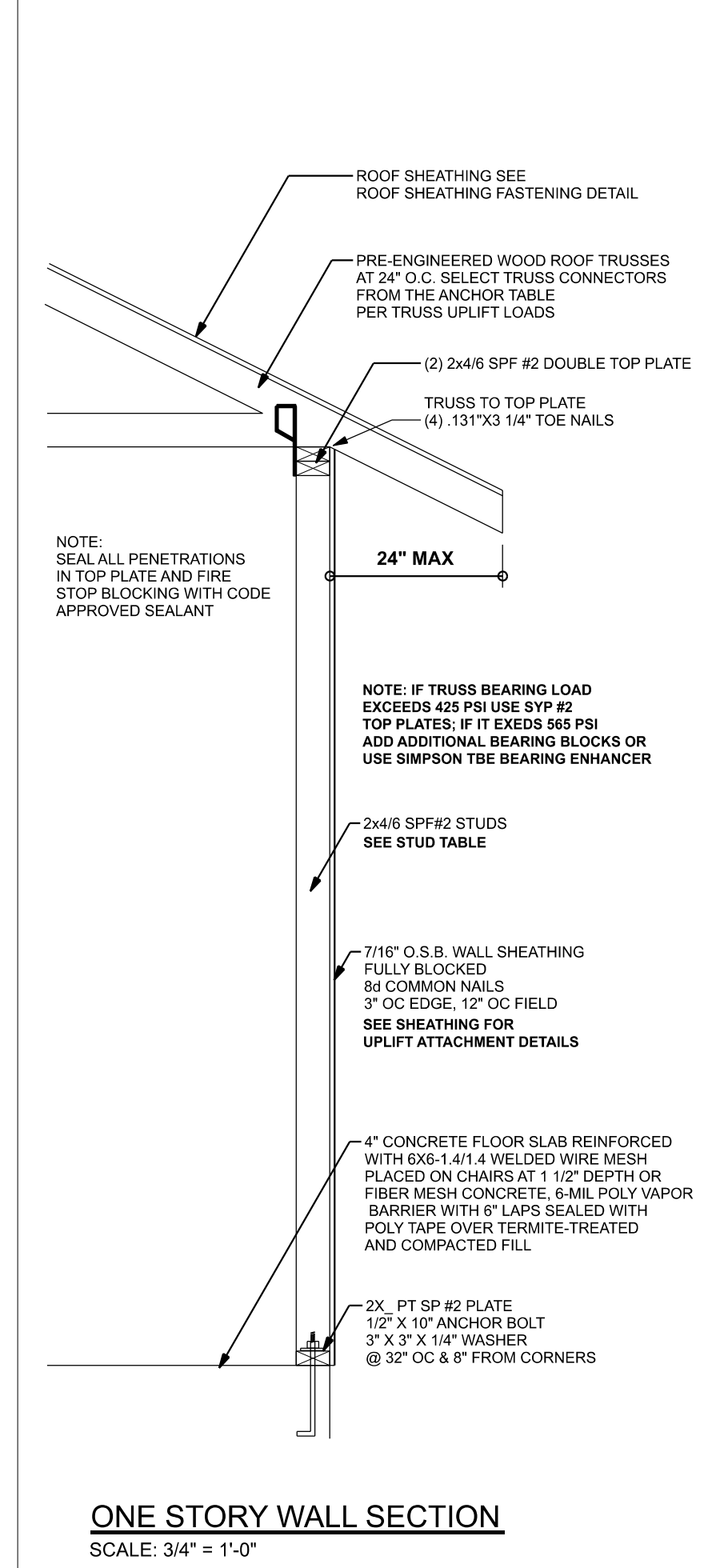
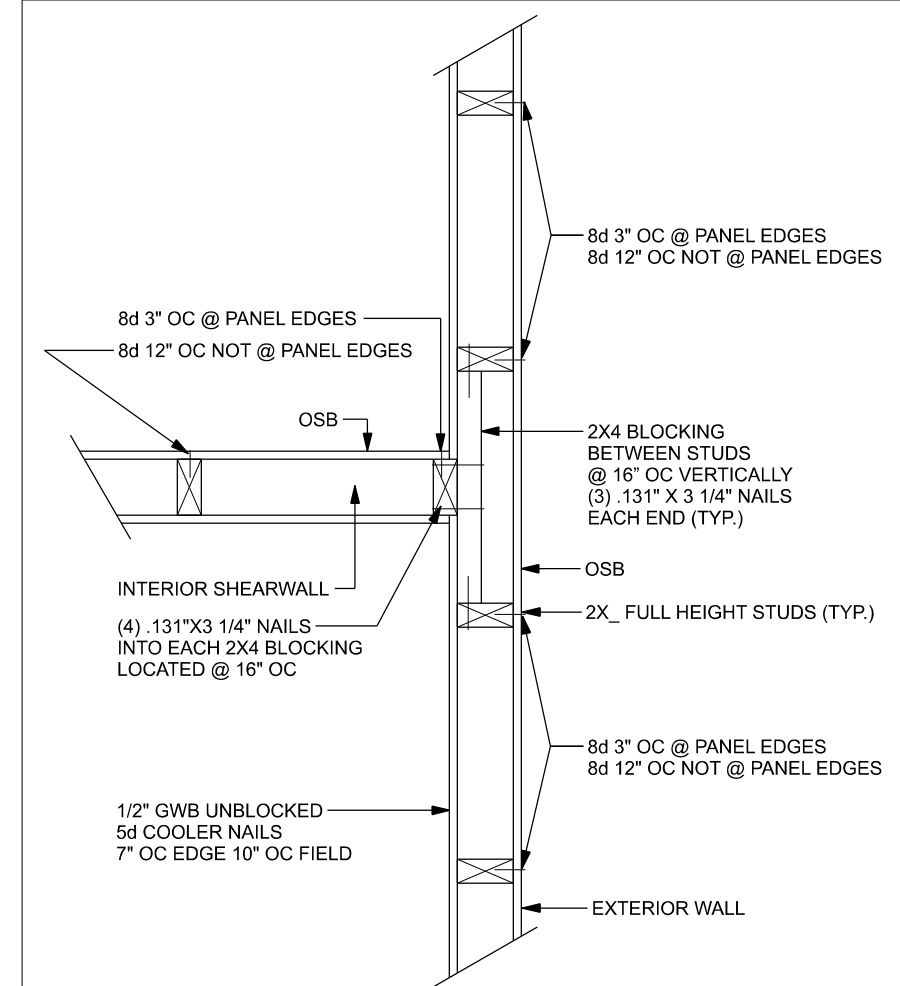
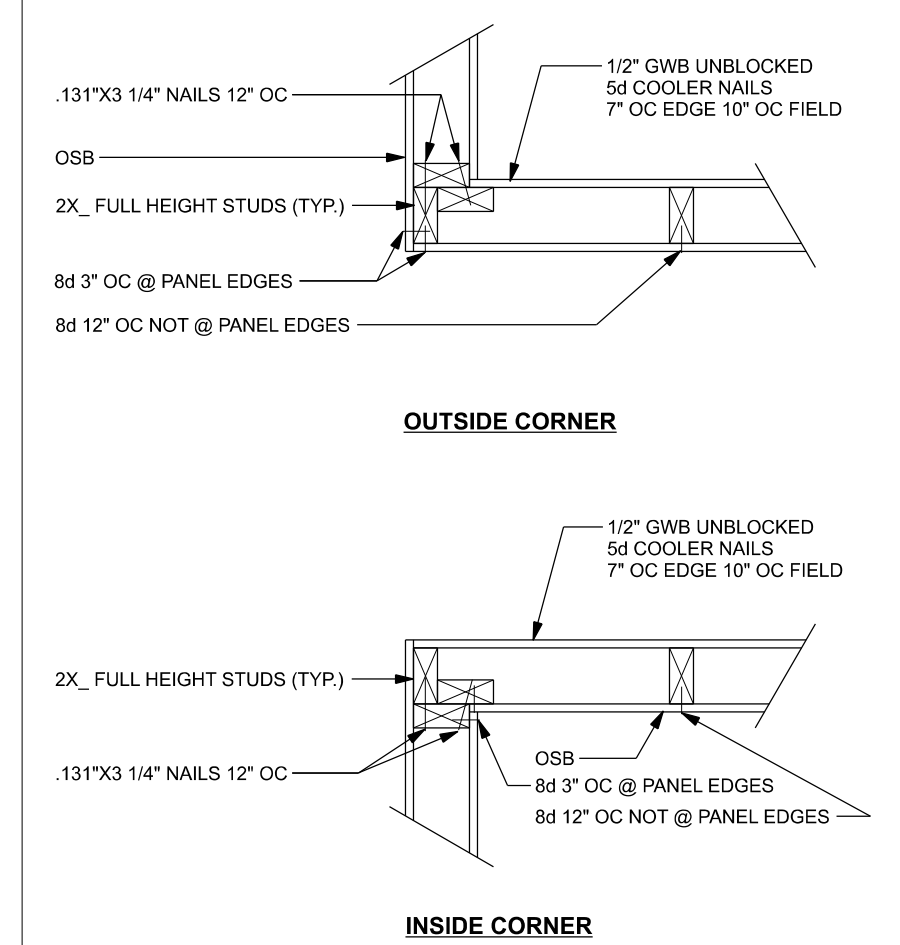




ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



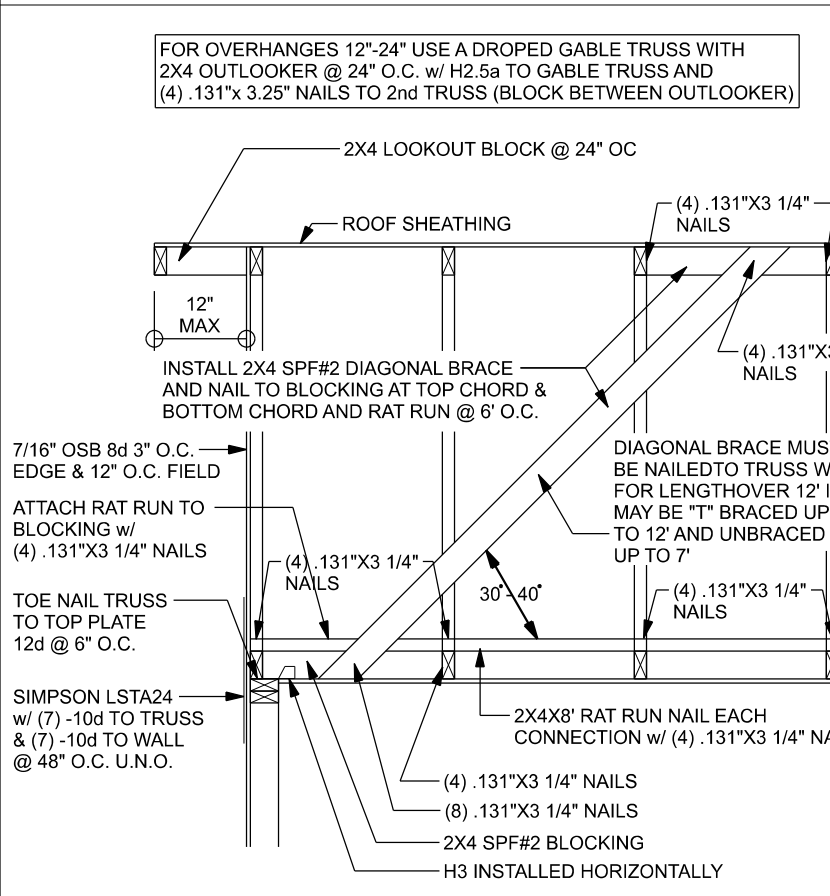
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



(TYP.) CORNER FRAMING
WOOD FRAME

Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.113")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.113")	6" oc	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.113")	6" oc	6" oc
130 mph Exp. C	15/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.113")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc

Note:
For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field.
Note:
This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



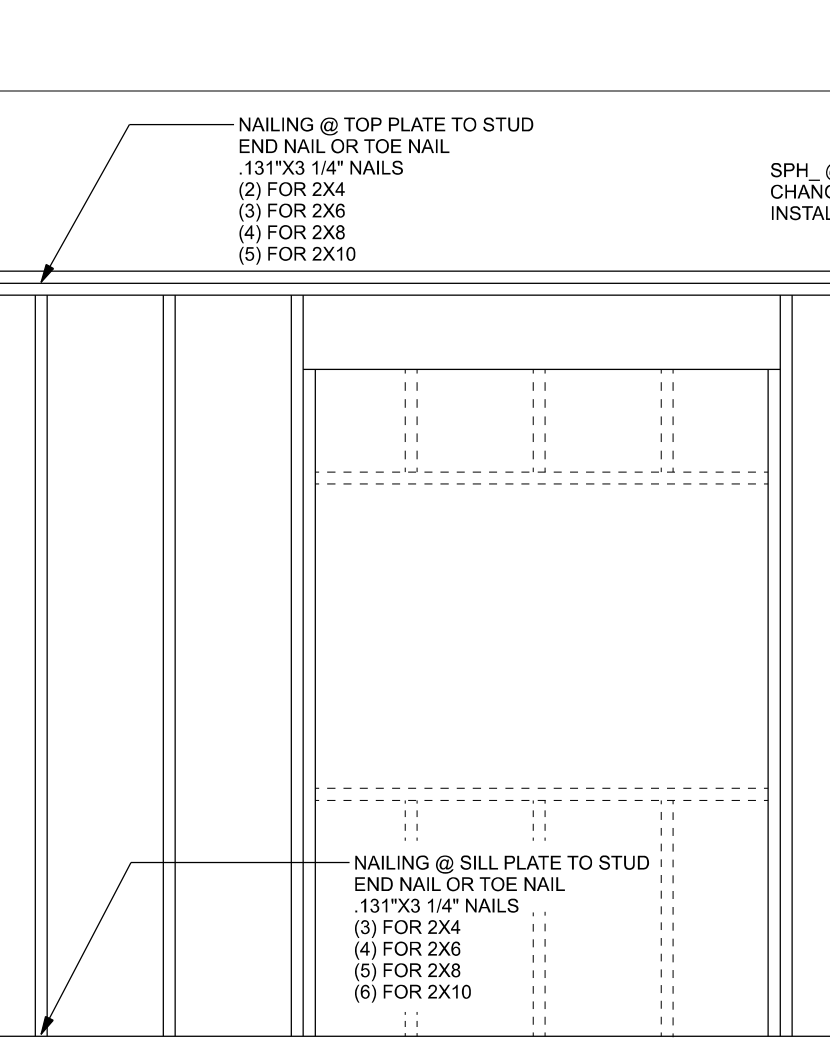
(TYP.) GABLE BRACING DETAIL
WOOD FRAME

FOR OVER-HANGES 12'-24" USE A DROPPED GABLE TRUSS WITH 2X4 OUTLOOKER @ 24" O.C. w/ H2.5A TO GABLE TRUSS AND (4) 131"X3 1/4" NAILS TO BLOCK BETWEEN OUTLOOKER)

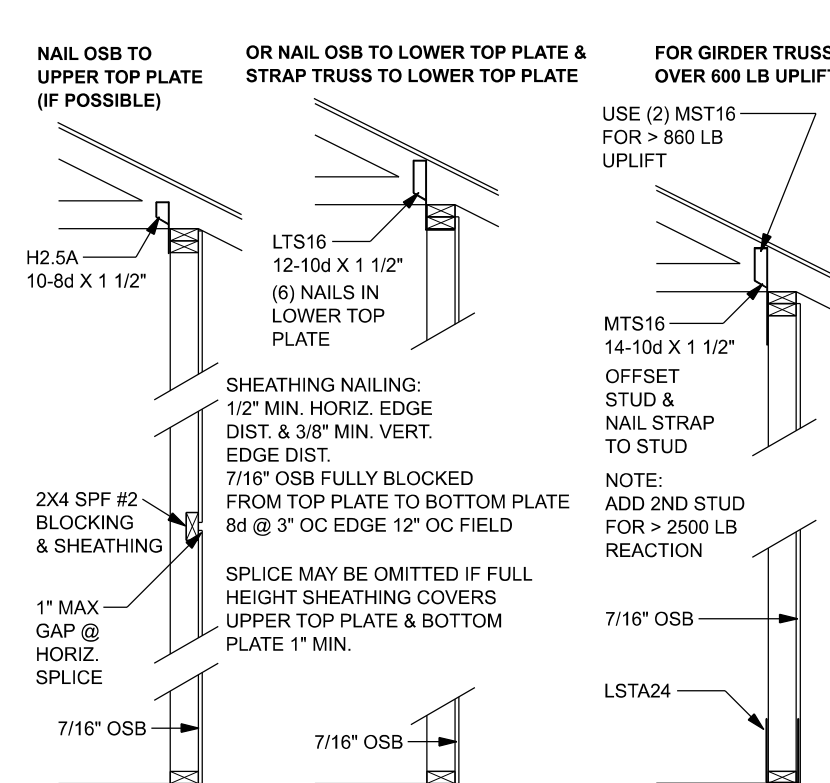
Uplift	Top Connection	Bottom Connection
< 1235	LSTA24, 14-10d wrap over plate	LSTA24, 14-10d wrap under plate
< 1455	MSTA24, 18-10d header to jacks	D1T22
< 1800	(2) MSTA24, 18-10d header to jacks	D1T22
< 2910	(2) MSTA24, 18-10d header to jacks	HTT4

DESIGN	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-3.2.8B
WIND SPEED	(1) 2x4 (2) 2x4 (1) 2x6 (2) 2x6	
130 MPH EXP. C	5'-3" 7'-9" 7'-7" 11'-3"	

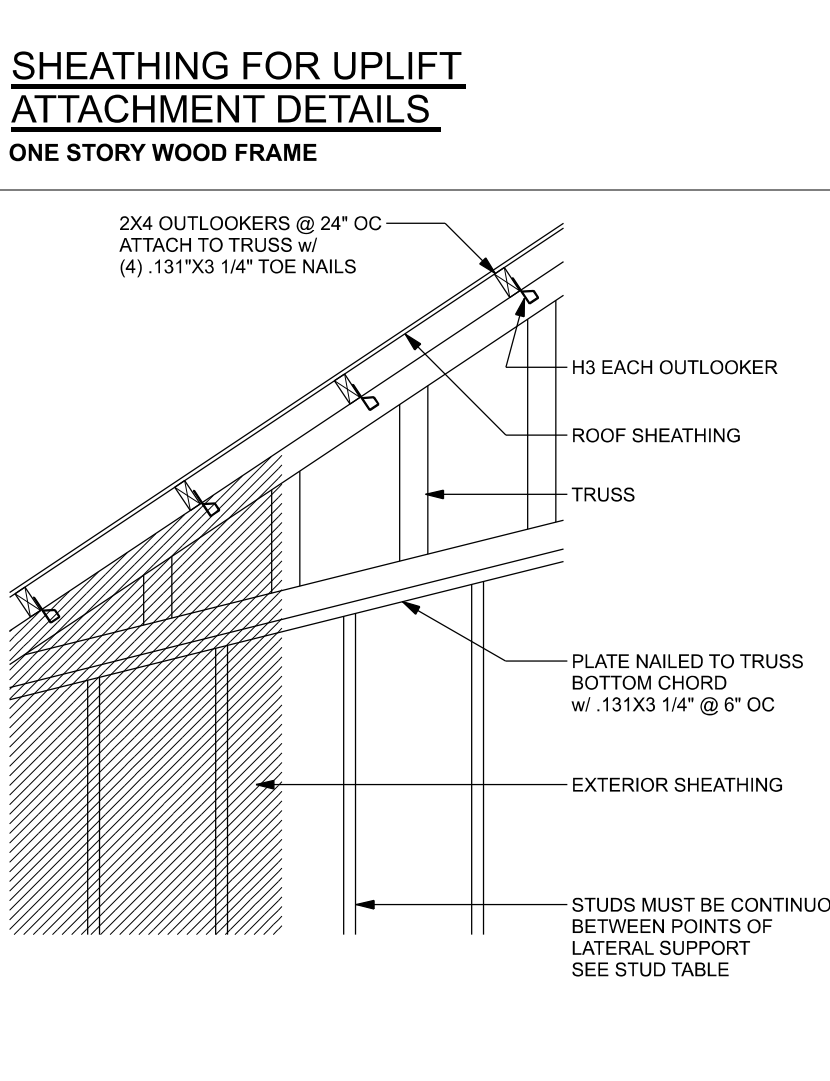
FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)



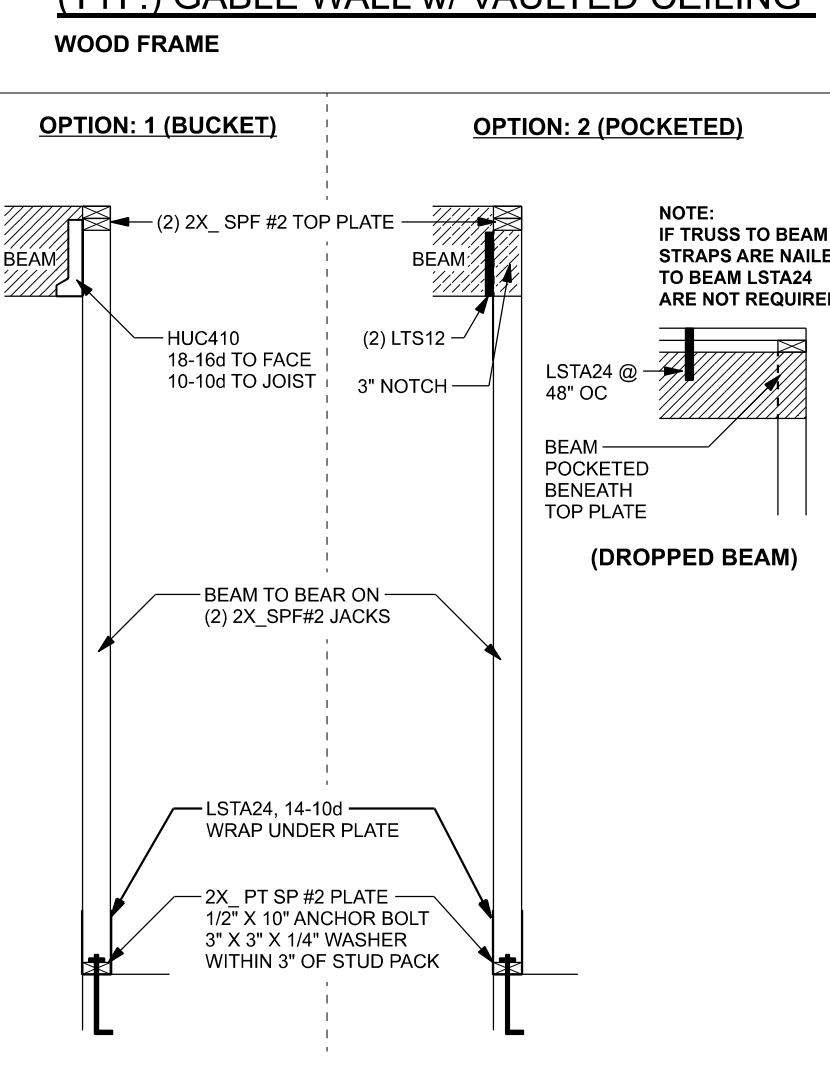
(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



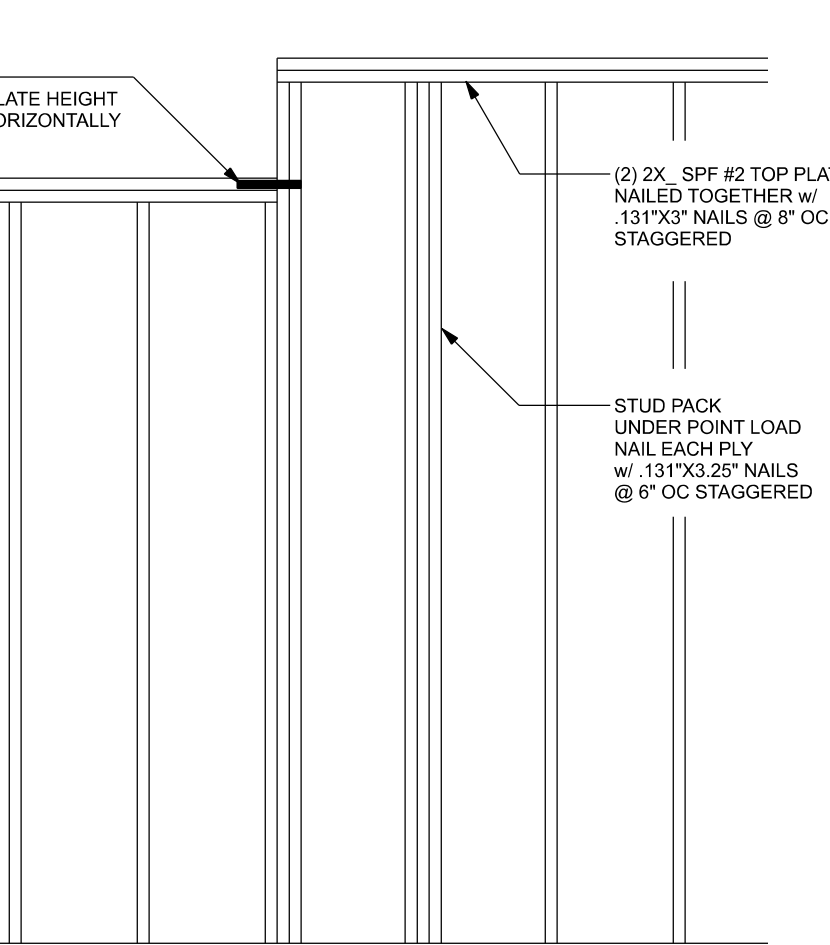
SHEATHING FOR UPLIFT ATTACHMENT DETAILS
ONE STORY WOOD FRAME



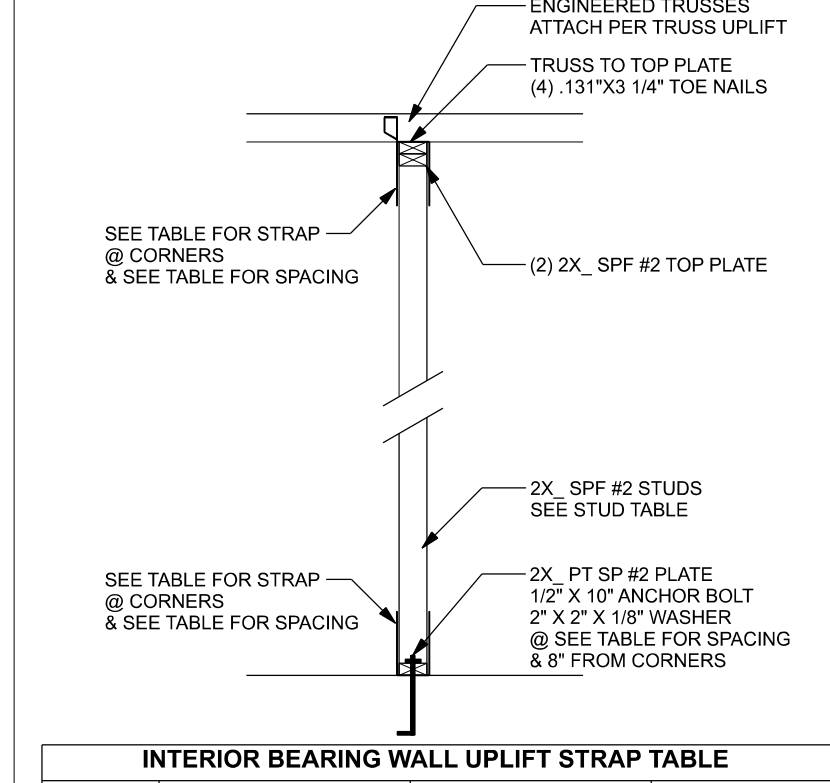
(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME



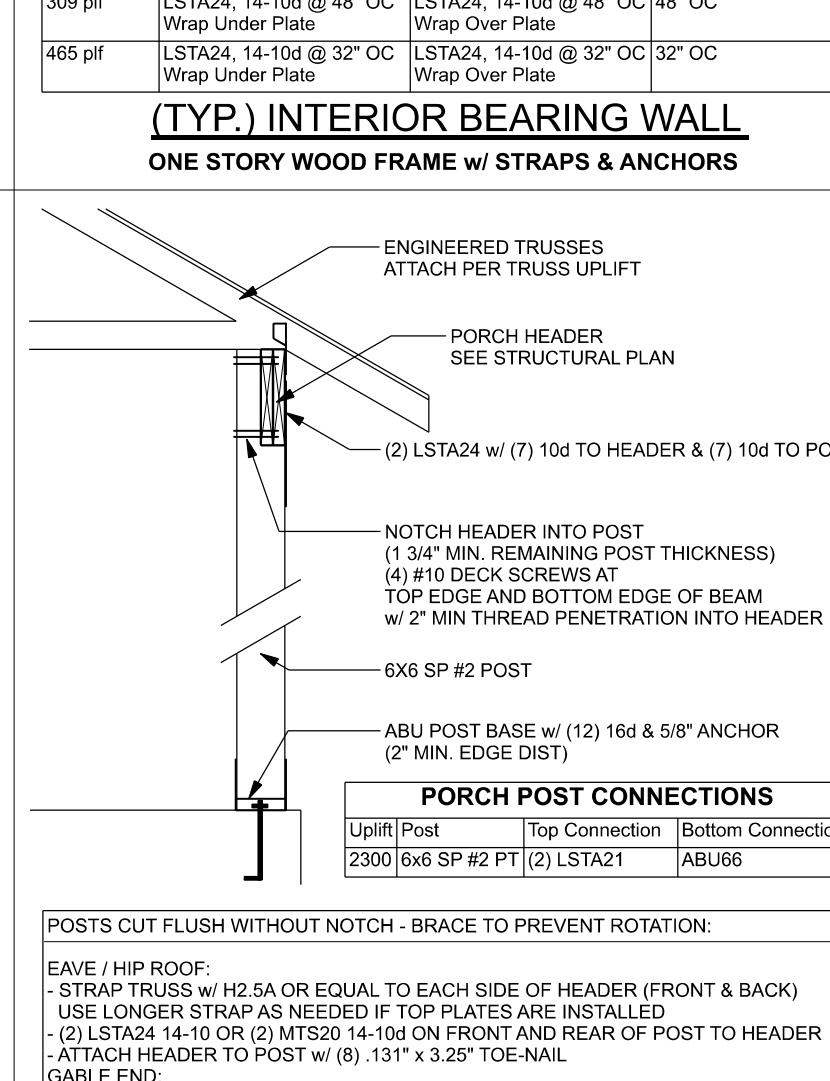
(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



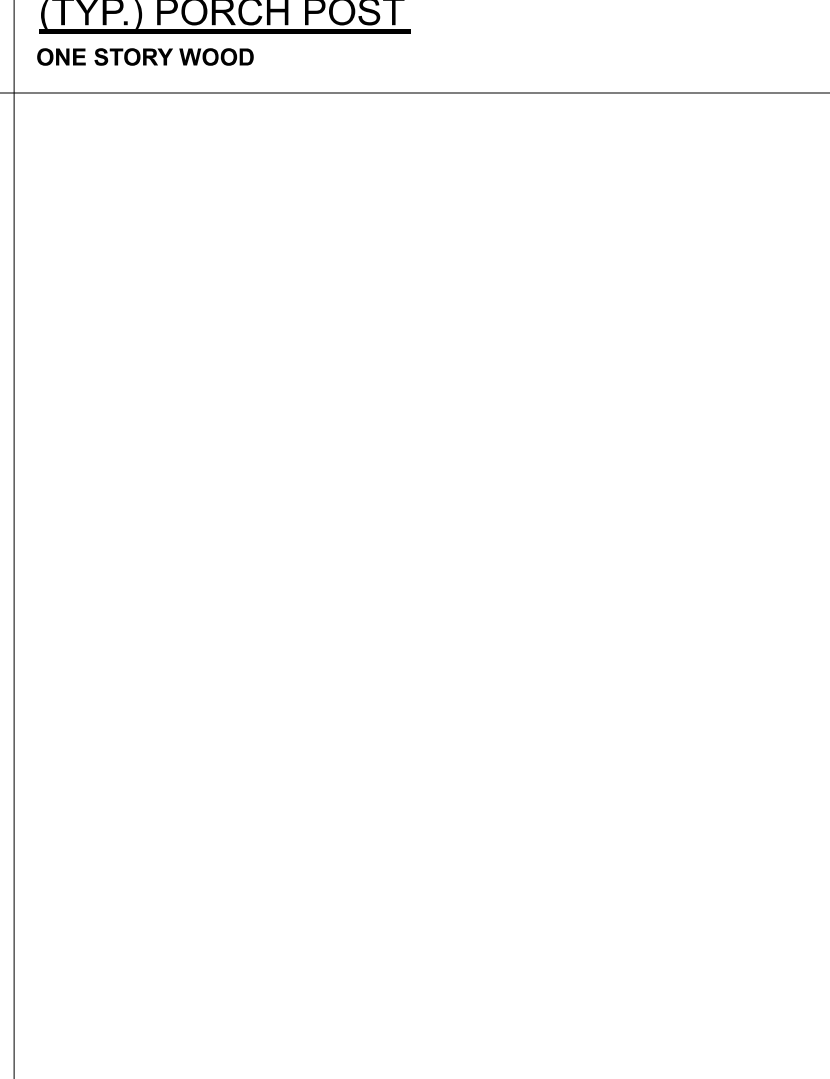
(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME



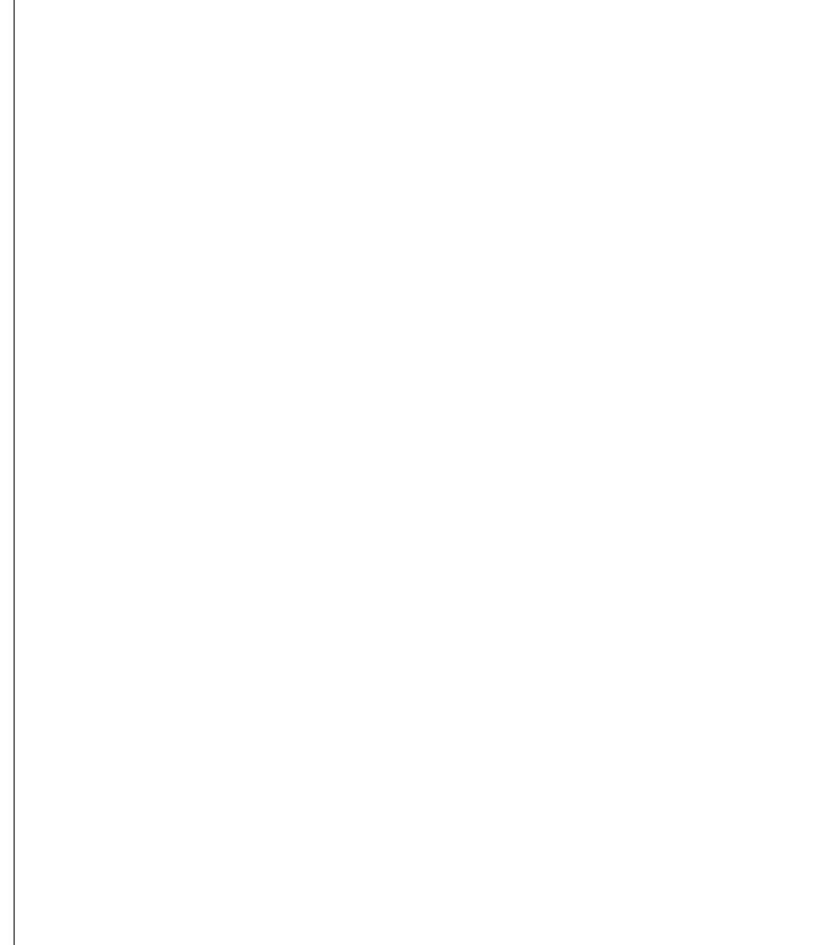
(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) PORCH POST
ONE STORY WOOD



(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME



(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	-	-
415	290	H3	4-8d x 1 1/2"	4-8d x 1 1/2"
575	496	H2.5A	5-8d x 1 1/2"	5-8d x 1 1/2"
1340	1015	H10A	9-10d1 1/2"	9-10d1 1/2"
720	620	LTS12-20	6-10d1 1/2"	6-10d1 1/2"
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	6-10d	6-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stewall	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	16-16d x 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD
4235	3640	HTT4	16-16d x 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stewall	To Post	Anchor
2200	ABU44	12-16d	5/8"x12" Drill & Epoxy	
2300	ABU66	12-16d	5/8"x12" Drill & Epoxy	
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
2200	ABU44	12-16d	5/8"x7" Drill & Epoxy	
2300	ABU66	12-16d	5/8"x7" Drill & Epoxy	

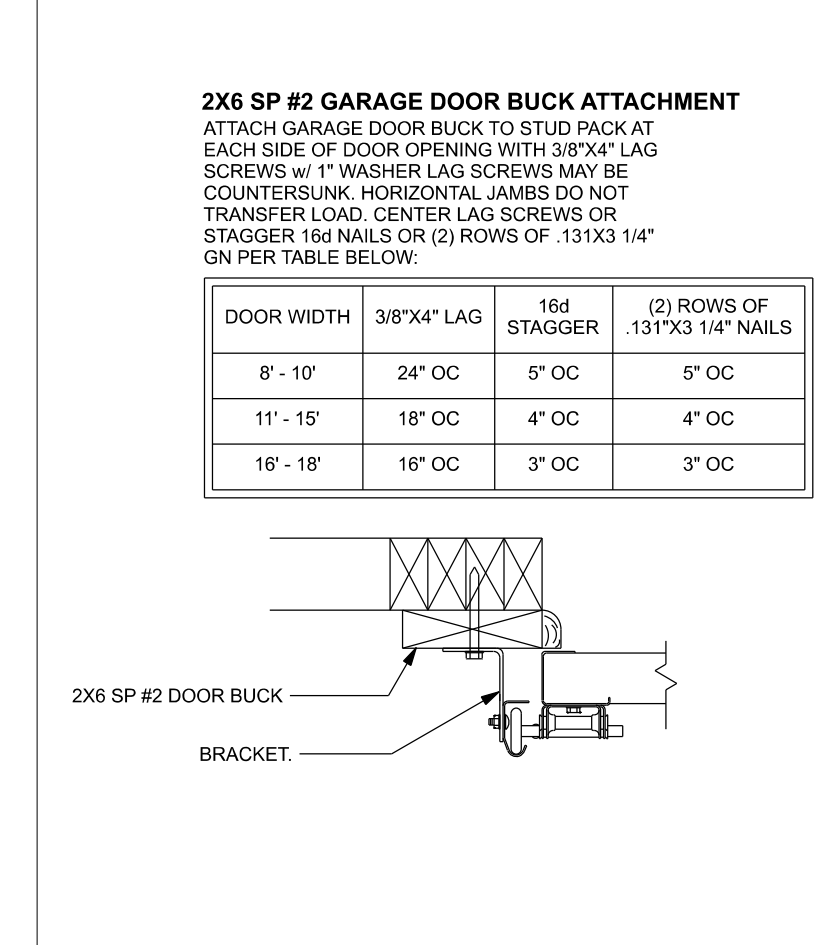
(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)			
Uplift	Top Connection	Bottom Connection	Anchor Bolt Spacing
227 pif	SP2 @ 32" OC	SP1 @ 32" OC	48" OC
454 pif	SP2 @ 16" OC	SP1 @ 16" OC	32" OC
309 pif	LSTA24, 14-10d @ 48" OC	LSTA24, 14-10d @ 48" OC	48" OC
465 pif	LSTA24, 14-10d @ 32" OC	LSTA24, 14-10d @ 32" OC	32" OC

(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER HAS SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO RATED LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1600 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOIL TEST PROVIDES OTHERWISE). CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, F_y = 80ksi, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C-1116. SUPPLIER TO PROVIDE ASTM C-1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR APPROVALS. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, F_y = 40 KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAIL AND PLACED IN ACCORDANCE WITH ACI 315-86, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS, MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GROUDED CMU.

BUILDER'S RESPONSIBILITY: THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN: THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR. IT IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER. ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THESE TRUSS SHEETS.

(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

BUILDING CODE	7TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)
CODE FOR DESIGN LOADS	ASCE 7-16
WINDLOADS	130 MPH
BASIC WIND SPEED (BASED 7-16, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	40 PSF LIVE LOAD
ROOMS OTHER THAN SLEEPING ROOM	30 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	20 PSF LIVE LOAD
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

EFFECTIVE WIND AREA (FT ²)	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	+25.6 (Vasd) -27.8 (Vasd)	+25.6 (Vasd) -34.2 (Vasd)
0 - 20	+42.6 (Vult) -46.2 (Vult)	+42.6 (Vult) -57 (Vult)

(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME

(TYP.) GARAGE DOOR BUCK INSTALLATION
WOOD FRAME



AMIRI BUILDERS
WEISH RES.

PROJECT ADDRESS:
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MARK DISOWAY P.E. 53915
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

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Thursday, February 3, 2022

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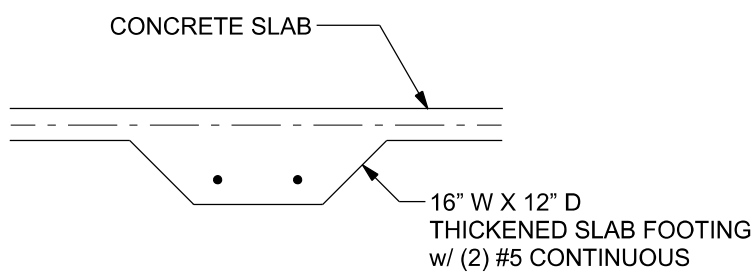
JOB NUMBER:
220120
S-1
OF 3 SHEETS



TALL STEM WALL TABLE:

The table assumes 40 #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the exterior side of the CMU wall (away from the soil pressure within 2" of the exterior surface of the wall). If the wall is over 8' high, add a Durowall ladder reinforcement at 16"0C vertically or a horizontal bond beam with #5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BENTWALL HEIGHT	VERTICAL REINFORCEMENT FOR 8" 0C DCMU (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" 0C STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

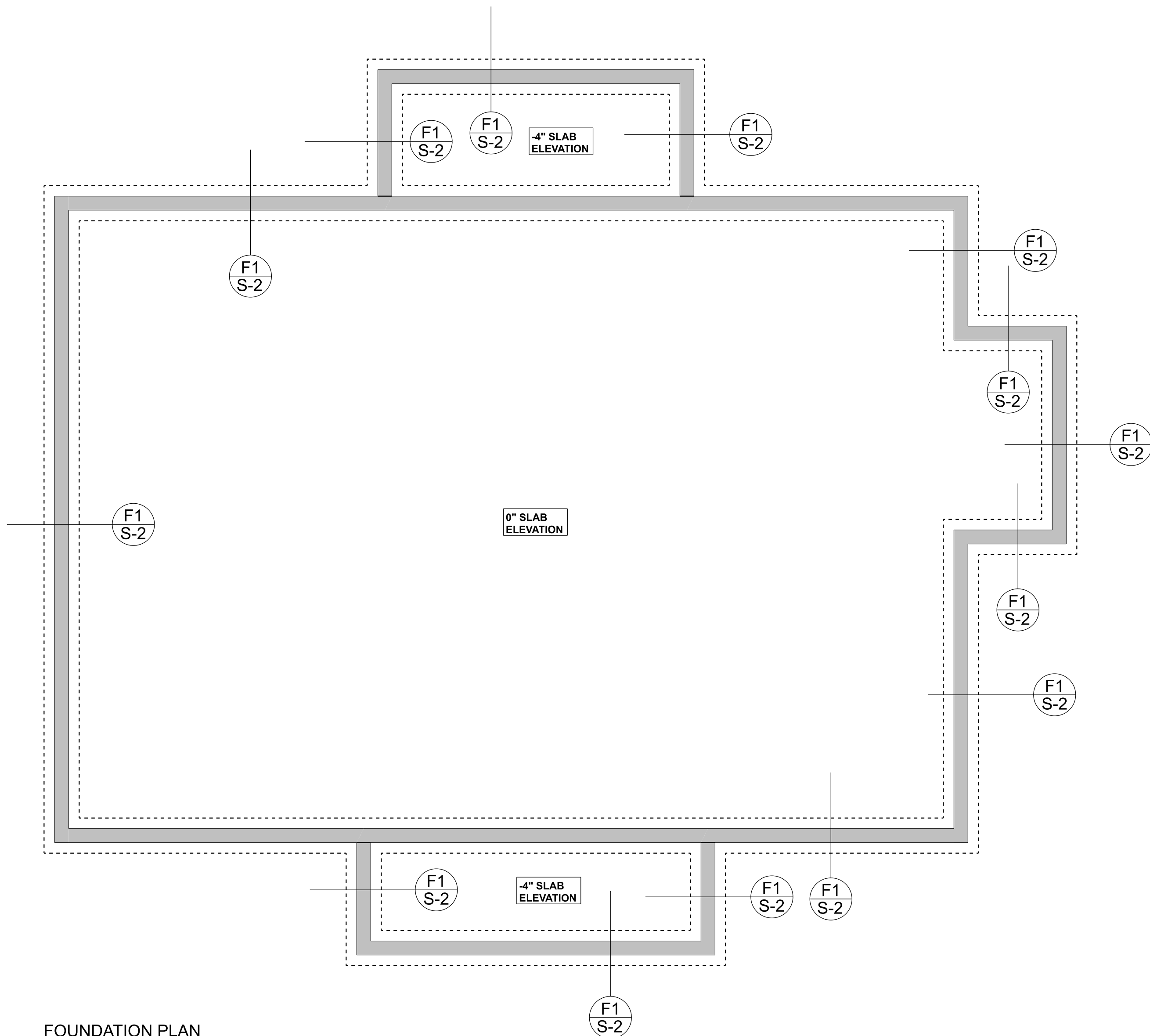


F3 INTERIOR B
S-2 SCALE: 1/2" = 1'-0"

MASONRY NOTE:	
CONCRETE CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 308-1/NASCC 6MTMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 308-1/402 AND THESE NOTES. THERE WILL BE NO EXCEPTIONS TO ACI 308-1/402 MUST BE APPROVED BY THE ENGINEER IN WRITING.	
ACI308-1/02 Section	Specific Requirements
1.4A Compressive strength	F _m = 2800 psi; masonry units F _m = 1500 psi
2 Mortar	ASTM 720 Type N UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, minimum surface finish, maximum thermal expansion joint and flange size of 12"x12" or 16"x16" hollow core block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, S-2x4 75x75x111"
2.4 Reinforcing bars, #3 - #11	ASTM A615, Grade 60, Fy = 40 x ksi, Lap splice min 40 bar dia., 22d for bars
2.4F Coating for corrosion protection	ASTM G65, steel metal lies completely embedded in mortar or grout, ASTM A205, Class G60, 5/8 inch thick, 16045B
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or water and/or air, steel metal lies completely embedded in mortar or grout, ASTM A193, Class B2, 1/2 inch o/d or 3/16ES
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES	
FN - 1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISMISSORY DESIGN GROUP OR MOUNTAIN DISMISSORY PER IS RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN - 2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SLAB) BEFORE FINALIZING FOUNDATION PLAN.
FN - 3	THE SLAB SHALL BE 12" CONCRETE SLAB REINFORCED W/ 6" POLY W/ 2# WELDED WIRE @ 12" ON CENTER @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY PAPER BARRIER W/ 6" LAPs SEALED W/ POLYURETHANE. ALL CHAIRS TO BE PLACED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED THERMITE-TREATMENT METHOD CAN BE USED INSTEAD)



Amira Builders

Welsh Res.

PROJECT ADDRESS:
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DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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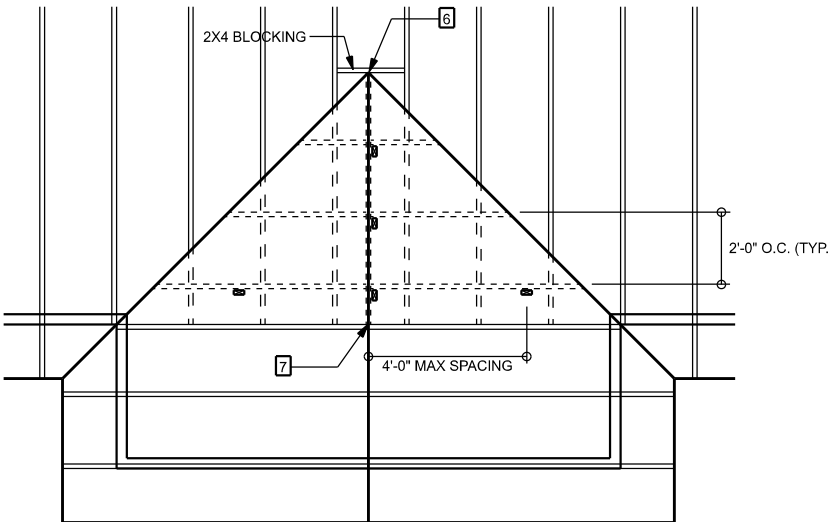
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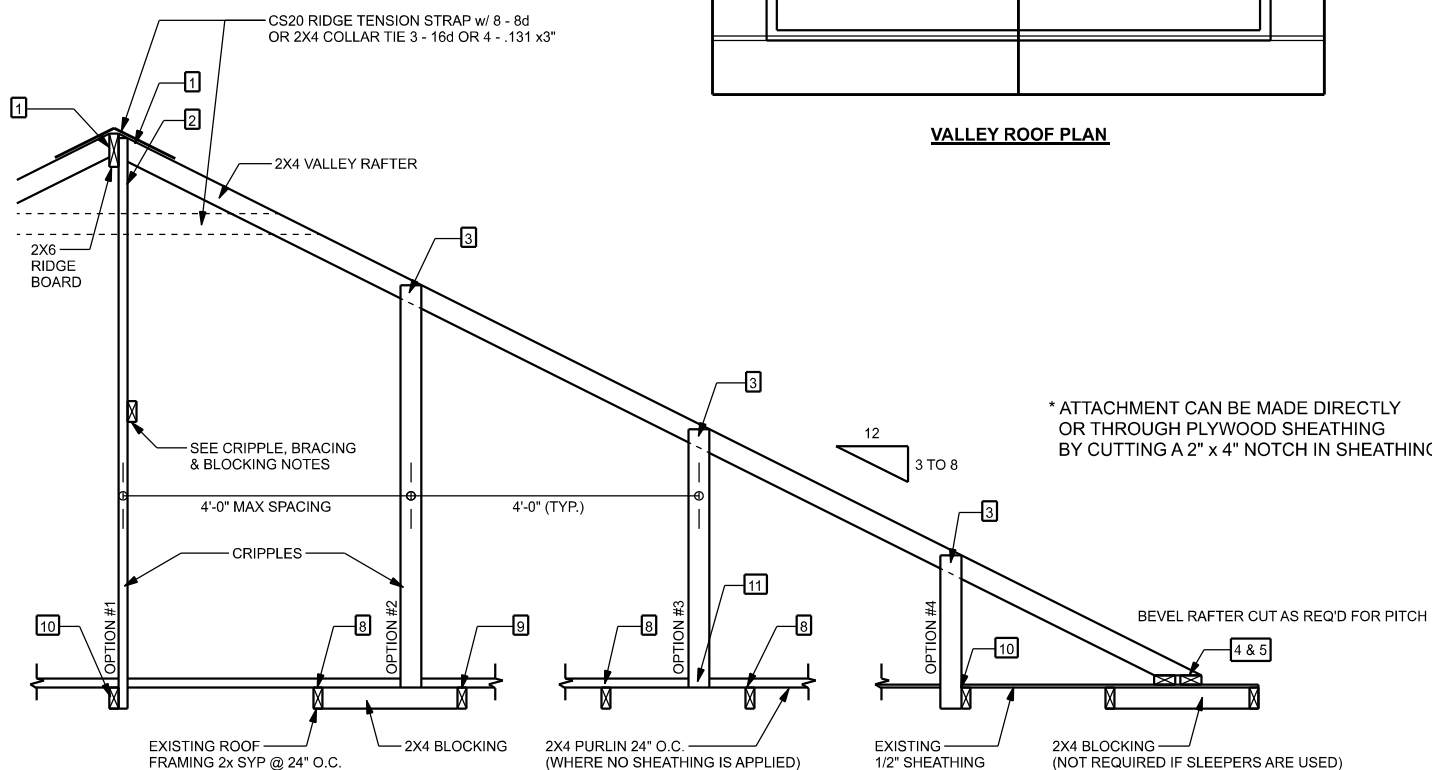
S-2
OF 3 SHEETS:

LUMBER SIZE & GRADE MINIMUM REQUIREMENTS

RISE BOARD	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
FURLIN/LATERAL BRACING	2X4 SYP #2
SLEEPERS	2X (WIDTH OF RAFTER SEAT CUT) SPF #3 OR 2 PARALLEL 2X4 SPF #3
CRIPPLES & BLOCKING	2X4 SPF #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



VALLEY ROOF PLAN



SECTION CUT PARALLEL TO VALLEY RAFTER

ROOF OVER FRAMING & BRACING DETAIL

SCALE: 1/4\"/>

VALLEY ROOF PLAN MEMBER LEGEND

— TRUSS
 — TRUSS UNDER VALLEY FRAMING
 — VALLEY RAFTER OR RIDGE
 — CRIPPLE

CRIPPLES 4'-0\"/>

CONNECTION REQUIREMENT NOTES

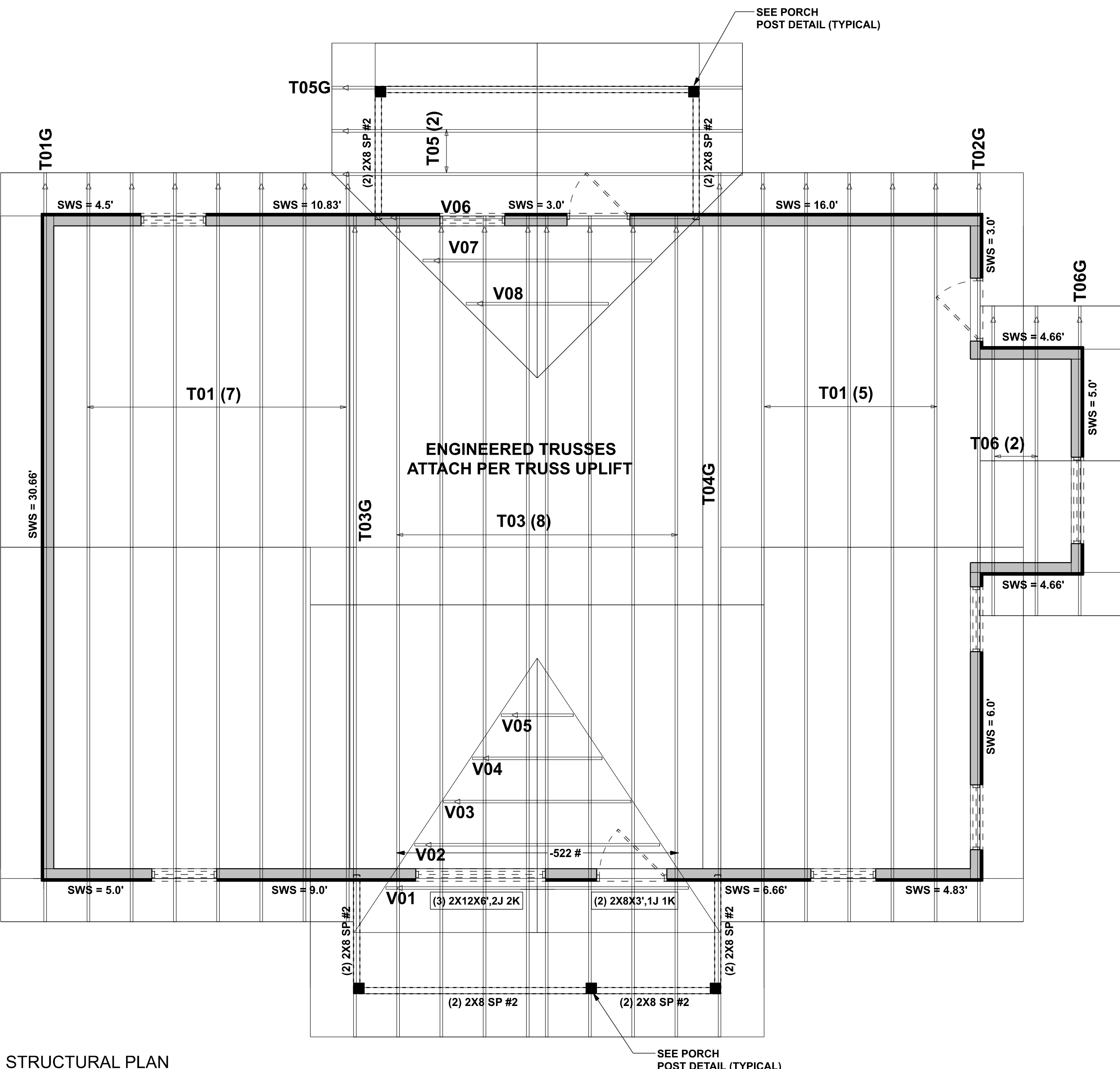
1. 2X4 RAFTERS TO RIDGE	3-16d OR 6-131 x 3\"/>
2. CRIPPLE TO RIDGE	3-16d OR 6-131 x 3\"/>
3. CRIPPLE TO RAFTERS	3-16d OR 6-131 x 3\"/>
4. RAFTER TO SLEEPER OR BLOCKING	6-16d OR 12-131 x 3\"/>
5. SLEEPER TO TRUSS	4-16d OR 6-131 x 3\"/>
6. RIDGE BOARD TO ROOF BLOCK	3-16d OR 6-131 x 3\"/>
7. RIDGE BOARD TO TRUSS	3-16d OR 6-131 x 3\"/>
8. FURLIN TO TRUSS (TYP)	3-16d OR 6-131 x 3\"/>
9. FURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO FURLIN)	4-16d OR 6-131 x 3\"/>
10. TRUSS TO BLOCKING	3-16d OR 6-131 x 3\"/>
11. CRIPPLE TO TRUSS	3-16d OR 6-131 x 3\"/>
12. CRIPPLE TO FURLIN	3-16d OR 6-131 x 3\"/>

GENERAL NOTES

MAXIMUM RAFTER SPAN: 8'-0\"/>

CRIPPLE BRACING & BLOCKING NOTES

2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 9'-0\"/>



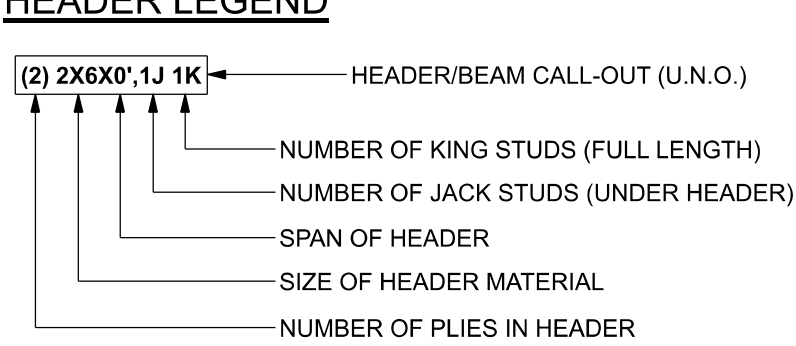
STRUCTURAL PLAN

SCALE: 1/4\"/>

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL WRAP UNDER BOTTOM PLATE & OVER TOP PLATE 1/2\"/>

HEADER LEGEND



ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	15631 LBF	16593 LBF
REQUIRED	11506 LBF	7785 LBF

Amira Builders

Weist Res.

PROJECT ADDRESS:
 163 SW Midtown Place
 Suite 103
 Lake City, FL 32025

DIMENSIONS:
 Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

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Thursday, February 3, 2022

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JOB NUMBER:
 220120

S-3
 OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2530630