

This Permit Expires One Year From the Date of Issue

APPLICANTGARY CLEMMONS

PHONE386.418.2114

ADDRESS15544NW 25TH TERRACE

GAINESVILLEFL32609

OWNERDWAYNE & TERRI STIDHAM

PHONE352.494.2813

ADDRESS193SW GOPHER COURT

FT. WHITEFL32038

CONTRACTORGARY CLEMMONS

PHONE386.418.2114

LOCATION OF PROPERTY441-S TO C-778,TURN WEST, TO FORTUNE LANE,TURN WEST TO
QUAIL, TURN WEST TO GOPHER TURN SOUTH, 1/4 MILE ON R.

TYPE DEVELOPMENTSFD/UTILITY

ESTIMATED COST OF CONSTRUCTION85000.00

HEATED FLOOR AREA1700.00

TOTAL AREA3300.00

HEIGHT21.10

STORIES1

FOUNDATIONCONC

WALLSFRAMED

ROOF PITCH6'12

FLOORCONC

LAND USE & ZONINGA-3

MAX. HEIGHT35

Minimum Set Back Requirments:

STREET-FRONT30.00

REAR25.00

SIDE25.00

NO. EX.D.U.1

FLOOD ZONEXPS

DEVELOPMENT PERMIT NO.

PARCEL ID18-7S-17-10013-030

SUBDIVISIONCOLUMBIA ACRES

LOT30

BLOCK

PHASE

UNIT

TOTAL ACRES5.01

CGC052925

Culvert Permit No.

Culvert Waiver

Contractor's License Number

Applicant/Owner/Contractor

EXISTING07-0507-E

BLK

JTH

N

Driveway Connection

Septic Tank Number

LU & Zoning checked by

Approved for Issuance

New Resident

COMMENTS: NOC ON FILE. 1 FOOT ABOVE ROAD. M/H WILL BE REMOVED ONCE SFD IS COMPLETE WITHIN 45 DAYS.

Check # or Cash5665

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power

Foundation

Monolithic

date/app. by

date/app. by

date/app. by

Under slab rough-in plumbing

Slab

Sheathing/Nailing

date/app. by

date/app. by

date/app. by

Framing

Rough-in plumbing above slab and below wood floor

date/app. by

date/app. by

Electrical rough-in

Heat & Air Duct

Peri. beam (Lintel)

date/app. by

date/app. by

date/app. by

Permanent power

C.O. Final

Culvert

date/app. by

date/app. by

date/app. by

M/H tie downs, blocking, electricity and plumbing

Pool

date/app. by

Reconnection

Pump pole

Utility Pole

date/app. by

date/app. by

date/app. by

M/H Pole

Travel Trailer

Re-roof

date/app. by

date/app. by

date/app. by

BUILDING PERMIT FEE \$425.00

CERTIFICATION FEE \$16.50

SURCHARGE FEE \$16.50

MISC. FEES \$0.00

ZONING CERT. FEE \$50.00

FIRE FEE \$0.00

WASTE FEE \$

FLOOD DEVELOPMENT FEE \$

FLOOD ZONE FEE \$25.00

CULVERT FEE \$

TOTAL FEE533.00

INSPECTORS OFFICE

CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0707-09 Date Received 7/3 By JW Permit # 26011
 Application Approved by - Zoning Official BLK Date 10-07-07 Plans Examiner DKJH Date 7-6-07
 Flood Zone XPS Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3
 Comments * N/A will be moved - once SD is complete with in 45 days

Applicants Name Gary Clemmons Phone (386) 418-2114
 Address 15544 NW 25th Terr. Gainesville, FL 32609
 Owners Name Dwayne + Terri Stidham Phone (352) 494-2813
 911 Address 193 SW Gopher Court, Ft. White, FL 32088
 Contractors Name GC Construction of Gainesville, Inc. Phone (386) 418-2114
 Address 15544 NW 25th Terr. Gainesville, FL 32609
 Fee Simple Owner Name & Address N/A
 Bonding Co. Name & Address N/A
 Architect/Engineer Name & Address Steve Ryall Architectural Drafting 11094 NW 212th Terr.
 Mortgage Lenders Name & Address SunTrust Mortgage Inc 350 N Lake Destiny Rd High Springs, FL 32644
 Circle the correct power company - FL Power & Light - Clay Elec - Suwannee Valley Elec. - Progressive Energy
 Property ID Number TP# 18-75-17-10013-030 Estimated Cost of Construction \$175,000.00
 Subdivision Name Columbia Acres Lot 30 Block Unit Phase
 Driving Directions South on 441 to CR 778, West on 778, South on Fortune Lane, West on Augil Street, South on Gopher Court AND 7.5 1/4 MILE ON Right -
 Type of Construction Single Family Residence Number of Existing Dwellings on Property 1
 Total Acreage 5.01 Lot Size N/A Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive
 Actual Distance of Structure from Property Lines - Front 78.5' Side 102.5' Side 442.5' Rear 198.5'
 Total Building Height 21'-10" Number of Stories 1 Heated Floor Area 1700 Roof Pitch 6/12
 TOTAL 3,300

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT

Gary Clemmons
 Owner Builder or Agent (Including Contractor)

STATE OF FLORIDA
 COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me

this 3rd day of July 20 07

Personally known or Produced Identification DL



Gary Clemmons
 Contractor Signature
 GALEY, J. L. License Number CFC052925
 Card Number

Therrell
 Notary Signature

- CK# - ~~1565~~ 5665

JW called Gary 7.11.07



STATE OF FLORIDA
DEPARTMENT OF HEALTH

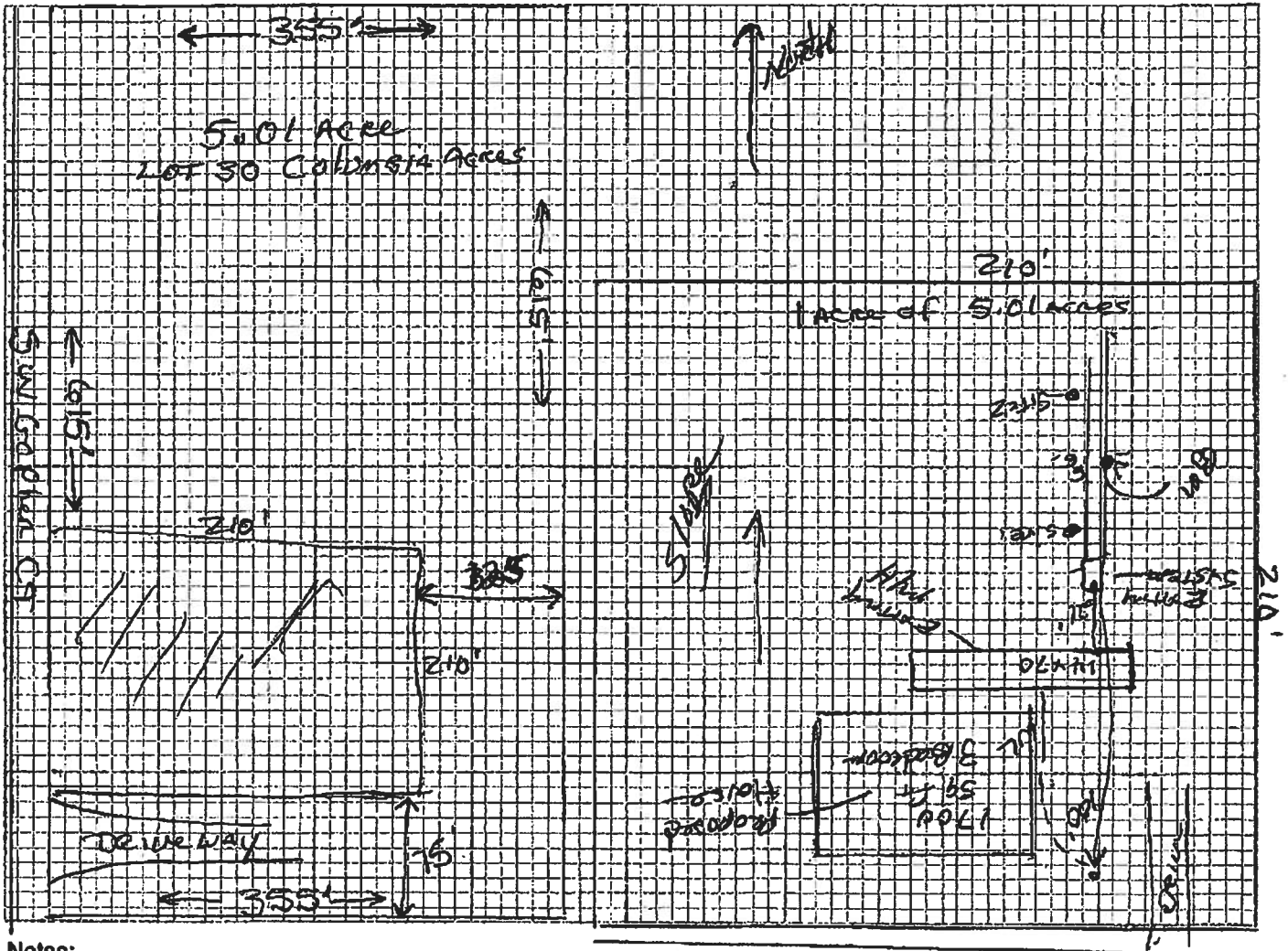
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 02-0507E

STIDHAM 18-75-17-10013-030

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes:

Site Plan submitted by:

Robert W. Ford

Signature

Agas

Title

Plan Approved ☒

Not Approved ☐

Date 6/25/07

By

M O H

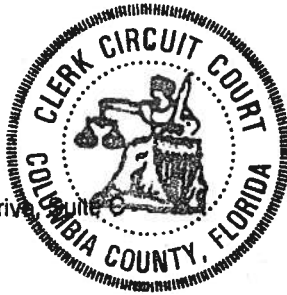
Columbia

County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

15.20
0.00 Copy
1.50 CLEET

Prepared by and Return to:
Mary T. Dotson
Alachua Title Services, LLC
P.O. Box 2408 (32616), 16407 N.W. 174th Drive, Suite 6
Alachua, Florida 32615
386-418-8183



STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing
is a true copy of the original filed in this office.
P. DEWITT CASON, CLERK OF COURTS

By *Donna Row* Deputy Clerk
Date 5-23-07

File No.: 06-041

Inst:2007011536 Date:05/23/2007 Time:11:29

P. Dewitt Cason DC, P. Dewitt Cason, Columbia County B:1120 P:52

NOTICE OF COMMENCEMENT

Pursuant of Section 713.13 Florida Statutes, the undersigned gives notice of the following:

Within 30 days from the recording of this Notice, Improvements generally described as **single family residence** will be commenced on the real property in Columbia County, Florida, legally described as:

Lot 30 of COLUMBIA ACRES, according to the Plat thereof as recorded in Plat Book 4, Page(s) 42, of the Public Records of Columbia County, Florida.

Street address, if any, of said real property: **193 SW Gopher Court, Fort White, Florida 32038.** This property is owned by **Ira D. Stidham a/k/a Ira Dwayne Stidham and Terry R. Stidham**, whose address is: **193 SW Gopher Court, Fort White, Florida 32038.**

The owner's interest in the site of the improvement is fee simple.

Name and address of fee simple title holder if other than the above owner:
Same

The name and address of the contractor(s) is (are):
**GC Clemmons
P.O. Box 5893
Gainesville, FL 32627**

The name and address of the lender is:
**SunTrust Mortgage, Inc.
350 N. Lake Destiny Road
3rd Floor-Suite 300
Maitland, FL 32751**

The name and address of the surety on the payment bond (if any) is: **n/a** The amount of said bond is **\$n/a**. **n/a** is designated as the person upon whom all 11 notices and other documents relating to Florida Mechanic's Lien Law shall be served. (Services upon such person shall constitute service upon the owner). Copy of Notice to Owner shall also be served on **May 17, 2007**.

Signed, sealed and delivered in the presence of these witnesses:

Mary T. Dotson
- Witness
Orlando W. Fernandez
- Witness

Ira D. Stidham
Ira D. Stidham a/k/a Ira Dwayne Stidham - Borrower
Terry R. Stidham
Terry R. Stidham - Borrower

State of Florida

County of Alachua

SWORN TO, SUBSCRIBED AND ACKNOWLEDGED before me on 17th day of May 2007 by Ira D. Stidham a/k/a Ira Dwayne Stidham and Terry R. Stidham, husband and wife, who is/are personally known to me or who has/have produced a valid driver's license as identification.

Mary T. Dotson
NOTARY PUBLIC

Printed Name of Notary
My Commission Expires: _____



Mary T. Dotson
My Commission DD255162
Expires December 18, 2007

Inst:2007011536 Date:05/23/2007 Time:11:29
DC,P.Dewitt Cason,Columbia County B:1120 P:53

09009 Application #

Sfidam owner

AC# 2662331

STATE OF FLORIDA

DEPARTMENT OF BUSINESS AND PROFESSIONAL REGULATION
CONSTRUCTION INDUSTRY LICENSING BOARD

SEC# 10601210085

DATE

BATCH NUMBER

LICENSE NBR

07/17/2006 066009886 GGC052925

The GENERAL CONTRACTOR
Named Below IS CERTIFIED
Under the provisions of Chapter 489, FS
Expiration date: Aug 31, 2008

CLEMONS, GARY ROBERT
G C CONSTRUCTION OF GAINESVILLE, INC
15544 NW 25TH TERR
P.O. Box 5883
GAINESVILLE
FL 32609

JIM BISH
GOVERNOR

DISPLAY AS REQUIRED BY LAW

SIMONE MARSHALLER
SECRETARY

GC Construction of Gainesville, Inc.
15544 NW 25th Terrace
Gainesville, FL 32609
386-418-2114 Phone/386-418-0527 Fax
#CGC052925

Dewayne and Terry Stidham
193 SW Gopher Ct
FT. White, FL 32038

Tax Parcel # 18-7s-17-10013-030

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Project Name: GCCO - Stidham Res	Builder: _____
Address: _____	Permitting Office: <i>Columbia</i>
City, State: _____	Permit Number: <i>26011</i>
Owner: _____	Jurisdiction Number: <i>221000</i>
Climate Zone: North	

1. New construction or existing New _____	12. Cooling systems
2. Single family or multi-family Single family _____	a. Central Unit Cap: 36.0 kBtu/hr
3. Number of units, if multi-family 1 _____	SEER: 13.00
4. Number of Bedrooms 3 _____	b. N/A _____
5. Is this a worst case? Yes _____	c. N/A _____
6. Conditioned floor area (ft²) 1700 ft² _____	13. Heating systems
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)	a. Electric Heat Pump Cap: 36.0 kBtu/hr
a. U-factor: _____ Description Area _____	HSPF: 8.60
(or Single or Double DEFAULT) 7a. (Dble Default) 257.0 ft² _____	b. N/A _____
b. SHGC: _____	c. N/A _____
(or Clear or Tint DEFAULT) 7b. (Clear) 257.0 ft² _____	14. Hot water systems
8. Floor types	a. Electric Resistance Cap: 40.0 gallons
a. Slab-On-Grade Edge Insulation R=0.0, 200.0(p) ft _____	EF: 0.89
b. N/A _____	b. N/A _____
c. N/A _____	c. Conservation credits
9. Wall types	(HR-Heat recovery, Solar
a. Frame, Wood, Exterior R=13.0, 1800.0 ft² _____	DHP-Dedicated heat pump)
b. N/A _____	15. HVAC credits PT, _____
c. N/A _____	(CF-Ceiling fan, CV-Cross ventilation,
d. N/A _____	HF-Whole house fan,
e. N/A _____	PT-Programmable Thermostat,
10. Ceiling types	MZ-C-Multizone cooling,
a. Under Attic R=30.0, 1700.0 ft² _____	MZ-H-Multizone heating)
b. N/A _____	
c. N/A _____	
11. Ducts	
a. Sup: Con. Ret: Con. AH: Interior Sup. R=6.0, 160.0 ft _____	
b. N/A _____	

Glass/Floor Area: 0.15

Total as-built points: 21461

Total base points: 24410

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: *[Signature]*
DATE: *1/10/07*

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____
DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____
DATE: _____



¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X SPM X SOF = Points				
.18	1700.0	18.59	5689.0	1.Double, Clear	N	0.0	0.0	62.0	19.20	1.00	1190.0
				2.Double, Clear	E	0.0	0.0	108.0	42.06	1.00	4542.0
				3.Double, Clear	W	0.0	0.0	57.0	38.52	1.00	2195.0
				4.Double, Clear	S	0.0	0.0	30.0	35.87	1.00	1075.0
				As-Built Total:				257.0	9002.0		
WALL TYPES											
Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Adjacent	0.0	0.00	0.0	1. Frame, Wood, Exterior	13.0		1800.0	1.50		2700.0	
Exterior	1800.0	1.70	3060.0								
Base Total:				1800.0		3060.0		As-Built Total:			
								1800.0		2700.0	
DOOR TYPES											
Area X BSPM = Points				Type	Area X SPM = Points						
Adjacent	0.0	0.00	0.0								
Exterior	0.0	0.00	0.0								
Base Total:				0.0		0.0		As-Built Total:			
								0.0		0.0	
CEILING TYPES											
Area X BSPM = Points				Type	R-Value		Area X SPM X SCM = Points				
Under Attic	1700.0	1.73	2941.0	1. Under Attic	30.0		1700.0	1.73 X 1.00		2941.0	
Base Total:				1700.0		2941.0		As-Built Total:			
								1700.0		2941.0	
FLOOR TYPES											
Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Slab	200.0(p)	-37.0	-7400.0	1. Slab-On-Grade Edge Insulation	0.0		200.0(p)	-41.20		-8240.0	
Raised	0.0	0.00	0.0								
Base Total:				-7400.0		200.0		As-Built Total:			
								200.0		-8240.0	
INFILTRATION											
Area X BSPM = Points								Area X SPM = Points			
1700.0 10.21 17357.0								1700.0 10.21 17357.0			

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
Summer Base Points: 21647.0				Summer As-Built Points: 23760.0						
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Cooling Points
				(sys 1: Central Unit 36000btuh ,SEER/EFF(13.0) Ducts:Con(S),Con(R),Int(AH),R6.0(INS)						
				23760	1.00	(1.00 x 1.147 x 0.91)	0.260	0.950	6125.6	
21647.0	0.3250		7035.3	23760.0	1.00	1.044	0.260	0.950	6125.6	

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X WPM X WOF = Points				
.18	1700.0	20.17	6172.0	1.Double, Clear	N	0.0	0.0	62.0	24.58	1.00	1523.0
				2.Double, Clear	E	0.0	0.0	108.0	18.79	1.00	2029.0
				3.Double, Clear	W	0.0	0.0	57.0	20.73	1.00	1181.0
				4.Double, Clear	S	0.0	0.0	30.0	13.30	1.00	398.0
				As-Built Total:				257.0	5131.0		
WALL TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Adjacent	0.0	0.00	0.0	1. Frame, Wood, Exterior	13.0		1800.0	3.40	6120.0		
Exterior	1800.0	3.70	6660.0								
Base Total: 1800.0 6660.0				As-Built Total:		1800.0		6120.0			
DOOR TYPES Area X BWPM = Points				Type	Area X WPM = Points						
Adjacent	0.0	0.00	0.0								
Exterior	0.0	0.00	0.0								
Base Total: 0.0 0.0				As-Built Total:		0.0		0.0			
CEILING TYPES Area X BWPM = Points				Type	R-Value		Area X WPM X WCM = Points				
Under Attic	1700.0	2.05	3485.0	1. Under Attic	30.0		1700.0	2.05 X 1.00	3485.0		
Base Total: 1700.0 3485.0				As-Built Total:		1700.0		3485.0			
FLOOR TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Slab	200.0(p)	8.9	1780.0	1. Slab-On-Grade Edge Insulation	0.0		200.0(p)	18.80	3760.0		
Raised	0.0	0.00	0.0								
Base Total: 1780.0				As-Built Total:		200.0		3760.0			
INFILTRATION Area X BWPM = Points				Area X WPM = Points							
1700.0 -0.59 -1003.0				1700.0 -0.59 -1003.0							

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT						
Winter Base Points: 17094.0				Winter As-Built Points: 17493.0						
Total Winter Points	X System Multiplier	= Heating Points		Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points	
17094.0	0.5540	9470.1		(sys 1: Electric Heat Pump 36000 btuh , EFF(8.6) Ducts:Con(S),Con(R),Int(AH),R6.0 17493.0 1.000 (1.000 x 1.169 x 0.93) 0.397 0.950 7163.8						
				17493.0	1.00	1.087	0.397	0.950	7163.8	

WATER HEATING & CODE COMPLIANCE STATUS**Residential Whole Building Performance Method A - Details**

ADDRESS: , , ,

PERMIT #:

BASE				AS-BUILT					
WATER HEATING									
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank X Ratio	Multiplier X Credit	= Total Multiplier
3		2635.00	7905.0	40.0	0.89	3	1.00	2723.82	1.00 8171.5
				As-Built Total:					8171.5

CODE COMPLIANCE STATUS

BASE					AS-BUILT				
Cooling Points	+	Heating Points	+	Hot Water Points = Total Points	Cooling Points	+	Heating Points	+	Hot Water Points = Total Points
7035		9470		7905 24410	6126		7164		8171 21461

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq. ft. window area; .5 cfm/sq. ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 86.7

The higher the score, the more efficient the home.

1. New construction or existing	New	___	12. Cooling systems	
2. Single family or multi-family	Single family	___	a. Central Unit	Cap: 36.0 kBtu/hr
3. Number of units, if multi-family	1	___		SEER: 13.00
4. Number of Bedrooms	3	___	b. N/A	___
5. Is this a worst case?	Yes	___	c. N/A	___
6. Conditioned floor area (ft ²)	1700 ft ²	___		___
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		___	13. Heating systems	
a. U-factor:	Description Area	___	a. Electric Heat Pump	Cap: 36.0 kBtu/hr
(or Single or Double DEFAULT)	7a. (Dble Default) 257.0 ft ²	___		HSPF: 8.60
b. SHGC:		___	b. N/A	___
(or Clear or Tint DEFAULT)	7b. (Clear) 257.0 ft ²	___	c. N/A	___
8. Floor types		___	14. Hot water systems	
a. Slab-On-Grade Edge Insulation	R=0.0, 200.0(p) ft	___	a. Electric Resistance	Cap: 40.0 gallons
b. N/A	___	___		EF: 0.89
c. N/A	___	___	b. N/A	___
9. Wall types		___	c. Conservation credits	___
a. Frame, Wood, Exterior	R=13.0, 1800.0 ft ²	___	(HR-Heat recovery, Solar	___
b. N/A	___	___	DHP-Dedicated heat pump)	___
c. N/A	___	___	15. HVAC credits	PT, ___
d. N/A	___	___	(CF-Ceiling fan, CV-Cross ventilation,	___
e. N/A	___	___	HF-Whole house fan,	___
10. Ceiling types		___	PT-Programmable Thermostat,	___
a. Under Attic	R=30.0, 1700.0 ft ²	___	MZ-C-Multizone cooling,	___
b. N/A	___	___	MZ-H-Multizone heating)	___
c. N/A	___	___		___
11. Ducts		___		___
a. Sup: Con. Ret: Con. AH: Interior	Sup. R=6.0, 160.0 ft	___		___
b. N/A	___	___		___

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____



**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.*

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLRCSB v0.)

#26011

Oct. 15, 2007

Mr. Randy Jones
Assistant Building Official
Columbia County Building & Zoning Dept.
135 N.E. Hernando Ave, Suite B-21
Lake City, FL 32055

Ref: GCC Job Name: Stidham
Permit #: 000026011
Tax Parcel #: 18-7S-17-10013-030
Job Address: 193 S.W. Gopher Court, Ft. White, FL 32038

Dear Mr. Jones

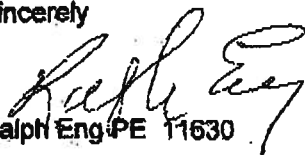
As requested by Mr. Gary Clemmon, I have reviewed the boundary and topographic survey of this site prepared by Brinkman Surveying and Mapping, Inc., dated August 21, 2007. The purpose of my review is to determine if the house, that is under construction, is subject to flooding.

It is my professional opinion that the house under construction is not subject to flooding based on the following:

1. The site is not located in any FEMA Flood Zone.
2. The existing ground elevations adjacent to the house is (based on assume datum on survey) 92.53 at N.E. corner to 96.0 at SW. corner. The lowest finished floor is 97.17 elevation. The house is located about 78 feet east of Gopher Circle Road with road elevations of 99.0 to 100.5.
3. The finished floor of 97.17 is 14 inches above the highest adjacent ground at the S.W. corner of the house. Any stormwater runoff from Gopher Circle roadway is routed around the house.

I trust this professional opinion satisfies your concern.

Sincerely



Ralph Eng PE 11630

SCHAFER ENGINEERING, LLC.

7104 N.W. 42nd Lane \ Gainesville, Florida 32606

May 10, 2007

Job: G. C. Construction

Re: Stidham Residence \ 9 Course Stemwall

Dear Mr. Clemons:

It is to our understanding that due to variations in the terrain, the foundation stemwall will be a height of 9 courses maximum with (1) #5 cont. in the top pour. Install #5 vertical reinforcing steel at 32" o.c. minimum with a standard 90 degree ACI hook in the footing. Install an intermediate bond beam at 48" with (1) #5 continuous. The footing will be a minimum of 24" wide by 12" deep with (3) #5 rebars continuous.

Install #5 vertical reinforcing steel at 48" o.c. minimum with a standard 90 degree hook in the footing from 1 to 6 courses maximum with (1) #5 rebar continuous in the top pour.

There will be a maximum of 6 courses exposed above finish grade. The backfill behind the wall must be compacted with a maximum of 12" loose lifts involving clean compacted fill.

Provide temporary bracing and support at top of wall until slab concrete has developed minimum specified strength. Concrete strength must be 3000 P.S.I. minimum. Masonry must conform to ASTM C-90, grade N-11 with minimum net area compressive strength of 1900 P.S.I.

It is assumed that ideal soil conditions and pad preparation are provided.

If you have any questions or if we can be any further assistance, please feel free to contact us at your convenience.

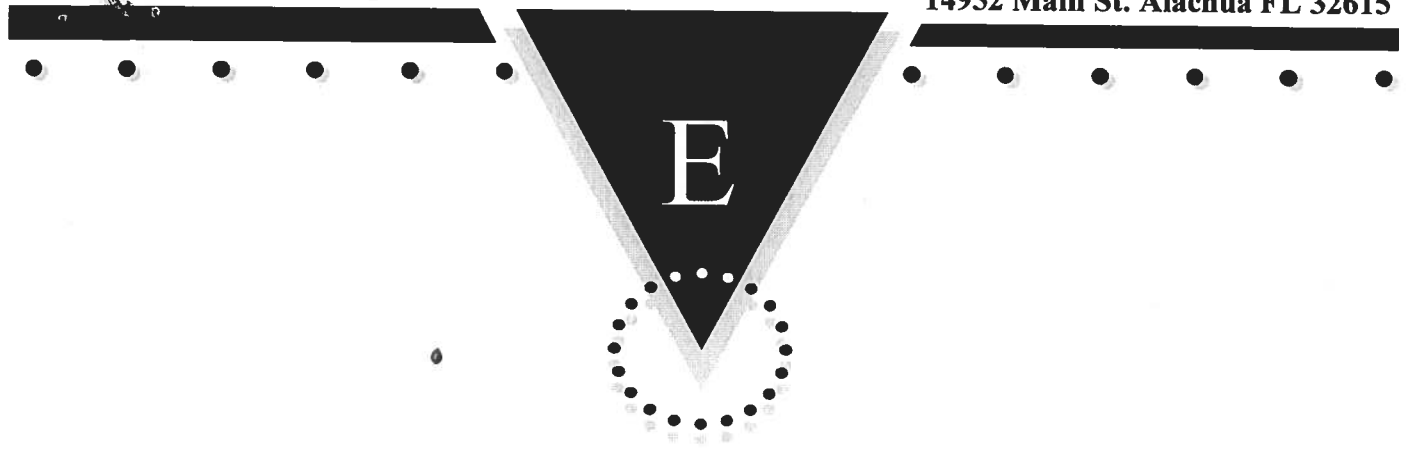


5-11-07

Bruce Shafer, P.E. #48984
7104 N.W. 42nd Lane
Gainesville, Florida 32606

Schafer Engineering LLC

14952 Main St. Alachua FL 32615



E

Prepared for:

**G. C. CONSTRUCTION
THE STIDHAM RESIDENCE**

By:

Schafer Engineering, LLC

386-462-1340 / 352-375-6329

NO COPIES ARE TO BE PERMITTED

SCHAFER ENGINEERING LLC

Trusses: Pre-engineered with manufacturer's required bracing system installed.

Roof sheathing: Type OSB Size 7/16 Fastener type nails 8d/.113 Ring Shank

Interior zone spacing: Interior 6 in. Periphery 4 in.

Edge and end zone spacing: Interior 6 in. Periphery 4 in.

Top double pl: Type Spruce Grade #1 #2 Size 2 x 4 Nail spacing 10 in.

Studs: Type Spruce Grade #1 #2 Size 2 x 4

Interior stud spacing 16 in. Composite (yes or no) Y

End stud spacing 16 in. Composite (yes or no) Y

Shearwall siding: Type OSB Thickness 7/16 in.

47' — Trans: Fastener 8d/.131 Spacing: Int 8 in. Edge 4 in.

46' — Long: Fastener 8d/.131 Spacing: Int 8 in. Edge 4 in.

Allowable unit shear on shearwalls: 314 pounds per linear foot

Unit shear transferred from diaphragm: Trans: 117 Long: 120

Wall tension transferred by: Siding nails 8d/.131 @ 4 O.C. edges

Foundation anchor bolts: Concrete strength 3000 psi Size 1/2 in. Shape L

Washer 2" Embedment 7 in. Location of first anchor bolt from corner 8 in.

Anchor Bolts @ 48" O.C. Model A307 Loc. from corner 8 in.

Type of foundation: 1 #5 rebar continuous required in bond beam.

Floor slab 4 in. CMU: Size 8 x 16 in. Height 24 in. Reinf. #5 at 72 in.

Monolithic footing: Depth 20 in. Bottom width 12 in. Reinf. 2 # 5 bars

Footing: Width 20 in. Depth 12 in. Reinforcing 2 # 5 bars

Interior Footings: 16" W X 10" D

Porch Columns: 4x4x9'5yp #2 PT @ 99" oc max **Column Fasteners:** Simpson CB44/CC44 ARECUL

Special Comments: Install ceiling diaphragm on open porches using same nails,
nail sizes, + same grade material as roof sheathing.

Install Simpson HUS28 on each end of the 2x8 5yp #2 PT floor joists @ 24" oc.
Install Simpson P451 @ 99" oc max. spacing or equal for Rim Joists.

NOTE:

1. Balloon frame ALL gable ends unless this summary is accompanied by Gable End Wall Brace detail.
2. All trusses must bear on exterior walls & porch beams.
3. All walls to be nailed with same nailing pattern as shear walls.
4. This is a wind load only, NOT a structural analysis.
5. This wind load is not valid without a raised, embossed seal.
6. It is assumed that ideal soil conditions and pad preparations are provided.
7. Fiber mesh or WWM may be used in concrete slab.
8. Trusses must be anchored and supported in accordance to the truss engineering.
9. Wind design and analysis valid for one use only, no copies permitted.
10. The foundation is for minimum design use and may be increased.
11. All headers over 12 feet to be pre-engineered.

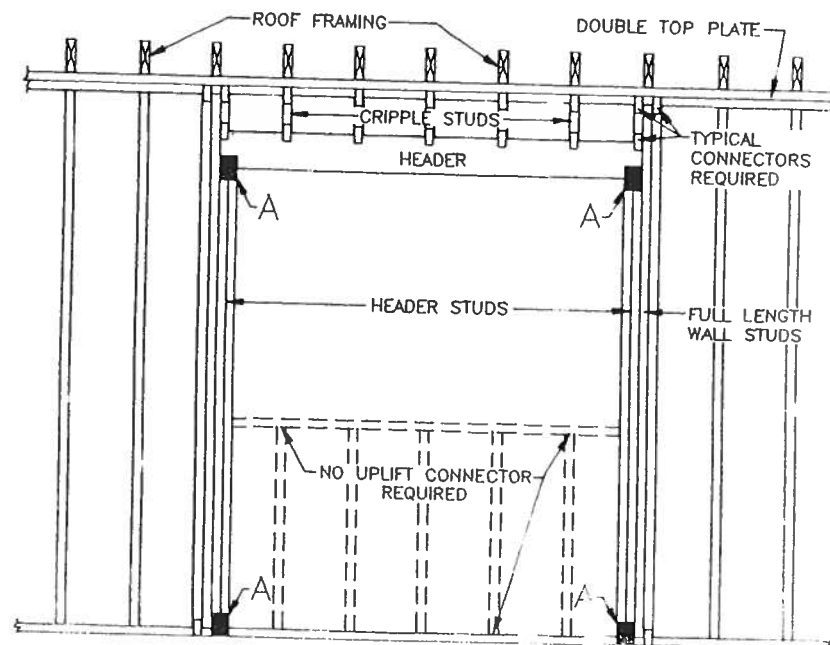
B. Schaf
6-6-07

		Maximum Header Span (ft.)					
		3'	6'	9'	12'	15'	18'
		Number of Header Studs Supporting End of Header					
		1 ¹	1	2	2	2	2
Unsupported Wall Height	Stud Spacing	Number of Full-Length Studs at Each End of Header					
10' or less	12 in.	2	2	3	3	3	3
	16 in.	2	2	3	3	3	3
	24 in.	1	2	2	2	2	2
greater than 10'	12 in.	2	2	3	4	5	5
	16 in.	2	2	3	3	4	4
	24 in.	1	2	2	2	3	3

1. The header stud shall not be required if the header is supported by a suitable framing anchor.

Uplift connection requirement at points A (top and bottom of header studs): Uplift load per framing member above the header from Table 307F1 or 307A, as appropriate, multiplied by the number of framing members displaced divided by two.

NOTE: Uplift connection is required at each end of header and at bottom of header studs in addition to connectors at wall studs and at top and bottom of cripples.



TIE-DOWN TABLES

HEADERS				
Uplift Force Lbs	Top Connector **	Rating Lbs	Bottom Connector **	Rating Lbs
to 455	LSTA9	725	H3	455
to 910	LSTA12	905	2-H3	910
to 1265	LSTA18	1265	LTT19	1350
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2565
to 2865	3-LSTA18	3255	HD2A-3.5	2865
to 3700	3-LSTA24	3880	HD5A-3	3700
Total uplift for each truss resting on the header and divide by 2 to determine the uplift force. Use proper bolt anchors sufficient to support required load.				

TRUSSES/GIRDERS		
Uplift Force Lbs	Top Connector **	Bottom Connector **
to 500	H2.5	N/A
501-1049	H10	N/A
1050-1350	TS22	LTT19
1351-1750	2-TS22	LTT20
1751-2570	2-TS22	HD2A
2571-3665	3-TS22	HD5A
3666-5260	2-MST148	HTT22
5261-8300	2-MST48	HD10A
Two 12d common toenails are required per truss/rafter per bearing point into plate. Use proper bolt anchors. Strap rafters to truss or at each end with minimum uplift resistance of 450# each end. Strap ridge beam at each end with minimum uplift resistance of 1000#. It is the contractors' responsibility to provide a continuous load path from truss/rafter/ridge beam to foundation.		

	Top Connector **	Rating Lbs	Bottom Connector **	Rating
BEAM SEATS	LSTA18*	1200	LTT19*	1250
POSTS (max 17' spacing)	2-LSTA18	2400	ABU44	2300
*or per truss engineering Use proper bolt anchors All beams to be sheathed or strapped to Double Top Plate when applicable.				

CRIPPLES	Sheathing nailing alone adequate w/8d nails @ 3" O.C.
-----------------	---

STUDS
Wall sheathing nailing Adequate exterior walls bottom w/8d nails.
Use SP1 & SP2 @32" O.C. on all interior non-sheathed bearing walls.
Interior anchor bolts to be ½" x 8" A307 or ½" x 7" wedge anchor or equivalent.

** Equivalent Simpson hardware, or other manufacturer, may be substituted for any of the hardware specified on this page as long as it meets the required load capacities/uplift resistance.

NOTE:

1. For nailing into SPF members, multiply table values by .86
2. See truss engineering for anchor tie-down values.

ASCE 7-02

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Wind Load Design per ASCE 7-02

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	26.6	Deg
Type of Roof	Hipped	
Eave Height (Eht)	9.00	ft
Ridge Height (RHt)	24.33	ft
Mean Roof Height (Ht)	17.38	ft
Width Perp. to Wind (B)	50.00	ft
Width Parallel to Wind (L)	66.00	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.35
Flexible Structure	No

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
I =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	$Cc * (33/z)^{0.167}$	0.3048	
Lzm	$I*(zm/33)^{Epsilon}$	309.99	ft
Q	$(1/(1+0.63*((B+Ht)/Lzm)^{0.63}))^{0.5}$	0.8977	
Gust2	$0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))$	0.8646	
Gust Factor Category III: Flexible or Dynamically Sensitive Structures			
Vhref	$V*(5280/3600)$	161.33	ft/s
Vzm	$bm*(zm/33)^{Am}*Vhref$	70.89	ft/s
NF1	$NatFreq*Lzm/Vzm$	4.37	Hz
Rn	$(7.47*NF1)/(1+10.302*NF1)^{1.667}$	0.0552	
Nh	$4.6*NatFreq*Ht/Vzm$	1.13	
Nb	$4.6*NatFreq*B/Vzm$	3.24	
Nd	$15.4*NatFreq*Depth/Vzm$	14.34	
Rh	$1/Nh-(1/(2*Nh^2)*(1-Exp(-2*Nh)))$	0.5348	
Rb	$1/Nb-(1/(2*Nb^2)*(1-Exp(-2*Nb)))$	0.2608	
Rd	$1/Nd-(1/(2*Nd^2)*(1-Exp(-2*Nd)))$	0.0673	
RR	$((1/Beta)*Rn*Rh*Rb*(0.53+0.47*Rd))^{0.5}$	0.6574	
gg	$+(2*LN(3600*n1))^{0.5}+0.577/(2*LN(3600*n1))^{0.5}$	4.19	
Gust3	$0.925*((1+1.7*Izm*(3.4^2*Q^2+GG^2*RR^2)^{0.5})/(1+1.7*3.4*Izm))$	1.05	

Gust Factor Summary			
Main Wind-force resisting system:		Components and Cladding:	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.86	Gust Factor (G)	0.86

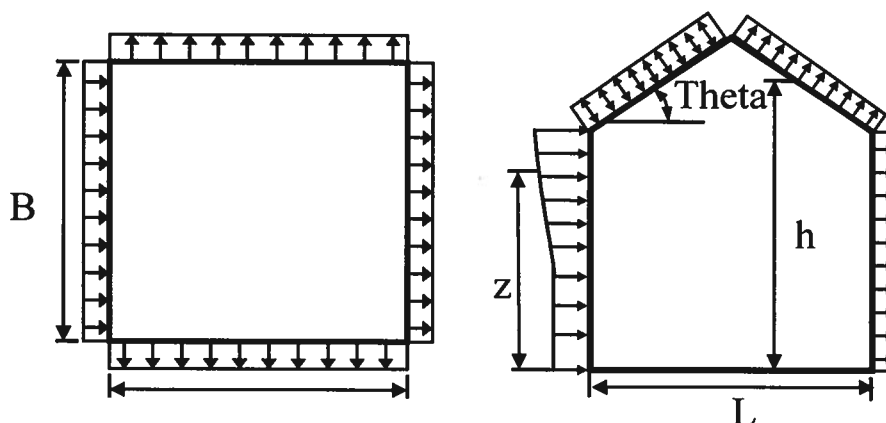
Wind Load Design per ASCE 7-02

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev. ft	Kz	Kzt	Kd	qz lb/ft^2	Pressure (lb/ft^2)	
					Windward Wall*	
					+GCpi	-GCpi
24.33	0.70	1.00	1.00	21.70	11.67	18.35
20	0.70	1.00	1.00	21.70	11.67	18.35
17.38	0.70	1.00	1.00	21.70	11.67	18.35
15	0.70	1.00	1.00	21.70	11.67	18.35

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	$2.01 \cdot (Ht/zg)^{(2/\alpha)}$	0.60	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	$.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot Kh \cdot Kht \cdot Kd$	18.57	psf

Wall Pressure Coefficients, Cp	
Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf)	
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 50 ft wall)	-0.44	-10.34	-3.66
Leeward Walls (Wind Dir Parallel to 66 ft wall)	-0.50	-11.37	-4.69
Side Walls	-0.70	-14.58	-7.90
Roof - Normal to Ridge (Theta >= 10)			
Windward - Max Negative	-0.20	-6.61	0.07
Windward - Max Positive	0.29	1.39	8.07
Leeward Normal to Ridge	-0.60	-12.97	-6.29
Overhang Top	-0.20	-3.27	-3.27
Overhang Bottom	0.80	0.69	0.69
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 8.69 ft	-0.90	-17.79	-11.11

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Wind Load Design per ASCE 7-02

Dist from Windward Edge: 8.69 ft to 17.38 ft	-0.90	-17.79	-11.11
Dist from Windward Edge: 17.38 ft to 34.76 ft	-0.50	-11.37	-4.69
Dist from Windward Edge: > 34.76 ft	-0.30	-8.16	-1.47

* Horizontal distance from windward edge

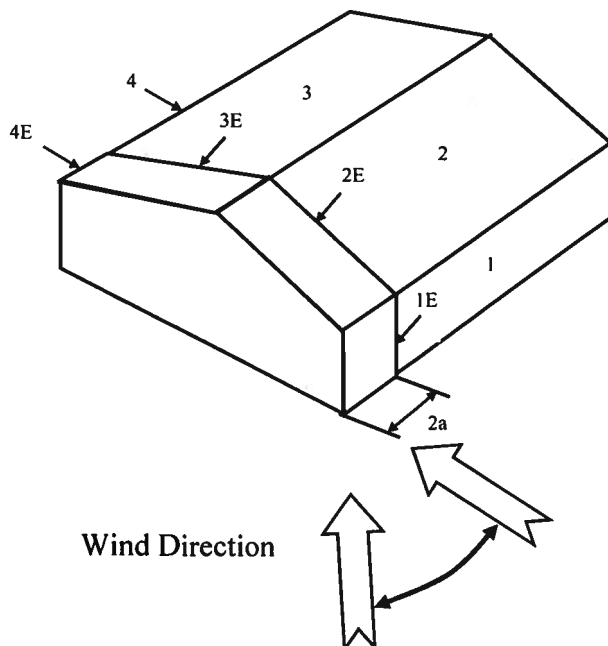
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht ≤ 60 ft

Kh =	$2.01 \cdot (H_t/z_g)^{2/\alpha}$	=	0.60
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	$0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d$	=	18.57

Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.55	0.18	-0.18	21.70	8.03	15.84
2	-0.10	0.18	-0.18	21.70	-5.99	1.82
3	-0.45	0.18	-0.18	21.70	-13.61	-5.79
4	-0.39	0.18	-0.18	21.70	-12.38	-4.57
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.73	0.18	-0.18	21.70	11.88	19.69
2E	-0.19	0.18	-0.18	21.70	-7.93	-0.12
3E	-0.58	0.18	-0.18	21.70	-16.59	-8.78
4E	-0.53	0.18	-0.18	21.70	-15.50	-7.69
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

* $p = q_h \cdot (GC_{pf} - GC_{pi})$



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Wind Load Design per ASCE 7-02

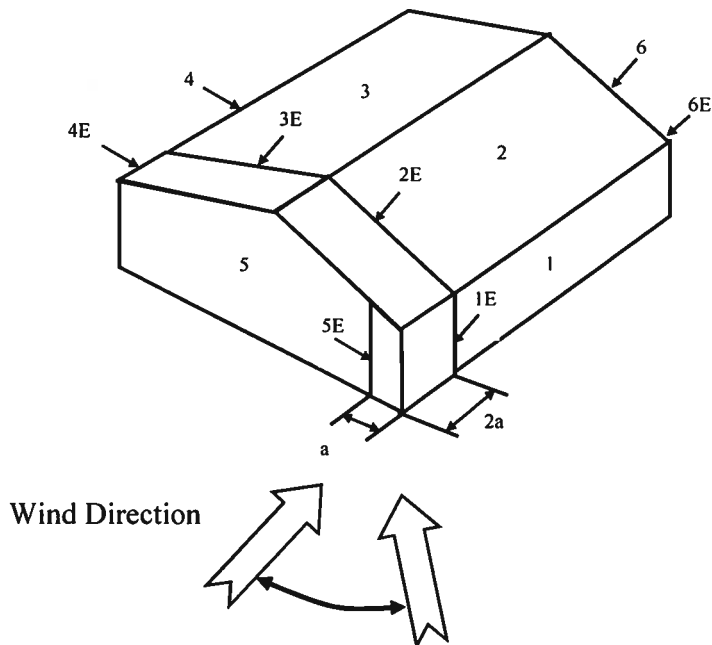
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht ≤ 60 ft

$$\begin{aligned} K_h &= 2.01 \cdot (H_t/z_g)^{2/\alpha} &= & 0.60 \\ K_{ht} &= \text{Topographic factor (Fig 6-2)} &= & 1.00 \\ Q_h &= 0.00256 \cdot (V)^2 \cdot \text{ImpFac} \cdot K_h \cdot K_{ht} \cdot K_d &= & 18.57 \end{aligned}$$

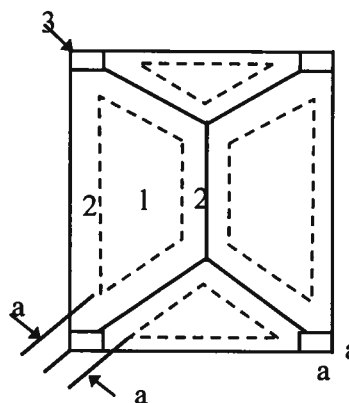
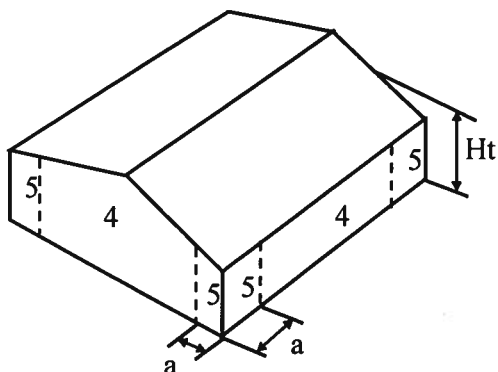
Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86
5	0.40	0.18	-0.18	21.70	4.77	12.59
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.61	0.18	-0.18	21.70	9.33	17.14
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43

$$* p = q_h * (GC_{pf} - GC_{pi})$$



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Figure 6-5 - External Pressure Coefficients, G_{Cp}
Loads on Components and Cladding for Buildings w/ $H_t \leq 60$ ft



Hipped Roof
 $10 < \text{Theta} \leq 30$

$$a = 5 \implies \boxed{5.00 \text{ ft}}$$
[illegible]

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Table 6-7 Internal Pressure Coefficients for Buildings, G_{cpi}

ASCE 7-02

6/6/07

Wind Load Design per ASCE 7-02

Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	0.18	-0.18

Table 6-8 External Pressure Coefficients for Arched Roofs, Cp

r (Rise-to-Span Ratio) = 0.3

Condition	Variable	Cp		
		Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	Cp	0.13	-1	-0.5
	P (+GCpi) - psf	-1.34	-19.40	-11.37
	P (-GCpi) -psf	5.35	-12.71	-4.69
Roof Springing from Ground	Cp	0.42	-1	-0.5
	P (+GCpi) - psf	3.40	-19.40	-11.37
	P (-GCpi) -psf	3.40	-19.40	-11.37

Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings, Cf

Variable	Description	Value	
L	Roof dimension normal to wind direction	66.00	ft
B	Roof dimension parallel to wind direction	50.00	ft
L/B	Ratio of L to B	1.320	
Theta	Slope of Roof	26.6	Deg
Cf	Force Coefficient	1.14	
X	Distance to center of pressure from windward edge	0.40	ft

Windows				Attachments			
TYPE	MANUFACTURER	SERIES	SIZE	JAMB-TYPE/SPACING	HEADER TYPE/SPACING	SILLS TYPE/SPACING	MULLIONS
Single hung	AWT	3950	3050	4" from corners 8" on center 1 5/8" #10 screws	4" from corners 8" on center 1 5/8" #10 screws	4" from corners 8" on center 1 5/8" #10 screws	Jamb/Header/Sill to be sealed to back w/ silicone
Acrylic Black Fixed Light	U.S. Black	F1-15-8	49"x49" DB	#10 x 2" 3" on end w" oc.	" "	" "	N/A

Doors				Attachment of Frame			
TYPE	MANUFACTURER	SERIES	SIZE	JAMB	HEADER	SILL	
Insulated/Steel	ThermaTru	Steel	3068	#10 x 2 1/2" 10" at corner set 14" in field	#8 x 2 1/2" 5 1/4 x 1 1/2" lag	3 1/4" Tapecon x 175	
Insulated/Steel	Gradco	7825		5/16 x 1 1/2" lag 6" from floor 19 1/2" in field			

Single Specs Attached



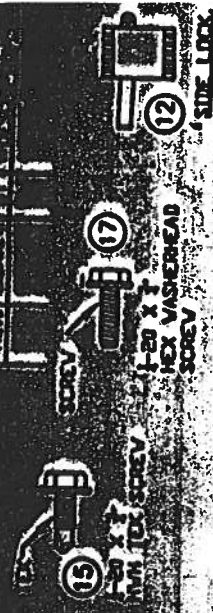
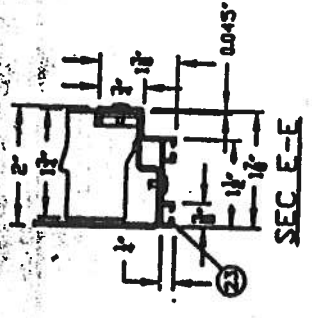
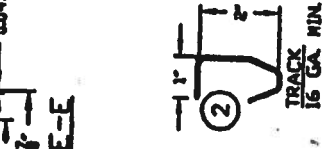
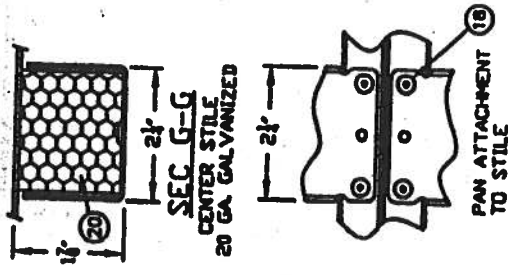
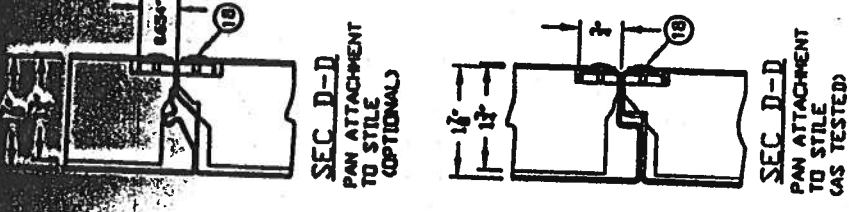
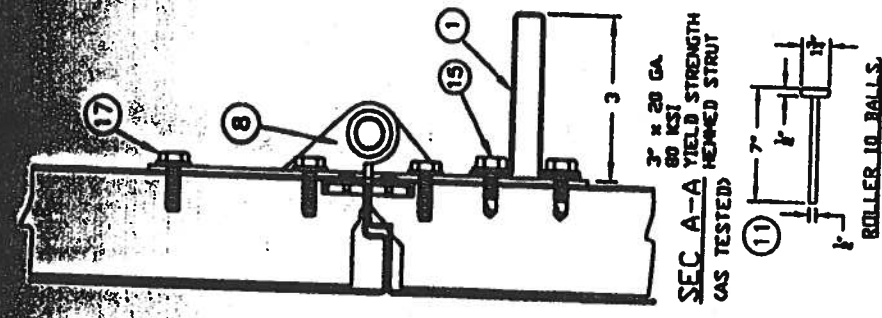
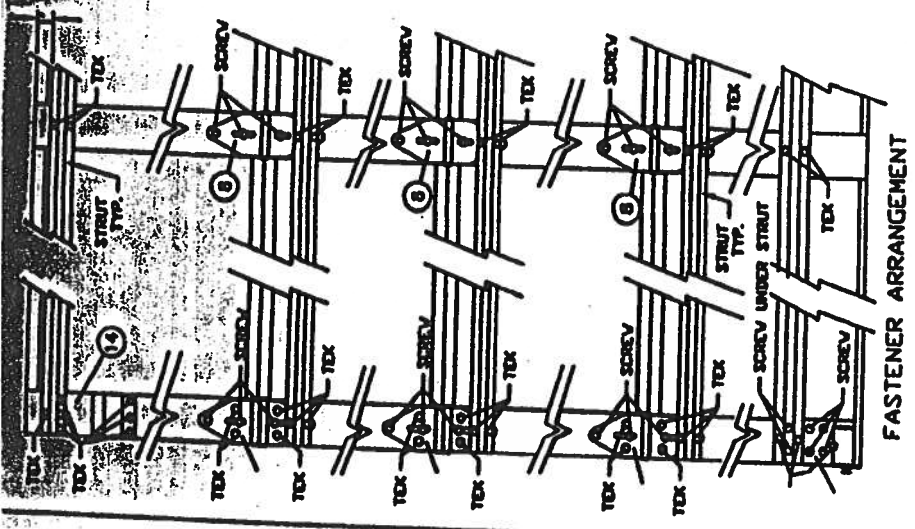
Quality Garage Doors
General American Door Company

IMPORTANT! WINDLOAD DOORS

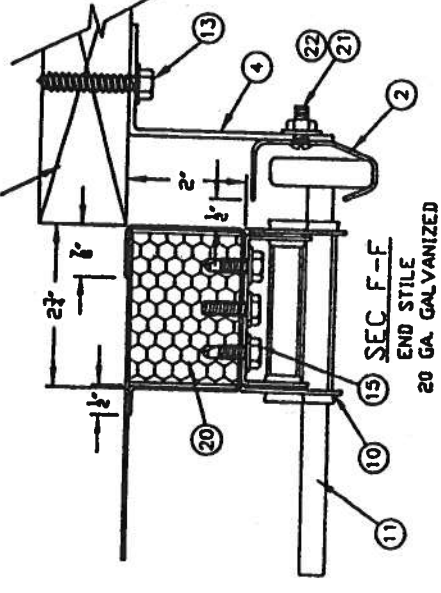
ATTACHED ENVELOPE CONTAINS
YOUR WINDLOAD INSTALLATION
DRAWINGS AND LABEL AS WELL AS A
DUPLICATE SET OF WARNING LABELS.

THE INSTALLER OF THIS DOOR IS
RESPONSIBLE FOR REPLACEMENT OF
WARNING LABELS THAT ARE COVERED
WITH STRUTS OR CHANNELS AS
REQUIRED BY THE WINDLOAD
DRAWINGS. THE BOTTOM FIXTURE
WARNING LABELS MUST BE NEAR THE
BOTTOM FIXTURES. ALL LABELS
MUST BE LEGIBLE UPON COMPLETION
OF THE DOOR INSTALLATION.

A10570 1107-00035



2x6 PRESSURE TREATED LUMBER
(GRADE #2 OR BETTER SOUTHERN PINE)



ITEM NO.	DESCRIPTION	QUANTITY
1	ROLLER BALL, 10 BALLS	10
2	1/2" x 3/4" ROLLER BRACKET	2
3	1/2" x 3/4" ROLLER BRACKET	2
4	1/2" x 3/4" ROLLER BRACKET	2
5	1/2" x 3/4" ROLLER BRACKET	2
6	1/2" x 3/4" ROLLER BRACKET	2
7	1/2" x 3/4" ROLLER BRACKET	2
8	1/2" x 3/4" ROLLER BRACKET	2
9	1/2" x 3/4" ROLLER BRACKET	2
10	1/2" x 3/4" ROLLER BRACKET	2
11	1/2" x 3/4" ROLLER BRACKET	2
12	1/2" x 3/4" ROLLER BRACKET	2
13	1/2" x 3/4" ROLLER BRACKET	2
14	1/2" x 3/4" ROLLER BRACKET	2
15	1/2" x 3/4" ROLLER BRACKET	2
16	1/2" x 3/4" ROLLER BRACKET	2
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18	1/2" x 3/4" ROLLER BRACKET	2
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24	1/2" x 3/4" ROLLER BRACKET	2
25	1/2" x 3/4" ROLLER BRACKET	2
26	1/2" x 3/4" ROLLER BRACKET	2
27	1/2" x 3/4" ROLLER BRACKET	2
28	1/2" x 3/4" ROLLER BRACKET	2
29	1/2" x 3/4" ROLLER BRACKET	2
30	1/2" x 3/4" ROLLER BRACKET	2
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73	1/2" x 3/4" ROLLER BRACKET	2
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95	1/2" x 3/4" ROLLER BRACKET	2
96	1/2" x 3/4" ROLLER BRACKET	2
97	1/2" x 3/4" ROLLER BRACKET	2
98	1/2" x 3/4" ROLLER BRACKET	2
99	1/2" x 3/4" ROLLER BRACKET	2
100	1/2" x 3/4" ROLLER BRACKET	2

GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538

DATE: 6/9/04
BY: J. K. C.
REVIEWED: -
16' X 7' MAX. RAISED PANEL STEEL DOOR - VINDLOAD
PAGE 2 OF 3
J13220-2
REVISED NUMBER

THE SEAL ON THIS DRAWING ONLY
CERTIFIES THAT THE PRODUCTS
ILLUSTRATED AND DESCRIBED HEREIN
REPRESENT THE CONFIGURATION(S),
DIMENSIONS AND INSTALLATION(S) OF
THE DOOR AS TESTED.

REV. 4 DATE 4/87 BY DESCRIPTION

2X6 JAMB AND SUPPORTING STRUCTURE ATTACHMENT

2X6 PRESSURE TREATED GRADE #2 OR BETTER SOUTHERN PINE WOOD JAMB SHALL BE ANCHORED TO BUILDING WOOD FRAME, GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS.

NOTES
1) ALL DOOR OPENING SURROUNDING STRUCTURE TO BE DESIGNED BY REGISTERED ENGINEER OR ARCHITECT TO RESIST APPLIED LOADS.

2) ALL DOOR OPENING STRUCTURE AND FASTENERS TO COMPLY WITH ALL APPLICABLE CODES INCLUDING SBCCI STANDARD FOR HURRICANE RESISTANT RESIDENTIAL CONSTRUCTION SSTD 10, CURRENT EDITION.

3) ALL FASTENERS TO BE INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INSTRUCTIONS AND RECOMMENDATIONS.

4) REINFORCED CMU OR CONCRETE 2X6 WOOD JAMB SHALL BE ANCHORED TO SOLIDLY GROUTED AND REINFORCED CONCRETE MASONRY UNIT (CMU) WALLS OR COLUMNS, OR REINFORCED CONCRETE COLUMNS. ANCHOR SPACING AND EMBEDMENT IS BASED ON CONCRETE MASONRY UNITS COMPLYING WITH ASTM C90 WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 2150 PSI. GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI. REINFORCED CONCRETE COLUMNS WITH A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.

5) EMBEDMENTS LISTED ARE THE MINIMUM ALLOWABLE EMBEDMENTS.

6) WASHERS ARE REQUIRED ON ALL FASTENERS. SEE CHART FOR ACCEPTABLE WASHERS.

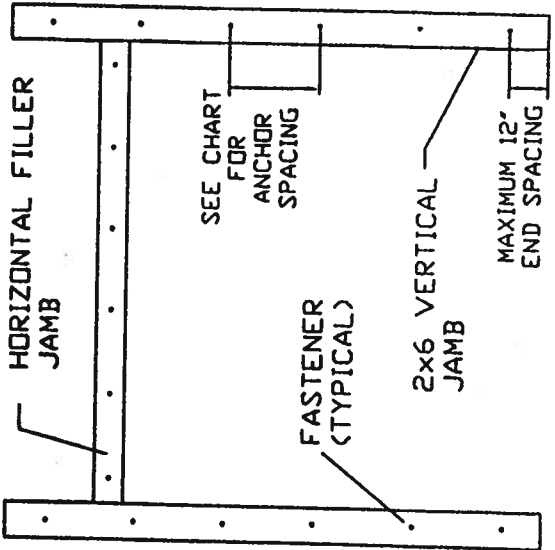
7) USE THE DOOR WIDTH / ANCHOR SPACING TABLE TO DETERMINE THE ANCHOR SPACING IN INCHES. ANCHOR SPACINGS ON THE TABLE ARE FOR USE WITH SPECIAL INSPECTION. REDUCE ANCHOR SPACING BY HALF FOR USE WITHOUT INSPECTION.

8) FOR THE UPPER THREE INDIVIDUAL STEEL JAMB BRACKETS, BRACKETS SHALL BE CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS. IF THE STEEL JAMB BRACKET IS NOT CENTERED BETWEEN THE TWO CLOSEST 2X6 WOOD JAMB ANCHORS, ADD AN ADDITIONAL 2X6 WOOD JAMB ANCHOR NEAR THAT STEEL BRACKET TO INSURE THAT THE LOAD FROM THE STEEL BRACKET IS EQUALLY TRANSFERRED TO TWO WOOD JAMB ANCHORS.

9) FOR CONCRETE COMPRESSIVE STRENGTH OF 2500 PSI USE ANCHOR SPACING FOR 2000 PSI CONCRETE.

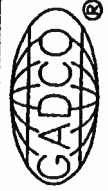
	DOOR WIDTH					
	9'-0"	10'-0"	12'-0"	14'-0"	16'-0"	18'-0"
1 1/4" X 3" ITW TAPCON CONCRETE ANCHORS, WITH 2-1/2" MIN EDGE DISTANCE & 1" OD WASHER						
IN CMU BLOCK, 1-1/4" EMBEDMENT	16.0	14.0	12.0	10.0	9.00	8.00
IN 2000 PSI CONCRETE, 1-1/2" EMBEDMENT	16.0	14.0	12.0	10.0	9.00	8.00
IN 4000 PSI CONCRETE, 1-1/2" EMBEDMENT	24.0	24.0	24.0	22.0	20.0	17.0
3/8" X 3" SIMPSON STRONG-TIE SLEEVE - ALL SLEEVE EXPANSION ANCHORS, WITH 3" MIN EDGE DISTANCE & WASHERS PROVIDED						
IN 2000 PSI CONCRETE, 1-1/2" EMBEDMENT	24.0	24.0	24.0	24.0	22.0	20.0
IN 4000 PSI CONCRETE, 1-1/2" EMBEDMENT	24.0	24.0	24.0	24.0	24.0	24.0
3/8" X 3-1/2" SIMPSON STRONG-TIE WEDGE-ALL WEDGE STAINLESS STEEL EXPANSION ANCHORS, WITH 3" MIN EDGE DISTANCE & WASHERS PROVIDED						
IN 2000 PSI CONCRETE, 1-1/2" EMBEDMENT	24.0	24.0	24.0	24.0	24.0	24.0
IN 4000 PSI CONCRETE, 1-1/2" EMBEDMENT	24.0	24.0	24.0	24.0	24.0	24.0
7/16" X 8" L BOLTS, W/ 2" MIN SQUARE WASHERS	36.0	36.0	36.0	36.0	36.0	36.0
MIN 3 ANCHORS PER JAMB, MIN 4" EDGE DISTANCE						
3/8" X 3" LAG SCREW, WITH 1" OD WASHER	24.0	24.0	24.0	24.0	24.0	24.0
SOUTHERN YELLOW PINE, SPECIFIC GRAVITY = .55, WITH 1-1/2" MIN EDGE DISTANCE & 1-1/2" EMBEDMENT IN 3/16" PREDRILLED HOLES						

DOOR WIDTH / ANCHOR SPACING TABLE



THE SEAL ON THIS DRAWING ONLY CERTIFIES THAT THE PRODUCT(S) ILLUSTRATED AND DESCRIBED HEREIN REPRESENT THE CONFIGURATION(S), DIMENSIONS AND INSTALLATION(S) OF THE DOOR AS TESTED.

GENERAL AMERICAN DOOR COMPANY
5050 BASELINE ROAD
MONTGOMERY, IL 60538



SCALE: NONE
DATE: 6/9/04
DESCRIPTION: 16' X 7' MAX. RAISED PANEL STEEL DOOR - WINDLOAD

APPROVED BY: J. K. C.
REVISED: -

MIAMI-DADE

**BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION**

**MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908**

NOTICE OF ACCEPTANCE (NOA)

**Therma-Tru Corporation
1687 Woodlands Drive
Maumee, OH 43537**

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: Series "Benchmark" Glazed Inswing Residential Insulated Steel Doors w/Adjusta-Fit Steel Frame

APPROVAL DOCUMENT: Drawing No. S-2007, titled "Benchmark Glazed Inswing Steel Door w/Adjusta-Fit Steel Frame", sheets 1 through 5 of 5, prepared by the Rick Wright Building Consulting, Inc., dated 2/28/00 with revision on 3/28/00, bearing the Miami-Dade County Product Control Renewal stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: None

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

This NOA renews NOA # 99-1028.11 and, consists of this page 1 as well as approval document mentioned above. The submitted documentation was reviewed by Jaime Eisen, P.E.



**NOA No 03-0109.04
Expiration Date: April 21, 2008
Approval Date: February 20, 2003
Page 1**

Therma-Tru Corporation

NOTICE OF ACCEPTANCE: EVIDENCE SUBMITTED

(For File ONLY. Not part of NOA)

A. DRAWINGS

1. Manufacturer's die drawings and sections.
2. Drawing No. S-2007, titled "Benchmark Inswing Glazed Steel Door w/Adjusta-Fit Steel Frame", sheets 1 through 5 of 5, prepared by the Rick Wright Building Consulting, Inc., dated 2/28/00 with revision on 3/28/00.

B. TESTS

Submitted under NOA#99-1028.11

1. Test reports on 1) Air Infiltration Test, per FBC, TAS 202-94
2) Uniform Static Air Pressure Test, Loading per FBC, TAS 202-94
3) Water Resistance Test, per FBC, TAS 202-94
4) Large Missile Impact Test per FBC, TAS 201-94
5) Cyclic Wind Pressure Loading per FBC, TAS 203-94
6) Forced Entry Test, per FBC 2411 3.2.1 and TAS 202-94
along with marked-up drawings and installation diagram of outswing and inswing steel edge opaque residential insulated steel pairs of doors, prepared by Certified Testing Laboratories, Test Report No. CTLA-469W, dated February 16, 2000, signed and sealed by Ramesh C. Patel, P.E.

C. CALCULATIONS

Submitted under NOA#99-1028.11

1. Anchor Calculations and structural analysis, dated November 1, 1999 prepared, signed and sealed by Mark Fetherman, P.E.

D. MATERIAL CERTIFICATIONS

Submitted under NOA#99-1028.11

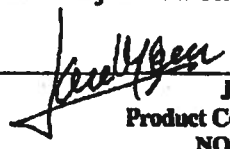
1. Tensile Test prepared by Certified Testing Laboratories, Project No. CTLA-469W, dated October 26, 1999, for steel samples referenced DADE 990044, tested per ASTM E8/A370 or B557, signed and sealed by Ramesh C. Patel, P.E.
2. Test report # WHI-495-0920, issued to General Products Company, Inc. for "Two single swing 3' 0" x 7' 0" door and frame assemblies" ASTM E-152, dated August 28, 1988.

E. STATEMENTS

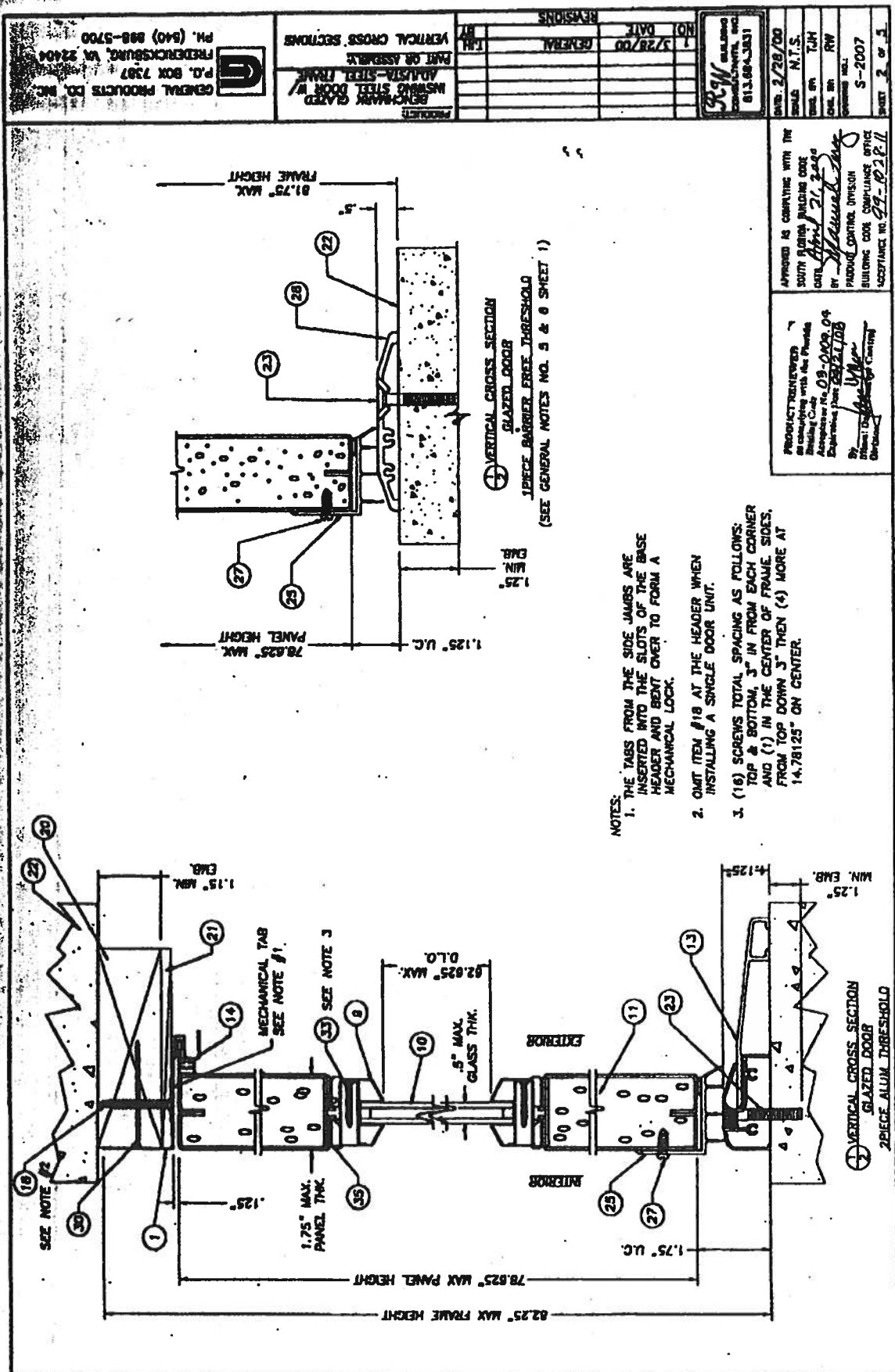
1. Statement letter of no change, prepared by R.W. Building Consultants, Inc., dated 12/22/02.

F. OTHER

1. Notice of Acceptance No. 99-1028.11, General Products, Co., Inc., Benchmark" Inswing Glazed Residential Insulated Steel Doors w/Adjusta-Fit Steel Frame, approved on 4/21/00 and expiring on 4/21/03.


Jaime Eisen, P.E.
Product Control Examiner
NOA No 03-0109.04
Expiration Date: April 21, 2008
Approval Date: February 20, 2003

DATE	2/28/00
SCALE	N.T.S.
DRAWN BY	TJM
CHECKED BY	RW
DATE	5-2007



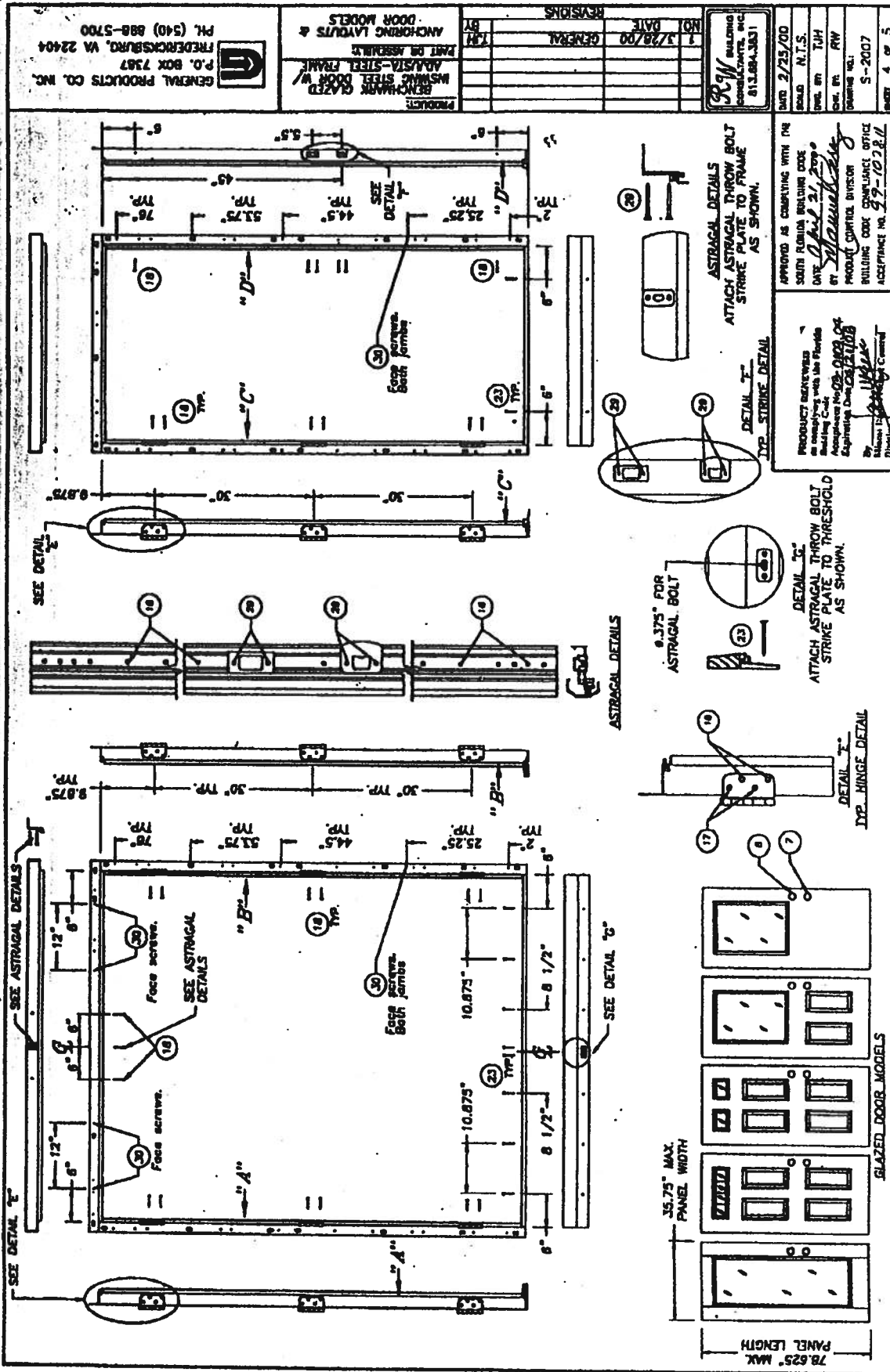
- NOTES:
1. THE TASS FROM THE SIDE JAMBS ARE INSERTED INTO THE SLOTS OF THE BASE HEADER AND BENT OVER TO FORM A MECHANICAL LOCK.
 2. OMIT ITEM #18 AT THE HEADER WHEN INSTALLING A SINGLE DOOR UNIT.
 3. (16) SCREWS TOTAL SPACING AS FOLLOWS:
 TOP & BOTTOM, 3" IN FROM EACH CORNER
 AND (1) IN THE CENTER OF FRAME SIDES,
 FROM TOP DOWN 3" THEN (4) MORE AT
 14.78125" ON CENTER.

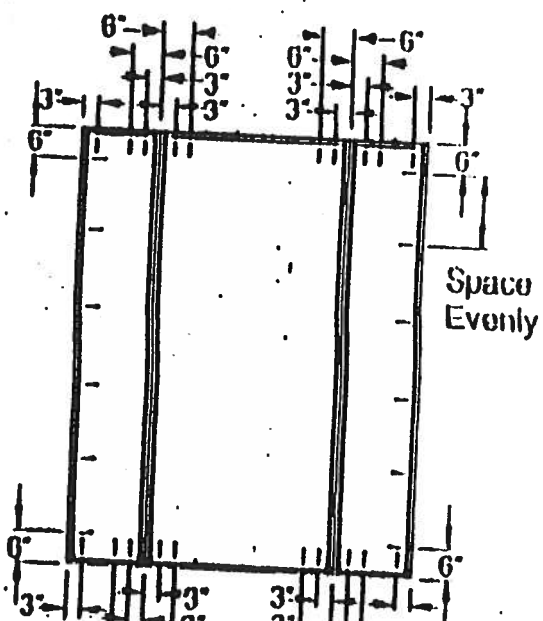
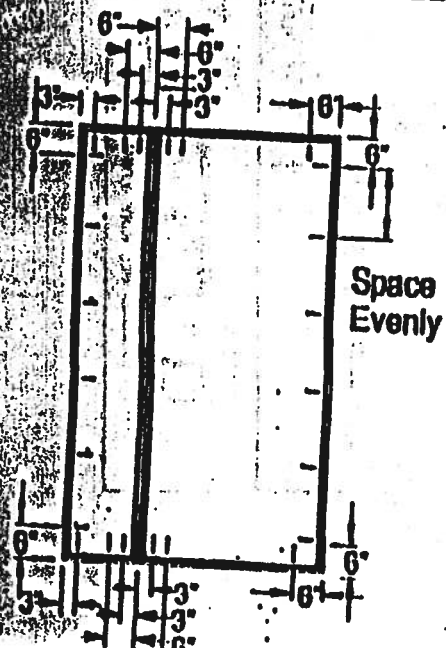
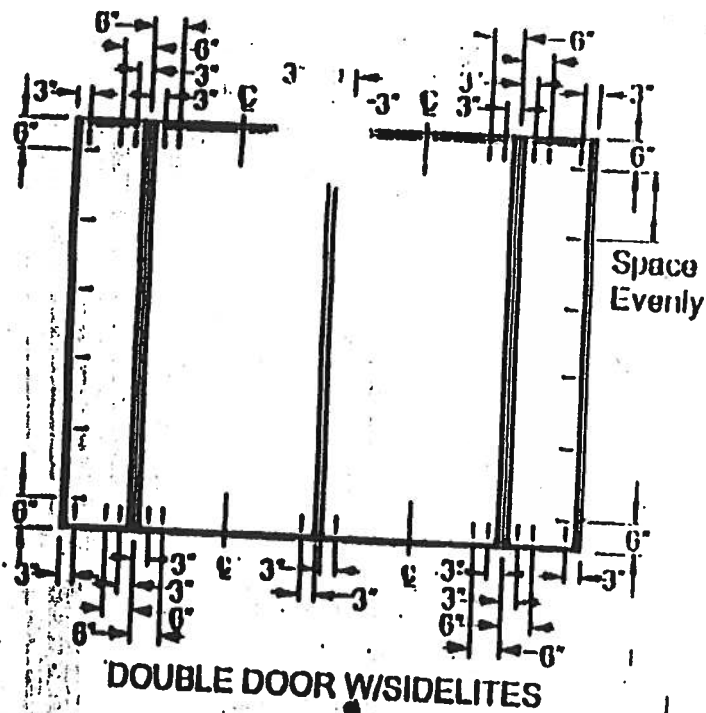
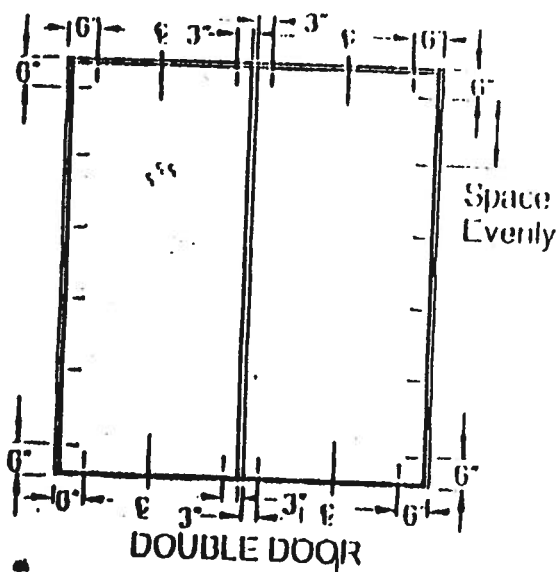
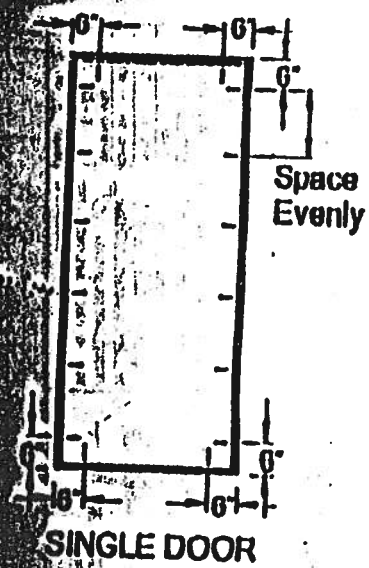
VERTICAL CROSS SECTION
 GLAZED DOOR
 2PIECE BARRIER FREE THRESHOLD
 (SEE GENERAL NOTES NO. 5 & 6 SHEET 1)

PRODUCT REVIEWED
 as complying with the Florida
 Building Code
 Acceptance No. 09-0009-0-9
 Inspection Date 03/21/08
 By [Signature]
 Division of Building Inspection

APPROVED AS COMPLYING WITH THE
 SOUTH FLORIDA BUILDING CODE
 DATE April 21, 2008
 BY [Signature]
 PRODUCT CONTROL DIVISION
 BUILDING CODE COMPLIANCE OFFICE
 ACCEPTANCE NO. 09-0288-11

GENERAL PRODUCTS CO., INC. FREDERICKSBURG, VA 22404 PH. (540) 888-5700		BENCHMARK GLAZED INSULATED STEEL FRAME PART OR ASSEMBLY VERTICAL CROSS SECTIONS	REVISIONS NO. DATE 1 5/28/00 GENERAL	DRAWING No. 66 BENCHMARK GLAZED INSULATED STEEL FRAME 811.684-3831	DATE: 2/28/00 SCALE: N.T.S. TYPED BY: TJH CHECKED BY: RW DESIGNED BY: [Signature] S-2007 SHEET 2 OF 3
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October 21, 2003

Page 1 of 5

1. PROJECT DATA

Project: AAMA 460-00 Performance Test
Series 3160 Fixed / 3960 Twin Single Hung

Date of Testing: October 23, 2003

Tested For: Action Window Technologies
1312 Crosby Rd
Carrollton, TX 75006

Witnessed By: (All or Partial Viewing)

Tony Rodriguez Action Window Technologies
Jay Halsey Action Window Technologies

Wesley A. Wilson Construction Consulting Laboratory, *International*

CONSTRUCTION CONSULTING LABORATORY, *INTERNATIONAL*



Page 2 of 5

2. INTRODUCTION

This report presents the performance results of Action Window Technologies Series 3180 PVC Fixed over Series 3950 PVC Twin Single Hung. Tests were conducted at Construction Consulting Laboratory, International, (CCLI) testing facility in Carrollton, TX.

3. SCOPE

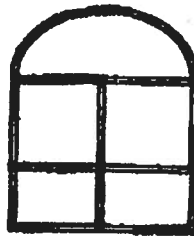
CCLI was requested to report the testing results for an Action Window Technologies Series 3180 / 3950. Tests were conducted in accordance with AAMA 450-00 and ASTM E 330-02.

4. SUMMARY

Action Window Technologies Series 3180 / 3950 was tested in accordance AAMA 450-00 and ASTM E 330-02 and achieved a Design 50 rating.

5. TEST SPECIMEN

PRODUCT TYPE:	PVC Fixed over PVC Twin Single Hung, Photograph, Appendix B, and Product Drawings, Appendix A
SERIES/MODEL:	Action Series 3180 fixed and Series 3950 Hung
SPECIFICATION:	AAMA 405-00
OVERALL SIZE:	5'-11 3/4" X 8'-11 1/16"
FIXED SIZE:	5'-11 3/4" X 3'-0"
HUNG SIZE:	2'-11 5/8" X 5'-11 1/2"
SASH SIZE:	2'-9 1/2" X 2'-5 3/16"
FRAME DIMENSION:	2.795"
CONFIGURATION:	



CONSTRUCTION CONSULTING LABORATORY, INTERNATIONAL



November 21, 2003

Page 3 of 5

Refer to Mock-Up drawing in Appendix A. This report is not complete unless this drawing is stamped and initialed by CCLI as illustrated below.



CONSTRUCTION CONSULTING
LABORATORY, INTERNATIONAL

1601 LBJ Road

Carrollton Texas 75006

Phone (972) 242-0556

03-261 GW 11-21-03

Weather-strip: One row pile weather-strip with integral plastic fin (0.300" thickness) at the exterior face sash top rail. One row pile weather-strip with integral plastic fin (0.260" thickness) at the exterior and lateral face sash stiles. One row bulb vinyl with foam walls (0.500" diameter) at the exterior leg beneath sash bottom rail.

Hardware: Cam action locks, one (1), 7" on center from each end of sash top rail, attached with two (2), #8 x 1" self tapping screws with keeper groove at fixed interlock rail. One tilt latch and one steel pivot bar at top and bottom of sash stiles. One spiral type balance per jamb for sash operation.

Glass Fixed Lite: 5/8" overall thickness sealed insulating glass, two pieces DSB with 3/8" swiggle strip air spacer.

Sash Glass: 5/8" overall thickness sealed insulating glass, two pieces SSB with 7/16" swiggle strip metal air spacer.

Glazing: Exterior glazed with silicone sealant at interior of glass and rigid vinyl snap-in glazing bead at exterior of glass.

Fixed Window Weep Arrangement: 1/2" x 1/4" weep slot spaced 5 1/4" from each end in the glazing pocket at frame sill with a 1" x 1/8" weep slot at the exterior of frame sill spaced 3 1/4" from each end.

Hung Weep Arrangement: 3/16" x 1/8" weep slot spaced 2 1/4" from each end in the glazing pocket punched through fixed interlock. 3/16" x 1/8" weep slot punched through sash bottom rail spaced 2 1/4" from each end. Screen retaining legs and frame sill center leg notched 1/2" at each end

Reinforcement: T-shaped extruded aluminum reinforcement at sash members and L-shaped at fixed interlock. T-shaped extruded aluminum at vertical and horizontal mullion.

CONSTRUCTION CONSULTING LABORATORY, INTERNATIONAL



November 21, 2003

Page 4 of 5

Installation: Test buck was constructed from nominal 2" x 6" and 2" x 4" at the perimeter with 1/2" OSB or strand board at the infil. Window opening is cut into the OSB and then framed with nominal 2" x 4" SPF. Window nail fin is attached through OSB into 2" x 4" with # 8 x 1 1/2" wood screws spaced 2-3" from each end and on approximately 12" centers.

Other Features: Frame and sash corners are mitered and welded. Fixed interlock reinforcement attached to frame jambs with one (1) #8 x 2" wafer head screw and one (1) 7/8" dia flat washer per end.

Mullion Features: Mullion brackets (steel plate 2 1/4" x 3 1/4" x 0.050"t) are attached to each end of horizontal mullion and bottom of vertical mullion with two (2) # 8 x 1" wafer or pan head screws into mullion screw spline, Photograph 2, Appendix B. Mullion brackets are then attached into test buck with two (2) #8 x 1 1/2" wood screws. Frame members are attached to extruded aluminum mullion with #8 x 1" screws spaced approximately 2-3" from each end and on 12" centers. Rigid vinyl mull cover is applied to the extruded aluminum mulls at interior and exterior face of mullion.

Date Tests Started: October 23, 2003

Date Tests Completed: October 23, 2003

Testing Performed at: Construction Consulting Laboratory, International in Carrollton, Texas

6. PERFORMANCE RESULTS

<u>Title Of Test</u>	<u>Test Method</u>	<u>Measured</u>	<u>Allowed</u>
Uniform Deflection @ Horizontal Mullion	ASTM E 330-02 (10 Second Duration)		
-Positive @ 50 PSF		.550"	.408"
-Permanent Set		.040"	.286"
-Negative @ 50 PSF		.595"	.408"
-Permanent Set		.080"	.286"
Uniform Deflection @ Vertical Mullion	ASTM E 330-02 (10 Second Duration)		
-Positive @ 50 PSF		.830"	.408"
-Permanent Set		.000"	.286"
-Negative @ 50 PSF		.880"	.408"
-Permanent Set		.050"	.286"

CONSTRUCTION CONSULTING LABORATORY, INTERNATIONAL

8

Page 5 of 5

Uniform Structural	ASTM E 330-02 (10 Second Duration)	
-Positive	75.00 PSF	75.00 PSF
-Negative	75.00 PSF	75.00 PSF
-Permanent Set (Horizontal)	.110	0.288"
-Permanent Set (Vertical)	.085	0.336"

Detailed extrusion and assembly drawings indicating measured wall thickness and corner construction are on file and were compared to the test sample submitted. These records will be retained at CCLI for a period of four years.

7. CONCLUSION

The tests noted in Section 6 of this report were conducted in accordance with the structural requirements of AAMA 450-00 using ASTM E 330-02.

Respectfully submitted,

CONSTRUCTION CONSULTING LABORATORY, INTERNATIONAL


JUVENAL AZUA
TECHNICIAN


WESLEY A. WILSON
LABORATORY MANAGER

CONSTRUCTION CONSULTING LABORATORY, INTERNATIONAL

HERITAGE® 30 AR

LAMINATED ASPHALT SHINGLES

PRODUCT DATA



Manufactured in Tuscaloosa, AL.

HERITAGE® 30 AR shingles feature a double-layer fiberglass mat construction with a random-cut sawtooth design. The two layers of mat are coated with asphalt and then laminated together and surfaced with ceramic granules. A self-sealing strip of asphalt helps provide added wind resistance.

USES

For application to roof decks with inclines of not less than 2 inches per foot. For slopes between 2 inches and 4 inches per foot, refer to wrapper instructions.

ADVANTAGES

- 30-year limited warranty, 5-year FULL START, limited transferability, winds up to 70 MPH.
- Affordable upgrade from 3-tab shingles.
- Superior fire resistance compared to organic shingles.
- Rustic beauty of wood shakes.
- Shadowtone feature adds depth and dimensional appearance.
- 10-year Algae-Relief (AR) limited warranty that provides for cleaning of discoloration caused by certain algae growth that may occur in areas with high humidity.

CERTIFICATIONS/REPORTS

Listed by UL for:
Class A Fire Rating
Wind Resistance
ASTM D 3462

Tested in compliance with:
UL 790/ASTM D 3018, Type I; ASTM E 108, Class A
UL 997/ASTM D 3161, Type I, Class F (110 mph)
TAS 100-95 Wind and Wind Driven Rain

Fed. Spec.: Exceeds SS-S-001534
Class A, Type I

Miami Dade County Florida NOA 05-0311.04,
Expiration Date 03/21/2007

COLORS

Classic Heritage Colors:

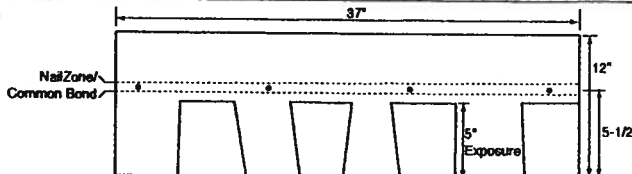
- Weathered Wood
- Rustic Cedar
- Rustic Hickory
- Driftwood
- Oxford Grey
- Shadow Grey
- Desert Sand
- Rustic Black
- Olde English Pewter
- Glacier White
- Rustic Evergreen

America's Natural Colors:

- Thunderstorm Grey
- Black Walnut
- Harvest Gold
- Mountain Slate
- Natural Timber
- Painted Desert

PRODUCT DATA*

Shingle size	12" X 37"
Exposure	5"
Shingles per square	78
Bundles per square	3



*All values stated as nominal

CAUTION: The National Institute for Occupational Safety and Health (NIOSH) has concluded that fumes of heated asphalt are a potential occupational carcinogen. Do not heat or burn this product.



TAMKO
ROOFING PRODUCTS

TAMKO Logo, Roofer Logo, Heritage 30 AR® and TAMKO® are registered trademarks of TAMKO® Roofing Products, Inc.

For information regarding a copy of TAMKO's limited warranty, contact your local TAMKO representative, visit us online at www.tamko.com, or call us at 800-641-4691.

08/2005

Central District	220 West 4th St., Joplin, MO	64801	800-641-4691
Northeast District	4500 Tamko Dr., Frederick, MD	21701	800-368-2055
Southeast District	2300 35th St., Tuscaloosa, AL	35401	800-228-2656
Southwest District	7910 S. Central Exp., Dallas, TX	75216	800-443-1834
Western District	5300 East 43rd Ave., Denver, CO	80216	800-530-8868



DUCT SYSTEM SUMMARY

Entire House

New Age Dimensions

Job: Stidham Residence
10/24/06

17600 S.E. 28th Court, Summerfield, FL 34491-7571 Phone: (352) 307-0692 Fax: (352) 307-9149 Email: www.NewAgeDimension@aol.com

Project Information

For: Newmans Htg. & A/C
P.O. Box 5425, Gainesville, FL 32602
Phone: (352) 375-8555 Fax: (352) 375-0275

External Static Pressure:	HEATING 0.57 in H2O	COOLING 0.57 in H2O
Pressure Losses:	0.12 in H2O	0.12 in H2O
Available Static Pressure:	0.45 in H2O	0.45 in H2O
Friction Rate:	0.880 in/100ft	0.880 in/100ft
Actual AVF:	1200 cfm	1200 cfm

Total Effective Length (TEL): 0 ft

Supply Branch Detail Table

Name	Htg (Btuh)	Clg (Btuh)	Htg (cfm)	Clg (cfm)	Dsn FR	Vel (fpm)	Dia (in)	Rect Sz (in)	Duct Matl	Trnk
Master Bedroom	2429	1616	86	79	0.880	632	5	0x 0	VIFx	st1
Master Bedroom-A	2429	1616	86	79	0.880	632	5	0x 0	VIFx	st1
WIC Hers	311	142	11	7	0.880	126	4	0x 0	VIFx	st1
WIC His	26	32	1	2	0.880	18	4	0x 0	VIFx	st1
Pantry	23	28	1	1	0.880	16	4	0x 0	VIFx	st1
Master Toilet	32	40	1	2	0.880	22	4	0x 0	VIFx	st2
Master Bathroom	1283	711	46	35	0.880	522	4	0x 0	VIFx	st2
Kitchen / Dine-A	4634	3936	164	192	0.880	549	8	0x 0	VIFx	st1
Kitchen / Dine	4634	3936	164	192	0.880	549	8	0x 0	VIFx	st1
Great Room	4601	2680	163	130	0.880	611	7	0x 0	VIFx	st1
Great Room-A	4601	2680	163	130	0.880	611	7	0x 0	VIFx	st2
Laundry Room	388	1487	14	72	0.880	531	5	0x 0	VIFx	st2
Bathroom #2	56	69	2	3	0.880	38	4	0x 0	VIFx	st2
Bedroom #2	4188	2841	149	138	0.880	556	7	0x 0	VIFx	st2
Bedroom #3	4188	2841	149	138	0.880	556	7	0x 0	VIFx	st2

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Vel (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	677	681	817	12	15 x 8	ShtMetl	st
st2	Peak AVF	523	519	627	12	15 x 8	ShtMetl	st
st	Peak AVF	1200	1200	823	16	15 x 14	ShtMetl	

Return Branch Detail Table

Name	Diffus Sz (in)	Design AVF (cfm)	Design (in H2O)	Design FR	Vel (fpm)	Dia (in)	Rect Sz (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	25 x 20 25	996	0.00	0.880	564	18	0 x 0		VIFx	ra
rb2	12 x 12	232	0.00	0.880	664	8	0 x 0		VIFx	ra

Return Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Vel (fpm)	Diam (in)	Rect Duct Sz (in)	Duct Material	Trunk
ra	Peak AVF	1200	1200	600	18	18 x 15 16	RectFbg	





RIGHT-J SHORT FORM Entire House

New Age Dimensions

Job: Stidham Residence
10/24/06

17600 S.E. 28th Court, Summerfield, FL 34491-7571 Phone: (352) 307-0692 Fax: (352) 307-9149 Email: www.NewAgeDimension@aol.com

Project Information

For: Newmans Htg. & A/C
P.O. Box 5425, Gainesville, FL 32602
Phone: (352) 375-8555 Fax: (352) 375-0275

Design Information

	Htg	Clg	Infiltration	Simplified
Outside db (°F)	33	94	Method	Average
Inside db (°F)	68	75	Construction quality	0
Design TD (°F)	35	19	Fireplaces	
Daily range	-	M		
Inside humidity (%)	-	50		
Moisture difference (gr/lb)	-	48		

HEATING EQUIPMENT

Make Ruud
Trade Ruud UPMC Series
UPMC-036JA

Efficiency 8.6 HSPF

Heating input 36200 Btuh @ 47°F

Heating output 27 °F

Heating temperature rise 1200 cfm

Actual heating fan 0.035 cfm/Btuh

Heating air flow factor

Space thermostat

COOLING EQUIPMENT

Make Ruud
Trade Ruud UPMC Series
UPMC-036JA
UBHJ-21+RCBA-37**+RXCT-CHK

Efficiency 13.0 SEER

Sensible cooling 25752 Btuh

Latent cooling 11037 Btuh

Total cooling 36789 Btuh

Actual cooling fan 1200 cfm

Cooling air flow factor 0.049 cfm/Btuh

Load sensible heat ratio 72 %

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
Master Bedroom	240	4858	3232	172	157
WIC Hers	36	311	142	11	7
WIC His	20	26	32	1	2
Pantry	18	23	28	1	1
Master Toilet	26	32	40	1	2
Master Bathroom	131	1283	711	46	35
Kitchen / Dine	312	9267	7872	329	383
Great Room	480	9202	5361	326	261
Laundry Room	36	388	1487	14	72
Bathroom #2	44	56	69	2	3
Bedroom #2	179	4188	2841	149	138
Bedroom #3	179	4188	2841	149	138

Printout certified by ACCA to meet all requirements of Manual J 7th Ed.



Entire House	d	1700	33822	24655	1200	1200
Ventilation air			0	0		
Equip. @ 0.99 RSM				24409		
Latent cooling				9620		
TOTALS		1700	33822	34029	1200	1200

Printout certified by ACCA to meet all requirements of Manual J 7th Ed.



Truss Tie Downs for Columbia County Application

Tax Parcel # _____

Truss #	Anchor Point Uplifts			
R1	A 247 (1)	D 354 Hanger (LUS24)		
R2	A 170 (1)	B-152 - 2-16D Toe Nail	C-59 - 2-16D Toe Nail	
R3	A 216 (1)	B-179 - 2-16D Toe Nail	C-159 - 3-16D Toe Nail	
R4	A 51 (1)	B-20 - 2-16D Toe Nail	C-3 - 2-16D Toe Nail	
R5	A 89 (1)	B-71 - 2-16D Toe Nail	C-25 - 2-16D Toe Nail	
R6	A 127 (1)	B-113 - 2-16D Toe Nail	C-42 - 2-16D Toe Nail	
R7	A 248 (1)	C-393 (1)		
R8	A 219 (1)	C-224 (1)		
R9	A 266G (1)	G-775 (2)	R 946 Q2 (1)	D 675G (2)
R10	A 164 (1)	G-679 (2)	S 820 Q2 (1)	D 287G (1)
R11	A 272 (1)	G 559 (2)	R 769 (2)	D 168G (1)
R12	A 299 (1)	G 521 (2)	H 793 (2)	D 158 (1)
R13	A 290 (1)	G 560 (2)	S 655 (2)	D 217 (1)
R14	A 307 (1)	G 541 (2)	R 649 (2)	D 224 (1)
R15	A 337 (1)	G 504 (1)	H 652 (2)	D 228 (1)
R16	A 307 (1)	G 543 (2)	R 637 (2)	D 235 (1)
R17	A 293 (1)	G 558 (2)	R 648 (2)	D 223 (1)
R18	A 169 (1)	G 495 (1)	H 566 (2)	D 117 (1)
R19	A 135 (1)	G 528 (2)	R 555 (2)	D 134 (1)
R20	A 284 (1)	O 575 (1)	P 575 (2)	D 284 (1)
R21	A 205 (1)	I 654 (2)	G 654 (2)	D 205 (1)
R22	A 249 (1)	O 608 (2)	P 620 (2)	D 241 (1)
R23	A 170 (1)	P 689 (2)	Q 689 (2)	D 170 (1)
R24	A 112 (1)	N 747 (2)	O 747 (2)	D 112 (1)
T-1	H 253 (1)	LSTA12 @ 48" OC		
T-2	E 253 (1)	LSTA12 @ 48" OC		

Simpson Strong Tie #	Uplift Amount
LUS26	730
(1) H2.5A	520
(2) H10	780

26011

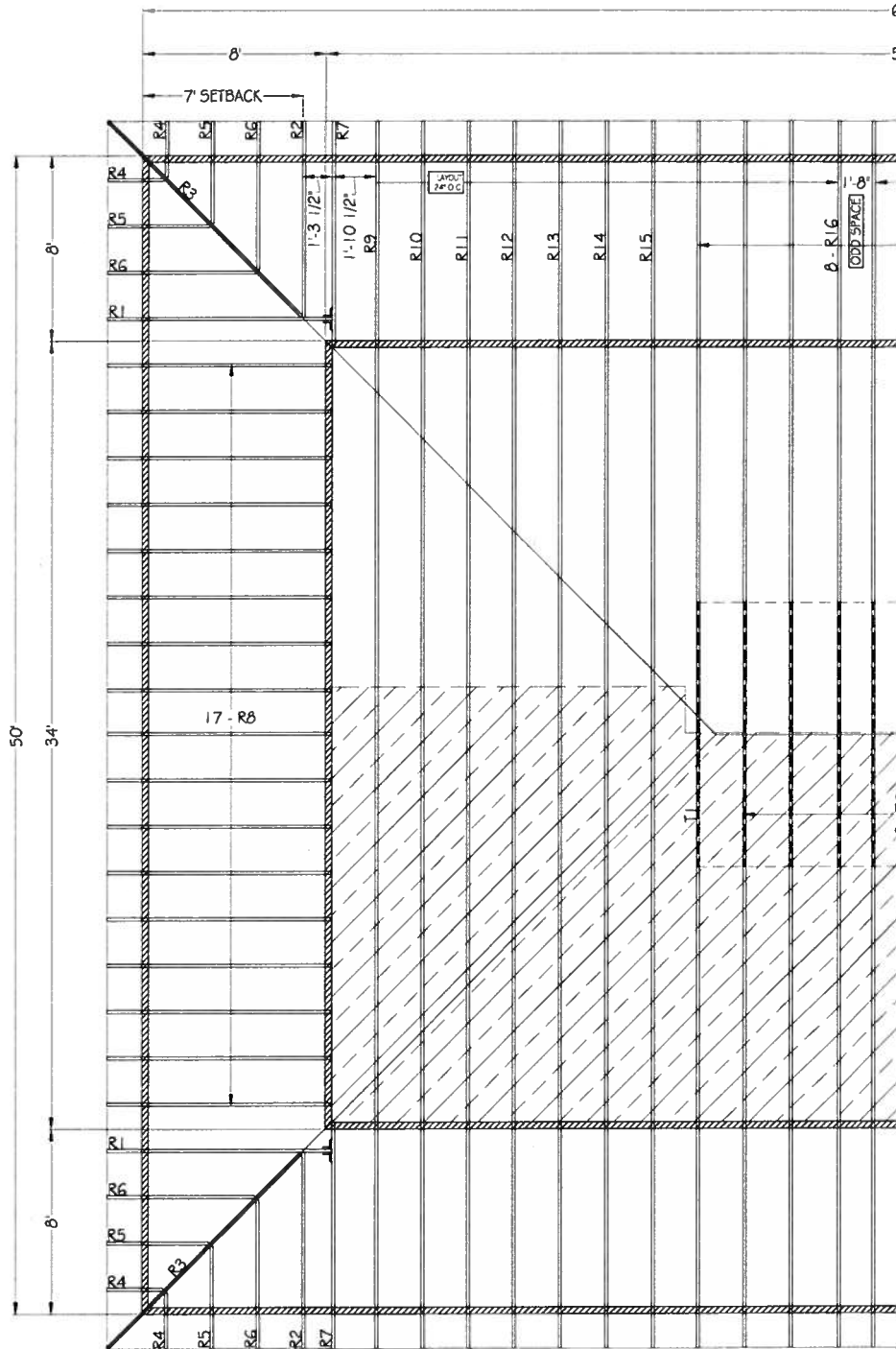
FIELD DENSITY WORKSHEET

CLIENT G C DATE 8-6-07
 PROJECT NO. _____
 PROJECT NAME 1935 S.W. GULPHER COURT (FORT WHITE) PERMIT NO. 000026011
 EARTH CONTRACTOR _____ TESTED BY SC
 COMPACTION REQUIREMENT (%) 95 ☐ Standard Proctor ☒ Modified Proctor _____ FIELD CONTACT
 TOTAL ON-SITE TIME _____ MILES FROM OFFICE _____
☐ Limerock ☐ Subgrade ☐ Pipe Backfill ☒ Building Pad ☒ Building Footing ☐ Other _____

TEST LOCATION	LAB PROCTOR		TEST DEPTH	PROBE DEPTH	% MOIST.	WET DENSITY (PCF)	DRY DENSITY (PCF)	% COMP.
	DENS.	OMC						
	102.1	10.9	FLG	10.9				
CNTR OF E. FOUNDATION					11.4	117.3	105.3	98.3
CNTR OF N.E. CORNER OF FOUNDATION					12.1	118.9	106.0	99.0
CNTR OF S. 1/2 OF BLDG. PAD					11.8	119.0	106.5	99.4

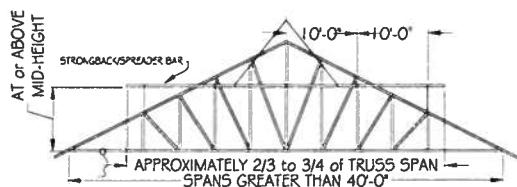
REMARKS _____

- * Density failed to meet minimum project requirement
- ** Retest indicates minimum density requirement was obtained.
- () Client is aware of unsatisfactory test results.

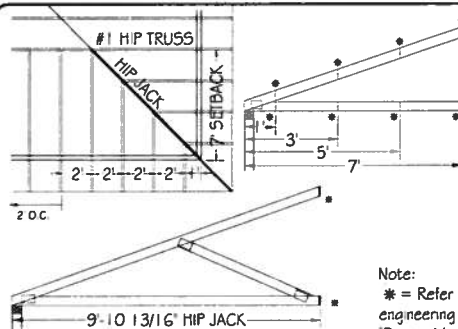


"WARNING"

FAILURE TO FOLLOW RECOMMENDATIONS IN "BCSI-B1" COULD RESULT IN SEVERE PERSONAL INJURY, LOSS OF LIFE OR PROPERTY DAMAGE

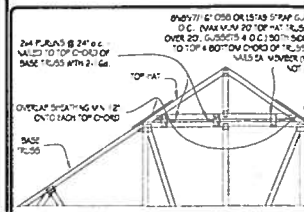


FOR ERECTION INSTRUCTIONS
SEE BCSI-B1 IN ENGINEERING BOOK



Note:
* = Refer to truss
engineering in
"Design Manual" for
connection.

TYPICAL HIP CORNER DETAIL



Ridgway Roof Truss Company

(Trusses and Prefabricated Building Components)

Mailing: P.O. Box 1309 – Gainesville, Florida 32602

Physical: 235 SW 11th Place – Gainesville, Florida 32601

Telephone: (352) 376-4436

FAX: (352) 371-3316

Email: Sales@RidgwayTruss.com

www.RidgwayTruss.com

WARNING

**THESE TRUSSES MUST BE
HANDLED ACCORDING TO
BCSI-B1 SUMMARY SHEETS**

SEE TABLE OF CONTENTS

SEALED

DESIGN MANUAL

PROJECT NAME: Stidham Residence, Roof

JOB NUMBER: 07-0500

CONTRACTOR: GC Construction

DATE: 06/13/2007 jrt

REVISIONS: _____

COMMENTS: _____

SEALED

Permit Number: _____ Lot Number: _____
Miscellaneous: _____ Address: _____ Alachua County _____

The information in this box is for administrative purposes only and is not part of this engineering review.

Truss Fabricator: RIDGWAY ROOF TRUSS

Job Reference: Stidham Residence, Roof, GC Construction

Gary Dounson-
(PE35054) (truss
designer)

GARY DOUNSON AND ASSOC.,
INC., 2830 NW 41st STREET SUITE D
GAINESVILLE, FL 32606
(352) 375-8593, CA 5201

Engineering Index Sheet

Index Page 1 of 1

Job Number 070500 Date 06/13/2007 FBC - 2004 Chapter 16 and 23 Specification Quantity 27

A Professional Engineer's Seal affixed to this Index Sheet indicates the acceptance of professional Engineering responsibilities for individual truss components fabricated in accordance with the listed and attached Truss Specification Sheets. Determination as to the suitability of these individual truss components for any structure is the responsibility of the Building Designer, as defined in ANSI/TPI 1-95, Section 2.2.

Questions regarding this Index Sheet and/or the attached Specification Sheets may be directed to the truss fabricator listed above or Gary Dounson- (PE35054) (truss designer), (Software - Online Plus)

Standard Loading:

T.C. Live 20 psf
T.C. Dead 7 psf
B.C. Live 0 psf
B.C. Dead 10 psf
Total 37 psf

ANSI / ASCE 7-02 Components and Claddings*

Exterior
Wind Speed: 110
Mean Roof Ht.: 25
Exposure Category: B
Occupancy Factor: 1.00
Building Type: Enclosed
TC Dead Load: 4.0
BC Dead Load: 6.0

Notes: Refer to individual truss design drawings for special loading conditions.

Date	Mark	Date	Mark	Date	Mark	Date	Mark
1 06/13/07 R1	2 06/12/07 R2	3 06/12/07 R3	4 06/12/07 R4	5 06/12/07 R5	6 06/12/07 R6	7 06/12/07 R7	8 06/12/07 R8
9 06/13/07 R9	10 06/13/07 R10	11 06/13/07 R11	12 06/13/07 R12	13 06/13/07 R13	14 06/13/07 R14	15 06/13/07 R15	16 06/13/07 R16
17 06/13/07 R17	18 06/13/07 R18	19 06/13/07 R19	20 06/12/07 R20	21 06/12/07 R21	22 06/12/07 R22	23 06/12/07 R23	24 06/12/07 R24
25 06/12/07 T1	26 06/12/07 T2						

JP
6-13-07

TABLE OF CONTENTS

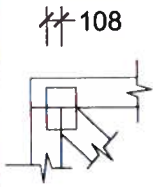
1. GENERAL NOTES
2. WTCA 1-1995 (STANDARD RESPONSIBILITIES IN THE DESIGN PROCESS INVOLVING METAL PLATE CONNECTED WOOD TRUSSES)

ENGINEERING (INSERT)

BLUEPRINT OF TRUSS PLACEMENT PLAN (INSERT)
6. **BCSI-B1 SUMMARY SHEET**
HANDLING, INSTALLING & BRACING INFORMATION
8. STANDARD CHORD & WEB REPAIRS
11. EXAMPLES OF PERMANENT WEB BRACING

ROBBINS ENG. GENERAL NOTES & SYMBOLS

PLATE LOCATION



Center plates on joints unless otherwise noted in plate list or on drawing. Dimensions are given in inches (i.e. 1 1/2" or 1.5") or IN-16ths (i.e. 108)

FLOOR TRUSS SPLICE

(3X2, 4X2, 6X2)



(W) = Wide Face Plate
(N) = Narrow Face Plate

LATERAL BRACING

Designates the location for continuous lateral bracing (CLB) for support of individual truss members only. CLBs must be properly anchored or restrained to prevent simultaneous buckling of adjacent truss members.

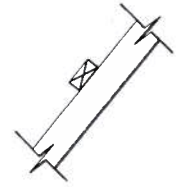
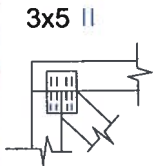


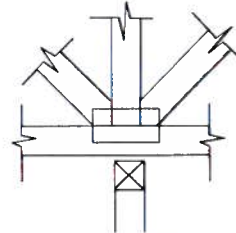
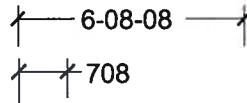
PLATE SIZE AND ORIENTATION



The first dimension is the width measured perpendicular to slots. The second dimension is the length measured parallel to slots. Plate orientation, shown next to plate size, indicates direction of slots in connector plates.

DIMENSIONS

All dimensions are shown in FT-IN-SX (i.e. 6' 8 1/2" or 6-08-08). Dimensions less than one foot are shown in IN-SX only (i.e. 708).



W = Actual Bearing Width (IN-SX)
R = Reaction (lbs.)
U = Uplift (lbs.)

BEARING

When truss is designed to bear on multiple supports, interior bearing locations should be marked on the truss. Interior support or temporary shoring must be in place before erecting this truss. If necessary, shim bearings to assure solid contact with truss.

ROBBINS connector plates shall be applied on both faces of truss at each joint. Center the plates, unless indicated otherwise. No loose knots or wane in plate contact area. Splice only where shown. Overall spans assume 4" bearing at each end, unless indicated otherwise. Cutting and fabrication shall be performed using equipment which produces snug-fitting joints and plates. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication and the attached truss designs are not applicable for use with fire retardant lumber and some preservative treatments. Nails specified on truss design drawings refer to common wire nails, except as noted. The attached design drawings were prepared in accordance with " National Design Specifications for Wood Construction" (AF & PA), " National Design Standard for Metal Plate Connected Wood Truss Construction" (ANSI/TPI 1), and HUD Design Criteria for Trussed Rafters.

Robbins Eng. Co. bears no responsibility for the erection of trusses, field bracing or permanent truss bracing. Refer to BCSI 1-03 as published by Truss Plate Institute, 218 North Lee Street, Suite 312, Alexandria, Virginia 22314. Persons erecting trusses are cautioned to seek professional advice concerning proper erection bracing to prevent toppling and " dominoing ". Care should be taken to prevent damage during fabrication, storage, shipping and erection. Top and bottom chords shall be adequately braced in the absence of sheathing or rigid ceiling, respectively. It is the responsibility of others to ascertain that design loads utilized on these drawings meet or exceed the actual dead loads imposed by the structure and the live loads imposed by the local building code or historical climatic records.

FURNISH A COPY OF THE ATTACHED TRUSS DESIGN DRAWINGS TO ERECTION CONTRACTOR. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO REVIEW THESE DRAWINGS AND VERIFY THAT DATA, INCLUDING DIMENSIONS & LOADS, CONFORM TO ARCHITECTURAL PLAN / SPECS AND THE TRUSS PLACEMENT DIAGRAM FURNISHED BY THE TRUSS FABRICATOR.



6904 Parke East Blvd.
Tampa, FL 33610-4115
Tel: 813-972-1135 Fax: 813-971-6117

www.robbseng.com



Wood Truss Council of America

5937 Meadowood Dr., Ste. 14 • Madison, WI 53711-4125 • 608/274-3329 (fax)

**Standard Responsibilities
in the Design Process
Involving Metal Plate
Connected Wood Trusses**

WTCA 1-1995

Developed by the WTCA Engineering Review Committee
in cooperation with the Truss Plate Institute

The Wood Truss Council of America publishes standard practice materials prepared and edited by knowledgeable authors from the construction industry to give as much assistance as possible to owners, architects, engineers, contractors, building officials, and others involved in the metal plate connected wood truss industry. The competence of the authors ensures accurate and authoritative information in regard to the subject matter covered, but, of course, neither the Wood Truss Council of America, nor the authors make either express or implied warranties in regard to the use of the materials.

1.0 SCOPE AND DEFINITIONS

- 1.1 This standard defines the design responsibilities of the individuals and organizations involved in the preparation, submittal, review and approval of each Truss Design Drawing and Truss Placement Plan associated with the use of metal plate connected wood trusses. These guidelines are presented as industry standard practice. The guidelines are not intended to preclude alternate provisions as agreed upon by the parties involved.
- 1.2 The following definitions shall apply:
 - 1.2.1 "Architect" shall mean the individual registered architect responsible for the architectural design of the structure and who produces the architectural drawings included in the Construction Design Documents.
 - 1.2.2 "Building Designer" is the individual or organization having responsibility for the overall building or structure design in accordance with the state's statutes and regulations governing the professional registration and certification of architects or engineers. This responsibility includes but is not limited to foundation design, structural member sizing, load transfer, bearing conditions, and the structure's compliance with the applicable building codes. Also referred to as registered architect or engineer, building designer, and registered building designer, but hereinafter will be referred to as Building Designer.
 - 1.2.3 "Construction Design Documents" are the architectural drawings, structural drawings, mechanical drawings, electrical drawings, and any other drawings, specifications, and addenda which set forth the overall design of the structure and issued by the Building Designer.
 - 1.2.4 "Contractor" shall mean the individual or organization responsible for the field storage, handling, and installation of trusses including, but not limited to, temporary bracing, permanent bracing, anchorage, connections and field assembly. The term "Contractor" shall include those subcontractors who have a direct contract with the Contractor to perform all or a portion of the storage, handling, and installation of the trusses.
 - 1.2.5 "Engineer-of-Record" shall mean the registered professional engineer responsible for the structural design of the structure and who produces the structural drawings included in the Construction Design Documents.
 - 1.2.6 "Owner" shall mean the individual or organization for whom the structure is designed.
 - 1.2.7 "Truss" is an individual metal plate connected wood structural component manufactured by the Truss Manufacturer.
 - 1.2.8 "Truss Designer" is the design professional, individual or organization, having responsibility for the design of metal plate connected wood trusses. This responsibility shall be in accordance with the state's statutes and regulations governing the professional registration and certification of architects or engineers. Also referred to as truss engineer, design engineer, registered engineer, and engineer, but hereinafter will be referred to as Truss Designer.
 - 1.2.9 "Truss Design Drawing" shall mean the graphic depiction of an individual Truss prepared by the Truss Designer.
 - 1.2.10 "Truss Manufacturer" shall mean an individual or organization regularly engaged in the manufacturing of Trusses.
 - 1.2.11 "Truss Placement Plan" is the drawing identifying the location assumed for each Truss based on the Truss Manufacturer's interpretation of the Construction Design Documents.

2.0 OWNER RESPONSIBILITIES

- 2.1 Directly or through its representatives, which may include the Contractor and/or Building Designer; (a) review and approve each Truss Design Drawing; (b) review and approve the Truss Placement Plan; (c) resolve and approve all design issues arising out of the preparation of each Truss Design Drawing and Truss Placement Plan; and (d) coordinate the return of each approved Truss Design Drawing and Truss Placement Plan to the Truss Manufacturer prior to truss manufacturing.

3.0 BUILDING DESIGNER RESPONSIBILITIES

- 3.1 Design a structure suitable to ensure that the intended function of each Truss is not affected by adverse influences including, but not limited to, moisture, temperature, corrosive chemicals and gases;
- 3.2 Prepare the Construction Design Documents, showing all trussed areas, which must provide as a minimum the following:
- 3.2.1 All truss orientations and locations;
 - 3.2.2 Information to fully determine all truss profiles;
 - 3.2.3 Adequate support of the Truss and all truss bearing conditions;
 - 3.2.4 Permanent bracing design for the structure including the Trusses, except as provided in 3.4 and 6.2.12.
 - 3.2.5 The location, direction and magnitude of all dead and live loads applicable to each Truss including, but not limited to, loads attributable to: roof, floor, partition, mechanical, fire sprinkler, attic, storage, wind, snow drift and seismic;
 - 3.2.6 All Truss anchorage designs required to resist uplift, gravity, and lateral loads;
 - 3.2.7 Allowable vertical and horizontal deflection criteria;
 - 3.2.8 Proper transfer of design loads affecting the Truss; and
 - 3.2.9 Adequate connections between Truss and non-Truss components, except as noted in Section 6.2.9.
- 3.3 Review and approve the Truss Placement Plan and each Truss Design Drawing for conformance with the requirements and intent of the Construction Design Documents, the effect of each Truss Design Drawing and Truss Placement Plan on other parts of the structure, and the effect of the structure on each Truss.
- 3.4 Specify permanent lateral bracing where indicated by the Truss Designer on the Truss Design Drawings, to prevent buckling of the individual truss members due to design loads. The Building Designer shall specify how the permanent lateral bracing is to be anchored or restrained to prevent lateral movement if all truss members, so braced, buckle together. This shall be accomplished by: (a) anchorage to solid end walls; (b) permanent diagonal bracing in the plane of the web members; or (c) other means when demonstrated by the Building Designer to provide equivalent bracing.

4.0 CONTRACTOR RESPONSIBILITIES

- 4.1 Provide to the Truss Manufacturer the Construction Design Documents and all revisions and supplements thereto.
- 4.2 Review and approve the Truss Placement Plan and each Truss Design Drawing for conformance with the requirements and intent of the Construction Design Documents, and the effect of the Truss Placement Plan and each Truss Design Drawing on other trades involved in the construction of the structure and the effect of the other trades on the Trusses.
- 4.3 Coordinate the review, approval and return of each Truss Design Drawing and the Truss Placement Plan by the Owner and Building Designer.
- 4.4 Provide the approved Truss Design Drawings, approved Truss Placement Plans, and any supplemental information provided by the Truss Manufacturer to the individual or organization responsible for the installation of the Trusses.
- 4.5 Comply with the field storage, handling, installation, permanent bracing, anchorage, connections and field assembly requirements of the Construction Design Documents.
- 4.6 Determine and install the temporary bracing for the structure, including the Trusses.

5.0 TRUSS MANUFACTURER RESPONSIBILITIES

- 5.1 Communicate the design criteria from the Construction Design Documents to the Truss Designer.
- 5.2 Where required by the Construction Design Documents, prepare the Truss Placement Plan, providing as a minimum the location assumed for each Truss based on the Truss Manufacturer's interpretation of the Construction Design Documents.
- 5.3 Submit to the Contractor the Truss Placement Plan, as may be required, and each Truss Design Drawing for review and approval.
- 5.4 Manufacture the Trusses in accordance with the final approved Truss Design Drawings using the quality criteria for Metal Plate Connected Wood Trusses established by the ANSI/TPI 1-1995 "National Design Standard for Metal Plate Connected Wood Truss Construction."

6.0 TRUSS DESIGNER RESPONSIBILITIES

- 6.1 Prepare the Truss Design Drawings in conformance with the requirements set forth in the latest approved edition of ANSI/TPI 1-1995 "National Design Standard for Metal Plate Connected Wood Truss Construction."
- 6.2 For each Truss Design Drawing, set forth as a minimum the following:
 - 6.2.1 Slope or depth, span and spacing;
 - 6.2.2 Location of all joints;
 - 6.2.3 Required bearing widths;
 - 6.2.4 Design loads as applicable:
 - 6.2.4.1 Top chord live load (including snow loads);
 - 6.2.4.2 Top chord dead load;
 - 6.2.4.3 Bottom chord live load;
 - 6.2.4.4 Bottom chord dead load;
 - 6.2.4.5 Concentrated loads and their points of application; and
 - 6.2.4.6 Controlling wind and earthquake loads;
 - 6.2.5 Adjustments to lumber and metal connector plate design values for conditions of use;
 - 6.2.6 Each reaction force and direction;
 - 6.2.7 Metal connector plate type, size, thickness or gage, and the dimensioned location of each metal connector plate except where symmetrically located relative to the joint interface;
 - 6.2.8 Lumber size, species, and grade for each member;
 - 6.2.9 Connection Requirements for:
 - (a) Truss to Truss girder;
 - (b) Truss ply to ply; and
 - (c) Field splices;
 - 6.2.10 Calculated deflection ratio and/or maximum deflection for live and total load;
 - 6.2.11 Maximum axial compression forces in the Truss members to enable the Building Designer to design the size, connections and anchorage of the permanent continuous lateral bracing. Forces may be shown on the Truss Design Drawing or on supplemental documents; and
 - 6.2.12 Required permanent Truss member bracing location.

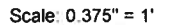
7.0 OTHER RESPONSIBILITIES

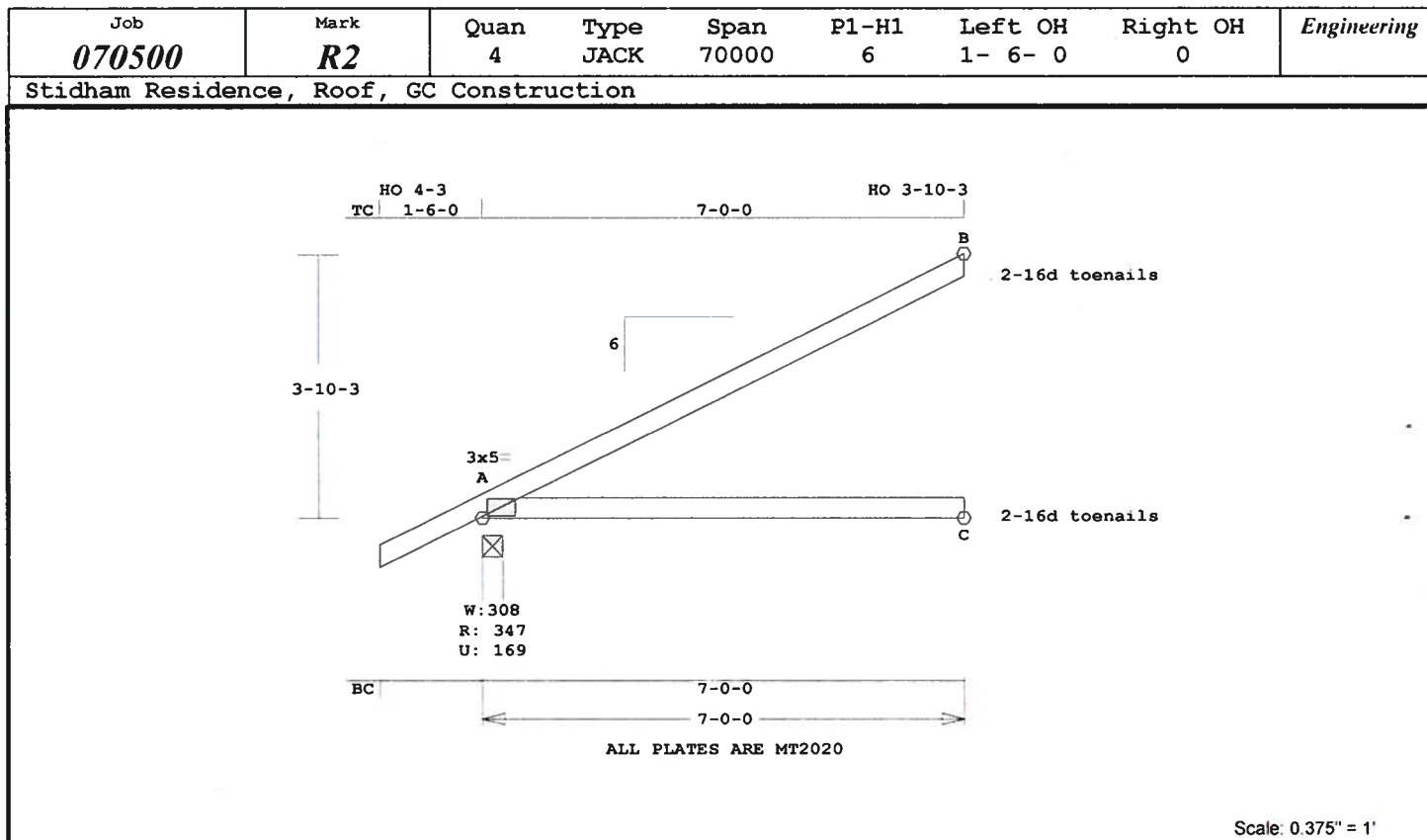
- 7.1 Any party who cuts or damages a truss shall be responsible for securing the engineering required for the repair and for subsequent costs.

Wood Truss Council of America's Objective

WTCA is committed to promoting the common interests of all engaged in the manufacture of wood trusses and related components to ensure growth, continuity, and increased professionalism in our industry. Fundamental to this is promoting the safe, economic, and structurally sound use of trusses in all applications.

Stidham Residence, Roof, GC Construction





Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 31.5 LBS

Online Plus -- Version 21.0.032
RUN DATE: 12-JUN-07

CSI -Size- ----Lumber----
TC 0.72 2x 4 SP-#2
BC 0.70 2x 4 SP-#2

Brace truss as follows:

	O.C.	From	To
TC Cont.	0- 0- 0	7- 0- 0	
BC Cont.	0- 0- 0	7- 0- 0	

psf-Ld Dead Live
TC 7.0 20.0
BC 10.0 0.0
TC+BC 17.0 20.0
Total 37.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	348	170 U	286 R
C	125	59 U	
B	171	152 U	84 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	1.5"	1.5"
B	1.5"	1.5"

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Membr	CSI	P	Lbs	Ax1	CSI-Bnd
-----Top Chords-----					
A -B	0.72	177	C	0.00	0.72
-----Bottom Chords-----					

A -C 0.70 0 T 0.00 0.70

TL Defl -0.18" in A -C L/437
LL Defl -0.08" in A -C L/999
Shear // Grain in A -B 0.32

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A MT20 3.0x 5.0 Ctr Ctr 0.50

For proper installation of
toe-nails, refer to the 2001
National Design Specification
(NDS) for Wood Construction
NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

User-defined wind-exposed BC
regions --From-- ---To---

Max comp. force 177 Lbs

Max tens. force 53 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss
Designer)

Gary Dounson, PE 35054

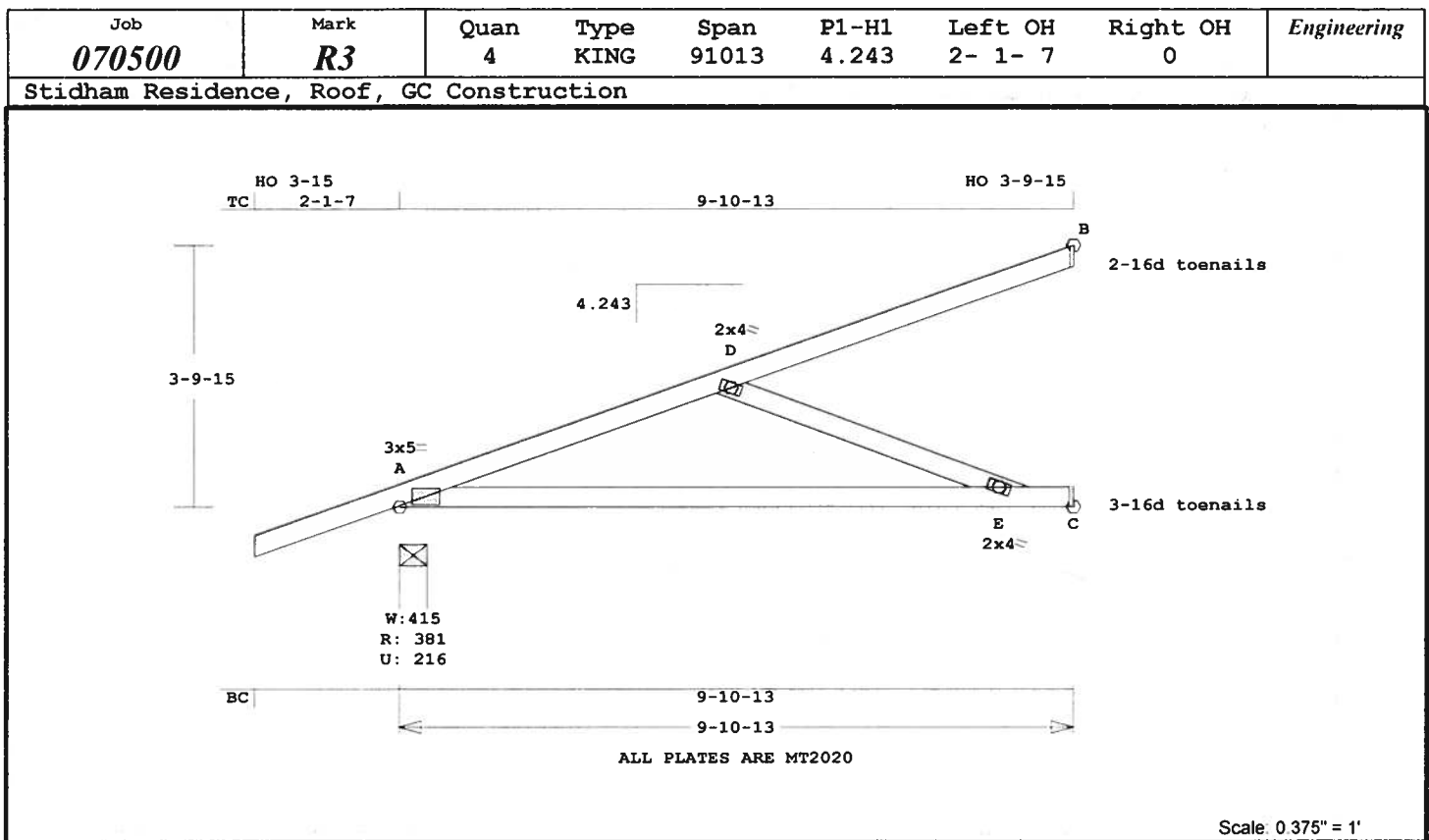
Gary Dounson & Associates,
Inc.

2830 NW 41st Street Suite D

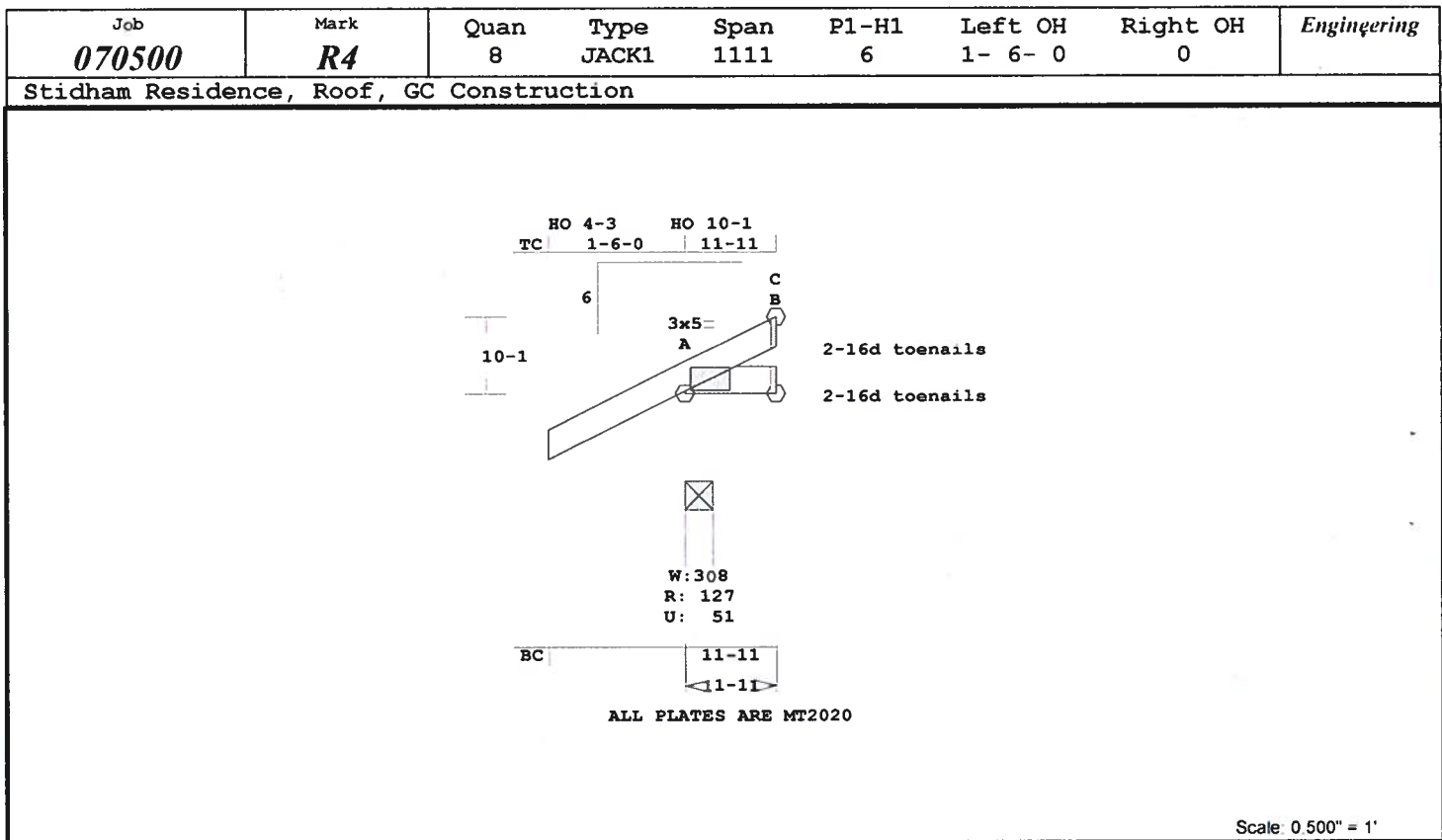
Gainesville, FL 32606

(352)375-8593

CA 5201



Robbins Engineering, Inc./Online Plus™				APPROX. TRUSS WEIGHT: 52.0 LBS			
Online Plus -- Version 21.0.032				16	45	9.9'	Girder King Jack
RUN DATE: 12-JUN-07				BC V	-20	0 0.0'	Loading TC and BC
					22	0 9.9'	Setback 7- 0- 0
CSI -Size- ----Lumber----				OH Loading			
TC	0.65	2x 4	SP-#2	Soffit psf 2.0			
BC	0.83	2x 4	SP-#2	Design checked for 10 psf non-			
WB	0.21	2x 4	SP-#3	concurrent LL on BC.			
Brace truss as follows:				Use properly rated hangers for			
O.C.	From	To		loads framing into girder			
TC Cont.	0- 0- 0	9-10-13		truss.			
BC Cont.	0- 0- 0	9-10-13		Wind Loads - ANSI / ASCE 7-02			
				Truss is designed as			
psf-Ld	Dead	Live		Components and Claddings*			
TC	7.0	20.0		for Exterior zone location.			
BC	10.0	0.0		Wind Speed: 110 mph			
TC+BC	17.0	20.0		Mean Roof Height: 25-0			
Total	37.0	Spacing	24.0"	Exposure Category: B			
Lumber Duration Factor 1.25				Occupancy Factor : 1.00			
Plate Duration Factor 1.25				Building Type: Enclosed			
TC Fb=1.00 Fc=1.00 Ft=1.00				TC Dead Load: 4.0 psf			
BC Fb=1.00 Fc=1.00 Ft=1.00				BC Dead Load: 6.0 psf			
Total Load Reactions (lbs)				User-defined wind-exposed BC			
Jt	Down	Uplift	Horiz-	regions --From-- ---To---			
A	382	216 U	263 R	0- 0- 0 9-10-13			
C	330	159 U	90 R	Max comp. force 745 Lbs			
B	228	179 U		Max tens. force 794 Lbs			
				Quality Control Factor 1.25			
				FABRICATOR NOTES:			
Jt	Brg Size	Required		1. Delegated Engineer (Truss			
A	4.9"	1.5"		Designer)			
C	3.5"	1.5"		Gary Dounson, PE 35054			
B	3.5"	1.5"		Gary Dounson & Associates,			
				Inc.			
LC# 1 Girder Loading				2830 NW 41st Street Suite D			
Dur Fctrs - Lbr 1.25 Plt 1.25				Gainesville, FL 32606			
plf - Dead Live* From To				(352)375-8593			
TC V	14	40	0.0' 9.9'	CA 5201			
BC V	20	0	0.0' 9.9'				
TC V	-14	-40	0.0'				
				For proper installation of			
				toe-nails, refer to the 2001			
				National Design Specification			
				(NDS) for Wood Construction			
				NOTES:			
				Trusses Manufactured by:			
				RIDGWAY ROOF TRUSS			
				Analysis Conforms To:			
				FBC2004			



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 7.3 LBS

Online Plus -- Version 21.0.032
RUN DATE: 12-JUN-07

A -C 0.00 15 T

Quality Control Factor 1.25
FABRICATOR NOTES:

CSI -Size- ----Lumber----
TC 0.00 2x 4 SP-#2
BC 0.00 2x 4 SP-#2

TL Defl 0.00" in A -E L/999
LL Defl 0.00" in A -E L/999
Shear // Grain in A -B 0.04

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201

Brace truss as follows:
O.C. From To
TC Cont. 0- 0- 0 0-11-11
BC Cont. 0- 0- 0 0-11-11

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A MT20 3.0x 5.0 Ctr Ctr 0.47

psf-Ld Dead Live
TC 7.0 20.0
BC 10.0 0.0
TC+BC 17.0 20.0
Total 37.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)
Jt Down Uplift Horiz-
A 127 51 U 53 R
B 17 20 U
C 13 3 U 15 R

Jt Brg Size Required
A 3.5" 1.5"
B 1.5" 1.5"
C 1.5" 1.5"

Plus 8 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Ax1-CSI-Bnd
-----Top Chords-----
A -B 0.00 21 C
-----Bottom Chords-----

For proper installation of
toe-nails, refer to the 2001
National Design Specification
(NDS) for Wood Construction

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Analysis Conforms To:
FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-
concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

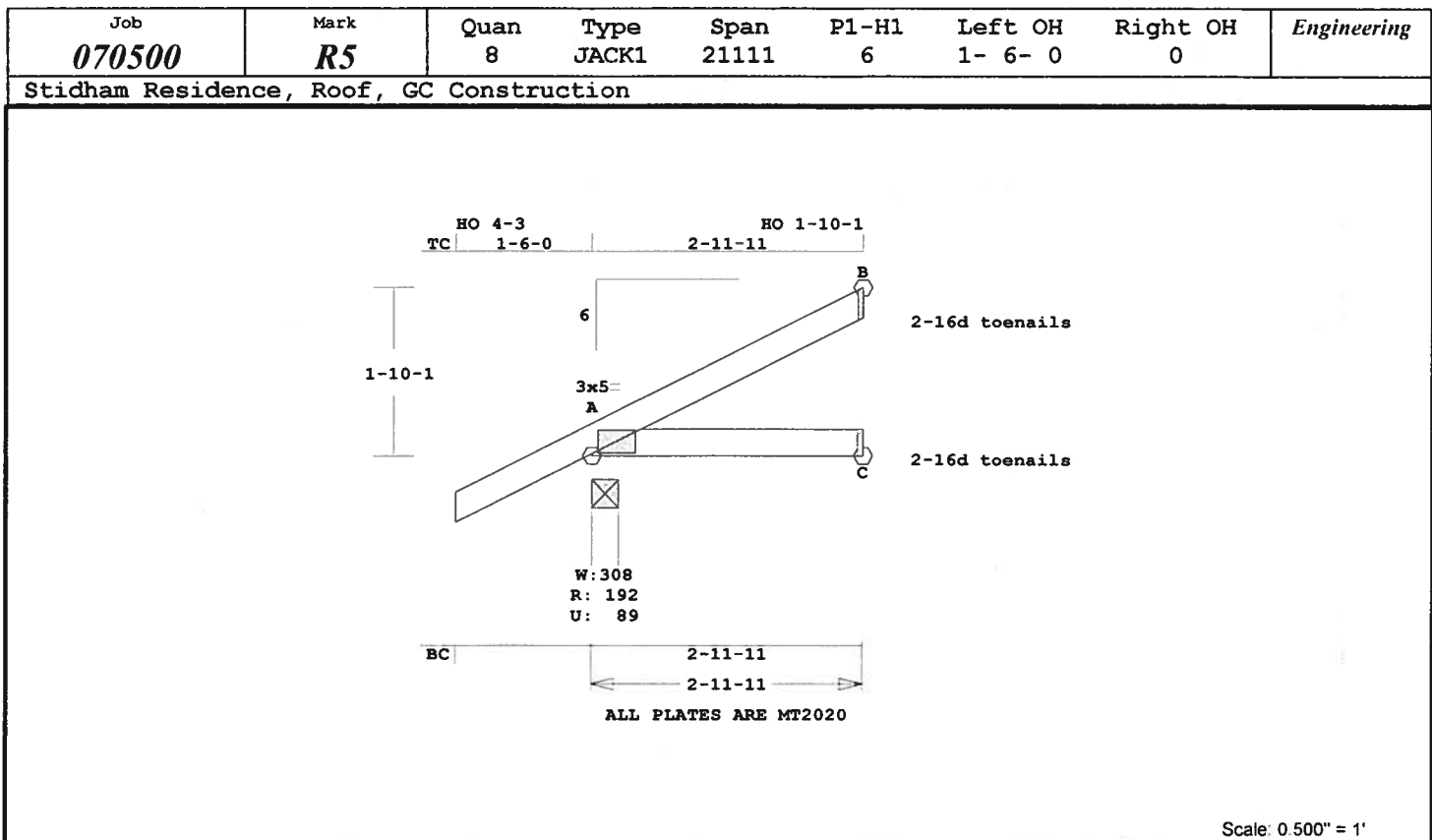
Building Type: Part. Enc

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 21 Lbs

Max tens. force 15 Lbs



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 15.3 LBS

Online Plus -- Version 21.0.032 A -C 0.14 0 T 0.00 0.14 0- 0- 0 2-11-11
 RUN DATE: 12-JUN-07

CSI -Size- ----Lumber-----
 TC 0.14 2x 4 SP-#2
 BC 0.14 2x 4 SP-#2

Brace truss as follows:
 O.C. From To
 TC Cont. 0- 0- 0 2-11-11
 BC Cont. 0- 0- 0 2-11-11

psf-Ld Dead Live
 TC 7.0 20.0
 BC 10.0 0.0
 TC+BC 17.0 20.0
 Total 37.0 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)
 Jt Down Uplift Horiz-
 A 193 89 U 169 R
 C 54 25 U
 B 79 71 U 35 R

Jt Brg Size Required
 A 3.5" 1.5"
 C 3.5" 1.5"
 B 3.5" 1.5"

Plus 8 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Axl-CSI-Bnd
 -----Top Chords-----
 A -B 0.14 98 C 0.00 0.14
 -----Bottom Chords-----

TL Defl 0.00" in A -C L/999
 LL Defl 0.00" in A -C L/999
 Shear // Grain in A -B 0.15

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A MT20 3.0x 5.0 Ctr Ctr 0.47

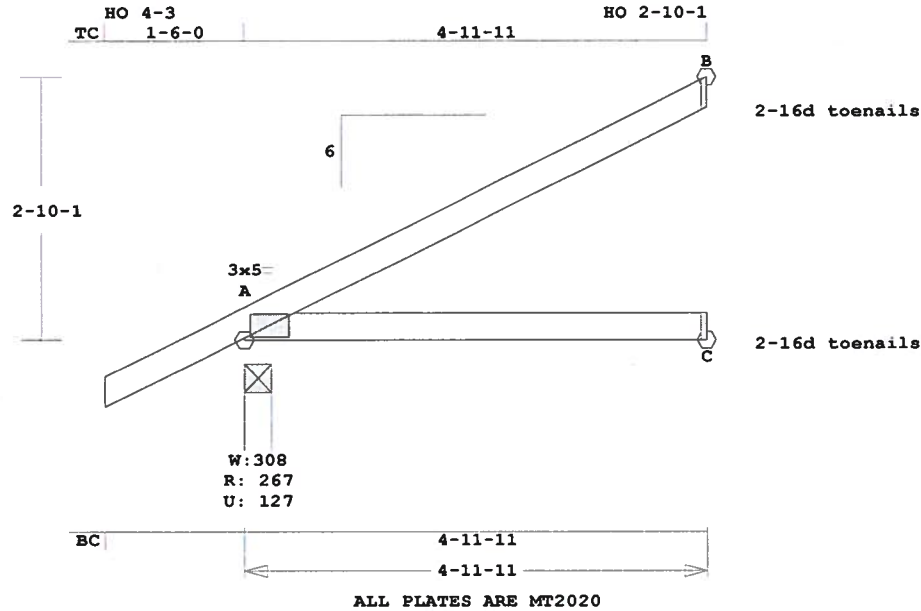
For proper installation of
 toe-nails, refer to the 2001
 National Design Specification
 (NDS) for Wood Construction

NOTES:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2004
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 25-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 User-defined wind-exposed BC
 regions --From-- --To--

Max comp. force 98 Lbs
 Max tens. force 21 Lbs
 Quality Control Factor 1.25
 FABRICATOR NOTES:
 1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352) 375-8593
 CA 5201

Job	Mark	Quan	Type	Span	Pl-H1	Left OH	Right OH	Engineering
070500	R6	8	JACK1	41111	6	1- 6- 0	0	

Stidham Residence, Roof, GC Construction



Scale: 0.500" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 23.3 LBS

Online Plus -- Version 21.0.032 A -C 0.40 0 T 0.00 0.40 0- 0- 0 4-11-11
 RUN DATE: 12-JUN-07

CSI -Size- ----Lumber----
 TC 0.40 2x 4 SP-#2
 BC 0.40 2x 4 SP-#2

Brace truss as follows:
 O.C. From To
 TC Cont. 0- 0- 0 4-11-11
 BC Cont. 0- 0- 0 4-11-11

psf-Ld Dead Live
 TC 7.0 20.0
 BC 10.0 0.0
 TC+BC 17.0 20.0
 Total 37.0 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)
 Jt Down Uplift Horiz-
 A 267 127 U 237 R
 C 91 42 U
 B 128 113 U 59 R

Jt Brg Size Required
 A 3.5" 1.5"
 C 3.5" 1.5"
 B 3.5" 1.5"

Plus 8 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Ax1-CSI-Bnd
 -----Top Chords-----
 A -B 0.40 151 C 0.00 0.40
 -----Bottom Chords-----

TL Defl -0.04" in A -C L/999
 LL Defl -0.02" in A -C L/999
 Shear // Grain in A -B 0.25

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A MT20 3.0x 5.0 Ctr Ctr 0.47

For proper installation of
 toe-nails, refer to the 2001
 National Design Specification
 (NDS) for Wood Construction
 NOTES:

Trusses Manufactured by:
 RIDGWAY ROOF TRUSS

Analysis Conforms To:
 FBC2004

OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02
 Truss is designed as

Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph

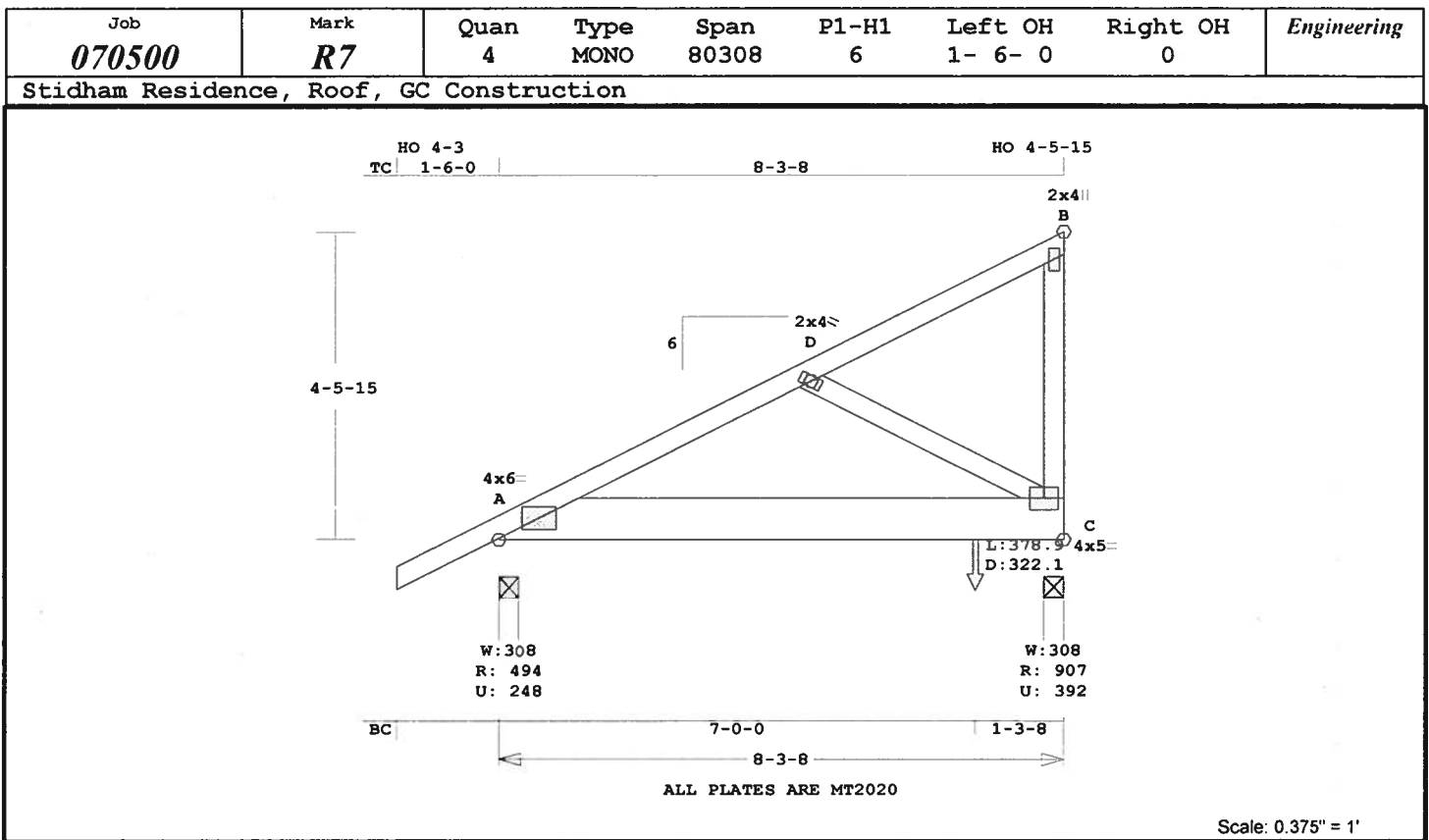
Mean Roof Height: 25-0
 Exposure Category: B

Occupancy Factor : 1.00
 Building Type: Enclosed

TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf

User-defined wind-exposed BC
 regions --From-- ---To---

Max comp. force 151 Lbs
 Max tens. force 36 Lbs
 Quality Control Factor 1.25
 FABRICATOR NOTES:
 1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352) 375-8593
 CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 66.4 LBS
 Online Plus -- Version 21.0.032 Plus 1 UBC LL Load Case(s)
 RUN DATE: 12-JUN-07

CSI	Size	Lumber
TC	0.34	2x 4 SP-#2
BC	0.23	2x 8 SP-2400
WB	0.12	2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	8- 3- 8
BC Cont.	0- 0- 0	8- 3- 8

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.00	Fc=1.00	Ft=1.00
BC Fb=1.00	Fc=1.00	Ft=1.00

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	494	248 U	111 R
C	907	393 U	164 R

Jt	Brg Size	Required
A	3.5"	1.5"
C	3.5"	1.5"

LC# 1 Standard Loading

Dur Fctrs	- Lbr	1.25	Plt	1.25
plf - Dead	Live*	From	To	
TC V	14	40	0.0'	8.3'
BC V	20	0	0.0'	8.3'
BC V	322	379	7.0'	CL-LB

Plus 8 Wind Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -D	0.34	276 T	0.04	0.30
D -B	0.30	64 C	0.00	0.30
-----Bottom Chords-----				
A -C	0.23	313 C	0.01	0.22
-----Webs-----				
D -C	0.12	481 T		
C -B	0.12	135 T	WindLd	

TL Defl -0.05" in A -C L/999
 LL Defl -0.03" in A -C L/999
 Shear // Grain in A -C 0.39

Plates for each ply each face.

Plate	MT20	20 Ga	Gross Area		
Plate - MT20	20 Ga <td>Gross Area</td> <td></td>	Gross Area			
Jt Type	Plt Size	X	Y	JSI	
A	MT20	4.0x 6.0	Ctr	Ctr	0.68
D	MT20	2.0x 4.0	Ctr	Ctr	0.37
B	MT20	2.0x 4.0	Ctr	Ctr	0.38
C	MT20	4.0x 5.0	Ctr	Ctr	0.59

NOTES:

Trusses Manufactured by:
 RIDGWAY ROOF TRUSS

Analysis Conforms To:
 FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*
 for Exterior zone location.

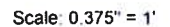
Wind Speed: 110 mph

Mean Roof Height: 25-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 User-defined wind-exposed BC regions --From-- --To--
 0- 0- 0 8- 3- 8
 Max comp. force 313 Lbs
 Max tens. force 481 Lbs
 Quality Control Factor 1.25

FABRICATOR NOTES:

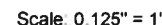
1. Delegated Engineer (Truss Designer)
 Gary Dounson, PE 35054
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 2830 NW 41st Street Suite D
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 CA 5201

Stidham Residence, Roof, GC Construction



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Stidham Residence, Roof, GC Construction

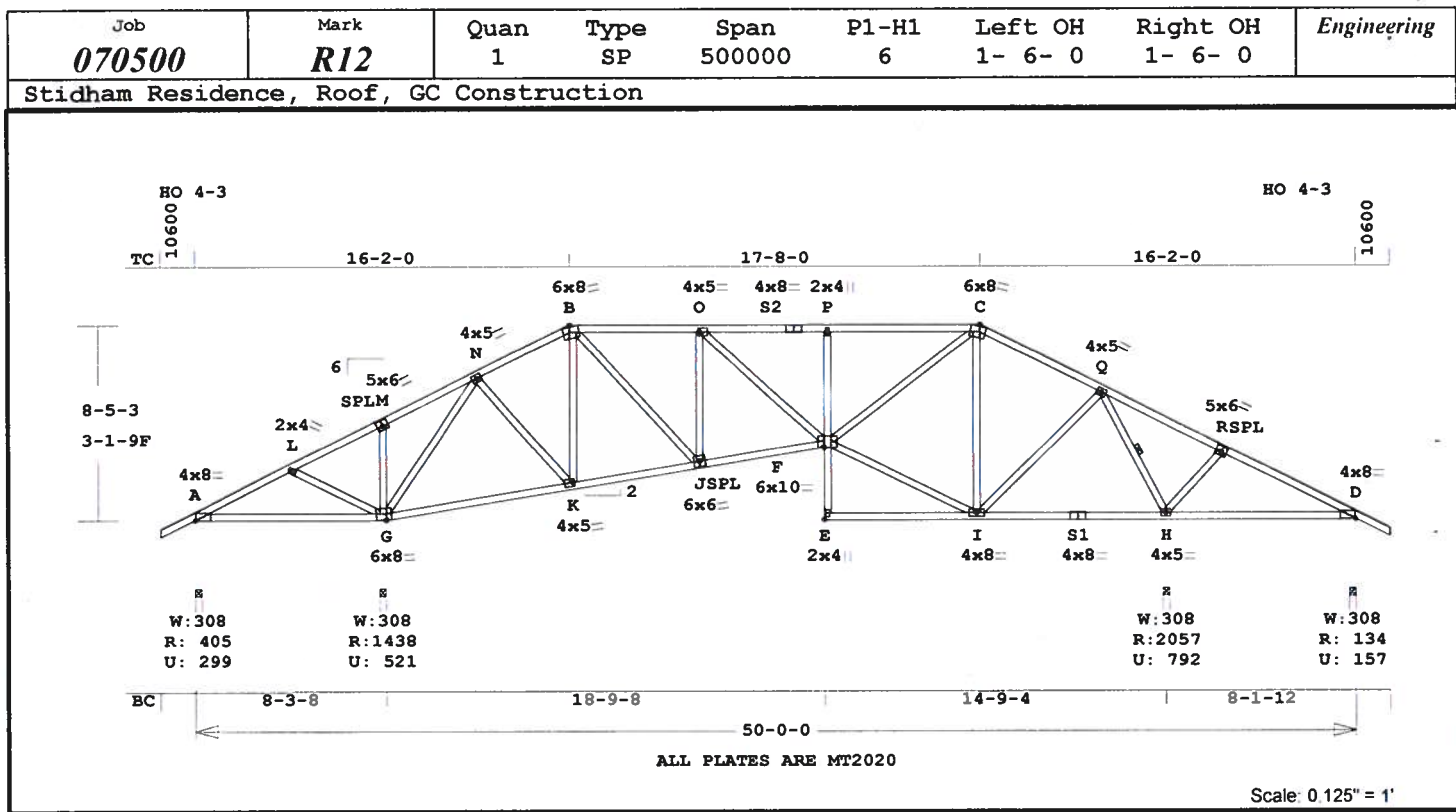


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Stidham Residence, Roof, GC Construction



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Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 383.5 LBS
 Online Plus -- Version 21.0.032
 RUN DATE: 13-JUN-07

CSI -Size- ----Lumber-----
 TC 0.47 2x 4 SP-#2
 BC 0.69 2x 4 SP-#2
 CW 0.11 2x 4 SP-#3
 WB 0.98 2x 4 SP-#3

Brace truss as follows:
 O.C. From To
 TC Cont. 0- 0- 0 50- 0- 0
 BC Cont. 0- 0- 0 50- 0- 0
 WB 1 rows CLB on Q -H
 Attach CLB with (2)-10d nails
 at each web.

psf-Ld Dead Live
 TC 7.0 20.0
 BC 10.0 0.0
 TC+BC 17.0 20.0
 Total 37.0 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)
 Jt Down Uplift Horiz-
 A 405 299 U 189 R
 G 1439 521 U
 H 2058 793 U
 D 134 158 U 189 R

Jt Brg Size Required
 A 3.5" 1.5"
 G 3.5" 1.5"
 H 3.5" 2.2"
 D 3.5" 1.5"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Membr	CSI	P	Lbs	Ax1	CSI-Bnd
-----Top Chords-----					
A -L	0.34	598	T	0.07	0.27
L -M	0.27	355	T	0.04	0.23
M -N	0.17	458	T	0.04	0.13
N -B	0.22	1099	C	0.09	0.13
B -O	0.25	1482	C	0.01	0.24
O -S2	0.25	1687	C	0.11	0.14

S2-P	0.27	1687	C	0.11	0.16
P -C	0.28	1682	C	0.01	0.27
C -Q	0.29	718	C	0.06	0.23
Q -R	0.47	880	T	0.13	0.34
R -D	0.46	671	T	0.12	0.34
-----Bottom Chords-----					
A -G	0.41	485	C	0.05	0.36
G -K	0.69	706	T	0.11	0.58
K -J	0.30	1003	T	0.10	0.20
J -F	0.37	1501	T	0.25	0.12
E -I	0.32	9	C	0.00	0.32
I -S1	0.32	90	T	0.00	0.32
S1-H	0.41	90	T	0.00	0.41
H -D	0.41	580	C	0.00	0.41
-----Chord Webs-----					
E -F	0.08	97	T	0.02	0.06
F -P	0.11	318	C	0.00	0.11
-----Webs-----					
L -G	0.11	415	T		
G -M	0.05	209	C		
G -N	0.98	1260	C		
N -K	0.13	434	T		
K -B	0.20	295	C		
B -J	0.23	727	T		
J -O	0.25	485	C		
O -F	0.08	276	T		
F -C	0.41	1304	T		
F -I	0.22	710	T		
I -C	0.75	781	C		
I -Q	0.27	865	T		
Q -H	0.38	1823	C		
H -R	0.08	352	T		

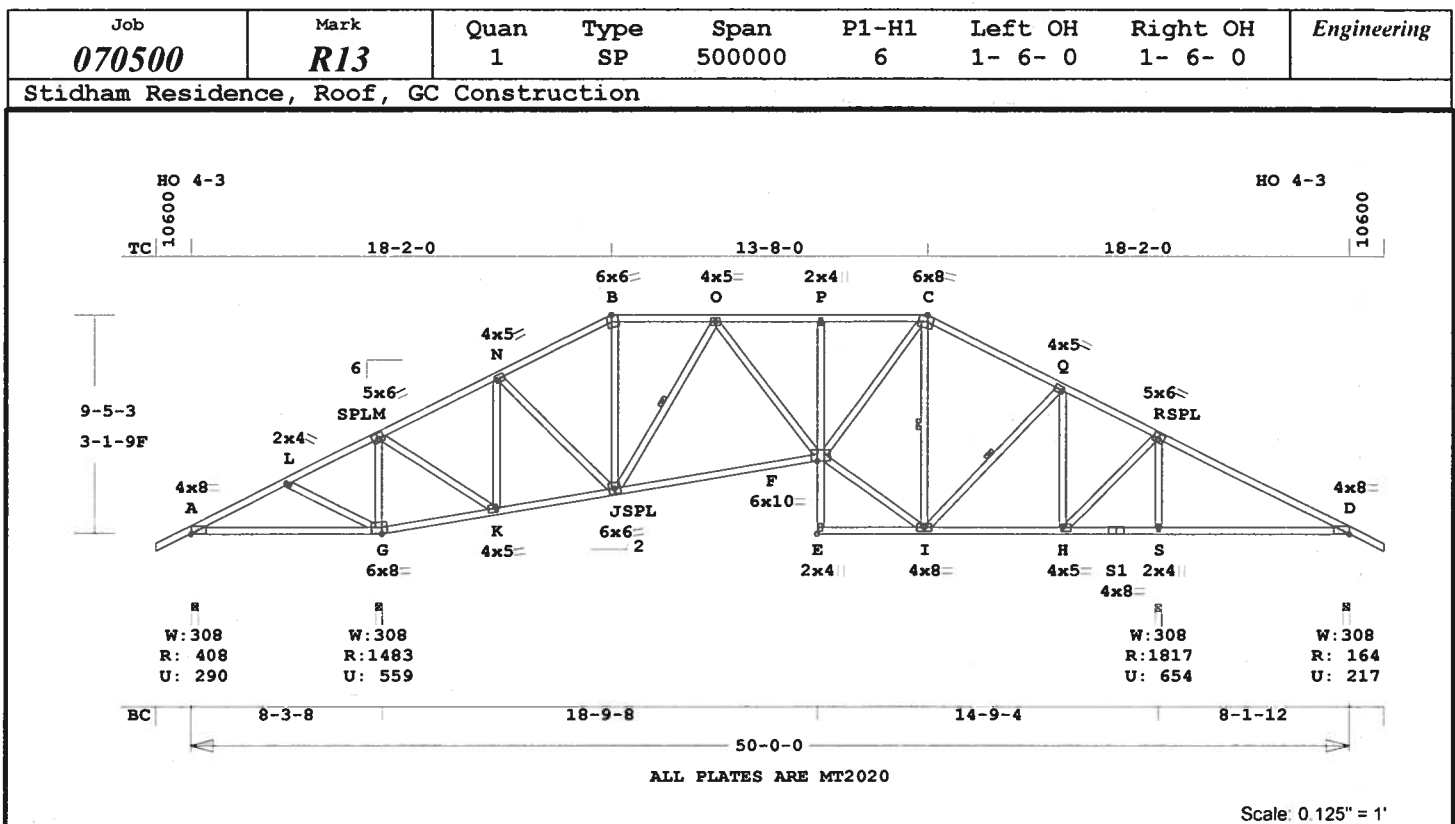
TL Defl -0.26" in A -G L/358
 LL Defl -0.11" in A -G L/817
 Hz Disp LL DL TL
 Jt H 0.06" 0.05" 0.11"
 Shear // Grain in P -C 0.22

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A MT20 4.0x 8.0 0.8 0.3 0.79
 L MT20 2.0x 4.0 Ctr Ctr 0.54
 M MT20 5.0x 6.0-0.2 0.5 0.86
 N MT20 4.0x 5.0 Ctr Ctr 0.61
 B MT20 6.0x 8.0 0.9-3.8 0.75
 O MT20 4.0x 5.0 Ctr Ctr 0.60
 S2 MT20 4.0x 8.0 Ctr-0.2 0.75
 P MT20 2.0x 4.0 Ctr Ctr 0.29

C	MT20	6.0x 8.0-0.9-3.8	0.77
Q	MT20	4.0x 5.0 Ctr Ctr	0.71
R	MT20	5.0x 6.0 0.2 0.5	0.86
D	MT20	4.0x 8.0-0.8 0.3	0.79
G	MT20	6.0x 8.0-1.3 3.3	0.79
K	MT20	4.0x 5.0 Ctr Ctr	0.61
J	MT20	6.0x 6.0 Ctr Ctr	0.87
F	MT20	6.0x10.0 Ctr	1.2 0.75
E	MT20	2.0x 4.0 Ctr Ctr	0.67
I	MT20	4.0x 8.0 Ctr Ctr	0.55
S1	MT20	4.0x 8.0 Ctr	0.2 0.75
H	MT20	4.0x 5.0 Ctr Ctr	0.56

NOTES:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2004
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 25-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 User-defined wind-exposed BC
 regions --From-- --To--
 0- 0- 0 8- 3- 8
 41-10- 4 50- 0- 0

Max comp. force 1823 Lbs
 Max tens. force 1501 Lbs
 Quality Control Factor 1.25
 FABRICATOR NOTES:
 1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 398.0 LBS
 Online Plus -- Version 21.0.032
 RUN DATE: 13-JUN-07

CSI -Size- ----Lumber-----
 TC 0.58 2x 4 SP-#2
 BC 0.59 2x 4 SP-#2
 CW 0.38 2x 4 SP-#3
 WB 0.45 2x 4 SP-#3

Brace truss as follows:

O.C. From To
 TC Cont. 0- 0- 0 50- 0- 0
 BC Cont. 0- 0- 0 50- 0- 0
 WB 1 rows CLB on J -O
 WB 1 rows CLB on I -C
 WB 1 rows CLB on I -Q
 Attach CLB with (2)-10d nails
 at each web.

psf-Ld Dead Live
 TC 7.0 20.0
 BC 10.0 0.0
 TC+BC 17.0 20.0
 Total 37.0 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)
 Jt Down Uplift Horiz-
 A 408 290 U 213 R
 G 1483 560 U
 S 1818 655 U
 D 165 217 U 213 R

Jt Brg Size Required
 A 3.5" 1.5"
 G 3.5" 1.6"
 S 3.5" 1.9"
 D 3.5" 1.5"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Ax1-CSI-Bnd
 -----Top Chords-----
 A -L 0.33 581 T 0.07 0.26
 L -M 0.26 351 T 0.04 0.22
 M -N 0.31 938 C 0.08 0.23
 N -B 0.32 1243 C 0.09 0.23
 B -O 0.19 1107 C 0.09 0.10

O -P	0.21	1556	C	0.11	0.10
P -C	0.25	1548	C	0.11	0.14
C -Q	0.37	934	C	0.08	0.29
Q -R	0.50	562	C	0.00	0.50
R -D	0.58	386	T	0.08	0.50
-----Bottom Chords-----					
A -G	0.42	469	C	0.05	0.37
G -K	0.59	179	C	0.00	0.59
K -J	0.45	861	T	0.09	0.36
J -F	0.49	1407	T	0.13	0.36
E -I	0.20	38	C	0.00	0.20
I -H	0.24	487	T	0.04	0.20
H -SI	0.10	333	C	0.01	0.09
SI -S	0.35	333	C	0.00	0.35
S -D	0.37	333	C	0.00	0.37
-----Chord-Webs-----					
E -F	0.38	60	T	0.01	0.37
F -P	0.27	214	C	0.00	0.27
-----Webs-----					
L -G	0.11	402	T		
G -M	0.32	1167	C		
M -K	0.30	965	T		
K -N	0.34	678	C		
N -J	0.11	370	T		
J -B	0.13	359	T		
J -O	0.17	558	C	1 Br	
O -F	0.08	276	T		
F -C	0.37	1179	T		
F -I	0.33	1043	T		
I -C	0.28	825	C	1 Br	
I -Q	0.15	492	T	1 Br	
H -Q	0.43	764	C		
H -R	0.37	1156	T		
S -R	0.45	1613	C		

TL Defl -0.27" in A -G L/338
 LL Defl -0.12" in A -G L/764
 Hz Disp LL DL TL
 Jt S 0.06" 0.05" 0.11"
 Shear // Grain in R -D 0.27

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A MT20 4.0x 8.0 0.8 0.3 0.79
 L MT20 2.0x 4.0 Ctr Ctr 0.54
 M MT20 5.0x 6.0-0.2 0.5 0.86
 N MT20 4.0x 5.0 Ctr Ctr 0.67
 B MT20 6.0x 6.0 0.9-3.6 0.75
 O MT20 4.0x 5.0 Ctr Ctr 0.54
 P MT20 2.0x 4.0 Ctr Ctr 0.29

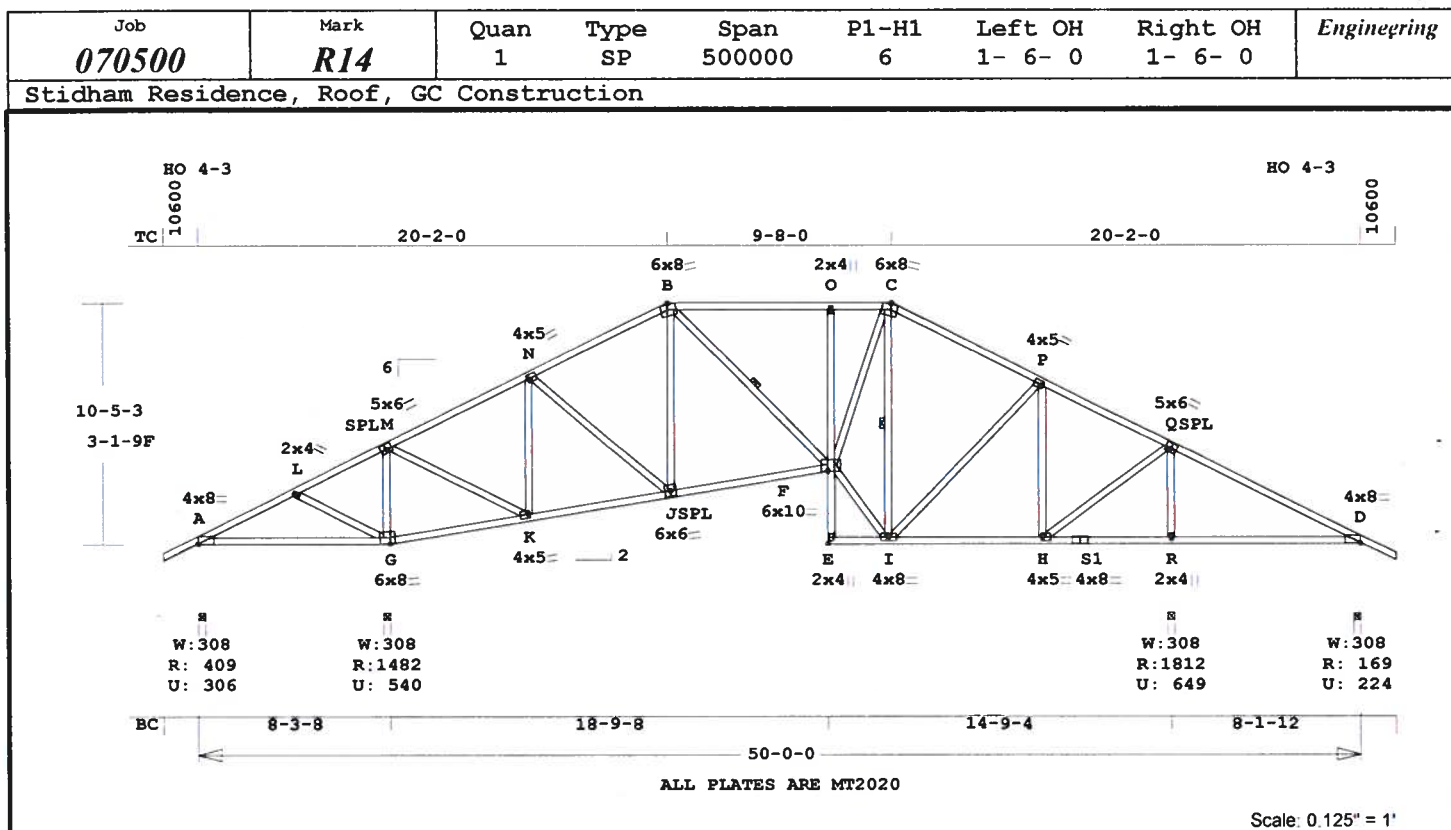
C	MT20	6.0x 8.0-0.9-3.8	0.75
Q	MT20	4.0x 5.0 Ctr Ctr	0.67
R	MT20	5.0x 6.0 0.2 0.5	0.86
D	MT20	4.0x 8.0-0.8 0.3	0.79
G	MT20	6.0x 8.0-1.3 3.3	0.79
K	MT20	4.0x 5.0 Ctr Ctr	0.61
J	MT20	6.0x 6.0 Ctr Ctr	0.87
F	MT20	6.0x10.0 Ctr	1.2 0.75
E	MT20	2.0x 4.0 Ctr Ctr	0.67
I	MT20	4.0x 8.0 Ctr Ctr	0.51
H	MT20	4.0x 5.0 Ctr Ctr	0.60
SI	MT20	4.0x 8.0 Ctr Ctr	0.2 0.75
S	MT20	2.0x 4.0 Ctr Ctr	0.54

NOTES:

Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2004
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 25-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 User-defined wind-exposed BC
 regions --From-- --To--
 0- 0- 0 8- 3- 8
 41-10- 4 50- 0- 0
 Max comp. force 1613 Lbs
 Max tens. force 1407 Lbs
 Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 398.6 LBS
 Online Plus -- Version 21.0.032
 RUN DATE: 13-JUN-07

CSI -Size- ----Lumber-----
 TC 0.57 2x 4 SP-#2
 BC 0.58 2x 4 SP-#2
 CW 0.20 2x 4 SP-#3
 WB 0.48 2x 4 SP-#3

Brace truss as follows:
 O.C. From To
 TC Cont. 0- 0- 0 50- 0- 0
 BC Cont. 0- 0- 0 50- 0- 0
 WB 1 rows CLB on B -F
 WB 1 rows CLB on I -C
 Attach CLB with (2)-10d nails
 at each web.

psf-Ld Dead Live
 TC 7.0 20.0
 BC 10.0 0.0
 TC+BC 17.0 20.0
 Total 37.0 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)
 Jt Down Uplift Horiz-
 A 409 307 U 237 R
 G 1482 541 U
 R 1813 649 U
 D 169 225 U 237 R

Jt Brg Size Required
 A 3.5" 1.5"
 G 3.5" 1.6"
 R 3.5" 1.9"
 D 3.5" 1.5"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Axl-CSt-Bnd
 -----Top Chords-----
 A -L 0.35 621 T 0.08 0.27
 L -M 0.26 399 T 0.05 0.21
 M -N 0.40 1049 C 0.09 0.31
 N -B 0.41 1251 C 0.10 0.31
 B -O 0.39 1342 C 0.02 0.37

O -C	0.30	1337	C	0.01	0.29
C -P	0.42	954	C	0.09	0.33
P -Q	0.50	722	C	0.07	0.43
Q -D	0.57	377	T	0.07	0.50

-----Bottom Chords-----

A -G	0.43	504	C	0.05	0.38
G -K	0.58	165	C	0.00	0.58
K -J	0.36	959	T	0.10	0.26
J -F	0.37	1129	T	0.18	0.19
E -I	0.22	14	C	0.00	0.22
I -H	0.28	637	T	0.06	0.22
H -S1	0.15	325	C	0.00	0.15
S1 -R	0.35	325	C	0.00	0.35
R -D	0.37	325	C	0.00	0.37

-----Chord-Webs-----

E -F	0.14	11	C	0.00	0.14
F -O	0.20	275	C	0.04	0.16

-----Webs-----

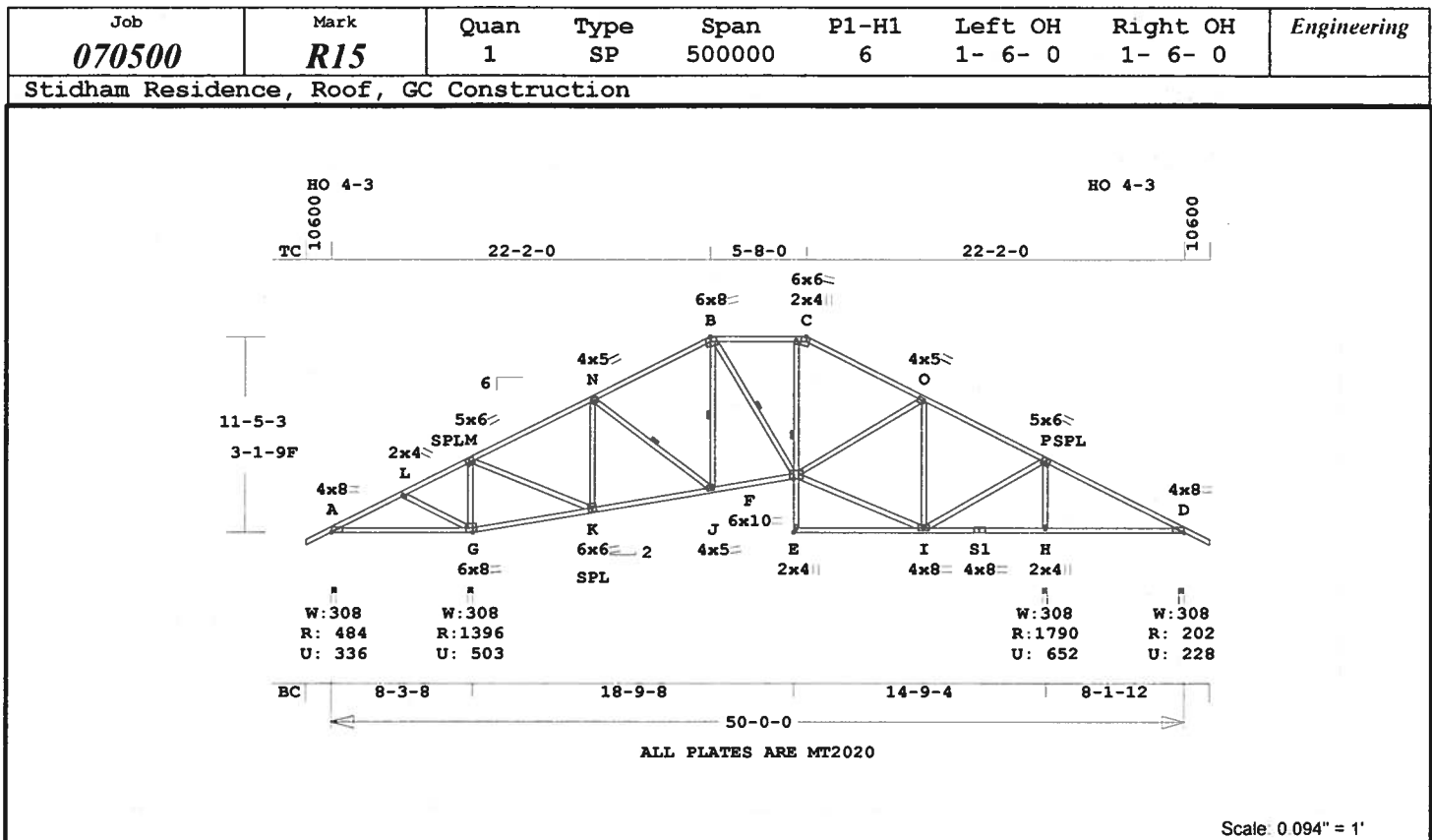
L -G	0.10	389	T		
G -M	0.33	1186	C		
M -K	0.32	1012	T		
K -N	0.32	573	C		
N -J	0.07	226	T		
J -B	0.04	173	T		
B -F	0.10	321	T	1 Br	
F -C	0.46	1459	T		
F -I	0.44	1388	T		
I -C	0.48	1173	C	1 Br	
I -P	0.09	298	T		
H -P	0.44	637	C		
H -Q	0.38	1204	T		
R -Q	0.45	1610	C		

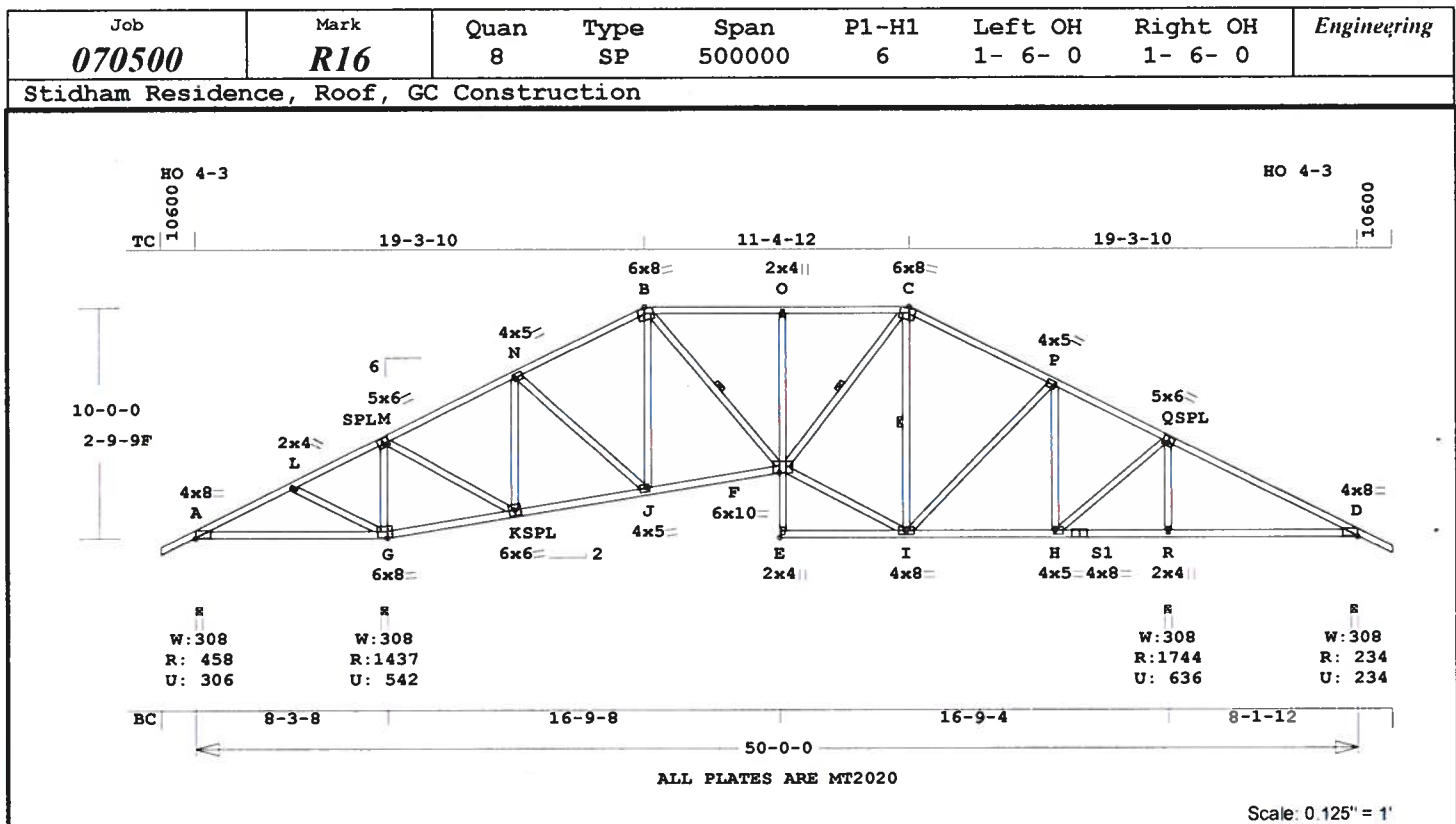
TL Defl -0.28" in A -G L/334
 LL Defl -0.12" in A -G L/759
 Hz Disp LL DL TL
 Jt R 0.07" 0.06" 0.13"
 Shear // Grain in Q -D 0.27

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A MT20 4.0x 8.0 0.8 0.3 0.79
 L MT20 2.0x 4.0 Ctr Ctr 0.54
 M MT20 5.0x 6.0-0.2 0.5 0.86
 N MT20 4.0x 5.0 Ctr Ctr 0.67
 B MT20 6.0x 8.0 0.9-3.8 0.75
 O MT20 2.0x 4.0 Ctr Ctr 0.29
 C MT20 6.0x 8.0-0.9-3.8 0.90
 P MT20 4.0x 5.0 Ctr Ctr 0.67

Q	MT20	5.0x 6.0	0.2 0.5	0.86
D	MT20	4.0x 8.0	-0.8 0.3	0.79
G	MT20	6.0x 8.0	-1.3 3.3	0.79
K	MT20	4.0x 5.0	Ctr Ctr	0.61
J	MT20	6.0x 6.0	Ctr Ctr	0.87
F	MT20	6.0x10.0	Ctr 1.2	0.94
E	MT20	2.0x 4.0	Ctr Ctr	0.67
I	MT20	4.0x 8.0	Ctr Ctr	0.58
H	MT20	4.0x 5.0	Ctr Ctr	0.60
S1	MT20	4.0x 8.0	Ctr 0.2	0.75
R	MT20	2.0x 4.0	Ctr Ctr	0.54

NOTES:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2004
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 25-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 User-defined wind-exposed BC
 regions --From-- ---To---
 0- 0- 0 8- 3- 8
 41-10- 4 50- 0- 0
 Max comp. force 1610 Lbs
 Max tens. force 1459 Lbs
 Quality Control Factor 1.25
 FABRICATOR NOTES:
 1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201





Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 396.5 LBS

Online Plus -- Version 21.0.032
RUN DATE: 13-JUN-07

CSI	Size	Lumber
TC	0.54	2x 4 SP-#2
BC	0.58	2x 4 SP-#2
CW	0.13	2x 4 SP-#3
WB	0.43	2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	50- 0- 0
BC Cont.	0- 0- 0	50- 0- 0
WB 1 rows CLB on B -F		
WB 1 rows CLB on F -C		
WB 1 rows CLB on I -C		

Attach CLB with (2)-10d nails at each web.

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15 Fc=1.10 Ft=1.10		
BC Fb=1.10 Fc=1.10 Ft=1.10		

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz
A	458	307 U	226 R
G	1437	543 U	
R	1745	637 U	
D	234	235 U	227 R

Jt	Brg Size	Required
A	3.5"	1.5"
G	3.5"	1.5"
R	3.5"	1.9"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
A	-L	0.34	626 T	0.08 0.26
L	-M	0.27	401 T	0.05 0.22
M	-N	0.36	1085 C	0.08 0.28
N	-B	0.38	1311 C	0.10 0.28

Member	Size	Length	Area	Weight
B -O	0.25	1462	C	0.01 0.24
O -C	0.26	1460	C	0.11 0.15
C -P	0.40	1007	C	0.09 0.31
P -Q	0.49	745	C	0.00 0.49
Q -D	0.54	238	T	0.05 0.49
Bottom Chords				
A -G	0.43	508	C	0.06 0.37
G -K	0.58	170	T	0.01 0.57
K -J	0.31	991	T	0.16 0.15
J -F	0.34	1183	T	0.19 0.15
E -I	0.24	11	C	0.00 0.24
I -H	0.30	655	T	0.06 0.24
H -S1	0.11	202	C	0.00 0.11
S1 -R	0.34	202	C	0.00 0.34
R -D	0.35	202	C	0.00 0.35
Chord-Webs				
E -F	0.11	77	T	0.02 0.09
F -O	0.13	307	C	0.05 0.08
Webs				
L -G	0.10	394	T	
G -M	0.32	1153	C	
M -K	0.31	966	T	
K -N	0.32	590	C	
N -J	0.08	259	T	
J -B	0.05	122	T	
B -F	0.14	454	T	1 Br
F -C	0.29	908	T	1 Br
F -I	0.32	1017	T	
I -C	0.22	581	C	1 Br
I -P	0.11	344	T	
H -P	0.41	647	C	
H -Q	0.35	1119	T	
R -Q	0.43	1544	C	

Defl	Value	Location	Notes
TL Defl	-0.27"	in A -G	L/338
LL Defl	-0.12"	in A -G	L/764
Hz Disp	LL	DL	TL
Jt R	0.05"	0.04"	0.09"
Shear	// Grain	in Q -D	0.27

Plates for each ply each face.

Plate	MT20	20 Ga	Gross Area
Plate - MT20	20	Ga	Gross Area
Plate - MT2H	20	Ga	Gross Area

Jt Type	Plt Size	X	Y	JSI
A	MT20	4.0x 8.0	0.8 0.3	0.79
L	MT20	2.0x 4.0	Ctr Ctr	0.54
M	MT20	5.0x 6.0	0.2 0.5	0.86
N	MT20	4.0x 5.0	Ctr Ctr	0.67
B	MT20	6.0x 8.0	0.9-3.8	0.75
O	MT20	2.0x 4.0	Ctr Ctr	0.29
C	MT20	6.0x 8.0	0.9-3.8	0.75

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor: 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

User-defined wind-exposed BC

regions --From-- --To--

0- 0- 0 8- 3- 8

41-10- 4 50- 0- 0

Max comp. force 1544 Lbs

Max tens. force 1183 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss

Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates,

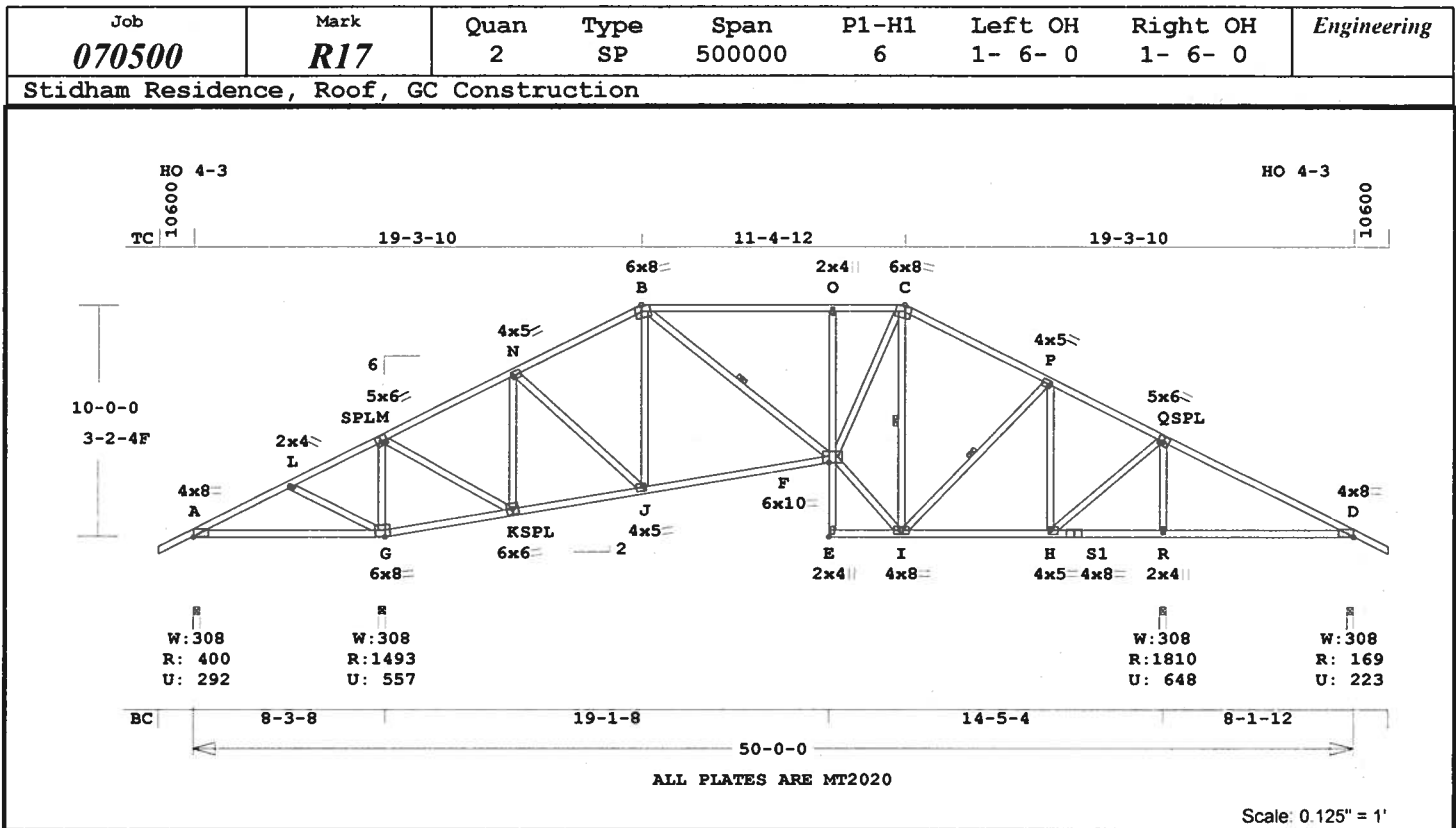
Inc.

2830 NW 41st Street Suite D

Gainesville, FL 32606

(352)375-8593

CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 393.5 LBS
 Online Plus -- Version 21.0.032
 RUN DATE: 13-JUN-07

CSI -Size- ----Lumber-----
 TC 0.57 2x 4 SP-#2
 BC 0.59 2x 4 SP-#2
 CW 0.27 2x 4 SP-#3
 WB 0.45 2x 4 SP-#3

Brace truss as follows:

O.C. From To
 TC Cont. 0- 0- 0 50- 0- 0
 BC Cont. 0- 0- 0 50- 0- 0
 WB 1 rows CLB on B -F
 WB 1 rows CLB on I -C
 WB 1 rows CLB on I -P
 Attach CLB with (2)-10d nails
 at each web.

psf-Ld Dead Live
 TC 7.0 20.0
 BC 10.0 0.0
 TC+BC 17.0 20.0
 Total 37.0 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (lbs)

Jt Down Uplift Horiz-
 A 401 293 U 226 R
 G 1493 558 U
 R 1810 648 U
 D 170 223 U 227 R

Jt Brg Size Required
 A 3.5" 1.5"
 G 3.5" 1.6"
 R 3.5" 1.9"
 D 3.5" 1.5"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Ax1-CSI-Bnd
 -----Top Chords-----
 A -L 0.34 589 T 0.07 0.27
 L -M 0.26 364 T 0.04 0.22
 M -N 0.34 992 C 0.08 0.26
 N -B 0.36 1249 C 0.10 0.26

B -O 0.57 1446 C 0.04 0.53
 O -C 0.45 1439 C 0.01 0.44
 C -P 0.41 948 C 0.09 0.32
 P -Q 0.50 660 C 0.00 0.50
 Q -D 0.57 377 T 0.07 0.50
 -----Bottom Chords-----
 A -G 0.42 476 C 0.04 0.38
 G -K 0.59 177 C 0.00 0.59
 K -J 0.42 907 T 0.10 0.32
 J -F 0.43 1132 T 0.11 0.32
 E -I 0.20 28 C 0.00 0.20
 I -H 0.25 579 T 0.05 0.20
 H -S1 0.12 326 C 0.01 0.11
 S1 -R 0.34 326 C 0.00 0.34
 R -D 0.37 326 C 0.00 0.37
 -----Chord Webs-----
 E -F 0.27 16 T 0.00 0.27
 F -O 0.26 354 C 0.05 0.21
 -----Webs-----
 L -G 0.10 394 T
 G -M 0.33 1186 C
 M -K 0.32 997 T
 K -N 0.34 621 C
 N -J 0.09 304 T
 J -B 0.06 149 T
 B -F 0.13 417 T 1 Br
 F -C 0.45 1409 T
 F -I 0.40 1256 T
 I -C 0.40 1058 C 1 Br
 I -P 0.11 373 T 1 Br
 H -P 0.44 688 C
 H -Q 0.37 1182 T
 R -Q 0.45 1608 C

TL Defl -0.28" in A -G L/335
 LL Defl -0.12" in A -G L/761
 Hz Disp LL DL TL
 Jt R 0.06" 0.05" 0.12"
 Shear // Grain in B -O 0.30

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A MT20 4.0x 8.0 0.8 0.3 0.79
 L MT20 2.0x 4.0 Ctr Ctr 0.54
 M MT20 5.0x 6.0-0.2 0.5 0.86
 N MT20 4.0x 5.0 Ctr Ctr 0.67
 B MT20 6.0x 8.0 0.9-3.8 0.76
 O MT20 2.0x 4.0 Ctr Ctr 0.29
 C MT20 6.0x 8.0-0.9-3.8 0.82

P MT20 4.0x 5.0 Ctr Ctr 0.67
 Q MT20 5.0x 6.0 0.2 0.5 0.86
 D MT20 4.0x 8.0-0.8 0.3 0.79
 G MT20 6.0x 8.0-1.3 3.3 0.79
 K MT20 6.0x 6.0 Ctr Ctr 0.87
 J MT20 4.0x 5.0 Ctr Ctr 0.61
 F MT20 6.0x10.0 Ctr 1.2 0.84
 E MT20 2.0x 4.0 Ctr Ctr 0.67
 I MT20 4.0x 8.0 Ctr Ctr 0.52
 H MT20 4.0x 5.0 Ctr Ctr 0.60
 S1 MT20 4.0x 8.0 Ctr 0.2 0.75
 R MT20 2.0x 4.0 Ctr Ctr 0.54

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-
 concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*
 for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

User-defined wind-exposed BC

regions --From-- --To--

0- 0- 0 8- 3- 8

41-10- 4 50- 0- 0

Max comp. force 1608 lbs

Max tens. force 1409 lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss

Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates,

Inc.

2830 NW 41st Street Suite D

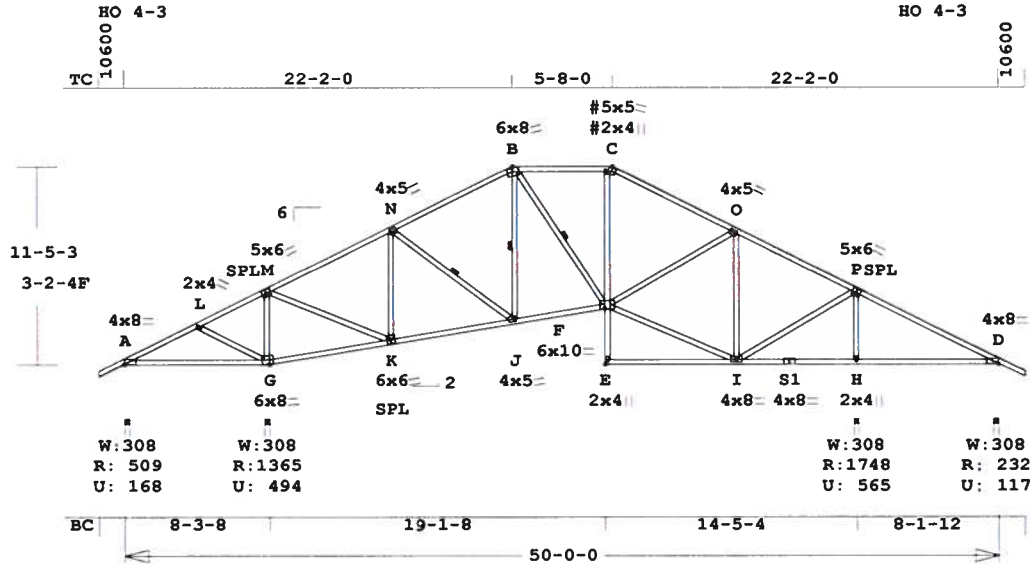
Gainesville, FL 32606

(352)375-8593

CA 5201

Job 070500	Mark R18	Quan 1	Type SP	Span 500000	Pl-H1 6	Left OH 1- 6- 0	Right OH 1- 6- 0	Engineering
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Stidham Residence, Roof, GC Construction



ALL PLATES ARE MT2020, # = PLATE SELECTED IN PLATE MONITOR

Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 393.1 LBS

Online Plus -- Version 21.0.032
RUN DATE: 13-JUN-07

CSI -Size- ---Lumber---
TC 0.61 2x 4 SP-#2
BC 0.57 2x 4 SP-#2
CW 0.56 2x 4 SP-#3
WB 0.68 2x 4 SP-#3

Brace truss as follows:

O.C. From To
TC Cont. 0- 0- 0 50- 0- 0
BC Cont. 0- 0- 0 50- 0- 0
WB 1 rows CLB on N -J
WB 1 rows CLB on J -B
WB 1 rows CLB on B -F
Attach CLB with (2)-10d nails
at each web.

psf-Ld Dead Live
TC 7.0 20.0
BC 10.0 0.0
TC+BC 17.0 20.0
Total 37.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz
A	509	169 U	262 R
G	1365	495 U	
H	1748	566 U	
D	233	117 U	262 R

Jt	Brg Size	Required
A	3.5"	1.5"
G	3.5"	1.5"
H	3.5"	1.9"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Axl-Csi-Bnd
-----Top Chords-----
A -L 0.25 511 C 0.00 0.25
L -M 0.26 235 C 0.00 0.26
M -N 0.51 1258 C 0.08 0.43

N -B	0.52	1291 C	0.09	0.43
B -C	0.29	1227 C	0.10	0.19
C -O	0.45	1369 C	0.10	0.35
O -P	0.56	914 C	0.00	0.56
P -D	0.61	235 T	0.05	0.56

-----Bottom Chords-----
A -G 0.45 484 T 0.08 0.37
G -K 0.57 242 T 0.03 0.54
K -J 0.36 1149 T 0.19 0.17
J -F 0.36 1158 T 0.19 0.17
E -I 0.31 4 C 0.00 0.31
I -S1 0.31 197 C 0.00 0.31
S1 -H 0.35 197 C 0.00 0.35
H -D 0.35 197 C 0.00 0.35

-----Chord-Webs-----
E -F 0.07 121 T 0.03 0.04
F -C 0.19 349 T 0.00 0.19
C -C 0.56 710 C 0.09 0.47

-----Webs-----
L -G 0.09 302 C
G -M 0.31 1121 C
M -K 0.30 960 T
K -N 0.27 430 C
N -J 0.03 153 T
J -B 0.07 265 T
B -F 0.04 183 T
F -O 0.15 477 T
F -I 0.28 883 T
I -O 0.68 812 C
I -P 0.37 1174 T
H -P 0.43 1549 C

TL Defl -0.27" in A -G L/344
LL Defl -0.12" in A -G L/782
Hz Disp LL DL TL
Jt H 0.05" 0.04" 0.08"
Shear // Grain in C -C 0.50

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A MT20 4.0x 8.0 0.8 0.3 0.79
L MT20 2.0x 4.0 Ctr Ctr 0.54
M MT20 5.0x 6.0-0.2 0.5 0.86
N MT20 4.0x 5.0 Ctr Ctr 0.67
B MT20 6.0x 8.0 0.9-3.8 0.75
C# MT20 2.0x 4.0-0.8-0.1 0.29
C MT20 5.0x 5.0 0.1-1.7 0.86
O MT20 4.0x 5.0 Ctr Ctr 0.67

P MT20	5.0x 6.0	0.2 0.5 0.86
D MT20	4.0x 8.0-0.8	0.3 0.79
G MT20	6.0x 8.0-1.3	3.3 0.79
K MT20	6.0x 6.0 Ctr	Ctr 0.87
J MT20	4.0x 5.0 Ctr	Ctr 0.61
F MT20	6.0x10.0 Ctr	1.2 0.75
E MT20	2.0x 4.0 Ctr	Ctr 0.67
I MT20	4.0x 8.0 Ctr	Ctr 0.57
S1 MT20	4.0x 8.0 Ctr	0.2 0.75
H MT20	2.0x 4.0 Ctr	Ctr 0.54

= Plate Monitor used

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS

Analysis Conforms To:
FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

NOTE: USER MODIFIED PLATES

This design may have plates selected through a plate monitor.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 1549 Lbs

Max tens. force 1174 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates, Inc.

2830 NW 41st Street Suite D

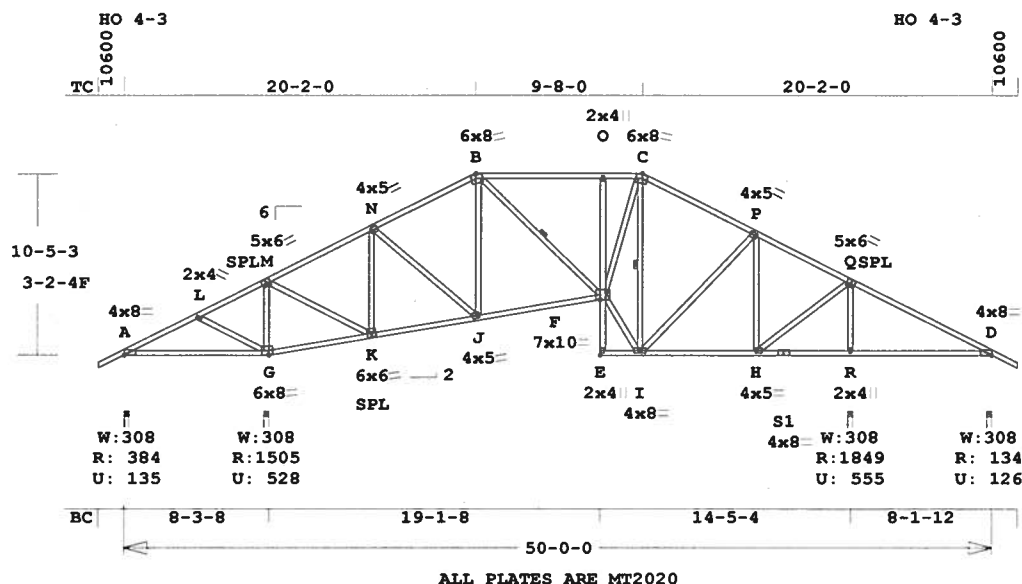
Gainesville, FL 32606

(352)375-8593

CA 5201

Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
070500	R19	1	SP	500000	6	1- 6- 0	1- 6- 0	

Stidham Residence, Roof, GC Construction



Scale: 0.094" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 398.5 LBS

Online Plus -- Version 21.0.032
RUN DATE: 13-JUN-07

CSI	Size	---	Lumber	---
TC	0.60	2x 4	SP-#2	
BC	0.59	2x 4	SP-#2	
CW	0.24	2x 4	SP-#3	
WB	0.54	2x 4	SP-#3	

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	50- 0- 0
BC Cont.	0- 0- 0	50- 0- 0
WB 1 rows CLB on B -F		
WB 1 rows CLB on I -C		
Attach CLB with (2)-10d nails at each web.		

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor 1.25		
Plate Duration Factor 1.25		
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (lbs)

Jt	Down	Uplift	Horiz
A	384	135 U	238 R
G	1506	528 U	
R	1850	555 U	
D	134	126 U	238 R

Jt	Brg Size	Required
A	3.5"	1.5"
G	3.5"	1.6"
R	3.5"	2.0"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Axl	CSI-Bnd
-----Top Chords-----				
A -L	0.28	249 C	0.00	0.28
L -M	0.25	78 T	0.00	0.25
M -N	0.38	1008 C	0.07	0.31
N -B	0.40	1220 C	0.09	0.31

B -O	0.43	1311 C	0.02	0.41
O -C	0.35	1304 C	0.01	0.34
C -P	0.40	927 C	0.08	0.32
P -Q	0.51	678 C	0.00	0.51
Q -D	0.60	452 T	0.09	0.51
-----Bottom Chords-----				
A -G	0.43	263 T	0.04	0.39
G -K	0.59	139 T	0.00	0.59
K -J	0.38	922 T	0.10	0.28
J -F	0.39	1102 T	0.11	0.28
E -I	0.23	15 C	0.00	0.23
I -H	0.28	598 T	0.05	0.23
H -S1	0.15	392 C	0.00	0.15
S1 -R	0.35	392 C	0.00	0.35
R -D	0.38	392 C	0.00	0.38
-----Chord Webs-----				
E -F	0.15	26 C	0.00	0.15
F -O	0.24	291 C	0.04	0.20
-----Webs-----				
L -G	0.10	331 C		
G -M	0.33	1199 C		
M -K	0.32	1027 T		
K -N	0.33	584 C		
N -J	0.07	238 T		
J -B	0.04	174 T		
B -F	0.09	305 T	1 Br	
F -C	0.52	1624 T		
F -I	0.48	1503 T		
I -C	0.54	1340 C	1 Br	
I -P	0.10	319 T		
H -P	0.46	660 C		
H -Q	0.39	1239 T		
R -Q	0.46	1645 C		

TL Defl	-0.28"	in A -G	L/332
LL Defl	-0.12"	in A -G	L/756
Hx Disp	LL	DL	TL
Jt R	0.08"	0.06"	0.14"
Shear //	Grain	in Q -D	0.27

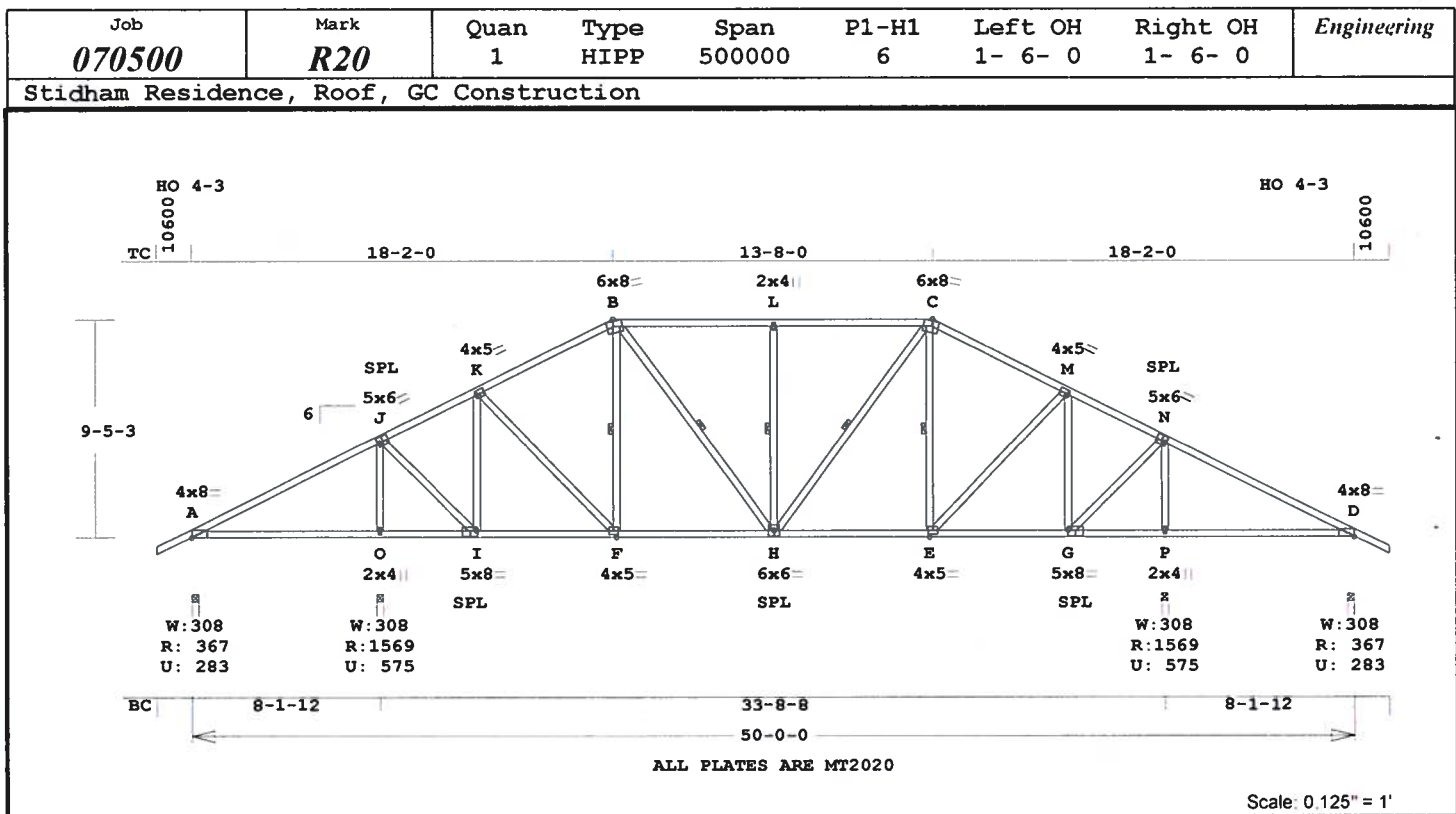
Plates for each ply each face.				
Plate -	MT20	20 Ga,	Gross Area	
Plate -	MT2H	20 Ga,	Gross Area	
Jt Type	Plt Size	X	Y	JSI
A	MT20	4.0x 8.0	0.8 0.3	0.79
L	MT20	2.0x 4.0	Ctr Ctr	0.54
M	MT20	5.0x 6.0	-0.2 0.5	0.86
N	MT20	4.0x 5.0	Ctr Ctr	0.67
B	MT20	6.0x 8.0	0.9-3.8	0.75
O	MT20	2.0x 4.0	Ctr Ctr	0.29

C	MT20	6.0x 8.0	-0.9-3.8	0.94
P	MT20	4.0x 5.0	Ctr Ctr	0.67
Q	MT20	5.0x 6.0	0.2 0.5	0.86
D	MT20	4.0x 8.0	-0.8 0.3	0.79
G	MT20	6.0x 8.0	-1.3 3.3	0.79
K	MT20	6.0x 6.0	Ctr Ctr	0.87
J	MT20	4.0x 5.0	Ctr Ctr	0.61
F	MT20	7.0x10.0	Ctr	1.7 0.90
E	MT20	2.0x 4.0	Ctr Ctr	0.67
I	MT20	4.0x 8.0	Ctr Ctr	0.65
H	MT20	4.0x 5.0	Ctr Ctr	0.60
S1	MT20	4.0x 8.0	Ctr	0.2 0.75
R	MT20	2.0x 4.0	Ctr Ctr	0.54

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
Max comp. force 1645 Lbs
Max tens. force 1624 Lbs
Quality Control Factor 1.25
FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates, Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201



Robbins Engineering, Inc./Online Plus™
 Online Plus -- Version 21.0.032
 RUN DATE: 12-JUN-07

CSI -Size- ----Lumber-----
 TC 0.53 2x 4 SP-#2
 BC 0.38 2x 4 SP-#2
 WB 0.38 2x 4 SP-#3

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	50- 0- 0
BC Cont.	0- 0- 0	50- 0- 0
WB 1 rows CLB on F -B		
WB 1 rows CLB on B -H		
WB 1 rows CLB on H -L		
WB 1 rows CLB on H -C		
WB 1 rows CLB on E -C		

Attach CLB with (2)-10d nails
 at each web.

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
A	367	284 U	213 R
O	1570	575 U	
P	1570	575 U	
D	367	284 U	213 R

Jt	Brg Size	Required
A	3.5"	1.5"
O	3.5"	1.7"
P	3.5"	1.7"
D	3.5"	1.5"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Axl-Csi-Bnd
 -----Top Chords-----

APPROX. TRUSS WEIGHT: 384.2 LBS

Member	Length	Area	Weight
A -J	0.49	344 T	0.00 0.49
J -K	0.53	818 C	0.08 0.45
K -B	0.36	1077 C	0.09 0.27
B -L	0.42	1159 C	0.00 0.42
L -C	0.42	1159 C	0.00 0.42
C -M	0.36	1077 C	0.09 0.27
M -N	0.49	818 C	0.00 0.49
N -D	0.49	284 T	0.00 0.49

Bottom Chords

Member	Length	Area	Weight
A -O	0.38	176 C	0.00 0.38
O -I	0.34	176 C	0.00 0.34
I -F	0.29	714 T	0.06 0.23
F -H	0.32	965 T	0.09 0.23
H -E	0.32	965 T	0.09 0.23
E -G	0.29	714 T	0.06 0.23
G -P	0.33	119 C	0.00 0.33
P -D	0.33	119 C	0.00 0.33

Webs

Member	Length	Area	Weight
O -J	0.38	1373 C	
J -I	0.29	928 T	
I -K	0.34	601 C	
K -F	0.11	358 T	
F -B	0.04	117 C	1 Br
B -H	0.10	326 T	1 Br
H -L	0.13	395 C	1 Br
L -C	0.10	326 T	1 Br
E -C	0.04	117 C	1 Br
E -M	0.11	358 T	
G -M	0.34	601 C	
G -N	0.29	928 T	
P -N	0.38	1373 C	

TL Defl -0.20" in A -O L/465
 LL Defl -0.09" in A -O L/999
 Shear // Grain in A -J 0.27

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A MT20 4.0x 8.0 0.8 0.3 0.79
 J MT20 5.0x 6.0-0.2 0.5 0.86
 K MT20 4.0x 5.0 Ctr Ctr 0.67
 B MT20 6.0x 8.0 0.9-3.8 0.75
 L MT20 2.0x 4.0 Ctr Ctr 0.54
 C MT20 6.0x 8.0-0.9-3.8 0.75
 M MT20 4.0x 5.0 Ctr Ctr 0.67
 N MT20 5.0x 6.0 0.2 0.5 0.86

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-
 concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

User-defined wind-exposed BC

regions --From-- --To--

0- 0- 0 8- 1-12

41-10- 4 50- 0- 0

Max comp. force 1373 Lbs

Max tens. force 965 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss

Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates,

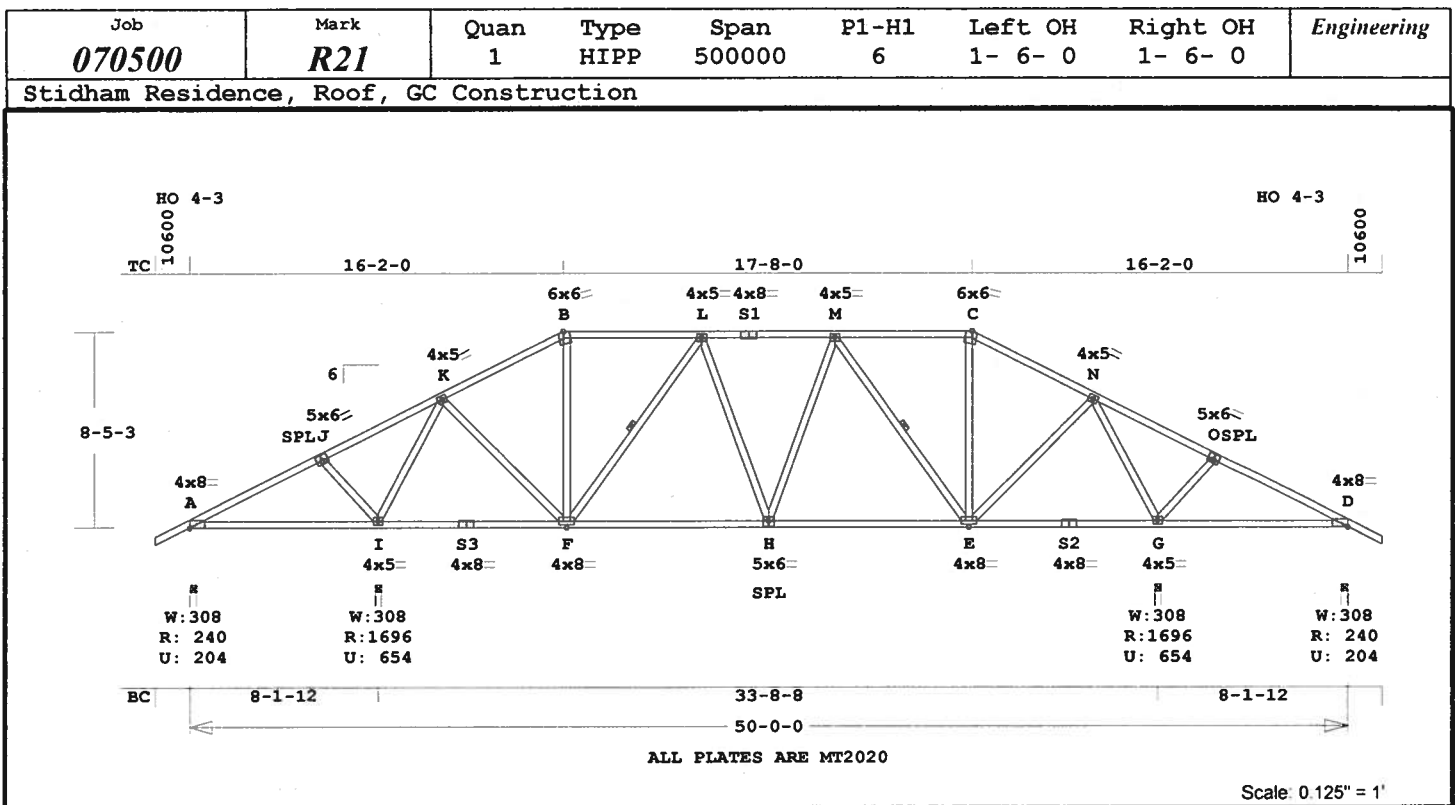
Inc.

2830 NW 41st Street Suite D

Gainesville, FL 32606

(352)375-8593

CA 5201



Robbins Engineering, Inc./Online Plus™
 Online Plus -- Version 21.0.032
 RUN DATE: 12-JUN-07

CSI -Size- ----Lumber-----
 TC 0.36 2x 4 SP-#2
 BC 0.52 2x 4 SP-#2
 WB 0.81 2x 4 SP-#3

Brace truss as follows:
 O.C. From To
 TC Cont. 0- 0- 0 50- 0- 0
 BC Cont. 0- 0- 0 50- 0- 0
 WB 1 rows CLB on F -L
 WB 1 rows CLB on M -E
 Attach CLB with (2)-10d nails
 at each web.

psf-Ld Dead Live
 TC 7.0 20.0
 BC 10.0 0.0
 TC+BC 17.0 20.0
 Total 37.0 Spacing 24.0"
 Lumber Duration Factor 1.25
 Plate Duration Factor 1.25
 TC Fb=1.15 Fc=1.10 Ft=1.10
 BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)
 Jt Down Uplift Horiz-
 A 240 205 U 190 R
 I 1697 654 U
 G 1697 654 U
 D 240 205 U 190 R

Jt Brg Size Required
 A 3.5" 1.5"
 I 3.5" 1.8"
 G 3.5" 1.8"
 D 3.5" 1.5"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Ax1-CSI-Bnd
 -----Top Chords-----
 A -J 0.34 215 T 0.02 0.32
 J -K 0.36 303 T 0.04 0.32
 K -B 0.29 912 C 0.07 0.22

APPROX. TRUSS WEIGHT: 364.6 LBS

B -L	0.28	813 C	0.00	0.28	D MT20	4.0x 8.0-0.8	0.3	0.79
L -S1	0.28	1143 C	0.00	0.28	I MT20	4.0x 5.0 Ctr	0.56	
S1-M	0.28	1143 C	0.00	0.28	S3 MT20	4.0x 8.0 Ctr	0.2	0.75
M -C	0.28	813 C	0.00	0.28	F MT20	4.0x 8.0 Ctr	0.51	
C -N	0.29	912 C	0.07	0.22	H MT20	5.0x 6.0 Ctr	0.5	0.88
N -O	0.36	303 T	0.04	0.32	E MT20	4.0x 8.0 Ctr	0.51	
O -D	0.34	166 T	0.02	0.32	S2 MT20	4.0x 8.0 Ctr	0.2	0.75
					G MT20	4.0x 5.0 Ctr	0.56	

-----Bottom Chords-----
 A -I 0.38 145 C 0.00 0.38
 I -S3 0.41 369 T 0.03 0.38
 S3-F 0.38 369 T 0.03 0.35
 F -H 0.52 1112 T 0.11 0.41
 H -E 0.52 1112 T 0.11 0.41
 E -S2 0.38 369 T 0.03 0.35
 S2-G 0.41 369 T 0.03 0.38
 G -D 0.38 145 C 0.00 0.38

-----Webs-----
 J -I 0.09 367 T
 I -K 0.81 1432 C
 K -F 0.20 638 T
 F -B 0.06 225 T
 F -L 0.20 511 C 1 Br
 L -H 0.05 188 T
 H -M 0.05 188 T
 M -E 0.20 511 C 1 Br
 E -C 0.06 225 T
 E -N 0.20 638 T
 N -G 0.81 1432 C
 G -O 0.08 348 T

TL Defl -0.14" in G -D L/639
 LL Defl -0.07" in G -D L/999
 Shear // Grain in B -L 0.22

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 A MT20 4.0x 8.0 0.8 0.3 0.79
 J MT20 5.0x 6.0-0.2 0.5 0.86
 K MT20 4.0x 5.0 Ctr Ctr 0.60
 B MT20 6.0x 6.0 0.9-3.6 0.75
 L MT20 4.0x 5.0 Ctr Ctr 0.62
 S1 MT20 4.0x 8.0 Ctr-0.2 0.75
 M MT20 4.0x 5.0 Ctr Ctr 0.62
 C MT20 6.0x 6.0-0.9-3.6 0.75
 N MT20 4.0x 5.0 Ctr Ctr 0.60
 O MT20 5.0x 6.0 0.2 0.5 0.86

NOTES:
 Trusses Manufactured by:
 RIDGWAY ROOF TRUSS
 Analysis Conforms To:
 FBC2004
 OH Loading
 Soffit psf 2.0
 Design checked for 10 psf non-
 concurrent LL on BC.
 Wind Loads - ANSI / ASCE 7-02
 Truss is designed as
 Components and Claddings*
 for Exterior zone location.
 Wind Speed: 110 mph
 Mean Roof Height: 25-0
 Exposure Category: B
 Occupancy Factor : 1.00
 Building Type: Enclosed
 TC Dead Load: 4.0 psf
 BC Dead Load: 6.0 psf
 User-defined wind-exposed BC
 regions --From-- --To--
 0- 0- 0 8- 1-12
 41-10- 4 50- 0- 0

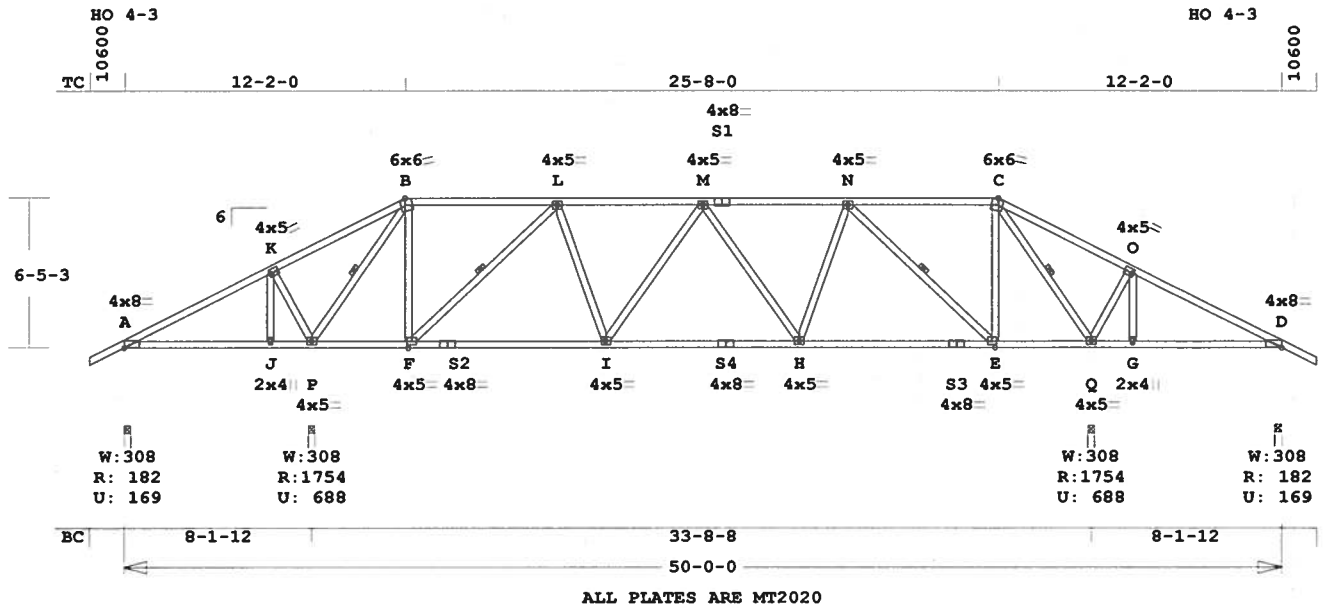
Max comp. force 1432 Lbs
 Max tens. force 1112 Lbs
 Quality Control Factor 1.25
 FABRICATOR NOTES:
 1. Delegated Engineer (Truss
 Designer)
 Gary Dounson, PE 35054
 Gary Dounson & Associates,
 Inc.
 2830 NW 41st Street Suite D
 Gainesville, FL 32606
 (352)375-8593
 CA 5201

Scale: 0.125" = 1'

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Job	Mark	Quan	Type	Span	P1-H1	Left OH	Right OH	Engineering
070500	R23	1	HIPP	500000	6	1- 6- 0	1- 6- 0	

Stidham Residence, Roof, GC Construction



Scale: 0.125" = 1'

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 357.1 LBS

Online Plus -- Version 21.0.032
RUN DATE: 12-JUN-07

CSI -Size- ---Lumber---
TC 0.49 2x 4 SP-#2
BC 0.52 2x 4 SP-#2
WB 0.40 2x 4 SP-#3

Brace truss as follows:

O.C. From To
TC Cont. 0- 0- 0 50- 0- 0
BC Cont. 0- 0- 0 50- 0- 0
WB 1 rows CLB on P -B
WB 1 rows CLB on F -L
WB 1 rows CLB on N -E
WB 1 rows CLB on C -Q
Attach CLB with (2)-10d nails
at each web.

psf-Ld Dead Live
TC 7.0 20.0
BC 10.0 0.0
TC+BC 17.0 20.0
Total 37.0 Spacing 24.0"
Lumber Duration Factor 1.25
Plate Duration Factor 1.25
TC Fb=1.15 Fc=1.10 Ft=1.10
BC Fb=1.10 Fc=1.10 Ft=1.10

Total Load Reactions (Lbs)

Jt Down Uplift Horiz-
A 182 170 U 142 R
P 1755 689 U
Q 1755 689 U
D 182 170 U 142 R

Jt Brg Size Required
A 3.5" 1.5"
P 3.5" 1.9"
Q 3.5" 1.9"
D 3.5" 1.5"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Ax1-CSI-Bnd
-----Top Chords-----
A -K 0.47 214 T 0.04 0.43
K -B 0.49 438 T 0.06 0.43
B -L 0.32 511 C 0.00 0.32
L -M 0.32 1342 C 0.01 0.31

M -S1 0.31 1342 C 0.01 0.30
S1 -N 0.32 1342 C 0.01 0.31
N -C 0.32 511 C 0.00 0.32
C -O 0.49 438 T 0.06 0.43
O -D 0.47 214 T 0.04 0.43
-----Bottom Chords-----
A -J 0.23 175 C 0.00 0.23
J -P 0.23 175 C 0.00 0.23
P -F 0.32 496 T 0.05 0.27
F -S2 0.39 1231 T 0.12 0.27
S2 -I 0.50 1231 T 0.12 0.38
I -S4 0.52 1456 T 0.14 0.38
S4 -H 0.52 1456 T 0.14 0.38
H -S3 0.50 1231 T 0.12 0.38
S3 -E 0.39 1231 T 0.12 0.27
E -Q 0.32 496 T 0.05 0.27
Q -G 0.20 175 C 0.00 0.20
G -D 0.20 175 C 0.00 0.20
-----Webs-----
J -K 0.06 266 C
K -P 0.15 613 T
P -B 0.40 1601 C 1 Br
F -B 0.26 816 T
F -L 0.32 992 C 1 Br
L -I 0.11 412 T
I -M 0.16 201 C
M -H 0.16 201 C
H -N 0.11 412 T
N -E 0.32 992 C 1 Br
E -C 0.26 816 T
C -Q 0.40 1601 C 1 Br
Q -O 0.12 515 T
G -O 0.06 232 T

TL Defl -0.07" in A -J L/999
LL Defl -0.03" in A -J L/999
Shear // Grain in A -K 0.23

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
A MT20 4.0x 8.0 0.8 0.3 0.79
K MT20 4.0x 5.0 Ctr Ctr 0.67
B MT20 6.0x 6.0 0.9-3.6 0.75
L MT20 4.0x 5.0 Ctr Ctr 0.62
M MT20 4.0x 5.0 Ctr Ctr 0.51
S1 MT20 4.0x 8.0 Ctr-0.2 0.75
N MT20 4.0x 5.0 Ctr Ctr 0.62
C MT20 6.0x 6.0 0.9-3.6 0.75
O MT20 4.0x 5.0 Ctr Ctr 0.67

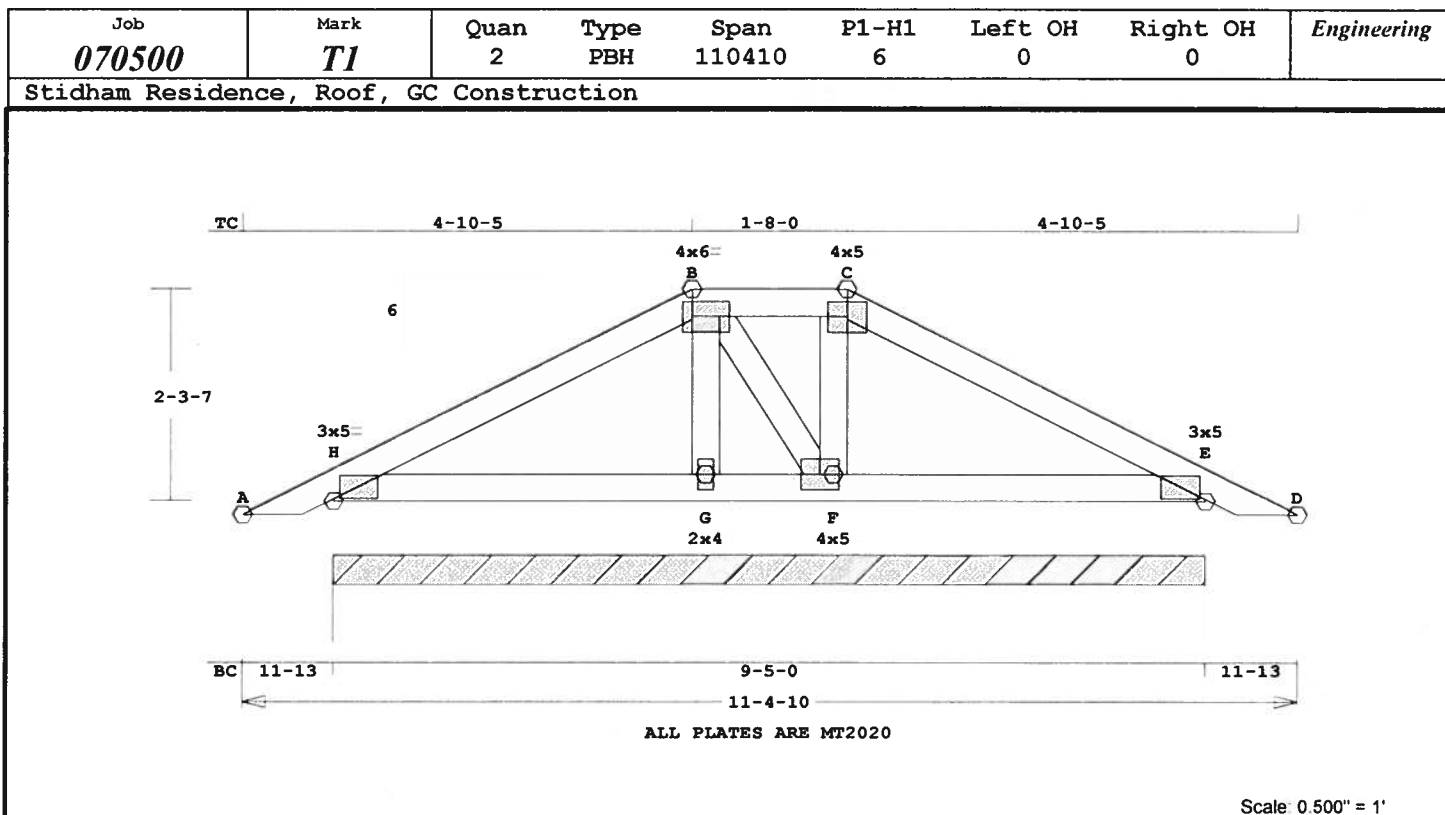
D MT20 4.0x 8.0-0.8 0.3 0.79
J MT20 2.0x 4.0 Ctr Ctr 0.54
P MT20 4.0x 5.0 Ctr Ctr 0.55
F MT20 4.0x 5.0 Ctr Ctr 0.60
S2 MT20 4.0x 8.0 Ctr 0.2 0.75
I MT20 4.0x 5.0 Ctr Ctr 0.62
S4 MT20 4.0x 8.0 Ctr 0.2 0.75
H MT20 4.0x 5.0 Ctr Ctr 0.62
S3 MT20 4.0x 8.0 Ctr 0.2 0.75
E MT20 4.0x 5.0 Ctr Ctr 0.60
Q MT20 4.0x 5.0 Ctr Ctr 0.55
G MT20 2.0x 4.0 Ctr Ctr 0.54

NOTES:

Trusses Manufactured by:
RIDGWAY ROOF TRUSS
Analysis Conforms To:
FBC2004
OH Loading
Soffit psf 2.0
Design checked for 10 psf non-
concurrent LL on BC.
Wind Loads - ANSI / ASCE 7-02
Truss is designed as
Components and Claddings*
for Exterior zone location.
Wind Speed: 110 mph
Mean Roof Height: 25-0
Exposure Category: B
Occupancy Factor : 1.00
Building Type: Enclosed
TC Dead Load: 4.0 psf
BC Dead Load: 6.0 psf
User-defined wind-exposed BC
regions --From-- --To--
0- 0- 0 8- 1-12
41-10- 4 50- 0- 0

Max comp. force 1601 Lbs
Max tens. force 1456 Lbs
Quality Control Factor 1.25
FABRICATOR NOTES:

1. Delegated Engineer (Truss
Designer)
Gary Dounson, PE 35054
Gary Dounson & Associates,
Inc.
2830 NW 41st Street Suite D
Gainesville, FL 32606
(352)375-8593
CA 5201



Online Plus -- Version 21.0.032
 RUN DATE: 12-JUN-07

	CSI	-Size-	----	Lumber----
TC	0.17	2x 4	SP-#2	
BC	0.12	2x 4	SP-#2	
WB	0.04	2x 4	SP-#3	

Brace truss as follows:

	O.C.	From	To
TC	Cont.	0- 0- 0	11- 4-10
BC	Cont.	0- 0- 0	11- 4-10

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor 1.25		
Plate Duration Factor 1.25		
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
H	735	253 U	37 R

Jt	Brg Size	Required
H	113.0"	0"-to- 113"

Plus 9 Wind Load Case(s)
 Plus 1 UBC LL Load Case(s)

Membr CSI P Lbs Axl-CSI-Bnd

-----Top Chords-----					
H -B	0.17	376 T	0.04	0.13	
B -C	0.09	327 T	0.04	0.05	
C -E	0.17	281 T	0.03	0.14	

-----Bottom Chords-----					
H -G	0.10	15 T	0.00	0.10	
G -F	0.06	0 T	0.00	0.06	
F -E	0.12	19 T	0.00	0.12	

Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 51.7 LBS

-----Webs-----			
G -B	0.02	139 C	
B -F	0.04	195 T	
F -C	0.01	78 T	

TL Defl	-0.01"	in F -E	L/999
LL Defl	0.00"	in F -E	L/999
Shear //	Grain	in H -B	0.15

Plates for each ply each face.
 Plate - MT20 20 Ga, Gross Area
 Plate - MT2H 20 Ga, Gross Area
 Jt Type Plt Size X Y JSI
 H MT20 3.0x 5.0 Ctr Ctr 0.54
 B MT20 4.0x 6.0 Ctr Ctr 0.66
 C MT20 4.0x 5.0 Ctr Ctr 0.66
 E MT20 3.0x 5.0 Ctr Ctr 0.54
 G MT20 2.0x 4.0 Ctr Ctr 0.38
 F MT20 4.0x 5.0 Ctr Ctr 0.60

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 283 Lbs

Max tens. force 376 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054

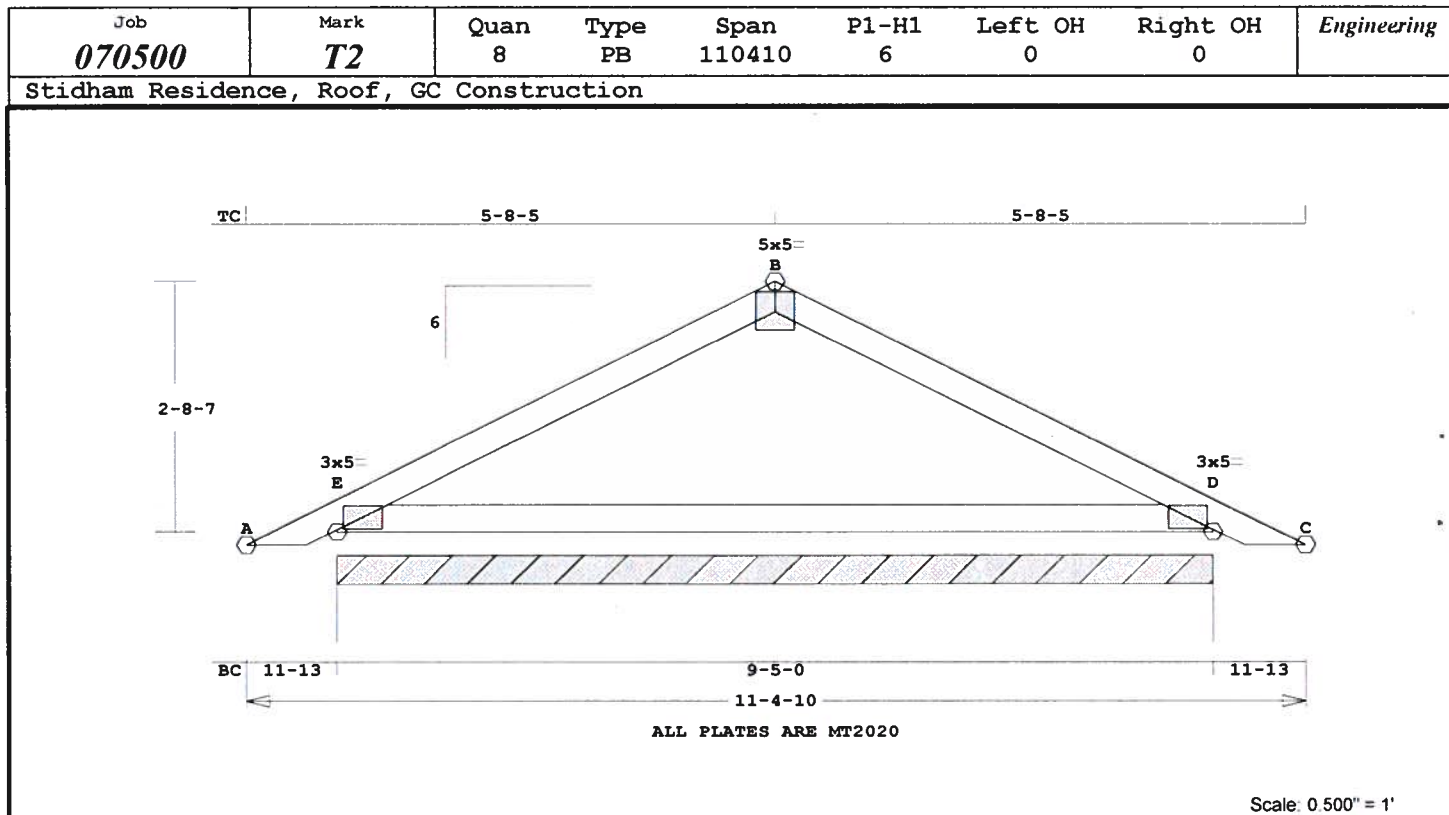
Gary Dounson & Associates, Inc.

2830 NW 41st Street Suite D

Gainesville, FL 32606

(352) 375-8593

CA 5201



Robbins Engineering, Inc./Online Plus™ APPROX. TRUSS WEIGHT: 41.8 LBS

Online Plus -- Version 21.0.032
RUN DATE: 12-JUN-07

CSI -Size- ----Lumber----
TC 0.28 2x 4 SP-#2
BC 0.30 2x 4 SP-#2

Brace truss as follows:

O.C.	From	To
TC Cont.	0- 0- 0	11- 4-10
BC Cont.	0- 0- 0	11- 4-10

psf-Ld	Dead	Live
TC	7.0	20.0
BC	10.0	0.0
TC+BC	17.0	20.0
Total	37.0	Spacing 24.0"
Lumber Duration Factor	1.25	
Plate Duration Factor	1.25	
TC Fb=1.15	Fc=1.10	Ft=1.10
BC Fb=1.10	Fc=1.10	Ft=1.10

Total Load Reactions (Lbs)

Jt	Down	Uplift	Horiz-
E	735	253 U	44 R

Jt	Brg Size	Required
E	113.0"	0"-to- 113"

Plus 9 Wind Load Case(s)
Plus 1 UBC LL Load Case(s)

Membr	CSI	P Lbs	Ax1-CSI-Bnd
-----Top Chords-----			
E -B	0.28	345 T	0.00 0.28
B -D	0.28	345 T	0.00 0.28
-----Bottom Chords-----			
E -D	0.30	53 C	0.00 0.30

TL Defl	-0.25" in E -D	L/444
LL Defl	-0.12" in E -D	L/931

Shear // Grain in E -B 0.19

Plates for each ply each face.
Plate - MT20 20 Ga, Gross Area
Plate - MT2H 20 Ga, Gross Area
Jt Type Plt Size X Y JSI
E MT20 3.0x 5.0 Ctr Ctr 0.54
B MT20 5.0x 5.0 Ctr-1.8 0.38
D MT20 3.0x 5.0 Ctr Ctr 0.54

NOTES:

Trusses Manufactured by:

RIDGWAY ROOF TRUSS

Analysis Conforms To:

FBC2004

OH Loading

Soffit psf 2.0

Design checked for 10 psf non-concurrent LL on BC.

Refer to Gen Det 3 series for web bracing and plating.

Wind Loads - ANSI / ASCE 7-02

Truss is designed as

Components and Claddings*

for Exterior zone location.

Wind Speed: 110 mph

Mean Roof Height: 25'-0

Exposure Category: B

Occupancy Factor : 1.00

Building Type: Enclosed

TC Dead Load: 4.0 psf

BC Dead Load: 6.0 psf

Max comp. force 337 Lbs

Max tens. force 345 Lbs

Quality Control Factor 1.25

FABRICATOR NOTES:

1. Delegated Engineer (Truss Designer)

Gary Dounson, PE 35054

Gary Dounson & Associates,

BCSI-B1 SUMMARY SHEET 1 of 2

GENERAL NOTES NOTAS GENERALES

Trusses are not marked in any way to identify the frequency or location of temporary bracing. Follow the recommendations for handling, installing and temporary bracing of trusses. Refer to BCSI-1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses for more detailed information.

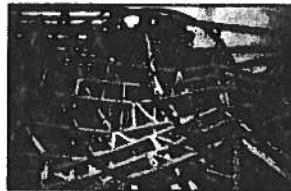
Truss Design Drawings may specify locations of permanent bracing on individual compression members. Refer to the BCSI-B3 Summary Sheet - Web Member Permanent Bracing/Web Reinforcement for more information. All other permanent bracing design is the responsibility of the Building Designer.

Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de los arriostres (bracing) temporales. Use las recomendaciones de manejo, instalación y arriostre temporal de los trusses. Vea el folleto BCSI-1-03 Guía de Buena Práctica para el Manejo, Instalación y Arriostre de los Trusses de Madera Conectados con Placas de Metal para mayor información.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de los arriostres permanentes en los miembros individuales en compresión. Vea la hoja resumen BCSI-B3 para los arriostres permanentes y refuerzos de los miembros secundarios (webs) para mayor información. El resto de arriostres permanentes son la responsabilidad del Diseñador del Edificio.

⚠ The consequences of improper handling, installing and bracing may be a collapse of the structure, or worse, serious personal injury or death.

El resultado de un manejo, instalación y arriostre inadecuados, puede ser la caída de la estructura o aun peor, muertos o heridos.



⚠ Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

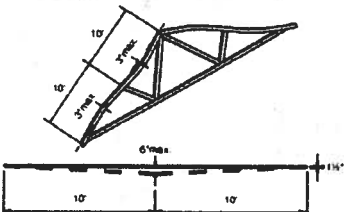
Empaques y placas de metal tienen bordes afilados. Use guantes y lentes protectores cuando corte los empaques.



HANDLING — MANEJO

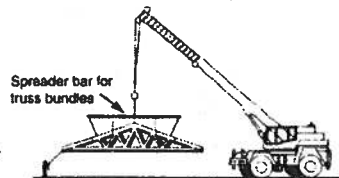
✓ Allow no more than 3" of deflection for every 10' of span.

No permita más de 3 pulgadas de pandeo por cada 10 pies de tramo.



⚠ Use special care in windy weather or near power lines and airports.

Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



✓ Check banding prior to moving bundles.

Revise los empaques antes de mover los paquetes de trusses.

⚠ Avoid lateral bending. — Evite la flexión lateral.



✓ Pick up vertical bundles at the top chord.

Levante de la cuerda superior los grupos verticales de trusses.



ONE WEEK OR LESS

MORE THAN ONE WEEK



✓ Bundles stored on the ground for one week or more should be raised by blocking at 8' to 10' on center.

Los paquetes almacenados en la tierra por una semana o más deben ser elevados con bloques a cada 8 o 10 pies.

✓ For long term storage, cover bundles to prevent moisture gain but allow for ventilation.

Para almacenamiento por mayor tiempo, cubra los paquetes para prevenir aumento de humedad pero permita ventilación.



⚠ Do not store on uneven ground.

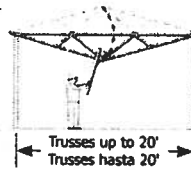
No almacene en tierra desigual.



HAND ERECTION — LEVANTAMIENTO A MANO

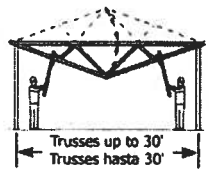
✓ Trusses 20' or less, support at peak.

Levante del pico los trusses de 20 pies o menos



✓ Trusses 30' or less, support at quarter points.

Levante de los cuartos de tramo los trusses de 30 pies o menos



HOISTING — LEVANTAMIENTO

✓ Hold each truss in position with the erection equipment until temporary bracing is installed and truss is fastened to the bearing points.

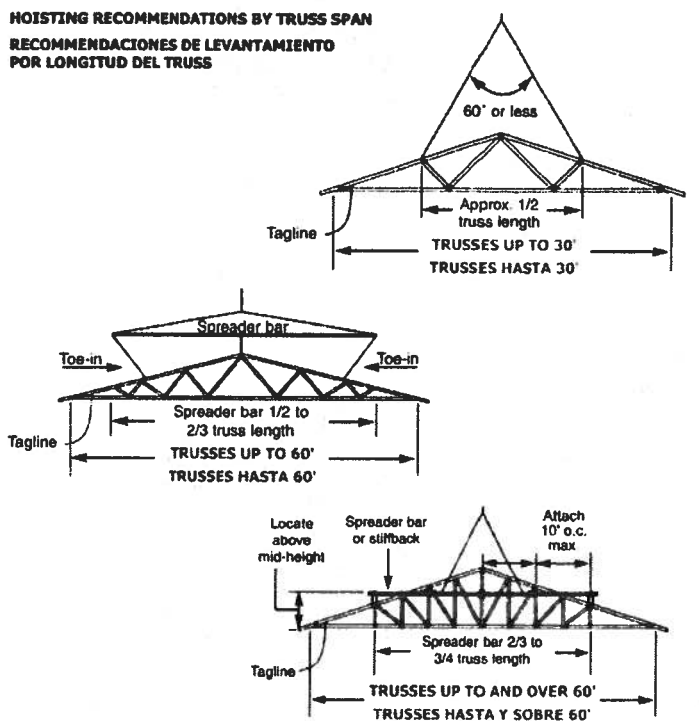
Sostenga cada truss en posición con la grúa hasta que el arriostre temporal esté instalado y el truss asegurado en los soportes.

⚠ Do not lift trusses over 30' by the peak.

No levante del pico los trusses de más de 30 pies.



HOISTING RECOMMENDATIONS BY TRUSS SPAN RECOMENDACIONES DE LEVANTAMIENTO POR LONGITUD DEL TRUSS



BRACING — ARRIOSTRE

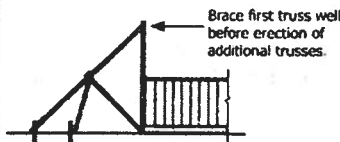
⚠ Refer to BCSI-B2 Summary Sheet - Truss Installation and Temporary Bracing for more information. Vea el resumen BCSI-B2 - Instalación de Trusses y Arriostre Temporal para mayor información.

⚠ Do not walk on unbraced trusses. No camine en trusses sueltos.

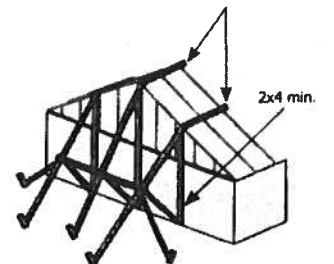


✓ Locate ground braces for first truss directly in line with all rows of top chord temporary lateral bracing.

Coloque los arriostres de tierra para el primer truss directamente en línea con cada uno de las filas de arriostres laterales temporales de la cuerda superior.



Top Chord Temporary Lateral Bracing (TCLTB)



BCSI-B1 SUMMARY SHEET 2 of 2

BRACING FOR THREE PLANES OF ROOF EL ARRIOSTRE EN TRES PLANOS DE TECHO

- ☒ This bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses.
Este método de arriostre es para todos los trusses excepto trusses de cuerdas paralelas 3x2 y 4x2

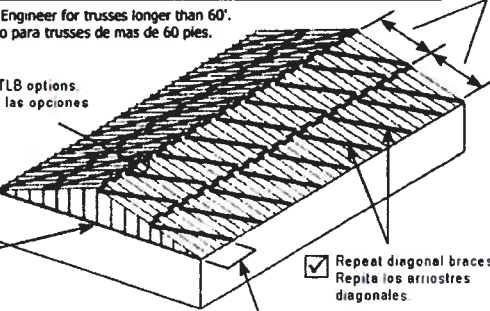
1) TOP CHORD — CUERDA SUPERIOR

Truss Span Longitud de Tramo	Top Chord Temporary Lateral Brace (TCLB) Spacing Espaciamiento del Arriostre Temporal de la Cuerda Superior
Up to 30'	10' o.c. max.
Hasta 30 pies	10 pies máximo
30' to 45'	8' o.c. max.
30 a 45 pies	8 pies máximo
45' to 60'	6' o.c. max.
45 a 60 pies	6 pies máximo
60' to 80'	4' o.c. max.
60 a 80 pies*	4 pies máximo

*Consult a Professional Engineer for trusses longer than 60'.
*Consulte a un ingeniero para trusses de mas de 60 pies.

- ☒ See BCSI-B2 for TCLB options.
Vea el BCSI-B2 para las opciones de TCLB.

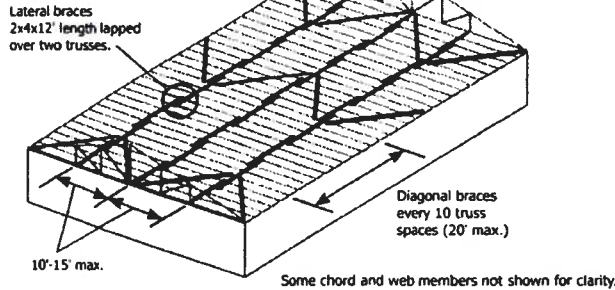
☒ Refer to BCSI-B6 Summary Sheet - Gable End Frame Bracing.
Vea el resumen BCSI-B6 Arriostre del truss terminal de un techo a dos aguas.



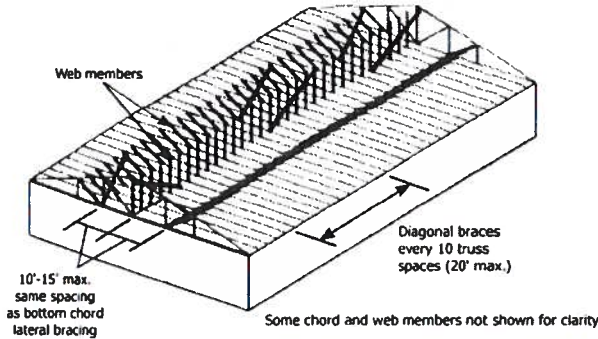
- ☒ Repeat diagonal braces every 10 truss spaces (20' max.).
Repita los arriostres diagonales.

Instale los cinco primeros trusses con espaciadores. Luego los arriostres diagonales. Repita este procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.

2) BOTTOM CHORD — CUERDA INFERIOR



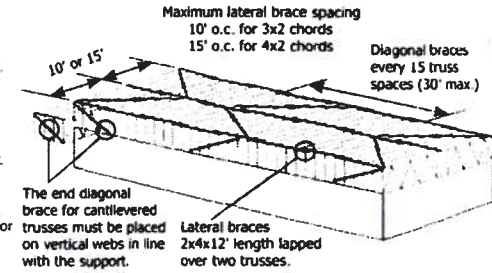
3) WEB MEMBER PLANE — PLANO DE LOS MIEMBROS SECUNDARIOS



DIAGONAL BRACING IS VERY IMPORTANT
¡EL ARRIOSTRE DIAGONAL ES MUY IMPORTANTE!

BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES EL ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3x2 Y 4x2

- ☒ Refer to BCSI-B2 Summary Sheet - Temporary and Permanent Bracing for Parallel Chord Trusses for more information.
Vea el resumen BCSI-B2 Arriostre temporal y permanente de trusses de cuerdas paralelas para mayor información.



INSTALLING — INSTALACION

- ☒ Tolerances for Out-of-Plane. Tolerancias para Fuera-de-Plano

Length	Max. Bow	Length	Max. Bow
1/4"	1"	1/4"	1"
1/2"	2"	1/2"	2"
3/4"	3"	3/4"	3"
1"	4"	1"	4"
1-1/4"	5"	1-1/4"	5"
1-1/2"	6"	1-1/2"	6"
1-3/4"	7"	1-3/4"	7"
2"	8"	2"	8"

Max. Bow	Truss Length
3/4"	12.5'
7/8"	14.6'
1"	16.7'
1-1/8"	18.8'
1-1/4"	20.8'
1-3/8"	22.9'
1-1/2"	25.0'
1-3/4"	29.2'
2"	≥33.3'

CONSTRUCTION LOADING — CARGA DE CONSTRUCCION

- ☒ Do not proceed with construction until all bracing is securely and properly in place.
No proceda con la construcción hasta que todos los arriostres estén colocados en forma apropiada y segura.

Do not exceed maximum stack heights. Refer to BCSI-B4 Summary Sheet - Construction Loading for more information.
No exceda las máximas alturas recomendadas. Vea el resumen BCSI-B4 Carga de Construcción para mayor información.

Maximum Stack Height for Materials on Trusses

Material	Height (h)
Gypsum Board	12'
Plywood or OSB	16'
Asphalt Shingles	2 bundles
Concrete Block	8'
Clay Tile	3-4 tiles high



- ☒ Do not overload small groups or single trusses.
No sobrecargue pequeños grupos o trusses individuales.

- ☒ Place loads over as many trusses as possible.
Coloque las cargas sobre tantos trusses como sea posible.

- ☒ Position loads over load bearing walls.
Coloque las cargas sobre las paredes soportantes.

ALTERATIONS — ALTERACIONES

- ☒ Refer to BCSI-B5 Summary Sheet - Truss Damage, Jobsite Modifications and Installation Errors.
Vea el resumen BCSI-B5 Daños de trusses, Modificaciones en la Obra y Errores de Instalación.

- ☒ Do not cut, alter, or drill any structural member of a truss unless specifically permitted by the Truss Design Drawing.
No corte, altere o perforo ningún miembro estructural de los trusses, a menos que esté específicamente permitido en el dibujo del diseño del truss.

- ☒ Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin una autorización previa del Fabricante de Trusses, pueden reducir o eliminar la garantía del Fabricante de Trusses.

NOTE: The Truss Manufacturer and Truss Designer must rely on the fact that the Contractor and crane operator (if applicable) are capable of undertaking the work they have agreed to do on a particular project. The Contractor should seek any required approvals regarding construction practices from a competent party. The methods and procedures outlined are intended to ensure that the overall construction techniques employed will put floor and roof trusses into place SAFELY. These recommendations for handling, installing and bracing wood trusses are based upon the collective experience of leading technical personnel in the wood truss industry, but must, due to the nature of responsibilities involved, be presented only as a GUIDE for use by a qualified Building Designer or Erector/Installation Contractor. It is not intended that these recommendations be interpreted as superior to any design specification (provided by other an Architect, Engineer, the Building Designer, the Erector/Installation Contractor or otherwise) for handling, installing and bracing wood trusses and it does not preclude the use of other equivalent methods for bracing and proving stability for the walls and columns as may be determined by the truss Erector/Installation Contractor. Thus, the Wood Truss Council of America and the Truss Plate Institute expressly disclaim any responsibility for damages arising from the use, application, or reliance on the recommendations and information contained herein.



WOOD TRUSS COUNCIL OF AMERICA
One WTCOA Center • 6300 Enterprise Lane • Madison, WI 53719
608/274-4849 • www.woodtruss.com

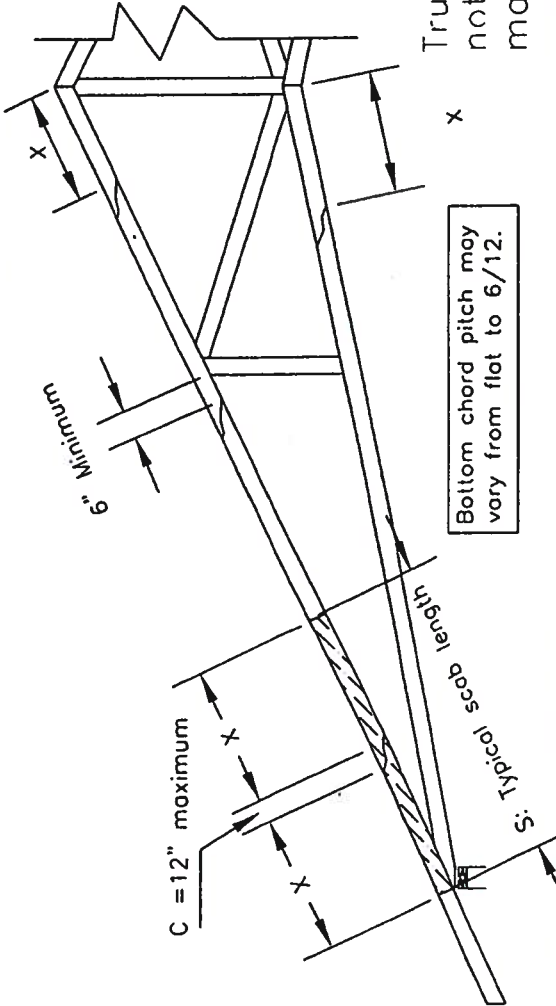


TRUSS PLATE INSTITUTE
583 D'Oroff Drive • Madison, WI 53719
608/833-5900 • www.tpi.org

- * Truss must be single ply.
- * No more than two broken or cracked chords per truss.
- * Perimeter of break area must be minimum of X distance from any heel or peak, and minimum of 6" away from any interior joint locations.

Minimum end distance of 2" must be provided for all scab members.

TOTAL NUMBER OF NAILS ON EACH SIDE OF BREAK		.X (INCHES)	MAXIMUM FORCE (LBS.) AT 33% LOAD DURATION	
2X4 SYP	2X6 SYP		2X4 SYP	2X6 SYP
13	20	24"	1924	2886
17	26	30"	2501	3752
21	32	36"	3079	4618
25	38	42"	3656	5484
29	44	48"	4233	6349



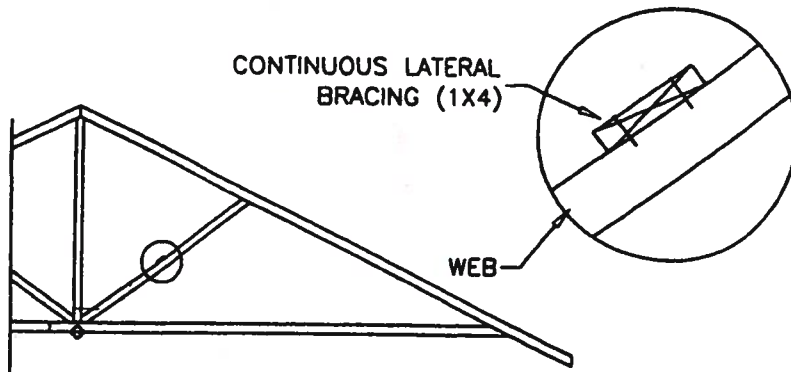
Truss manufacturer must be notified if field conditions do not match this general detail.

[illegible]

FURNISH A COPY OF THIS DESIGN TO ERECTION CONTRACTOR
IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FAB. TO REVIEW THIS DRWG. &
VERIFY THAT DATA INCLUDING DIM. & LOADS CONFORM TO ARCH. PLAN/SPECS & FAB TRUSS LAYOUTS.

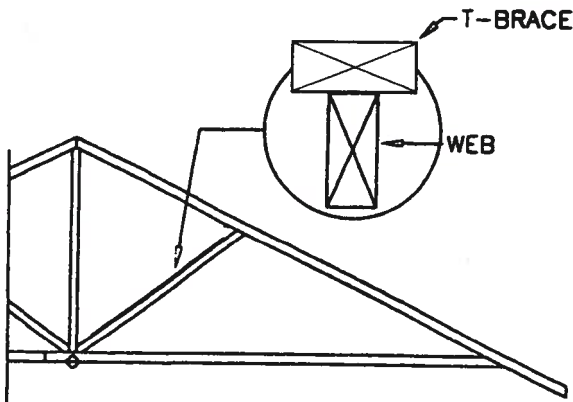
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PERMANENT WEB BRACING



CONTINUOUS LATERAL BRACING

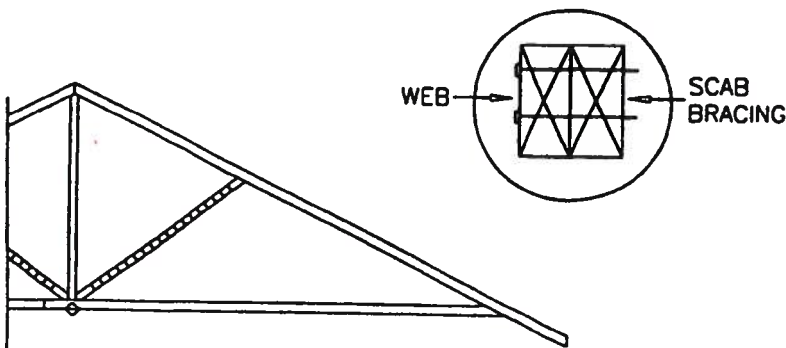
1x4 #3 HEM-FIR OR BETTER CONTINUOUS LATERAL BRACING TO BE EQUALLY SPACED. ATTACH WITH (2) 8d NAILS. BRACING MATERIAL TO BE SUPPLIED AND ATTACHED AT BOTH ENDS TO A SUITABLE SUPPORT BY ERECTION CONTRACTOR.



T-BRACE

THESE DETAILS APPLY TO 1.5" WIDE WOOD TRUSSES.

- USE A 2x4 T-BRACE IF THE TRUSS DESIGN SPECIFIES ONE LATERAL BRACE (MID POINT OF WEB).
- USE A 2x6 T-BRACE IF THE TRUSS DESIGN SPECIFIES TWO LATERAL BRACES (AT THE THIRD POINTS OF THE WEB).
- USE A CONTINUOUS PIECE FOR THE T-BRACE, OF THE SAME GRADE AS THE WEB AND COVERING AT LEAST 90% OF THE WEB LENGTH.
- CENTER THE T-BRACE ON THE WEB AND FASTEN WITH 10d COMMON NAILS SPACED 4" ON CENTER.



SCAB BRACE

SCAB BRACE SAME SIZE, GRADE, AND LENGTH AS WEB MEMBER. ATTACH WITH 10d NAILS @ 4" O.C. BRACING MATERIAL TO BE SUPPLIED BY ERECTION CONTRACTOR.