORROSION RESISTANT METAL OF MINIMUM NOMINAL THICKNESS 0.019 INCH OR MINERAL SURFACE ROLL ROOFING WEIGHING A MINIMUM OF TI LBS PER 100 SQUARE FEET. CAP FLASHING SHALL BE CORROSION RESISTANT METAL OF MINIMUM NOMINAL THICKNESS OF 0.019 INCH.

VALLEYS:

VALLEY LININGS SHALL BE INSTALLED IN ACCORDANCE W/ MANUFACTURER'S INSTALLATION INSTRUCTIONS BEFORE APPLYING ASPHALT SHINGLES. VALLEY LININGS OF THE FOLLOWING TYPES SHALL BE PERMITTED.

I. FOR OPEN VALLEYS LINED WITH METAL, THE VALLEY LINING SHALL BE AT LEAST 16" WIDE AND OF ANY OF THE CORROSION RESISTANT METALS IN FBC TABLE 1507.3.9.2. 2. FOR OPEN VALLEYS, VALLEY LINING OF TWO PLIES OF MINERAL SURFACE

ROLL ROOFING SHALL BE PERMITTED. THE BOTTOM LAYER SHALL BE IS INCHES AND THE TOP LAYER A MINIMUM OF 36 INCHES WIDE. 3. FOR CLOSED VALLEYS VALLEY LINING SHALL BE ONE OF THE FOLLOWING:

I. BOTH TYPES I AND 2 ABOVE, COMBINED. 2. ONE PLY OF SMOOTH ROLL ROOFING AT LEAST 36 INCHES WIDE AND COMPLYING WITH ASTM D 224. 3. SPECIALTY UNDERLAYMENT AT LEAST 36 INCHES WIDE AND COMPLYING

WITH ASTM D 1970.

ROOFSHINGLES SHALL BE AS MANUFACTURED BY "TAMKO ROOFING PRODUCTS" OF THE FOLLOWING MODELS:

> GLASS-SEAL AR ELITE GLASS-SEAL AR HERITAGE 30 AR HERITAGE 40 AR HERITAGE 50 AR

THESE SHINGLES MEET THE REQUIREMENTS OF ASTM D-3161 TYPE I MODIFIED TO 110 MPH WINDS & FBC TAS 100, USING 4 NAILS/SHINGLE

SOFTPIXN

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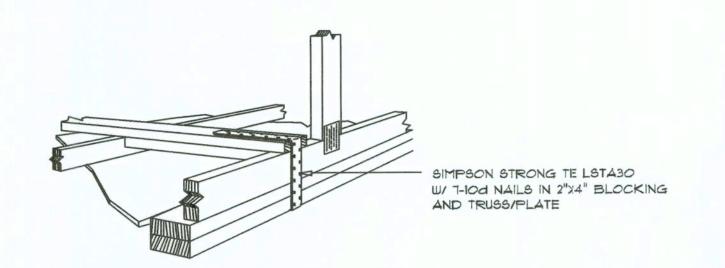
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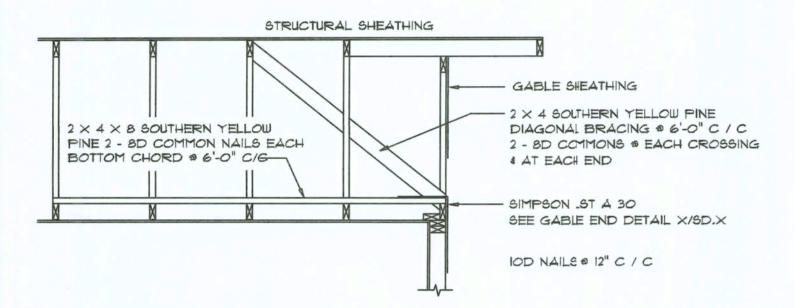
JOB NUMBER 060805

SHEET NUMBER



GABLE END GYPSUM DIAPHRAGM HOLDOWN CONNECTOR

SCALE: NONE

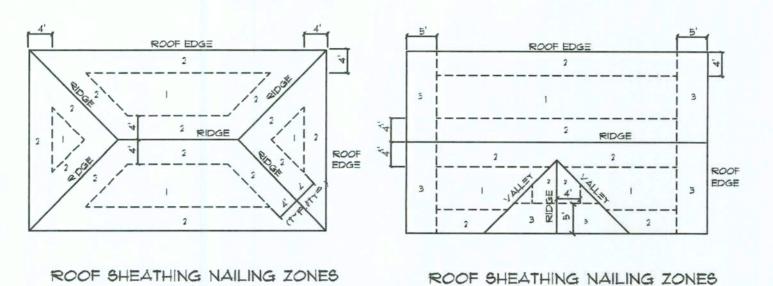


END WALL BRACING FOR CEILING DIAPHRAGM

(ALTERNATIVE TO BALLOON FRAMING)

NOTE: ALL WOOD TO BE NUMBER 2 GRADE SOUTHERN YELLOW PINE

ROOF SHEATHING FASTENINGS NAILING SHEATHING SPACING 6 In Go.c. EDGE 12 'n Go.c. FIELD 1/16 " 0.5.B. 8d HOT DIPPED 6 in. ob.c. EDGE 6 in Op.c. FIELD CR 15/32 CDX GALYANIZED In. a.c. @ (GABLE ENDWA BCX NAILS OR GAESLE TRUSS 6 in. op.c. EDGE 6 in. op.c. FIELD

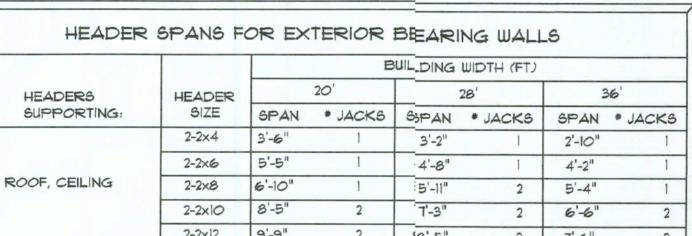


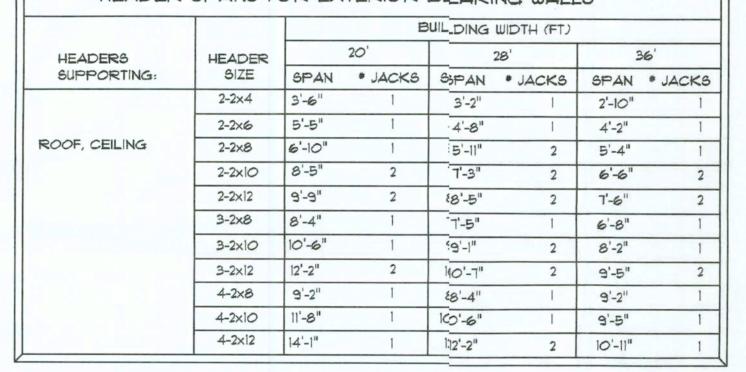
(GABLE ROOF)

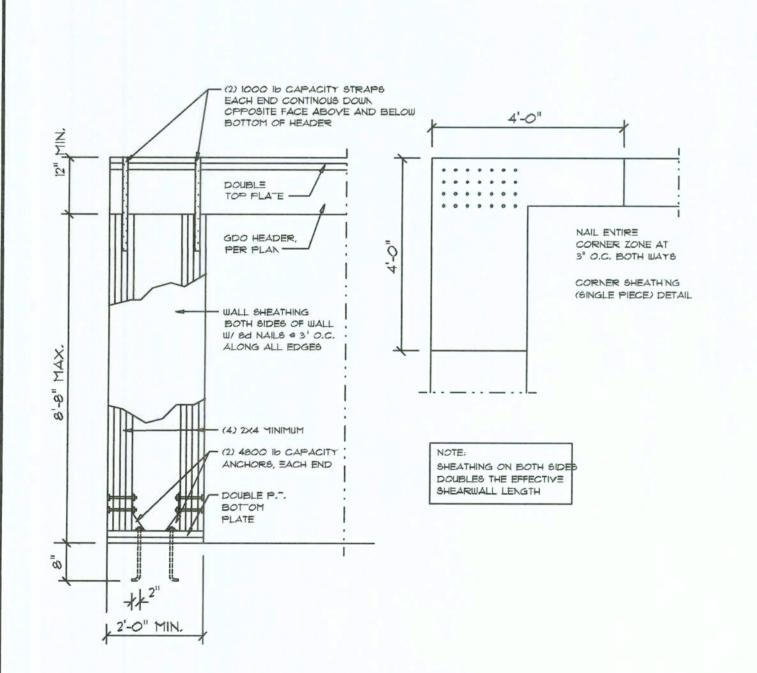
Roof Nail Pattern DET.

(HIP ROOF)

SCALE: NONE

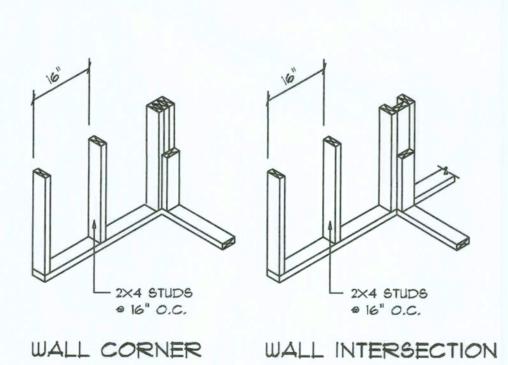


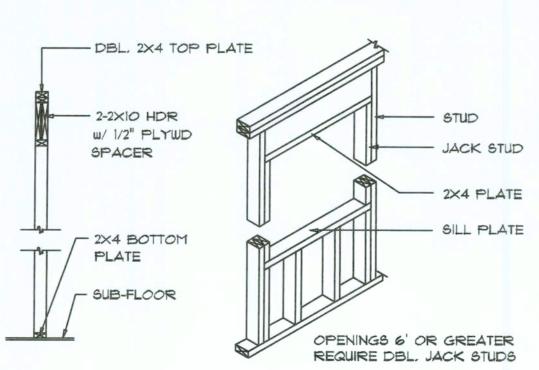


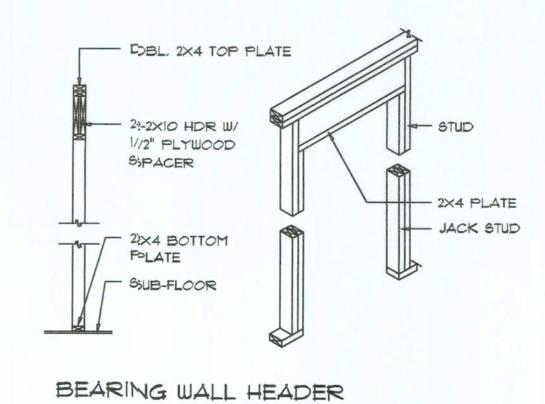


Garage End Wall DETAILS

SCALE: 1/2" = 1'-0"







DBL. 2X4 TOP PLATE

2-2×4 HEADER WJ/ BLOCKING

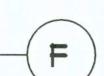
SUB-FLOOR

NON-BEARING WALL HEADER

TATE 2>X4 BOTTOM POLATE

TYPICAL WINDOW HEADER

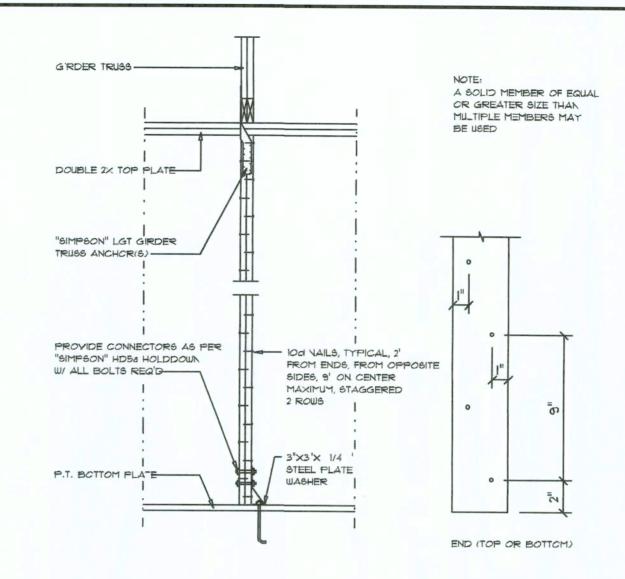
Wall Framing/Header DETAILS



2-2×4 HDR W/ BLOCK'G

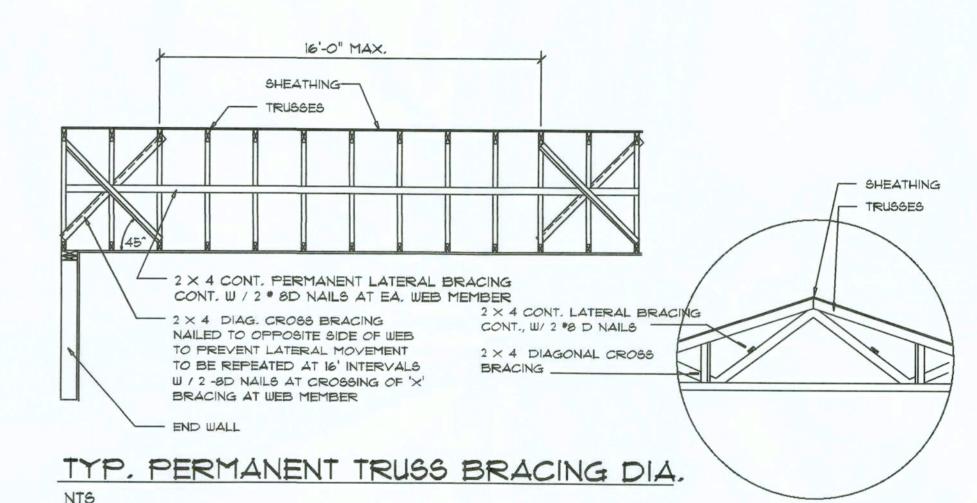
NOTE: ALL INTERIOR DOOR CPENINGS SHOULD BE FRAMED 2" WIDER THAN

THEIR SPECIFIED SIZE.





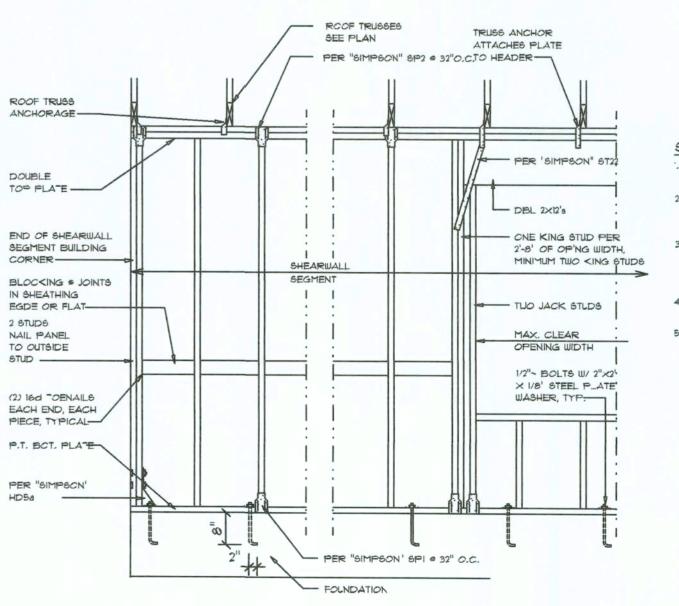
SCALE: 1/2" = 1'-0"



Truss Bracing DETAILS

NOTE: ALL WOOD TO BE NUMBER 2 GRADE SOUTHERN YELLOW PINE

SCALE: AS NOTED



SHEARWALL NOTES:

D

- ALL SHEARWALLS SHALL BE TYPE 2 SHEARWALLS AS DEFINED BY STD 10-9" SBBCI 305.4.3.
- THE WALL SHALL BE ENTIRELY SHEATHED WITH 7/16 " O.S.B. INCLUDING AREAS ABOVE AND BELOW
- 3. ALL SHEATHING SHALL BE ATTACHED TO FRAMING ALONG ALL FOUR EDGES WITH JOINTS FOR ADJACENT PANELS OCCURING OVER COMMON FRAMING MEMBERS OR ALONG BLOCKING.
- 4. NAIL SPACING SHALL BE 6" O.C. EDGES AND 12" C.C. IN THE FIELD.
- 7. TYPE 2 SHEARWALLS ARE DESIGNED FOR THE OPENING IT CONTAINS. MAXIMUM HEIGHT OF OPENING SHALL BE 5/6 TIMES THE WALL HEIGHT. THE MINIMUM DISTANCE BE WEEN OPENINGS SHALL BE THE WALL HEIGHT/3.5 FOR 8'-0" WALLS (2'-3').

OPENING WIDTH	SILL PLATES	16d TOE NAILS EACH END
UP TO 6'-0"	(I) 2x4 OR (I) 2x6	1
> 6' TO 9'-0'	(3) 2x4 OR (1) 2x6	2
> 9' TO 12'-0'	(5) 2x4 OR (2) 2x6	3

Shear Wall DETAILS

SCALE: NONE

E

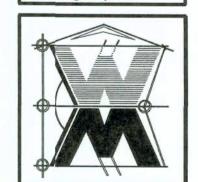
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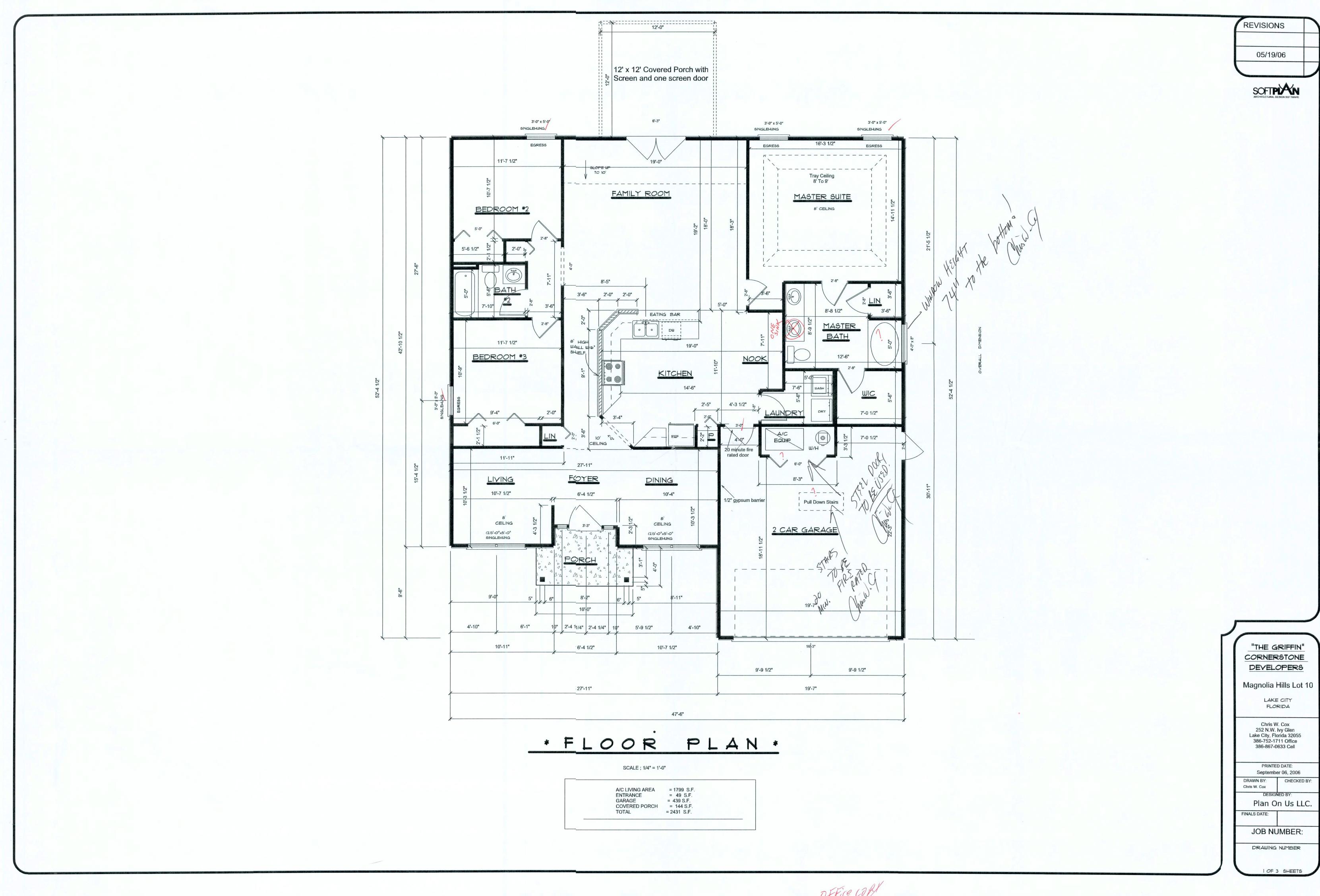
JOINT VENTURED WITH

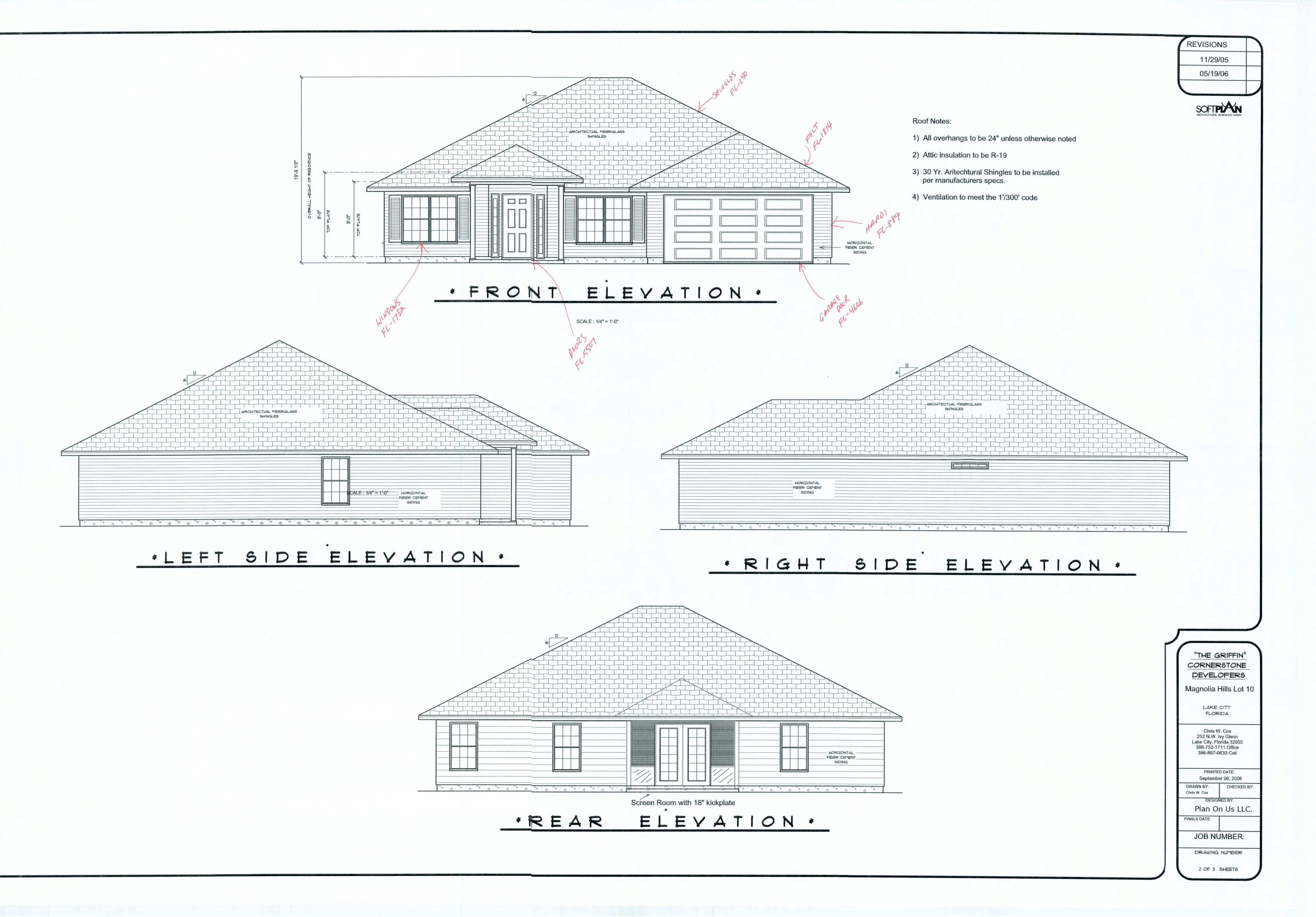
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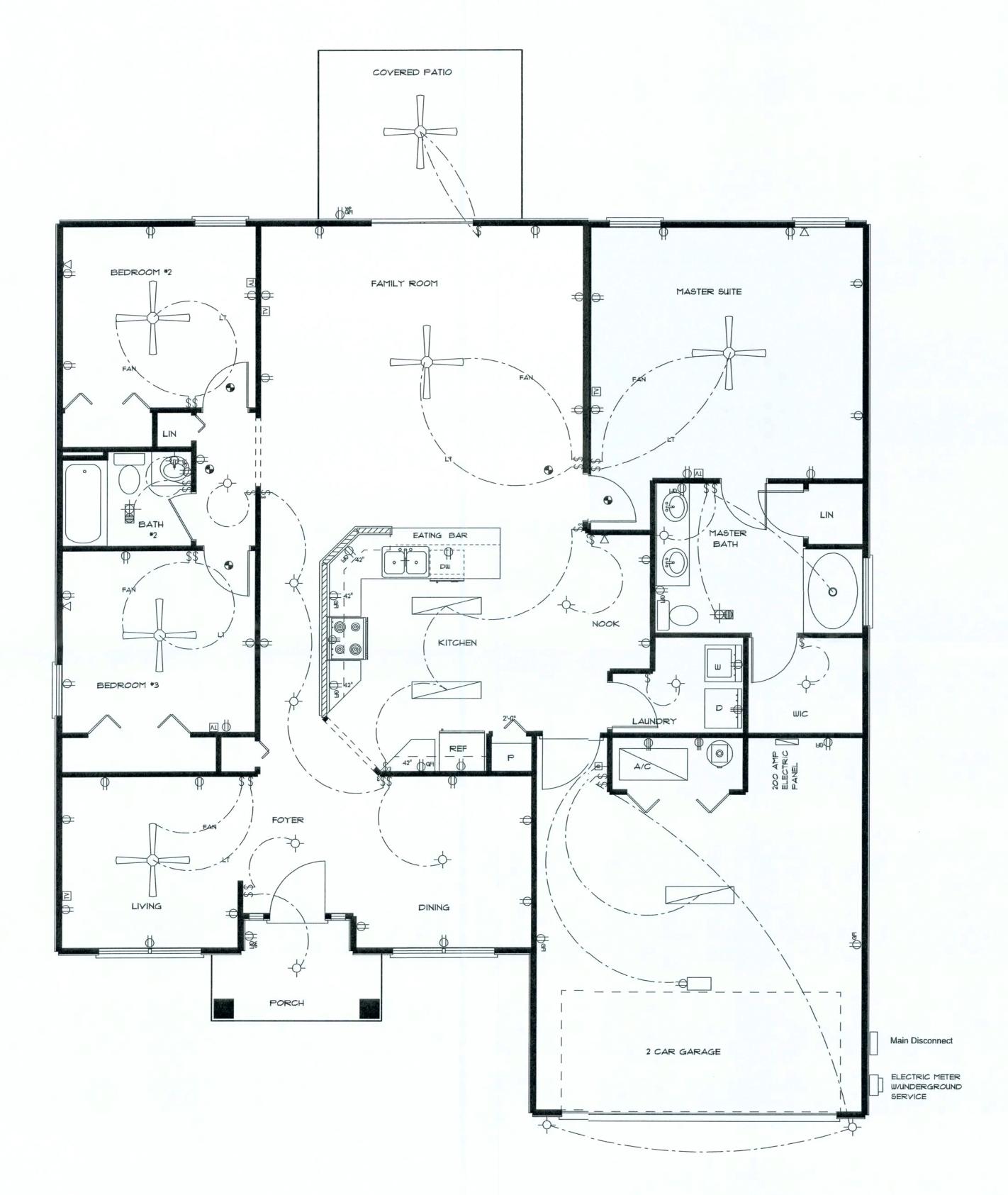


JOB NUMBER 060805

SHEET NUMBER







* ELECTRIC PLAN *

	ELECTRICAL LEGEND
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
QD	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
0	RECESSED CAN LIGHT
- ♦	BATH EXAUST FAN WITH LIGHT
₩	BATH EXAUST FAN
	LIGHT FIXTURE
Ф	DUPLEX OUTLET
(220v OUTLET
d an	GFI DUPLEX OUTLET
•	SMOKE DETECTOR
\$	WALL SWITCH
\$3	3 WAY WALL SWITCH
\$	4 WAY WALL SWITCH
W _{P/GFI}	WATER PROOF GFI OUTLET
∇	PHONE JACK
10	TELEVISION JACK
9	GARAGE DOOR OPENER
	WALL HEATER

Electrical Plan Notes

Electrical Plan Notes
E-1 Wire all appliances, HVAC units and other equipment per manufactors specifications.
E-2 Consult the owner for the number of seperate telephone lines to be installed.
E-3 All installations shall be per national electric code.
E-4 All smoke detectors shall be 120V w/battery backup of the photoelectric type, and shall be interlocked together Install inside and near all bedrooms.
E-5 Telephone, television and other low voltage devices or outlets shall be as per the owners direction and in accordance with applicable sections or national electrical code latest edition.
E-6 Electrical contractor shall be responsible for the design and sizing of electrical service and circuits.
E-7 Entry of service underground or overhead is to be determined by the power company.
E-8 All bedroom recepticals are to be AFCI (Arc Fault Circuit Interrupt).
E-9 All outside recepticals are to be weatherproofed GFIs.

"THE GRIFFIN" CORNERSTONE DEVELOPERS

Magnolia Hills Lot 10

LAKE CITY FLORIDA

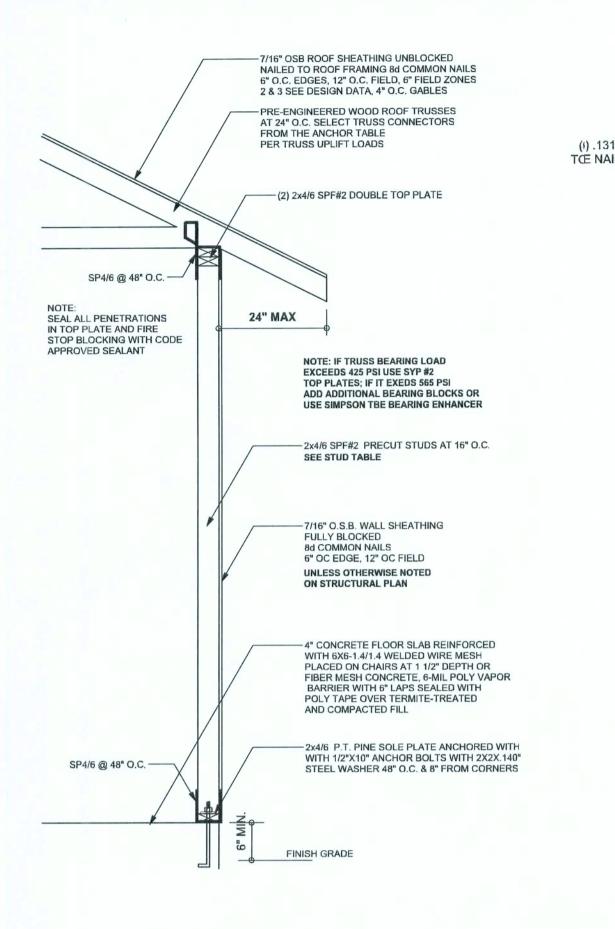
Chris W. Cox 252 N.W. Ivy Glen Lake City, Florida 32055 386-752-1711 Office 386-867-0633 Cell

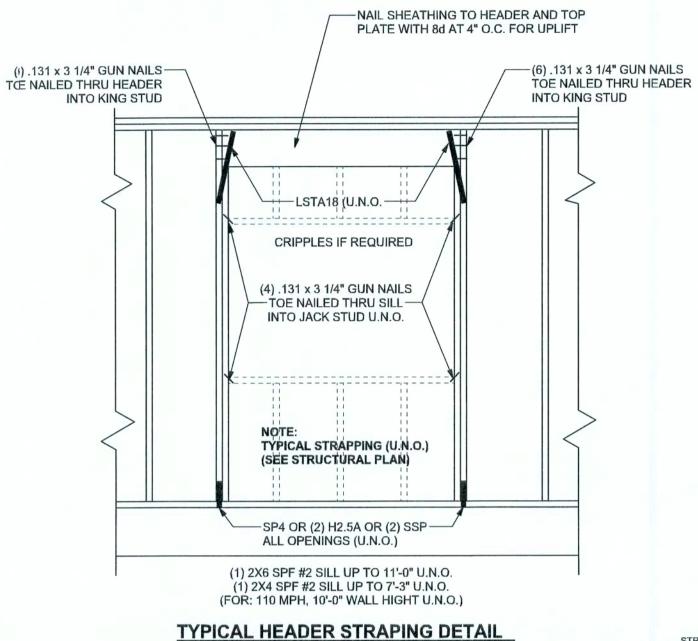
September 06, 2006 Chris W. Cox

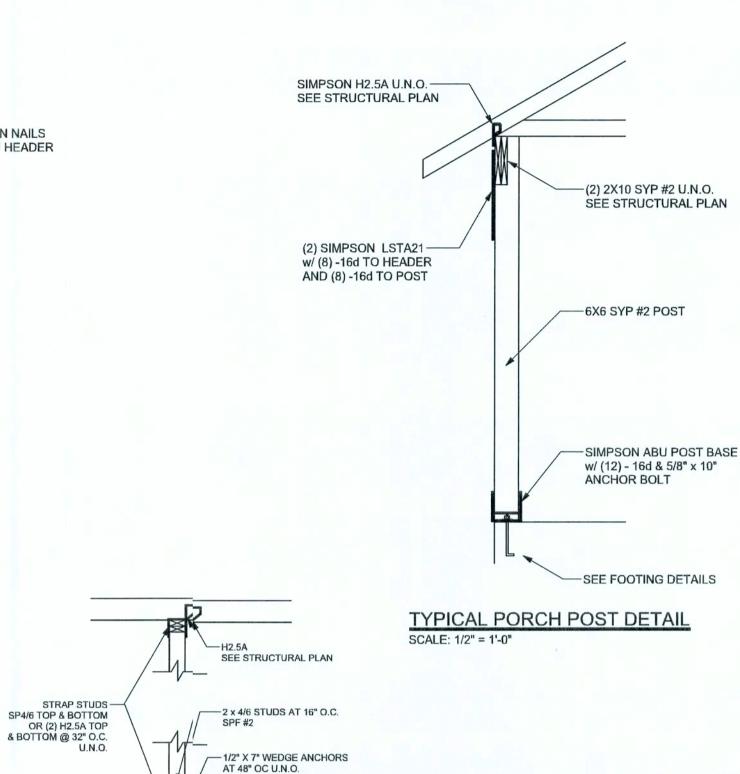
Plan On Us LLC.

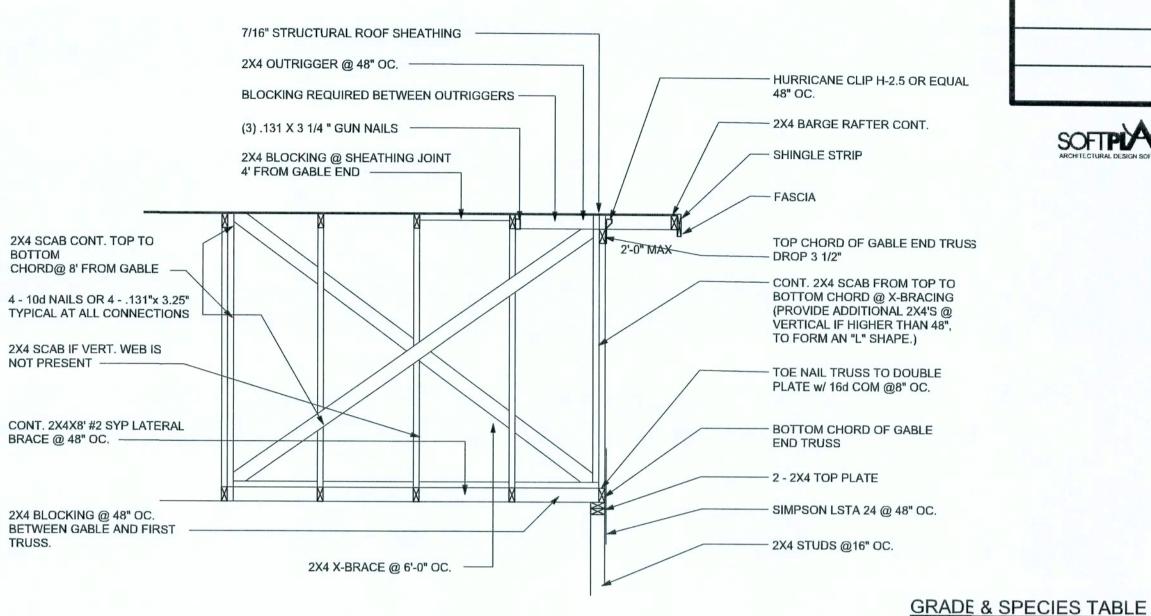
FINALS DATE: JOB NUMBER:

DRAWING NUMBER









TYPICAL GABLE END (X-BRACING)

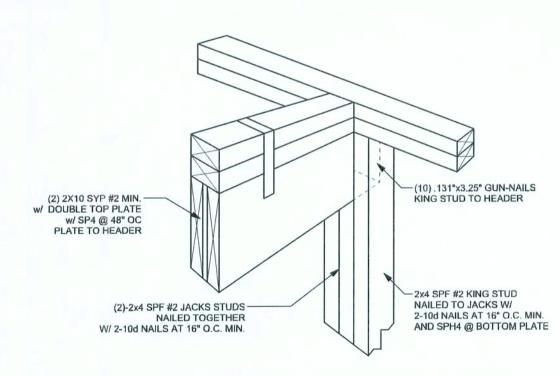
ALL MEMBERS SHALL BE SYP

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP#2	1200	1.6
2x10	SYP#2	1050	1.6
2x12	SYP#2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

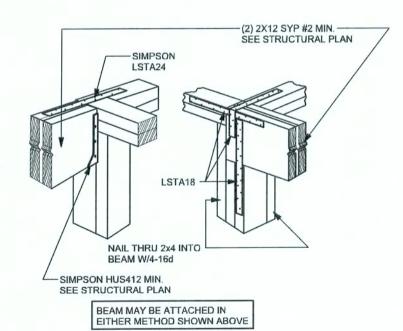
REVISIONS

SOFTPLAN

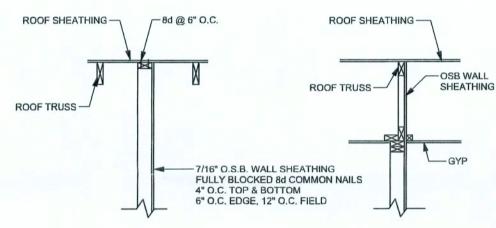
ONE STORY WALL SECTION SCALE: 3/4" = 1'-0"

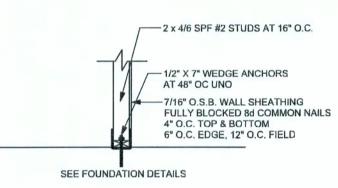


BEAM MID-WALL CONNECTION DETAIL SCALE: N.T.S.

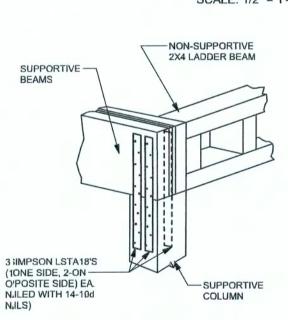


BEAM CORNER CONNECTION. DETAIL SCALE: N.T.S.

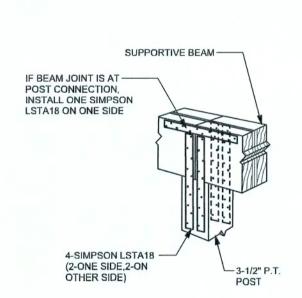




INTERIOR SHEAR WALL DETAIL



SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM SCALE: N.T.S.



SUPPORTIVE CENTER POST TO BEAM DETAIL

ANCHOR TABLE OBTAIN UPLIFT REQUIREMENTS FROM TRUSSS MANUFACTURER'S ENGINEERING

SEE FOUNDATION DETAILS

INTERIOR BE/ARING WALL

SCALE: 1/2" = 1'-0"

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUJSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		ST _{TUD} ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS

VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED

FIBER CONCRETE SLABS: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS

BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.) REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 48 * DB (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO. STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS,

AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU. WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST

REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

-	BUILDER'S RESPONSIBILITY
	THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
	PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
	PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
	VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R302.1.2 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN. PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

DESIGN DATA

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WEAN ROOF HEIGHT NOT EXCEEDING LEAST ON UPPER HALF OF HILL OR ESCARPMENT 60 SLOPE AND UNOBSTRUCTED UPWIND FOR 50	HORIZONTAL D FT IN EXP. B. 3	IMEN	SION	OR 60	OFT; NO
BUILDING IS NOT IN THE HIGH VELOCITY HURI	RICANE ZONE				
BUILDING IS NOT IN THE WIND-BORNE DEBRIS	REGION				
1.) BASIC WIND SPEED = 110 MPH					
2.) WIND EXPOSURE = B					
· ·					
3.) WIND IMPORTANCE FACTOR = 1.0					
4.) BUILDING CATEGORY = II					
5.) ROOF ANGLE = 10-45 DEGREES					
6.) MEAN ROOF HEIGHT = <30 FT					
7.) INTERNAL PRESSURE COEFFICIENT = N/A	(ENCLOSED B	UILDI	NG)		
8.) COMPONENTS AND CLADDING DESIGN W	IND PRESSUR	ES (T/	ABLE	R301	2(2))
A CONTRACTOR OF THE PARTY OF TH					-(-)/
~~	Zone	Effect	tive W	ind Are	ea (ft2)
			0		100
	1		-21.8	_	-18.1
6 3 1	2	19.9		18.1	-21.8
			-40.6		-40.6
5	2 O'hg	100	25 5	101	
2 2 2 2 5	3	19.9	-25.5	18.1	-21.8
2 2 2 2 5 5	3 3 O'hg		-68.3		-21.8 -42.4
2 2 2 5 5 5	3	21.8	-		-21.8
2 2 2 2 5 5	3 3 O'hg 4 5	21.8 21.8	-68.3 -23.6 -29.1	18.5 18.5	-21.8 -42.4 -20.4 -22.6
5 2 2 2 2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	3 O'hg 4 5	21.8 21.8 & Wind	-68.3 -23.6 -29.1	18.5	-21.8 -42.4 -20.4
5 2 2 2 2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	3 O'hg 4 5	21.8 21.8	-68.3 -23.6 -29.1 dows	18.5 18.5	-21.8 -42.4 -20.4 -22.6
5 2 2 2 2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	3 O'hg 4 5 Doors Wor (Zone	21.8 21.8 & Wind st Case 5, 10	-68.3 -23.6 -29.1 dows e ft2)	18.5 18.5 21.8	-21.8 -42.4 -20.4 -22.6
2 2 2 2 2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	3 O'hg 4 5	21.8 21.8 & Wind st Case 5, 10	-68.3 -23.6 -29.1 dows e ft2)	18.5 18.5	-21.8 -42.4 -20.4 -22.6
5 2 2 2 2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	3 O'hg 4 5 Doors Wor (Zone	21.8 21.8 & Wind st Case 5, 10	-68.3 -23.6 -29.1 dows e ft2)	18.5 18.5 21.8	-21.8 -42.4 -20.4 -22.6 -29.1 -22.9
5 2 2 2 2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	3 O'hg 4 5 Doors Wor (Zone	21.8 21.8 & Wind st Case 5, 10	-68.3 -23.6 -29.1 dows e ft2)	18.5 18.5 21.8	-21.8 -42.4 -20.4 -22.6 -29.1 -22.9
2 2 2 1 2 2 1 5 5 5 5 5 5 5 5 5 5 5 5 5	3 O'hg 4 5 Doors Wor (Zone	21.8 21.8 & Wind st Case 5, 10	-68.3 -23.6 -29.1 dows e ft2)	18.5 18.5 21.8	-21.8 -42.4 -20.4 -22.6 -29.1 -22.9
5 2 2 2 2 2 5 5 5 5 5 5 5 5 5 5 5 5 5 5	3 O'hg 4 5 Doors Wor (Zone	21.8 21.8 & Wind st Case 5, 10	-68.3 -23.6 -29.1 dows e ft2)	18.5 18.5 21.8	-21.8 -42.4 -20.4 -22.6 -29.1 -22.9
DESIGN LOADS	3 O'hg 4 5 Doors Wor (Zone	21.8 21.8 & Wind st Case 5, 10	-68.3 -23.6 -29.1 dows e ft2)	18.5 18.5 21.8	-21.8 -42.4 -20.4 -22.6 -29.1 -22.9
4 3 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	3 O'hg 4 5 Doors Wor (Zone 8x7 Gai	21.8 21.8 & Wind st Case 5, 10	-68.3 -23.6 -29.1 dows e ft2)	18.5 18.5 21.8	-21.8 -42.4 -20.4 -22.6 -29.1 -22.9

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

ROOF 20 PSF (FLAT OR <4:12)

SOIL BEARING CAPACITY 1000PSF

16 PSF (4:12 TO <12:12)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

INDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

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ermission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have xamined this plan, and that the applicable

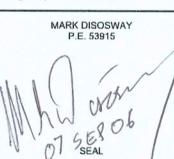
imensions. Refer all questions to

DIMENSIONS:

tated dimensions

portions of the plan, relating to wind engineerin comply with section R301.2.1, florida building code residential 2004, to the best of my

LIMITATION: This design is valid for one ouilding, at specified location.



Cornerstone Developm Zecher Bryan

The Griffin Model

ADDRESS: Lot #10 Magnolia Hills S/D Lake City, Florida

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PRINTED DATE: September 06, 2006 STRUCTURAL BY

Fax: (386) 269 - 4871

EVAN BEAMSLEY

FINALS DATE:

Sept. 6, 2006 JOB NUMBER: 608232

5-1

DRAWING NUMBER

