

Daniel Shaheen



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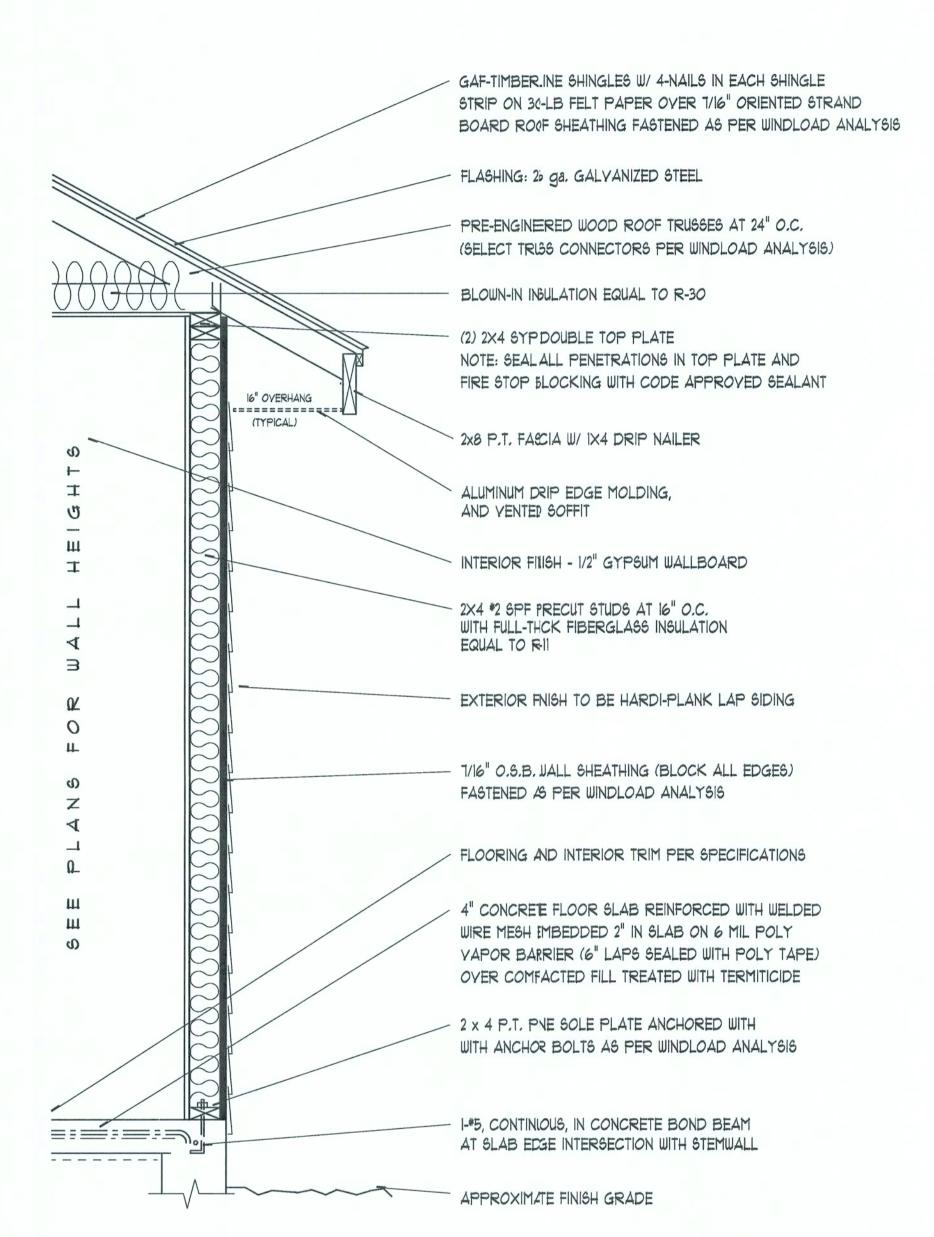
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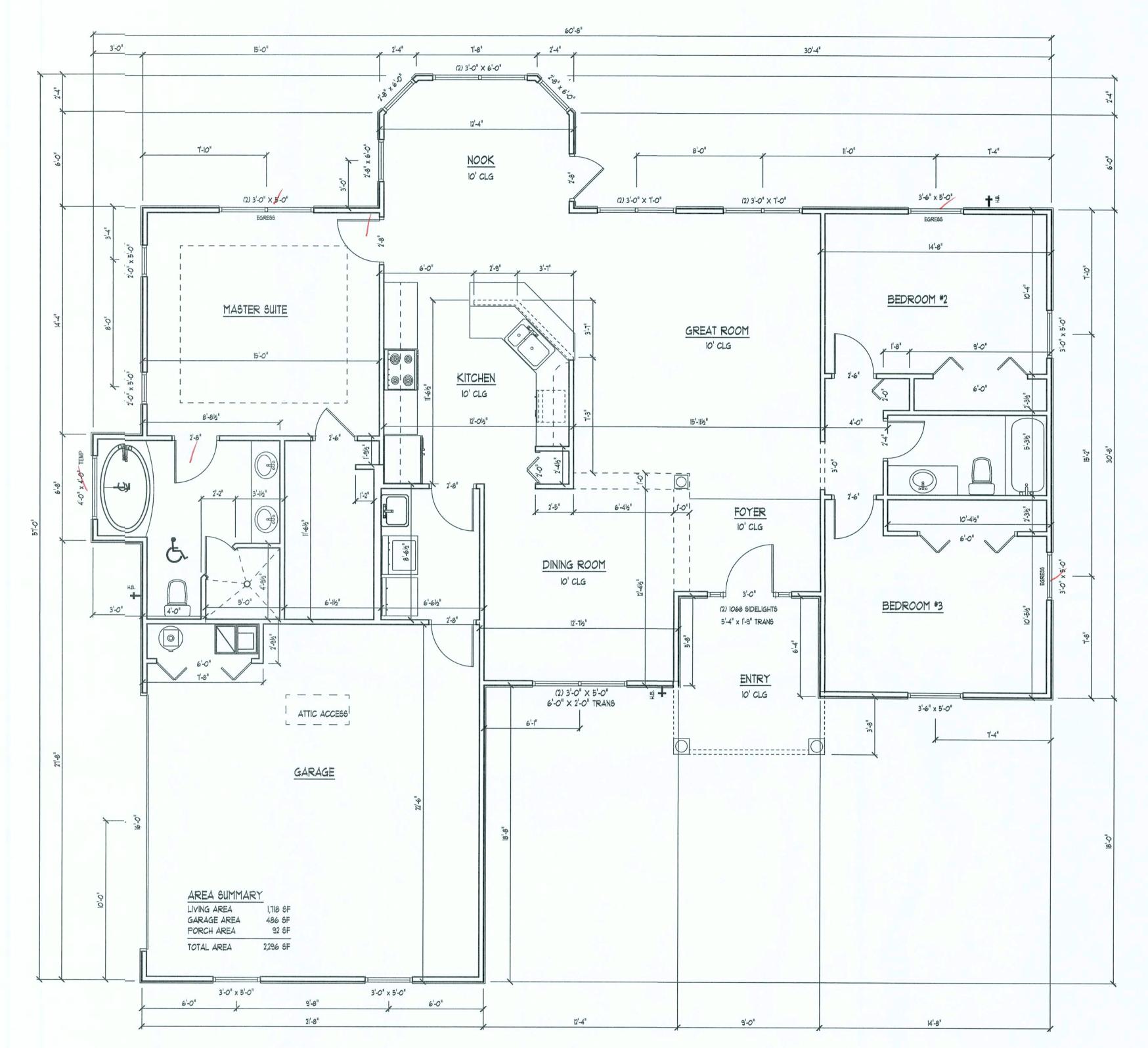
A

SHEET NUMBER 2 of 3

All work shall comply with the standard building code, and all applicable local codes and ordinances. Contractor shall verify all dimensions prior to commencing construction.



TYPICAL WALL SECTION SCALE: 1" = 1'0"



FLOOR PLAN SCALE: 1/4" = 1'

Daniel Shaheen

April 20, 2006

ARCHITECTURA O DESIGN

P.O. Box 273 LAKE CITY FL. 32056 (386) 754-0181

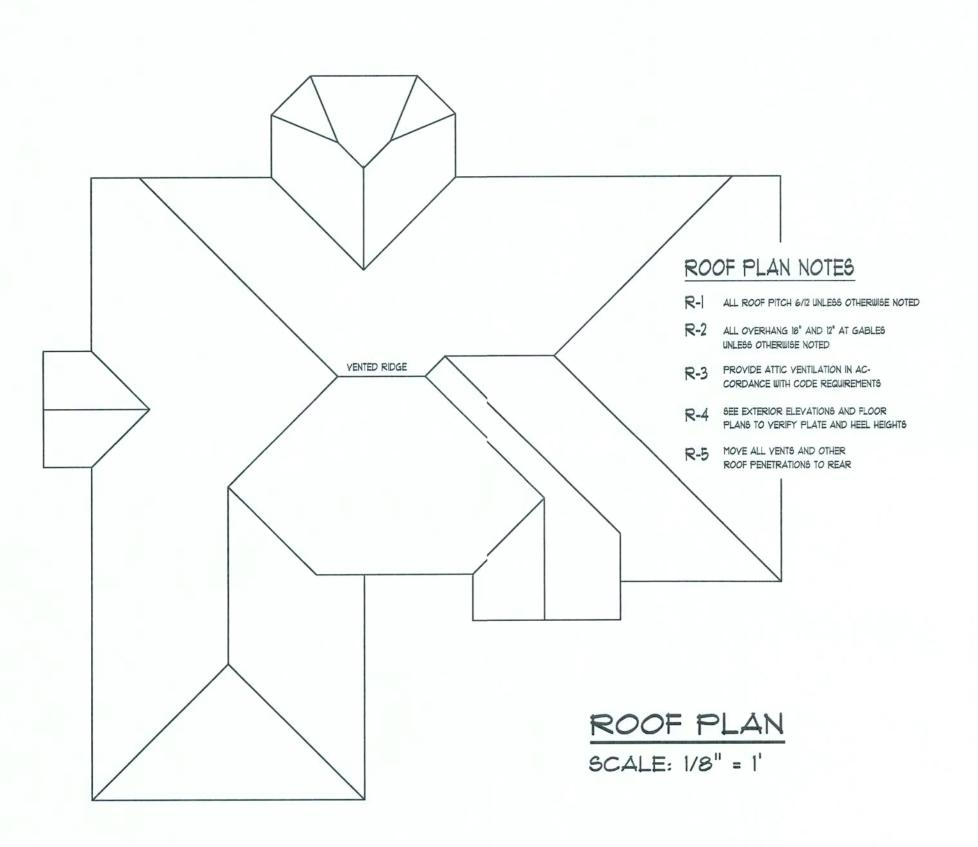
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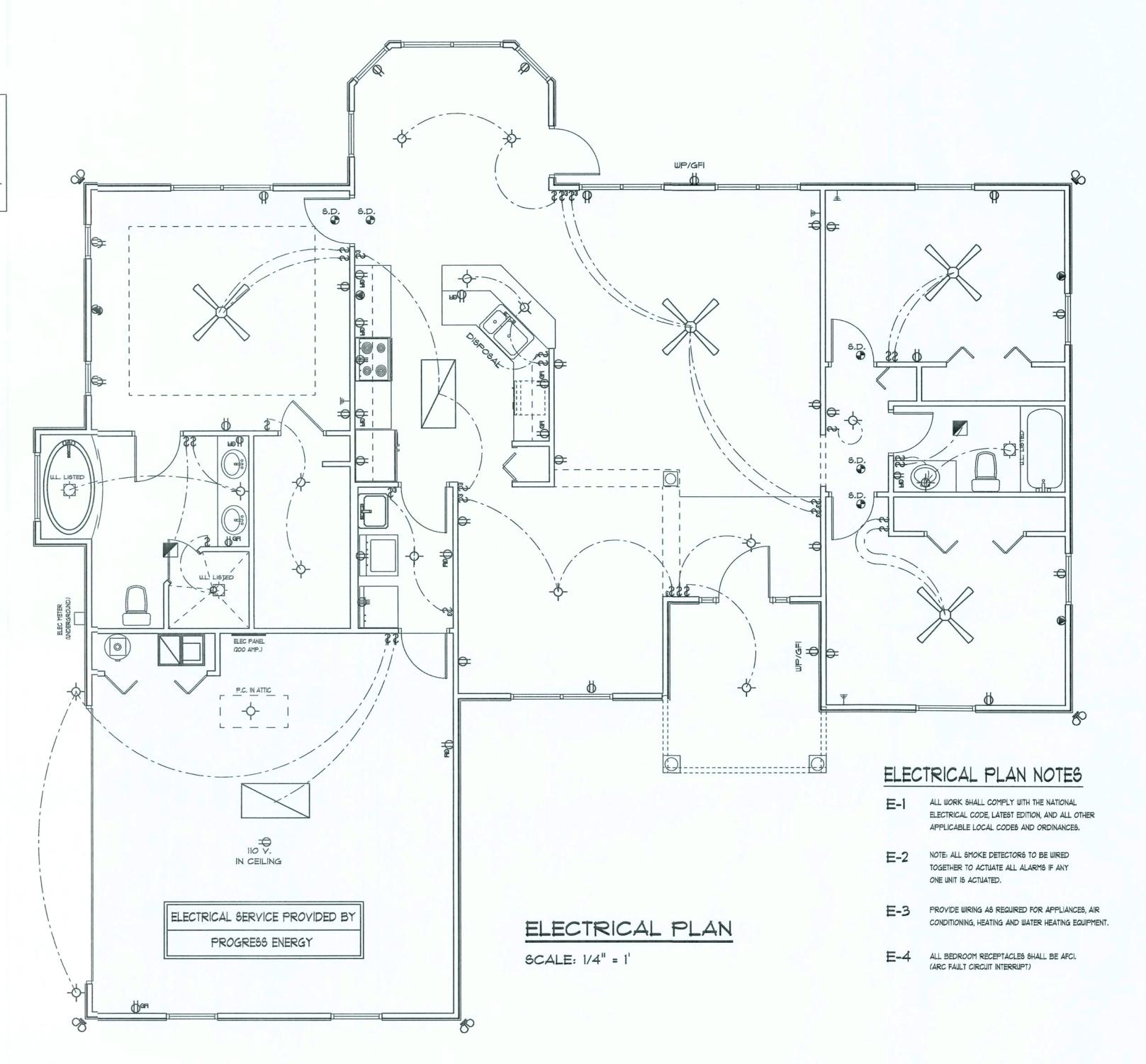
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NOTE:

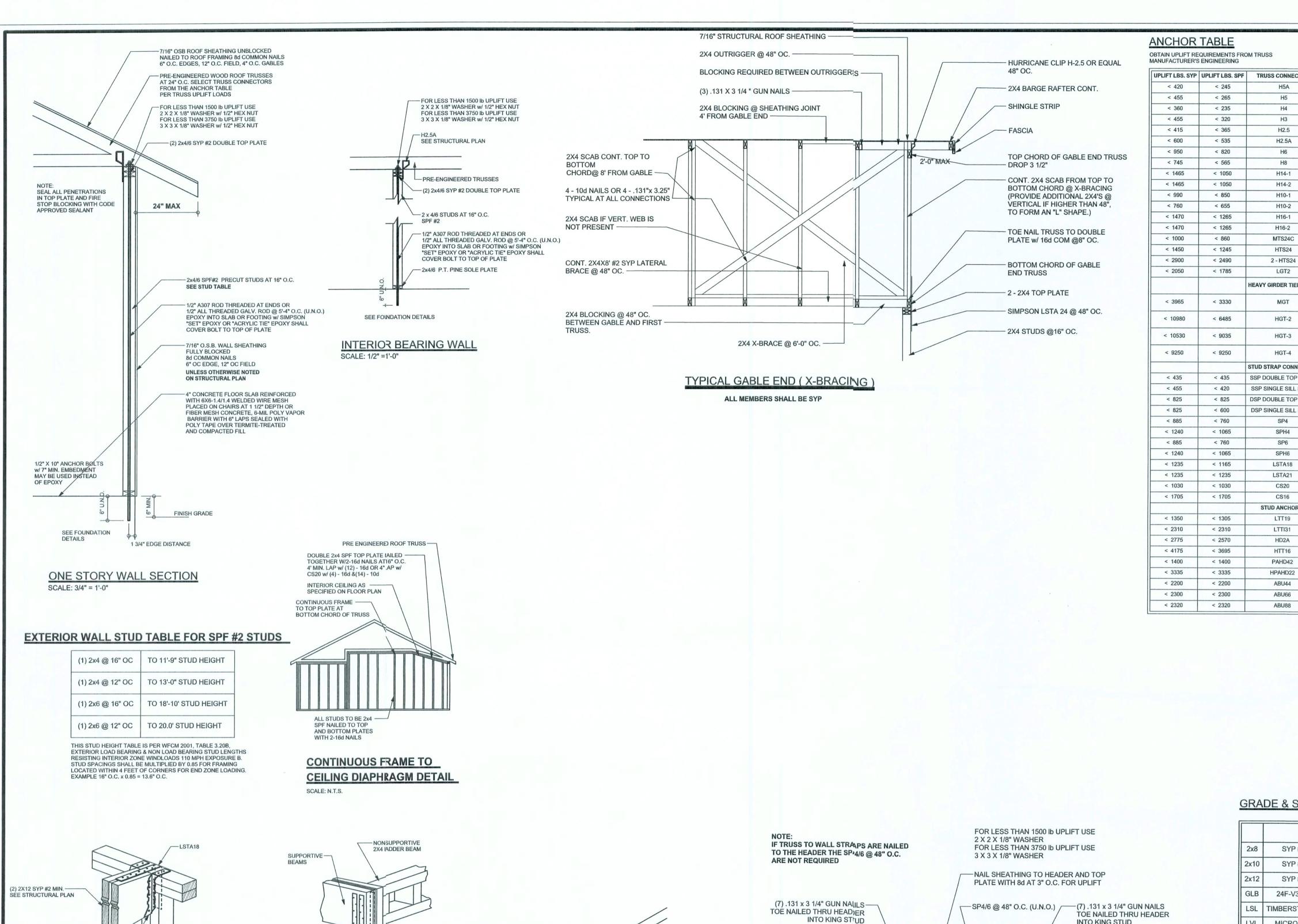
THIS ELECTRICAL PLAN IS A SCHEMATIC WITH SUGGESTED SWITCH, RECEPTACLE, AND LIGHT FIXTURE LOCATIONS. DUE TO VARYING LOCAL AND STATE CODES, REGULATIONS, AND STATUTES, IT IS THE RESPONSIBILITY OF THE OWNER AND/OR CONTRACTOR TO COMPLY WITH ALL LOCAL AND STATE CODES, REGULATIONS AND STATUTES.





SHEET NUMBER 3 of 3

All work shall comply with the standard building code, and all applicable local codes and ordinances. Contractor shall verify all dimensions prior to commencing construction.



SIMPSON H2.5A U.N.O. — SEE STRUCTURAL PLAN

(2) SIMPSON LSTA21 ---

w/ (8) -16d TO HEADER

AND (8) -16d TO POST

(2) 2X10 SYP #2 U.N.O.

6X6 SYP #2 POST

SEE STRUCTURAL PLAN

-SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10"

SEE FOOTING DETAILS

TYPICAL PORCH POST DETAIL

ANCHOR BOLT

3 SIMPSON LSTA18'S (1-ONE SIDE, 2-ON -

OPPOSITE SIDE) EA. NAILED WITH 14-10d

SCALE: N.T.S.

4-SIMPSON LSTA18 ----

(2-ONE SIDE, 2-ON

OTHER SIDE)

SCALE: N.T.S.

POST CONNECTION,

LSTA18 ON ONE SIDE

INSTALL ONE SIMPSON

COLUMN

SUPPORTIVE POSTTO BEAM

DETAIL FOR SINGLE BEAM

SUPPORTIVE BEAM ----

SUPPORTIVE CENTER POST TO BEAM DETAIL

SIMPSON HUS412 MIN. SEE STRUCTURAL PLAN

SCALE: N.T.S.

LSTA24

NAIL THRU 2x4 INTO

BEAM MAY BE ATTACHED IN

BEAM CORNER CONNECTION. DETAIL

BEAM W/4-16d

- SIMPSON HUS412 MIN.

SCALE: N.T.S.

SEE STRUCTURAL PLAN

-(4)-2x4 SPF #2 NAILED

MIN. (SEE STRUCTURAL PLAN)

SEE STRUCTURAL PLAN

NAILS AT 16" O.C.

BEAM MID-WALL CONNECTION DETAI

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

PLIFT LBS. SYP	T LBS. SYP UPLIFT LBS. SPF TRUSS C		NNECTOR* TO PLATES TO RA		TO STUDS		
< 420	< 245	H5A	3-8d	3-8d			
< 455	< 265	H5	4-8d	4-8d			
< 360	< 235	H4	4-8d	4-8d			
< 455	< 320	H3	4-8d	4-8d			
< 415	< 365	H2.5	5-8d	5-8d			
< 600	< 535	H2.5A	5-8d	5-8d			
< 950	< 820	H6	8-8d	8-8d			
< 745	< 565	H8	3 100, 1112				
< 1465	< 1050	H14-1	H14-1 13-8d 12-8d, 1 1/2"				
< 1465	< 1050	H14-2	-2 15-8d 12-8d, 1 1/2"				
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"			
< 760	< 655	H10-2	6-10d	6-10d			
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"			
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"			
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"			
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"			
< 2900	< 2490	2 - HTS24					
< 2050	< 1785	LGT2	14 -16d	14 -16d			
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION		
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT		
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT		
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT		
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT		
		STUD STRAP CONNECTOR*			TO STUDS		
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d		
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d		
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d		
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d		
< 885	< 760	SP4			6-10d, 1 1/2"		
< 1240	< 1065	SPH4	1		10-10d, 1 1/2"		
< 885	< 760	SP6			6-10d, 1 1/2"		
< 1240	< 1065	SPH6			10-10d, 1 1/2"		
< 1235	< 1165	LSTA18	14-10d				
< 1235	< 1235	LSTA21	16-10d				
< 1030	< 1030	CS20	18-8d				
< 1705	< 1705	CS16	28-8d				
		STUD ANCHORS*	TO STUDS		TO FOUNDATION		
< 1350	< 1305	LTT19	8-16d		1/2" AB		
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB		
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB		
< 4175	< 3695	HTT16	18 - 16d		5/8" AB		
< 1400	< 1400	PAHD42	16-16d				
< 3335	< 3335	HPAHD22	16-16d				
< 2200	< 2200	ABU44	12-16d		1/2" AB		
< 2300	< 2300	ABU66	12-16d				
< 2320	< 2320	ABURR	12-16d		1/2" AB		

GRADE & SPECIES TABLE

SYP #2

SYP #2

24F-V3 SP

TIMBERSTRAND

MICROLAM

PARALAM

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

1 DOOR WIDTH 3/8" x 4" LAG STAGGER .131 x 3 1/4" GN

24" O.C. 5" O.C.

18" O.C. 4" O.C.

16" O.C. 3" O.C.

GARAGE DOOR BUCK INSTALLATION DETAIL

ATTACH GARAGE DOOR BUCK TO STUD PACK AT

EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG

SCREWS w/ 1" WASHER LAG SCREWS MAY BE

TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4"

GN PER TABLE BELOW:

11' - 15'

16' - 18'

SCALE: N.T.S.

COUNTERSUNK. HORIZONTAL JAMBS DO NOT

GLB

Fb (psi) E (10⁶ psi)

1.6

1.6

1.6

1.8

2.0

5" O.C.

4" O.C.

1200

1050

975

2400

2900

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" × 6" W1.4 × W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OF SUPPORTED AT SUPPORTED A MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH MIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO N∂T CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO DWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS) UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL

ROOF SYSTEM DESIGN

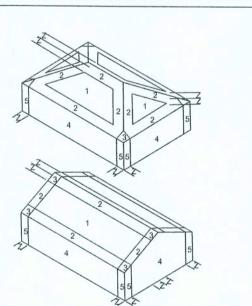
BEARING LOCATIONS.

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN IONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRE LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

2-5/8" AB

ON U	CLOSED SIMPLE DIAPHRAGM BUILDINGS WITH IN ROOF HEIGHT NOT EXCEEDING LEAST HORI JPPER HALF OF HILL OR ESCARPMENT 60FT IN PE AND UNOBSTRUCTED UPWIND FOR 50x HEI	ZONTAL D LEXP. B. 3	DIMENS OFT IN	ION EXP	OR 6	0 FT; NOT ND >10%
BUIL	DING IS NOT IN THE HIGH VELOCITY HURRICAN	NE ZONE				
BUIL	DING IS NOT IN THE WIND-BORNE DEBRIS REG	SION				
1.)	BASIC WIND SPEED = 110 MPH					
2.)	WIND EXPOSURE = B					
3.)	WIND IMPORTANCE FACTOR = 1.0					
4.)	BUILDING CATEGORY = II					
5.)	ROOF ANGLE = 10-45 DEGREES					
6.)	MEAN ROOF HEIGHT = <30 FT					
7.)	INTERNAL PRESSURE COEFFICIENT = N/A (EN	CLOSED E	BUILDIN	IG)		
8.)	COMPONENTS AND CLADDING DESIGN WIND F	PRESSUR	ES (TA	BLE	R301	.2(2))
						22.25
	*	Zone			r	ea (ft2)
	4// 4		10		33	100



SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

Doors & Windows 21.8 -29.1 Worst Case (Zone 5, 10 ft2)	2 2 O'hg 3	19.9	-25.5 -40.6		-21.8
2 O'hg	2 O'hg		-40.6	18.1	
3 19.9 -25.5 18.1 -21.8 3 O'hg -68.3 -42.4 4 21.8 -23.6 18.5 -20.4 5 21.8 -29.1 18.5 -22.6 Doors & Windows 21.8 -29.1 Worst Case (Zone 5, 10 ft2) 8x7 Garage Door 19.5 -22.9	3	19.9			40.0
3 O'hg		19.9	10000000		-40.6
4 21.8 -23.6 18.5 -20.4 5 21.8 -29.1 18.5 -22.6 Doors & Windows Worst Case (Zone 5, 10 ft2) 8x7 Garage Door 19.5 -22.9	3 O'hg		-25.5	18.1	-21.8
5 21.8 -29.1 18.5 -22.6 Doors & Windows Worst Case (Zone 5, 10 ft2) 8x7 Garage Door 19.5 -22.9			-68.3		-42.4
Doors & Windows 21.8 -29.1 Worst Case (Zone 5, 10 ft2) 8x7 Garage Door 19.5 -22.9	4	21.8	-23.6	18.5	-20.4
Worst Case (Zone 5, 10 ft2) 8x7 Garage Door 19.5 -22.9	5	21.8	-29.1	18.5	-22.6
(Zone 5, 10 ft2) 8x7 Garage Door 19.5 -22.9	Doors 8	& Wind	dows	21.8	-29.1
8x7 Garage Door 19.5 -22.9					
	8x7 Gara	age D	oor	19.5	-22.9
				18.5	-21.0
		Doors a Wors (Zone 8x7 Gar	Doors & Wind Worst Cas (Zone 5, 10 8x7 Garage Do	Doors & Windows Worst Case (Zone 5, 10 ft2) 8x7 Garage Door	Doors & Windows 21.8 Worst Case (Zone 5, 10 ft2) 8x7 Garage Door 19.5

DESIGN	LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)	
	30 PSF (SLEEPING ROOMS)	
	30 PSF (ATTICS WITH STORAGE)	
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)	
ROOF	20 PSF (FLAT OR <4:12)	
	16 PSF (4:12 TO <12:12)	
	12 PSF (12:12 AND GREATER)	
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)	

SOFTPIAN

REVISIONS

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PE No.53915, POB 868, Lake City, FL

tated dimensions supercede scaled

limensions. Refer all questions to

Mark Disosway, P.E. for resolution.

not proceed without clarification.

32056, 386-754-5419

Ewpl, Inc. Spec House Lot 31 Rolling Meadows S/D

ADDRESS: Lot 31 Rolling Meadows S/D Columbia County, Florida

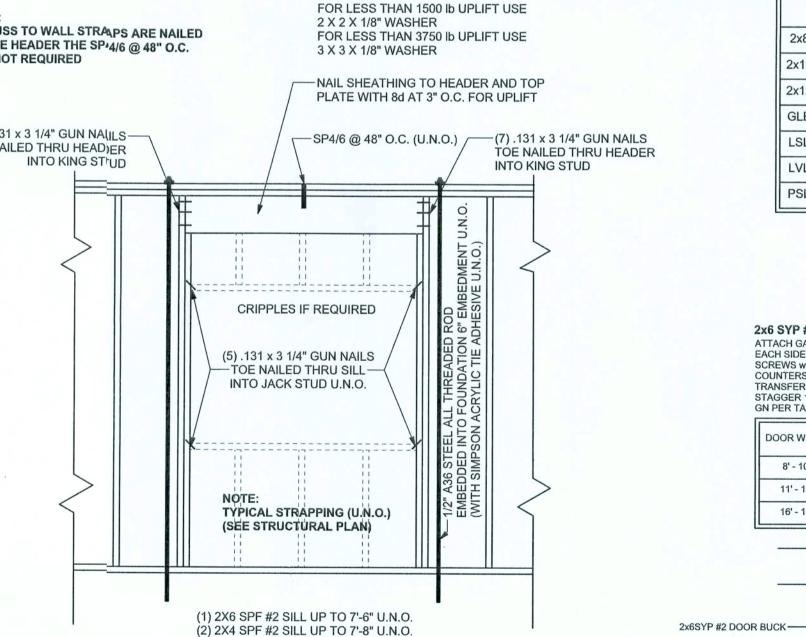
Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE:

April 28, 2006

DRAWN BY: STRUCTURAL BY David Disosway FINALS DATE: 28 / Apr / 06 JOB NUMBER: 604283

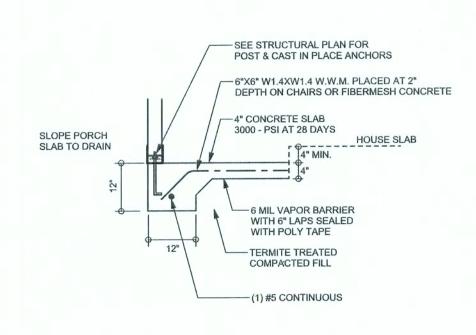
> DRAWING NUMBER S-1 OF 3 SHEETS



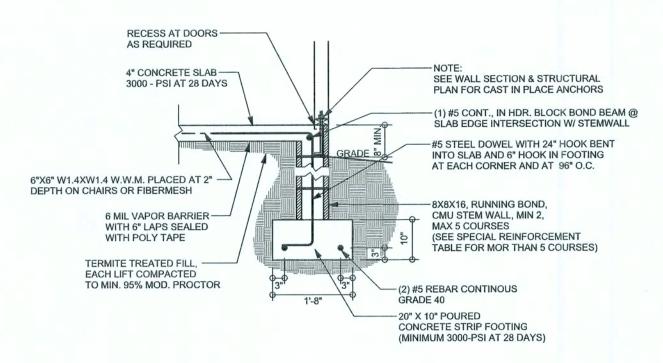
(1) 2X4 SPF #2 SILL UP TO 5'-1" U.N.O.

(FOR: 120 MPH, 10'-0" WALL HEIGHT U.N.O.)

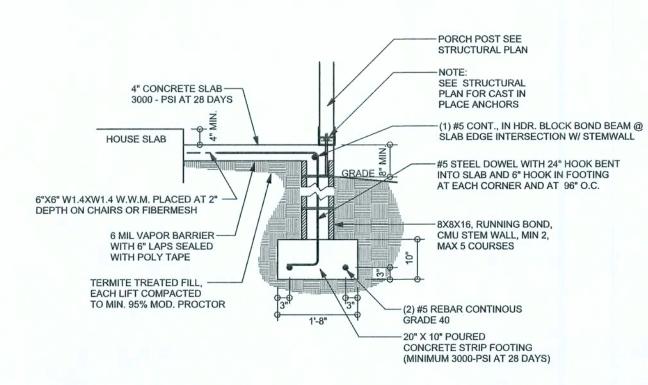
TYPICAL 1 STORY HEADER STRAPING DETAIL



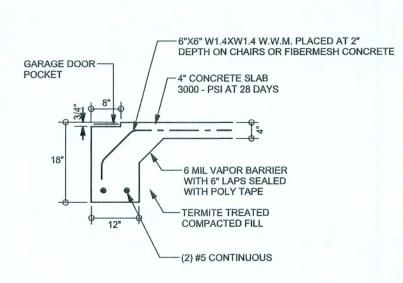
F5 PORCH FOOTING
S-2 SCALE: 1/2" = 1'-0"



F9 STEM WALL FOOTING S-2 SCALE: 1/2" = 1'-0"



F12 ALT. STEM WALL PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"

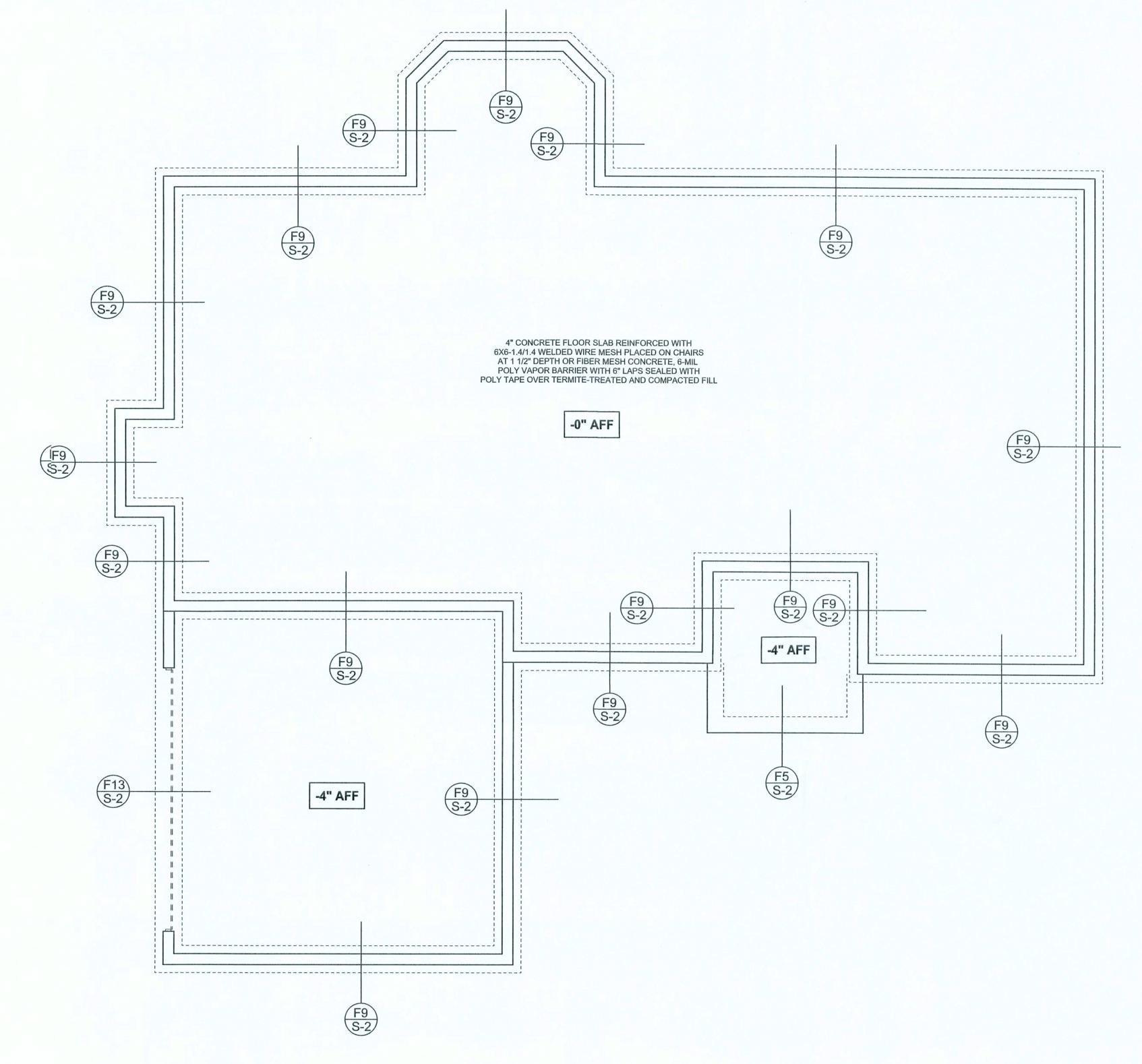


F13 ALT. STEM WALL GARAGE DOOR FOOTING
S-2 SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)			
		#5	#7	#8	#5	#7	#8	
3.3	3.0	96	96	96	96	96	96	
4.0	3.7	96	96	96	96	96	96	
4.7	4.3	88	96	96	96	96	96	
5.3	5.0	56	96	96	96	96	96	
6.0	5.7	40	80	96	80	96	96	
6.7	6.3	32	56	80	56	96	96	
7.3	7.0	24	40	56	40	80	96	
8.0	7.7	16	32	48	32	64	80	
8.7	8.3	8	24	32	24	48	64	
9.3	9.0	8	16	24	16	40	48	



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS

ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

SOFTPIAN APPLICATION OF THE PROPERTY OF THE PR

REVISIONS

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution.
Do not proceed without clarification.

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not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

A PROVI

Ewpl, Inc.

Spec House Lot 31 Rolling Meadows S/D

ADDRESS: Lot 31 Rolling Meadows S/D Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE:
April 28, 2006

DRAWN BY: STRUCTURAL BY:
David Disosway

FINALS DATE: 28 / Apr / 06

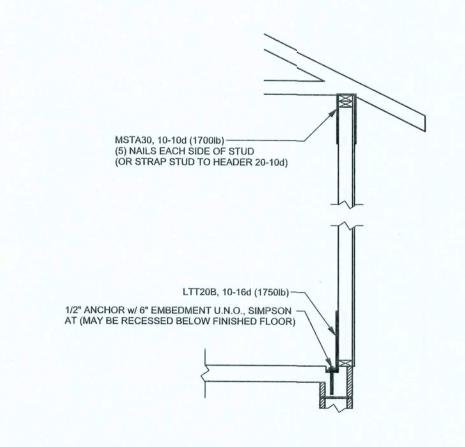
JOB NUMBER: 604283 DRAWING NUMBER

S-2

OF 3 SHEETS

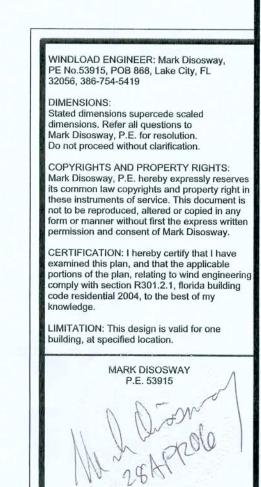
REVISIONS

SOFTPILAN



ALTERNATE WALL TIE CONNECTION WHERE THREADED ROD CANNOT BE PLACED IN WALL

SCALE: 1/2" = 1'-0"



Ewpl, Inc.

Spec House Lot 31 Rolling Meadows S/D

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PRINTED DATE: April 28, 2006 STRUCTURAL BY DRAWN BY: David Disosway

JOB NUMBER: 604283

DRAWING NUMBER

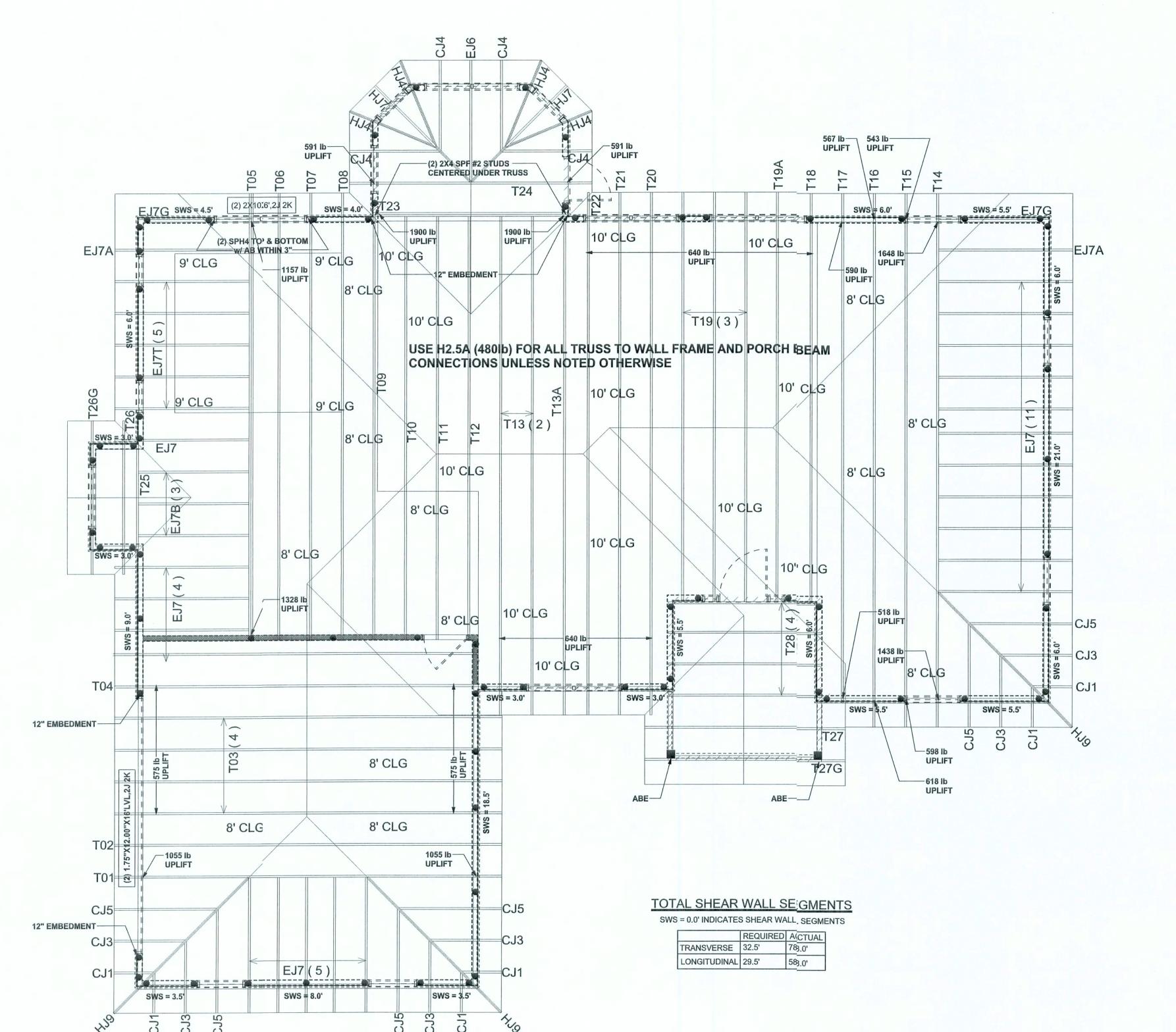
S-3

OF 3 SHEETS

FINALS DATE:

28 / Apr / 06

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #L142258



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- ALL LOAD BEARING FRAME WALL HEADERS SN-2 SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED

WALL LEGEND

sws = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

THREADED ROD LEGEND

- INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD - INDICATES LOCATION OF:

2ND FLOOR 1/2" A307 ALL THREADED ROD

HEADER LEGEND

