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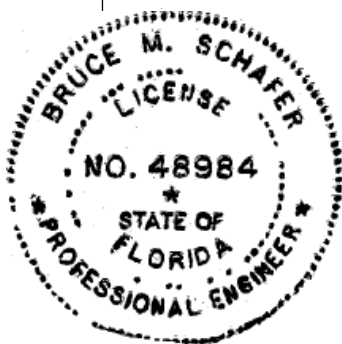
Prepared for:

Schafer Construction of Gainesville, Inc.
Beth Payne Residence
1601 SW Howell St Lake City FL

By:

Schafer Engineering, LLC

386-462-1340



Digitally signed
by Bruce M
Schafer

Date: 2022.07.25
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Schafer PE.

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SCHAFER ENGINEERING, LLC
7104 NW 42ND LANE \ GAINESVILLE FL. 32606
PHONE: 386-462-1340

Trusses: Pre-engineered, pre-fabricated with the manufacturer's required bracing system installed.

Roof Sheathing: Type: OSB Size: 7/16" Fastener type nails: 8d / .113 Ring Shank
Interior zone spacing: Interior: 6" Periphery: 4"
Edge and end zone spacing: Interior: 6" Periphery: 4"

Double Top Plate: Type: Spruce Grade: #2 Size: 2 x 4 Nail Size & Spacing: 10d min" @ 8 o.c.

Stud Type: Spruce Grade: #2 Size: 2 x 4 min.

Interior stud spacing: 16" End stud spacing: 16"

Required Shear Wall Siding: Type: OSB Thickness: 7/16"

96 ft Trans: Fastener 8d/131 Spacing: Int: 8 Edge: 4"

30 ft Long: Fastener 8d/131 Spacing: Int: 8 Edge: 4"

Allowable Unit Shear on Shear Walls: 314 pounds per linear foot
Allowable Unit Shear Transferred from Diaphragm: Trans: 170 Long: 142

Wall Tension Transferred by: Siding Nails: 8d/131 @ 4" O.C. Edges

Foundation Anchor Bolts: Concrete Strength: 3000 psi Size: 1/2"

Washer: 2" Embedment: 7" Location of first anchor bolt from corner: 8"

Anchor Bolts @ 48" o.c. Model: A307 Loc. from corner: 8"

Type of Foundation: (1) - #5 rebar continuous required in bond beam.

Floor Slab: 4" Cmu size: 8" x 16" Height: 48" Rein.: #5 at 72" o.c.

Monolithic Footing: Depth: 20" Bottom Width: 12 Rein.: 2 #5 rebars

Stemwall Footing: Width: 20 Depth: 10 Rein.: 2 #5 rebar

Interior Footings 20" Wide X 12" Deep with 2-#5 rebar continuous

Porch Columns: 6 X 6 X 10' syp #2 pt @
15'-3" o.c. max. spacing

Column Fasteners: Simpson PC66 \
PBS66 or equal

Special Comments: Install 2 ply 2 x 12 syp #2 with 7/16" osb flitch beam over all doors and
windows.

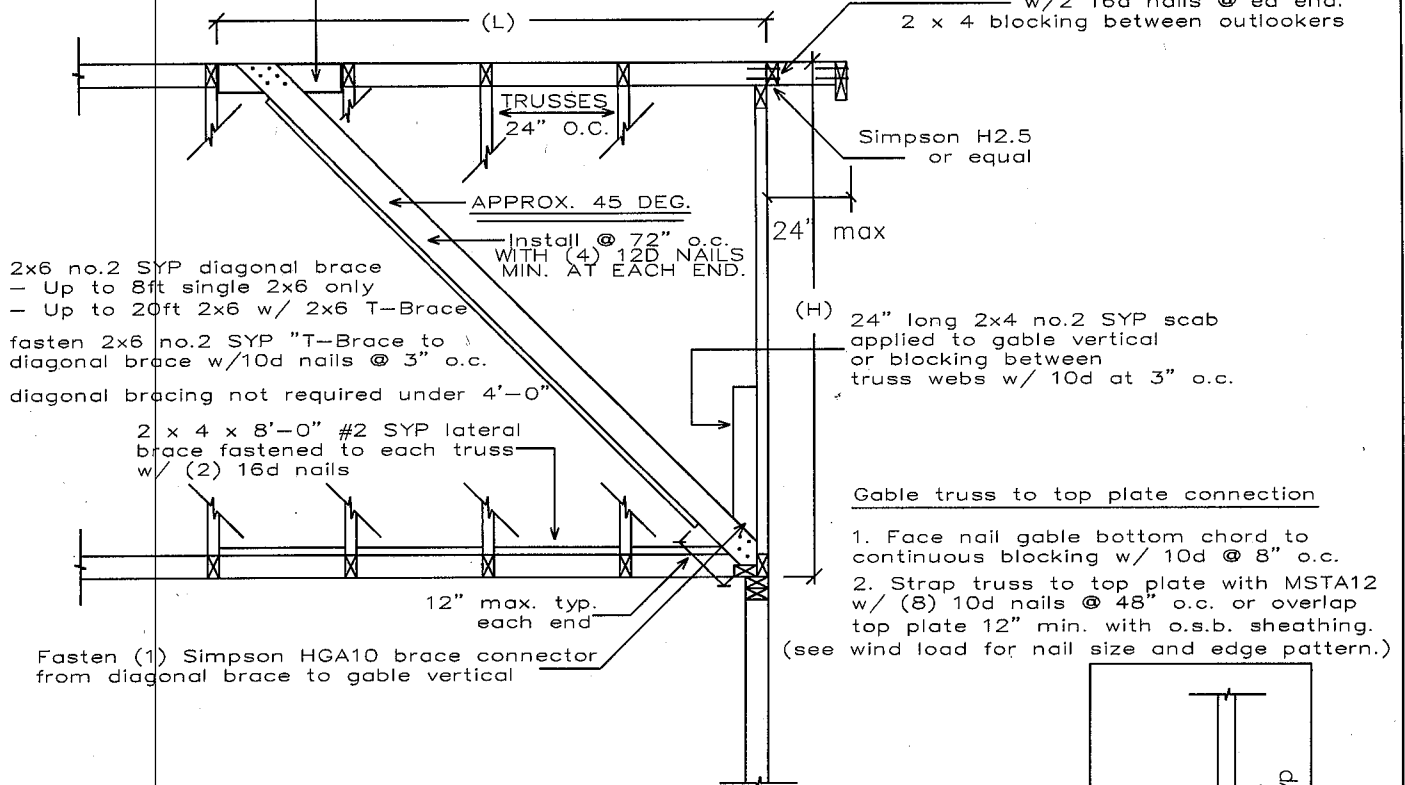
Notes:

1. Balloon frame all gable ends unless accompanied by gable end detail
2. All walls to be nailed with same nailing pattern as the shear walls.
3. This wind load is not valid without a raised, embossed seal. (NO COPIES).
4. 1500 psf soil bearing pressure minimum.
5. Fiber mesh or WWM may be used in concrete slab. All steel must be grade 40 min. Install standard 10" ACI hook top and bottom.
6. Trusses must be installed and anchored in accordance to the truss engineering.
7. All headers spanning 12' and over must be pre-engineered.
8. This is a windload only. Not a structural analysis. Schafer Engineering strongly recommends always having a structural analysis.
9. The foundation is for minimum design use, and may be increased.
10. Wind load is for one use only \ FBC-2017 \ No copies permitted
11. Install anchor bolts a 48" o.c., & Simpson SP1 at bottom plate and Simpson SP2 at top plate or equal @ 32" O.C. for all interior bearing walls.
12. Truss company to use all exterior porch walls for bearing when possible.

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Toe-Nail min 2x6 No 2 SYP blocking
between truss top chords with
(3) 10d each end min.

Install 2 x 4 syp outlookers @ 24" o.c.
w/ 2 16d nails @ ea end.
2 x 4 blocking between outlookers



2x6 no.2 SYP diagonal brace
- Up to 8ft single 2x6 only
- Up to 20ft 2x6 w/ 2x6 T-Brace

fasten 2x6 no.2 SYP "T-Brace to
diagonal brace w/ 10d nails @ 3" o.c.

diagonal bracing not required under 4'-0"

2 x 4 x 8'-0" #2 SYP lateral
brace fastened to each truss
w/ (2) 16d nails

Gable truss to top plate connection

1. Face nail gable bottom chord to continuous blocking w/ 10d @ 8" o.c.
2. Strap truss to top plate with MSTA12 w/ (8) 10d nails @ 48" o.c. or overlap top plate 12" min. with o.s.b. sheathing. (see wind load for nail size and edge pattern.)

2x6 #2 SYP diagonal brace
@ 72" o.c.

Fasten (1) Simpson HGA10 brace
connector from the diagonal
brace to gable vertical

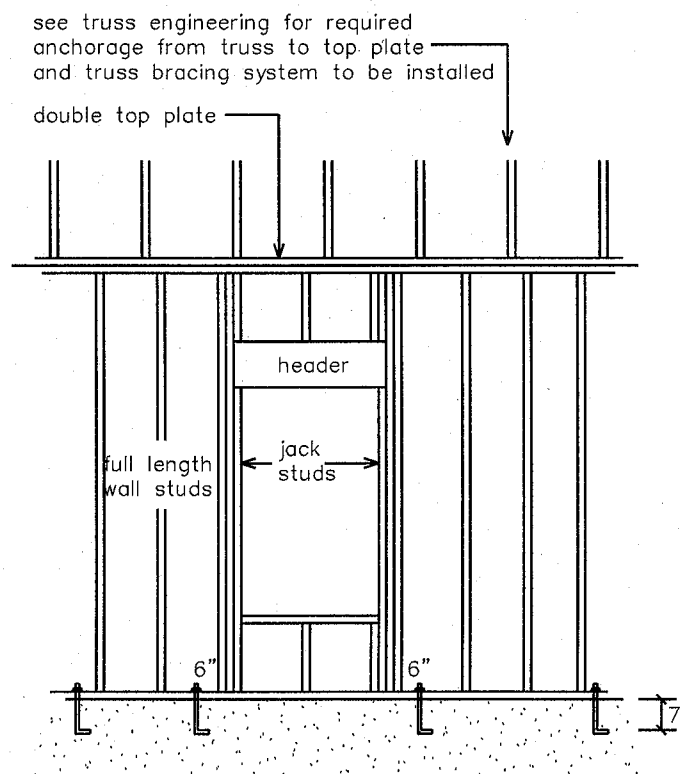
2 x 4 x 8'-0" #2 SYP lateral
brace @ 48" o.c. fastened to
each truss connection w/ (2) 16d nails

2x4 #2 SYP continuous blocking
fasten to dbl top plate w/ 10d
at 3" o.c.

TYPICAL GABLE END BRACING

Bruce Schafer, P. E. #48984
 7104 NW 42ND LN
 GAINESVILLE, FL. 32606

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total each truss uplift on the header and divide
by two for the top and bottom header anchorages.

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TIE-DOWN TABLES

HEADER STRAPPING				
Uplift Lbs	Top Connector	Rating Lbs	Bottom Connector	Rating Lbs
to 455	LSTA9	635	H3	320
to 910	LSTA12	795	2-H3	640
to 1265	LSTA18	1110	LTT19	1305
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2165
to 2865	3-LSTA18	3255	HD2A-3.5	2565
to 3700	3-LSTA24	3880	HD5A-3	3130

Total the uplift for each truss sitting on the header and divide by 2 to determine the uplift on the header. Use proper bolt anchors sufficient to support required uplift loads.

TRUSSES \ GIRDERS			
Uplift Lbs	Top Connector	Bottom Connector	Rating Lbs
to 535	H2.5A	NA	
to 1015	H10A	NA	
to 1215	TS22	LTT19	1305
to 1750	2-TS22	LTT20	1750
to 2570	2-TS22	HD2A	2565
to 3665	3-TS22	HD5A	3645
to 5420	2-MST37	HTT22	5250
to 9660	2-MST60	HD10A	8160

Two 12d common toenails are required per truss for each bearing point into top plate.
It is the contractors responsibility to provide a continuous load path from truss to foundation.

	TOP CONNECTOR	RATING LBS	BOTTOM CONNECTOR	RATING LBS
BEAM SEATS	LSTA18	1110	LTT19	1305
POSTS	2-LSTA18	2220	ABU44	2200

1. Simpson or equivalent hardware may be used.
For nailing into spruce members, multiply table values by .86
2. See truss engineering for anchor uplift values.
3. This schedule is not meant to be a replacement to the specified values of any manufactures values.

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	135	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	26.6	Deg
Type of Roof	Hipped	
Eave Height (Eht)	11.00	ft
Ridge Height (RHt)	25.58	ft
Mean Roof Height (Ht)	18.30	ft
Width Perp. to Wind (B)	66.50	ft
Width Parallel to Wind (L)	58.67	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.31
Flexible Structure	No

Calculated Parameters	
Importance Factor	1
Non-Hurricane, Hurricane (v=85-100 mph) & Alaska	
Table C6-4 Values	
Alpha =	7.000
zg =	1200.000
At =	0.143
Bt =	0.840
Am =	0.250
Bm =	0.450
Cc =	0.300
l =	320.00 ft
Epsilon =	0.333
Zmin =	30.00 ft

Gust Factor Category I: Rigid Structures - Simplified Method		
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85
Gust Factor Category II: Rigid Structures - Complete Analysis		
Zm	Zmin	30.00 ft
lzm	$Cc * (33/z)^{0.167}$	0.3048
Lzm	$l * (zm/33)^{Epsilon}$	309.99 ft
Q	$(1/(1+0.63*((B+Ht)/Lzm)^{0.63}))^{0.5}$	0.8844
Gust2	$0.925 * ((1+1.7 * lzm * 3.4 * Q)/(1+1.7 * 3.4 * lzm))$	0.8568
Gust Factor Category III: Flexible or Dynamically Sensitive Structures		
Vhref	$V * (5280/3600)$	198.00 ft/s
Vzm	$bm * (zm/33)^{Am} * Vhref$	87.00 ft/s
NF1	$NatFreq * Lzm / Vzm$	3.56 Hz
Rn	$(7.47 * NF1) / (1 + 10.302 * NF1)^{1.667}$	0.0627
Nh	$4.6 * NatFreq * Ht / Vzm$	0.97
Nb	$4.6 * NatFreq * B / Vzm$	3.52
Nd	$15.4 * NatFreq * Depth / Vzm$	10.39
Rh	$1 / (Nh - (1 / (2 * Nh^2) * (1 - Exp(-2 * Nh))))$	0.5766
Rb	$1 / (Nb - (1 / (2 * Nb^2) * (1 - Exp(-2 * Nb))))$	0.2440
Rd	$1 / (Nd - (1 / (2 * Nd^2) * (1 - Exp(-2 * Nd))))$	0.0917
RR	$((1/Beta) * Rn * Rh * Rb * (0.53 + 0.47 * Rd))^{0.5}$	0.7112
gg	$+(2 * LN(3600 * n1))^{0.5} + 0.577 / (2 * LN(3600 * n1))^{0.5}$	4.19
Gust3	$0.925 * ((1 + 1.7 * lzm * (3.4^2 * Q^2 + GG^2 * RR^2)^{0.5}) / (1 + 1.7 * 3.4 * lzm))$	1.07

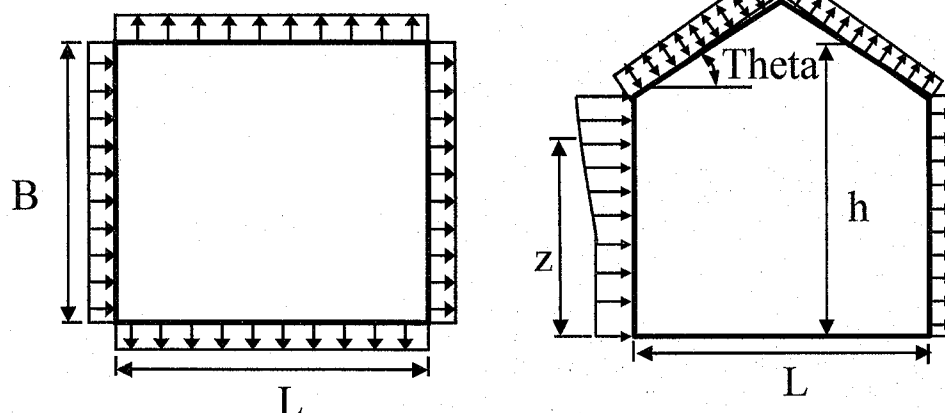
Gust Factor Summary	
Main Wind-force resisting system:	Components and Cladding:
Gust Factor Category: I	Gust Factor Category: I
Gust Factor (G) 0.86	Gust Factor (G) 0.86

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev. ft	Kz	Kzt	Kd	qz lb/ft ²	Pressure (lb/ft ²)	
					Windward Wall*	
			1.00		+GCpi	-GCpi
25.58	0.70	1.00	1.00	32.69	17.30	27.51
20	0.70	1.00	1.00	32.69	17.30	27.51
18.3	0.70	1.00	1.00	32.69	17.30	27.51
15	0.70	1.00	1.00	32.69	17.30	27.51

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	$2.01 \cdot (Ht/zg)^{(2/\alpha)}$	0.61	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	$.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot Kh \cdot Kht \cdot Kd$	28.38	psf

Wall Pressure Coefficients, Cp	
Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf)	
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 66.5 ft wall)	-0.50	-17.27	-7.05
Leeward Walls (Wind Dir Parallel to 58.67 ft wall)	-0.47	-16.62	-6.40
Side Walls	-0.70	-22.13	-11.91
Roof - Normal to Ridge (Theta >= 10)			
Windward - Max Negative	-0.22	-10.38	-0.16
Windward - Max Positive	0.28	1.58	11.80
Leeward Normal to Ridge	-0.60	-19.70	-9.48
Overhang Top	-0.22	-5.27	-5.27
Overhang Bottom	0.80	0.69	0.69
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 9.15 ft	-0.90	-26.99	-16.78
Dist from Windward Edge: 9.15 ft to 18.3 ft	-0.90	-26.99	-16.78
Dist from Windward Edge: 18.3 ft to 36.6 ft	-0.50	-17.27	-7.05
Dist from Windward Edge: > 36.6 ft	-0.30	-12.40	-2.19

* Horizontal distance from windward edge

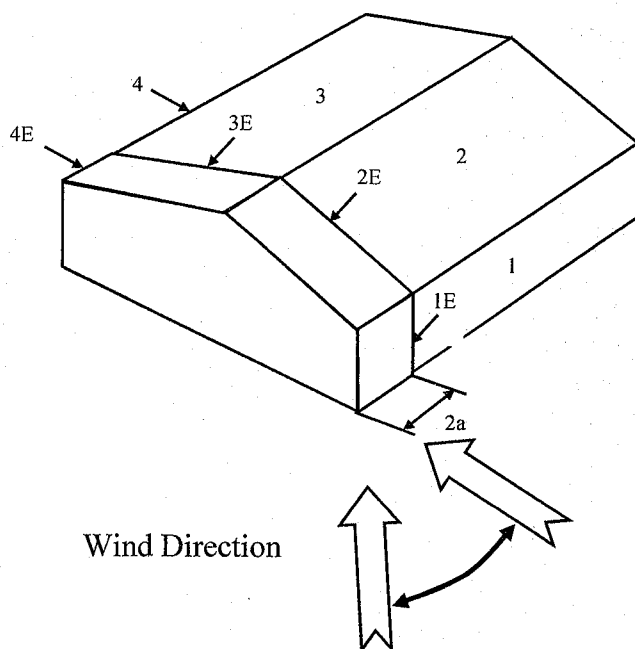
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht ≤ 60 ft

$$\begin{aligned}
 K_h &= 2.01 \cdot (H_t/z_g)^{(2/\alpha)} &= & 0.61 \\
 K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\
 Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 28.38
 \end{aligned}$$

Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.55	0.18	-0.18	32.69	12.09	23.85
2	-0.10	0.18	-0.18	32.69	-9.02	2.75
3	-0.45	0.18	-0.18	32.69	-20.49	-8.73
4	-0.39	0.18	-0.18	32.69	-18.64	-6.88
5	0.00	0.18	-0.18	32.69	-5.88	5.88
6	0.00	0.18	-0.18	32.69	-5.88	5.88
1E	0.73	0.18	-0.18	32.69	17.89	29.66
2E	-0.19	0.18	-0.18	32.69	-11.95	-0.18
3E	-0.58	0.18	-0.18	32.69	-24.99	-13.22
4E	-0.53	0.18	-0.18	32.69	-23.35	-11.58
5E	0.00	0.18	-0.18	32.69	-5.88	5.88
6E	0.00	0.18	-0.18	32.69	-5.88	5.88

$$* p = q_h * (GC_{pf} - GC_{pi})$$

**Figure 6-4 - External Pressure Coefficients, GCpf**

Loads on Main Wind-Force Resisting Systems w/ Ht ≤ 60 ft

$$\begin{aligned}
 K_h &= 2.01 \cdot (H_t/z_g)^{(2/\alpha)} &= & 0.61 \\
 K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\
 Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 28.38
 \end{aligned}$$

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	32.69	-20.59	-8.83
2	-0.69	0.18	-0.18	32.69	-28.44	-16.67
3	-0.37	0.18	-0.18	32.69	-17.98	-6.21
4	-0.45	0.18	-0.18	32.69	-20.59	-8.83
5	0.40	0.18	-0.18	32.69	7.19	18.96
6	-0.29	0.18	-0.18	32.69	-15.36	-3.60
1E	-0.48	0.18	-0.18	32.69	-21.57	-9.81
2E	-1.07	0.18	-0.18	32.69	-40.86	-29.09
3E	-0.53	0.18	-0.18	32.69	-23.21	-11.44
4E	-0.48	0.18	-0.18	32.69	-21.57	-9.81
5E	0.61	0.18	-0.18	32.69	14.06	25.82
6E	-0.43	0.18	-0.18	32.69	-19.94	-8.17

$$* p = qh * (GCpf - GCpi)$$

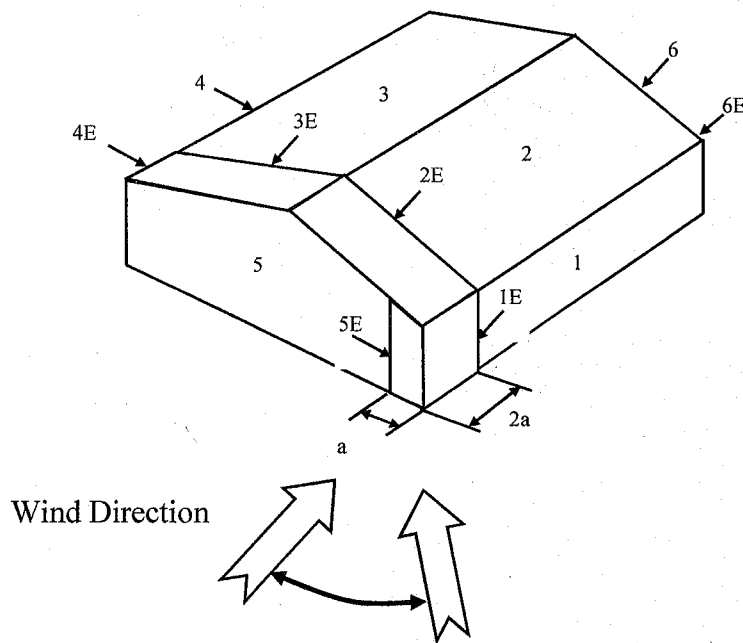
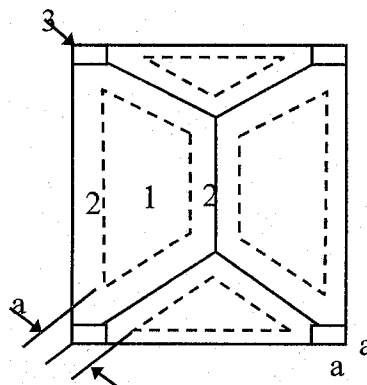
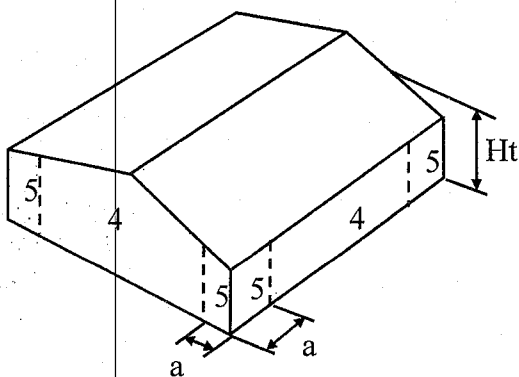


Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht ≤ 60 ft



Hipped Roof
 $10 < \text{Theta} \leq 30$

$$a = 5.867 \quad \Rightarrow \quad \boxed{5.87 \text{ ft}}$$
[illegible]

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Table 6-7 Internal Pressure Coefficients for Buildings, C_{pi}

Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	0.18	-0.18