DATE 03/14/2007

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000025616

APPLICANT JOHN NORRIS PHONE 758-3663

ADDRESS 351 NW CORWIN GLEN LAKE CITY FL 32055

OWNER IMAGE DEVELOPMENT GROUP, INC PHONE 758-3663

ADDRESS 320 SW GREENWOOD TERR FORT WHITE FL 32038

CONTRACTOR JOHN NORRIS PHONE 961-4549

LOCATION OF PROPERTY 47 S, L 27, L CR 18, .5 MILES ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 89400.00

HEATED FLOOR AREA 1788.00 TOTAL AREA 2630.00 HEIGHT 17.00 STORIES 1

FOUNDATION CONCRETE WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING FORT WHITE MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT REAR SIDE

NO. EX.D.U. 0 FLOOD ZONE FW DEVELOPMENT PERMIT NO.

PARCEL ID 34-6S-16-04056-130 SUBDIVISION THORNWOOD

LOT 30 BLOCK PHASE UNIT TOTAL ACRES 1.25

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
		RG0066597		
Driveway Connection	**Septic Tank Number**	**LU & Zoning checked by**	**Approved for Issuance**	**New Resident**
FORT WHITE	07-200	FW	JH	N

COMMENTS: TOWN OF FORT WHITE COMPLIANCE LETTER IN FILE

NOC ON FILE

Check # or Cash 3963

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by — Foundation date/app. by — Monolithic date/app. by

Under slab rough-in plumbing date/app. by — Slab date/app. by — Sheathing/Nailing date/app. by

Framing date/app. by — Rough-in plumbing above slab and below wood floor date/app. by

Electrical rough-in date/app. by — Heat & Air Duct date/app. by — Peri. beam (Lintel) date/app. by

Permanent power date/app. by — C.O. Final date/app. by — Culvert date/app. by

M/H tie downs, blocking, electricity and plumbing date/app. by — Pool date/app. by

Reconnection date/app. by — Pump pole date/app. by — Utility Pole date/app. by

M/H Pole date/app. by — Travel Trailer date/app. by — Re-roof date/app. by

BUILDING PERMIT FEE $ 450.00 CERTIFICATION FEE $ 13.15 SURCHARGE FEE $ 13.15

MISC. FEES $ 0.00 ZONING CERT. FEE $ FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ CULVERT FEE $ **TOTAL FEE** 476.30

INSPECTORS OFFICE LH CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

THIS INSTRUMENT PREPARED BY
AND RETURN TO:
TITLE OFFICES, LLC
343 NW COLE TERRACE
SUITE 105
LAKE CITY, FLORIDA 32055

Parcel I.D. #: ***04056-130***

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing
is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By Sharon Teagle
Deputy Clerk
Date 03-09-2007

CLERK CIRCUIT COURT
COLUMBIA COUNTY, FLORIDA

Inst:2007005610 Date:03/09/2007 Time:09:39
D.T DC,P.DeWitt Cason,Columbia County B:1113 P:349

SPACE ABOVE THIS LINE FOR PROCESSING DATA

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713.13, Florida Statutes, the following information is provided in this Notice of Commencement. This Notice shall be void and of no force and effect if construction is not commenced within ninety (90) days after recordation.

1. Description of property: (Legal description of property, and street address if available)

LOT 30, THORNWOOD, A SUBDIVISION ACCORDING TO THE MAP OR PLAT THEREOF AS RECORDED IN PLAT BOOK 7, PAGES 202-204, PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

2. General description of improvement: **construction of single family dwelling**

3. Owner information:
   a. Name and address:
   IMAGE DEVELOPMENT GROUP, LLC
   20074 NW 258TH DRIVE, HIGH SPRINGS, FL 32643
   b. Interest in property: **Fee Simple**
   c. Name and Address of Fee Simple Titleholder (if other than owner):

4. Contractor: (Name and Address) (John Norris)
IMAGE DEVELOPMENT GROUP, LLC
20074 NW 258TH DRIVE, HIGH SPRINGS, FLORIDA 32643
Telephone Number: **(352) 316-0454**

5. Surety (if any):
   a. Name and Address:
   Telephone Number: _____
   b. Amount of Bond $_____

6. Lender: (Name and Address)
CAPITAL CITY BANK
1301 METROPOLITAN BLVD., TALLAHASSEE, FLORIDA 32308
Telephone Number: **850-402-2223**

7. Persons within the State of Florida designated by Owner upon whom notice or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: (Name and Address)
N/A

8. In addition to himself, Owner designates the following person(s) to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes: (Name and Address)
CAPITAL CITY BANK
1301 METROPOLITAN BLVD., TALLAHASSEE, FLORIDA 32308
Telephone Number: **850-402-2223**

9. Expiration date of Notice of Commencement (the expiration date is 1 year from the date of recording unless a different date is specified) _____.

IMAGE DEVELOPMENT GROUP, LLC

By: Richard Parker {SEAL}
RICHARD C. PARKER

_____ {SEAL}

Sworn to and subscribed before me this 9th day of March, 2007, by **RICHARD C. PARKER OF IMAGE DEVELOPMENT GROUP, LLC**, who is personally known to me or who has produced Dr License as identification.

Martha Bryan
Notary Public
My Commission Expires: _____

Martha Bryan
Commission # DD232534
Expires August 10, 2007
Bonded Thru Troy Fain - Insurance, Inc. 800-385-7019

Columbia County Building Permit Application

Ck# 3963

For Office Use Only Application # 0703-17 Date Received 3/7 By DW Permit # 25616

Application Approved by - Zoning Official _____ Date _____ Plans Examiner OK TH Date 3-12-06

Flood Zone _____ Development Permit _____ Zoning _____ Land Use Plan Map Category _____

Comments IN TOWN of Ft. White

NOC EH ☐ Deed or PA ☐ Site Plan ☐ State Road Info ☐ Parent Parcel # ☐ Development Permit

Fax 758-9830

Name Authorized Person Signing Permit John Norris Phone 758-3663 - 961-4549

Address 351 NW Corwin Gln.

Owners Name Image Development Group, LLC Phone _____

911 Address 320 SW Greenwood Terrace

Contractors Name John Norris Phone 961-4549

Address 351 NW Corwin Gln

Fee Simple Owner Name & Address _____

Bonding Co. Name & Address _____

Architect/Engineer Name & Address Freeman

Mortgage Lenders Name & Address _____

Circle the correct power company - FL Power & Light - (Clay Elec.) - Suwannee Valley Elec. - Progressive Energy

Property ID Number 34-6S-16-04056-130 Estimated Cost of Construction 90,000

Subdivision Name Thornwood Lot 30 Block ____ Unit ____ Phase ____

Driving Directions 47 to Ft. White left on 27 left on 18 1/2 mile on left

Type of Construction Single dewelling Number of Existing Dwellings on Property _____

Total Acreage 1.25 Lot Size _____ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 35' Side 40' Side 40' Rear _____

Total Building Height 16' 8" Number of Stories 1 Heated Floor Area 1473 - 1788 TOTAL 2630 Roof Pitch 6'

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Authorized Person by Notarized Letter

John S Norris
Contractor Signature
Contractors License Number _____
Competency Card Number _____
NOTARY STAMP/SEAL

STATE OF FLORIDA
COUNTY OF COLUMBIA

BRANDY C. LEMLEY
MY COMMISSION # DD 638237
EXPIRES: September 27, 2010
Bonded Thru Notary Public Underwriters

Sworn to (or affirmed) and subscribed before me this 1 day of March 20 07.

Personally known _____ or Produced Identification N620 404 50 378 0

Brandy Lemley
Notary Signature

(Revised Sept. 2006)

TOWN OF FORT WHITE

Home of the Ichetucknee River

Post Office Box 129 Fort White, FL 32038
Email: townofftwhite@alltel.net Web: townoffortwhitefl.com
Tel: (386) 497-2321/(386) 497-3345 Fax: (386) 497-4946
Office Hours: Monday through Friday 9:00 a.m. to 1:00 p.m

CERTIFICATE OF COMPLIANCE & REQUEST FOR ISSUANCE OF BUILDING PERMIT

The undersigned hereby certify the following property is in compliance with the Town of Fort White's Comprehensive Plan and Land Development Regulations for the stated development purposes:

OWNER'S NAME: Image Development Group, LLC

ADDRESS: P.O. Box 305 Newberry, FL 32669

PROPERTY DESCRIPTION: Lot #30 1.25 ac parcel#4056-130
w/ parcel number

DEVELOPMENT: RSF / single family dwelling

You are hereby authorized to issue the appropriate permits

2/26/07
DATE

Janice S. Kravels
LDR ADMINISTRATOR
Town of Fort White

Mayor	*District 1*	*District 2*	*District 3*	*District 4*
Truett George	Donald Cook	Henry Maini	Warren Barnes	Demetric Jackson
497-4741	497-1086	497-2992	497-3112	497-2078

```
@ CAM112M01     S  CamaUSA Appraisal System                      Columbia County
 3/07/2007 10:24  Legal Description Maintenance          40500  Land   001
Year T Property                              Sel                AG     000
2007 R 34-6S-16-04056-130 ..... ... ..                          Bldg   000
                                                                Xfea   000
       IMAGE DEVELOPMENT GROUP LLC                       40500  TOTAL       B*

  1  LOT 30 THORNWOOD S/D ..... WD 1030-2804 ..................    2
  3  ..... ..... ..... ..... ..... ..... ..... ..... ..... .....    4
  5  ..... ..... ..... ..... ..... ..... ..... ..... ..... .....    6
  7  ..... ..... ..... ..... ..... ..... ..... ..... ..... .....    8
  9  ..... ..... ..... ..... ..... ..... ..... ..... ..... .....   10
 11  ..... ..... ..... ..... ..... ..... ..... ..... ..... .....   12
 13  ..... ..... ..... ..... ..... ..... ..... ..... ..... .....   14
 15  ..... ..... ..... ..... ..... ..... ..... ..... ..... .....   16
 17  ..... ..... ..... ..... ..... ..... ..... ..... ..... .....   18
 19  ..... ..... ..... ..... ..... ..... ..... ..... ..... .....   20
 21  ..... ..... ..... ..... ..... ..... ..... ..... ..... .....   22
 23  ..... ..... ..... ..... ..... ..... ..... ..... ..... .....   24
 25  ..... ..... ..... ..... ..... ..... ..... ..... ..... .....   26
 27  ..... ..... ..... ..... ..... ..... ..... ..... ..... .....   28
                                                Mnt 12/03/2004 LARRY
F1=Task  F3=Exit  F4=Prompt  F10=GoTo  PgUp/PgDn  F24=More
```

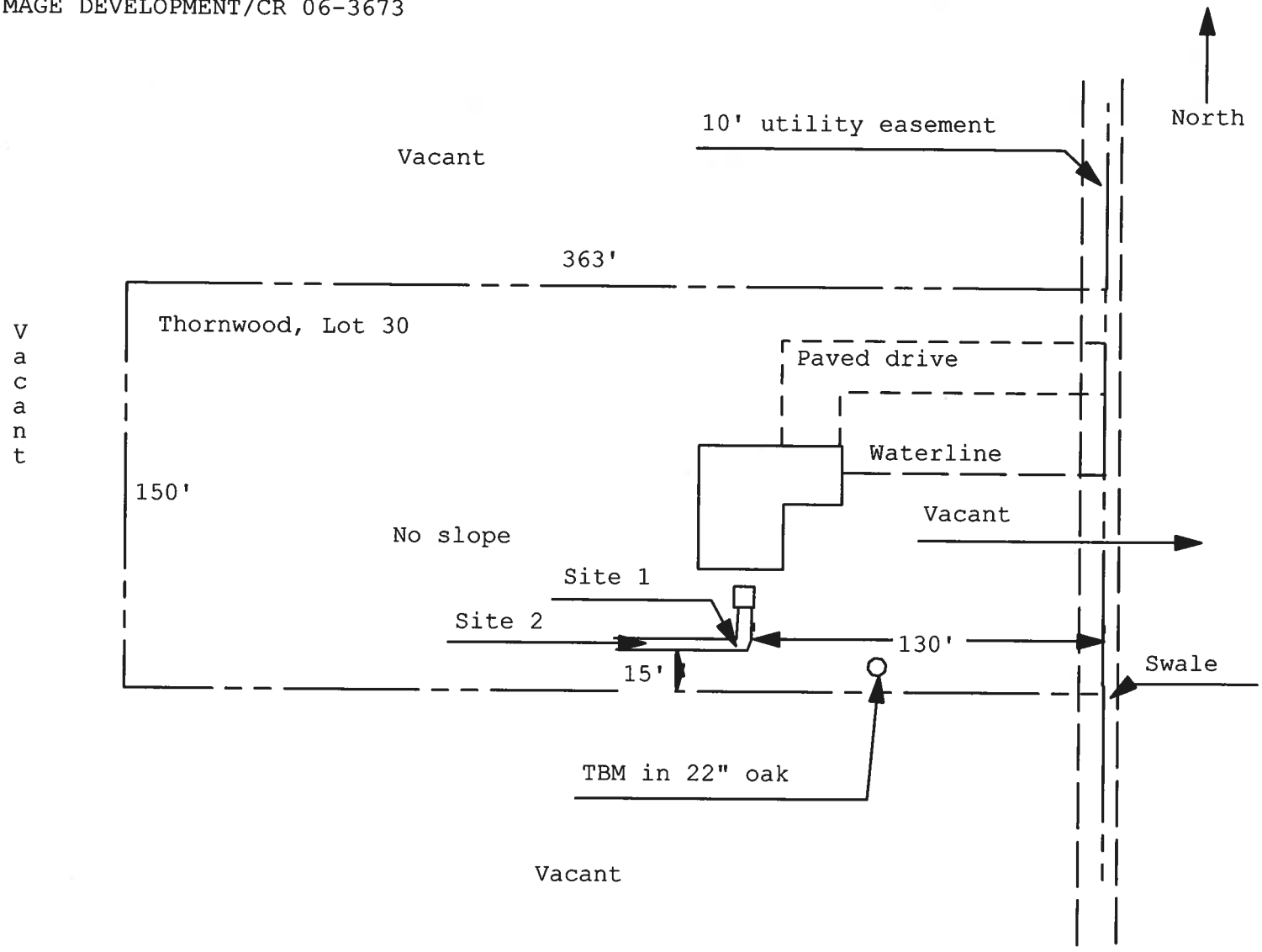
07-200

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 12-56-112536

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

IMAGE DEVELOPMENT/CR 06-3673



1 inch = 60 feet

Site Plan Submitted By Paul Lloyd **Date** 9/6/06

Plan Approved ✓ **Not Approved** ____ **Date** 3/9/07

By _____ Colvbil **CPHU**

Notes: _____

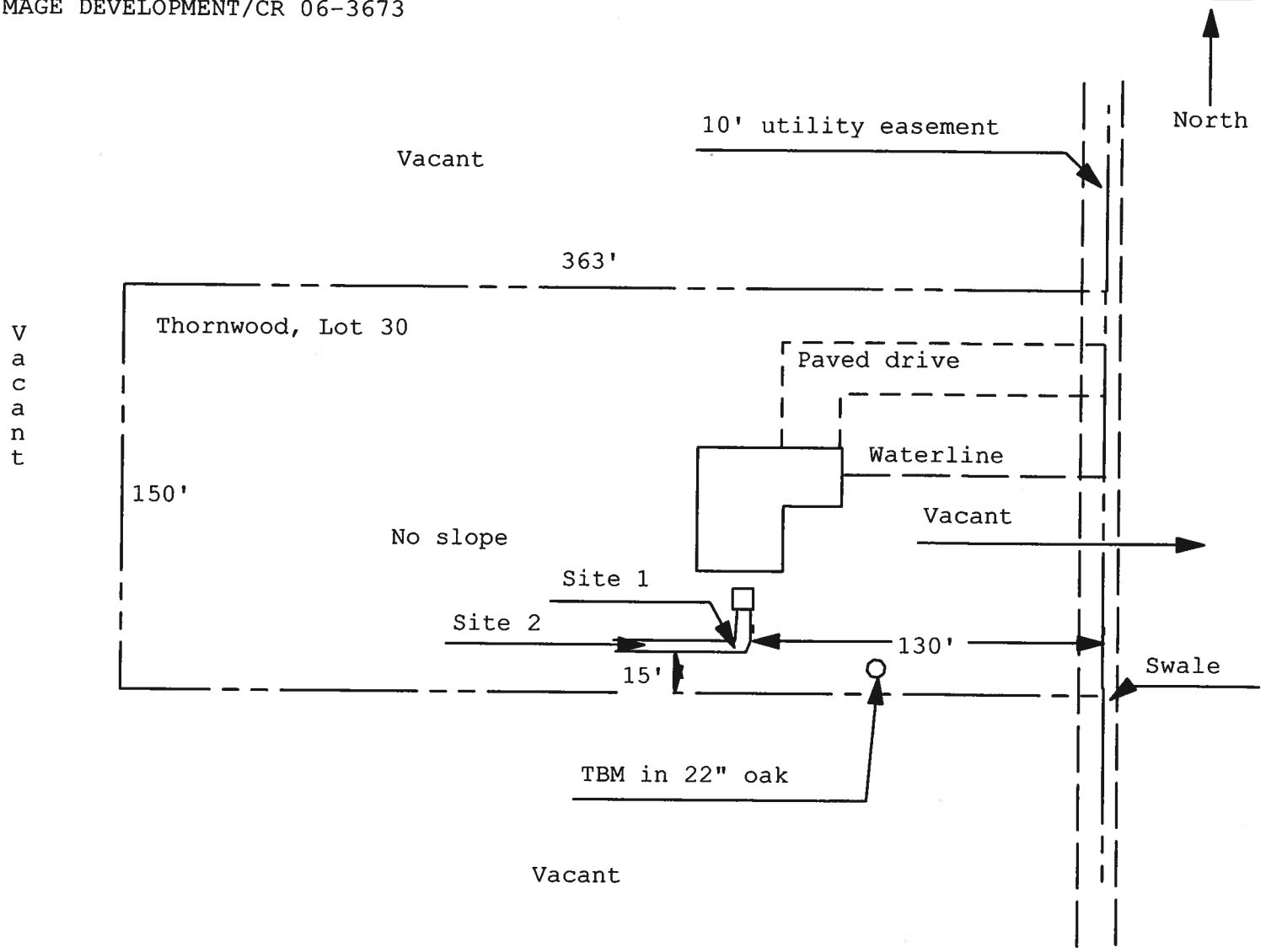
07-200

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 12-SG-112536

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

IMAGE DEVELOPMENT/CR 06-3673



1 inch = 60 feet

Site Plan Submitted By Paul L [illegible] **Date** 9/6/06

Plan Approved ✓ **Not Approved** _____ **Date** 3/9/07

By [signature] [signature] **CPHU**

Notes: _____

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Thornwood Lot 30**	Builder:	John Norris
Address:	**Lot: 30, Sub: Thronwood, Plat:**	Permitting Office:	Columbia Co.
City, State:	**Lake City, Fl 32055-**	Permit Number:	25616
Owner:	**Norris**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing		New	
2. Single family or multi-family		Single family	
3. Number of units, if multi-family		1	
4. Number of Bedrooms		3	
5. Is this a worst case?		Yes	
6. Conditioned floor area (ft²)		1473 ft²	
7. Glass area & type	Single Pane	Double Pane	
a. Clear glass, default U-factor	0.0 ft²	179.0 ft²	
b. Default tint	0.0 ft²	0.0 ft²	
c. Labeled U or SHGC	0.0 ft²	0.0 ft²	
8. Floor types			
a. Slab-On-Grade Edge Insulation		R=0.0, 210.0(p) ft	
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior		R=13.0, 1680.0 ft²	
b. N/A			
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic		R=30.0, 1620.3 ft²	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior		Sup. R=6.0, 67.0 ft	
b. N/A			

12. Cooling systems		
a. Central Unit	Cap: 32.0 kBtu/hr	
	SEER: 13.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 32.0 kBtu/hr	
	HSPF: 8.00	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 50.0 gallons	
	EF: 0.90	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	MZ-C, PT, CF,	

Glass/Floor Area: 0.12

Total as-built points: 20578

Total base points: 25056

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Tallon Sullens

DATE: 8-1-06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 30, Sub: Thronwood, Plat: , Lake City, Fl, 32055-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points	
.18	1473.0	20.04	5313.4	Double, Clear	W	1.5	6.0	15.0	38.52	0.91	527.8	
				Double, Clear	W	1.5	4.0	9.0	38.52	0.82	283.5	
				Double, Clear	W	1.5	6.0	25.0	38.52	0.91	879.7	
				Double, Clear	E	1.5	6.0	30.0	42.06	0.91	1151.8	
				Double, Clear	E	1.5	6.0	50.0	42.06	0.91	1919.6	
				Double, Clear	E	1.5	6.0	50.0	42.06	0.91	1919.6	
				As-Built Total:				**179.0**			**6682.0**	
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1680.0	1.50		2520.0	
Exterior	1680.0	1.70	2856.0									
Base Total:	**1680.0**		**2856.0**	**As-Built Total:**				**1680.0**			**2520.0**	
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points	
Adjacent	0.0	0.00	0.0	Exterior Wood				17.7	6.10		107.8	
Exterior	58.5	6.10	356.7	Exterior Wood				40.8	6.10		248.9	
Base Total:	**58.5**		**356.7**	**As-Built Total:**				**58.5**			**356.7**	
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points	
Under Attic	1473.0	1.73	2548.3	Under Attic			30.0	1620.3	1.73 X 1.00		2803.1	
Base Total:	**1473.0**		**2548.3**	**As-Built Total:**				**1620.3**			**2803.1**	
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points	
Slab	210.0(p)	-37.0	-7770.0	Slab-On-Grade Edge Insulation			0.0	210.0(p	-41.20		-8652.0	
Raised	0.0	0.00	0.0									
Base Total:			**-7770.0**	**As-Built Total:**				**210.0**			**-8652.0**	
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points	
	1473.0	10.21	15039.3					1473.0	10.21		15039.3	

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 30, Sub: Thronwood, Plat: , Lake City, Fl, 32055-** PERMIT #:

BASE			AS-BUILT					
Summer Base Points:		**18343.8**	**Summer As-Built Points:**					**18749.1**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
18343.8	**0.4266**	**7825.4**	18749.1 **18749.1**	1.000 **1.00**	(1.090 x 1.147 x 0.91) **1.138**	0.263 **0.263**	0.857 **0.857**	4801.5 **4801.5**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 30, Sub: Thronwood, Plat: , Lake City, Fl, 32055-** PERMIT #:

BASE				AS-BUILT								
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points	
.18	1473.0	12.74	3377.9	Double, Clear	W	1.5	6.0	15.0	20.73	1.02	318.2	
				Double, Clear	W	1.5	4.0	9.0	20.73	1.05	196.4	
				Double, Clear	W	1.5	6.0	25.0	20.73	1.02	530.4	
				Double, Clear	E	1.5	6.0	30.0	18.79	1.04	583.8	
				Double, Clear	E	1.5	6.0	50.0	18.79	1.04	973.0	
				Double, Clear	E	1.5	6.0	50.0	18.79	1.04	973.0	
				As-Built Total:				**179.0**			**3574.8**	
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior			13.0	1680.0	3.40		5712.0	
Exterior	1680.0	3.70	6216.0									
Base Total:	**1680.0**		**6216.0**	**As-Built Total:**				**1680.0**			**5712.0**	
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points	
Adjacent	0.0	0.00	0.0	Exterior Wood				17.7	12.30		217.5	
Exterior	58.5	12.30	719.3	Exterior Wood				40.8	12.30		501.8	
Base Total:	**58.5**		**719.3**	**As-Built Total:**				**58.5**			**719.3**	
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points	
Under Attic	1473.0	2.05	3019.6	Under Attic			30.0	1620.3	2.05 X	1.00	3321.6	
Base Total:	**1473.0**		**3019.6**	**As-Built Total:**				**1620.3**			**3321.6**	
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points	
Slab	210.0(p)	8.9	1869.0	Slab-On-Grade Edge Insulation			0.0	210.0(p	18.80		3948.0	
Raised	0.0	0.00	0.0									
Base Total:			**1869.0**	**As-Built Total:**				**210.0**			**3948.0**	
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points	
	1473.0	-0.59	-869.1					1473.0	-0.59		-869.1	

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 30, Sub: Thronwood, Plat: , Lake City, Fl, 32055-** PERMIT #:

BASE			AS-BUILT						
Winter Base Points:		**14332.8**	**Winter As-Built Points:**						**16406.6**
Total Winter Points	X System Multiplier	= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
14332.8	**0.6274**	**8992.4**	16406.6 **16406.6**	1.000 **1.00**	(1.069 x 1.169 x 0.93) **1.162**	0.426 **0.426**	0.950 **0.950**	7721.2 **7721.2**

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 30, Sub: Thronwood, Plat: , Lake City, Fl, 32055-** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
3		2746.00	8238.0	50.0	0.90	3	1.00	2684.98	1.00	8054.9	
				As-Built Total:						**8054.9**	

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
7825	**8992**	**8238**	**25056**	**4801**	**7721**	**8055**	**20578**

PASS

FORM 600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 30, Sub: Thronwood, Plat: , Lake City, Fl, 32055-** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 86.4

The higher the score, the more efficient the home.

Norris, Lot: 30, Sub: Thronwood, Plat: , Lake City, Fl, 32055-

1. New construction or existing		New	
2. Single family or multi-family		Single family	
3. Number of units, if multi-family		1	
4. Number of Bedrooms		3	
5. Is this a worst case?		Yes	
6. Conditioned floor area (ft²)		1473 ft²	
7. Glass area & type	Single Pane	Double Pane	
a. Clear - single pane	0.0 ft²	179.0 ft²	
b. Clear - double pane	0.0 ft²	0.0 ft²	
c. Tint/other SHGC - single pane	0.0 ft²	0.0 ft²	
d. Tint/other SHGC - double pane			
8. Floor types			
a. Slab-On-Grade Edge Insulation		R=0.0, 210.0(p) ft	
b. N/A			
c. N/A			
9. Wall types			
a. Frame, Wood, Exterior		R=13.0, 1680.0 ft²	
b. N/A			
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic		R=30.0, 1620.3 ft²	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Interior		Sup. R=6.0, 67.0 ft	
b. N/A			

12. Cooling systems		
a. Central Unit	Cap: 32.0 kBtu/hr	
	SEER: 13.00	
b. N/A		
c. N/A		
13. Heating systems		
a. Electric Heat Pump	Cap: 32.0 kBtu/hr	
	HSPF: 8.00	
b. N/A		
c. N/A		
14. Hot water systems		
a. Electric Resistance	Cap: 50.0 gallons	
	EF: 0.90	
b. N/A		
c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)		
15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	MZ-C, PT, CF,	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/488-1824.*

Residential System Sizing Calculation

Summary

Norris

Lake City, Fl 32055-

Project Title:
Thornwood Lot 30

Code Only
Professional Version
Climate: North

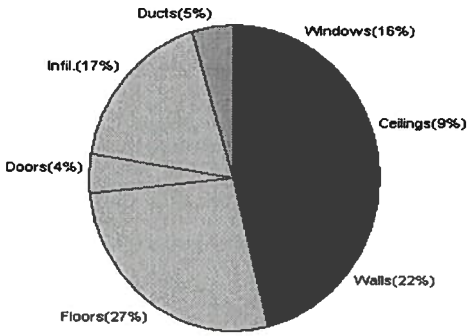
8/1/2006

Location for weather data: Gainesville - User customized: Latitude(29) Temp Range(M)					
Humidity data: Interior RH (50%) Outdoor wet bulb (78F) Humidity difference(51gr.)					
Winter design temperature	31	F	Summer design temperature	98	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	39	F	Summer temperature difference	23	F
Total heating load calculation	**24223**	**Btuh**	**Total cooling load calculation**	**22313**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	132.1	32000	Sensible (SHR = 0.5)	89.2	16000
Heat Pump + Auxiliary(0.0kW)	132.1	32000	Latent	366.5	16000
			Total (Electric Heat Pump)	143.4	32000

WINTER CALCULATIONS

Winter Heating Load (for 1473 sqft)

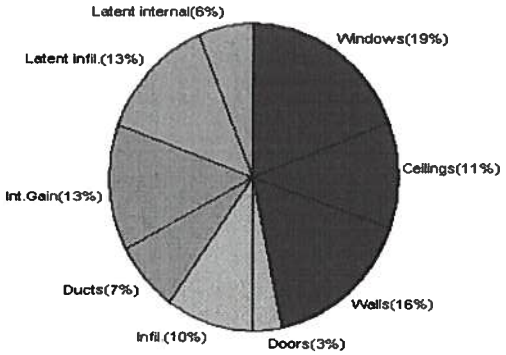
Load component			Load	
Window total	179	sqft	3849	Btuh
Wall total	1680	sqft	5208	Btuh
Door total	58	sqft	1049	Btuh
Ceiling total	1620	sqft	2106	Btuh
Floor total	210	ft	6636	Btuh
Infiltration	98	cfm	4221	Btuh
Subtotal			**23069**	**Btuh**
Duct loss			1153	Btuh
TOTAL HEAT LOSS			**24223**	**Btuh**



SUMMER CALCULATIONS

Summer Cooling Load (for 1473 sqft)

Load component			Load	
Window total	179	sqft	4296	Btuh
Wall total	1680	sqft	3595	Btuh
Door total	58	sqft	718	Btuh
Ceiling total	1620	sqft	2528	Btuh
Floor total			0	Btuh
Infiltration	86	cfm	2178	Btuh
Internal gain			3000	Btuh
Subtotal(sensible)			**16315**	**Btuh**
Duct gain			1632	Btuh
Total sensible gain			**17947**	**Btuh**
Latent gain(infiltration)			2986	Btuh
Latent gain(internal)			1380	Btuh
Total latent gain			**4366**	**Btuh**
TOTAL HEAT GAIN			**22313**	**Btuh**



EnergyGauge® System Sizing based on ACCA Manual J.
PREPARED BY: Allison Sterling
DATE: 8-1-06

System Sizing Calculations - Winter

Residential Load - Component Details

Norris
Lake City, Fl 32055-

Project Title:
Thornwood Lot 30

Code Only
Professional Version
Climate: North

Reference City: Gainesville (User customized) Winter Temperature Difference: 39.0 F

8/1/2006

Window	Panes/SHGC/Frame/U	Orientation	Area X	HTM=	Load
1	2, Clear, Wood, DEF	N	15.0	21.5	322 Btuh
2	2, Clear, Wood, DEF	N	9.0	21.5	194 Btuh
3	2, Clear, Wood, DEF	N	25.0	21.5	538 Btuh
4	2, Clear, Wood, DEF	S	30.0	21.5	645 Btuh
5	2, Clear, Wood, DEF	S	50.0	21.5	1075 Btuh
6	2, Clear, Wood, DEF	S	50.0	21.5	1075 Btuh
	Window Total		179		3849 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Exterior	13.0	1680	3.1	5208 Btuh
	Wall Total		1680		5208 Btuh
Doors	Type		Area X	HTM=	Load
1	Wood - Exter		18	17.9	317 Btuh
2	Wood - Exter		41	17.9	732 Btuh
	Door Total		58		1049Btuh
Ceilings	Type	R-Value	Area X	HTM=	Load
1	Under Attic	30.0	1620	1.3	2106 Btuh
	Ceiling Total		1620		2106Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab-On-Grade Edge Insul	0	210.0 ft(p)	31.6	6636 Btuh
	Floor Total		210		6636 Btuh
Infiltration	Type	ACH X	Building Volume	CFM=	Load
	Natural	0.40	14730(sqft)	98	4221 Btuh
	Mechanical			0	0 Btuh
	Infiltration Total			98	4221 Btuh

Totals for Heating	**Subtotal**	**23069 Btuh**
	Duct Loss(using duct multiplier of 0.05)	**1153 Btuh**
	Total Btuh Loss	**24223 Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)

Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

System Sizing Calculations - Summer

Residential Load - Component Details

Norris

Lake City, Fl 32055-

Project Title:
Thornwood Lot 30

Code Only
Professional Version
Climate: North

Reference City: Gainesville (User customized) Summer Temperature Difference: 23.0 F 8/1/2006

Window	Type Panes/SHGC/U/InSh/ExSh Ornt		Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load	
1	2, Clear, DEF, N, N	N	1.5	6	15.0	0.0	15.0	24	24	360	Btuh
2	2, Clear, DEF, N, N	N	1.5	4	9.0	0.0	9.0	24	24	216	Btuh
3	2, Clear, DEF, N, N	N	1.5	6	25.0	0.0	25.0	24	24	600	Btuh
4	2, Clear, DEF, N, N	S	1.5	6	30.0	30.0	0.0	24	39	720	Btuh
5	2, Clear, DEF, N, N	S	1.5	6	50.0	50.0	0.0	24	39	1200	Btuh
6	2, Clear, DEF, N, N	S	1.5	6	50.0	50.0	0.0	24	39	1200	Btuh
	Window Total				179					4296	Btuh

Walls	Type	R-Value	Area	HTM	Load	
1	Frame - Exterior	13.0	1680.0	2.1	3595	Btuh
	Wall Total		1680.0		3595	Btuh

Doors	Type	Area	HTM	Load	
1	Wood - Exter	17.7	12.3	217	Btuh
2	Wood - Exter	40.8	12.3	501	Btuh
	Door Total	58.5		718	Btuh

Ceilings	Type/Color	R-Value	Area	HTM	Load	
1	Under Attic/Dark	30.0	1620.3	1.6	2528	Btuh
	Ceiling Total		1620.3		2528	Btuh

Floors	Type	R-Value	Size	HTM	Load	
1	Slab-On-Grade Edge Insulation	0.0	210.0 ft(p)	0.0	0	Btuh
	Floor Total		210.0		0	Btuh

Infiltration	Type	ACH	Volume	CFM=	Load	
	Natural	0.35	14730	86.1	2178	Btuh
	Mechanical			0	0	Btuh
	Infiltration Total			86	2178	Btuh

Internal gain		Occupants		Btuh/occupant		Appliance	Load	
		6	X	300	+	1200	3000	Btuh

Totals for Cooling	**Subtotal**	**16315**	**Btuh**
	Duct gain(using duct multiplier of 0.10)	**1632**	**Btuh**
	Total sensible gain	**17947**	**Btuh**
	Latent infiltration gain (for 51 gr. humidity difference)	**2986**	**Btuh**
	Latent occupant gain (6 people @ 230 Btuh per person)	**1380**	**Btuh**
	Latent other gain	**0**	**Btuh**
	TOTAL GAIN	**22313**	**Btuh**

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds/Daperies(B) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(Ornt - compass orientation)

MIAMI-DADE
BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Ceco Door Products
9159 Telecom Drive
Milan, TN 38358

In Swing

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: The Ceco Series Single Flush / Embossed Inswing Commercial Steel Doors –Impact

APPROVAL DOCUMENT: Drawing No RD0728, titled "3-0 x 7-0 , Series Regent, Omega, Imperial, Versa door", prepared by manufacturer, sheets 1 through 9 of 9 dated 05/22/02 and latest revised on 10-10-02, bearing the Miami-Dade County Product Control Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

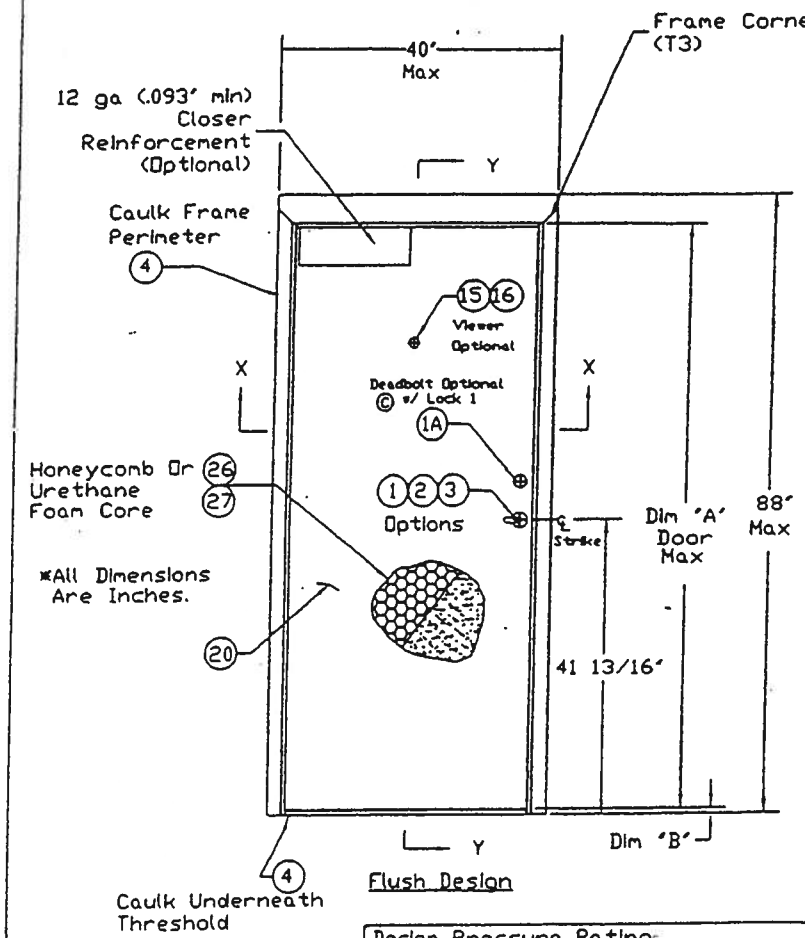
TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

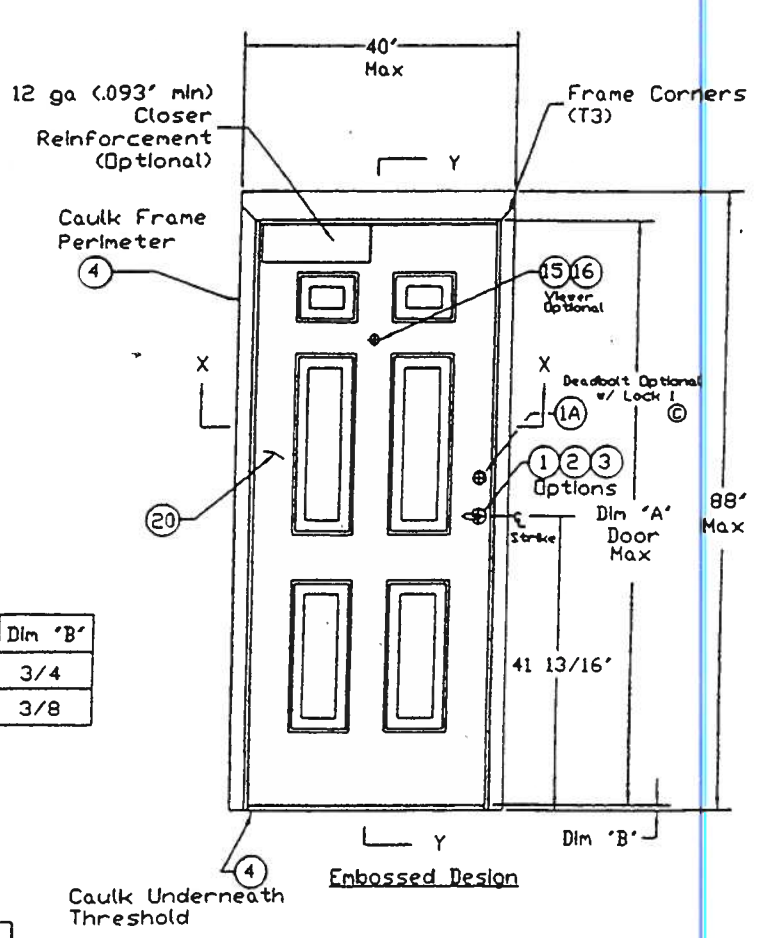
ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

This NOA consists of this page 1 as well as approval document mentioned above.

The submitted documentation was reviewed by **Ishaq I. Chanda, P.E.**





In-Swing Door (Exterior View)

	Dim "A"	Dim "B"
3/4" Undercut	83 1/8	3/4
3/8" Undercut	83 1/2	3/8

Approved as complying with the Florida Building Code
Date OCTOBER 31, 2002
NOA# 02-0807.04
Miami Dade Product Control Division
By Ishaq I. Chanda

Design Pressure Rating		
	Where Water Infiltration Requirement Is Needed	Where Water Infiltration Requirement Is Not Needed
Positive	Not Approved	+70 PSF
Negative	Not Approved	-70 PSF

Notes:
1) In-swing Not Approved For Water Infiltration
2) This Door Does Not Need A Hurricane Protection Systen
3) Hinge Spacing Is 33" O.C., 13" From Top Of Frame & 9" From The Botton.

MATERIAL SPECIFICATIONS:
Finish: Rust Inhibitive Primer

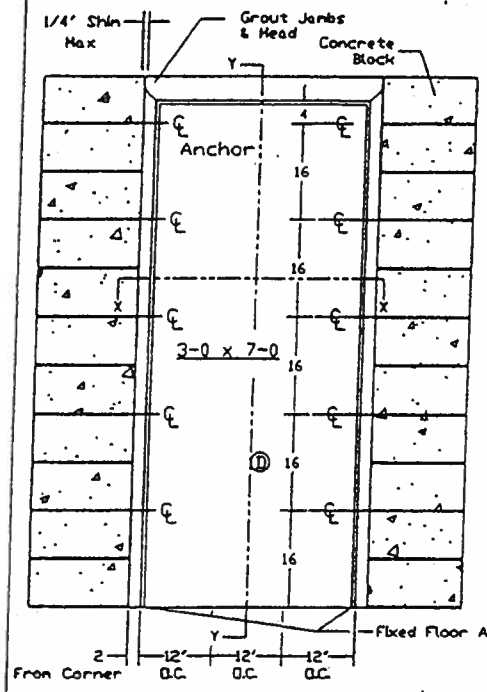
3-0 x 7-0 Series
Regent, Omega, Imperial, & Versadoor
In-Swing Elevation Drawing

CECO DOOR PRODUCTS
Milan, Tennessee 38358

ISSUE	REVISIONS
B 10/10/02 LT	Revised Per Marked Up Drawings From Ishaq Chanda.
A 8/28/02 LT	Revised Per Marked Up Drawings From Ishaq Chanda.
DRAWN BY: LT	DATE: 5/22/02
DRAWING NUMBER: RD0728 Sheet 1 of 9	

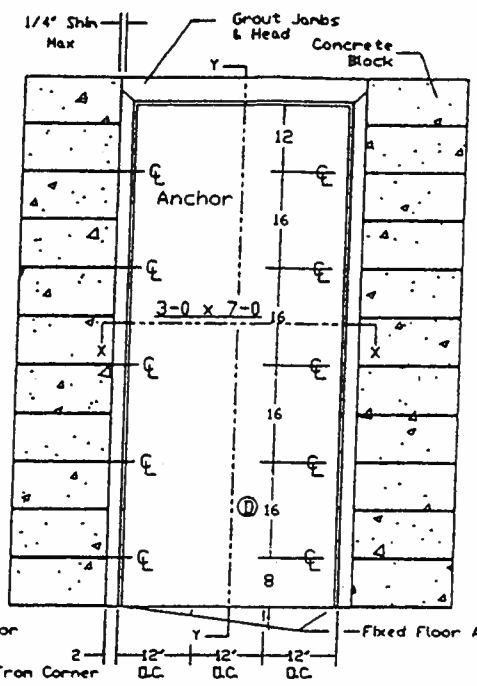
Masonry "T" Anchor

Min. 3500 PSI



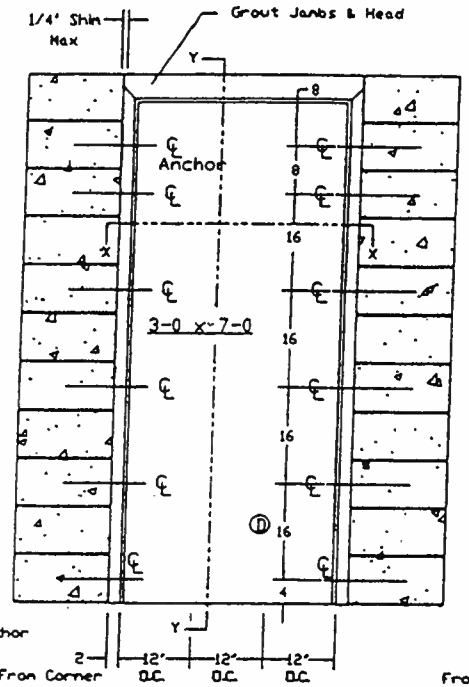
Masonry Wire Anchor

Min. 3500 PSI

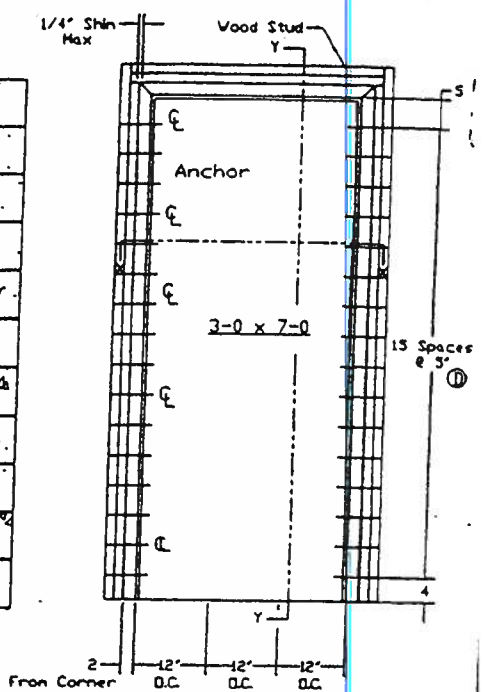


Existing Opening W/Lockbolt or Sleeve Anchor Into Block

Min. 3500 PSI

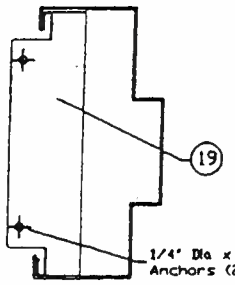


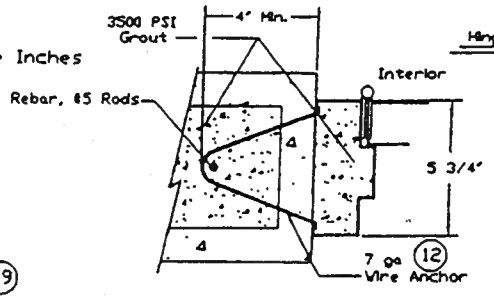
Existing Opening Anchor Into Wood Stu

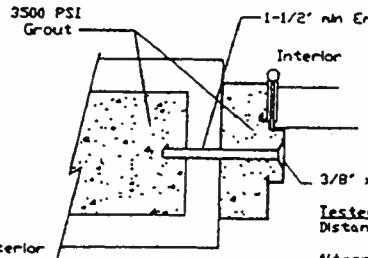


*All Dimensions Are Inches

Installation Details Hinge Jambs / Lock Jambs

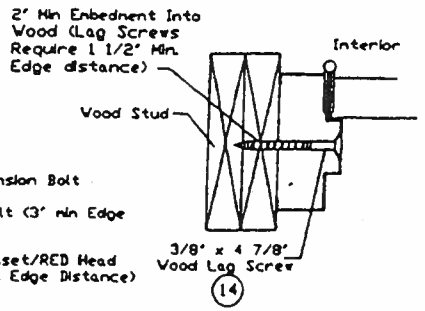


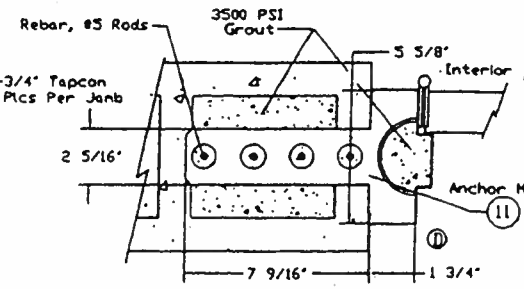


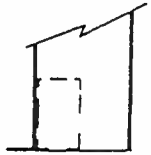


Tested Rawl Lok/Bolt (3" min Edge Distance)

Alternative ITV Ranset/RED Head Dynabolt (2-1/2" min Edge Distance)







MATERIAL SPECIFICATIONS:

Frame Anchor (Inswing Doors) Regent, Omega, Imperial & Versadoor Installation Details

CECO DOOR PRODUCTS
Milan, Tennessee 38358

Approved as complying with the Florida Building Code
Date OCT 3 2002
NOA# 02-0807.04
Miami Dade Product Control Division
By Ishaq I. Chanda

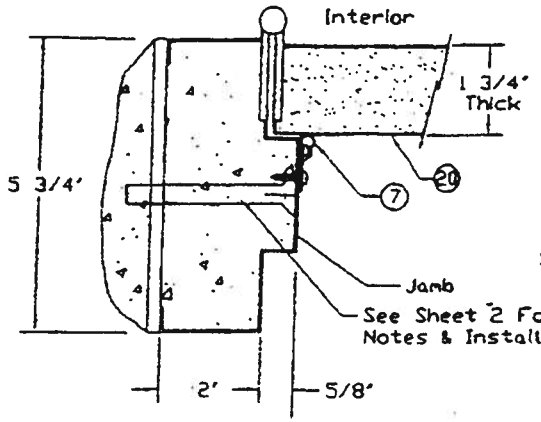
ISSUE	REVISIONS
A 5/22/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.

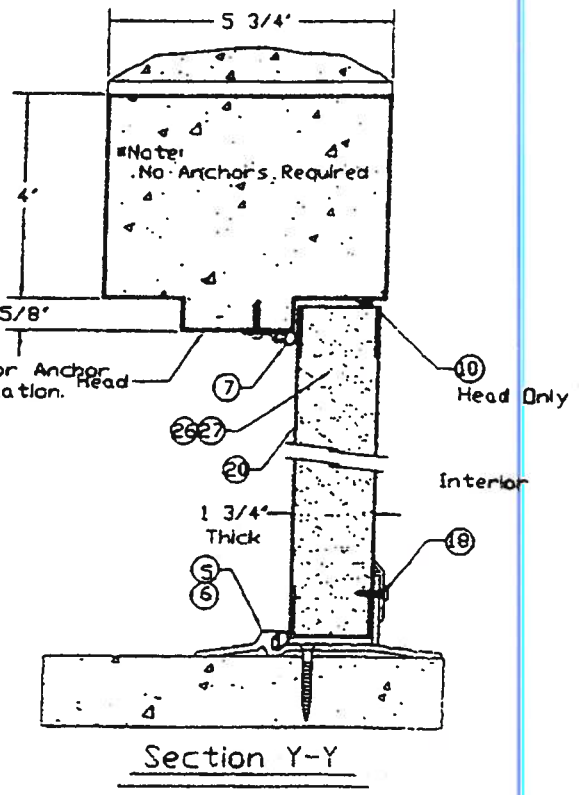
DRAWN BY: LT
DATE: 5/22/02

DRAWING NUMBER: RD0728

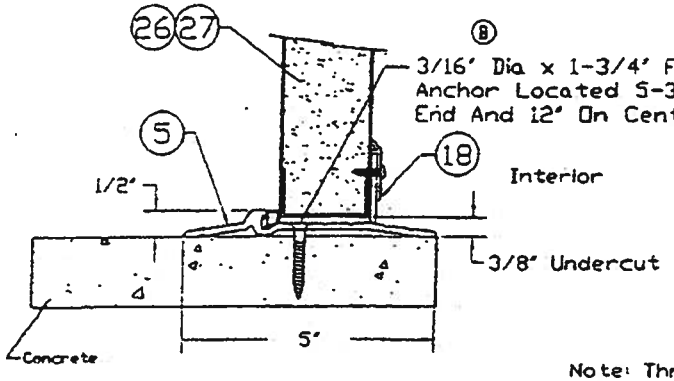
Sheet 2 of 9

*Note: Structural Member At Header Must Be Designed To carry 58.3#/ft load Imposed And Must Be Reviewed By Building Official.





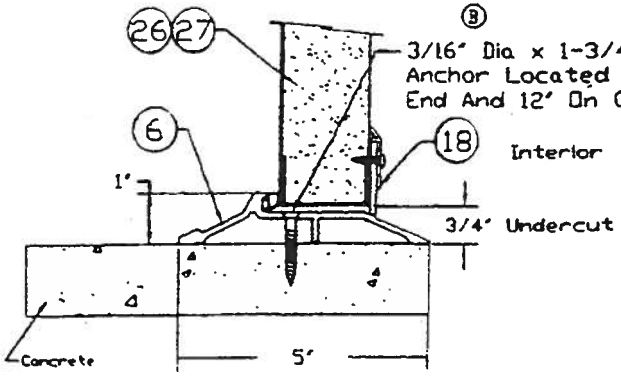
Section Y-Y



Threshold Pemko 200SAV

Inswing
(Not Approved For Water)

Note: Thresholds Not Approved For Water.



Threshold Pemko 181AV

Approved as complying with the Florida Building Code
Date OCT 31, 2002
NOA# 02-0807.04
Miami Dade Product Control Division
By Ishaq I. Chanda

B	LT	Revised Per Marked-Up Drawings From Ishaq Chanda
C	LT	Revised Per Marked-Up Drawings From Ishaq Chanda

MATERIAL SPECIFICATIONS:

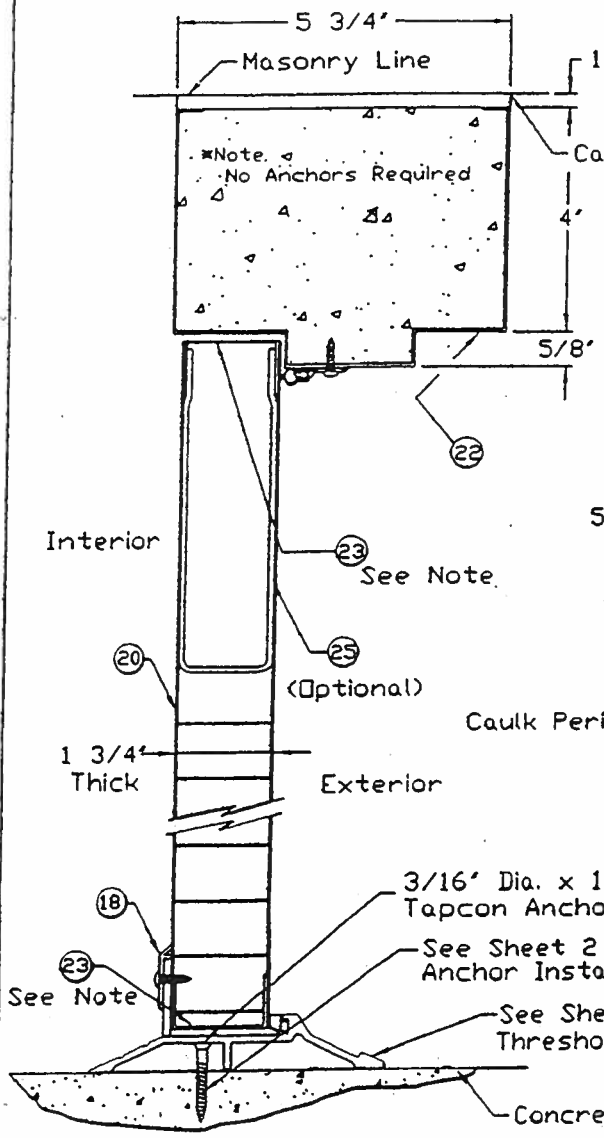
Threshold & Weatherstrip (Inswing Doors)
Regent, Omega, Imperial, Versadoor
Installation Details

CECO DOOR PRODUCTS
Milan, Tennessee 38358

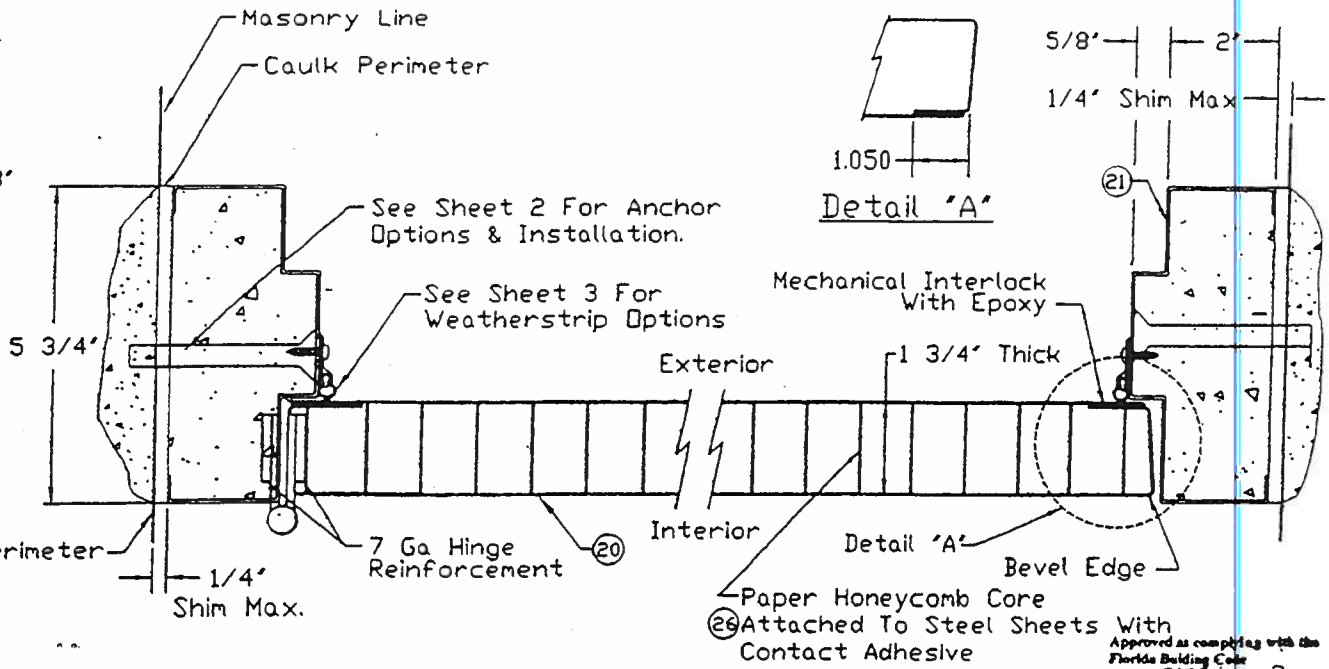
ISSUE	REVISIONS
DRAWN BY: LT	DATE: 5/22/02

RD0728
Sheet 3 Of 9

*Note: Structural Member At Head Must Be Designed To Carry 58.3 #/ft. Load Imposed And Must Be Reviewed By Building Official.



Section Y-Y



Section X-X

Note 1: Top and Bottom Channel Tack Welded To Both Skins 3 Inches From Lock Edge And 6 Inches On Centers

Approved as complying with the Florida Building Code
Date 0CT31,2002
NOA# 02-0207.04
Miami Dade Product Control Division
By Ishaq Chanda

C	10/28/02 LT	Revised Per Marked-up Drawings From Ishaq Chanda.
B	10/28/02 LT	Revised Per Marked-up Drawings From Ishaq Chanda.

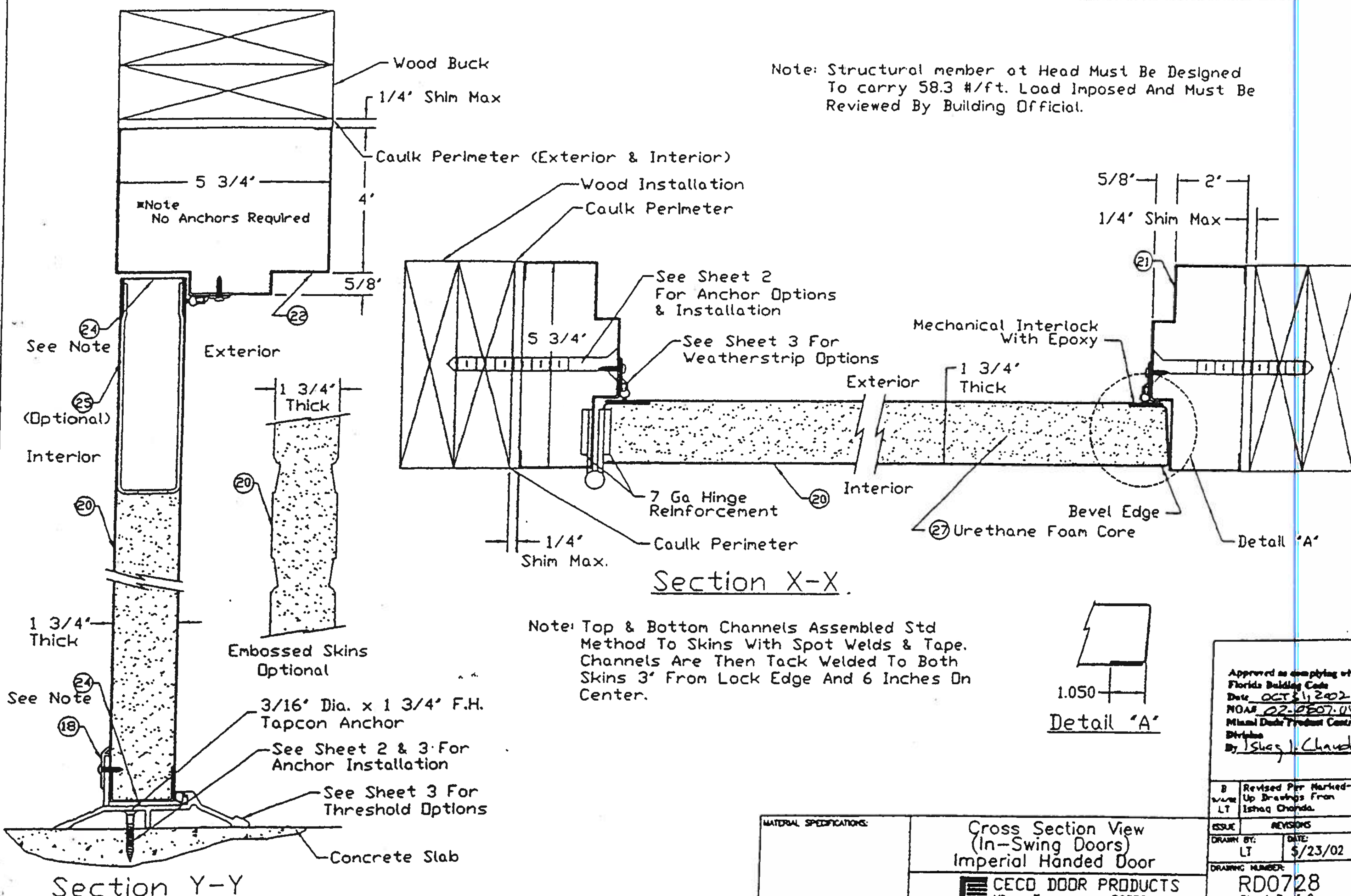
ISSUE	REVISIONS
DRAWN BY: LT	DATE: 5/22/02

MATERIAL SPECIFICATIONS:

Cross Section View (Inswing Doors) Regent Handed Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWING NUMBER: RD0728
Sheet 5 of 9

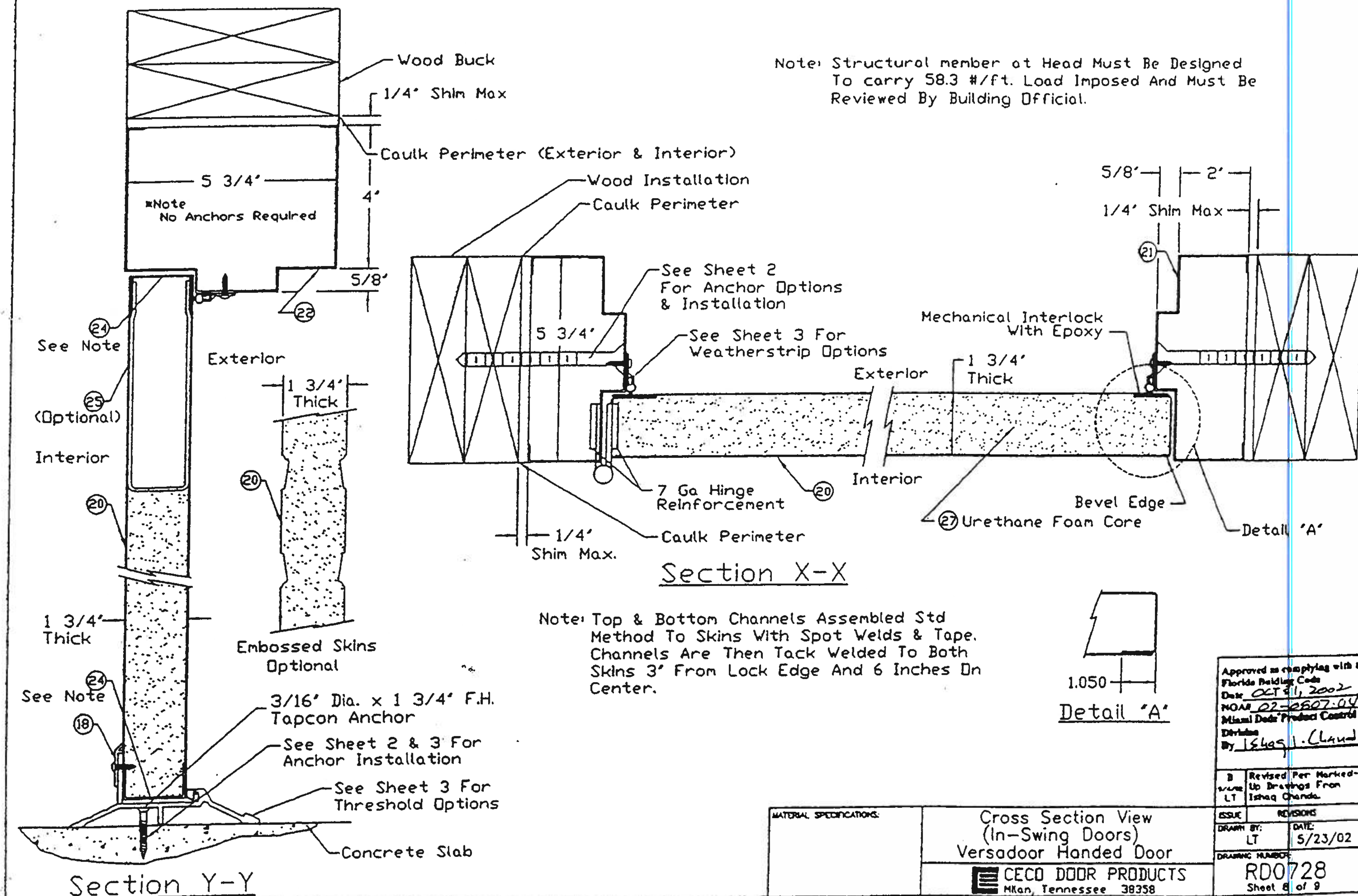


Note: Structural member at Head Must Be Designed To carry 58.3 #/ft. Load Imposed And Must Be Reviewed By Building Official.

Note: Top & Bottom Channels Assembled Std Method To Skins With Spot Welds & Tape. Channels Are Then Tack Welded To Both Skins 3' From Lock Edge And 6 Inches On Center.

Approved as complying with the Florida Building Code
Date OCT 31, 2002
NOA# 02-0807.04
Miami Dade Product Control Division
By Ishaq I. Chanda

MATERIAL SPECIFICATIONS:	
Cross Section View (In-Swing Doors) Imperial Handed Door	
CECO DOOR PRODUCTS, Milan, Tennessee 38358	
B	Revised Per Marked-Up Drawings From Ishaq Chanda. LT
ISSUE	REVISIONS
DRAWN BY: LT	DATE: 5/23/02
DRAWING NUMBER: RD0728	Sheet 7 of 9



Note: Structural member at Head Must Be Designed To carry 58.3 #/ft. Load Imposed And Must Be Reviewed By Building Official.

Note: Top & Bottom Channels Assembled Std Method To Skins With Spot Welds & Tape. Channels Are Then Tack Welded To Both Skins 3" From Lock Edge And 6 Inches On Center.

MATERIAL SPECIFICATIONS:	Cross Section View (In-Swing Doors) Versadoor Handed Door CECO DOOR PRODUCTS Milan, Tennessee 38358	Approved as complying with the Florida Building Code Date OCT 11, 2002 NOA# 02-0607.04 Miami Dade Product Control Division By Ishaq I. Chanda

ISSUE	REVISIONS	
B LT	Revised Per Marked-Up Drawings From Ishaq Chanda.	
DRAWN BY: LT	DATE: 5/23/02	
DRAWING NUMBER: RD0728	Sheet 8 of 9	

1	Cylindrical Lock & Lock Reinforcement (RD0528)	Schlage	AL53PD
1A	Deadbolt (Optional) Ⓓ	Schlage	B100
2	Or Cylindrical Lock & Lock Reinforcement	Saflok	Premier SL2500
3	Or Mortise Lock	Saflok	MT
4	Caulk	Dow Corning	899 Silicone Glazing Sealant
5	Threshold	Pemko	200SAV36
6	Or	Pemko	181AV36
7	Weatherstrip	Pemko	303AV3684
8	Hinge (Ball Bearing)	Hager or Equal (Attached w/ (8) #12-24 x 1/2 MS Per Hinge)	4-1/2 x 4-1/2 x .134 (Std Weight)
9	Or (Spring)	Hager or Equal (Attached w/ (8) #12-24 x 1/2 MS Per Hinge)	4-1/2 x 4-1/2 x .134 (Std Weight)
10	Weatherstrip	Pemko	S88
11	Frame Anchor	Masonary Tee (RD0057)	16 ga (.053" min) Galv Steel Fymin = 30ksi
12	Or	Wire, Relaxed Dimension 9' x 8'	#7 (.167 " min) Galv Steel Wire (70,000 - 90,000 psi Tensile Strength)
13	Or	Expansion Bolt	3/8" x 5" F.H. Rawl Lok/Bolt Or 3/8" x 5" F.H. Ranset/RED Head
14	Or	Wood Lag Screw	3/8" x 4-5/8"
15	Viewer	Hager	1755
16	Or	MAG Security	8724-C
17	Drip Cap; Top	Pemko	346
18	Sweep	Pemko	315 N
19	Floor Anchor	Fixed Floor Anchor	16 ga (.053" min) galvanized Steel
20	Face Sheet A60 Galv Conforming To ASTM A653	Connercial Steel Type B (Minimum Yield Strength 30,000psi)	16 Ga (.053" min)
21	Series SF, Frame Jamb, Double Rabbet Profile, A60 Galv Conforming To ASTM A653	16 Ga (.053" min) Connercial Steel Type B (Minimum Yield Strength 30,000psi)	2" Face, 5-3/4" Depth Min. (RD0033)
22	Series SF, Frame Head, Double Rabbet, Profile A60 Galv Conforming To ASTM A653	16 Ga (.053" min) Connercial Steel Type B (Minimum Yield Strength 30,000psi)	4" Face, 5-3/4" Depth Min. (RD0033)
23	Door Channels; Spot Welded To Bottom Skin Glued To Top Skin; Tack Welded To Both	16 Ga (.053" min) A60 Galv Conforming To ASTM A653 Connercial Steel Type B (Minimum Yield Strength 30,000psi)	16 ga (.053" min) x 1" x 1-3/4" x 1"
24	Door Channels; Spot Welded To Bottom Skin Taped To Top Skin; Tack Welded To Both	16 Ga (.053" min) A60 Galv Conforming To ASTM A653 Connercial Steel Type B (Minimum Yield Strength 30,000psi)	16 ga (.053" min x 1" x 1-3/4" x 1"
25	Closer Reinforcement (Optional)	12 Ga (.093" min) CS Type B	12 ga (.093" min) x 5-3/8" x 16"
26	Honeycomb Core	Non-Impregnated Kraft Paper Ⓔ	1.2" Nominal Cell Size
27	Urethane Core	Foam Enterprises	2 lb/ft³ Density

Approved as complying with the Florida Building Code
Date Oct 31, 2002
NOA# 02-0607.04
Miami Dade Product Control Division
By Ishaq I. Chanda

B	10/10/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.
A	9/4/02 LT	Revised Per Marked-Up Drawings From Ishaq Chanda.

MATERIAL SPECIFICATIONS:

3-0 x 7-0 Series In-Swing Bill Of Materials

CECO DOOR PRODUCTS
Milan, Tennessee 38358

ISSUE	REVISIONS
DRAWN BY: LT	DATE: 5/28/02
DRAWING NUMBER: RD0728	
Sheet 9 Of 9	

MIAMI-DADE

BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Ceco Door Products
9159 Telecom Drive
Milan, TN 38358

outswing

SCOPE:
This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: Series "Regent" & "Omega" 18 ga. 3^0-7^0 Outswing Commercial Steel Door

APPROVAL DOCUMENT: Drawing No. **RD0087**, titled "3-0 x 7-0 Series", sheets 1 through 7 of 7, dated 5/30/97 with revision C dated 2/24/00,prepared by the manufacturer, bearing the Miami-Dade County Product Control Renewal stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

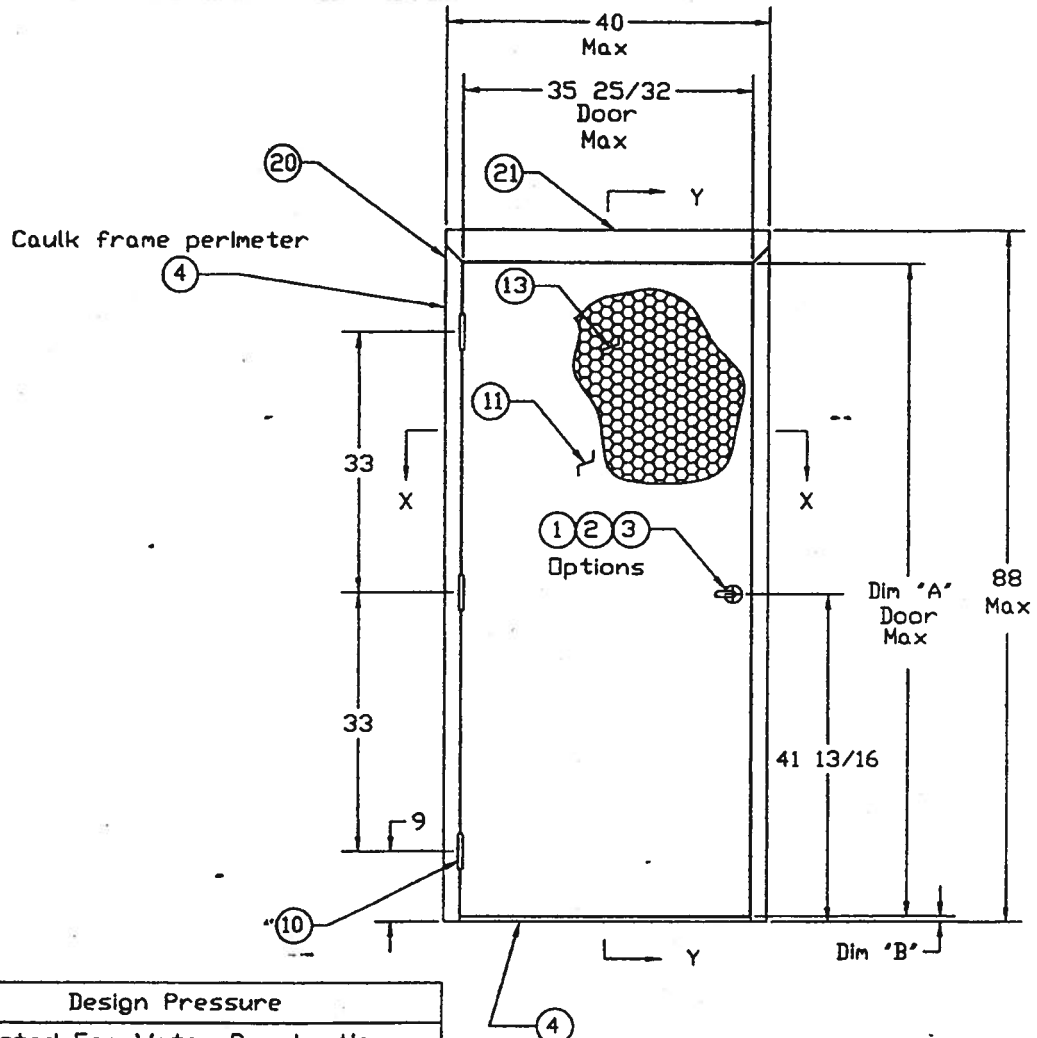
RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.
This NOA **renews NOA # 00-0315.03** and consists of this page 1 as well as approval document mentioned above.
The submitted documentation was reviewed by **Manuel Perez, P.E.**

PRODUCT CONTROL ACCEPTED



Sheet 2	Frame Anchor Installation
Sheet 3	Threshold Installation
Sheet 3	Weatherstrip Installation
Sheet 4	Door Latch Reinforcement
Sheet 5-6	Cross Section View
Sheet 7	Bill Of Material

Design Pressure		
Tested For Water Penetration		
With Overhang	+85 psf	-60 psf
Without Overhang	+60 psf	-60 psf

	Dim 'A'	Dim 'B'
3/4" Undercut	83 1/8	3/4
3/8" Undercut	83 1/2	3/8

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 09, 2008
BY Manuel Perez
PRODUCT CONTROL DIV.S.O.N
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

PRODUCT RENEWED
as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date Aug 14, 2008
By: Manuel Perez
Miami-Dade Product Control Division

ISSUE	REVISIONS
C 2/24/00 JAH	Revised Format, Transfered Information from NOA
B 7/22/97 GWS	Deleted CD Drawings Revised Sheet Numbers

MATERIAL SPECIFICATIONS:

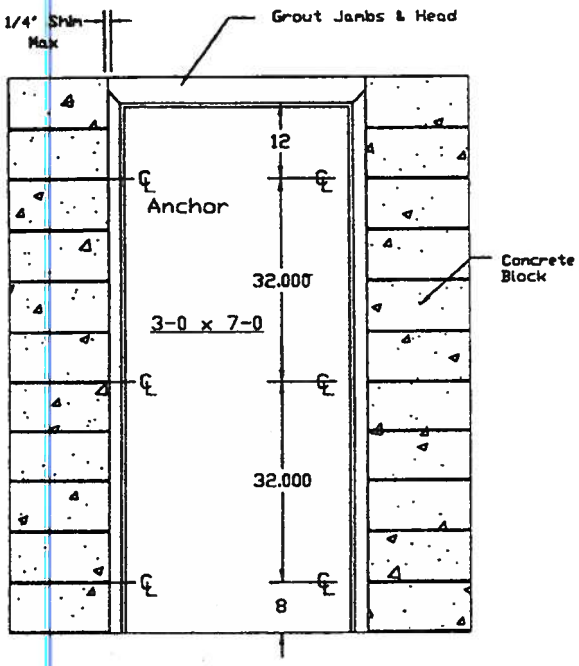
Finish: Rust Inhibitive Primer

3-0 x 7-0 Series Elevation Drawing

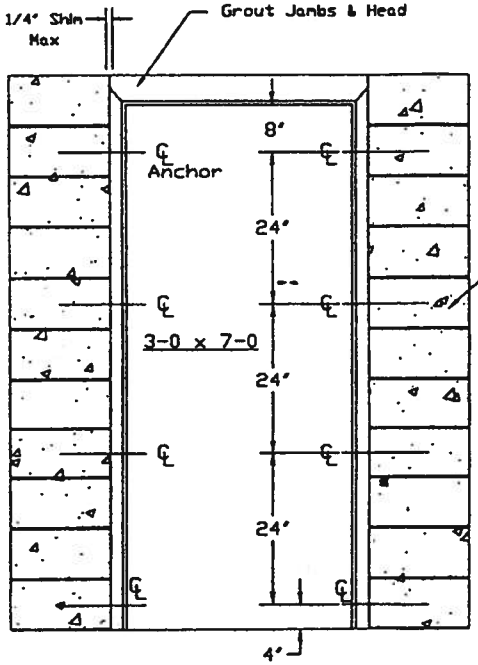
CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS
DATE: 5/30/97

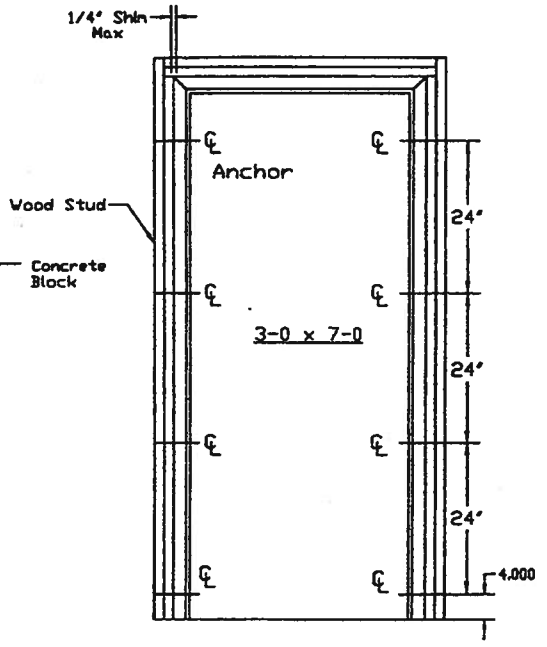
DRAWING NUMBER: RD0087



Masonry "T" Anchor

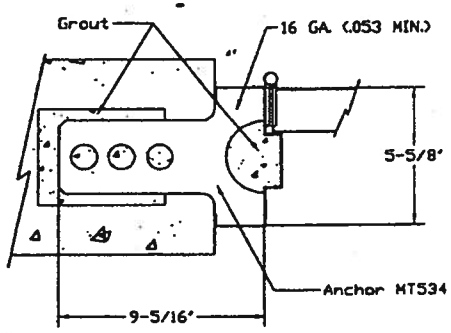


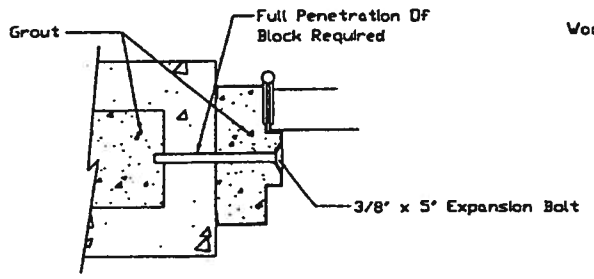
Existing Opening Anchor Into Block

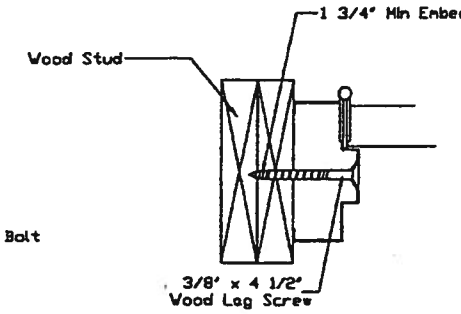


Existing Opening Anchor Into Wood Stud

PRODUCT RENEWED
as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date Aug. 14, 2008
By Manuel Perez
Miami-Dade Product Control Division







APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08, 2000
BY Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

NOTES:
1. SEE SHEET 7 FOR BILL OF MATERIALS

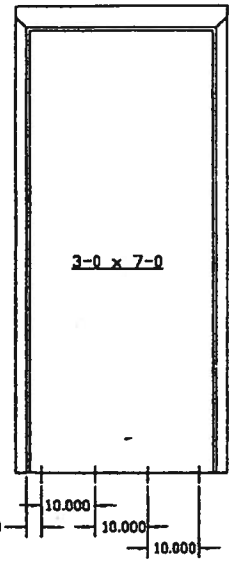
MATERIAL SPECIFICATIONS:

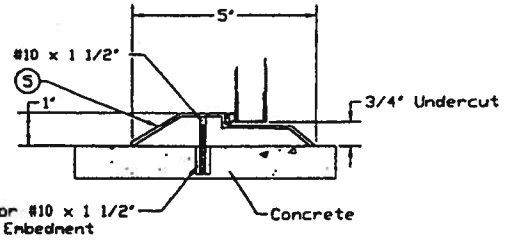
Frame Anchor Installation Details

CECO DOOR PRODUCTS
Milan, Tennessee 38358

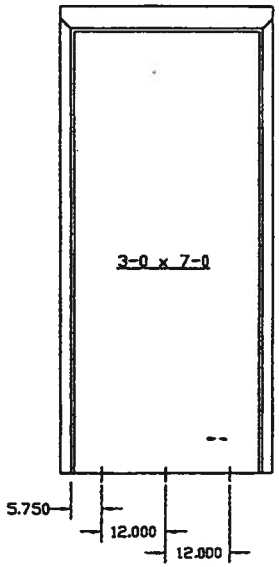
ISSUE	REVISIONS
B 2/24/00 JAR	Revised Format, Transfered Information from NOA
A 7/22/97 GWS	Revised Sheet Number

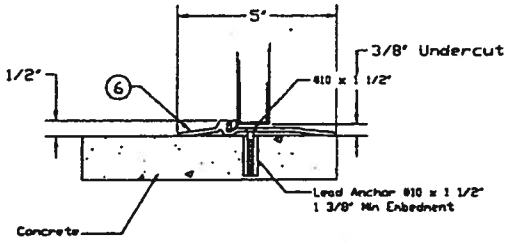
DRAWN BY: GWS
DATE: 5/30/97
DRAWING NUMBER: RD0087
Sheet 2 of 7



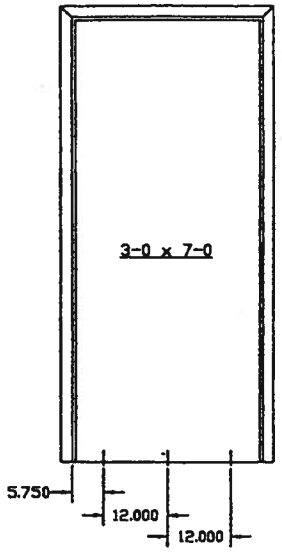


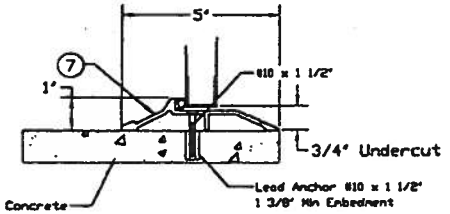
Threshold: National Guard 803S





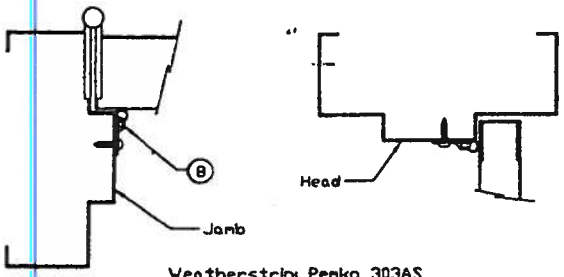
Threshold: Penko 2005AV





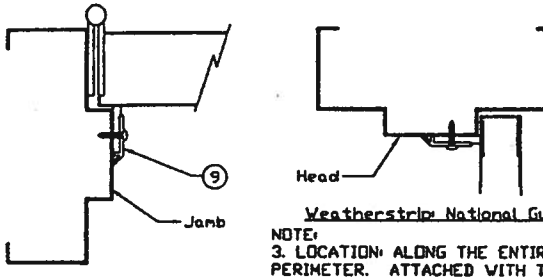
Threshold: Penko 181AV

NOTE: 1. All thresholds shown are made from extruded aluminum with slide-in vinyl weatherstrip insert.



Weatherstrip: Penko 303AS

NOTE:
2. LOCATION: ALONG THE ENTIRE HEAD AND JAMB PERIMETER. ATTACHED WITH THIRTY FOUR (34) #8 X 3/4" PPH SMS SPACED AT 6" O/C.



Weatherstrip: National Guard 130NA

NOTE:
3. LOCATION: ALONG THE ENTIRE HEAD AND JAMB PERIMETER. ATTACHED WITH THIRTY FOUR (34) #8 X 3/4" PPH SMS SPACED AT 6" O/C.

NOTE: 4. See Sheet 7 For Bill of Material

PRODUCT RENEWED
as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date AUG. 14, 2008
By Manuel Perez
Miami-Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08/2000
BY Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

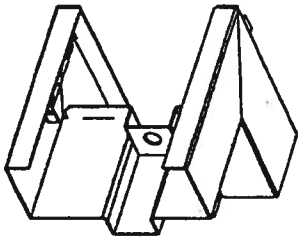
C 2/24/00 JAR	Revised Format, Transfered Information from NOA	
B 7/22/97 GWS	Revised Sheet Number	
ISSUE	REVISIONS	
DRAWN BY: GWS	DATE: 5/30/97	

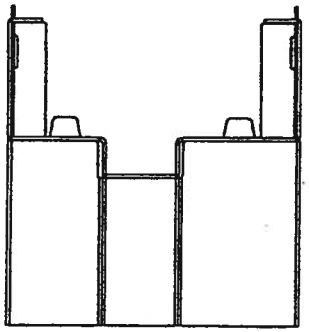
MATERIAL SPECIFICATIONS:

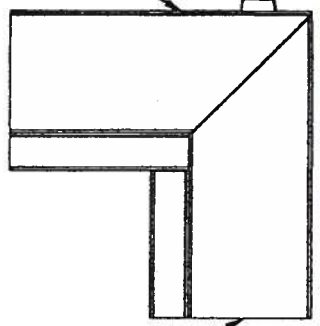
Threshold & Weatherstrip Installation details

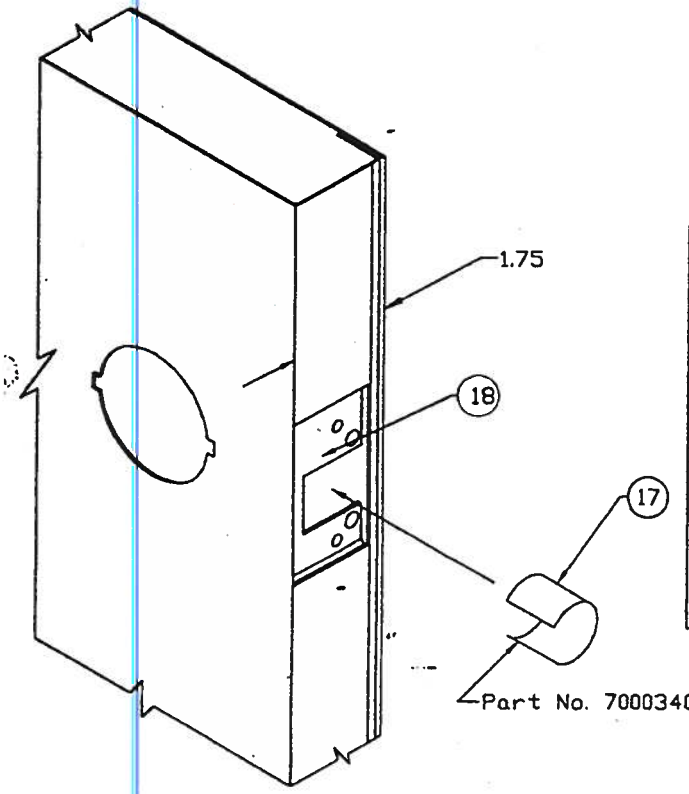
CECO DOOR PRODUCTS
Milan, Tennessee 38358

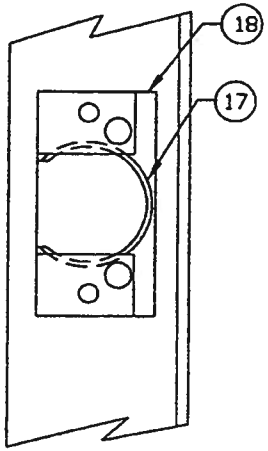
RD0087
Sheet 3 Of 7











PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date AUG. 14 2008
By Manuel Diaz
Miami Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08 2000
BY Manuel Diaz
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315-03

Note: 1. For Cylindrical Lock Only
2. See Sheet 7 For Bill Of Material

ISSUE	REVISIONS
B 2/24/00 JAR	Revised Format, Transfered Information from NOA
A 7/22/97 GWS	Revised Sheet Number

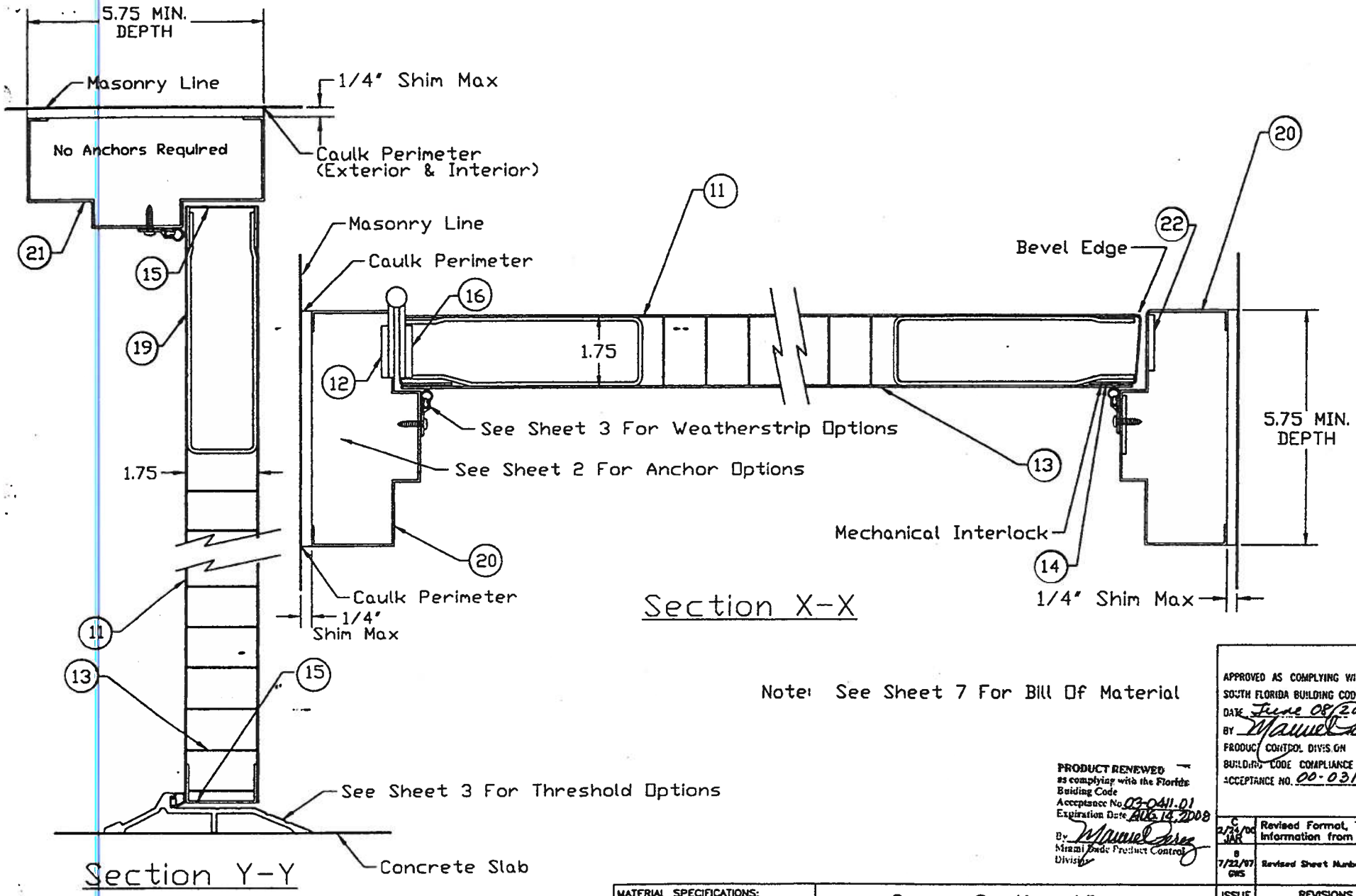
MATERIAL SPECIFICATIONS:

Cylindrical Lock Reinforcement and "SF" Seriers Frame Corner Installation Details

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS
DATE: 6/06/97

DRAWING NUMBER: RD0087



Note: See Sheet 7 For Bill Of Material

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08/2000
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date Aug. 14, 2008
By
Miami-Dade Product Control Division

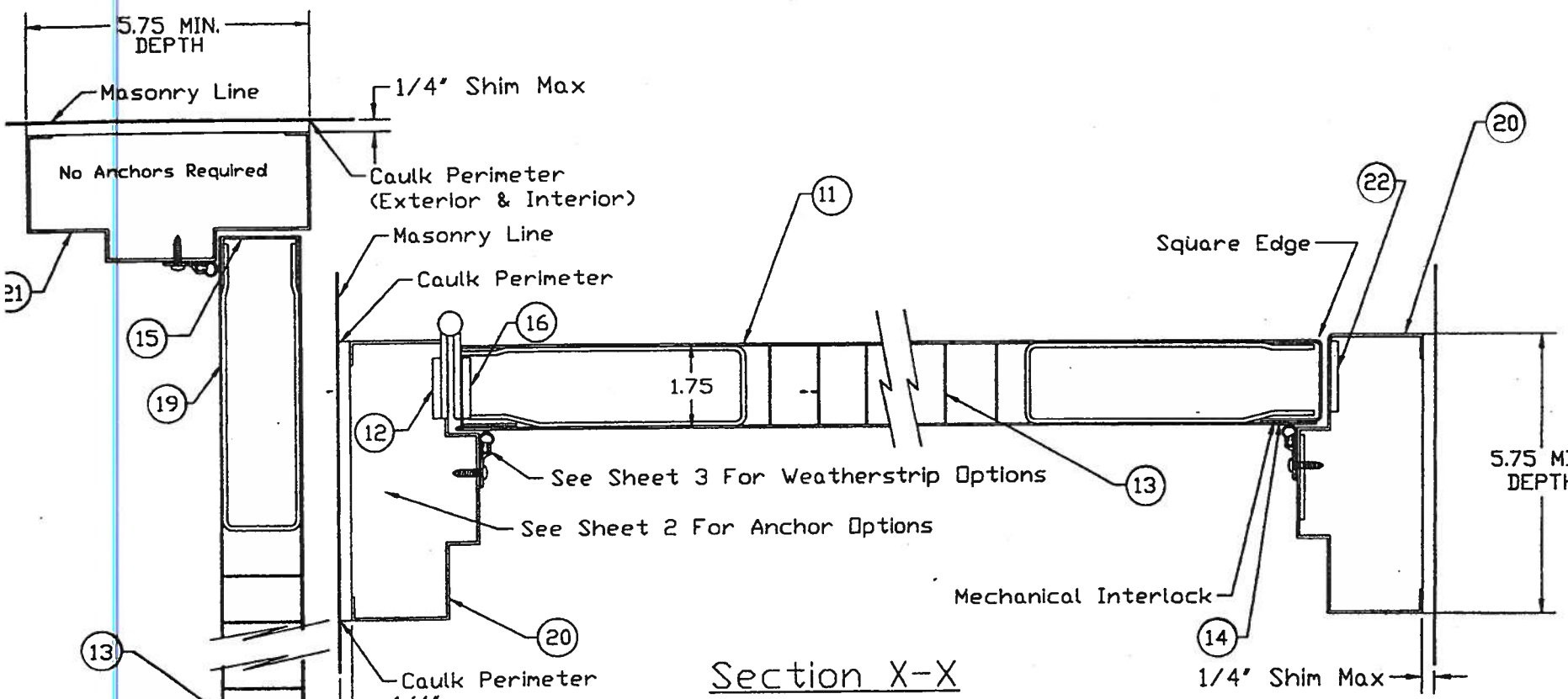
ISSUE	REVISIONS
C 2/24/00 JAR	Revised Format, Transfered Information from NOA
B 7/22/97 GWS	Revised Sheet Number

MATERIAL SPECIFICATIONS:

Cross Section View
Regent Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS
DATE: 5/30/97
DRAWING NUMBER: RD0087
Sheet 5 of 7



Section X-X

Section Y-Y

Note: See Sheet 7 For Bill Of Material

PRODUCT RENEWED as complying with the Florida Building Code Acceptance No 03-0411.01 Expiration Date AUG. 14, 2008 By Miami Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE DATE: June 06 2000 BY Manuel Perez PRODUCT CONTROL DIVISION BUILDING CODE COMPLIANCE OFFICE ACCEPTANCE NO. 00-0315.03

C 2/24/00 JAR	Revised Format, Transfered Information from NOA
B 7/22/97 GWS	Revised Sheet Number

MATERIAL SPECIFICATIONS:

Cross Section View Omega Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358

ISSUE	REVISIONS
DRAWN BY: GWS	DATE: 5/30/97

DRAWING NUMBER: RD0087

Sheet 6 of 7

ITEM	QTY	DESCRIPTION	MATERIAL	SIZE
1	1	SCHLAGE SERIES AL53PD GRADE 2, LATCH LOCK, SINGLE LEVER OR KNOB OPERATED		
2	1	MARKS SERIES 170AB GRADE 2, LATCH LOCK, INSIDE/OUTSIDE LEVER OPERATED		
3	1	YALE SERIES AU53070 GRADE 2 LATCH LOCK, SINGLE LEVER OR KNOB OPERATED.		
4	1	CAULK FOR INSTALLATION AND WEATHERSTRIP ADAPTER SCREWS FRAME PERIMETER (INSIDE & OUT) AND FRAME SILL CORNERS	GE SILICONE HOUSEHOLD SEALANT	
5	1	NATIONAL GUARD #803S		
6	1	PEMKO #2005AV		
7	1	PEMKO #181AV		
8	1 ROV	PEMKO #303AS HIGH SURFACE APPLIED EXTRUDED ALUMINUM WEATHERSTRIP ADAPTER WITH A SILICON (TM) BULB INSERT		
9	1 ROV	NATIONAL GUARD #130NA 1-1/4" WIDE X 0.188" SURFACE APPLIED EXTRUDED ALUMINUM WEATHERSTRIP ADAPT. WITH A FOAM INSERT		
10	3	HAGAR BB1279, 4-1/2" X 4-1/2" X .0134" THICK STEEL HINGE EACH ATTACHED WITH EIGHT #12-24 X 1/2" FH MS		
11	1	FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A568	COMMERCIAL QUALITY COLD ROLLED STEEL (MINIMUM YEILD STR. OF Fy=36,000 psi)	18 GAUGE (.042" MIN. THICK)
12	3	HINGE REINFORCING PLATE, PLATE SPOT WELDED TO FRAME JAMB AT EACH HINGE LOCATION	STEEL	1-1/4" X 9" X 7 GA.
13	1	CORE: FULL HONEYCOMB CORE PERMANENTLY BONDED TO THE INSIDE OF EACH FACE SKIN WITH NON-FLAMMABLE ADHESIVE	PHENOLIC RESIN-IMPREGNATED KRAFT PAPER	1-1/8" CELL
14	1	DENFLEX 3500 STRUCTURAL ADHESIVE EPOXY		
15	1	ROLL FORMED STEEL CHANNEL ON THE TOP AND BOTTOM OF THE DOOR SPOT WELDED TO EXTERIOR AND GLUED TO INTERIOR SKIN		1" X 1-3/4" X 1" X 16 GA. (.053" MIN)
16	3	DOOR HINGE REINFORCEMENT		1-1/4" X 9" X 7 GA.
17	1	DOOR LATCH REINFORCEMENT, STEEL "C" RING	28 GA. GALV.	.015" THICK X 1.313 INSIDE DIAMETER
18	1	DOOR LOCK REINFORCEMENT	STEEL	16 GA.
19	1	DOOR CLOSER REINFORCEMENT, ROLLED FORM CHANNELS TACK WELDED TO DOOR END CHANNELS	STEEL	12 GA. (.093")
20	2	SERIES "SF", FRAME JAMB, DOUBLE RABBET PROFILE FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A653	16 GA. (.053" MIN.) STEEL COMMERCIAL QUALITY COLD ROLLED STEEL (MINIMUM YEILD STR. OF Fy=40,000 psi)	2" FACE, 5-3/4" DEPTH MIN.
21	1	SERIES "SF", FRAME HEAD, DOUBLE RABBET PROFILE FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A653	16 GA. (.053" MIN.) STEEL COMMERCIAL QUALITY COLD ROLLED STEEL (MINIMUM YEILD STR. OF Fy=40,000 psi)	2" FACE, 5-3/4" DEPTH MIN.
22	1	JAMB LOCK STRIKE REINFORCING PLATE	STEEL	1-1/8" X 2-1/2" X 12 GA.

PRODUCT RENEWED
as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date AUG 14, 2008
By [signature]
Miami-Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08, 2000
BY [signature]
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

ISSUE	REVISIONS
B 2/24/00 JAH	Revised Format, Transfered Information from NOA
A 7/22/97 GWS	Revised Sheet Number

MATERIAL SPECIFICATIONS:

3-0 x 7-0 Series
Bill Of Materials

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS
DATE: 6/02/97
DRAWING NUMBER: RD0087
Sheet 7 of 7

MIAMI-DADE

BUILDING CODE COMPLIANCE OFFICE (BCCO)
PRODUCT CONTROL DIVISION

MIAMI-DADE COUNTY, FLORIDA
METRO-DADE FLAGLER BUILDING
140 WEST FLAGLER STREET, SUITE 1603
MIAMI, FLORIDA 33130-1563
(305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

Ceco Door Products
9159 Telecom Drive
Milan, TN 38358

outswing

SCOPE:
This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the High Velocity Hurricane Zone of the Florida Building Code.

DESCRIPTION: Series "Regent" & "Omega" 18 ga. 3^0-7^0 Outswing Commercial Steel Door

APPROVAL DOCUMENT: Drawing No. **RD0087**, titled "3-0 x 7-0 Series", sheets 1 through 7 of 7, dated 5/30/97 with revision C dated 2/24/00,prepared by the manufacturer, bearing the Miami-Dade County Product Control Renewal stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact

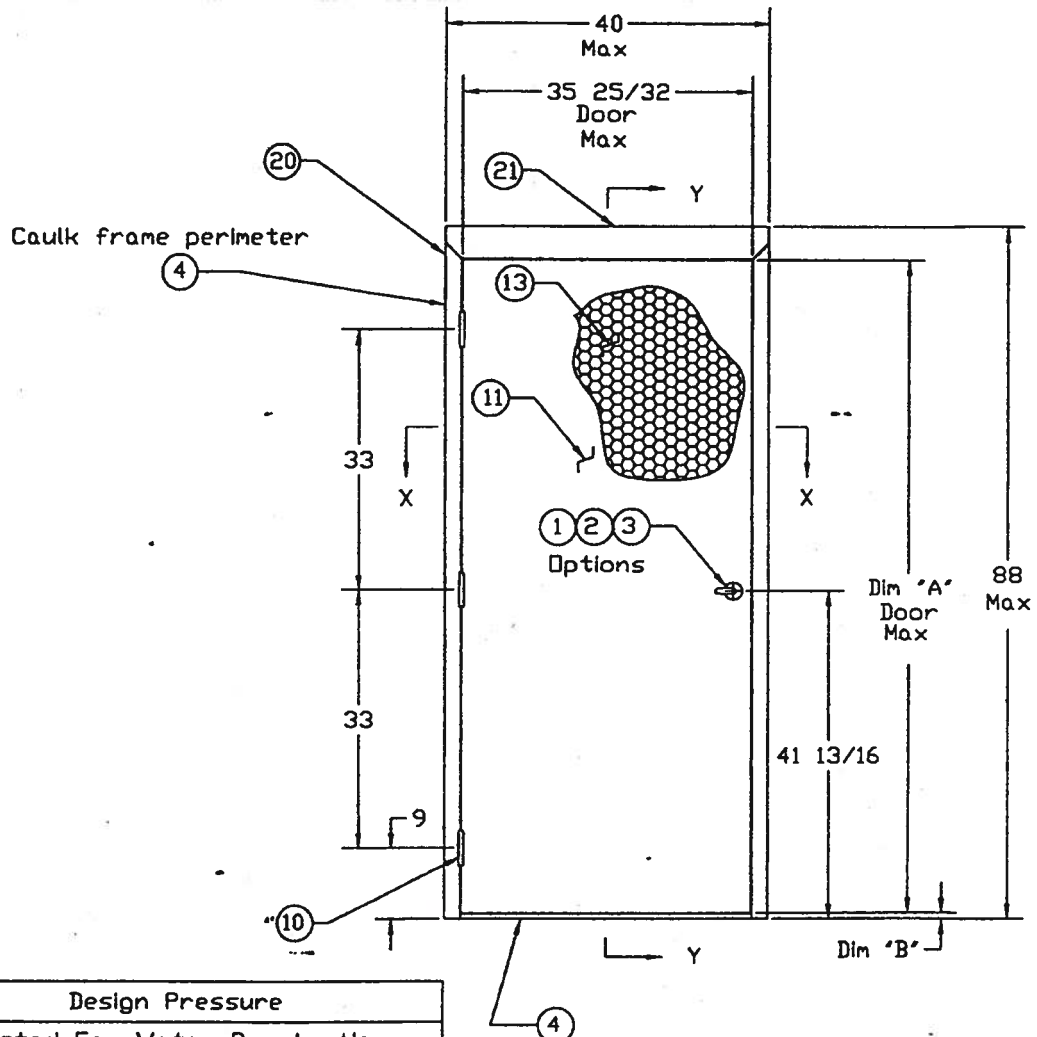
LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.
This NOA **renews NOA # 00-0315.03** and consists of this page 1 as well as approval document mentioned above.
The submitted documentation was reviewed by **Manuel Perez, P.E.**



Design Pressure		
Tested For Water Penetration		
With Overhang	+85 psf	-60 psf
Without Overhang	+60 psf	-60 psf

	Dim 'A'	Dim 'B'
3/4" Undercut	83 1/8	3/4
3/8" Undercut	83 1/2	3/8

Sheet 2	Frame Anchor Installation
Sheet 3	Threshold Installation
Sheet 3	Weatherstrip Installation
Sheet 4	Door Latch Reinforcement
Sheet 5-6	Cross Section View
Sheet 7	Bill Of Material

PRODUCT RENEWED
as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date Nov 14 2008
By: Miami-Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE Sept 09, 2009
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

ISSUE	REVISIONS
C 2/24/00 JAR	Revised Format, Transfered Information from NOA
B 7/22/97 GWS	Deleted CD Drawings Revised Sheet Numbers

MATERIAL SPECIFICATIONS:

Finish: Rust Inhibitive Primer

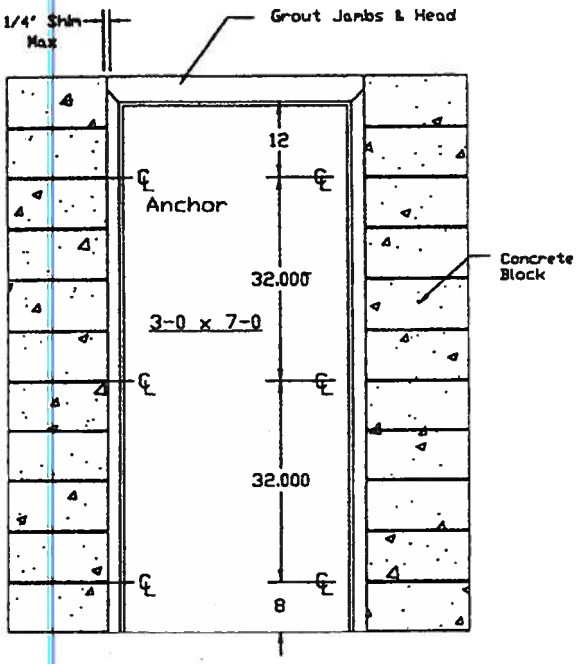
3-0 x 7-0 Series Elevation Drawing

CECO DOOR PRODUCTS
Milan, Tennessee 38358

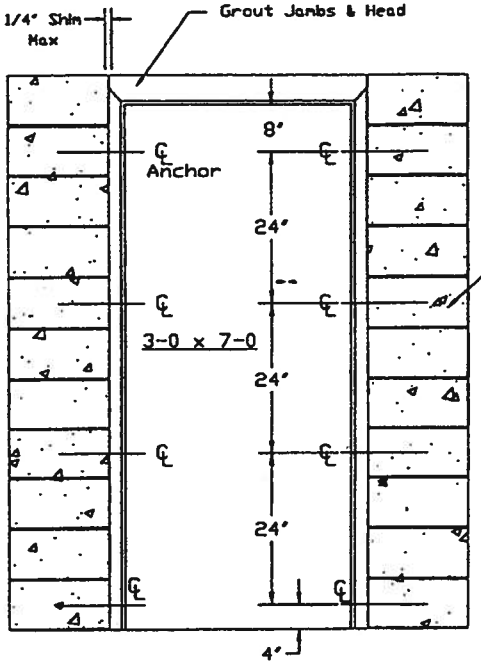
DRAWN BY: GWS
DATE: 5/30/97

DRAWING NUMBER: RD0087

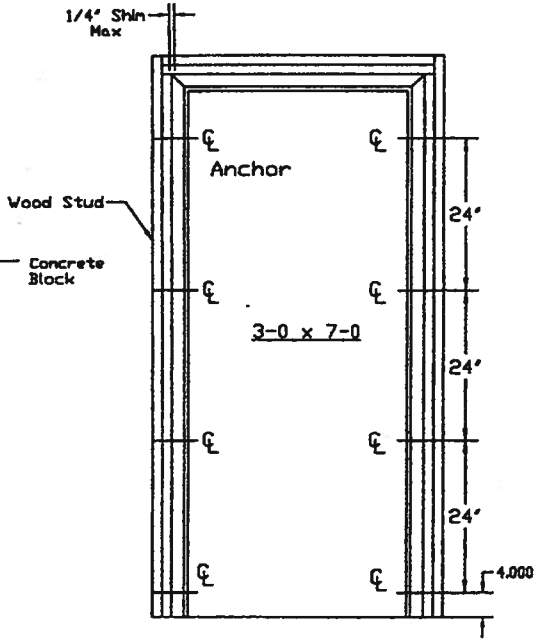
Sheet 1 Of 7



Masonry "T" Anchor

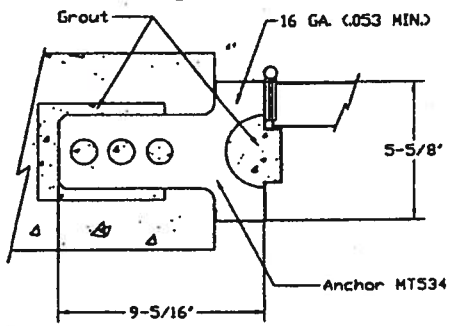


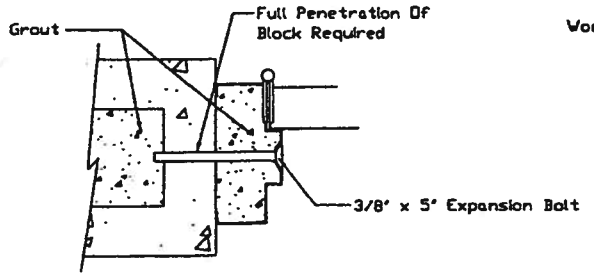
Existing Opening Anchor Into Block

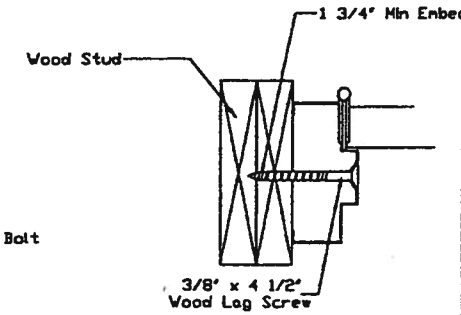


Existing Opening Anchor Into Wood Stud

PRODUCT RENEWED
as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date Aug. 14, 2008
By Manuel Perez
Miami-Dade Product Control Division







APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08, 2000
BY Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

NOTES:
1. SEE SHEET 7 FOR BILL OF MATERIALS

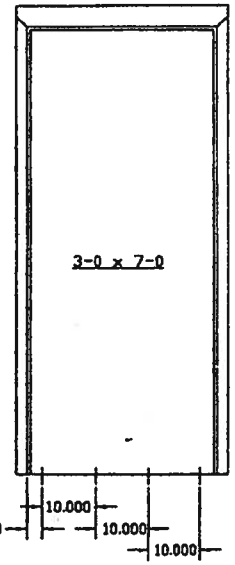
MATERIAL SPECIFICATIONS:

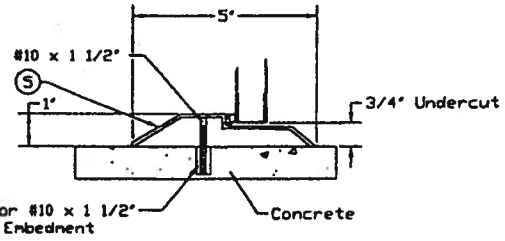
Frame Anchor Installation Details

CECO DOOR PRODUCTS
Milan, Tennessee 38358

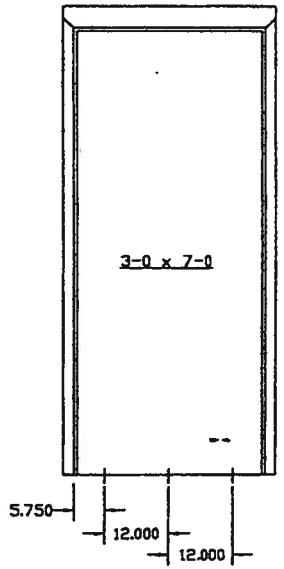
ISSUE	REVISIONS
B 2/21/00 JAR	Revised Format, Transfered Information from NOA
A 7/22/97 GWS	Revised Sheet Number

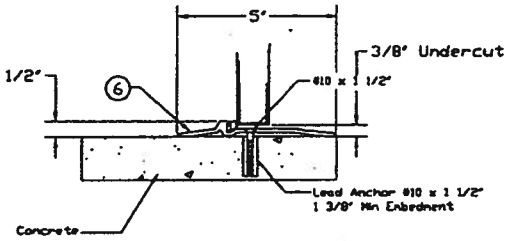
DRAWN BY: GWS
DATE: 5/30/97
DRAWING NUMBER: RD0087
Sheet 2 of 7



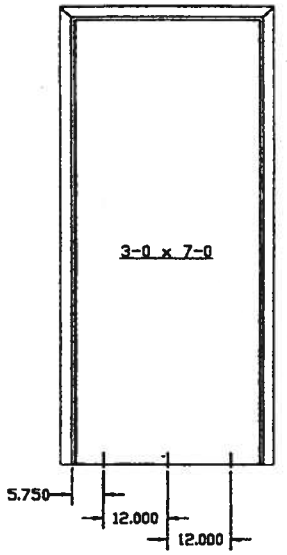


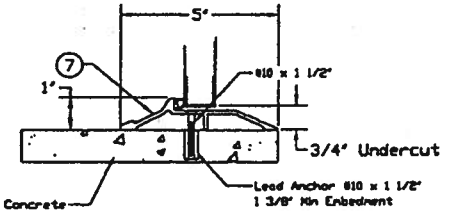
Threshold: National Guard 803S





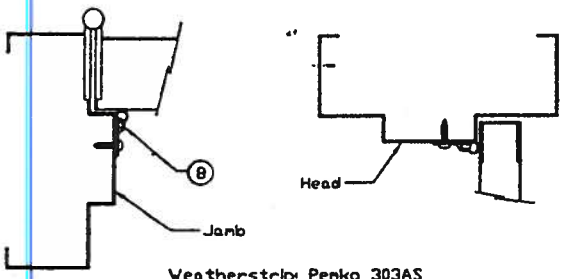
Threshold: Penko 2005AV





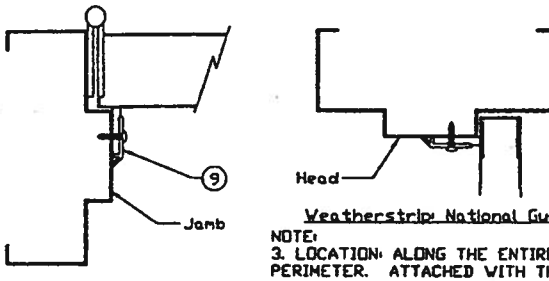
Threshold: Penko 181AV

NOTE: 1. All thresholds shown are made from extruded aluminum with slide-in vinyl weatherstrip insert.



Weatherstrip: Penko 303AS

NOTE:
2. LOCATION: ALONG THE ENTIRE HEAD AND JAMB PERIMETER. ATTACHED WITH THIRTY FOUR (34) #8 X 3/4" PPH SMS SPACED AT 6" O/C.



Weatherstrip: National Guard 130NA

NOTE:
3. LOCATION: ALONG THE ENTIRE HEAD AND JAMB PERIMETER. ATTACHED WITH THIRTY FOUR (34) #8 X 3/4" PPH SMS SPACED AT 6" O/C.

NOTE: 4. See Sheet 7 For Bill of Material

PRODUCT RENEWED
as complying with the Florida Building Code
Acceptance No. 03-0611.01
Expiration Date AUG. 16, 2008
By Manuel Perez
Miami-Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08/2000
BY Manuel Perez
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

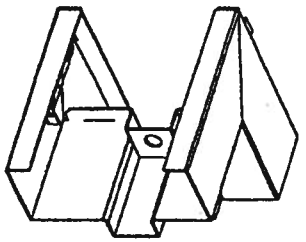
C	2/24/00 JAR	Revised Format, Transfered Information from NOA
B	7/22/97 GWS	Revised Sheet Number
ISSUE		REVISIONS
DRAWN BY: GWS		DATE: 5/30/97

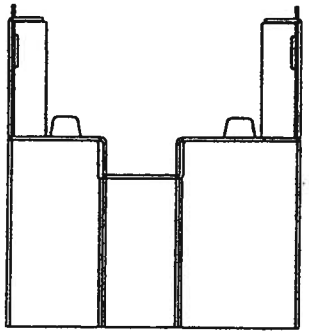
MATERIAL SPECIFICATIONS:

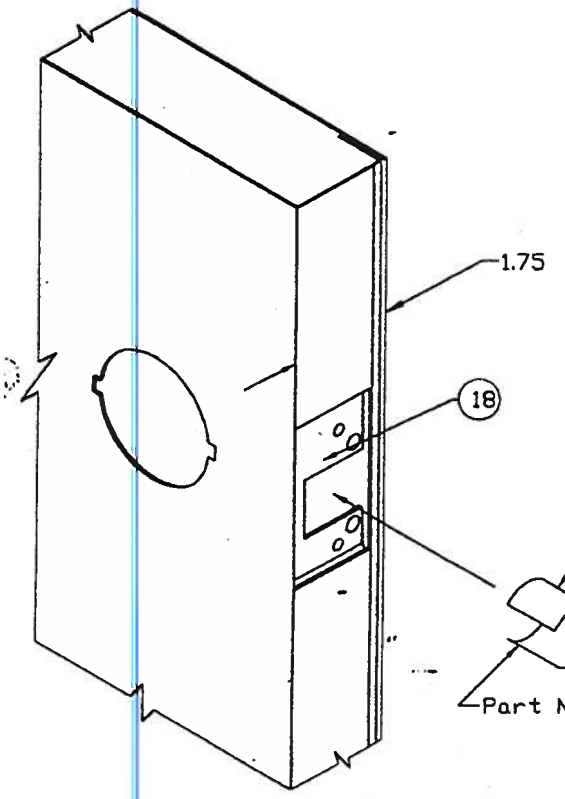
Threshold & Weatherstrip Installation details

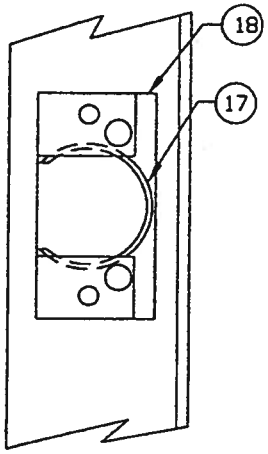
CECO DOOR PRODUCTS
Milan, Tennessee 38358

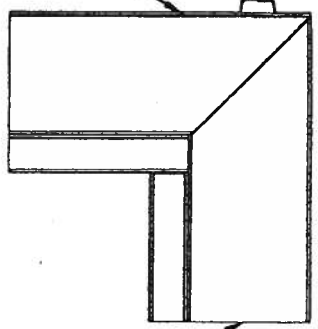
RD0087
Sheet 3 Of 7











PRODUCT RENEWED
as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date AUG. 16 2008
By: Manuel Arz
Miami Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08 2000
BY Manuel Arz
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315-03

Note: 1. For Cylindrical Lock Only
2. See Sheet 7 For Bill Of Material

ISSUE	REVISIONS
B 2/24/00 JAR	Revised Format, Transfered Information from NOA
A 7/22/97 GWS	Revised Sheet Number

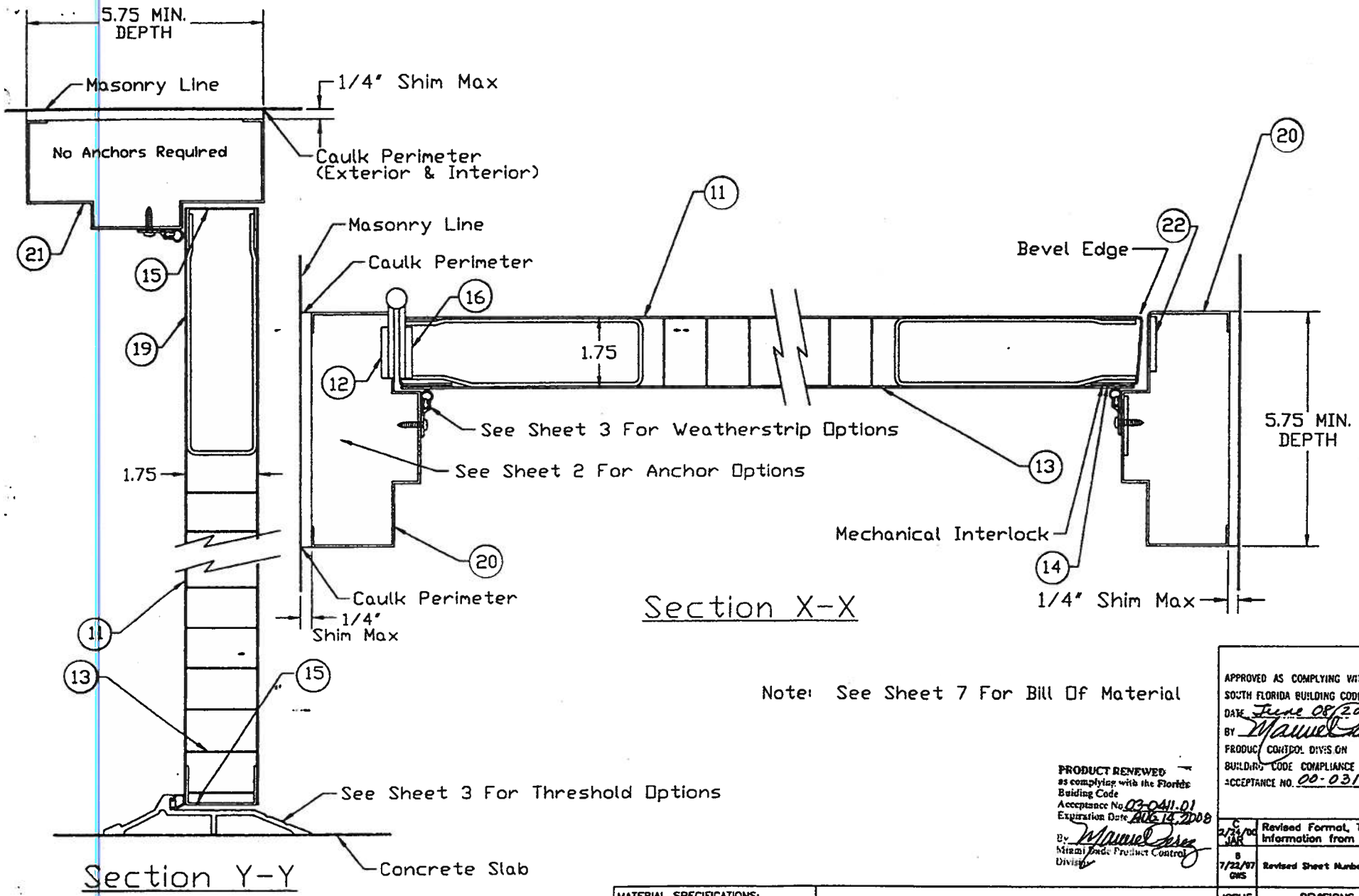
MATERIAL SPECIFICATIONS:

Cylindrical Lock Reinforcement and "SF" Seriers Frame Corner Installation Details

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS
DATE: 6/06/97

DRAWING NUMBER: RD0087



Note: See Sheet 7 For Bill Of Material

PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date Aug. 14, 2008
By Miami-Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE June 08 2000
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

ISSUE	REVISIONS
C 2/24/00 JAR	Revised Format, Transfered Information from NOA
B 7/22/97 GWS	Revised Sheet Number

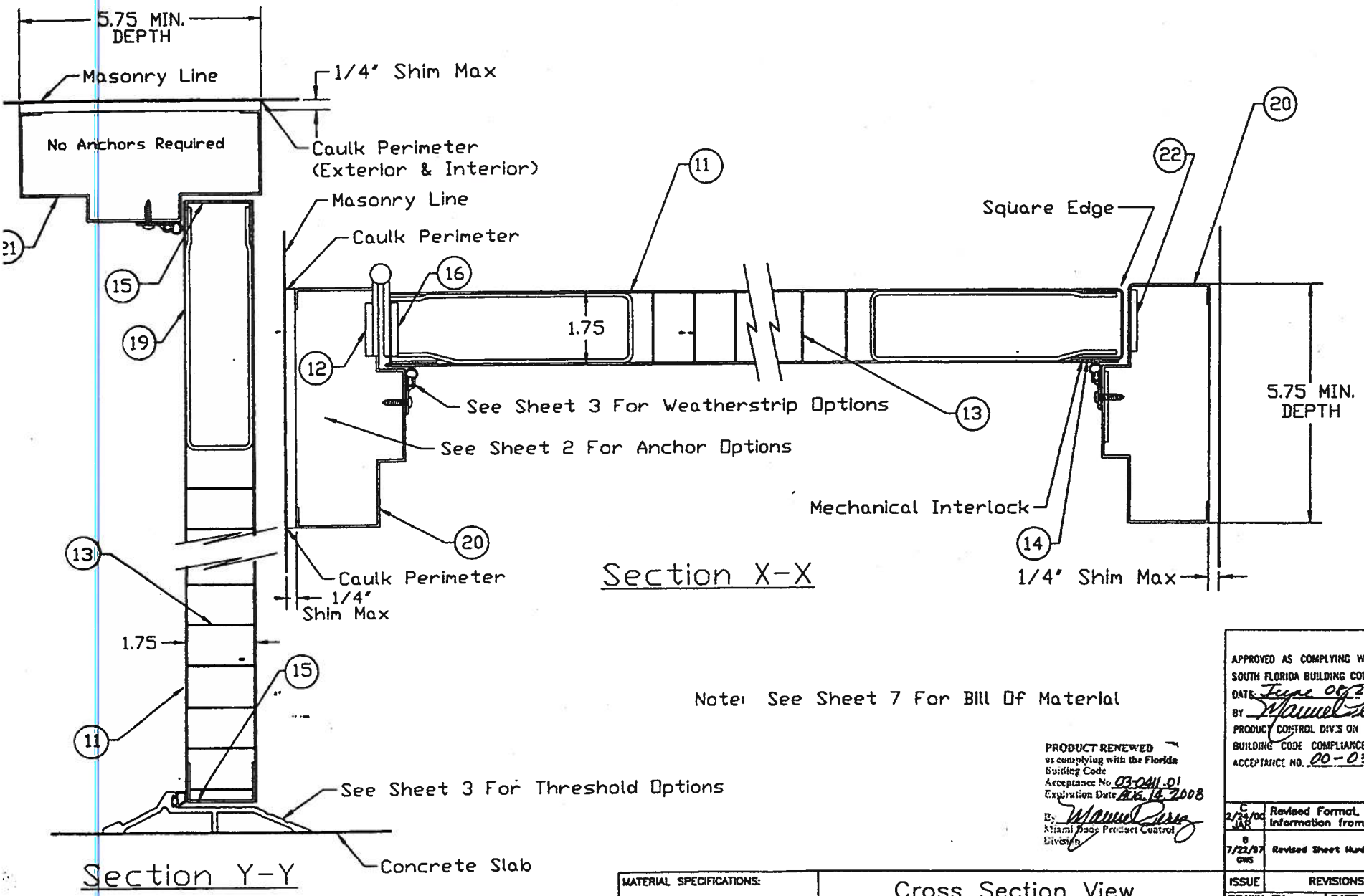
MATERIAL SPECIFICATIONS:

Cross Section View
Regent Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS
DATE: 5/30/97

DRAWING NUMBER: RD0087



Note: See Sheet 7 For Bill Of Material

PRODUCT RENEWED
as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date AUG. 14, 2008
By Miami Dade Product Control Division

APPROVED AS COMPLYING WITH THE SOUTH FLORIDA BUILDING CODE
DATE: June 06, 2000
BY
PRODUCT CONTROL DIVISION
BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

ISSUE	REVISIONS
C 2/24/00 JAR	Revised Format, Transfered Information from NOA
B 7/22/97 GWS	Revised Sheet Number

MATERIAL SPECIFICATIONS:

Cross Section View
Omega Door

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS
DATE: 5/30/97

DRAWING NUMBER: RD0087

ITEM	QTY	DESCRIPTION	MATERIAL	SIZE
1	1	SCHLAGE SERIES AL53PD GRADE 2, LATCH LOCK, SINGLE LEVER OR KNOB OPERATED		
2	1	MARKS SERIES 170AB GRADE 2, LATCH LOCK, INSIDE/OUTSIDE LEVER OPERATED		
3	1	YALE SERIES AU53070 GRADE 2 LATCH LOCK, SINGLE LEVER OR KNOB OPERATED.		
4	1	CAULK FOR INSTALLATION AND WEATHERSTRIP ADAPTER SCREWS FRAME PERIMETER (INSIDE & OUT) AND FRAME SILL CORNERS	GE SILICONE HOUSEHOLD SEALANT	
5	1	NATIONAL GUARD #8035		
6	1	PEMKO #2005AV		
7	1	PEMKO #181AV		
8	1 ROV	PEMKO #303AS HIGH SURFACE APPLIED EXTRUDED ALUMINUM WEATHERSTRIP ADAPTER WITH A SILICON (TM) BULB INSERT		
9	1 ROV	NATIONAL GUARD #130NA 1-1/4" WIDE X 0.188" SURFACE APPLIED EXTRUDED ALUMINUM WEATHERSTRIP ADAPT. WITH A FOAM INSERT		
10	3	HAGAR BB1279, 4-1/2" X 4-1/2" X .0134" THICK STEEL HINGE EACH ATTACHED WITH EIGHT #12-24 X 1/2" FH MS		
11	1	FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A568	COMMERCIAL QUALITY COLD ROLLED STEEL (MINIMUM YEILD STR. OF Fy=36,000 psi)	18 GAUGE (.042" MIN. THICK)
12	3	HINGE REINFORCING PLATE, PLATE SPOT WELDED TO FRAME JAMB AT EACH HINGE LOCATION	STEEL	1-1/4" X 9" X 7 GA.
13	1	CORE: FULL HONEYCOMB CORE PERMANENTLY BONDED TO THE INSIDE OF EACH FACE SKIN WITH NON-FLAMMABLE ADHESIVE	PHENOLIC RESIN-IMPREGNATED KRAFT PAPER	1-1/8" CELL
14	1	DENFLEX 3500 STRUCTURAL ADHESIVE EPOXY		
15	1	ROLL FORMED STEEL CHANNEL ON THE TOP AND BOTTOM OF THE DOOR SPOT WELDED TO EXTERIOR AND GLUED TO INTERIOR SKIN		1" X 1-3/4" X 1" X 16 GA. (.053" MIN)
16	3	DOOR HINGE REINFORCEMENT		1-1/4" X 9" X 7 GA.
17	1	DOOR LATCH REINFORCEMENT, STEEL "C" RING	28 GA. GALV.	.015" THICK X 1.313 INSIDE DIAMETER
18	1	DOOR LOCK REINFORCEMENT	STEEL	16 GA.
19	1	DOOR CLOSER REINFORCEMENT, ROLLED FORM CHANNELS TACK WELDED TO DOOR END CHANNELS	STEEL	12 GA. (.093")
20	2	SERIES "SF", FRAME JAMB, DOUBLE RABBET PROFILE FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A653	16 GA. (.053" MIN.) STEEL COMMERCIAL QUALITY COLD ROLLED STEEL (MINIMUM YEILD STR. OF Fy=40,000 psi)	2" FACE, 5-3/4" DEPTH MIN.
21	1	SERIES "SF", FRAME HEAD, DOUBLE RABBET PROFILE FACE SHEET CONFORMING TO ASTM A366 AND ASTM-A653	16 GA. (.053" MIN.) STEEL COMMERCIAL QUALITY COLD ROLLED STEEL (MINIMUM YEILD STR. OF Fy=40,000 psi)	2" FACE, 5-3/4" DEPTH MIN.
22	1	JAMB LOCK STRIKE REINFORCING PLATE	STEEL	1-1/8" X 2-1/2" X 12 GA.

PRODUCT RENEWED as complying with the Florida Building Code
Acceptance No. 03-0411.01
Expiration Date AUG 11, 2008
By Miami Dade Product Control Division

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BUILDING CODE COMPLIANCE OFFICE
ACCEPTANCE NO. 00-0315.03

ISSUE	REVISIONS
B 2/24/00 JAR	Revised Format, Transfered Information from NOA
A 7/22/97 GWS	Revised Sheet Number

MATERIAL SPECIFICATIONS:

3-0 x 7-0 Series
Bill Of Materials

CECO DOOR PRODUCTS
Milan, Tennessee 38358

DRAWN BY: GWS
DATE: 6/02/97

DRAWING NUMBER: RD0087
Sheet 7 of 7

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97
TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.

SERIES/MODEL: 480/680/880 Drop-in
PRODUCT TYPE: Aluminum Horizontal Sliding Window (XO-Fin)

	Results	
Title	**Test Specimen #1**	**Test Specimen #2**
Rating	HS-C30 71 x 71	HS-C40 71 x 59
Operating Force	11 lbf max.	14 lbf max.
Air Infiltration	0.11 cfm/ft^2	0.09 cfm/ft^2
Water Resistance Test Pressure	5.3 psf	6.0 psf
Uniform Load Deflection Test Pressure	± 30.0 psf	+ 45.0 psf -47.2 psf
Uniform Structural Load Test Pressure	± 45.0 psf	+ 67.5 psf -70.8 psf
Forced Entry Resistance	Grade 10	Grade 10

Reference should be made to ATI Report Identification No. 01-47320.03 for complete test specimen description and data

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Architectural Testing

ANSI/AAMA/NWWDA 101/I.S.2-97 TEST REPORT

Rendered to:

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gratz, Pennsylvania 17030-0370

ATI Report Identification No.: 01-47320.03
Test Dates: 10/07/03
Through: 10/08/03
And: 12/01/03
And: 12/15/03
And: 03/17/04
Report Date: 04/16/04
Expiration Date: 10/07/07

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on two Series/Model 480/680/880 Drop-in, aluminum horizontal sliding windows at MI Home Products, Inc. test facility in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for the following ratings: Test Specimen #1: HS-C30 71 x 71; Test Specimen #2: HS-C40 71 x 59. Test specimen description and results are reported herein.

Test Specification: The test specimens were evaluated in accordance with ANSI/AAMA/NWWDA 101/I.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*

Test Specimen Description:

Series/Model: 480/680/880 Drop-in

Product Type: Aluminum Horizontal Sliding Window (XO Fin)

Test Specimen #1: HS-C30 71 x 71

Overall Size: 5' 11-7/16" wide by 5' 11" high

Active Sash Size: 2' 11-5/8" wide by 5' 8-3/8" high

Fixed Daylight Opening Size: 2' 8-3/16" wide by 5' 5-5/8" high

Screen Size: 2' 10" wide by 5' 6-1/2" high

130 Derry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com

Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.250" high by 0.187" backed polypile with center fin	1 Row	Active sash top and bottom rails and fixed meeting rail interlock
0.250" high by 0.187" backed polypile with center fin	2 Rows	Jamb stile

Test Specimen #2: HS-C40 71 x 59

Overall Size: 5' 11-3/8" wide by 4' 11-1/8" high

Active Sash Size: 2' 11-5/8" wide by 4' 8-1/4" high

Fixed Daylight Opening Size: 2' 8-1/4" wide by 4' 5-7/8" high

Screen Size: 2' 10-1/4" wide by 4' 7-1/8" high

Weatherstripping:

Description	Quantity	Location
0.310" high by 0.187" backed polypile with center fin	1 Row	Active sash top and bottom rails
0.250" high by 0.187" backed polypile with center fin	1 Rows	Fixed meeting rail interlock
0.310" high by 0.187" backed polypile with center fin	2 Rows	Jamb stile
0.550" high by 1" by 1" backed polypile pad	1 Pad	Corner of bottom rail and locking stile

Test Specimen Description: (Continued)

The following descriptions apply to all specimens.

Finish: All aluminum was white.

Glazing Details: The window utilized 5/8" thick sealed insulating glass constructed from two sheets of 1/8" thick clear annealed glass and a Swiggle spacer system. The lites were interior glazed onto double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

Frame Construction: The frame was constructed of thermally broken extruded aluminum. The corners were secured utilizing three #8 x 1" screws per corner through the jambs into the head and sill screw bosses. End caps were utilized on the ends of the fixed meeting rails and secured with two #8 x 3/4" screws per cap. The meeting rails were then secured to the frame with two #8 x 3/4" screws.

Sash Construction: The sash was constructed of thermally broken extruded aluminum. The corners were secured utilizing one #8 x 1" screw per corner through the head and sill into the jambs screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible vinyl spline.

Hardware:

Description	Quantity	Location
Cam lock	1	One midspan of active panel with integral lock keeper on fixed meeting stile
Roller assembly	2	One each end of bottom rail
Screen constant force spring	2	5" from rails on screen stiles
Screen lift handles	2	5" from rails on screen stiles

Drainage:

Description	Quantity	Location
1-1/4" long by 1/4" wide weepslot with cover	2	3-1/2" from jambs on sill face
1/2" long by 1/8" wide weepslot	2	2" from jambs on sill track

Reinforcement: No reinforcement was utilized.

Installation: The window was installed into a #2 Spruce-Pine-Fir wood buck. The window was secured utilizing #8 x 1-5/8" drywall screws located in corners and 12" on center around nail-fin perimeter. Silicone was utilized around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: HS-C30 71 x 71			
2.2.2.5.1	Operating Force	11 lbf	25 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.11 cfm/ft^2	0.3 cfm/ft^2 max.
	***Note #1**: The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*		
2.1.3	Water Resistance per ASTM E 547-00 (with and without screen) 4.50 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	30.0 psf (positive)	0.75"	See Note #2
	30.0 psf (negative)	0.71"	See Note #2
	***Note #2**: The Uniform Load Deflection test is not requirement of ANSI/AAMA/NWWDA 101/I.S.2-97 for this product designation. The deflection data is recorded in this report for special code compliance and information only.*		
2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	45.0 psf (positive)	0.13"	0.26" max.
	45.0 psf (negative)	<0.01"	0.26" max.
2.2.2.5.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Handle stile	0.13"/25%	0.50"/100%
	Lock stile	0.19"/38%	0.50"/100%
	In remaining direction - 50 lbs		
	Top rail	0.09"/19%	0.50"/100%
	Bottom rail	0.06"/13%	0.50"/100%

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #1: HS-C30 71 x 71 (Continued)			
2.1.8	Forced Entry Resistance per ASTM F 588		
Type: A	Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547-00 (with and without screen) 5.3 psf	No leakage	No leakage
Test Specimen #2: HS-C40 71 x 59			
2.2.2.5.1	Operating Force	14 lbf	25 lbf max.
2.1.2	Air Infiltration per ASTM E 283 1.57 psf (25 mph)	0.09 cfm/ft^2	0.3 cfm/ft^2 max.

***Note #1**: The tested specimen meets the performance levels specified in ANSI/AAMA/NWWDA 101/I.S. 2-97 for air infiltration.*

2.1.3	Water Resistance per ASTM E 547-00 (with and without screen) 4.50 psf	No leakage	No leakage
2.1.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	30.0 psf (positive)	0.62"	See Note #2
	30.0 psf (negative)	0.51"	See Note #2
2.1.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	45.0 psf (positive)	0.03"	0.21" max.
	45.0 psf (negative)	0.04"	0.21" max.

Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
Test Specimen #2: HS-C40 71 x 59 (Continued)			
2.2.2.5.2	Deglazing Test per ASTM E 987 In operating direction - 70 lbs		
	Handle stile	0.13"/25%	0.50"/100%
	Lock stile	0.13"/25%	0.50"/100%
	In remaining direction - 50 lbs		
	Top rail	0.03"/6%	0.50"/100%
	Bottom rail	0.03"/6%	0.50"/100%
2.1.8	Forced Entry Resistance per ASTM F 588		
	Type: A	Grade: 10	
	Lock Manipulation Test	No entry	No entry
	Test A1 thru A5	No entry	No entry
	Test A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry
Optional Performance			
4.3	Water Resistance per ASTM E 547-00 (with and without screen) 6.0 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330 (Deflections reported were taken on the meeting stile) (Loads were held for 52 seconds)		
	45.0 psf (positive)	0.62"	See Note #2
	47.2 psf (negative)	0.54"	See Note #2
4.4.2	Uniform Load Structural per ASTM E 330 (Permanent sets reported were taken on the meeting stile) (Loads were held for 10 seconds)		
	67.5 psf (positive)	0.04"	0.21" max.
	70.8 psf (negative)	0.08"	0.21" max.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years from the original test date. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without approval of Architectural Testing.

For ARCHITECTURAL TESTING, INC

Digitally Signed by: Eric Westphal

Eric Westphal
Technician

Digitally Signed by: Steven M. Urich

Steven M. Urich, P. E.
Senior Project Engineer

EW:dme
01-47320.03

APRIL 20, 2004

TAMKO®
ROOFING PRODUCTS

January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMK(s Tuscaloosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Buil ng Code TAS 100 for wind driven rain.

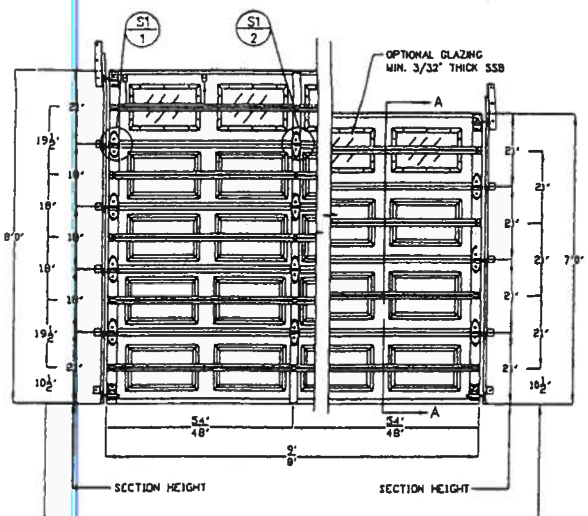
- Glass-Seal AR
- Elite Glass-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

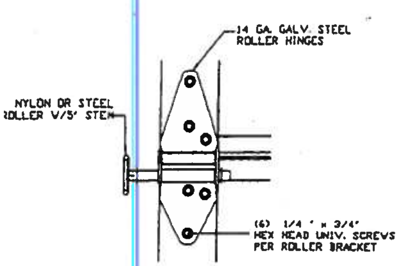
Please direct all questions to TAMKO's Technical Services Department at 1-800-641-46

TAMKO Roofing Products, Inc.

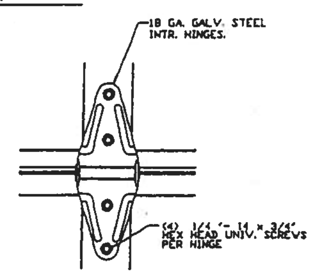
2300 35th STREET P.O. BOX 2149 TUSCALOOSA, AL 35403-2149 205-752-3555 FAX 205-349-2049



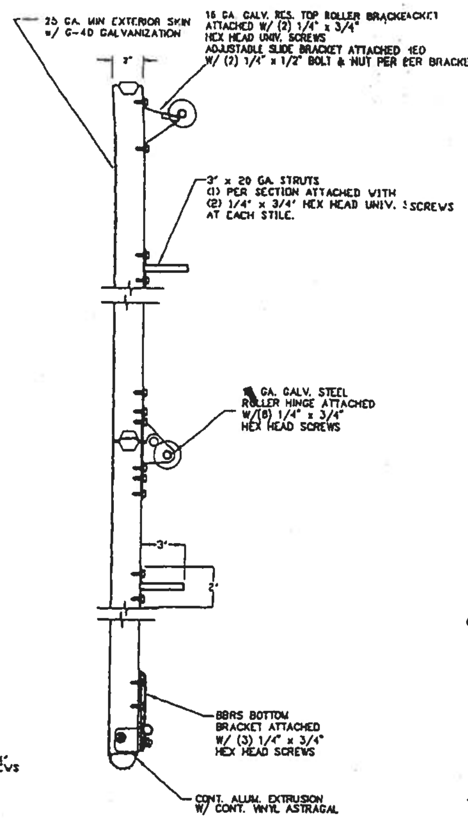
INSIDE ELEVATION



TYP. ROLLER BRACKET (S1/1)
N.T.S.



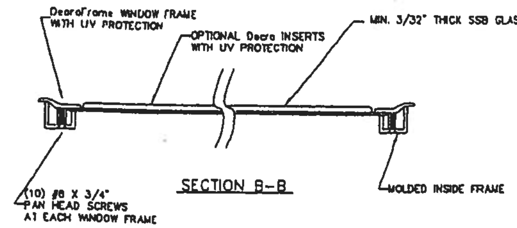
TYP. HINGE CONNECTION (S1/2)
N.T.S.



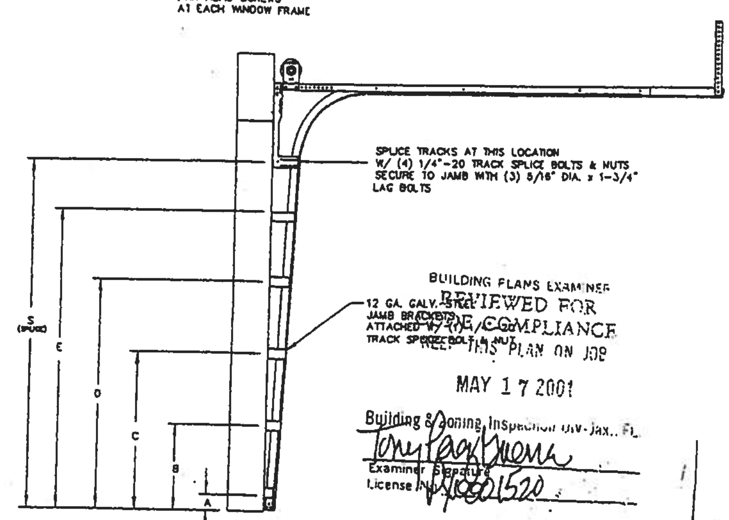
SECTION A–A (SIDE VIEW)

GLAZING OPTION CROSS SECTION

TEST No. SBC-580-011 ON OCTOBER 12, 1995 INCLUDED GLASS WINDOWS IN THE DOOR BEING USED. THE TEST PRESSURES WERE +48.4 PSF AND -54.7 PSF. BY COMPARISON, FOUR (4) WINDOWS MAY BE INSTALLED IN (1) ONE SECTION OF THE 9' X 7' AND 9' X 8' MODEL 600 AND 950 DOORS.



SECTION B–B



BUILDING PLANS EXAMINER
REVIEWED FOR CODE COMPLIANCE
KEEP THIS PLAN ON JOB
MAY 17 2001
Building & Zoning Inspection Div.-Jax., FL
Examiner Signature
License No.

4/19/01

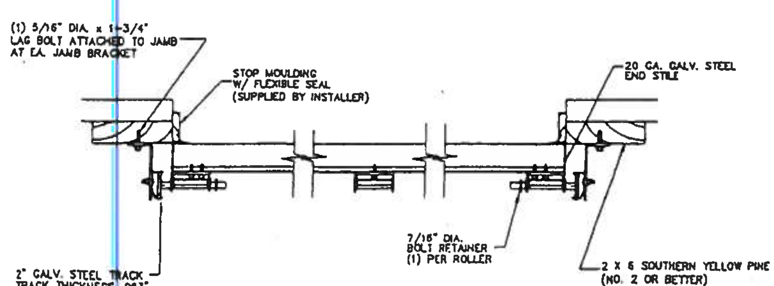
TRACK CONFIGURATION FOR 6'6" UP TO 8' TALL DOORS

JAMB BRACKET LOCATIONS

	A	B	C	D	E	S
6'-6"	4"	21-1/2"	39"	57"		70"
7'-0"	4"	21-1/2"	42"	63"		76"
7'-6"	4"	18"	36"	54"	72"	82"
8'-0"	4"	21-1/2"	39"	57"	75"	88"

SPECIFICATIONS AND NOTES

1. DOORS AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASMA.
2. DOOR SECTIONS SHALL BE 25 GA. (.019") MIN. ROLL FORMED LIGHT COMMERCIAL QUALITY. G-40 GALVANIZATION.
3. DOORS UPTO 7'0" HIGH CONSIST OF (4) SECTIONS AS SHOWN.
4. DOORS UPTO 8'0" HIGH CONSIST OF (5) SECTIONS AS SHOWN.
5. SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTRED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADINGS.
6. THE METHOD OF TESTING WAS IN SUBSTANTIAL CONFORMANCE WITH THE PROCEDURE DESCRIBED IN ASTM E330-90, AND THE SOUTHERN BUILDING CODE SECTION 1606 WIND LOAD DESIGN CRITERIA THE PRESSURES SHOWN ON THE DRAWINGS WERE CALCULATED USING THE FOLLOWING PARAMETERS:
   A. BASIC WIND SPEED OF 110 MPH
   B. DOOR CAN BE INSTALLED WITH 8 FEET OF DOORS WIDTH INSIDE THE EDGE STRIP.
   C. 15' MEAN ROOF HEIGHT AT ANY SLOPE
   D. USE FACTOR OF 1.0
   E. EXPOSURE RATING OF C



TRACK MOUNTING DETAIL

WOOD JAMB ATTACHMENT TO STRUCTURE

RATED FOR 110 MPH FASTEST-MILE BASIC WIND SPEEDS

VERTICAL JAMB ATTACHMENT TO WOOD FRAME STRUCTURE
5/16" X 3" LAG SCREWS STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO 2,000 PSI CONCRETE
HILTI KWIK BOLT 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C.
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET RED HEAD 3/8" X 3" STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO C-90 BLOCK
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET TAPCON 1/4" X 2-3/4" STARTING 6" FROM ENDS. USE PAIRS OF FASTENERS (3" APART) AT 24" O.C.

*LAGS AND BOLTS CAN BE COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE.
*PREPARATION OF WOOD JAMBS BY OTHERS

MAX SIZE 9' x 8'	
DESIGN LOADS +28.7 PSF -31.3 PSF	
TEST LOADS +43.0 PSF -46.9 PSF	
DESCRIPTION OF REVISIONS	
Amarr Garage Doors	
MODEL #600 STRATFORD MODEL #950 HERITAGE III	
SIZE B	DRAWING NUMBER SBC-580-007-J
SCALE: NOT TO SCALE	SHEET 1 OF 1

Project Information for:	L204776		
Builder:	JOHN NORRIS	Date:	7/31/2006
Lot:	LOT 32	Start Number:	**1003**
Subdivision:	THORNWOOD	SEI Ref:	L204776
County or City:	COLUMBIA COUNTY		
Truss Page Count:	38		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		**Wind**		**Building Code:**	**FBC2004**
Roof (psf):	**42**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

NORRIS, JOHN DAVID RG 0066597
Address: 351 NW CORWIN GLN
LAKE CITY, FL. 32025 Designer: **109**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

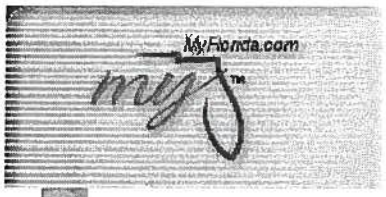
Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0731061003	7/31/2006				
2	CJ3	0731061004	7/31/2006				
3	CJ3T	0731061005	7/31/2006				
4	CJ5	0731061006	7/31/2006				
5	CJ5T	0731061007	7/31/2006				
6	EJ7	0731061008	7/31/2006				
7	EJ7A	0731061009	7/31/2006				
8	EJ7T	0731061010	7/31/2006				
9	HJ9	0731061011	7/31/2006				
10	HJ9T	0731061012	7/31/2006				
11	T01	0731061013	7/31/2006				
12	T01A	0731061014	7/31/2006				
13	T01G	0731061015	7/31/2006				
14	T02	0731061016	7/31/2006				
15	T03	0731061017	7/31/2006				
16	T04	0731061018	7/31/2006				
17	T04A	0731061019	7/31/2006				
18	T04G	0731061020	7/31/2006				
19	T05	0731061021	7/31/2006				
20	T06	0731061022	7/31/2006				
21	T07	0731061023	7/31/2006				
22	T08	0731061024	7/31/2006				
23	T09	0731061025	7/31/2006				
24	T10	0731061026	7/31/2006				
25	T11	0731061027	7/31/2006				
26	T12	0731061028	7/31/2006				
27	T13	0731061029	7/31/2006				
28	T14	0731061030	7/31/2006				
29	T15	0731061031	7/31/2006				
30	T17	0731061032	7/31/2006				
31	T18	0731061033	7/31/2006				
32	T19	0731061034	7/31/2006				
33	T20	0731061035	7/31/2006				
34	T21	0731061036	7/31/2006				
35	T22	0731061037	7/31/2006				
36	T23	0731061038	7/31/2006				
37	T24	0731061039	7/31/2006				
38	T25	0731061040	7/31/2006				

JUL 3 1 2006

Dwg.#0731061002



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Term Glossary

Online Help

Licensee Details

Licensee Information

Name:	**NORRIS, JOHN DAVID (Primary Name)**
	INDIVIDUAL (Alternate Name)
Main Address:	**351 NW CORWIN GLN** **LAKE CITY, Florida 32055**
Lic. Location:	**WOODGLEN DRIVE** **LAKE CITY, FL 32055** **Columbia**

License Information

License Type:	**Registered General Contractor**
Rank:	**Reg General**
License Number:	**RG0066597**
Status:	**Current, Active**
Licensure Date:	**06/20/1996**
Expires:	**08/31/2005**

Special Qualifications	Effective Date
Bldg Code Core Course Credit	
No Qualified Business License Required	02/20/2004

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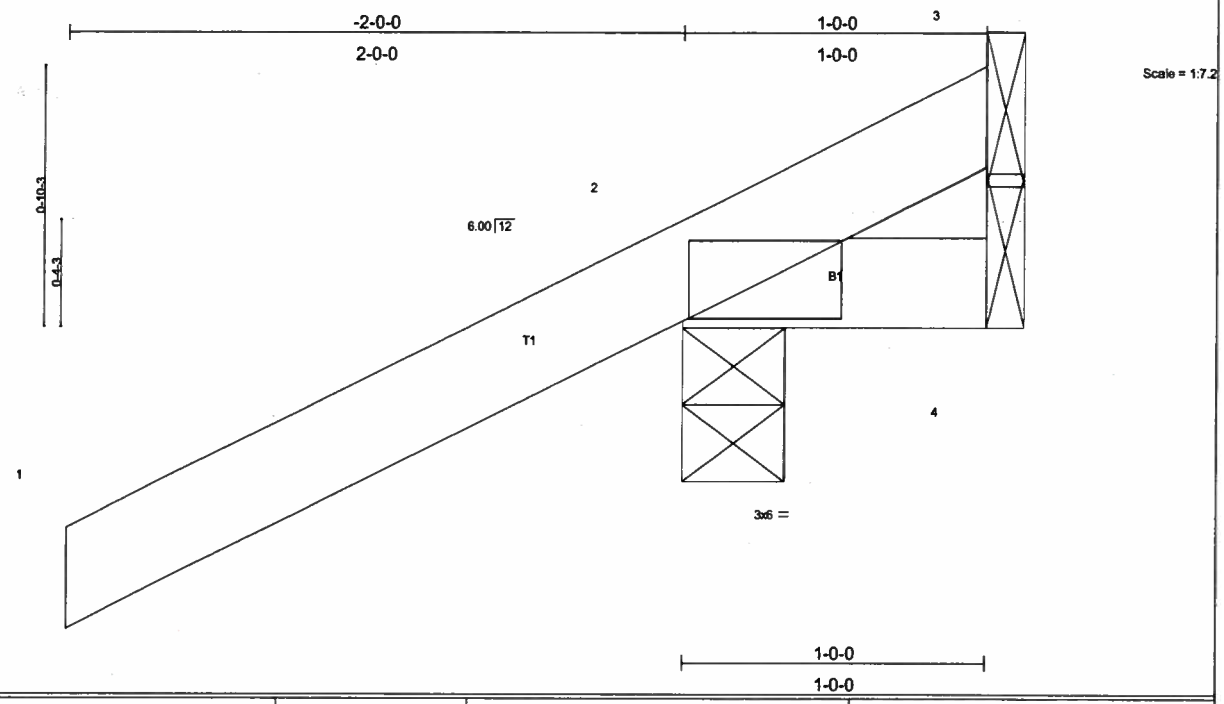
JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061003

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	CJ1	MONO TRUSS	12	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jul 31 13:11:44 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=267/0-4-0, 4=14/Mechanical, 3=-91/Mechanical
Max Horz 2=87(load case 5)
Max Uplift2=-287(load case 5), 4=-9(load case 3), 3=-91(load case 1)
Max Grav 2=267(load case 1), 4=14(load case 1), 3=128(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/76
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 2, 9 lb uplift at joint 4 and 91 lb uplift at joint 3.

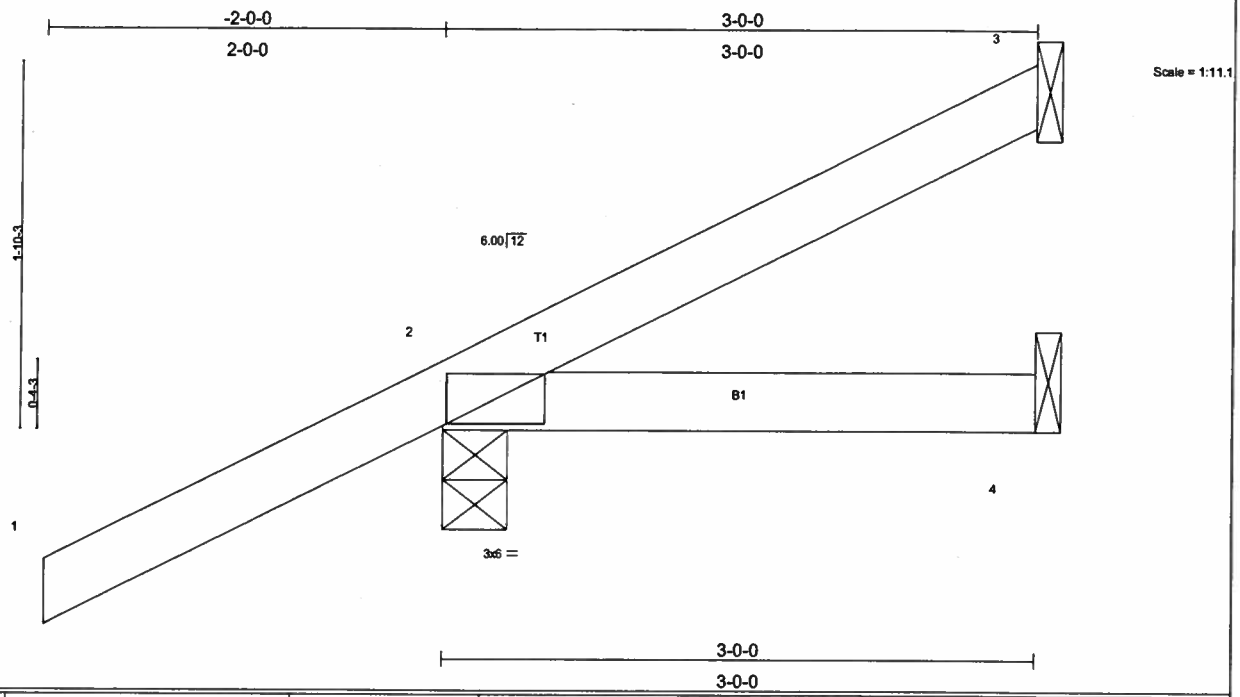
LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061004

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	CJ3	MONO TRUSS	10	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jul 31 13:11:44 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=29/Mechanical, 2=279/0-4-0, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-27(load case 6), 2=-240(load case 5), 4=-26(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-58/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3, 240 lb uplift at joint 2 and 26 lb uplift at joint 4.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061005

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	CJ3T	SPECIAL	2	1	Job Reference (optional)

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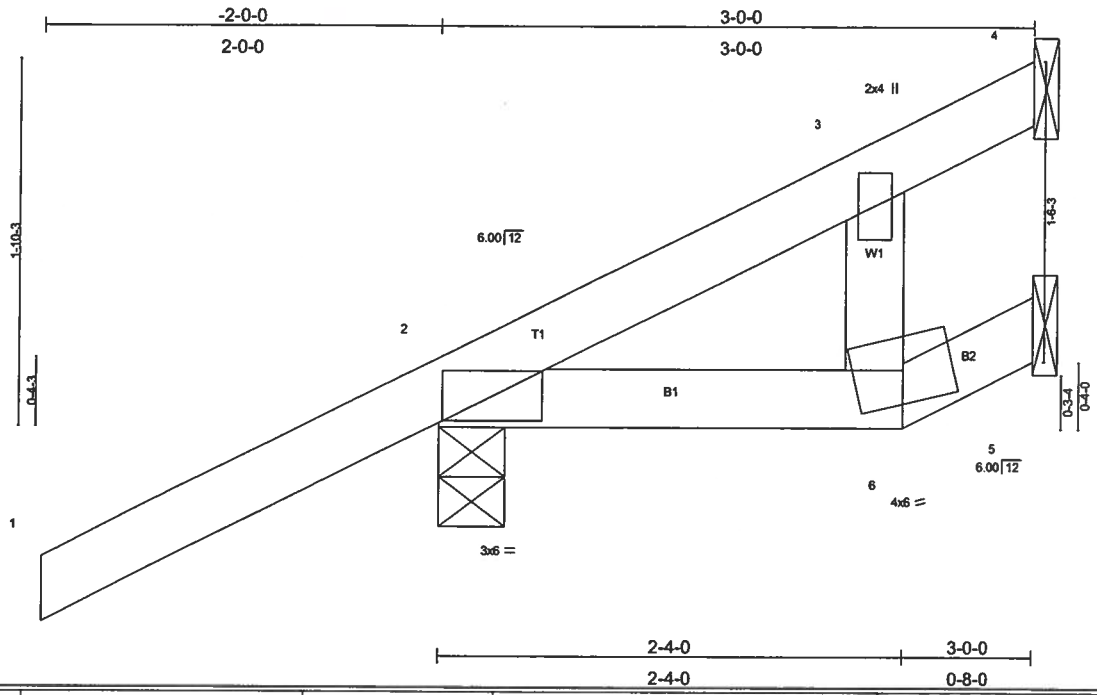

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.01	6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.01	6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.01	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 15 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=62/Mechanical, 2=279/0-4-0, 5=9/Mechanical
Max Horz 2=132(load case 5)
Max Uplift4=-21(load case 6), 2=-205(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-59/3, 3-4=-16/26
BOT CHORD 2-6=-5/0, 5-6=-3/3
WEBS 3-6=0/42

JOINT STRESS INDEX
2 = 0.13, 3 = 0.03 and 6 = 0.02

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 4 and 205 lb uplift at joint 2.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061006

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	CJ5	MONO TRUSS	10	1	Job Reference (optional)

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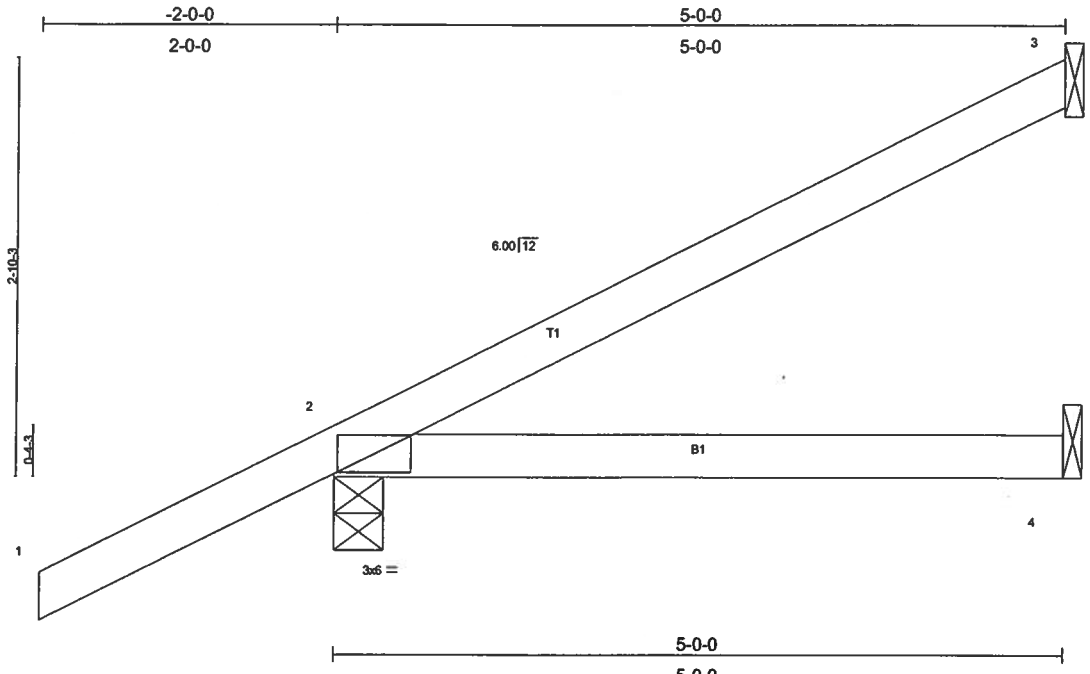

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.09	2-4	>671	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.07	2-4	>784	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=102/Mechanical, 2=344/0-4-0, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-86(load case 5), 2=-261(load case 5), 4=-46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-87/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 3, 261 lb uplift at joint 2 and 46 lb uplift at joint 4.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061007

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	CJ5T	SPECIAL	2	1	Job Reference (optional)

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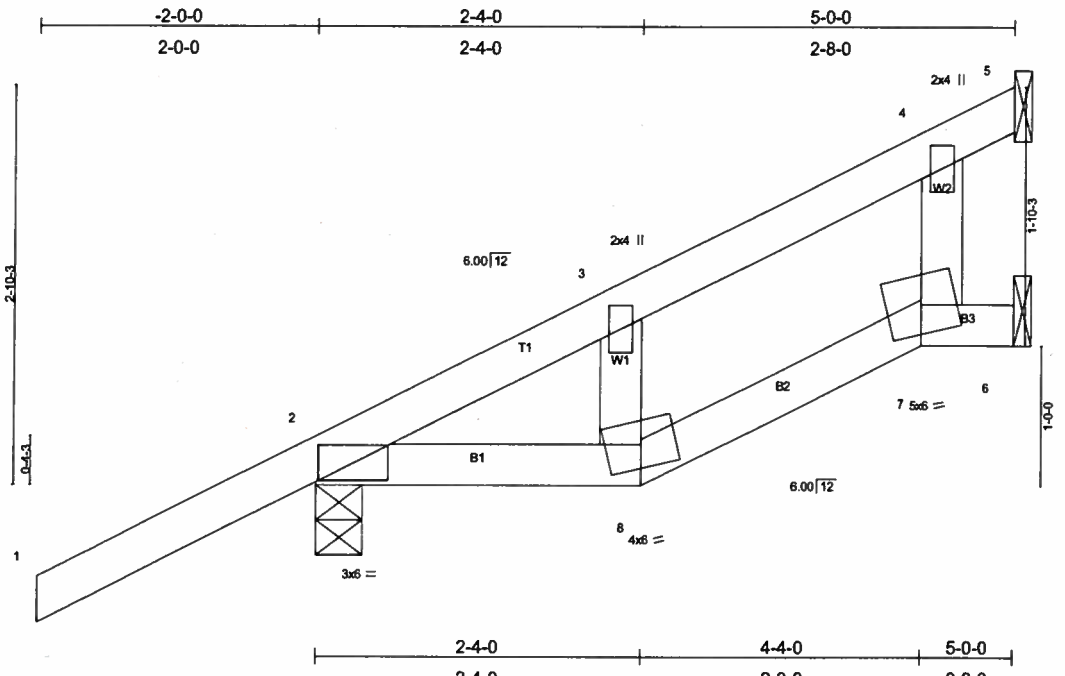

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.06	8	>910	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.09	8	>618	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(TL)	-0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 23 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 5=165/Mechanical, 2=344/0-4-0, 6=9/Mechanical
Max Horz 2=178(load case 5)
Max Uplift5=-74(load case 5), 2=-201(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-92/0, 3-4=-74/34, 4-5=-62/73
BOT CHORD 2-8=-3/0, 7-8=-5/16, 6-7=-0/0
WEBS 3-8=0/61, 4-7=0/42

JOINT STRESS INDEX
2 = 0.15, 3 = 0.04, 4 = 0.03, 7 = 0.01 and 8 = 0.03

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 74 lb uplift at joint 5 and 201 lb uplift at joint 2.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061008

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	EJ7	MONO TRUSS	24	1	Job Reference (optional)

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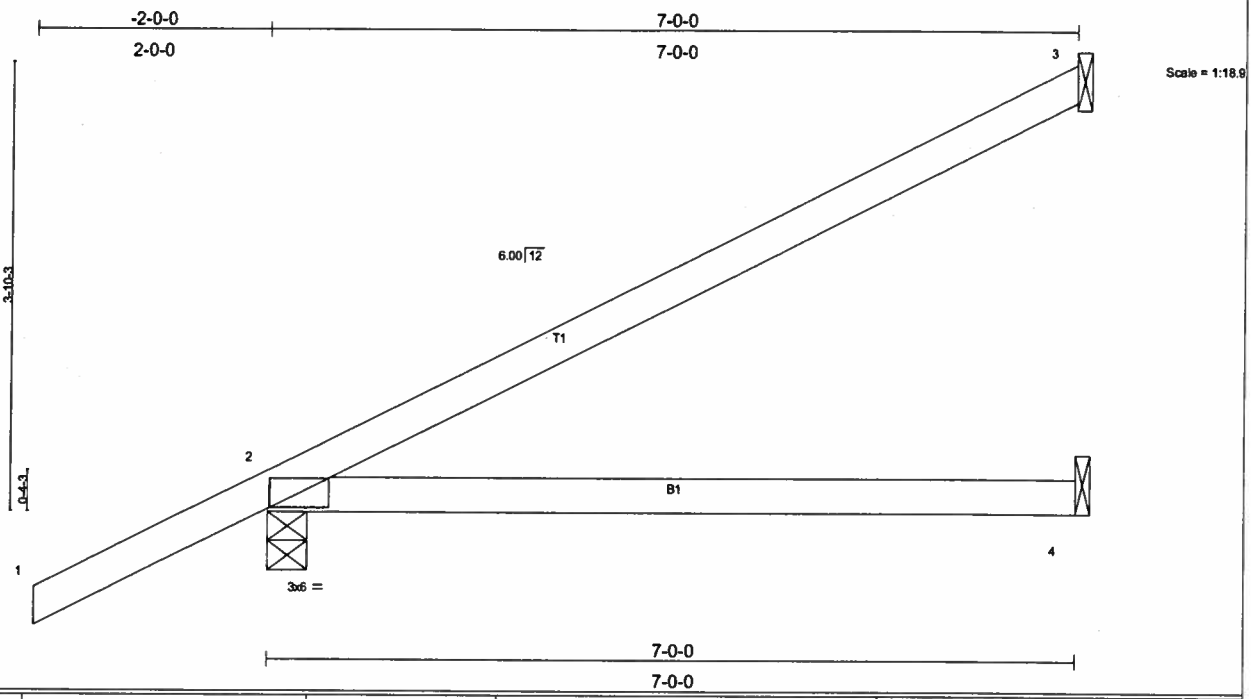

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	0.32	2-4	>253	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	0.27	2-4	>298	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=420/0-4-0, 4=104/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-144(load case 5), 2=-296(load case 5), 4=-67(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-131/58
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 3, 296 lb uplift at joint 2 and 67 lb uplift at joint 4.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061009

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	EJ7A	COMMON	3	1	Job Reference (optional)

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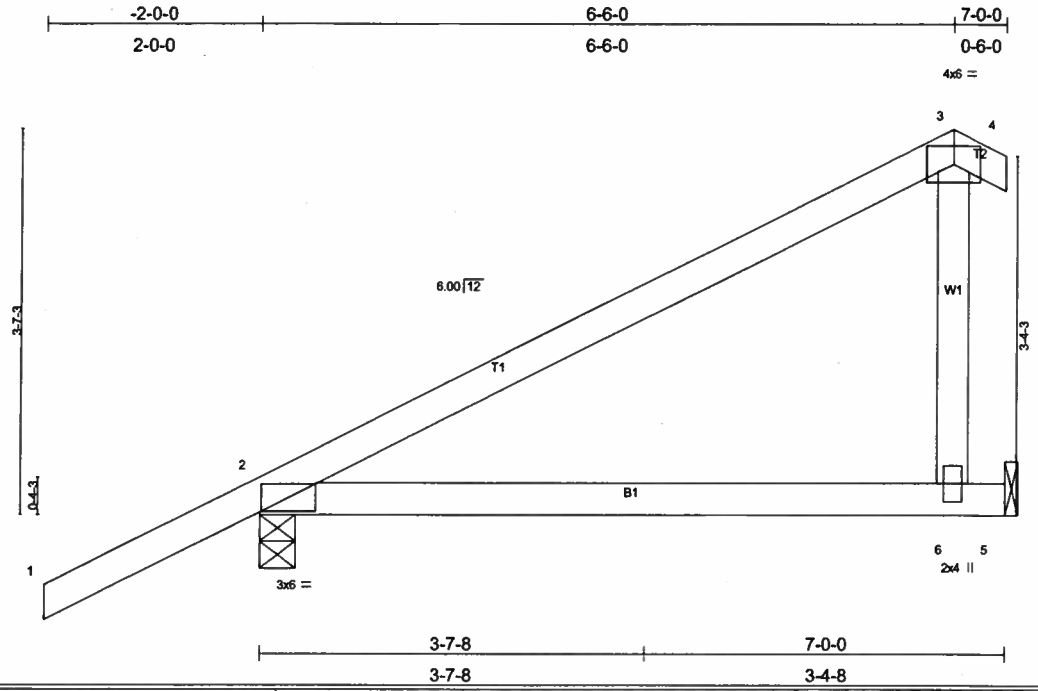

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.14	2-6	>589	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.23	2-6	>355	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00		n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 30 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=420/0-4-0, 5=269/Mechanical, 5=269/Mechanical
Max Horz 2=201(load case 5)
Max Uplift2=-222(load case 5), 5=-119(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-91/56, 3-4=0/12
BOT CHORD 2-6=0/0, 5-6=0/0
WEBS 3-6=-183/219

JOINT STRESS INDEX
2 = 0.51, 3 = 0.09 and 6 = 0.12

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 222 lb uplift at joint 2 and 119 lb uplift at joint 5.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061010

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	EJ7T	SPECIAL	4	1	Job Reference (optional)

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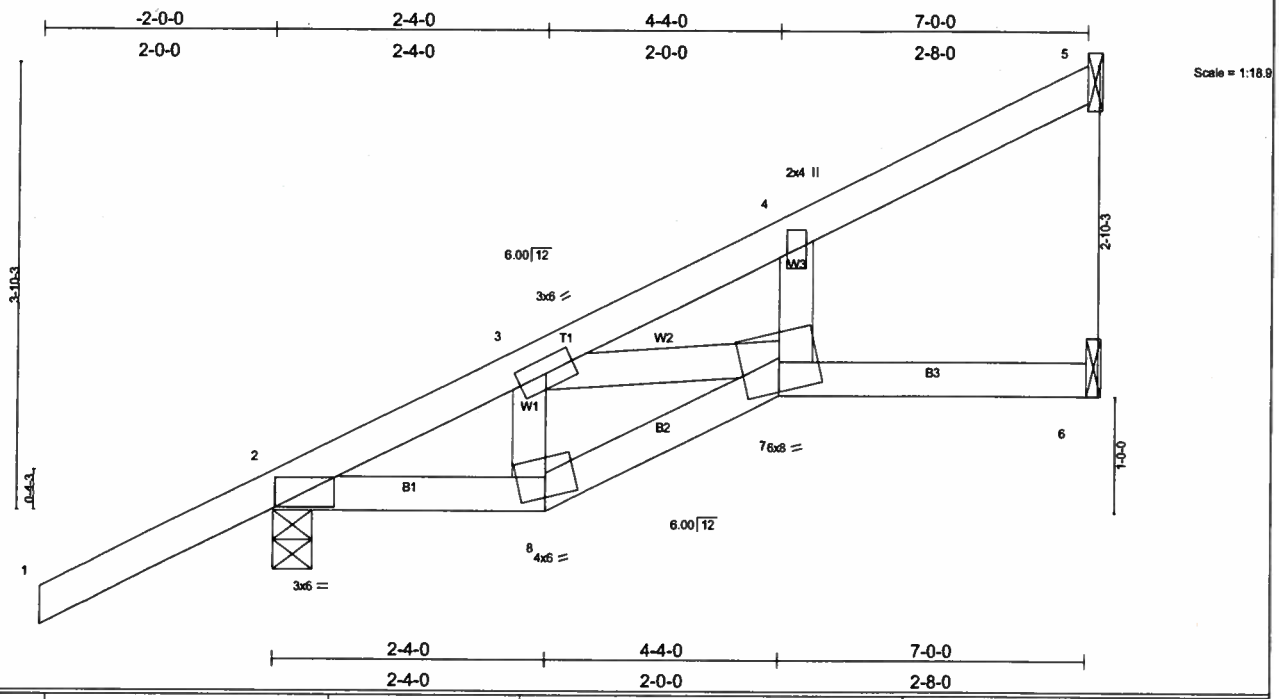

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	0.11	7	>726	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.14	7	>587	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 32 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 5=144/Mechanical, 2=420/0-4-0, 6=121/Mechanical
Max Horz 2=224(load case 5)
Max Uplift5=-91(load case 5), 2=-211(load case 5), 6=-35(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-443/65, 3-4=-110/0, 4-5=-71/59
BOT CHORD 2-8=-230/362, 7-8=-208/361, 6-7=-0/0
WEBS 3-8=-1/65, 3-7=-355/228, 4-7=0/61

JOINT STRESS INDEX
2 = 0.55, 3 = 0.09, 4 = 0.04, 7 = 0.74 and 8 = 0.29

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 91 lb uplift at joint 5, 211 lb uplift at joint 2 and 35 lb uplift at joint 6.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061011

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	HJ9	MONO TRUSS	5	1	Job Reference (optional)

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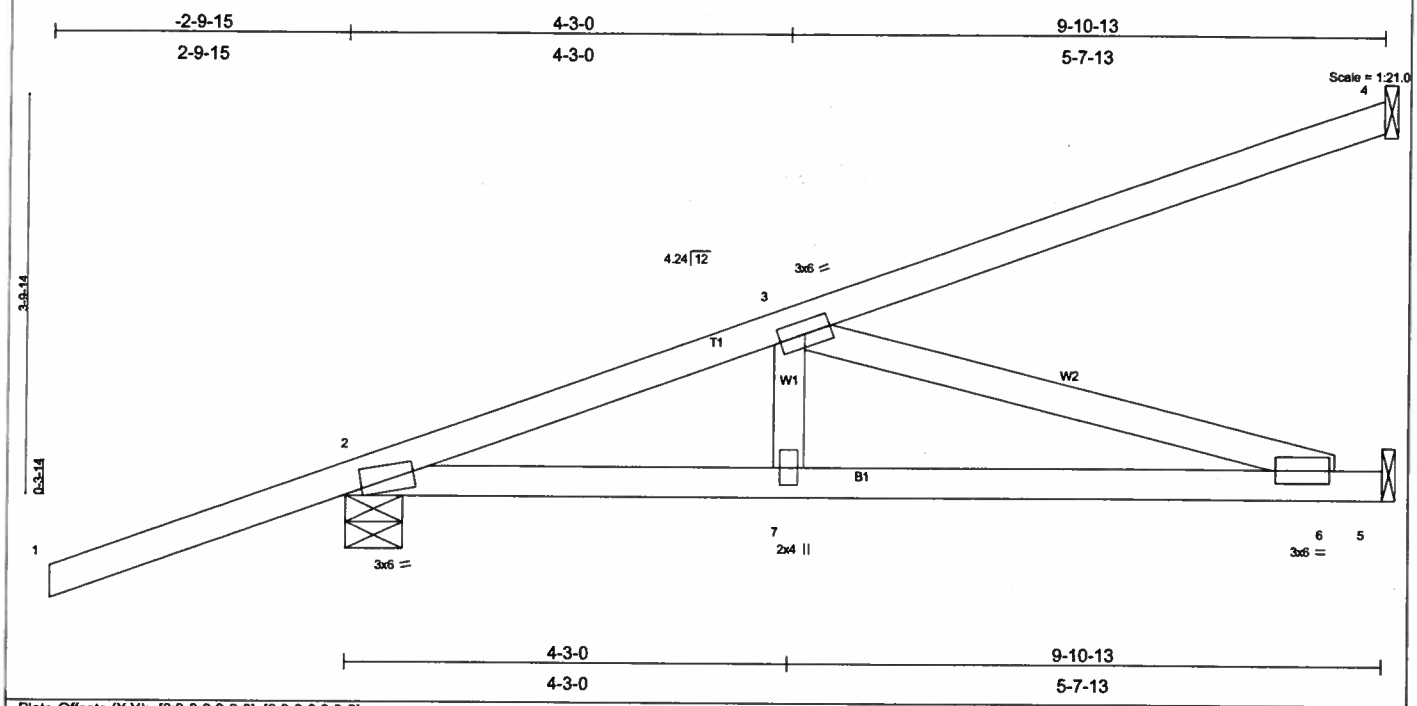

Plate Offsets (X,Y): [3:0-0-0,0-0-0], [6:0-0-0,0-0-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.11	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.18	6-7	>622	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.46	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-11-13 oc bracing.

REACTIONS (lb/size) 4=270/Mechanical, 2=537/0-6-7, 5=372/Mechanical
Max Horz 2=270(load case 2)
Max Uplift4=-233(load case 2), 2=-404(load case 2), 5=-180(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-877/359, 3-4=-105/66
BOT CHORD 2-7=-530/811, 6-7=-530/811, 5-6=0/0
WEBS 3-7=-94/187, 3-6=-844/553

JOINT STRESS INDEX
2 = 0.79, 3 = 0.23, 6 = 0.24 and 7 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 4, 404 lb uplift at joint 2 and 180 lb uplift at joint 5.
3) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-5=-74(F=-22, B=-22)

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061012

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	HJ9T	SPECIAL	1	1	Job Reference (optional)

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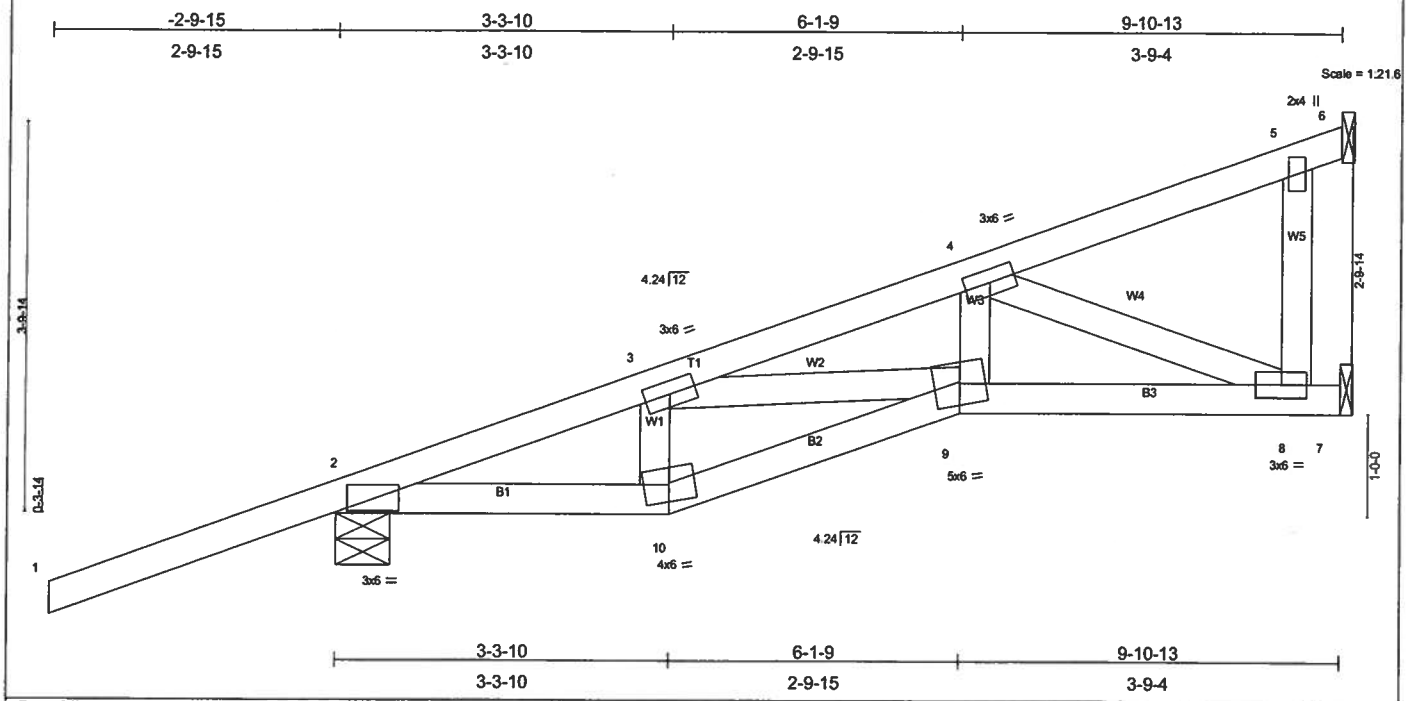

Plate Offsets (X,Y): [2:0-2-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.05	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.50	Vert(TL)	-0.08	8-9	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0.02	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 50 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-8-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-1-8 oc bracing.

REACTIONS (lb/size) 6=272/Mechanical, 2=537/0-6-6, 7=370/Mechanical
Max Horz 2=267(load case 2)
Max Uplift6=-46(load case 2), 2=-286(load case 2), 7=-240(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-652/16, 3-4=-1152/310, 4-5=-76/27, 5-6=-18/89
BOT CHORD 2-10=-163/581, 9-10=-165/616, 8-9=-449/1027, 7-8=0/0
WEBS 3-10=-218/140, 3-9=-325/522, 4-9=-50/414, 4-8=-1102/481, 5-8=0/121

JOINT STRESS INDEX
2 = 0.75, 3 = 0.26, 4 = 0.30, 5 = 0.07, 8 = 0.31, 9 = 0.50 and 10 = 0.31

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 6, 286 lb uplift at joint 2 and 240 lb uplift at joint 7.
3) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-6=-134(F=-40, B=-40), 2=0(F=15, B=15)-to-10=-24(F=3, B=3), 10=-24(F=3, B=3)-to-9=-45(F=-8, B=-8), 9=-45(F=-8, B=-8)-to-7=-74(F=-22, B=-22)

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061013

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T01	COMMON	3	1	Job Reference (optional)

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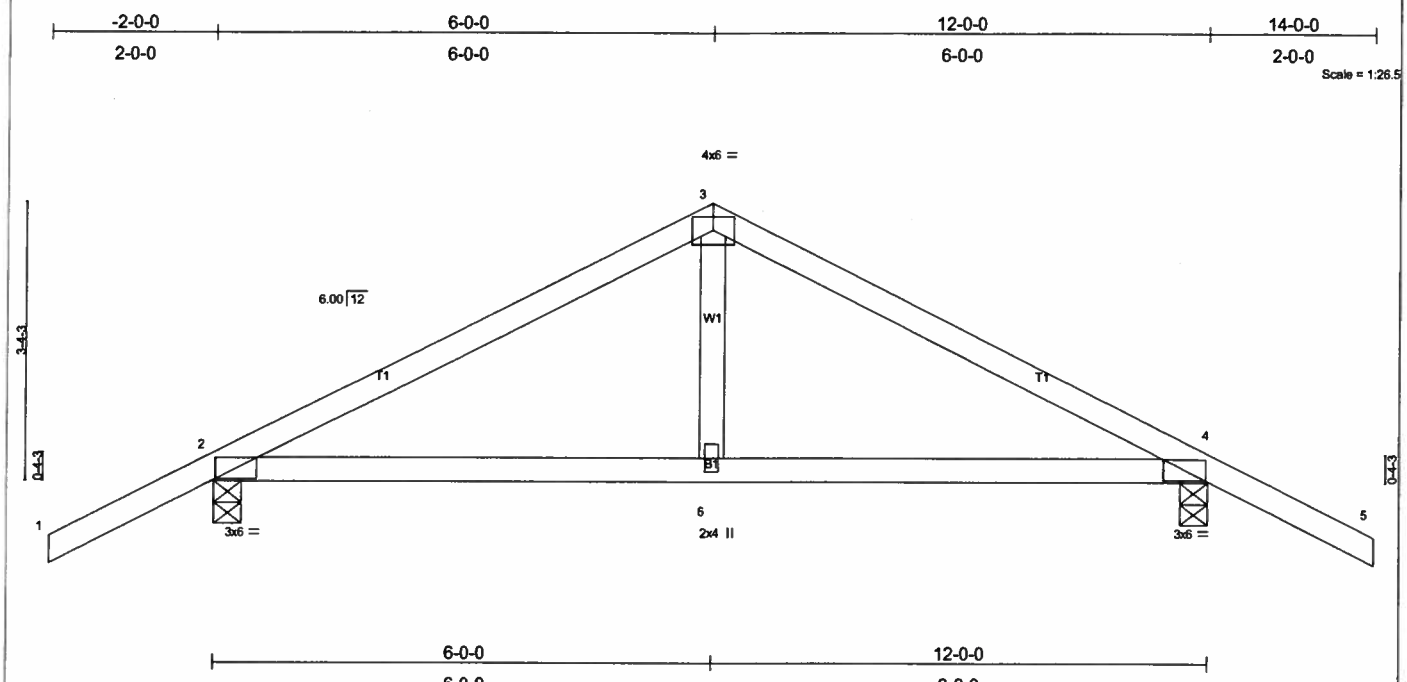

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.04	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	-0.06	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 49 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=607/0-4-0, 4=607/0-4-0
Max Horz 2=80(load case 5)
Max Uplift2=-290(load case 5), 4=-290(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-653/255, 3-4=-653/255, 4-5=0/47
BOT CHORD 2-6=-71/520, 4-6=-71/520
WEBS 3-6=0/201

JOINT STRESS INDEX
2 = 0.42, 3 = 0.67, 4 = 0.42 and 6 = 0.15

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 290 lb uplift at joint 2 and 290 lb uplift at joint 4.

LOAD CASE(S) Standard

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Dwg.#0731061014

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T01A	COMMON	1	2	Job Reference (optional)

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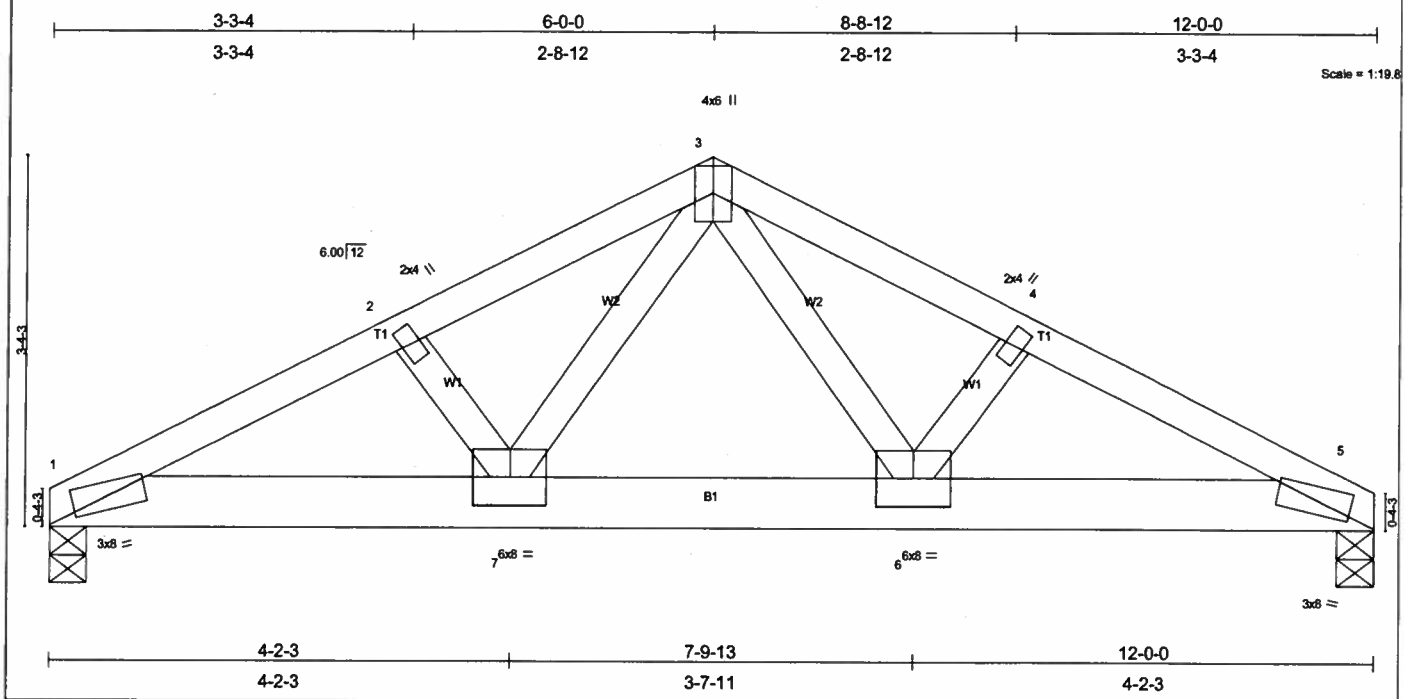

Plate Offsets (X,Y): [1:0-2-15,0-0-2], [5:0-2-15,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.07	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.11	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.44	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 125 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-6-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=3634/0-4-0, 5=3634/0-4-0
Max Horz 1=-43(load case 2)
Max Uplift1=-1346(load case 4), 5=-1346(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5585/2065, 2-3=-5505/2066, 3-4=-5505/2067, 4-5=-5585/2066
BOT CHORD 1-7=-1843/4952, 6-7=-1244/3479, 5-6=-1802/4952
WEBS 2-7=-74/103, 3-7=-1036/2752, 3-6=-1037/2752, 4-6=-74/103

JOINT STRESS INDEX
1 = 0.82, 2 = 0.04, 3 = 0.67, 4 = 0.04, 5 = 0.82, 6 = 0.40 and 7 = 0.40

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1346 lb uplift at joint 1 and 1346 lb uplift at joint 5.
6) Girder carries tie-in span(s): 28-0-0 from 0-0-0 to 12-0-0

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-54, 1-5=-569(F=-539)

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Dwg.#0731061015

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T01G	COMMON	1	1	Job Reference (optional)

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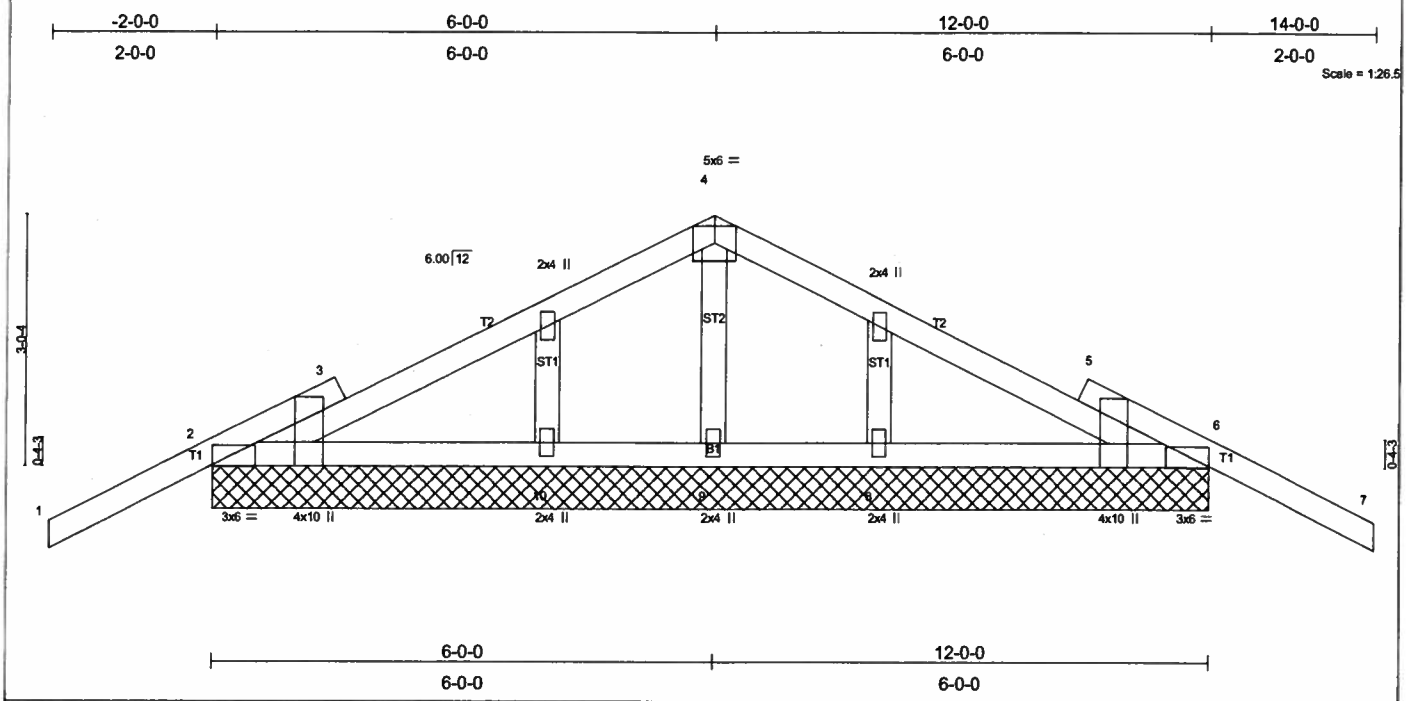

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [6:0-3-8,Edge], [6:0-0-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	0.05	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	0.07	7	n/r	90		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 56 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=890/12-0-0, 6=890/12-0-0, 9=-108/12-0-0, 10=256/12-0-0, 8=256/12-0-0
Max Horz 2=-76(load case 6)
Max Uplift2=-454(load case 5), 6=-454(load case 6), 9=-108(load case 1), 10=-45(load case 5), 8=-45(load case 6)
Max Grav 2=890(load case 1), 6=890(load case 1), 9=63(load case 5), 10=256(load case 1), 8=256(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-19/99, 2-3=-909/527, 3-4=-820/525, 4-5=-820/525, 5-6=-909/527, 6-7=-19/99
BOT CHORD 2-10=-322/733, 9-10=-322/733, 8-9=-322/733, 6-8=-322/733

JOINT STRESS INDEX
2 = 0.93, 2 = 0.00, 3 = 0.00, 3 = 0.00, 4 = 0.82, 5 = 0.00, 5 = 0.00, 6 = 0.93, 6 = 0.00, 8 = 0.00, 9 = 0.00, 10 = 0.00, 11 = 0.00 and 12 = 0.00

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 454 lb uplift at joint 2, 454 lb uplift at joint 6, 108 lb uplift at joint 9, 45 lb uplift at joint 10 and 45 lb uplift at joint 8.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-114(F=-60), 4-7=-114(F=-60), 2-6=-30

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Dwg.#0731061016

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T02	HIP	1	1	Job Reference (optional)

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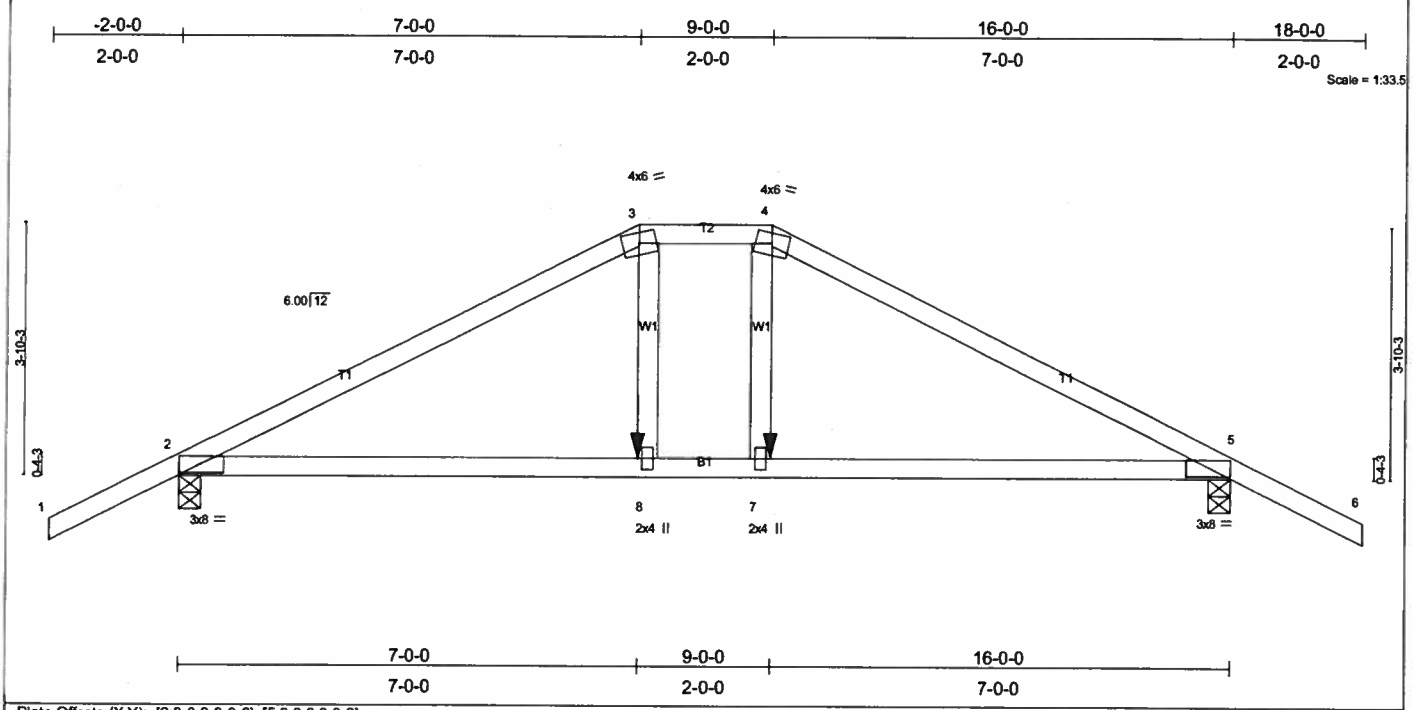

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [5:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.17	5-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.24	5-7	>770	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.25	Horz(TL)	0.05	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 67 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-7 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-1-3 oc bracing.

REACTIONS (lb/size) 2=1400/0-4-0, 5=1400/0-4-0
Max Horz 2=87(load case 4)
Max Uplift2=-858(load case 4), 5=-858(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2289/1218, 3-4=-1989/1146, 4-5=-2289/1217, 5-6=0/47
BOT CHORD 2-8=-994/1960, 7-8=-1008/1989, 5-7=-991/1960
WEBS 3-8=-399/788, 4-7=-399/788

JOINT STRESS INDEX
2 = 0.73, 3 = 0.82, 4 = 0.82, 5 = 0.73, 7 = 0.58 and 8 = 0.58

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 858 lb uplift at joint 2 and 858 lb uplift at joint 5.
5) Girder carries hip end with 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 9-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-113(F=-58), 4-6=-54, 2-8=-30, 7-8=-62(F=-33), 5-7=-30
Concentrated Loads (lb)
Vert: 8=-539(F) 7=-539(F)

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Dwg.#0731061017

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T03	COMMON	3	1	Job Reference (optional)

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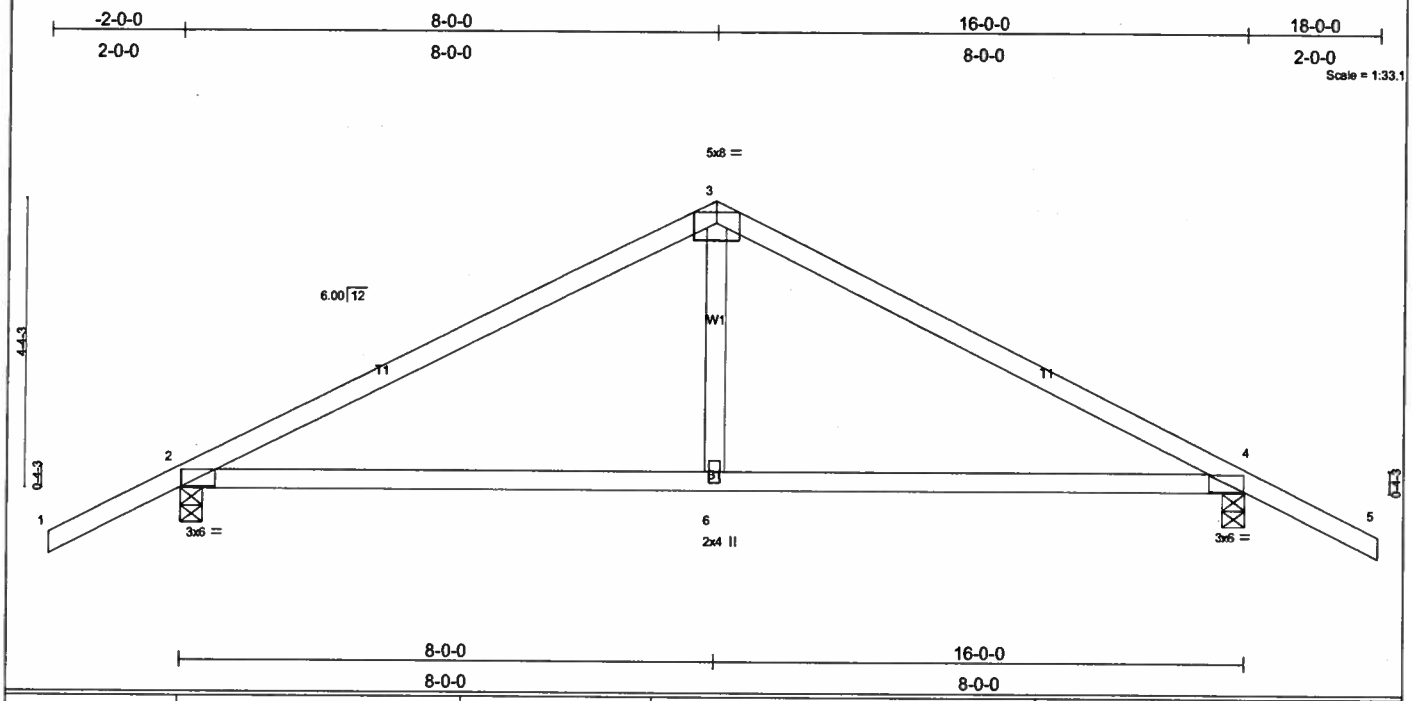

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	0.24	4-6	>789	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.20	4-6	>937	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 63 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-1-7 oc bracing.

REACTIONS (lb/size) 2=775/0-4-0, 4=775/0-4-0
Max Horz 2=-94(load case 6)
Max Uplift2=-539(load case 5), 4=-539(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-924/975, 3-4=-924/975, 4-5=0/47
BOT CHORD 2-6=-683/746, 4-6=-683/746
WEBS 3-6=-487/293

JOINT STRESS INDEX
2 = 0.62, 3 = 0.99, 4 = 0.62 and 6 = 0.21

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 539 lb uplift at joint 2 and 539 lb uplift at joint 4.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061018

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T04	COMMON	2	1	Job Reference (optional)

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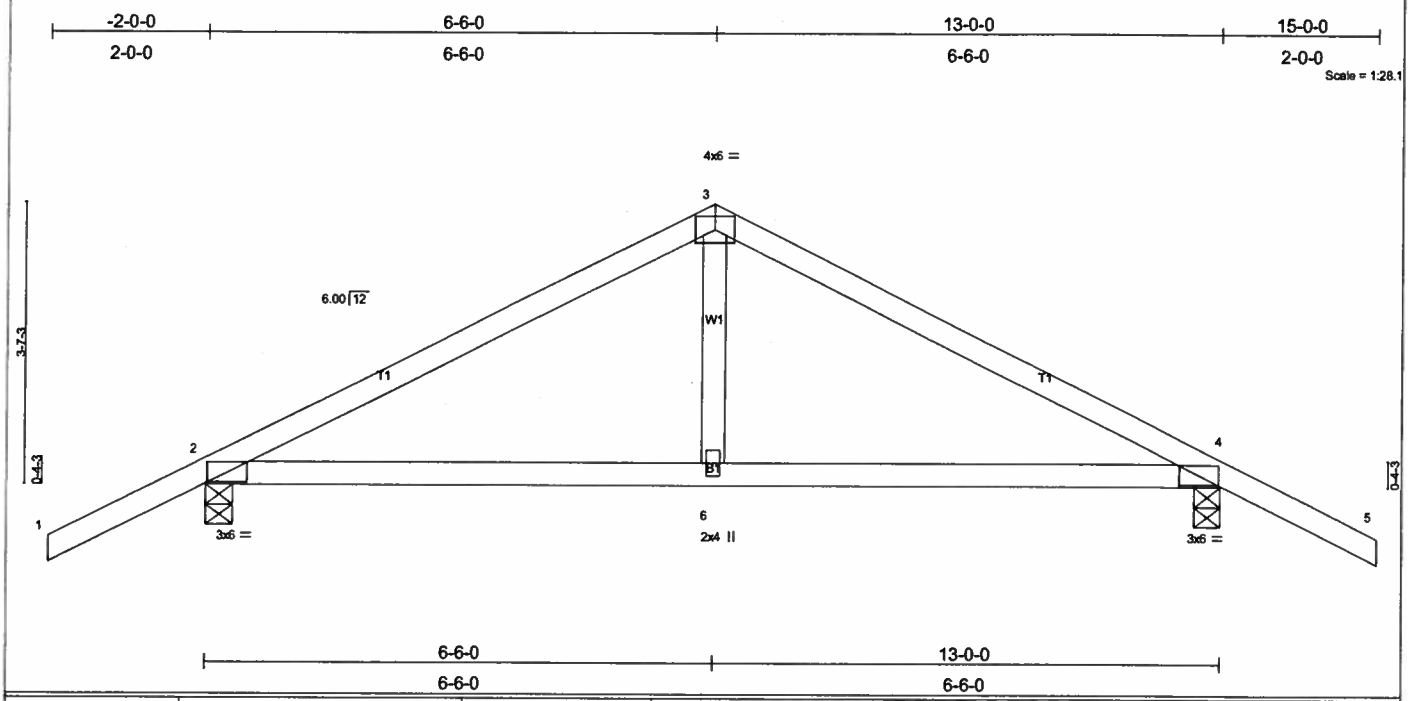

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.05	2-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.08	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 52 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=649/0-4-0, 4=649/0-4-0
Max Horz 2=-84(load case 6)
Max Uplift2=-303(load case 5), 4=-303(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-722/284, 3-4=-722/284, 4-5=0/47
BOT CHORD 2-6=-89/578, 4-6=-89/578
WEBS 3-6=0/225

JOINT STRESS INDEX
2 = 0.47, 3 = 0.79, 4 = 0.47 and 6 = 0.16

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 303 lb uplift at joint 2 and 303 lb uplift at joint 4.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061019

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T04A	COMMON	1	2	Job Reference (optional)

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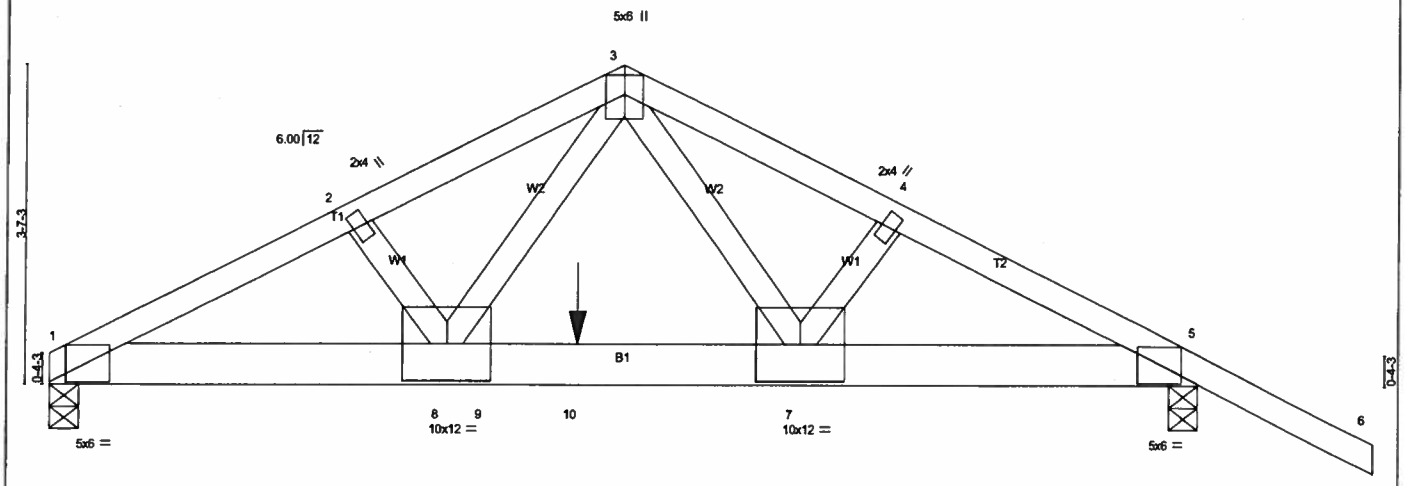
3-6-4	6-6-0	9-5-12	13-0-0	15-0-0
3-6-4	2-11-12	2-11-12	3-6-4	2-0-0

Scale = 1:24.8



4-6-3	6-0-0	8-5-13	13-0-0
4-6-3	1-5-13	2-5-13	4-6-3

Plate Offsets (X,Y): [1:0-3-0,0-2-9], [5:0-3-0,0-2-9]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.10	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.15	7-8	>983	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.71	Horz(TL)	0.03	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 142 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=4194/0-4-0, 5=2389/0-4-0
Max Horz 1=-112(load case 5)
Max Uplift1=-1555(load case 4), 5=-961(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-6694/2480, 2-3=-6610/2482, 3-4=-4687/1729, 4-5=-4774/1719, 5-6=0/51
BOT CHORD 1-8=-2184/5933, 8-9=-1245/3571, 9-10=-1245/3571, 7-10=-1245/3571, 5-7=-1461/4187
WEBS 2-8=-58/104, 3-8=-1676/4437, 3-7=-395/1182, 4-7=-12/113

JOINT STRESS INDEX
1 = 0.79, 2 = 0.04, 3 = 0.75, 4 = 0.04, 5 = 0.79, 7 = 0.38 and 8 = 0.38

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1555 lb uplift at joint 1 and 961 lb uplift at joint 5.
6) Girder carries tie-in span(s): 30-0-0 from 0-0-0 to 5-0-0
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2594 lb down and 979 lb up at 6-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-54, 1-9=-611(F=-581), 5-9=-30
Concentrated Loads (lb)
Vert: 10=-2594(F)

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061020

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T04G	COMMON	1	1	Job Reference (optional)

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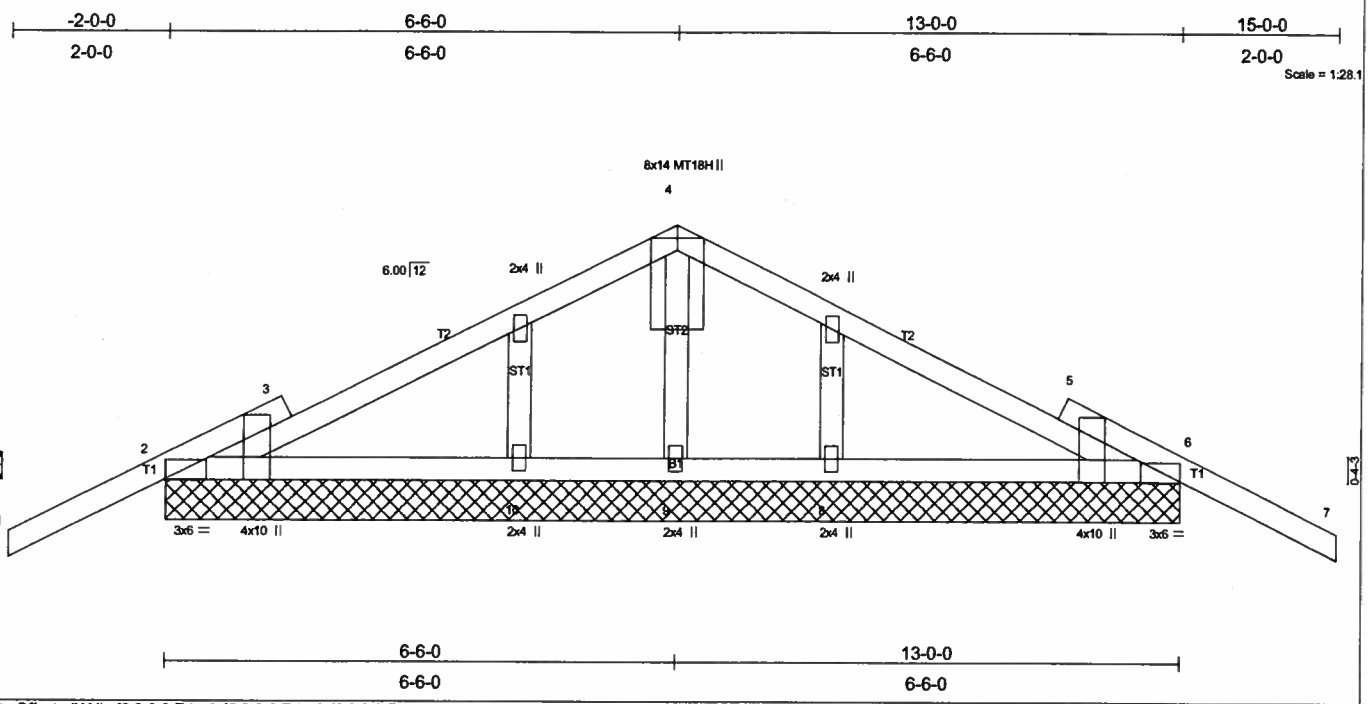

Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-0-8,Edge], [6:0-3-8,Edge], [6:0-0-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	0.08	7	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	0.12	7	n/r	90	MT18H	244/190
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 60 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-10-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-4 oc bracing.

REACTIONS (lb/size) 2=947/13-0-0, 6=947/13-0-0, 9=-155/13-0-0, 10=295/13-0-0, 8=295/13-0-0
Max Horz 2=-79(load case 6)
Max Uplift2=-475(load case 5), 6=-475(load case 6), 9=-155(load case 1), 10=-56(load case 5), 8=-56(load case 6)
Max Grav 2=947(load case 1), 6=947(load case 1), 9=76(load case 5), 10=295(load case 1), 8=295(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-18/99, 2-3=-993/561, 3-4=-898/561, 4-5=-898/561, 5-6=-993/561, 6-7=-18/99
BOT CHORD 2-10=-350/803, 9-10=-350/803, 8-9=-350/803, 6-8=-350/803

JOINT STRESS INDEX
2 = 1.00, 2 = 0.00, 3 = 0.00, 3 = 0.00, 4 = 0.91, 5 = 0.00, 5 = 0.00, 6 = 1.00, 6 = 0.00, 8 = 0.00, 9 = 0.00, 10 = 0.00, 11 = 0.00 and 12 = 0.00

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
4) All plates are MT20 plates unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 475 lb uplift at joint 2, 475 lb uplift at joint 6, 155 lb uplift at joint 9, 56 lb uplift at joint 10 and 56 lb uplift at joint 8.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-114(F=-60), 4-7=-114(F=-60), 2-6=-30

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061021

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T05	SPECIAL	1	2	Job Reference (optional)

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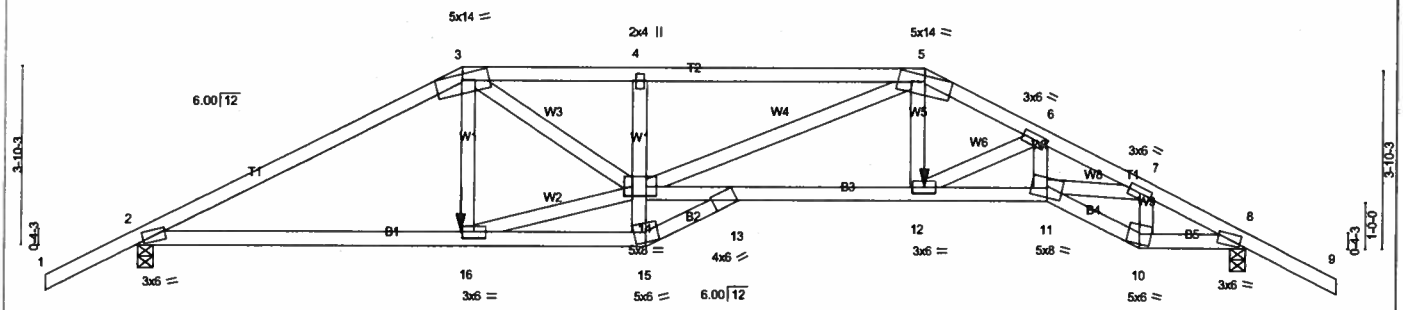
-2-0-0 | 3-9-4 | 7-0-0 | 11-0-0 | 17-0-0 | 19-8-0 | 21-8-0 | 24-0-0 | 26-0-0
2-0-0 | 3-9-4 | 3-2-12 | 4-0-0 | 6-0-0 | 2-8-0 | 2-0-0 | 2-4-0 | 2-0-0

Scale: 1/4"=1'



3-9-4 | 7-0-0 | 11-0-0 | 13-0-0 | 17-0-0 | 19-8-0 | 21-8-0 | 24-0-0
3-9-4 | 3-2-12 | 4-0-0 | 2-0-0 | 4-0-0 | 2-8-0 | 2-0-0 | 2-4-0

Plate Offsets (X,Y): [2:0-1-9,0-0-7], [8:0-1-9,0-0-7], [14:0-2-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.23	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.37	12-13	>770	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.48	Horz(TL)	0.14	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 256 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-1-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-2-9 oc bracing.

REACTIONS (lb/size) 2=2108/0-4-0, 8=2039/0-4-0
Max Horz 2=112(load case 4)
Max Uplift2=-1129(load case 4), 8=-929(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3824/1981, 3-4=-5335/2606, 4-5=-5362/2606, 5-6=-5002/2147, 6-7=-6379/2596, 7-8=-3675/1472, 8-9=0/47
BOT CHORD 2-16=-1745/3323, 15-16=-344/420, 13-15=-275/264, 13-14=-1616/4310, 12-13=-1868/4589, 11-12=-2193/5529, 10-11=-1285/3306, 8-10=-1237/3161
WEBS 3-16=-138/41, 3-14=-1018/2438, 14-15=0/267, 4-14=-527/452, 5-14=-733/942, 5-12=-481/1383, 6-12=-1131/451, 6-11=-402/1159, 7-11=-1076/2679, 7-10=-1350/558, 14-16=-1450/3000

JOINT STRESS INDEX
2 = 0.81, 3 = 0.47, 4 = 0.34, 5 = 0.67, 6 = 0.43, 7 = 0.66, 8 = 0.78, 10 = 0.43, 11 = 0.78, 12 = 0.45, 13 = 0.29, 14 = 0.85, 15 = 0.35 and 16 = 0.84

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1129 lb uplift at joint 2 and 929 lb uplift at joint 8.
7) Girder carries tie-in span(s): 4-4-11 from 11-0-0 to 14-0-0; 4-4-11 from 11-0-0 to 17-0-0; 4-5-0 from 11-0-0 to 17-0-0
8) Girder carries hip end with 13-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 499 lb up at 7-0-0, and 400 lb down and 151 lb up at 17-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-117(F=-63), 4-5=-97(F=-43), 5-9=-54, 2-16=-30, 15-16=-65(F=-35), 13-15=-74(F=-44), 13-14=-44(F), 12-13=-74(F=-44), 11-12=-30, 10-11=-30, 8-10=-30
Concentrated Loads (lb)
Vert: 16=-539(F) 12=-400(F)

JULY 31, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061022

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T06	SPECIAL	1	1	Job Reference (optional)

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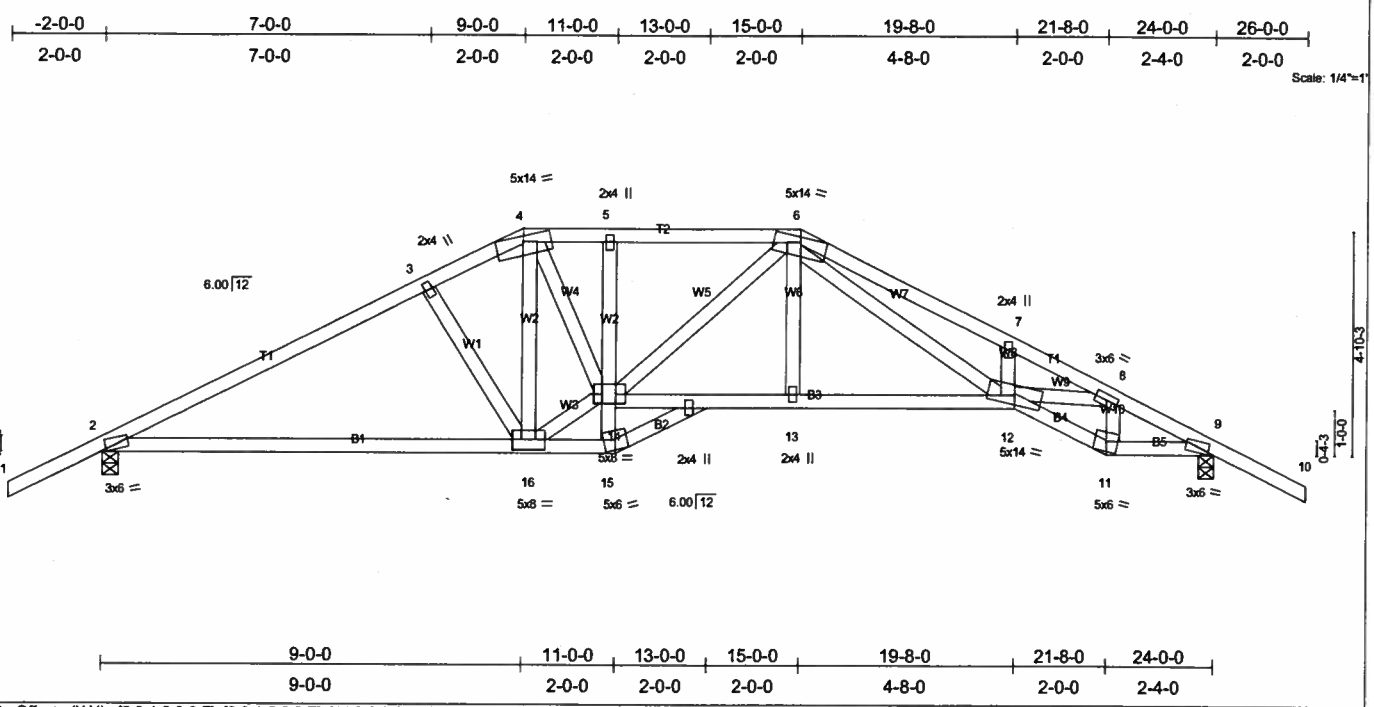

Plate Offsets (X,Y): [2:0-1-0,0-0-7], [9:0-1-8,0-0-7], [14:0-2-8,0-2-8], [17:0-2-0,0-1-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.21	2-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.35	2-16	>810	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.43	Horz(TL)	0.12	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 138 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-11-5 oc bracing.

REACTIONS (lb/size) 2=1109/0-4-0, 9=1109/0-4-0
Max Horz 2=-101(load case 6)
Max Uplift2=-431(load case 5), 9=-431(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1623/652, 3-4=-1426/634, 4-5=-1601/693, 5-6=-1603/690, 6-7=-2908/1125, 7-8=-2898/1016, 8-9=-1758/586, 9-10=0/47
BOT CHORD 2-16=-394/1376, 15-16=-10/14, 13-14=-366/1570, 12-13=-367/1562, 11-12=-378/1561, 9-11=-372/1492
WEBS 3-16=-322/274, 4-16=-161/90, 14-15=-99/0, 5-14=-190/141, 6-13=0/180, 6-12=-488/1284, 7-12=-161/205, 8-12=-406/1158, 8-11=-598/215, 4-14=-273/811, 14-16=-272/1352, 6-14=-97/166

JOINT STRESS INDEX
2 = 0.79, 3 = 0.34, 4 = 0.42, 5 = 0.34, 6 = 0.65, 7 = 0.34, 8 = 0.57, 9 = 0.78, 11 = 0.40, 12 = 0.51, 13 = 0.34, 14 = 0.71, 15 = 0.20, 16 = 0.63 and 17 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 431 lb uplift at joint 2 and 431 lb uplift at joint 9.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061023

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T07	SPECIAL	1	1	Job Reference (optional)

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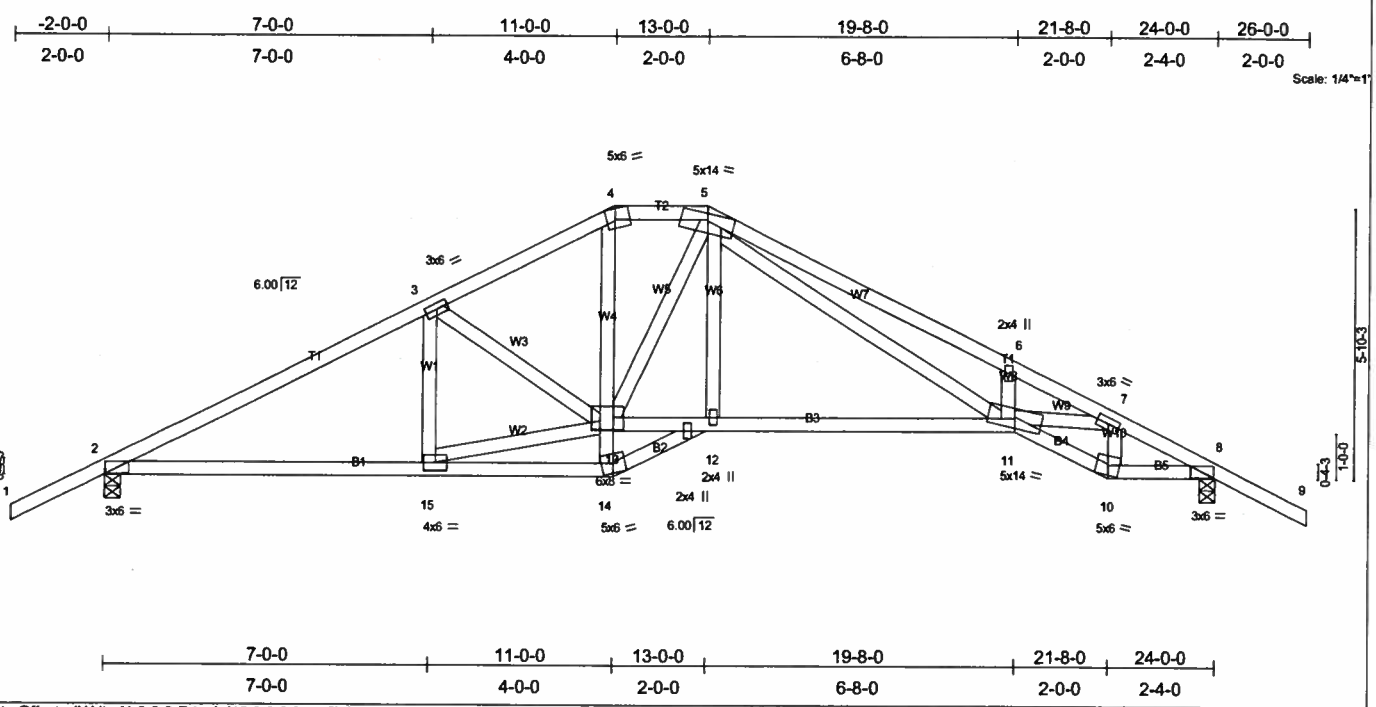

Plate Offsets (X,Y): [4:0-2-3,Edge], [12:0-2-0,0-1-15], [13:0-2-8,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.22	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	-0.35	11-12	>800	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.13	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 141 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-11-12 oc bracing.

REACTIONS (lb/size) 2=1109/0-4-0, 8=1109/0-4-0
Max Horz 2=-115(load case 6)
Max Uplift2=-444(load case 5), 8=-444(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1667/659, 3-4=-1460/634, 4-5=-1248/597, 5-6=-3035/1239, 6-7=-2968/1076, 7-8=-1740/592, 8-9=0/47
BOT CHORD 2-15=-401/1408, 14-15=-8/33, 12-13=-278/1321, 11-12=-277/1328, 10-11=-375/1543, 8-10=-372/1472
WEBS 3-15=-139/135, 13-14=0/34, 4-13=-283/615, 5-12=0/216, 5-11=-665/1614, 6-11=-266/308, 7-11=-476/1259, 7-10=-601/195, 3-13=-242/196, 13-15=-405/1418, 5-13=-289/137

JOINT STRESS INDEX
2 = 0.75, 3 = 0.41, 4 = 0.52, 5 = 0.81, 6 = 0.34, 7 = 0.62, 8 = 0.77, 10 = 0.40, 11 = 0.52, 12 = 0.34, 12 = 0.34, 13 = 0.85, 14 = 0.20 and 15 = 0.65

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 444 lb uplift at joint 2 and 444 lb uplift at joint 8.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061024

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T08	SPECIAL	1	1	Job Reference (optional)

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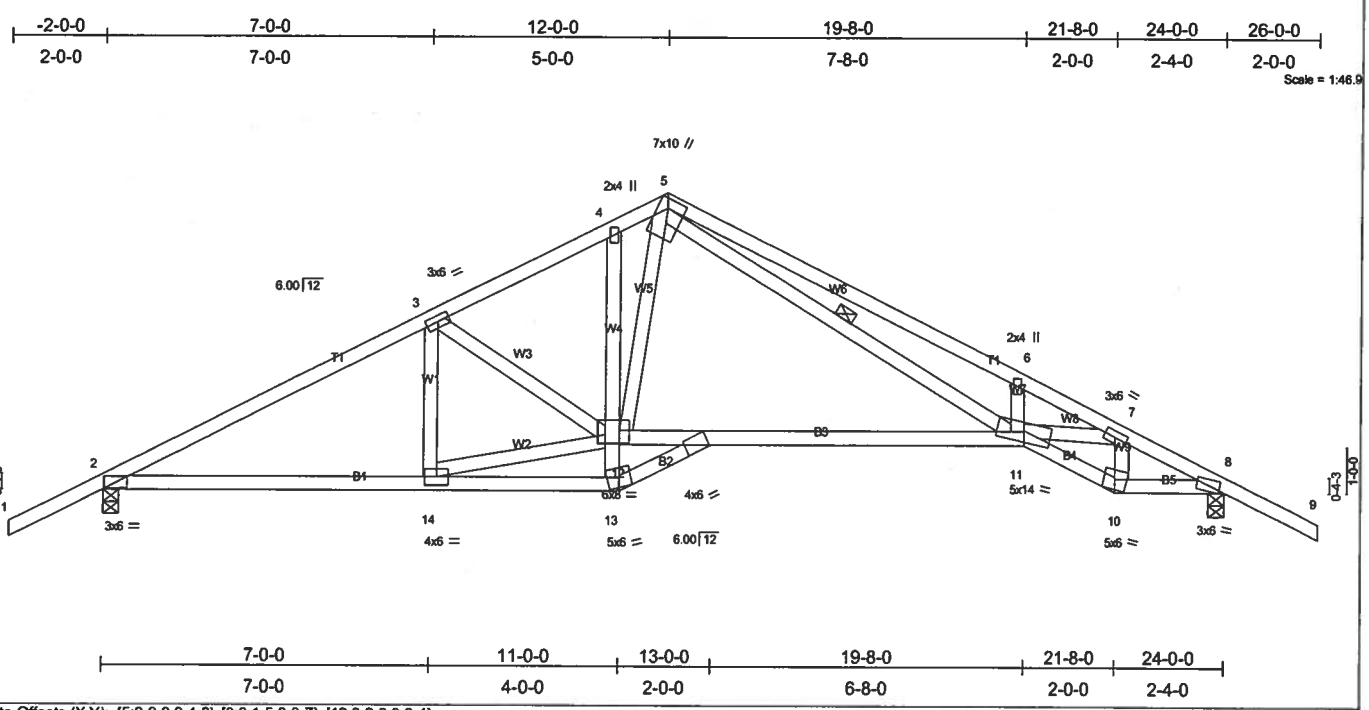

Plate Offsets (X,Y): [5:0-2-8,0-4-8], [8:0-1-5,0-0-7], [12:0-2-8,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.38	11-12	>745	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.63	11-12	>449	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.63	Horz(TL)	0.14	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 137 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-10-14 oc bracing.
WEBS 1 Row at midpt 5-11

REACTIONS (lb/size) 2=1109/0-4-0, 8=1109/0-4-0
Max Horz 2=122(load case 5)
Max Uplift2=-450(load case 5), 8=-450(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1668/667, 3-4=-1459/643, 4-5=-1278/587, 5-6=-3207/1291, 6-7=-3129/1104, 7-8=-1701/597, 8-9=0/47
BOT CHORD 2-14=-408/1411, 13-14=-8/32, 11-12=-246/1180, 10-11=-373/1534, 8-10=-373/1439
WEBS 3-14=-140/137, 3-12=-259/194, 5-11=-736/1949, 6-11=-298/354, 7-11=-508/1452, 7-10=-694/180, 12-13=0/34, 4-12=-172/282, 5-12=-103/541, 12-14=-412/1421

JOINT STRESS INDEX
2 = 0.75, 3 = 0.41, 4 = 0.34, 5 = 0.90, 6 = 0.34, 7 = 0.71, 8 = 0.78, 10 = 0.44, 11 = 0.63, 12 = 0.85, 13 = 0.20, 14 = 0.65 and 15 = 0.13

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection: 5.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 450 lb uplift at joint 2 and 450 lb uplift at joint 8.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061025

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T09	COMMON	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jul 31 13:11:58 2006 Page 1

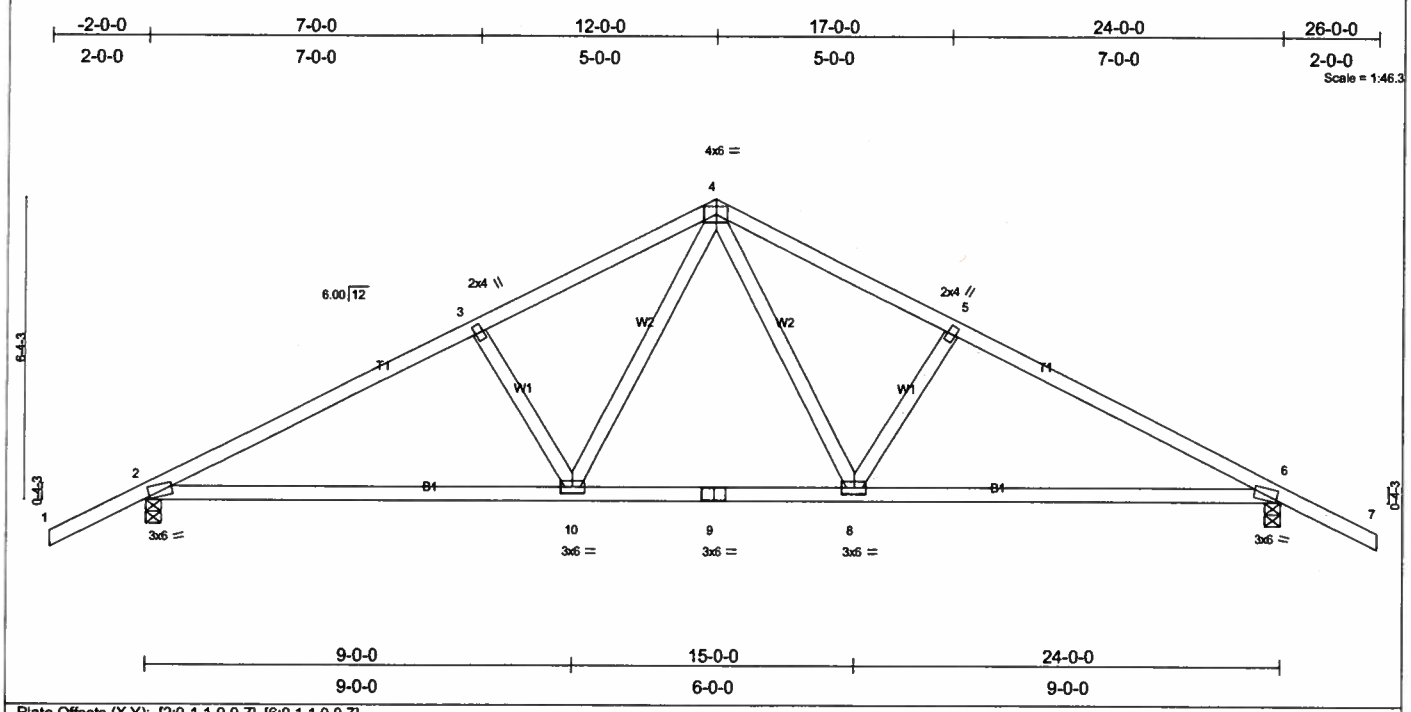

Plate Offsets (X,Y): [2:0-1-1,0-0-7], [6:0-1-1,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.21	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.47	Vert(TL)	-0.35	2-10	>818	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.18	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 113 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-9-2 oc bracing.

REACTIONS (lb/size) 2=1111/0-4-0, 6=1111/0-4-0
Max Horz 2=122(load case 5)
Max Uplift2=-449(load case 5), 6=-449(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1620/678, 3-4=-1433/678, 4-5=-1433/678, 5-6=-1620/678, 6-7=0/47
BOT CHORD 2-10=-418/1374, 9-10=-180/970, 8-9=-180/970, 6-8=-418/1374
WEBS 3-10=-321/285, 4-10=-224/569, 4-8=-224/569, 5-8=-321/285

JOINT STRESS INDEX
2 = 0.79, 3 = 0.34, 4 = 0.47, 5 = 0.34, 6 = 0.79, 8 = 0.47, 9 = 0.37 and 10 = 0.47

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 449 lb uplift at joint 2 and 449 lb uplift at joint 6.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061026

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T10	COMMON	1	2	Job Reference (optional)

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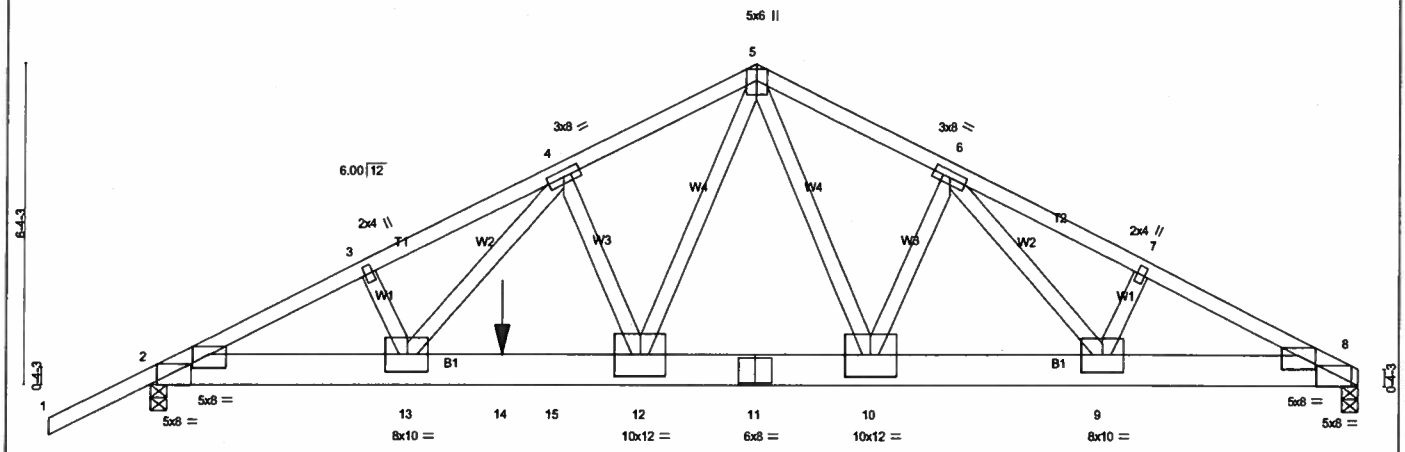
-2-0-0	4-4-5	8-2-3	12-0-0	15-9-13	19-7-11	24-0-0
2-0-0	4-4-5	3-9-13	3-9-13	3-9-13	3-9-13	4-4-5

Scale = 1:43.5



5-1-8	7-0-0	9-8-8	14-3-8	18-10-8	24-0-0
5-1-8	1-10-8	2-8-8	4-7-0	4-7-0	5-1-8

Plate Offsets (X,Y): [2:0-10-0,0-3-15], [2:0-1-11,Edge], [8:0-1-11,Edge], [8:0-10-0,0-3-15]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.26	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.41	12-13	>689	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.81	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 331 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=7409/0-4-0, 2=5797/0-4-0
Max Horz 2=155(load case 4)
Max Uplift8=-2745(load case 5), 2=-2220(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/54, 2-3=-12112/4410, 3-4=-12019/4439, 4-5=-10431/3917, 5-6=-10242/3861, 6-7=-13056/4886, 7-8=-13135/4855
BOT CHORD 2-13=-3979/10806, 13-14=-3519/9646, 14-15=-3519/9646, 12-15=-3519/9646, 11-12=-2660/7472, 10-11=-2660/7472, 9-10=-3478/9700, 8-9=-4303/11750
WEBS 3-13=-207/177, 4-13=-645/1789, 4-12=-979/456, 5-12=-1914/5041, 5-10=-1749/4576, 6-10=-1602/711, 6-9=-1231/3218, 7-9=-256/199

JOINT STRESS INDEX
2 = 0.81, 2 = 0.65, 3 = 0.34, 4 = 0.84, 5 = 0.95, 6 = 0.84, 7 = 0.34, 8 = 0.81, 8 = 0.65, 9 = 0.29, 10 = 0.47, 11 = 0.67, 12 = 0.47 and 13 = 0.29

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2745 lb uplift at joint 8 and 2220 lb uplift at joint 2.
6) Girder carries tie-in span(s): 28-0-0 from 8-0-0 to 24-0-0
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2567 lb down and 969 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-8=-54, 2-15=-30, 8-15=-569(F=-539)
Concentrated Loads (lb)
Vert: 14=-2567(F)

JULY 31, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

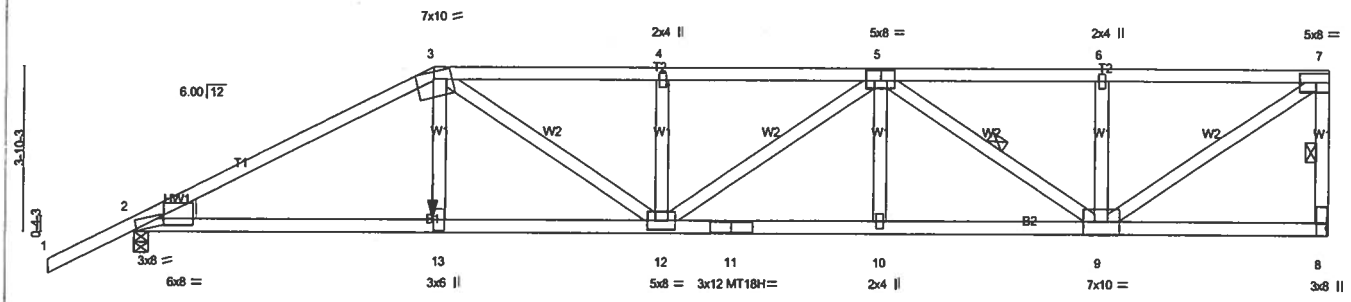
Dwg.#0731061027

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T11	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jul 31 13:12:00 2006 Page 1

-2-0-0	7-0-0	12-3-14	17-6-0	22-8-2	28-0-0
2-0-0	7-0-0	5-3-14	5-2-2	5-2-2	5-3-14

Scale = 1:51.5



7-0-0	12-3-14	17-6-0	22-8-2	28-0-0
7-0-0	5-3-14	5-2-2	5-2-2	5-3-14

Plate Offsets (X,Y): [2:0-0-10,Edge], [2:0-8-8,0-1-8], [5:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.89	Vert(LL)	-0.33	10-12	>992	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.54	10-12	>617	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	NO	WB	0.96	Horz(TL)	0.14	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 149 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2, W2 2 X 4 SYP No.2, W2 2 X 4 SYP No.2, W2 2 X 4 SYP No.2
WEDGE
Left: 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-4-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-6-10 oc bracing.
WEBS 1 Row at midpt 7-8, 5-9

REACTIONS (lb/size) 8=2567/Mechanical, 2=2453/0-4-0
Max Horz 2=227(load case 4)
Max Uplift8=-1155(load case 3), 2=-1050(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-4597/1951, 3-4=-5141/2308, 4-5=-5140/2309, 5-6=-3156/1419, 6-7=-3156/1419, 7-8=-2391/1171
BOT CHORD 2-13=-1776/4013, 12-13=-1785/4047, 11-12=-2170/4833, 10-11=-2170/4833, 9-10=-2170/4833, 8-9=-47/101
WEBS 3-13=-226/830, 3-12=-691/1314, 4-12=-606/531, 5-12=-185/373, 5-10=0/339, 5-9=-2034/912, 6-9=-619/517, 7-9=-1664/3706

JOINT STRESS INDEX
2 = 0.81, 2 = 0.69, 3 = 0.91, 4 = 0.34, 5 = 0.79, 6 = 0.34, 7 = 0.79, 8 = 0.46, 9 = 0.84, 10 = 0.34, 11 = 0.96, 12 = 0.61 and 13 = 0.27

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1155 lb uplift at joint 8 and 1050 lb uplift at joint 2.
5) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-117(F=-63), 2-13=-30, 8-13=-65(F=-35)
Concentrated Loads (lb)
Vert: 13=-539(F)

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061028

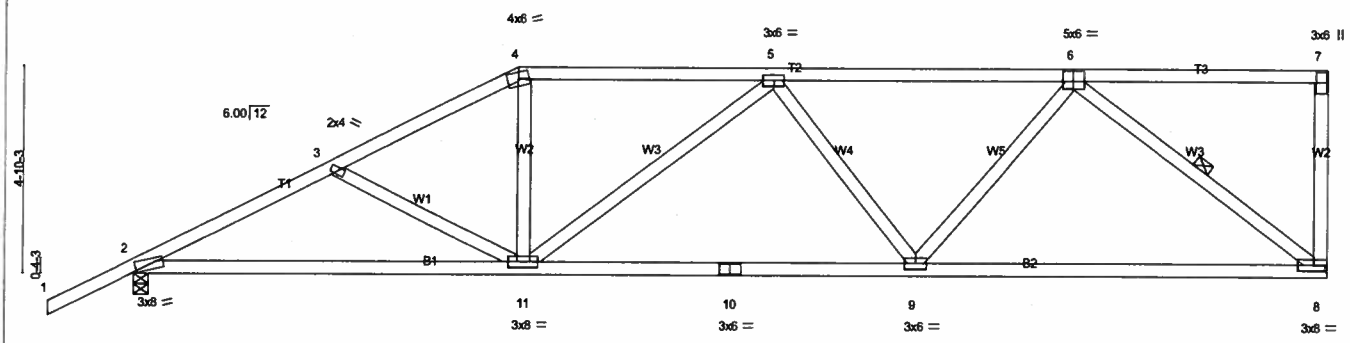
Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T12	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jul 31 13:12:00 2006 Page 1

-2-0-0 | 4-9-4 | 9-0-0 | 15-0-0 | 22-0-0 | 28-0-0

2-0-0 | 4-9-4 | 4-2-12 | 6-0-0 | 7-0-0 | 6-0-0

Scale = 1:51.5



9-0-0 | 18-4-4 | 28-0-0

9-0-0 | 9-4-4 | 9-7-12

Plate Offsets (X,Y): [2:0-0-10,Edge], [6:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	-0.18	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.30	8-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.41	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 146 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-9-1 oc bracing.
WEBS 1 Row at midpt 6-8

REACTIONS (lb/size) 8=1158/Mechanical, 2=1284/0-4-0
Max Horz 2=272(load case 5)
Max Uplift8=-423(load case 4), 2=-459(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2051/799, 3-4=-1820/699, 4-5=-1600/682, 5-6=-1604/626, 6-7=-63/6, 7-8=-145/95
BOT CHORD 2-11=-857/1782, 10-11=-766/1772, 9-10=-766/1772, 8-9=-521/1190
WEBS 3-11=-224/200, 4-11=-94/517, 5-11=-219/210, 5-9=-284/236, 6-9=-168/662, 6-8=-1428/653

JOINT STRESS INDEX
2 = 0.77, 3 = 0.34, 4 = 0.57, 5 = 0.40, 6 = 0.50, 7 = 0.34, 8 = 0.55, 9 = 0.45, 10 = 0.80 and 11 = 0.57

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 423 lb uplift at joint 8 and 459 lb uplift at joint 2.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

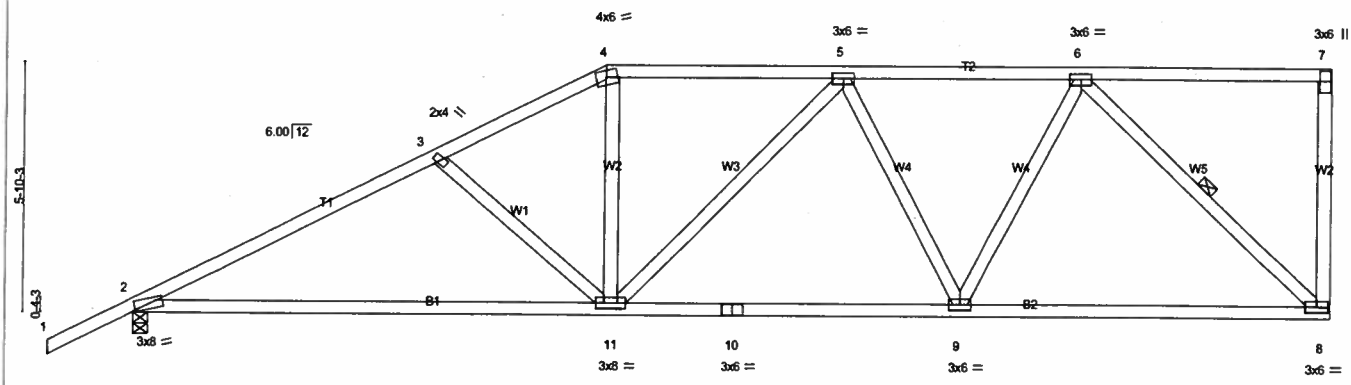
Dwg.#0731061029

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T13	MONO HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jul 31 13:12:01 2006 Page 1

-2-0-0	7-0-0	11-0-0	16-6-13	22-1-11	28-0-0
2-0-0	7-0-0	4-0-0	5-6-13	5-6-13	5-10-5

Scale = 1:51.5



11-0-0	19-4-4	28-0-0
11-0-0	8-4-4	8-7-12

Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	-0.38	2-11	>873	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.65	2-11	>512	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.41	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 152 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-9 oc bracing.
WEBS 1 Row at midpt 6-8

REACTIONS (lb/size) 8=1158/Mechanical, 2=1284/0-4-0
Max Horz 2=318(load case 5)
Max Uplift8=-415(load case 4), 2=-469(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1931/751, 3-4=-1656/660, 4-5=-1436/630, 5-6=-1241/511, 6-7=-45/7, 7-8=-149/99
BOT CHORD 2-11=-840/1668, 10-11=-623/1411, 9-10=-623/1411, 8-9=-424/954
WEBS 3-11=-324/281, 4-11=-118/502, 5-11=-47/123, 5-9=-377/251, 6-9=-192/641, 6-8=-1268/582

JOINT STRESS INDEX
2 = 0.81, 3 = 0.34, 4 = 0.42, 5 = 0.46, 6 = 0.53, 7 = 0.32, 8 = 0.68, 9 = 0.53, 10 = 0.47 and 11 = 0.57

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 415 lb uplift at joint 8 and 469 lb uplift at joint 2.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061030

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T14	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jul 31 13:12:02 2006 Page 1

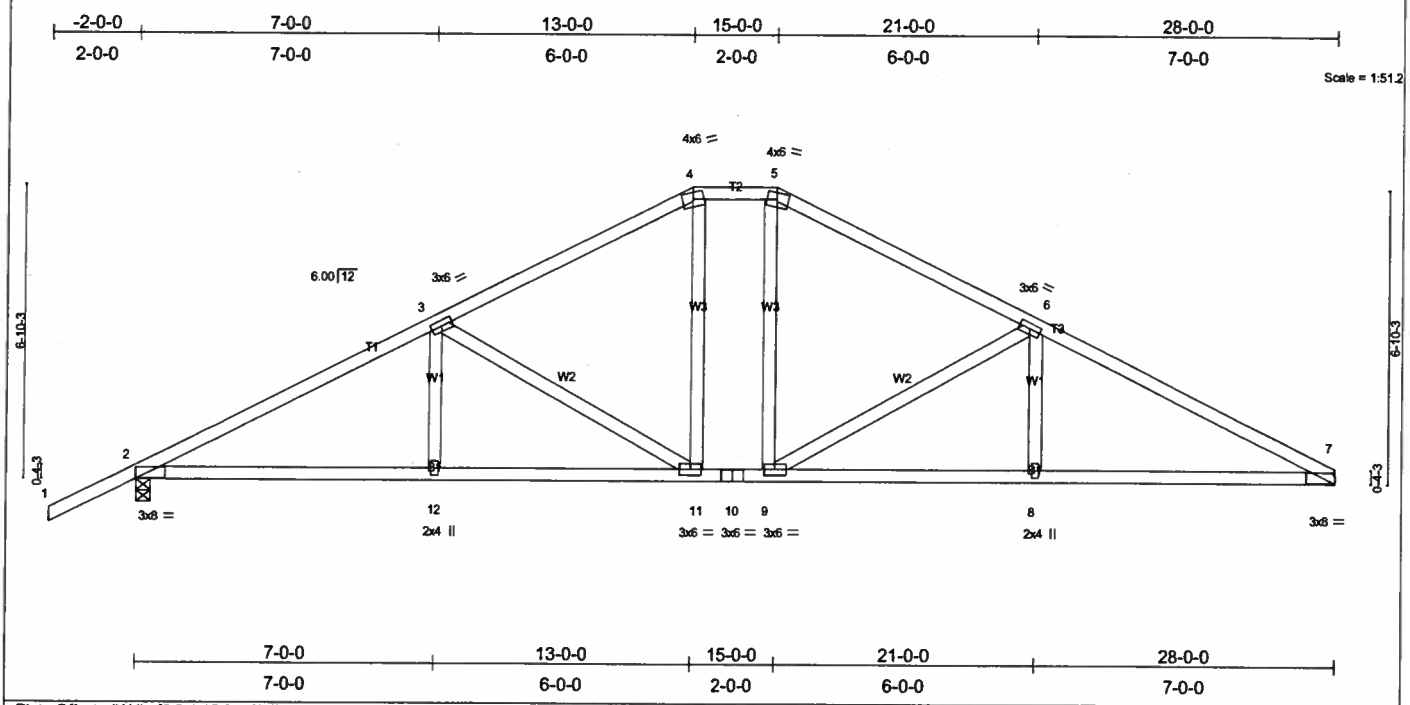


Plate Offsets (X,Y): [2:0-4-12,0-1-8], [7:0-4-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.15	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.23	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.62	Horz(TL)	0.08	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 140 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-2-9 oc bracing.

REACTIONS (lb/size) 7=1162/Mechanical, 2=1288/0-4-0
Max Horz 2=154(load case 5)
Max Uplift7=-372(load case 6), 2=-501(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2066/841, 3-4=-1462/702, 4-5=-1244/691, 5-6=-1464/705, 6-7=-2091/891
BOT CHORD 2-12=-643/1764, 11-12=-643/1764, 10-11=-358/1244, 9-10=-358/1244, 8-9=-698/1812, 7-8=-698/1812
WEBS 3-12=0/242, 3-11=-651/333, 4-11=-151/417, 5-9=-166/430, 6-9=-704/396, 6-8=0/261

JOINT STRESS INDEX
2 = 0.78, 3 = 0.41, 4 = 0.56, 5 = 0.56, 6 = 0.41, 7 = 0.76, 8 = 0.34, 9 = 0.35, 10 = 0.50, 11 = 0.35 and 12 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 372 lb uplift at joint 7 and 501 lb uplift at joint 2.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061031

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T15	COMMON	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jul 31 13:12:02 2006 Page 1

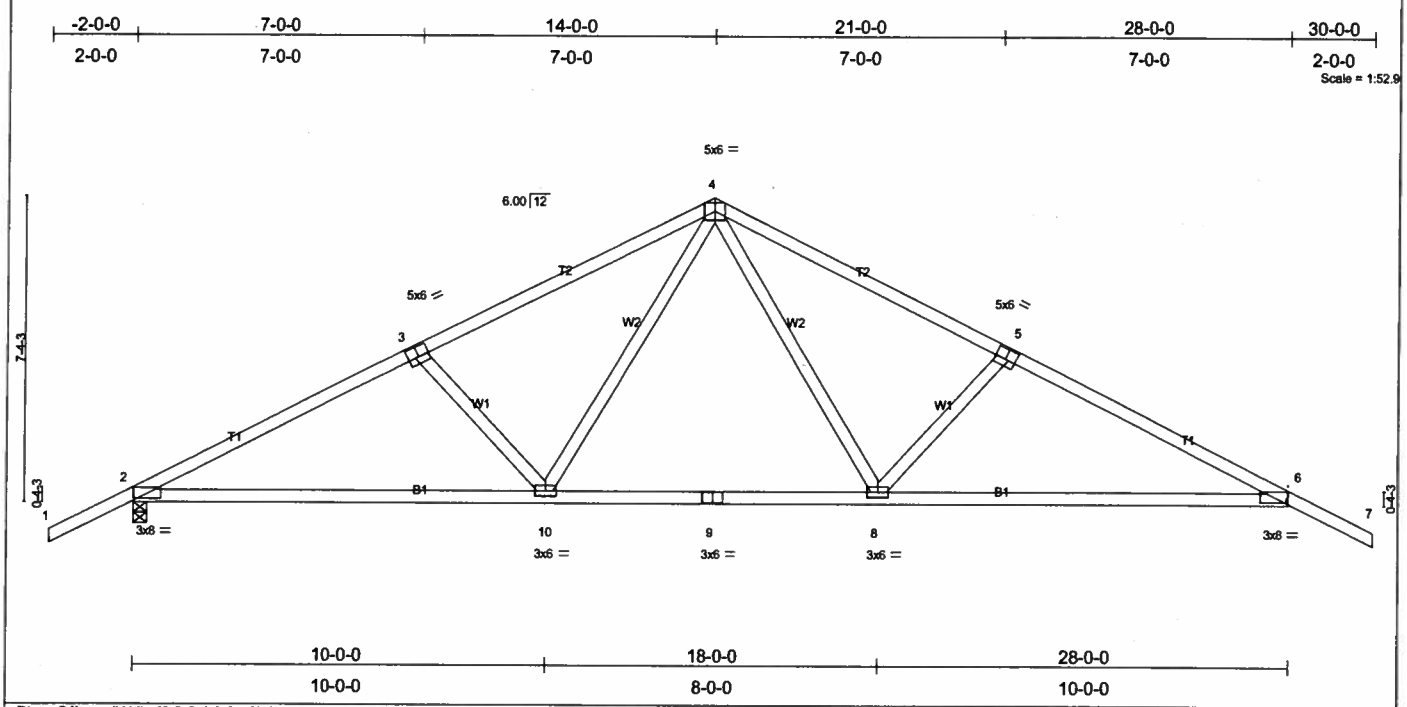


Plate Offsets (X,Y): [2:0-8-4,0-0-10], [3:0-3-0,0-3-0], [5:0-3-0,0-3-0], [6:0-8-4,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.53	Vert(LL)	-0.29	6-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.50	6-8	>670	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.27	Horz(TL)	0.07	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 132 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-2-5 oc bracing.

REACTIONS (lb/size) 2=1284/0-4-0, 6=1277/Mechanical
Max Horz 2=136(load case 5)
Max Uplift2=-505(load case 5), 6=-499(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1970/857, 3-4=-1729/798, 4-5=-1740/804, 5-6=-1987/867, 6-7=0/47
BOT CHORD 2-10=-581/1704, 9-10=-250/1160, 8-9=-250/1160, 6-8=-592/1726
WEBS 3-10=-378/333, 4-10=-221/641, 4-8=-231/659, 5-8=-393/342

JOINT STRESS INDEX
2 = 0.80, 3 = 0.69, 4 = 0.70, 5 = 0.69, 6 = 0.80, 8 = 0.53, 9 = 0.43 and 10 = 0.53

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 505 lb uplift at joint 2 and 499 lb uplift at joint 6.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061032

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T17	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jul 31 13:12:03 2006 Page 1

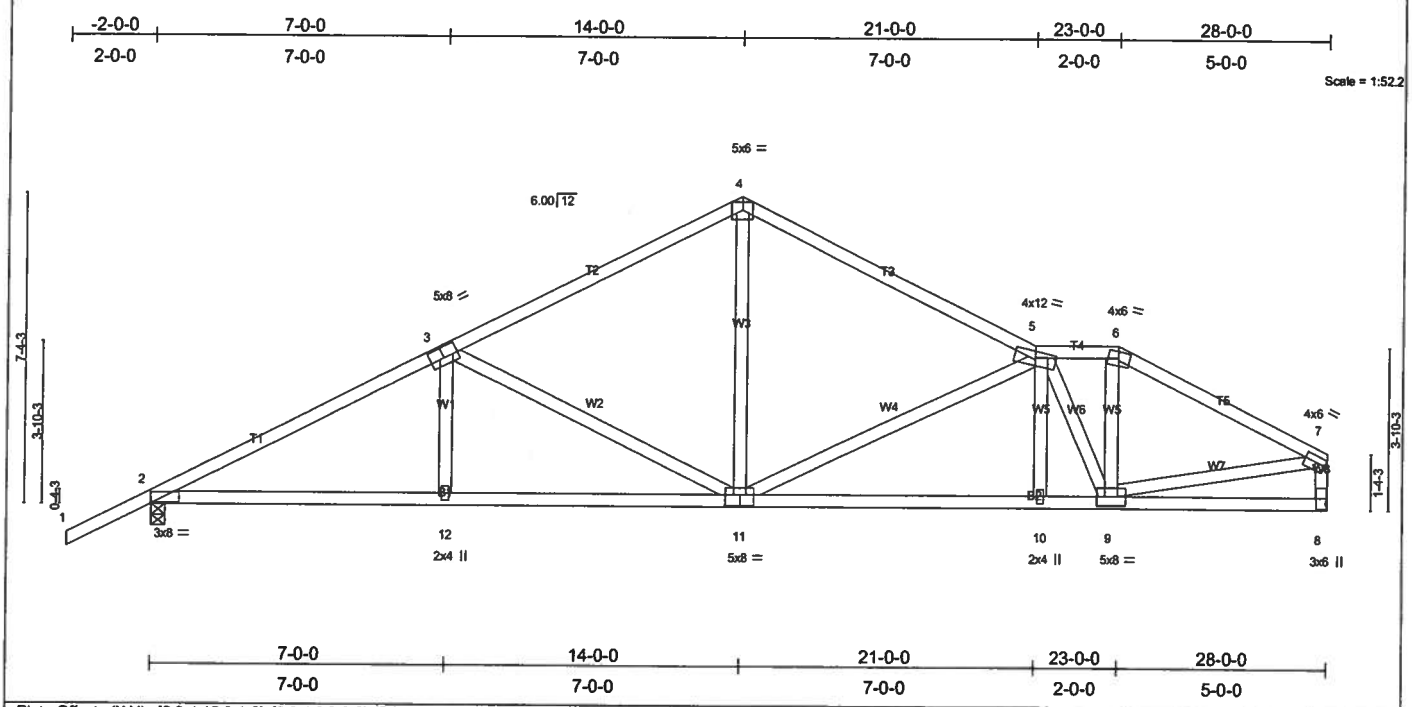


Plate Offsets (X,Y): [2:0-4-12,0-1-8], [3:0-4-0,0-3-0], [7:Edge,0-1-12], [11:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.14	10-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.23	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 153 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-6-1 oc bracing.

REACTIONS (lb/size) 2=1284/0-4-0, 8=1158/Mechanical
Max Horz 2=196(load case 5)
Max Uplift2=-505(load case 5), 8=-377(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2063/845, 3-4=-1413/679, 4-5=-1406/678, 5-6=-1356/660, 6-7=-1558/674, 7-8=-1067/502
BOT CHORD 2-12=-706/1765, 11-12=-705/1772, 10-11=-707/1740, 9-10=-706/1747, 8-9=-111/207
WEBS 3-12=0/220, 3-11=-678/353, 4-11=-278/786, 5-11=-651/359, 5-10=0/162, 5-9=-842/366, 6-9=-212/544, 7-9=-427/1154

JOINT STRESS INDEX
2 = 0.78, 3 = 0.47, 4 = 0.71, 5 = 0.99, 6 = 0.36, 7 = 0.77, 8 = 0.36, 9 = 0.55, 10 = 0.34, 11 = 0.49 and 12 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 505 lb uplift at joint 2 and 377 lb uplift at joint 8.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061033

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T18	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jul 31 13:12:04 2006 Page 1

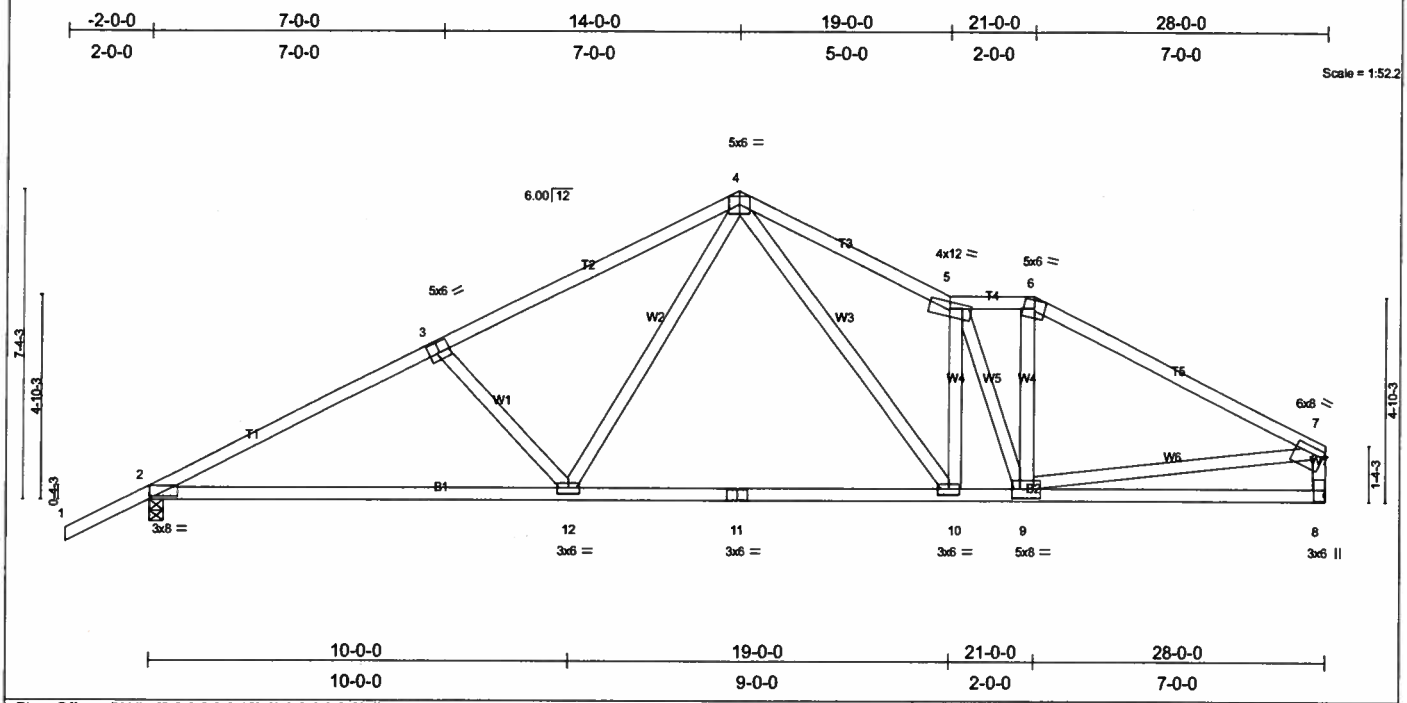


Plate Offsets (X,Y): [2:0-8-0,0-0-10], [3:0-3-0,0-3-0], [7:Edge,0-1-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.78	Vert(LL)	-0.24	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.41	2-12	>808	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.46	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 153 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-5-3 oc bracing.

REACTIONS (lb/size) 2=1284/0-4-0, 8=1158/Mechanical
Max Horz 2=196(load case 5)
Max Uplift2=-505(load case 5), 8=-377(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1984/863, 3-4=-1742/802, 4-5=-1795/917, 5-6=-1375/700, 6-7=-1612/698, 7-8=-1039/509
BOT CHORD 2-12=-721/1716, 11-12=-386/1149, 10-11=-386/1149, 9-10=-580/1566, 8-9=-188/348
WEBS 3-12=-370/341, 4-12=-219/679, 4-10=-345/776, 5-10=-432/305, 5-9=-505/146, 6-9=-48/376, 7-9=-339/1028

JOINT STRESS INDEX
2 = 0.74, 3 = 0.70, 4 = 0.61, 5 = 0.50, 6 = 0.60, 7 = 0.73, 8 = 0.56, 9 = 0.47, 10 = 0.57, 11 = 0.59 and 12 = 0.54

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 505 lb uplift at joint 2 and 377 lb uplift at joint 8.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061034

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T19	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jul 31 13:12:04 2006 Page 1

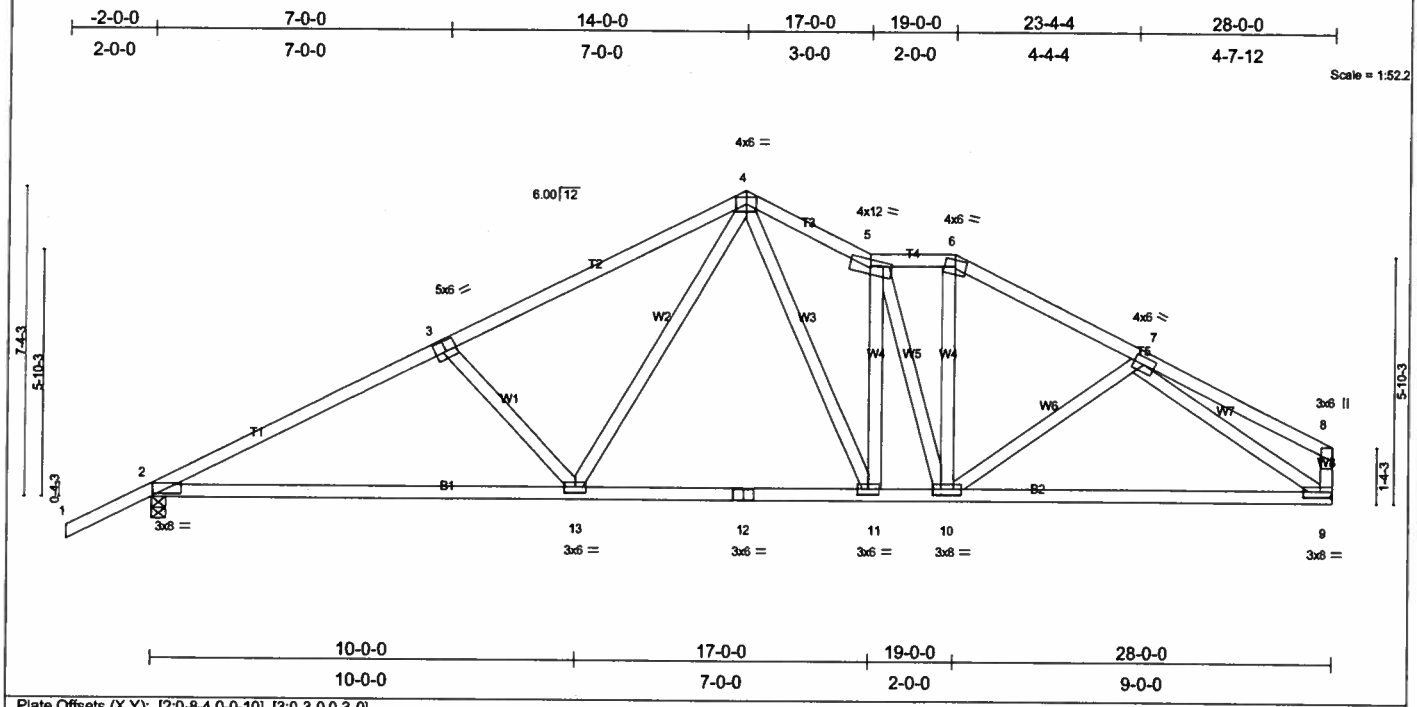


Plate Offsets (X,Y): [2:0-8-4,0-0-10], [3:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.72	Vert(LL)	-0.26	2-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.64	Vert(TL)	-0.44	2-13	>757	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.66	Horz(TL)	0.06	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 162 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-5-1 oc bracing.

REACTIONS (lb/size) 2=1284/0-4-0, 9=1158/Mechanical
Max Horz 2=196(load case 5)
Max Uplift2=-505(load case 5), 9=-377(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1979/863, 3-4=-1736/801, 4-5=-1545/809, 5-6=-1307/675, 6-7=-1502/697, 7-8=-397/126, 8-9=-283/144
BOT CHORD 2-13=-722/1713, 12-13=-382/1152, 11-12=-382/1152, 10-11=-482/1371, 9-10=-547/1275
WEBS 3-13=-370/343, 4-13=-224/655, 4-11=-262/638, 5-11=-539/243, 5-10=-202/104, 6-10=-112/393, 7-10=-23/132, 7-9=-1218/622

JOINT STRESS INDEX
2 = 0.79, 3 = 0.71, 4 = 0.81, 5 = 0.55, 6 = 0.46, 7 = 0.35, 8 = 0.41, 9 = 0.73, 10 = 0.68, 11 = 0.56, 12 = 0.51 and 13 = 0.52

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 505 lb uplift at joint 2 and 377 lb uplift at joint 9.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061035

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T20	SPECIAL	1	1	Job Reference (optional)

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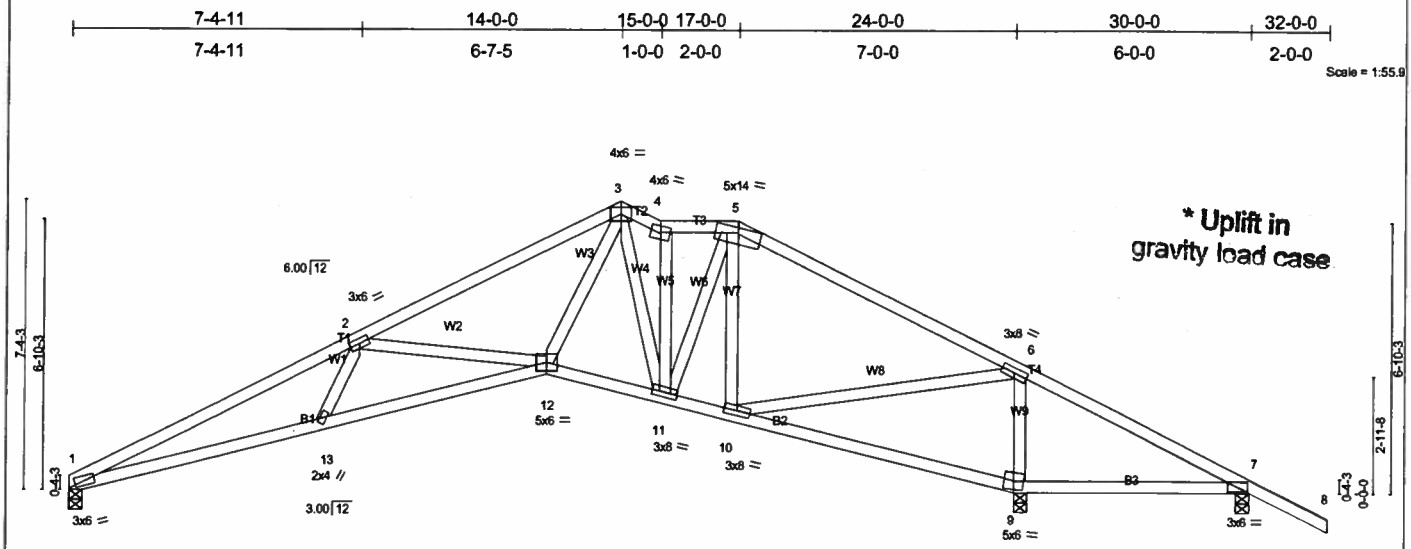


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	-0.19	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.30	12-13	>944	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.56	Horz(TL)	0.17	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 157 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-6-14 oc bracing.

REACTIONS (lb/size) 1=846/0-4-0, 9=2013/0-4-0, 7=-250/0-4-0
Max Horz 1=-160(load case 6)
Max Uplift1=-287(load case 5), 9=-634(load case 5), 7=-379(load case 9)
Max Grav 1=846(load case 1), 9=2013(load case 1), 7=19(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2593/1005, 2-3=-1530/565, 3-4=-734/363, 4-5=-734/405, 5-6=-721/334, 6-7=-365/1302, 7-8=0/47
BOT CHORD 1-13=-753/2325, 12-13=-754/2200, 11-12=-105/879, 10-11=-2/557, 9-10=-1219/512, 7-9=-1081/455
WEBS 2-13=0/270, 2-12=-847/587, 3-12=-274/1110, 3-11=-485/222, 4-11=-134/7, 5-11=-151/493, 5-10=-519/264, 6-10=-459/1742, 6-9=-1503/675

JOINT STRESS INDEX
1 = 0.80, 2 = 0.41, 3 = 0.93, 4 = 0.55, 5 = 0.92, 6 = 0.79, 7 = 0.67, 9 = 0.83, 10 = 0.76, 11 = 0.68, 12 = 0.80 and 13 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 1, 634 lb uplift at joint 9 and 379 lb uplift at joint 7.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061036

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T21	SPECIAL	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jul 31 13:12:06 2006 Page 1

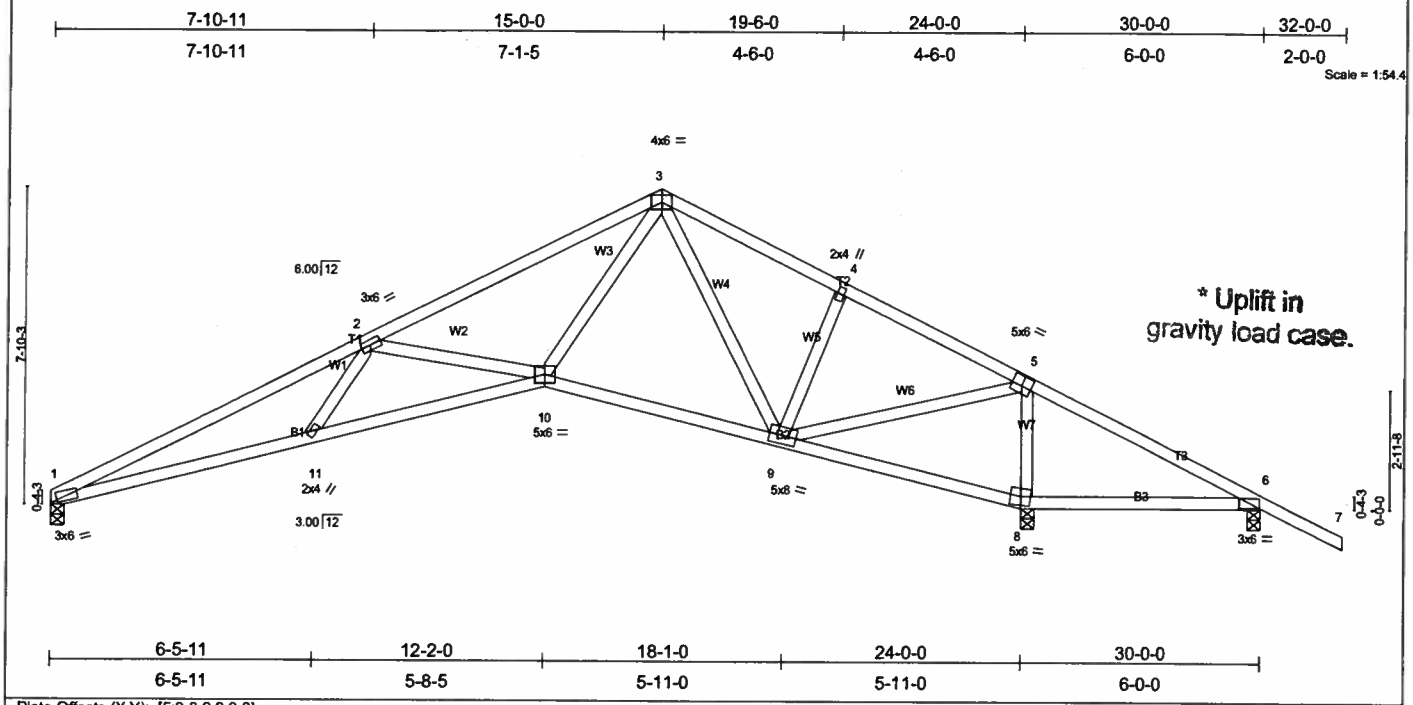


Plate Offsets (X,Y): [5:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.18	1-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.29	1-11	>977	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.16	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 146 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-3-7 oc bracing.

REACTIONS (lb/size) 1=832/0-4-0, 8=2081/0-4-0, 6=-305/0-4-0
Max Horz 1=-167(load case 6)
Max Uplift1=-285(load case 5), 8=-676(load case 5), 6=-443(load case 9)
Max Grav 1=832(load case 1), 8=2081(load case 1), 6=44(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2523/971, 2-3=-1469/555, 3-4=-416/316, 4-5=-512/285, 5-6=-432/1453, 6-7=0/47
BOT CHORD 1-11=-736/2258, 10-11=-735/2083, 9-10=-39/649, 8-9=-1317/566, 6-8=-1221/525
WEBS 2-11=0/304, 2-10=-808/588, 3-10=-318/1182, 3-9=-614/214, 4-9=-204/183, 5-9=-505/1704, 5-8=-1564/682

JOINT STRESS INDEX
1 = 0.84, 2 = 0.41, 3 = 0.95, 4 = 0.34, 5 = 0.74, 6 = 0.54, 8 = 0.86, 9 = 0.84, 10 = 0.82 and 11 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 285 lb uplift at joint 1, 676 lb uplift at joint 8 and 443 lb uplift at joint 6.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

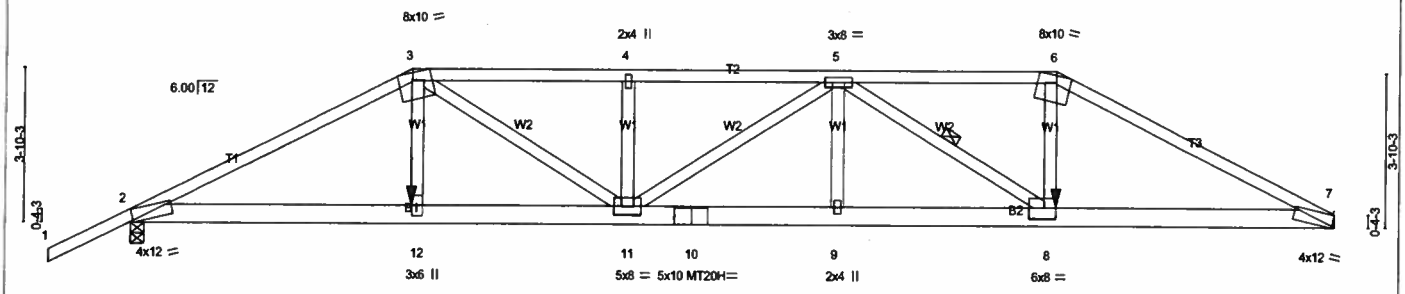
Dwg.#0731061037

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T22	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jul 31 13:12:06 2006 Page 1

-2-0-0	7-0-0	12-4-9	17-7-7	23-0-0	30-0-0
2-0-0	7-0-0	5-4-9	5-2-13	5-4-9	7-0-0

Scale = 1:54.5



7-0-0	12-4-9	17-7-7	23-0-0	30-0-0
7-0-0	5-4-9	5-2-13	5-4-9	7-0-0

Plate Offsets (X,Y): [2:0-0-13,Edge], [3:0-4-3,Edge], [6:0-4-3,Edge], [7:0-0-13,Edge], [8:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.92	Vert(LL)	-0.40	9-11	>892	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.64	9-11	>557	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.61	Horz(TL)	0.14	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 165 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-9-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-1-10 oc bracing.
WEBS 1 Row at midpt 5-8

REACTIONS (lb/size) 7=2594/Mechanical, 2=2729/0-4-0
Max Horz 2=116(load case 4)
Max Uplift7=-1042(load case 5), 2=-1175(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-5276/2219, 3-4=-6186/2713, 4-5=-6186/2714, 5-6=-4773/2068, 6-7=-5325/2231
BOT CHORD 2-12=-1974/4637, 11-12=-1984/4673, 10-11=-2673/6235, 9-10=-2673/6235, 8-9=-2673/6235, 7-8=-1942/4696
WEBS 3-12=-227/847, 3-11=-897/1897, 4-11=-649/537, 5-11=-98/50, 5-9=0/377, 5-8=-1871/883, 6-8=-686/1843

JOINT STRESS INDEX
2 = 0.79, 3 = 0.99, 4 = 0.34, 5 = 0.89, 6 = 0.99, 7 = 0.78, 8 = 0.51, 9 = 0.34, 10 = 0.87, 11 = 0.88 and 12 = 0.28

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1042 lb uplift at joint 7 and 1175 lb uplift at joint 2.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 23-0-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-120(F=-66), 6-7=-54, 2-12=-30, 8-12=-67(F=-37), 7-8=-30
Concentrated Loads (lb)
Vert: 12=-539(F) 8=-539(F)

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061038

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T23	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Mon Jul 31 13:12:07 2006 Page 1

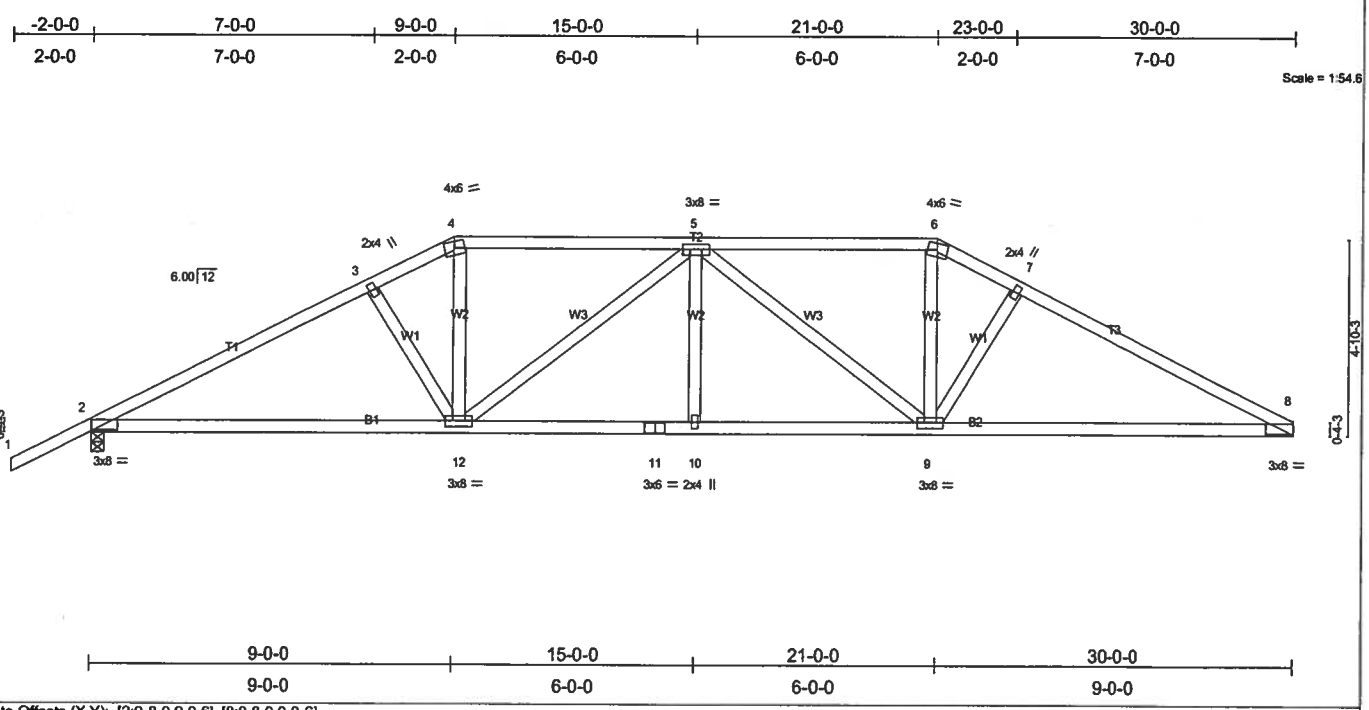


Plate Offsets (X,Y): [2:0-8-0,0-0-6], [8:0-8-0,0-0-6]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.58	Vert(LL)	-0.27	8-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.45	8-9	>793	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.45	Horz(TL)	0.10	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 147 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-0-10 oc bracing.

REACTIONS (lb/size) 8=1246/Mechanical, 2=1372/0-4-0
Max Horz 2=126(load case 5)
Max Uplift8=-363(load case 6), 2=-491(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2185/881, 3-4=-1995/868, 4-5=-1765/793, 5-6=-1791/821, 6-7=-2031/908, 7-8=-2210/928
BOT CHORD 2-12=-674/1873, 11-12=-717/2072, 10-11=-717/2072, 9-10=-717/2072, 8-9=-729/1923
WEBS 3-12=-248/226, 4-12=-266/715, 5-12=-469/222, 5-10=0/115, 5-9=-448/210, 6-9=-307/747, 7-9=-293/279

JOINT STRESS INDEX
2 = 0.71, 3 = 0.34, 4 = 0.55, 5 = 0.57, 6 = 0.55, 7 = 0.34, 8 = 0.69, 9 = 0.59, 10 = 0.34, 11 = 0.74 and 12 = 0.59

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 363 lb uplift at joint 8 and 491 lb uplift at joint 2.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061039

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T24	HIP	1	1	Job Reference (optional)

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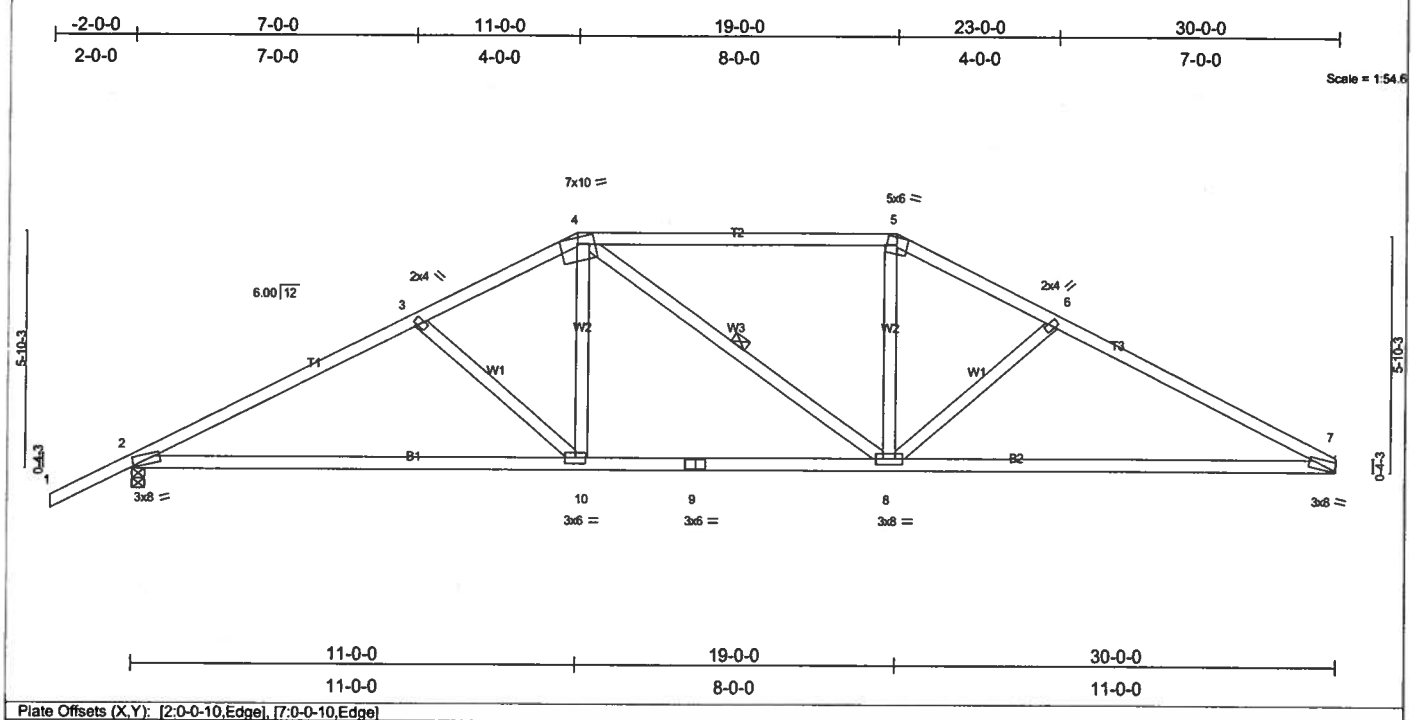

Plate Offsets (X,Y): [2:0-0-10,Edge], [7:0-0-10,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	-0.48	7-8	>740	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.81	7-8	>439	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.17	Horz(TL)	0.09	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 141 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-11-10 oc bracing.
WEBS 1 Row at midpt 4-8

REACTIONS (lb/size) 7=1246/Mechanical, 2=1372/0-4-0
Max Horz 2=140(load case 5)
Max Uplift7=-380(load case 6), 2=-508(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2108/898, 3-4=-1849/819, 4-5=-1640/799, 5-6=-1871/843, 6-7=-2146/939
BOT CHORD 2-10=-687/1824, 9-10=-501/1624, 8-9=-501/1624, 7-8=-735/1869
WEBS 3-10=-283/249, 4-10=-103/474, 4-8=-125/154, 5-8=-140/498, 6-8=-321/293

JOINT STRESS INDEX
2 = 0.87, 3 = 0.34, 4 = 0.66, 5 = 0.66, 6 = 0.34, 7 = 0.82, 8 = 0.57, 9 = 0.63 and 10 = 0.36

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 380 lb uplift at joint 7 and 508 lb uplift at joint 2.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0731061040

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 32 TW
L204776	T25	SPECIAL	1	1	Job Reference (optional)

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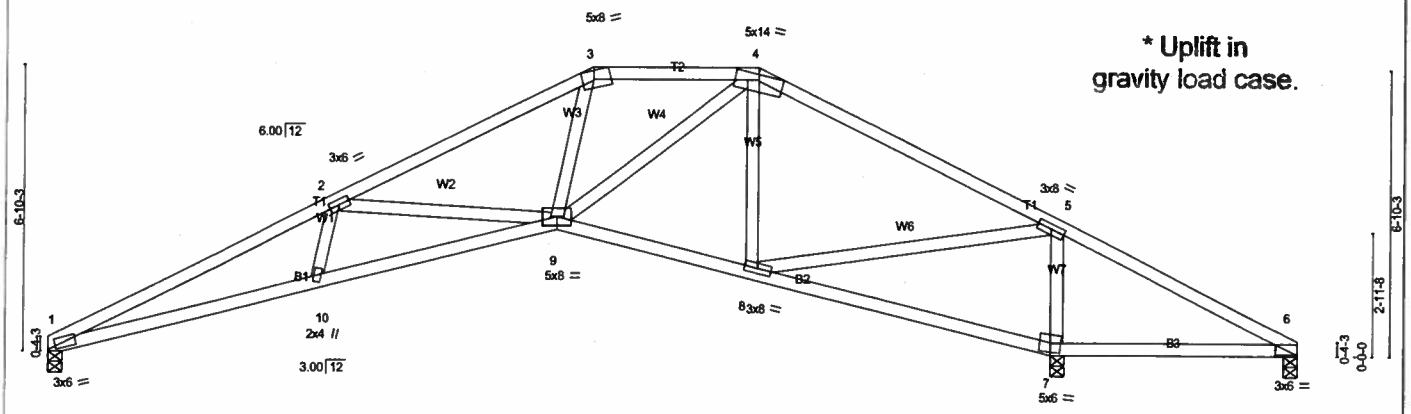
7-0-0 | 13-0-0 | 17-0-0 | 24-0-0 | 30-0-0
7-0-0 | 6-0-0 | 4-0-0 | 7-0-0 | 6-0-0

Scale = 1:52.7



6-5-11 | 12-2-0 | 17-0-0 | 24-0-0 | 30-0-0
6-5-11 | 5-8-5 | 4-10-0 | 7-0-0 | 6-0-0

Plate Offsets (X,Y): [3:0-3-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.68	Vert(LL)	-0.19	9-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	0.08	6-7	>845	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.55	Horz(TL)	0.16	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 140 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-5-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-7-0 oc bracing.

REACTIONS (lb/size) 1=847/0-4-0, 6=-384/0-4-0, 7=2030/0-4-0
Max Horz 1=96(load case 4)
Max Uplift1=-280(load case 5), 6=-447(load case 9), 7=-648(load case 5)
Max Grav 1=847(load case 1), 6=77(load case 5), 7=2030(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2611/1047, 2-3=-1549/587, 3-4=-1216/565, 4-5=-719/310, 5-6=-450/1284
BOT CHORD 1-10=-867/2344, 9-10=-869/2281, 8-9=-46/576, 7-8=-1197/508, 6-7=-1059/447
WEBS 2-10=0/230, 2-9=-886/590, 3-9=-86/428, 4-9=-285/887, 4-8=-520/266, 5-8=-507/1715, 5-7=-1507/741

JOINT STRESS INDEX
1 = 0.80, 2 = 0.44, 3 = 0.36, 4 = 0.84, 5 = 0.78, 6 = 0.46, 7 = 0.83, 8 = 0.75, 9 = 0.72 and 10 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 1, 447 lb uplift at joint 6 and 648 lb uplift at joint 7.

LOAD CASE(S) Standard

JULY 31, 2006 TRUSS DESIGN ENGINEER:
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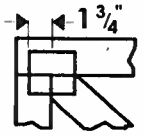
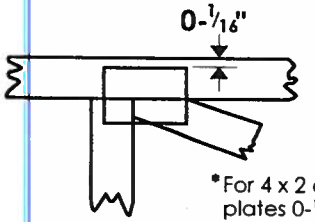
Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

*** Plate location details available in MiTek 20/20 software or upon request.**

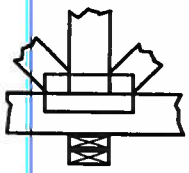
PLATE SIZE

4 x 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

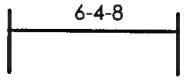


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

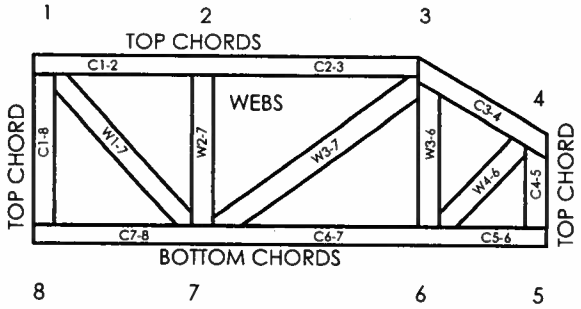
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.

COLUMBIA COUNTY BUILDING DEPARTMENT

Revised 10-01-05

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE
EFFECTIVE OCTOBER 1, 2005

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ——— 100 MPH
2. ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE ———110 MPH
3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Applicant	Plans Examiner	
☑	☐	All drawings must be clear, concise and drawn to scale ("Optional " details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.
☑	☐	Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.
☑	☐	**Site Plan including:** a) Dimensions of lot b) Dimensions of building set backs c) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements. d) Provide a full legal description of property.
☑	☐	**Wind-load Engineering Summary, calculations and any details required** Plans or specifications must state compliance with FBC Section 1609. *The following information must be shown as per section 1603.1.4 FBC* a. Basic wind speed (3-second gust), miles per hour (km/hr). b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7. c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated. d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient. e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.
☐	☐	**Elevations including:** a) All sides
☑	☐	b) Roof pitch
☐	☐	c) Overhang dimensions and detail with attic ventilation

☑ ☐ d) Location, size and height above roof of chimneys.
☐ N/A ☐ e) Location and size of skylights
☑ ☐ f) Building height
☑ ☐ e) Number of stories

Floor Plan including:

☑ ☐ a) Rooms labeled and dimensioned.
☑ ☐ b) Shear walls identified.
☑ ☐ c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (see attach forms).
☑ ☐ d) Show safety glazing of glass, where required by code.
☑ ☐ e) Identify egress windows in bedrooms, and size.
☐ N/A ☐ f) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, (Please circle applicable type).
☐ N/A ☐ g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
☑ ☐ h) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan including:

☑ ☐ a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
☑ ☐ b) All posts and/or column footing including size and reinforcing
☑ ☐ c) Any special support required by soil analysis such as piling
☐ N/A ☐ d) Location of any vertical steel.

Roof System:

☑ ☐ a) Truss package including:
1. Truss layout and truss details signed and sealed by Fl. Pro. Eng.
2. Roof assembly (FBC 106.1.1.2)Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

☑ ☐ b) Conventional Framing Layout including:
1. Rafter size, species and spacing
2. Attachment to wall and uplift
3. Ridge beam sized and valley framing and support details
4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

☑ ☐ a) Masonry wall
1. All materials making up wall
2. Block size and mortar type with size and spacing of reinforcement
3. Lintel, tie-beam sizes and reinforcement
4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
5. *All required* connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.
6. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
7. Fire resistant construction (if required)
8. Fireproofing requirements
9. Shoe type of termite treatment (termiticide or alternative method)
10. Slab on grade
    a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
    b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
11. Indicate where pressure treated wood will be placed
12. Provide insulation R value for the following:

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)

☑ ☐ b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers) shall be designed by a Windload engineer using the engineered roof truss plans.
7. Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termiticide or alternative method)
11. Slab on grade
    a. Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed
    b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
    a. Attic space
    b. Exterior wall cavity
    c. Crawl space (if applicable)

☐ N/A ☐ c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

☑ N/A ☐ a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer

☑ ☐ b) Floor joist size and spacing

☐ ☐ c) Girder size and spacing

☐ ☐ d) Attachment of joist to girder

☐ ☐ e) Wind load requirements where applicable

☑ ☐ **Plumbing Fixture layout**

Electrical layout including:

☑ ☐ a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified

☑ ☐ b) Ceiling fans

☑ ☐ c) Smoke detectors

☑ ☐ d) Service panel and sub-panel size and location(s)

☑ ☐ e) Meter location with type of service entrance (overhead or underground)

☑ ☐ f) Appliances and HVAC equipment

☑ ☐ g) Arc Fault Circuits (AFCI) in bedrooms

☑ ☐ h) Exhaust fans in bathroom

HVAC information

☑ ☐ a) Energy Calculations (dimensions shall match plans)

☑ ☐ b) Manual J sizing equipment or equivalent computation

☑ ☐ c) Gas System Type (LP or Natural) Location and BTU demand of equipment

☑ ☐ **Disclosure Statement for Owner Builders**

☑ ☐ ***Notice Of Commencement Required Before Any Inspections Will Be Done

☑ ☐ **Private Potable Water**

a) Size of pump motor
b) Size of pressure tank
c) Cycle stop valve if used

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

1. **Building Permit Application:** A current Building Permit Application form is to be completed and submitted for all residential projects.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 **(Toilet facilities shall be provided for construction workers)**

4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**
A development permit will also be required. Development permit cost is $50.00

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit ($25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver ($50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial. **If the project is to be located on a F.D.O.T. maintained road, than an F.D.O.T. access permit is required.**

7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE – TIME WILL NOT ALLOW THIS –PLEASE DO NOT ASK

PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING			
B. SLIDING	See Spec in Package		
C. SECTIONAL/ROLL UP			
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG			
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING			
B. SOFFITS			
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES			
B. NON-STRUCT METAL			
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS			
B. WOOD ANCHORS			
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			
A.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

_____

_____

APPLICANT SIGNATURE _____ DATE _____

Columbia County 9-1-1 Addressing / GIS Department
P.O. Box 1787, Lake City, FL 32056
Telephone: (386) 758-1125 * Fax: (386) 758-1365 * E-mail: ron_croft@columbiacountyfla.com

9-1-1 Address Request Form

NOTE: ADDRESS ASSIGNMENT MAY REQUIRE UP TO 10 WORKING DAYS. IF THE ADDRESSING DEPARTMENT NEEDS TO CONDUCT ON SITE GPS LOCATION IDENTIFICATION, ADDITIONAL TIME MAY BE REQUIRED.

Date of Request: _____

Requester Last Name: _____

First Name: _____

Contact Telephone Number: _____

(Cell Phone Number if Provided): _____

Requested for Self: _____ or Requested for Company: _____
(check one)
If Address is Requested by a Company, Provide Name of Requesting Company:

_____

Parcel Identification Number: ____ - ____ - ____ - _____

If in Subdivision, Provide Name Of Subdivision:

_____

Phase or Unit Number (if any): _____ Block Number (if any): _____

Lot Number: _____

Attach Site Plan or you may use back of Request Form for Site Plan:

Requirements for Site Plan Are Listed on Back of Request From:
(NOTE: Site Plan Does NOT have to be a survey or to scale; FURTHER a Environmental Health Dept. Site Plan showing only a 210 by 210 cutout of a property will NOT suffice for Addressing Requirements.)

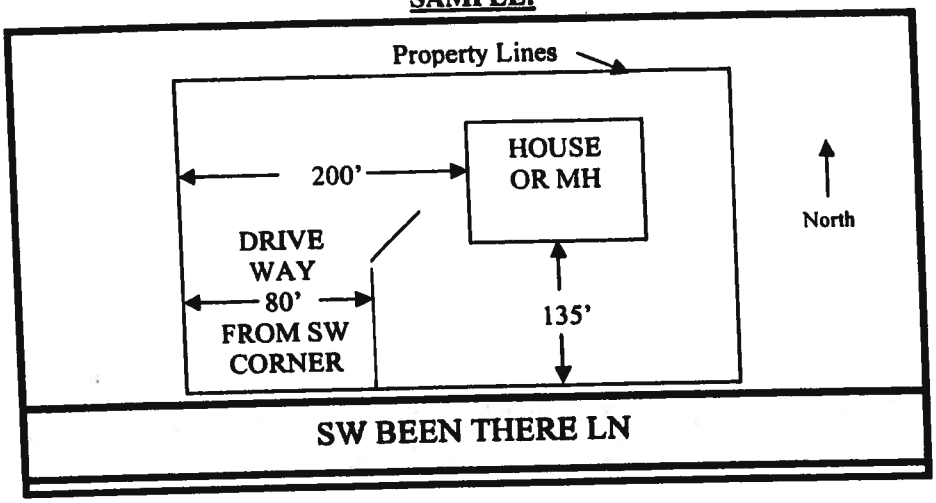
Addressing / GIS Department Use Only:

Date Received: _____ Date Assigned: _____

ID Number: _____

1. A PLAT, PLAN, OR DRAWING SHOWING THE PROPERTY LINES OF THE PARCEL.
2. LOCATION OF PLANNED RESIDENT OR BUSINESS STRUCTURE ON THE PROPERTY WITH DISTANCES FROM AT LEAST TWO OF THE PROPERTY LINES TO THE STRUCTURE (SEE SAMPLE BELOW).
3. LOCATION OF THE ACCESS POINT (DRIVEWAY, ETC.) ON THE ROADWAY FROM WHICH LOCATION IS TO BE ADDRESSED WITH A DISTANCE FROM A PARALLEL PROPERTY LINE AND OR PROPERTY CORNER (SEE SAMPLE BELOW).
4. TRAVEL OF THE DRIVEWAY FROM THE ACCESS POINT TO THE STRUCTURE (SEE SAMPLE BELOW).

SAMPLE:



SITE PLAN BOX:

located on plans

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 34-6S-16-04056-130 **Building permit No.** 000025616

Use Classification SFD,UTILITY **Fire:** 44.94

Permit Holder JOHN NORRIS **Waste:**

Owner of Building IMAGE DEVELOPMENT GROUP, INC **Total:** 44.94

Location: 320 SW GREENWOOD TERR, FT. WHITE, FL

Date: 03/12/2008

Harry Dicks

Building Inspector

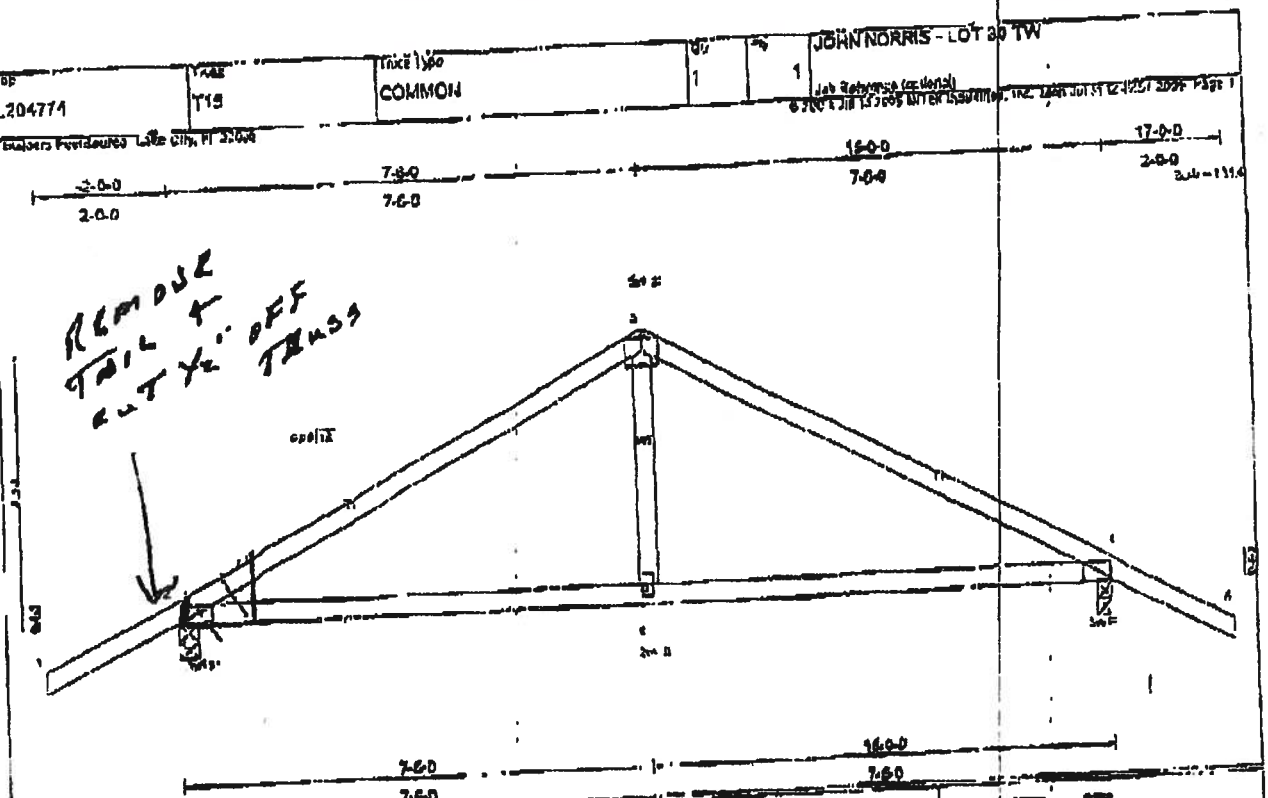
POST IN A CONSPICUOUS PLACE
(Business Places Only)

#25616

REMOVE TAIL & CUT ½" OFF THIS

JOB L204774 | TRUSS T15 | TRUSS TYPE COMMON | QTY 1 | PLY 1 | JOHN NORRIS - LOT 30 TW

Builders FirstSource, Lake City, FL 32055



LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d
TCLL 20.0	Plates Increase 1.25	TC 0.31	Vert(LL)	-0.16	4-8	>999	240
TCDL 7.0	Lumber Increase 1.25	BC 0.44	Vert(TL)	-0.15	4-8	>999	180
BCLL 10.0	Rep Stress Incr YES	WB 0.08	Horz(TL)	0.02	4	n/a	n/a
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)					

PLATES GRIP
MT20 244/190

Weight: 59 lb

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 2=704/0-3-8, 4=704/0-3-8
Max Horz 2=81 (load case 6)
Max Uplift 2=-67 (load case 5), 4=-328 (load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-852/328, 3-4=-852/328, 4-5=0/47
BOT CHORD 2-6=-124/696, 4-6=-124/696
WEBS 3-6=0/272

JOINT STRESS INDEX
2 = 0.27, 3 = 0.79, 4 = 0.57, and 6 = 0.20

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60 This truss is designed for C-C for members and forces, and for MWFRS for reactions specified
3) All bearings are assumed to be SYP No.2 @ grade capacity of 565.00 psi.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 328 lb uplift at joint 2 and 328 lb uplift at joint 4.

LOAD CASE(S) Standard

Architectural Services & Engineering, Inc
CA 7882
24710 SR 54 Lutz, Fl 33559

Robert W. Wall PE 46021

ROBERT W. WALL LICENSE No. 46021 STATE OF FLORIDA PROFESSIONAL ENGINEER

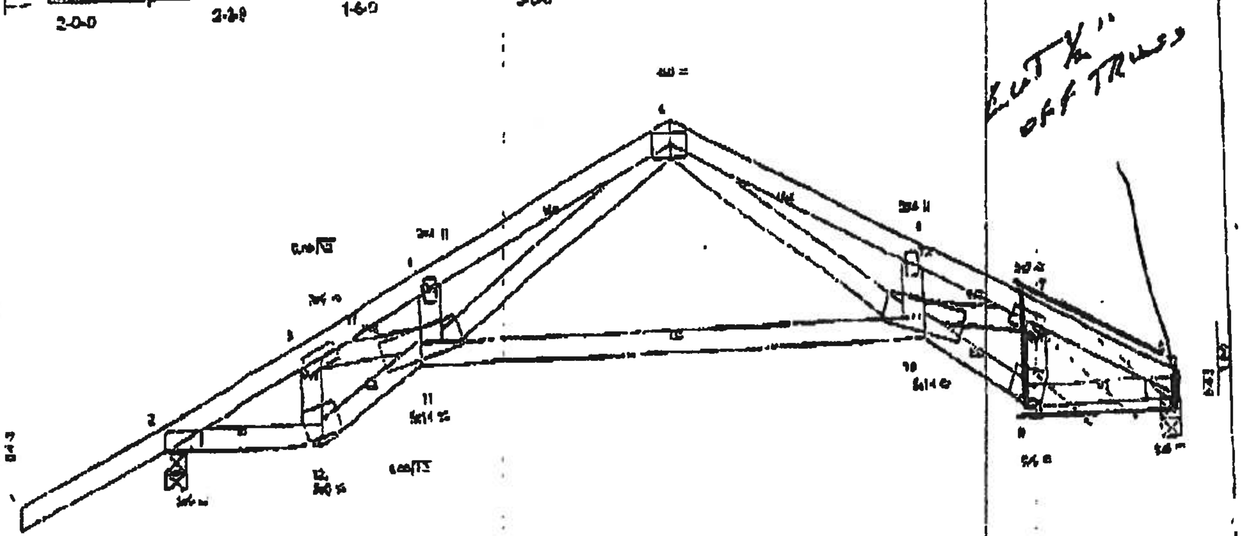
Modification needed: ½" CUT OFF THE RIGHT SIDE AS SHOWN ABOVE.

Solution : At one face ½" plywood or o.s.b. one face 12"x12" cut to fit with 8d [illegible] o.c. at each member.

All trusses must be in an un-deflected state. No loading or braced to no deflection. If conditions change from above notify truss manufacturer. Do not damage existing plates unless otherwise noted.

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - LOT 30 TW
L304774	T1ST	SPECIAL	3	1	Job Reference (optional)

2-0-0 | 2-3-8 | 3-9-8 | 7-6-0 | 11-3-8 | 12-9-8 | 15-0-0
2-0-0 | 2-3-8 | 1-6-0 | 3-8-8 | 3-9-8 | 1-6-0 | 2-3-8



2-3-8 | 3-9-8 | 11-3-8 | 12-9-8 | 15-0-0
2-3-8 | 1-6-0 | 7-6-0 | 1-6-0 | 2-3-8

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.16	10-11	>929	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.28	10-11	>678	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.32	Horz(TL)	0.09	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 75 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-4-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-6-11 oc bracing.

REACTIONS (lb/size) 9=822/0-3-8, 2=742/0-3-8
Max Horz 2=115(load case 5)
Max Uplift 9=-197(load case 5), 2=-332(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1091/655, 3-4=-1930/872, 4-5=-1866/761, 5-6=-1520/842, 6-7=-1549/772, 7-8=-1030/502
BOT CHORD 2-12=-530/939, 11-12=-243/617, 10-11=-288/737, 9-10=-426/1056, 8-9=-403/912
WEBS 2-12=-893/166, 3-11=-314/880, 4-11=-55/149, 5-11=-300/1051, 5-10=-104/1157, 6-10=-60/137, 7-10=-328/637, 7-8=-915/204

JOINT STRESS INDEX
2 = 0.54, 3 = 0.45, 4 = 0.96, 5 = 0.84, 6 = 0.09, 7 = 0.43, 8 = 0.54, 9 = 0.32, 10 = 0.45, 11 = 0.45 and 12 = 0.32

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=15ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 197 lb uplift at joint 9 and 332 lb uplift at joint 2.

LOAD CASE(S) Standard

Architectural Services & Engineering, Inc
CA 7882
24710 SR 54 Lutz, Fl 33559

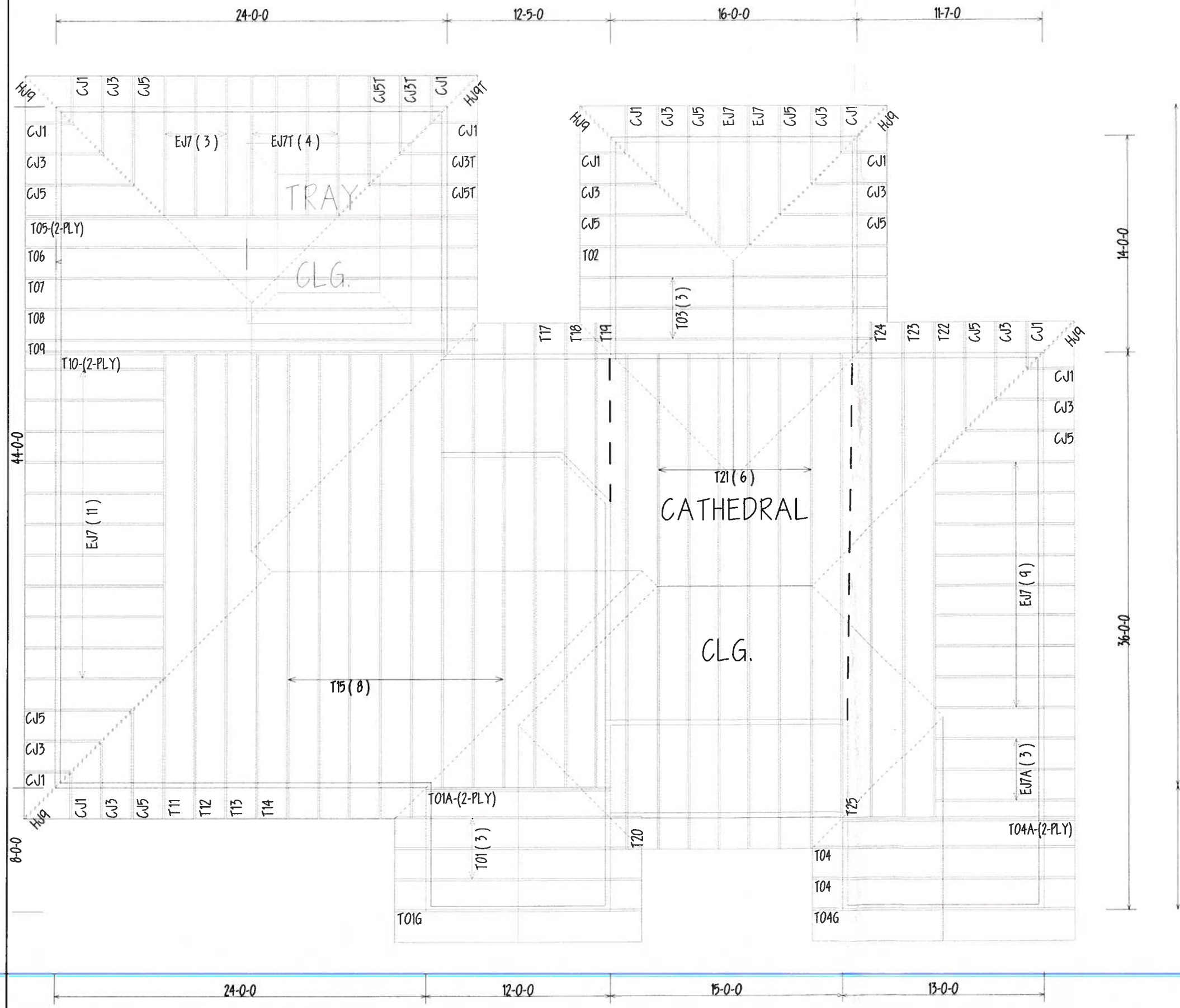
Robert W. Wall PE 46021

Modification needed: ½" CUT OFF THE RIGHT SIDE AS SHOWN ABOVE.

Solution : ½" plywood or o.s.b. one face 28"x28" cut to fit with 8d's 2" o.c. each member.

ROBERT W. WALL LICENSE No. 46021 STATE OF FLORIDA PROFESSIONAL ENGINEER

All trusses must be in an un-deflected state. No loading or braced to no deflection. If conditions change from above notify truss manufacturer. Do not damage existing plates unless otherwise noted.



BEARING HEIGHT SCHEDULE

8'-0"

6/12 PITCH

2' OH

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved By: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3494

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: JOHN NORRIS

LEGAL ADDRESS: LOT 32 THORNWOOD

MODEL: MADISON ELEVATION:

DATE: 7-31-06 DRAWN BY: KLH SCALE: NTS JOB #: L204776