



Project Information for: L280515

Builder: JOHN NORRIS CONST.
 Address : 144 SE BUNNY CT
 ... LAKE CITY, FL 32025
 County: COLUMBIA
 Truss Count: 28
 Design Program: MiTek 20/20 6.3
 Building Code: FBC2004/TPI2002

Truss Design Load Information:

Gravity: Wind:

Roof (psf): 42.0 Wind Standard: ASCE 7-02 Wind Exposure: B
 Floor (psf): N/A Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for structural engineering:

John David Norris Florida Registered General Contractor License No. RG0066597
 Address: 351 NW Corwin GLN Lake City, Florida 32055

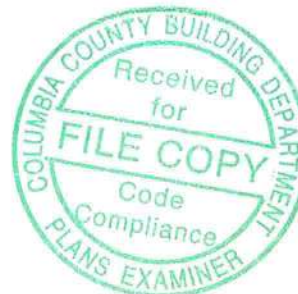
Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elements in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

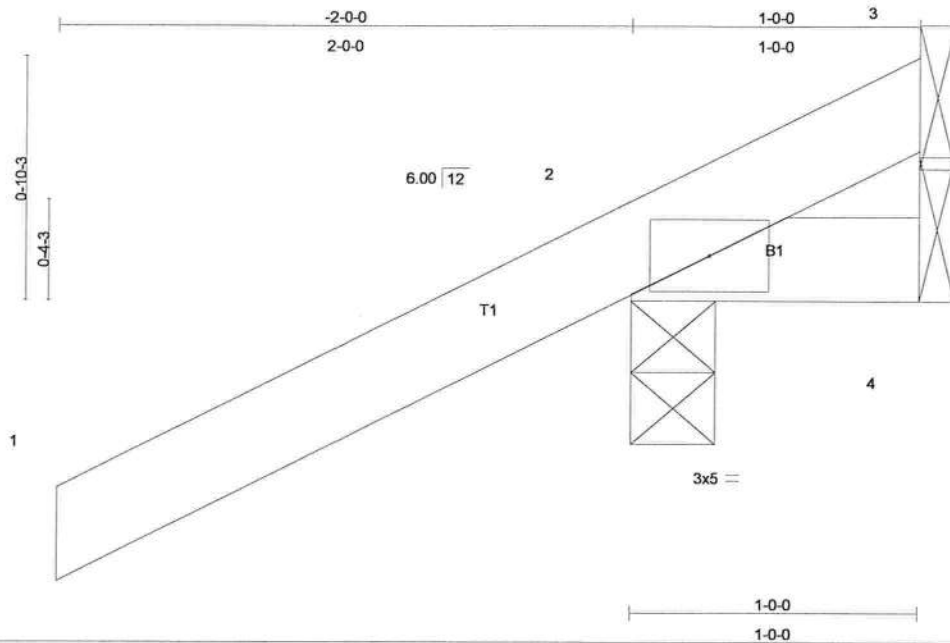
No.	Drwg. #	Truss ID	Date
1	J1973087	CJ1	6/13/08
2	J1973088	CJ3	6/13/08
3	J1973089	CJ5	6/13/08
4	J1973090	EJ7	6/13/08
5	J1973091	EJ7C	6/13/08
6	J1973092	HJ9	6/13/08
7	J1973093	T01	6/13/08
8	J1973094	T02	6/13/08
9	J1973095	T03	6/13/08
10	J1973096	T04	6/13/08
11	J1973097	T05	6/13/08
12	J1973098	T06	6/13/08
13	J1973099	T07	6/13/08
14	J1973100	T08	6/13/08
15	J1973101	T09	6/13/08
16	J1973102	T10	6/13/08
17	J1973103	T11	6/13/08
18	J1973104	T12	6/13/08
19	J1973105	T13	6/13/08
20	J1973106	T14	6/13/08
21	J1973107	T15	6/13/08
22	J1973108	T15G	6/13/08
23	J1973109	T16	6/13/08
24	J1973110	T17	6/13/08
25	J1973111	T18	6/13/08
26	J1973112	T19	6/13/08
27	J1973113	T20	6/13/08
28	J1973114	T20G	6/13/08



Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	CJ1	JACK	8	1	J1973087
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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Scale: 1.5"=1'

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.28	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.01	Vert(TL)	-0.00	2	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 7 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical
Max Horz 2=87(load case 6)
Max Uplift 2=-286(load case 6), 4=-9(load case 4), 3=-90(load case 1)
Max Grav 2=256(load case 1), 4=14(load case 2), 3=127(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-69/75
BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.17

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 9 lb uplift at joint 4 and 90 lb uplift at joint 3.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 24869
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

June 13, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	CJ1	JACK	8	1	J1973087
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34866
1408 Coastal Bay Blvd
Boynton Beach, FL 33435

June 13, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

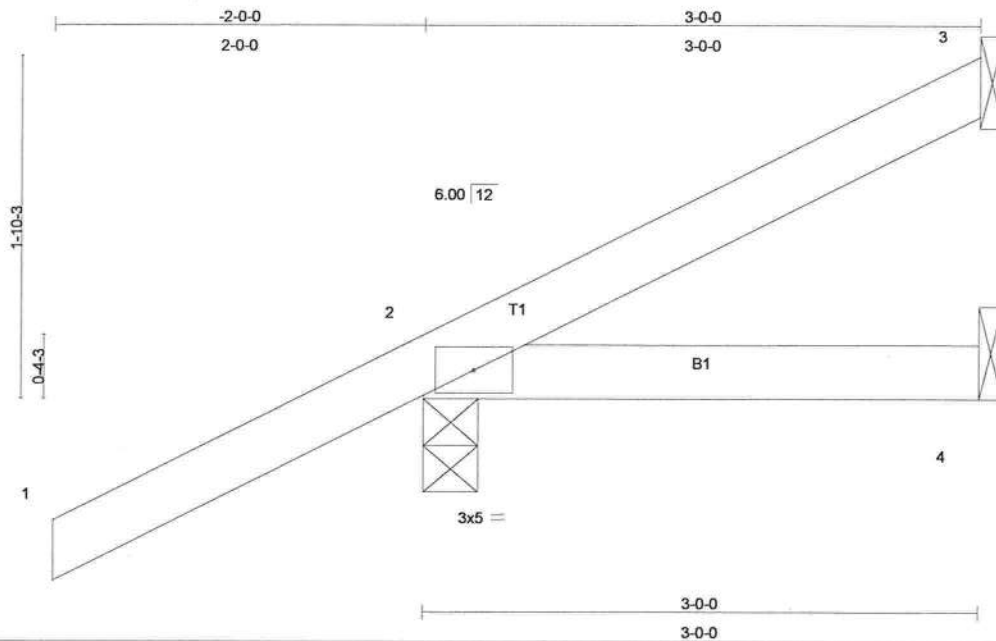
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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	CJ3	JACK	8	1	J1973088
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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Scale = 1:12.5

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.29	Vert(LL)	0.01	2-4	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.08	Vert(TL)	-0.01	2-4	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 13 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or
3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc
bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical
Max Horz 2=132(load case 6)
Max Uplift 3=-28(load case 7), 2=-238(load case 6), 4=-27(load case 4)
Max Grav 3=31(load case 1), 2=250(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.15

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 31889
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 13, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	CJ3	JACK	8	1	J1973088
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34888
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 13, 2008

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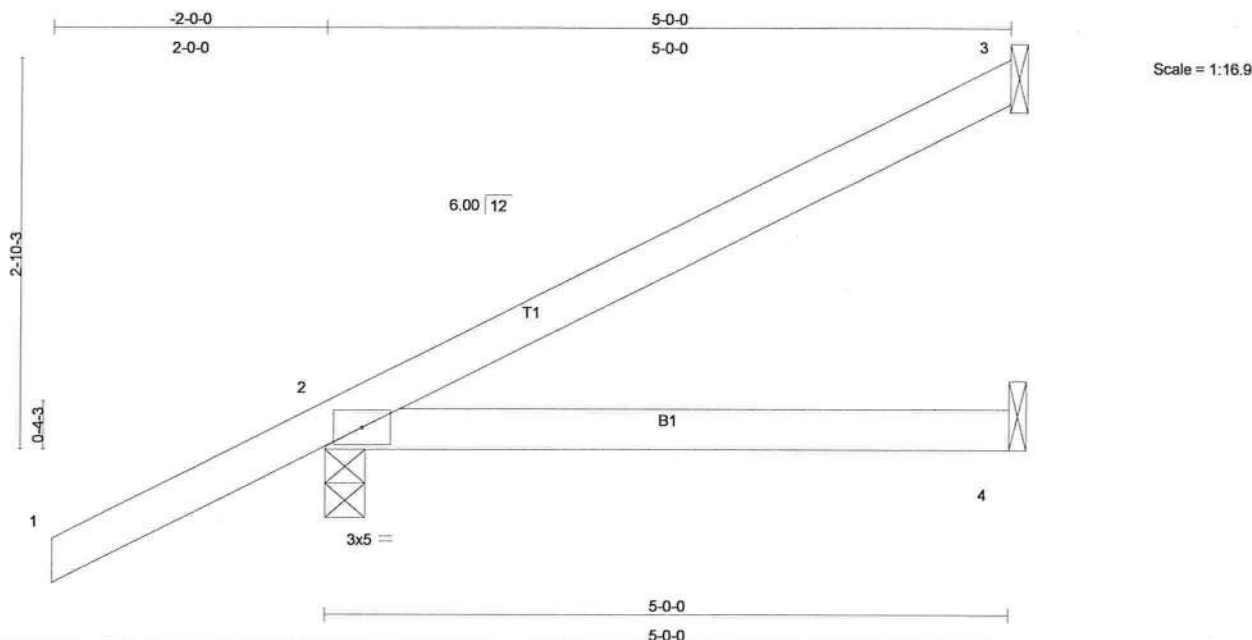
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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.	J1973089
L280515	CJ5	JACK	8	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.29	Vert(LL)	0.09	2-4	>663	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.24	Vert(TL)	-0.05	2-4	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 19 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=295/0-3-8, 4=24/Mechanical

Max Horz 2=178(load case 6)

Max Uplift 3=-87(load case 6), 2=-260(load case 6), 4=-46(load case 4)

Max Grav 3=103(load case 1), 2=295(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-88/36

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.17

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida P.E. No. 34869
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

June 13,2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	CJ5	JACK	8	1	J1973089
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:07 2008 Page 2

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34868
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

June 13, 2008

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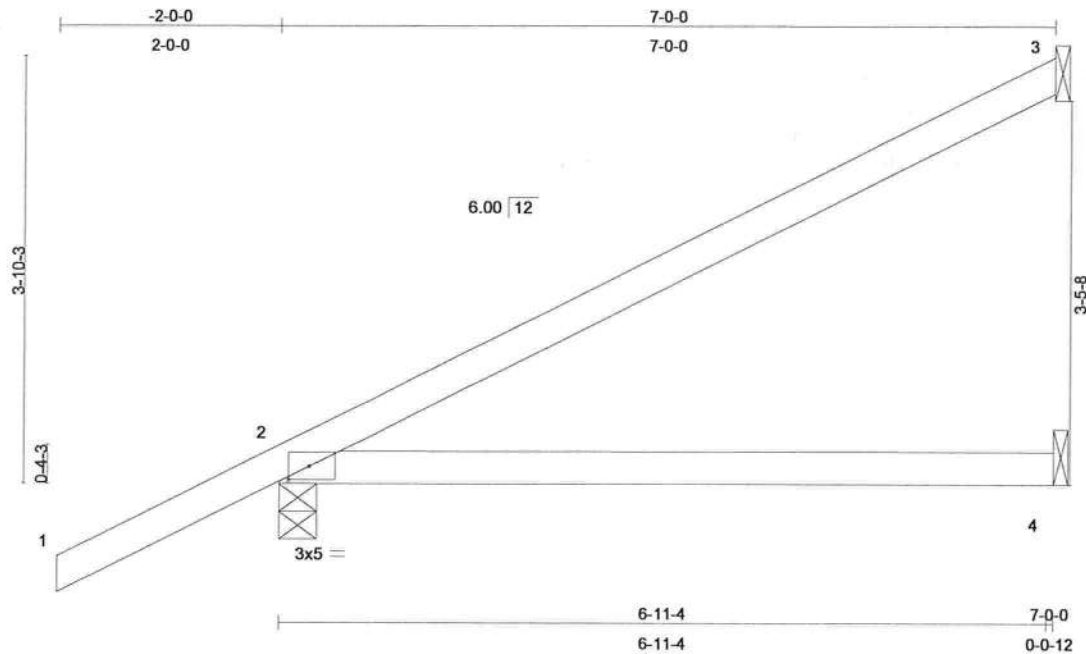
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Job	Truss	Truss Type	Qty	Ply	JIM WALTER - BRASWELL RES.
L280515	EJ7	JACK	26	1	J1973090
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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Scale = 1:20.8

Plate Offsets (X,Y): [2:0-2-4,0-1-8]

LOADING (psf)	SPACING		CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.50	Vert(LL)	0.32	2-4	>253	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.45	Vert(TL)	-0.16	2-4	>506	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
										Weight: 26 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=154/Mechanical, 2=352/0-4-0, 4=44/Mechanical

Max Horz 2=161(load case 6)

Max Uplift 3=-94(load case 6), 2=-225(load case 6), 4=-64(load case 5)

Max Grav 3=154(load case 1), 2=352(load case 1), 4=93(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-131/54

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.66

Julius Lee
Truss Design Engineer
Florida RES No. 31888
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 13,2008

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	JIM WALTER - BRASWELL RES.
L280515	EJ7	JACK	26	1	J1973090
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 3, 225 lb uplift at joint 2 and 64 lb uplift at joint 4.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34868
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

June 13, 2008

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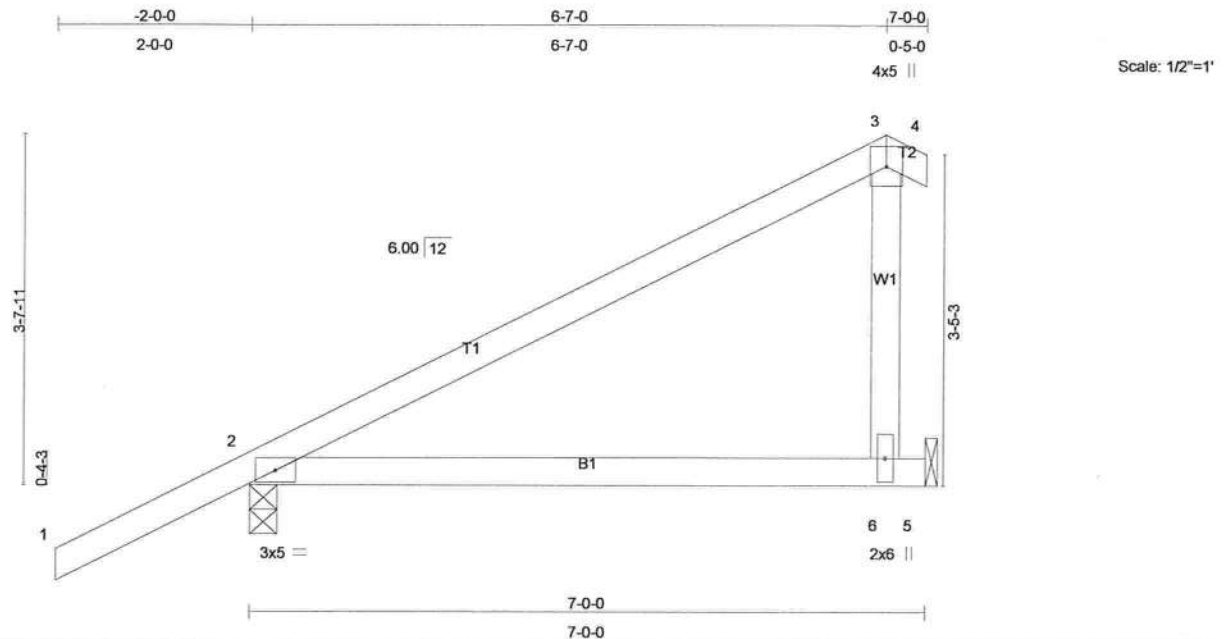
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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.	J1973091
L280515	EJ7C	COMMON	3	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:08 2008 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.56	Vert(LL)	-0.04	2-6	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.14	Vert(TL)	-0.08	2-6	>961	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00	5	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 30 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=351/0-3-8, 5=202/Mechanical
Max Horz 2=147(load case 6)
Max Uplift 2=-146(load case 6), 5=-69(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-168/4, 3-4=0/10, 3-6=-156/209
BOT CHORD 2-6=-75/79, 5-6=0/0

JOINT STRESS INDEX

2 = 0.53, 3 = 0.58 and 6 = 0.69

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) -2-0-0 to 7-0-0 zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 146 lb uplift at joint 2 and 69 lb uplift at joint 5.

Julius Lee
Truss Design Engineer
Florida PE No. 34888
1100 Coastal Bay Blvd.
Boynton Beach, FL 33435

LOAD CASE(S) Standard

June 13,2008

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REACTIONS (lb/size) 4=268/Mechanical, 2=456/0-5-11, 5=218/Mechanical
Max Horz 2=269(load case 3)
Max Uplift 4=-233(load case 3), 2=-401(load case 3), 5=-181(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/50, 2-3=-647/363, 3-4=-105/65
BOT CHORD 2-7=-535/599, 6-7=-535/599, 5-6=0/0
WEBS 3-7=-94/190, 3-6=-624/558

JOINT STRESS INDEX

2 = 0.77, 3 = 0.25, 6 = 0.21 and 7 = 0.13

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 4. 401 lb uplift at joint 2 and 181 lb uplift at joint 5.

Julius Lars
Truss Design Engineer
Florida PE No. 34888
1100 Coastal Bay Blvd
Dunedin Beach, FL 33435

June 13, 2008

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	HJ9	MONO TRUSS	4	1	J1973092
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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NOTES

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Julius Lee
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June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T01	HIP	1	2	J1973093
Builders FirstSource, Lake City, FL 32055					6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:10 2008 Page 1
					Job Reference (optional)

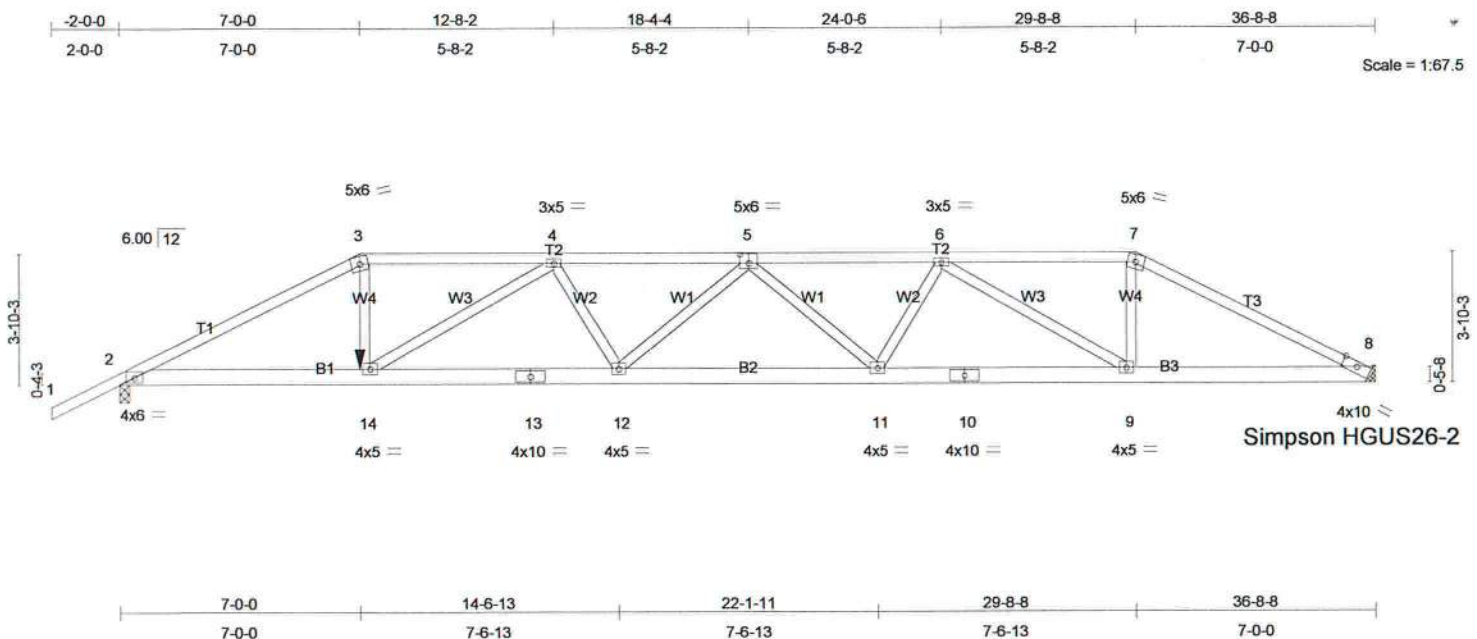


Plate Offsets (X,Y): [5:0-3-0,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.33	Vert(LL)	-0.24 11-12	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.41	Vert(TL)	-0.47 11-12	>939	240		
BCLL 10.0	* Rep Stress Incr	NO	WB 0.39	Horz(TL)	0.10 8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)					Weight: 398 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 6 SYP No.1D
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-1-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=2571/Mechanical, 2=2521/0-3-8
 Max Horz 2=94(load case 5)
 Max Uplift 8=-757(load case 3), 2=-765(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/51, 2-3=-4949/1594, 3-4=-4418/1475, 4-5=-6508/2135, 5-6=-6429/2087,
 6-7=-4339/1387, 7-8=-4856/1495
 BOT CHORD 2-14=-1419/4348, 13-14=-2075/6248, 12-13=-2075/6248, 11-12=-2252/6831,
 10-11=-1989/6166, 9-10=-1989/6166, 8-9=-1288/4271
 WEBS 3-14=-504/1700, 4-14=-2246/815, 4-12=-76/567, 5-12=-464/235, 5-11=-573/294,
 6-11=-98/579, 6-9=-2256/847, 7-9=-459/1628

JOINT STRESS INDEX

2 = 0.76, 3 = 0.49, 4 = 0.43, 5 = 0.47, 6 = 0.43, 7 = 0.52, 8 = 0.43, 9 = 0.42, 10 = 0.78, 11 = 0.29, 12 = 0.29, 13 = 0.81 and 14 = 0.44

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 Truss Design Engineer
 Florida PE No. 34868
 1100 Coastal Bay Blvd
 Boynton Beach, FL 33435

Continued on page 2

June 13, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.	J1973093
L280515	T01	HIP	1	2	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:10 2008 Page 2

NOTES

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section.
Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS;
Lumber DOL=1.60 plate grip DOL=1.60.
- 5) Provide adequate drainage to prevent water ponding.
- 6) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 757 lb uplift at joint 8 and 765 lb uplift at joint 2.

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-118(F=-64), 7-8=-54, 2-14=-10, 9-14=-22(F=-12), 8-9=-85(F=-75)
Concentrated Loads (lb)
Vert: 14=-411(F)

Julius Lee
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Florida P.E. No. 24888
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June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T02	HIP	1	1	J1973094
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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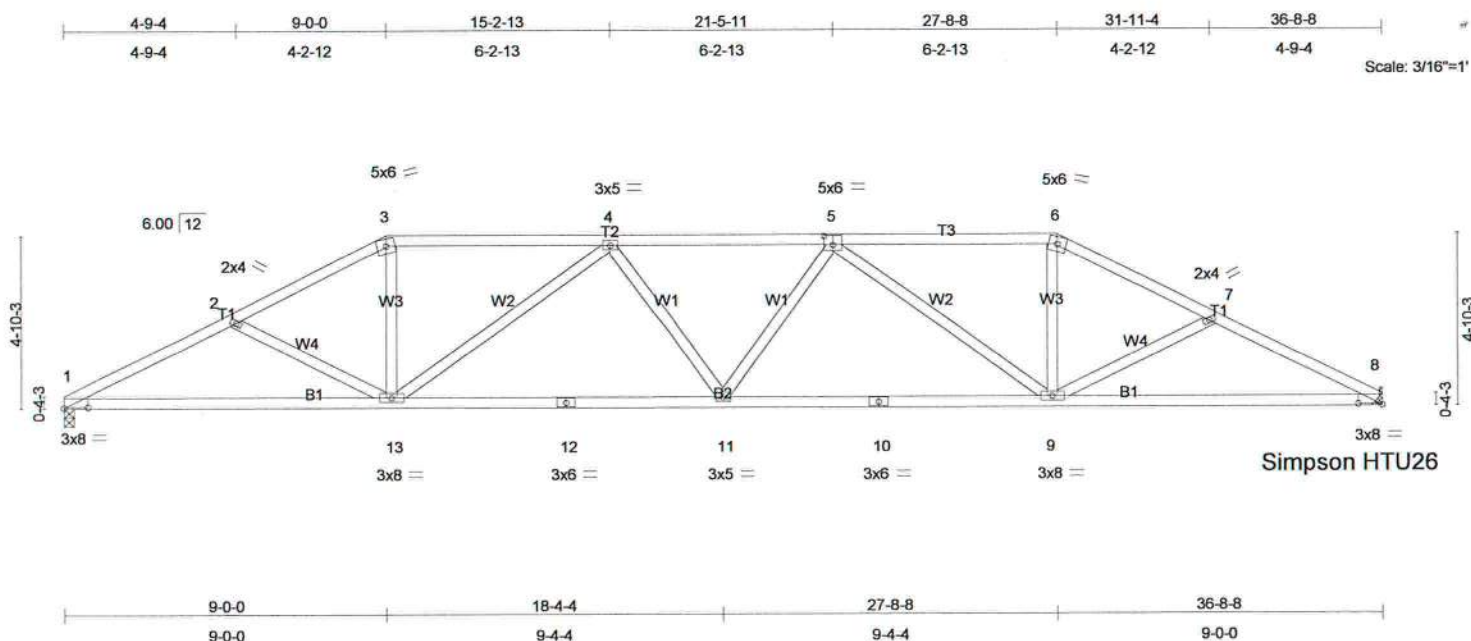


Plate Offsets (X,Y): [1:0-8-0,0-0-6], [5:0-3-0,0-3-0], [8:0-8-0,0-0-6]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.30	Vert(LL)	0.23	11	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.56	Vert(TL)	-0.39	9-11	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.74	Horz(TL)	0.13	8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 177 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-11-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-6 oc bracing.

REACTIONS (lb/size) 1=1165/0-3-8, 8=1165/Mechanical
Max Horz 1=-58(load case 4)
Max Uplift 1=-225(load case 5), 8=-225(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2239/1229, 2-3=-2016/1107, 3-4=-1779/1050, 4-5=-2321/1317,
5-6=-1779/1050, 6-7=-2016/1107, 7-8=-2239/1229
BOT CHORD 1-13=-1021/1949, 12-13=-1104/2291, 11-12=-1104/2291, 10-11=-1104/2291,
9-10=-1104/2291, 8-9=-1021/1949
WEBS 2-13=-215/238, 3-13=-272/583, 4-13=-722/357, 4-11=0/164, 5-11=0/164,
5-9=-722/357, 6-9=-272/583, 7-9=-215/238

JOINT STRESS INDEX

1 = 0.66, 2 = 0.33, 3 = 0.48, 4 = 0.41, 5 = 0.47, 6 = 0.48, 7 = 0.33, 8 = 0.66, 9 = 0.56, 10 = 0.81, 11 = 0.41, 12 = 0.81 and 13 = 0.56

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 31868
1100 Coastal Bay Blvd.
Boynton Beach, FL 33435

June 13, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T02	HIP	1	1	J1973094
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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NOTES

- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 225 lb uplift at joint 1 and 225 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lane
Truss Design Engineer
Florida PE No. 24888
1100 Coastal Bay Blvd
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June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T03	HIP	1	1	J1973095
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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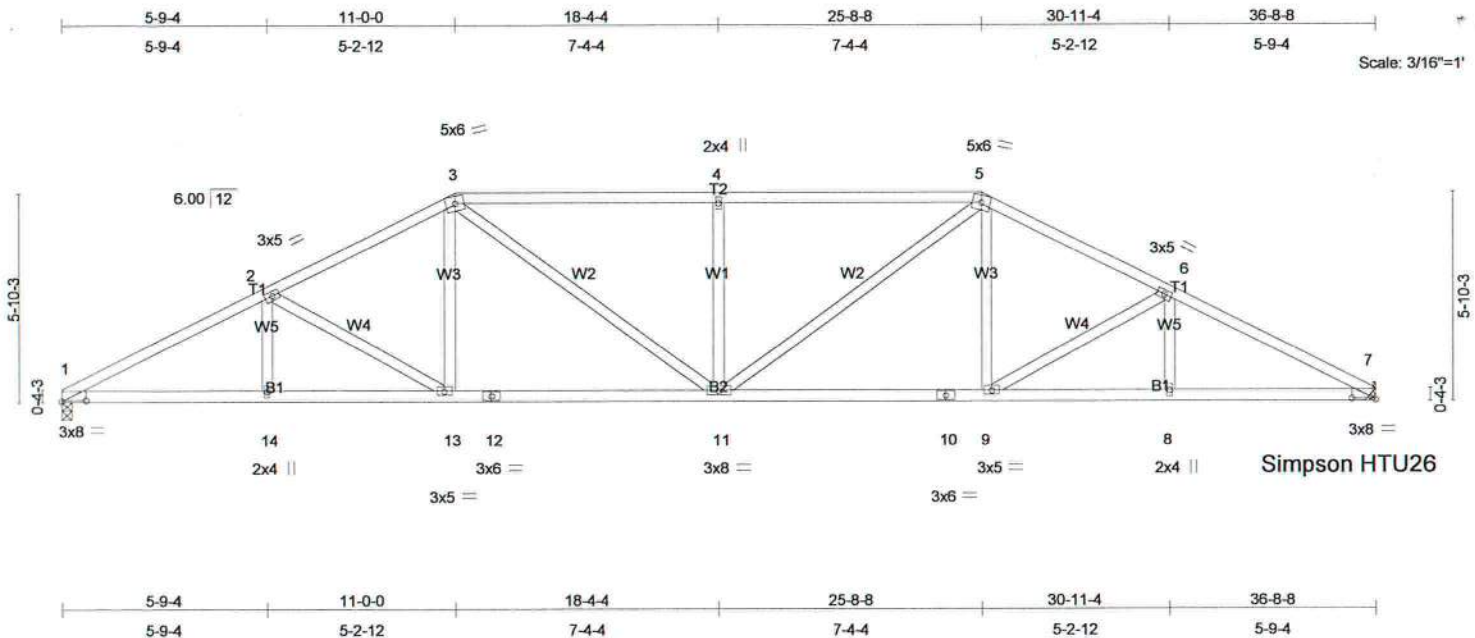


Plate Offsets (X,Y): [1:0-8-0,0-0-6], [7:0-8-0,0-0-6]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.38	Vert(LL)	0.19	11	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.47	Vert(TL)	-0.31	9-11	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.30	Horz(TL)	0.11	7	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 188 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-11-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-2-0 oc bracing.

REACTIONS (lb/size) 1=1165/0-3-8, 7=1165/Mechanical
Max Horz 1=-71(load case 4)
Max Uplift 1=-211(load case 6), 7=-211(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2254/1217, 2-3=-1898/1083, 3-4=-1965/1186, 4-5=-1965/1186,
5-6=-1898/1083, 6-7=-2254/1217
BOT CHORD 1-14=-999/1942, 13-14=-999/1942, 12-13=-742/1639, 11-12=-742/1639,
10-11=-742/1639, 9-10=-742/1639, 8-9=-999/1942, 7-8=-999/1942
WEBS 2-14=0/159, 2-13=-347/293, 3-13=-107/306, 3-11=-202/526, 4-11=-422/292,
5-11=-202/526, 5-9=-107/306, 6-9=-347/293, 6-8=0/159

JOINT STRESS INDEX

1 = 0.67, 2 = 0.46, 3 = 0.55, 4 = 0.33, 5 = 0.55, 6 = 0.46, 7 = 0.67, 8 = 0.33, 9 = 0.39, 10 = 0.52, 11 = 0.56, 12 = 0.52, 13 = 0.39 and 14 = 0.33

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 24888
1100 Coastal Bay Blvd
Beverly Beach, FL 32435

June 13, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T03	HIP	1	1	J1973095
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:12 2008 Page 2

NOTES

- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 211 lb uplift at joint 1 and 211 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 24898
1109 Coastal Bay Blvd
Boynton Beach, FL 33436

June 13, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T04	SPECIAL	1	1	J1973096
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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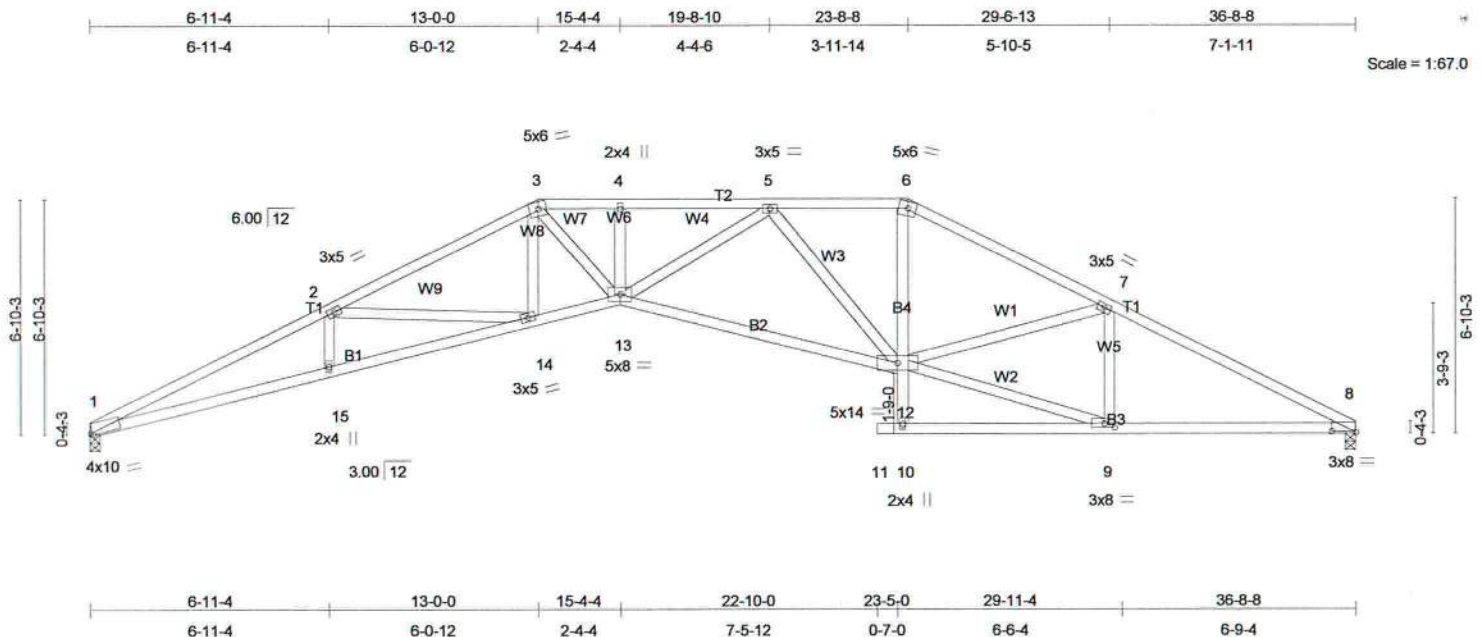


Plate Offsets (X,Y): [1:0-0-14,Edge], [8:0-8-4,0-0-6], [9:0-3-8,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.61	Vert(LL)	0.50	13	>878	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.83	Vert(TL)	-0.84	12-13	>523	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.73	Horz(TL)	0.50	8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 188 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 *Except*
 B4 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or
 2-6-11 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 4-4-13 oc
 bracing.

REACTIONS (lb/size) 1=1168/0-3-8, 8=1170/0-3-8
 Max Horz 1=85(load case 5)
 Max Uplift 1=-224(load case 6), 8=-222(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-4123/2171, 2-3=-3366/1731, 3-4=-3724/1929, 4-5=-3724/1929,
 5-6=-2072/1210, 6-7=-2357/1282, 7-8=-2225/1197
 BOT CHORD 1-15=-1892/3705, 14-15=-1890/3704, 13-14=-1306/3023, 12-13=-1247/2840,
 10-12=0/102, 6-12=-374/762, 10-11=0/0, 9-10=-21/29, 8-9=-965/1906
 WEBS 2-15=0/204, 2-14=-665/554, 3-14=-166/256, 3-13=-505/1227, 4-13=-156/81,
 5-13=-463/1154, 5-12=-1086/527, 9-12=-983/1956, 7-9=-460/296, 7-12=-77/290

JOINT STRESS INDEX

1 = 0.61, 2 = 0.46, 3 = 0.58, 4 = 0.33, 5 = 0.75, 6 = 0.43, 7 = 0.46, 8 = 0.72, 9 = 0.75, 10 = 0.70, 12 = 0.56, 13 = 0.92, 14 = 0.43 and 15 = 0.33

NOTES

1) Unbalanced roof live loads have been considered for this design.

Julius Lee
 Truss Design Engineer
 Florida PE No. 34888
 1100 Coastal Bay Blvd
 Boynton Beach, FL 33435

Continued on page 2

June 13,2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T04	SPECIAL	1	1	J1973096
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:13 2008 Page 2

NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 224 lb uplift at joint 1 and 222 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida P.E. No. 31898
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 13,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T05	SPECIAL	1	1	J1973097
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:14 2008 Page 1

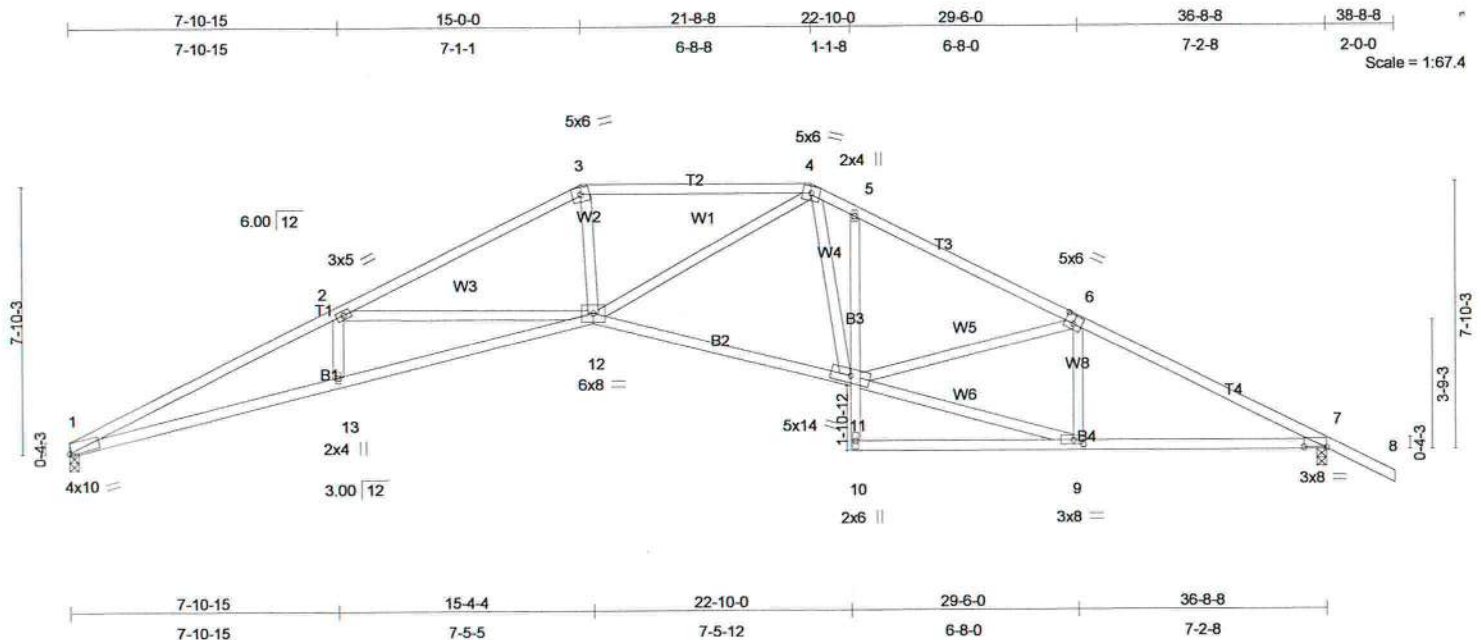


Plate Offsets (X,Y): [1:0-0-14,Edge], [6:0-3-0,0-3-0], [7:0-8-0,0-0-6], [9:0-3-8,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.85	Vert(LL)	0.42 12-13	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.88	Vert(TL)	-0.74 12-13	>592	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.86	Horz(TL)	0.44 7	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)					Weight: 191 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 *Except*
 B3 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied.
 BOT CHORD Rigid ceiling directly applied or 4-6-3 oc bracing.

REACTIONS (lb/size) 1=1162/0-3-8, 7=1285/0-3-8
 Max Horz 1=-137(load case 7)
 Max Uplift 1=-237(load case 6), 7=-331(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-4060/2070, 2-3=-3083/1485, 3-4=-2769/1436, 4-5=-2216/1319,
 5-6=-2309/1239, 6-7=-2179/1159, 7-8=0/47
 BOT CHORD 1-13=-1711/3647, 12-13=-1707/3643, 11-12=-697/1998, 10-11=0/90,
 5-11=-151/194, 9-10=-14/34, 7-9=-845/1862
 WEBS 2-13=0/237, 2-12=-869/712, 3-12=-375/967, 4-12=-352/976, 4-11=-273/289,
 9-11=-873/1915, 6-11=-57/242, 6-9=-460/292

JOINT STRESS INDEX

1 = 0.60, 2 = 0.46, 3 = 0.73, 4 = 0.51, 5 = 0.59, 6 = 0.73, 7 = 0.67, 9 = 0.73, 10 = 0.52, 11 = 0.71, 12 = 0.84 and 13 = 0.33

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Continued on page 2

Julius Lee
 Truss Design Engineer
 Florida PE No. 34888
 1409 Coastal Bay Blvd
 Boynton Beach, FL 33435

June 13, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
 This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T05	SPECIAL	1	1	J1973097
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:14 2008 Page 2

NOTES

- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 237 lb uplift at joint 1 and 331 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34888
1100 Coastal Bay Blvd.
Boynton Beach, FL 33435

June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T06	SPECIAL	1	1	J1973098
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:15 2008 Page 1

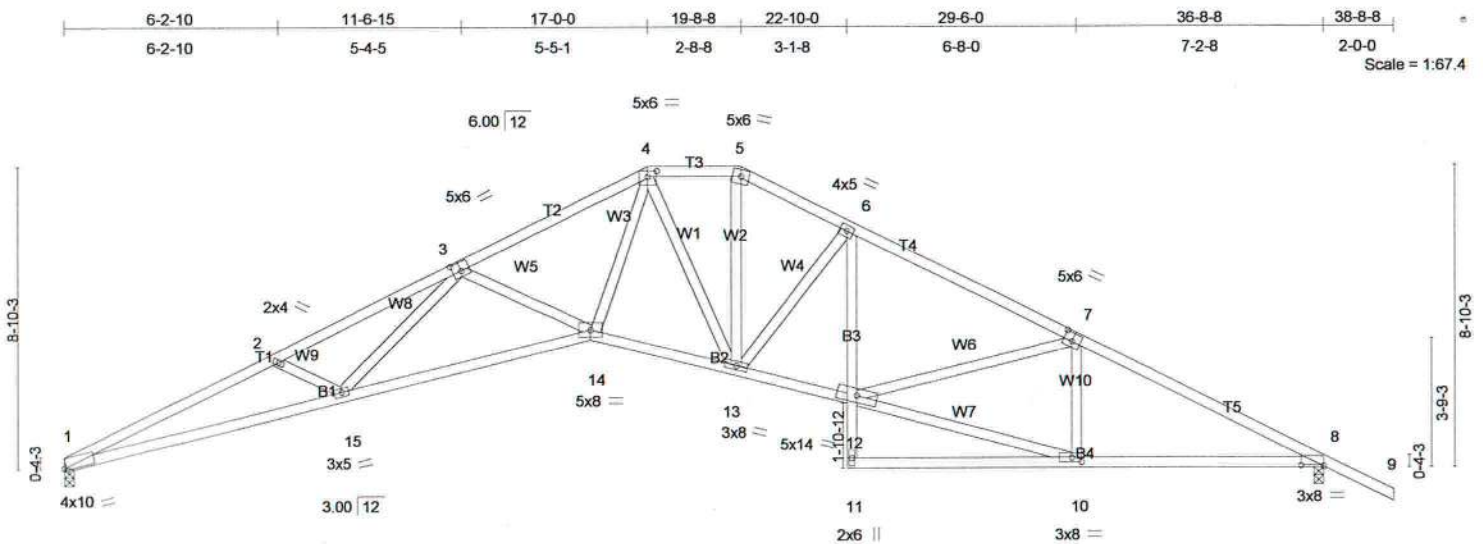


Plate Offsets (X,Y): [1:0-0-14,Edge], [3:0-3-0,0-3-0], [4:0-3-0,0-2-0], [7:0-3-0,0-3-0], [8:0-8-0,0-0-6], [10:0-3-8,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.46	Vert(LL)	0.43 14-15	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.81	Vert(TL)	-0.75 14-15	>586	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.71	Horz(TL)	0.43 8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)					Weight: 203 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 *Except*
 B3 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or
 2-8-15 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 4-6-7 oc
 bracing.

REACTIONS (lb/size) 1=1162/0-3-8, 8=1285/0-3-8
 Max Horz 1=-149(load case 7)
 Max Uplift 1=-248(load case 6), 8=-342(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-4104/2173, 2-3=-3820/2026, 3-4=-2916/1484, 4-5=-1810/1083,
 5-6=-2052/1171, 6-7=-2284/1256, 7-8=-2178/1174, 8-9=0/47
 BOT CHORD 1-15=-1822/3694, 14-15=-1344/3139, 13-14=-677/2106, 12-13=-780/2033,
 11-12=0/93, 6-12=-49/125, 10-11=-17/77, 8-10=-859/1861
 WEBS 2-15=-260/296, 3-15=-301/495, 3-14=-567/484, 4-14=-697/1641, 4-13=-613/216,
 5-13=-456/781, 6-13=-320/305, 10-12=-882/1877, 7-12=-36/192, 7-10=-442/291

JOINT STRESS INDEX

1 = 0.61, 2 = 0.33, 3 = 0.58, 4 = 0.83, 5 = 0.36, 6 = 0.47, 7 = 0.72, 8 = 0.67, 10 = 0.71, 11 = 0.60, 12 = 0.93, 13 = 0.62, 14 = 0.95 and 15 = 0.43

NOTES

1) Unbalanced roof live loads have been considered for this design.

Julius Lee
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 Florida PE No. 34856
 1109 Coastal Bay Blvd
 Boynton Beach, FL 33435

June 13, 2008

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T06	SPECIAL	1	1	J1973098
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:15 2008 Page 2

NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 248 lb uplift at joint 1 and 342 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida P.E. No. 24888
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

June 13,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T07	SPECIAL	3	1	J1973099
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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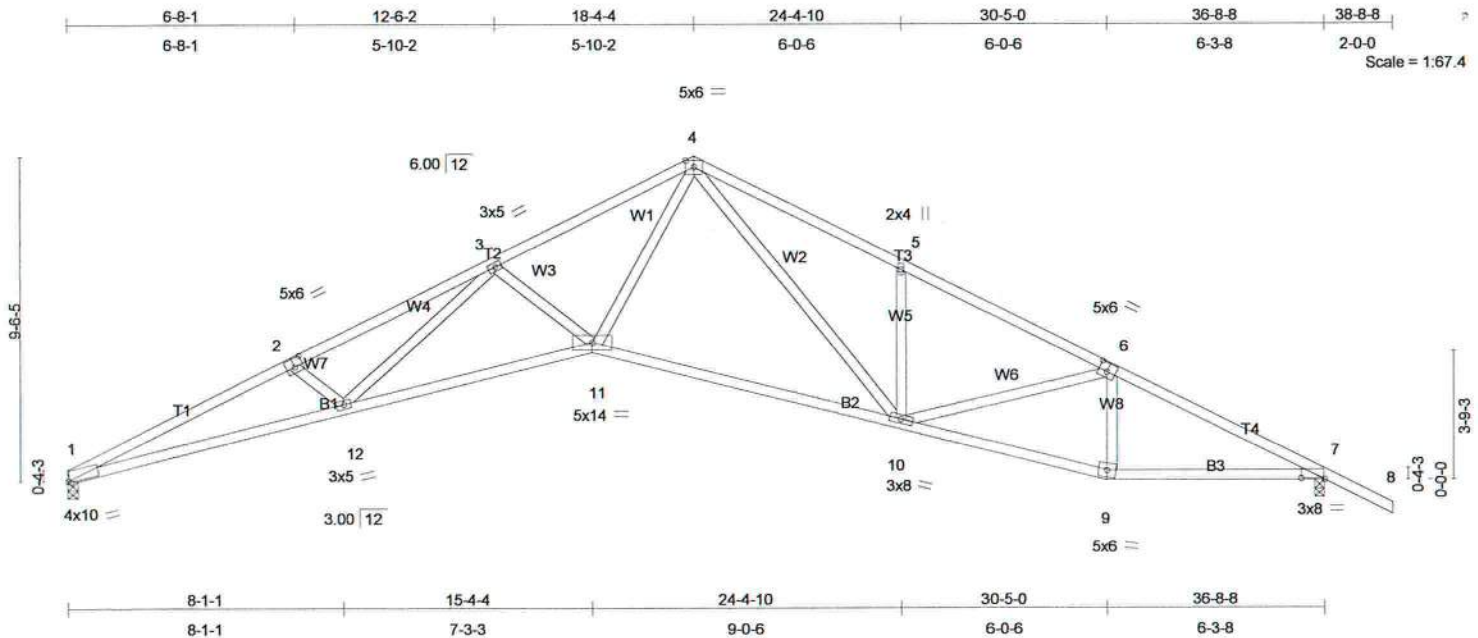


Plate Offsets (X,Y): [1:0-0-14,Edge], [2:0-3-0,0-3-0], [6:0-3-0,0-3-0], [7:0-8-4,0-0-6]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.55	Vert(LL)	0.44 11-12	>995	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.83	Vert(TL)	-0.73 10-11	>601	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.84	Horz(TL)	0.43 7	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)					Weight: 181 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-7-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-6-0 oc bracing.

REACTIONS (lb/size) 1=1162/0-3-8, 7=1285/0-3-8
Max Horz 1=-157(load case 7)
Max Uplift 1=-254(load case 6), 7=-348(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-4088/2173, 2-3=-3840/2094, 3-4=-2932/1531, 4-5=-2351/1470,
5-6=-2354/1298, 6-7=-2209/1203, 7-8=0/47
BOT CHORD 1-12=-1817/3675, 11-12=-1270/3036, 10-11=-548/1787, 9-10=-923/1954,
7-9=-897/1896
WEBS 2-12=-267/297, 3-12=-435/625, 3-11=-524/451, 4-11=-770/1778, 4-10=-502/633,
5-10=-338/334, 6-10=-13/185, 6-9=-413/265

JOINT STRESS INDEX

1 = 0.61, 2 = 0.67, 3 = 0.46, 4 = 0.78, 5 = 0.33, 6 = 0.57, 7 = 0.73, 9 = 0.51, 10 = 0.66, 11 = 0.90 and 12 = 0.44

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34868
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

June 13, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.	J1973099
L280515	T07	SPECIAL	3	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:16 2008 Page 2

NOTES

- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 254 lb uplift at joint 1 and 348 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34888
1409 Coastal Bay Blvd
Boynton Beach, FL 33435

June 13, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T08	SPECIAL	4	1	J1973100
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:17 2008 Page 1

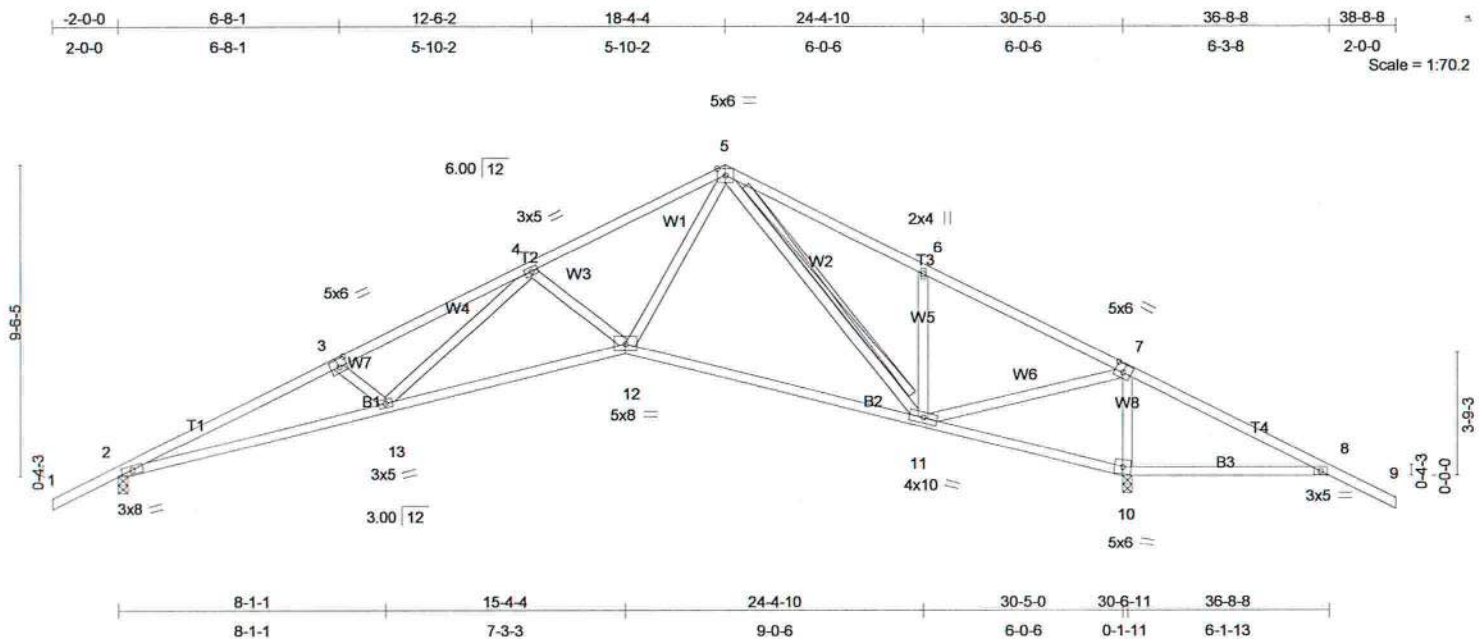


Plate Offsets (X,Y): [3:0-3-0,0-3-0], [7:0-3-0,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.39	Vert(LL)	0.23 12-13	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.59	Vert(TL)	-0.43 12-13	>836	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.54	Horz(TL)	0.26 10	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)					Weight: 184 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-4-5 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
 WEBS T-Brace: 2 X 4 SYP No.3 - 5-11
 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
 Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1021/0-3-8, 10=1543/0-3-8
 Max Horz 2=-144(load case 7)
 Max Uplift 2=-311(load case 6), 10=-588(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-2959/1272, 3-4=-2708/1194, 4-5=-1805/680, 5-6=-837/332,
 6-7=-842/199, 7-8=-846/714, 8-9=0/47
 BOT CHORD 2-13=-983/2639, 12-13=-479/1999, 11-12=-101/988, 10-11=-616/954,
 8-10=-565/896
 WEBS 3-13=-266/293, 4-13=-387/600, 4-12=-520/441, 5-12=-392/1281, 5-11=-427/201,
 6-11=-334/326, 7-11=-758/1320, 7-10=-1321/922

Julius Lee
 Truss Design Engineer
 Florida P.E. No. 34889
 4100 Coastal Bay Blvd.
 Boynton Beach, FL 33435

JOINT STRESS INDEX

2 = 0.68, 3 = 0.54, 4 = 0.46, 5 = 0.56, 6 = 0.33, 7 = 0.73, 8 = 0.55, 10 = 0.63, 11 = 0.56, 12 = 0.90 and 13 = 0.43

Continued on page 2

June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.	J1973100
L280515	T08	SPECIAL	4	1	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:17 2008 Page 2

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 311 lb uplift at joint 2 and 588 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34888
1405 Coastal Bay Blvd
Boynton Beach, FL 33435

June 13, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Builders FirstSource, Lake City, FL 32055 6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:18 2008 Page 1



LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 2-5-3 oc purlins, except end verticals.
BOT CHORD	2 X 6 SYP No.1D	BOT CHORD	Rigid ceiling directly applied or 6-6-5 oc bracing.
WEBS	2 X 4 SYP No.3	WEBS	T-Brace: 2 X 4 SYP No.3 - 4-12, 6-8 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length.

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/51, 2-3=-4025/1311, 3-4=-3578/1218, 4-5=-4750/1603, 5-6=-3673/1225, 6-7=-120/42, 7-8=-330/167
BOT CHORD	2-12=-1204/3525, 11-12=-1673/4758, 10-11=-1592/4557, 9-10=-1592/4557, 8-9=-1070/3012
WEBS	3-12=-357/1234, 4-12=-1372/577, 4-11=-14/170, 5-11=-22/269, 5-9=-1236/513, 6-9=-282/1202, 6-8=-3362/1195

Julius Lee
Truss Design Engineer
Florida PE No. 34889
1100 Coastal Bay Blvd.
Downtown Beach, FL 33435

Continued on page 2

June 13, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T09	HIP	1	1	J1973101
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:18 2008 Page 2

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 741 lb uplift at joint 8 and 654 lb uplift at joint 2.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-118(F=-64), 2-12=-10, 8-12=-22(F=-12)
Concentrated Loads (lb)
Vert: 12=-411(F)

Julius Lee
Truss Design Engineer
Florida P.E. No. 24888
1400 Coastal Bay Blvd
Boynton Beach, FL 33435

June 13, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T10	HIP	1	1	J1973102
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:19 2008 Page 1

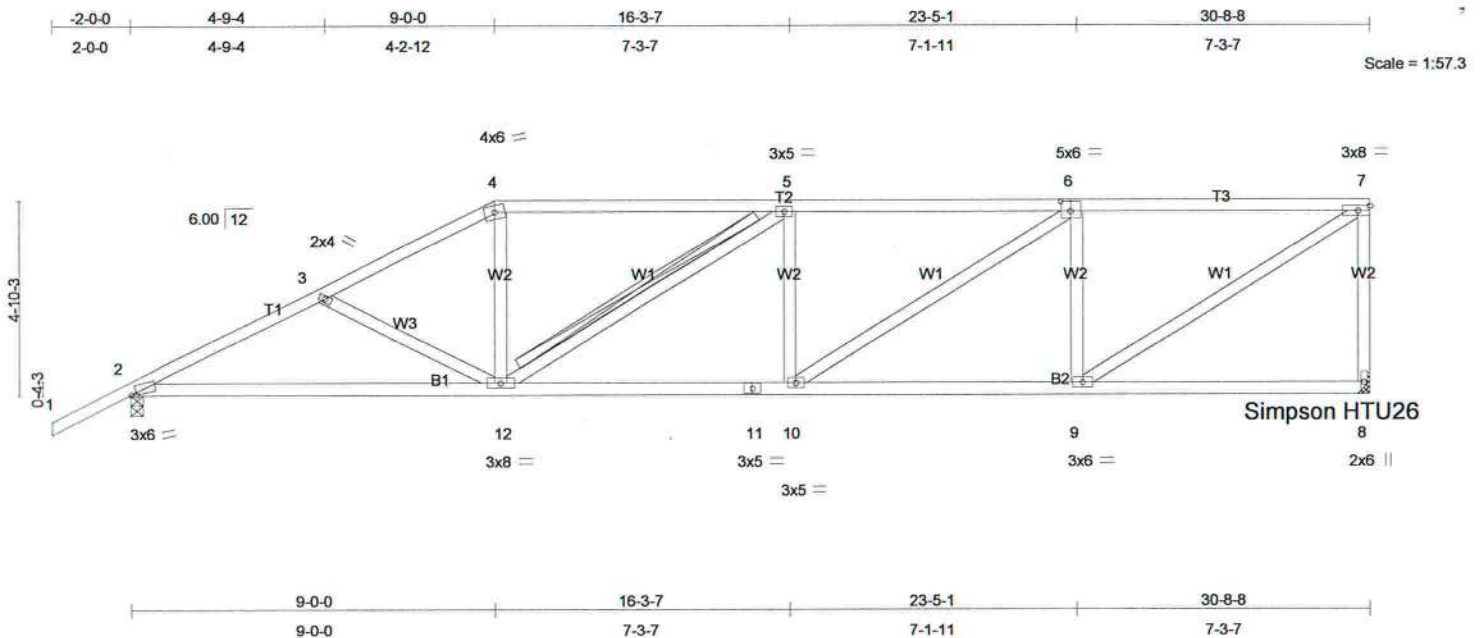


Plate Offsets (X,Y): [2:0-1-9,0-0-7], [6:0-3-0,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.89	Vert(LL)	-0.15	2-12	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.54	Vert(TL)	-0.28	2-12	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.91	Horz(TL)	0.06	8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 165 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-7-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-5-6 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 8=969/Mechanical, 2=1093/0-3-8
Max Horz 2=195(load case 6)
Max Uplift 8=-265(load case 5), 2=-262(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1791/898, 3-4=-1575/808, 4-5=-1384/785, 5-6=-1663/909, 6-7=-1213/648, 7-8=-928/528
BOT CHORD 2-12=-944/1533, 11-12=-909/1663, 10-11=-909/1663, 9-10=-662/1235, 8-9=-27/50
WEBS 3-12=-180/182, 4-12=-103/401, 5-12=-330/153, 5-10=-210/196, 6-10=-295/510, 6-9=-677/450, 7-9=-737/1379

JOINT STRESS INDEX

2 = 0.86, 3 = 0.33, 4 = 0.77, 5 = 0.39, 6 = 0.58, 7 = 0.64, 8 = 0.51, 9 = 0.78, 10 = 0.39, 11 = 0.54 and 12 = 0.56

Continued on page 2

June 13, 2008

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This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and/or contractor per ANSI/TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T10	HIP	1	1	J1973102
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:19 2008 Page 2

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 265 lb uplift at joint 8 and 262 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 24888
1100 Coastal Bay Blvd.
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June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T11	HIP	1	1	J1973103
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:20 2008 Page 1

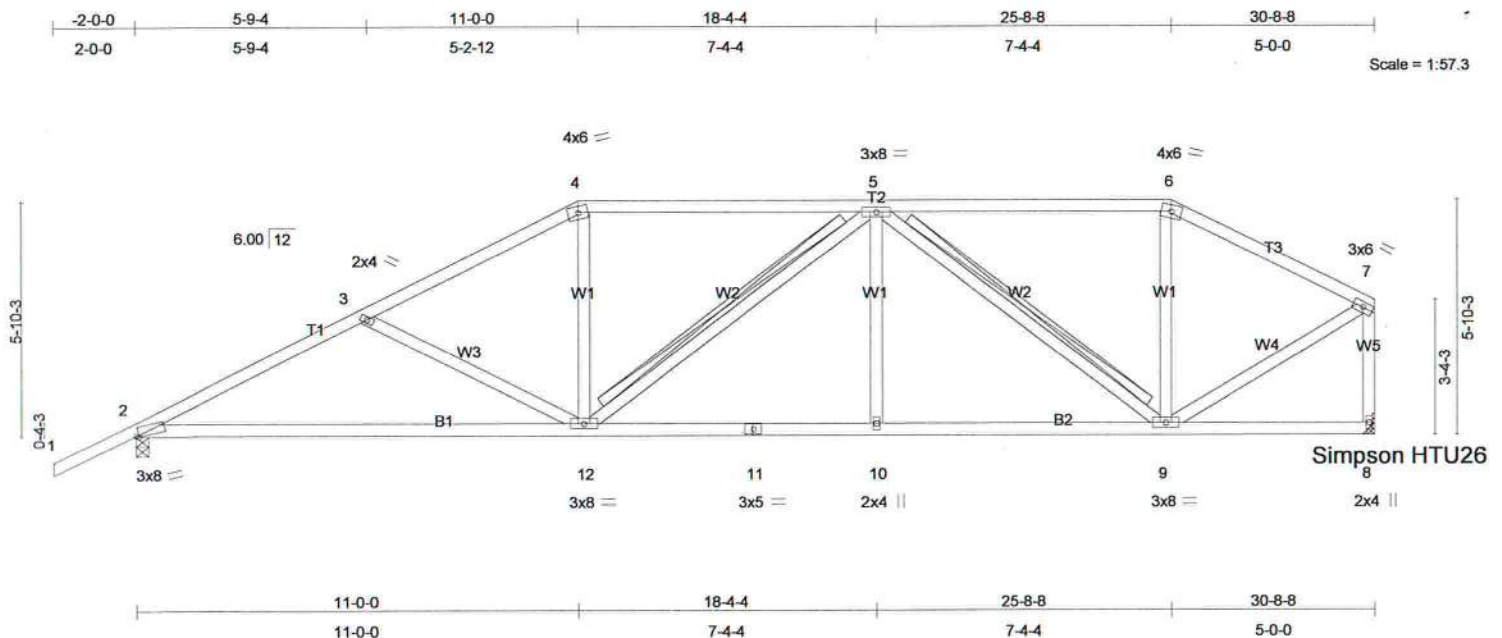


Plate Offsets (X,Y): [2:0-0-10,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.37	Vert(LL)	-0.30	2-12	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.65	Vert(TL)	-0.56	2-12	>650	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.35	Horz(TL)	0.06	8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 169 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-6-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-12, 5-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1093/0-3-8, 8=969/Mechanical
Max Horz 2=177(load case 6)
Max Uplift 2=-280(load case 6), 8=-182(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1768/934, 3-4=-1463/793, 4-5=-1262/776, 5-6=-765/506, 6-7=-901/498, 7-8=-946/531
BOT CHORD 2-12=-895/1512, 11-12=-681/1324, 10-11=-681/1324, 9-10=-681/1324, 8-9=-22/24
WEBS 3-12=-290/276, 4-12=-85/365, 5-12=-212/112, 5-10=0/190, 5-9=-741/378, 6-9=0/175, 7-9=-425/872

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Truss Design Engineer
Florida PE No. 34868
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JOINT STRESS INDEX

2 = 0.89, 3 = 0.33, 4 = 0.82, 5 = 0.56, 6 = 0.76, 7 = 0.71, 8 = 0.38, 9 = 0.81, 10 = 0.33, 11 = 0.45 and 12 = 0.56

Continued on page 2

June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T11	HIP	1	1	J1973103
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:20 2008 Page 2

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 280 lb uplift at joint 2 and 182 lb uplift at joint 8.

LOAD CASE(S) Standard

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Truss Design Engineer
Florida PE No. 24188B
1100 Coastal Bay Blvd.
Boynton Beach, FL 33435

June 13, 2008



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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T12	HIP	1	1	J1973104
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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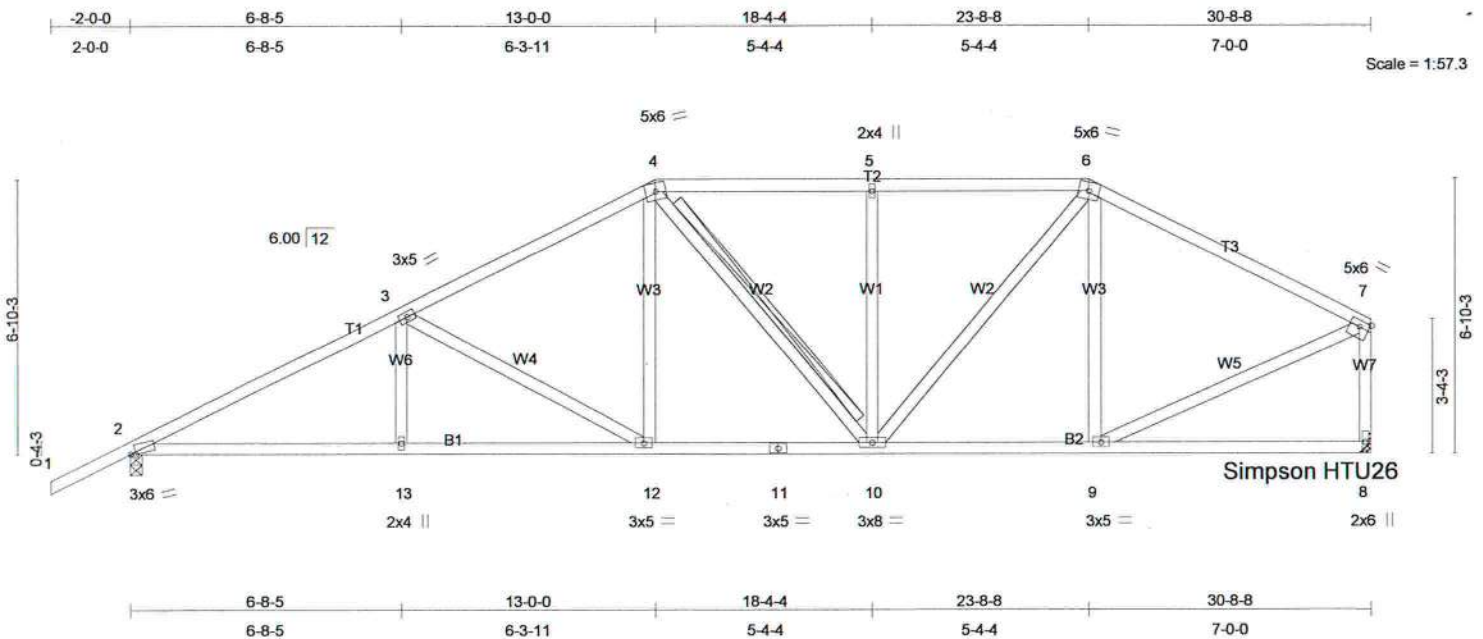


Plate Offsets (X,Y): [2:0-1-9,0-0-7], [7:Edge,0-1-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.63	Vert(LL)	0.10	12-13	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.35	Vert(TL)	-0.15	12-13	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.38	Horz(TL)	0.05	8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 180 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-5-11 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-9-8 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1093/0-3-8, 8=969/Mechanical
Max Horz 2=189(load case 6)
Max Uplift 2=-292(load case 6), 8=-158(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1781/908, 3-4=-1338/769, 4-5=-1096/720, 5-6=-1096/720, 6-7=-989/557, 7-8=-930/545
BOT CHORD 2-13=-866/1513, 12-13=-866/1513, 11-12=-577/1120, 10-11=-577/1120, 9-10=-405/818, 8-9=-58/73
WEBS 3-13=0/205, 3-12=-444/326, 4-12=-125/326, 4-10=-73/120, 5-10=-287/160, 6-10=-220/508, 6-9=-287/207, 7-9=-378/814

Julian Lee
Truss Design Engineer
Florida PE No. 34869
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Daytona Beach, FL 32105

JOINT STRESS INDEX

2 = 0.77, 3 = 0.46, 4 = 0.52, 5 = 0.33, 6 = 0.63, 7 = 0.69, 8 = 0.53, 9 = 0.52, 10 = 0.56, 11 = 0.39, 12 = 0.39 and 13 = 0.33

Continued on page 2

June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T12	HIP	1	1	J1973104
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:21 2008 Page 2

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2 and 158 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 24888
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T13	HIP	1	1	J1973105
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:21 2008 Page 1

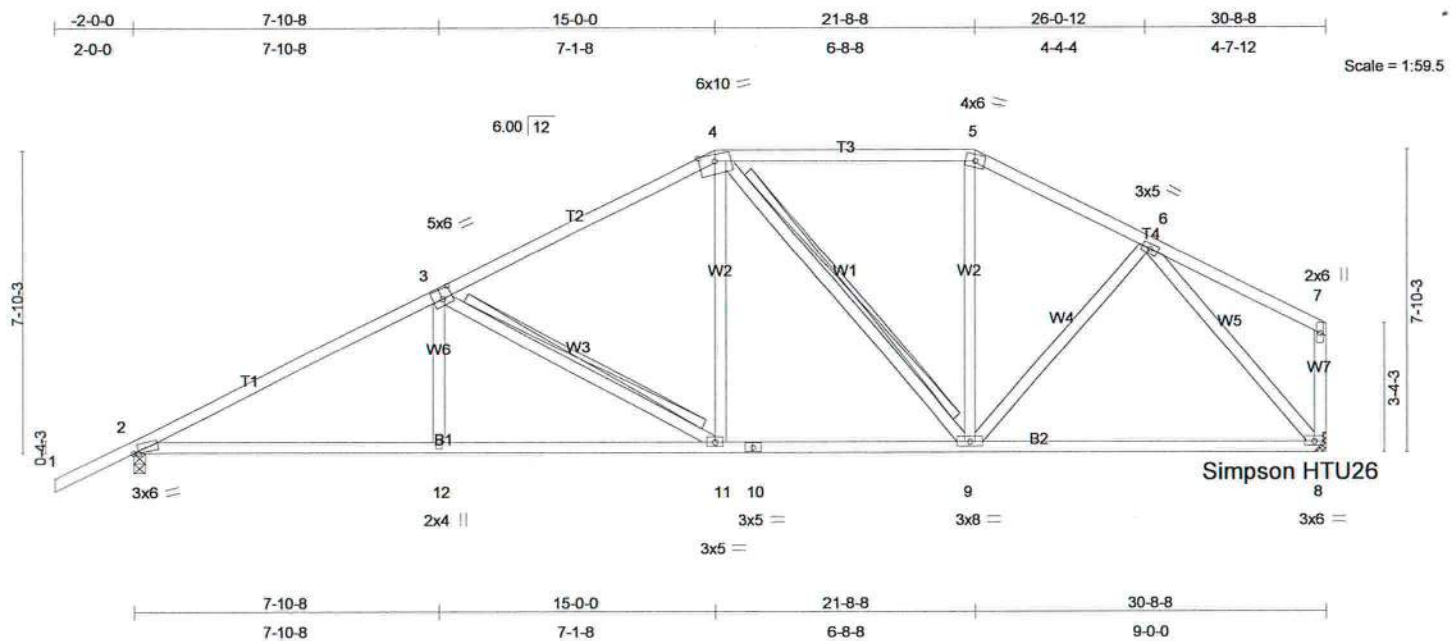


Plate Offsets (X,Y): [2:0-1-9,0-0-7], [3:0-3-0,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.62	Vert(LL)	-0.13	8-9	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.44	Vert(TL)	-0.23	8-9	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.84	Horz(TL)	0.06	8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 176 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-3-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-9-12 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-11, 4-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1093/0-3-8, 8=969/Mechanical
Max Horz 2=201(load case 6)
Max Uplift 2=-302(load case 6), 8=-172(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1742/902, 3-4=-1205/723, 4-5=-837/617, 5-6=-979/631, 6-7=-109/79, 7-8=-147/118
BOT CHORD 2-12=-846/1468, 11-12=-846/1467, 10-11=-498/1004, 9-10=-498/1004, 8-9=-382/691
WEBS 3-12=0/250, 3-11=-537/400, 4-11=-157/369, 4-9=-329/164, 5-9=-63/207, 6-9=-51/286, 6-8=-1008/582

Julius Lee
Truss Design Engineer
Florida PE No. 33869
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Daytona Beach, FL 32115

JOINT STRESS INDEX

2 = 0.76, 3 = 0.82, 4 = 0.69, 5 = 0.66, 6 = 0.43, 7 = 0.45, 8 = 0.77, 9 = 0.56, 10 = 0.33, 11 = 0.39 and 12 = 0.33

Continued on page 2

June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T13	HIP	1	1	J1973105
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:22 2008 Page 2

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 302 lb uplift at joint 2 and 172 lb uplift at joint 8.

LOAD CASE(S) Standard

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Truss Design Engineer
Florida PE No. 33858
1100 Coastal Bay Blvd.
Boynton Beach, FL 33435

June 13, 2008

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This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T14	HIP	1	1	J1973106
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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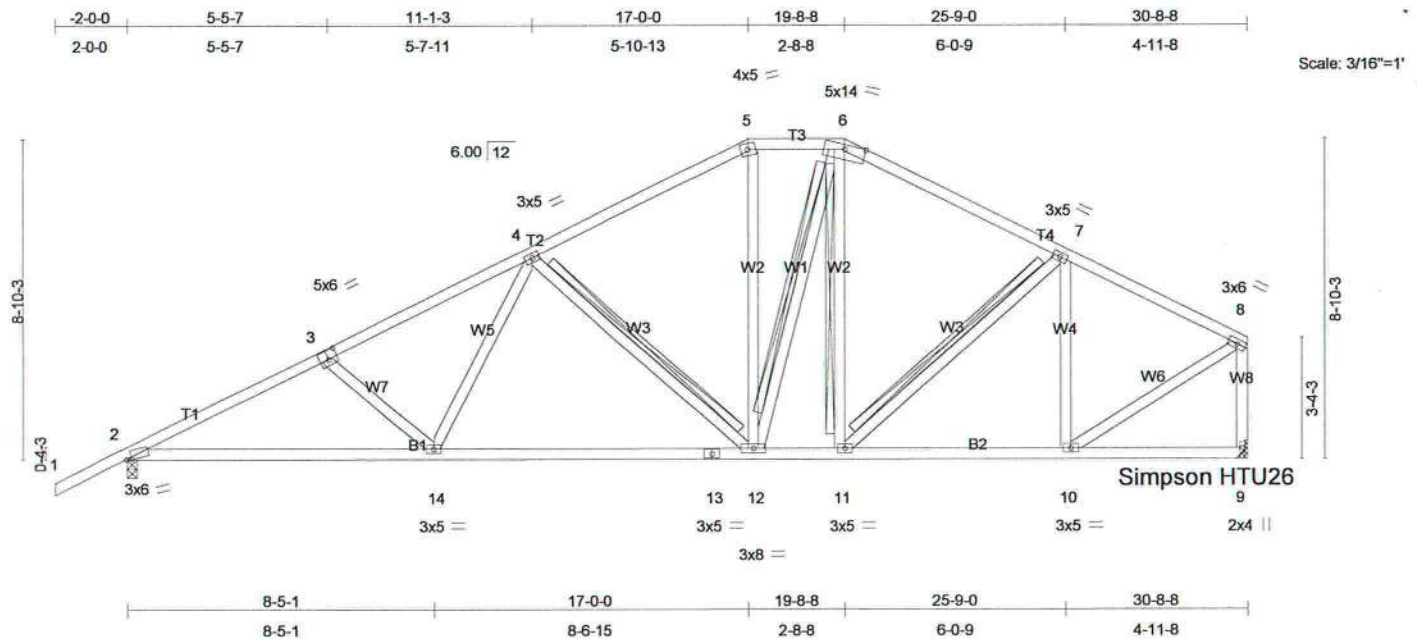


Plate Offsets (X,Y): [2:0-1-9,0-0-7], [3:0-3-0,0-3-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.32	Vert(LL)	-0.12 12-14	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.45	Vert(TL)	-0.24 12-14	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.27	Horz(TL)	0.05 9	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)					Weight: 195 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-7-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-6-5 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-12, 6-12, 6-11, 7-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1093/0-3-8, 9=969/Mechanical
Max Horz 2=213(load case 6)
Max Uplift 2=-310(load case 6), 9=-185(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1798/964, 3-4=-1580/900, 4-5=-1039/689, 5-6=-864/679,
6-7=-978/654, 7-8=-880/516, 8-9=-944/557
BOT CHORD 2-14=-927/1538, 13-14=-697/1231, 12-13=-697/1231, 11-12=-372/806,
10-11=-402/743, 9-10=-21/28
WEBS 3-14=-253/246, 4-14=-115/361, 4-12=-504/405, 5-12=-140/242, 6-12=-104/322,
6-11=-78/52, 7-11=-39/167, 7-10=-409/290, 8-10=-454/851

Julius Lee
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JOINT STRESS INDEX

2 = 0.77, 3 = 0.39, 4 = 0.46, 5 = 0.71, 6 = 0.62, 7 = 0.46, 8 = 0.69, 9 = 0.41, 10 = 0.56, 11 = 0.39, 12 = 0.66, 13 = 0.40 and 14

Continued on page 2

June 13, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T14	HIP	1	1	J1973106
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 310 lb uplift at joint 2 and 185 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida P.E. No. 34888
1100 Coastal Bay Blvd.
Boynton Beach, FL 33435

June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T15	COMMON	1	1	J1973107
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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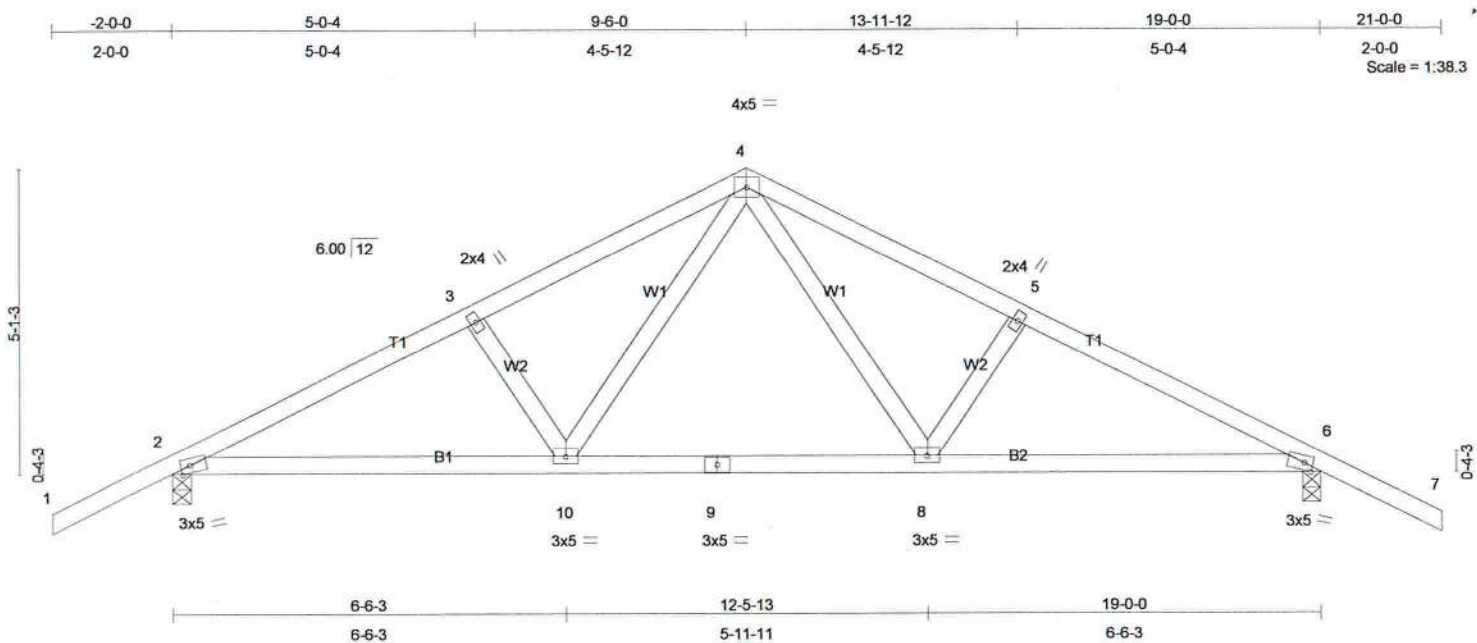


Plate Offsets (X,Y): [2:0-1-12,Edge], [6:0-1-12,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.34	Vert(LL)	0.15	8-10	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.51	Vert(TL)	-0.22	8-10	>999	240		
BCLL 10.0	* Rep Stress Incr	NO	WB 0.16	Horz(TL)	0.03	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 91 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-2-7 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 8-9-0 oc bracing.

REACTIONS (lb/size) 2=894/0-3-8, 6=894/0-3-8
 Max Horz 2=92(load case 6)
 Max Uplift 2=-276(load case 6), 6=-276(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/47, 2-3=-1416/749, 3-4=-1280/749, 4-5=-1280/749, 5-6=-1416/749, 6-7=0/47
 BOT CHORD 2-10=-502/1196, 9-10=-262/838, 8-9=-262/838, 6-8=-502/1196
 WEBS 3-10=-199/187, 4-10=-267/506, 4-8=-267/506, 5-8=-199/187

JOINT STRESS INDEX

2 = 0.76, 3 = 0.33, 4 = 0.62, 5 = 0.33, 6 = 0.76, 8 = 0.42, 9 = 0.80 and 10 = 0.42

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

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 Truss Design Engineer
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 Boynton Beach, FL 33435

June 13,2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T15	COMMON	1	1	J1973107
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:23 2008 Page 2

NOTES

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 276 lb uplift at joint 2 and 276 lb uplift at joint 6.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70(F=-60), 6-8=-10

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1100 Coastal Bay Blvd.
Boynton Beach, FL 33435

June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T15G	GABLE	1	1	J1973108
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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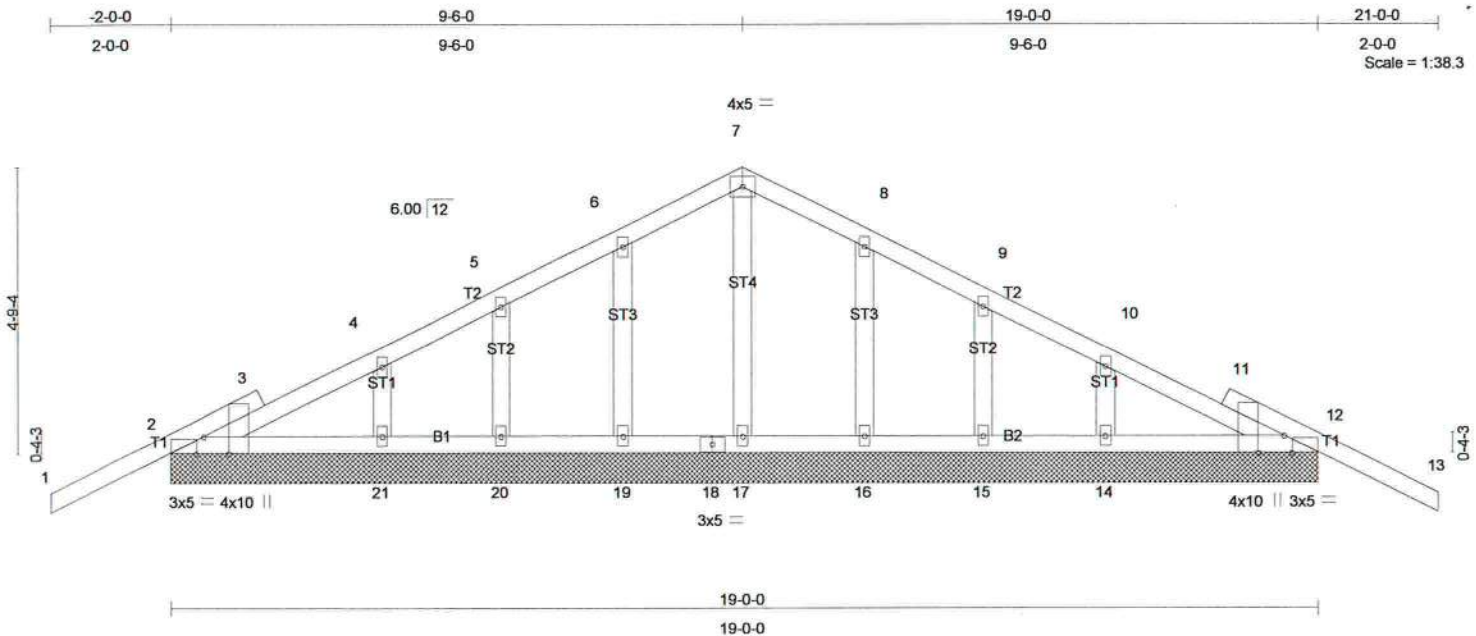


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-1-8,Edge], [12:0-3-8,Edge], [12:0-1-8,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.26	Vert(LL)	-0.02	13	n/r	120	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.05	Vert(TL)	-0.03	13	n/r	90		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.04	Horz(TL)	0.00	12	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 96 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=239/19-0-0, 12=239/19-0-0, 17=129/19-0-0, 19=132/19-0-0, 20=115/19-0-0, 21=166/19-0-0, 16=132/19-0-0, 15=115/19-0-0, 14=166/19-0-0

Max Horz 2=-100(load case 7)

Max Uplift 2=-185(load case 6), 12=-200(load case 7), 19=-81(load case 6), 20=-98(load case 6), 21=-65(load case 7), 16=-80(load case 7), 15=-97(load case 7), 14=-68(load case 7)

Max Grav 2=239(load case 10), 12=239(load case 11), 17=129(load case 1), 19=136(load case 10), 20=115(load case 1), 21=166(load case 10), 16=136(load case 11), 15=115(load case 1), 14=166(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-61/37, 3-4=-52/50, 4-5=-24/88, 5-6=-20/138, 6-7=-23/191, 7-8=-23/191, 8-9=-20/138, 9-10=-24/78, 10-11=-19/38, 11-12=-32/15, 12-13=0/47
BOT CHORD 2-21=-7/114, 20-21=-7/114, 19-20=-7/114, 18-19=-7/114, 17-18=-7/114, 16-17=-7/114, 15-16=-7/114, 14-15=-7/114, 12-14=-7/114
WEBS 7-17=-110/0, 6-19=-115/102, 5-20=-99/117, 4-21=-137/102, 8-16=-115/102, 9-15=-99/117, 10-14=-137/102

Julius Lee
Truss Design Engineer
Florida PE No. 24868
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

Continued on page 2

June 13, 2008

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This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T15G	GABLE	1	1	J1973108
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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JOINT STRESS INDEX

2 = 0.49, 2 = 0.18, 3 = 0.00, 3 = 0.00, 4 = 0.33, 5 = 0.33, 6 = 0.33, 7 = 0.24, 8 = 0.33, 9 = 0.33, 10 = 0.33, 11 = 0.00, 11 = 0.00, 12 = 0.49, 12 = 0.18, 14 = 0.33, 15 = 0.33, 16 = 0.33, 17 = 0.33, 18 = 0.18, 19 = 0.33, 20 = 0.33 and 21 = 0.33

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 185 lb uplift at joint 2, 200 lb uplift at joint 12, 81 lb uplift at joint 19, 98 lb uplift at joint 20, 65 lb uplift at joint 21, 80 lb uplift at joint 16, 97 lb uplift at joint 15 and 68 lb uplift at joint 14.

LOAD CASE(S) Standard

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Florida PE No. 21898
1109 Coastal Bay Blvd
Boynton Beach, FL 33426

June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T16	COMMON	3	1	J1973109
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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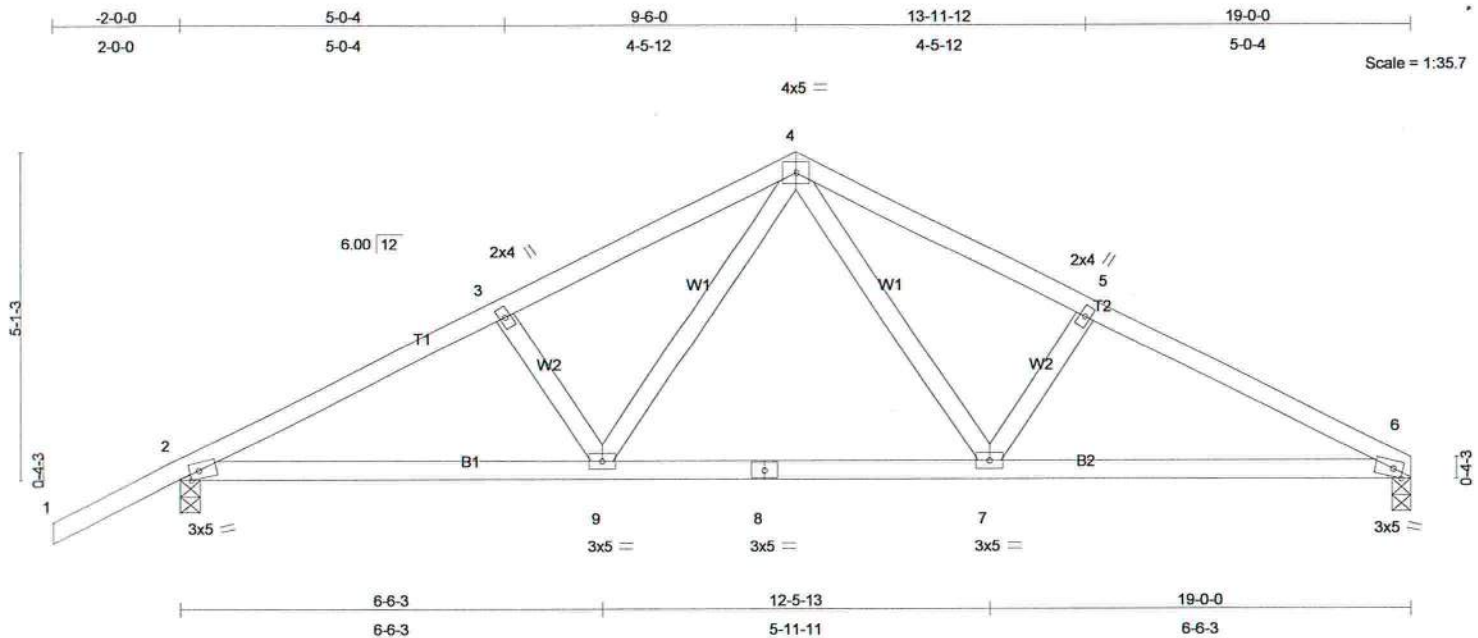


Plate Offsets (X,Y): [2:0-1-12,Edge], [6:0-1-12,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.34	Vert(LL)	0.14	7-9	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.54	Vert(TL)	-0.22	7-9	>999	240		
BCLL 10.0	* Rep Stress Incr	NO	WB 0.19	Horz(TL)	0.04	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 88 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-1-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-8-0 oc bracing.

REACTIONS (lb/size) 6=771/0-3-8, 2=900/0-3-8
Max Horz 2=105(load case 6)
Max Uplift 6=-179(load case 7), 2=-277(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1429/771, 3-4=-1293/771, 4-5=-1318/813, 5-6=-1456/815
BOT CHORD 2-9=-600/1208, 8-9=-361/851, 7-8=-361/851, 6-7=-651/1239
WEBS 3-9=-199/189, 4-9=-261/506, 4-7=-327/543, 5-7=-218/219

JOINT STRESS INDEX

2 = 0.77, 3 = 0.33, 4 = 0.59, 5 = 0.33, 6 = 0.77, 7 = 0.42, 8 = 0.79 and 9 = 0.42

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 34868
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

June 13,2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T16	COMMON	3	1	J1973109
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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NOTES

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 179 lb uplift at joint 6 and 277 lb uplift at joint 2.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 4-6=-54, 2-9=-10, 7-9=-70(F=-60), 6-7=-10

Julius Lee
Truss Design Engineer
Florida P.E. No. 24888
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June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T17	HOWE	1	2	J1973110
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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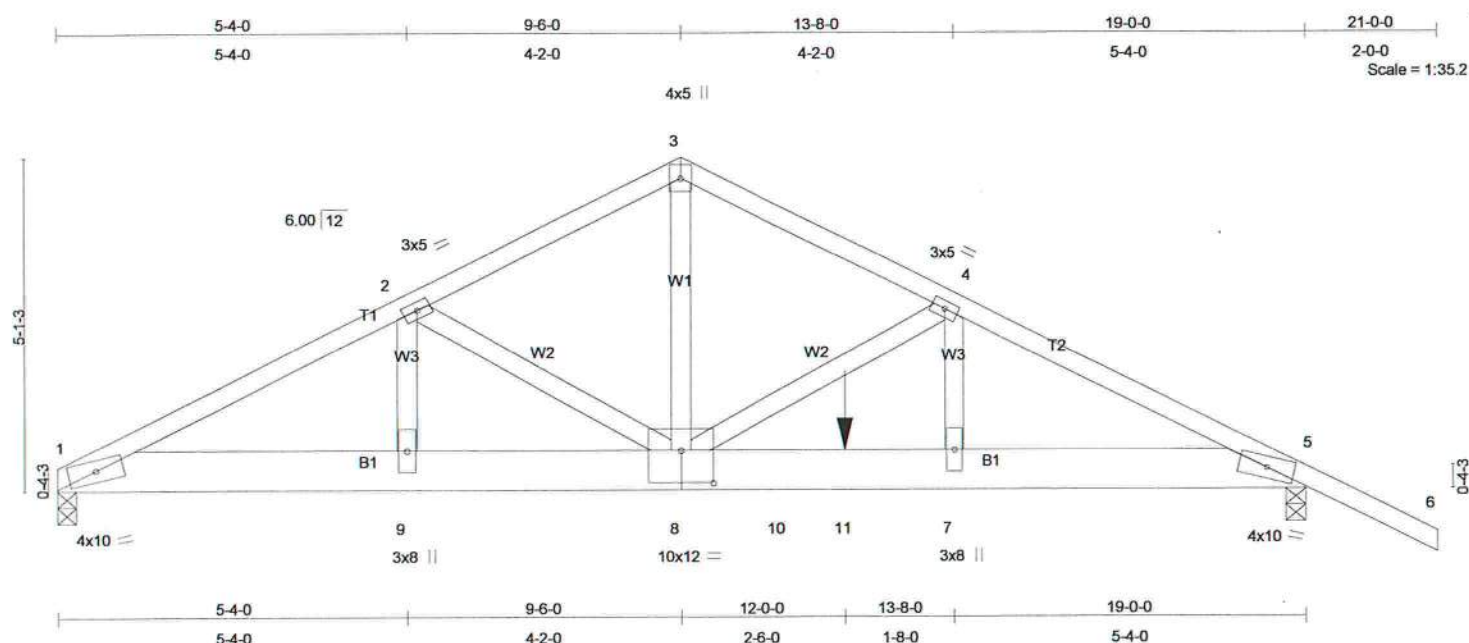


Plate Offsets (X,Y): [8:0-6-0,0-6-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.31	Vert(LL)	-0.11	7-8	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.43	Vert(TL)	-0.21	7-8	>999	240		
BCLL 10.0	* Rep Stress Incr	NO	WB 0.46	Horz(TL)	0.04	5	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 239 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 8 SYP 2400F 2.0E
 WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-6-6 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=4884/0-3-8, 5=3516/0-3-8
 Max Horz 1=-111(load case 6)
 Max Uplift 1=-1317(load case 5), 5=-1001(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-8132/2180, 2-3=-5888/1594, 3-4=-5876/1603, 4-5=-7289/1908, 5-6=0/53
 BOT CHORD 1-9=-1947/7255, 8-9=-1947/7255, 8-10=-1661/6476, 10-11=-1661/6476,
 7-11=-1661/6476, 5-7=-1661/6476
 WEBS 2-9=-545/1961, 2-8=-2393/709, 3-8=-1342/4963, 4-8=-1479/428, 4-7=-278/1151

JOINT STRESS INDEX

1 = 0.70, 2 = 0.83, 3 = 0.70, 4 = 0.83, 5 = 0.70, 7 = 0.31, 8 = 0.46 and 9 = 0.31

NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

Continued on page 2

Julius Lee
 Truss Design Engineer
 Florida PE No. 34868
 1100 Coastal Bay Blvd
 Boynton Beach, FL 33435

June 13, 2008

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T17	HOWE	1	2	J1973110
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:27 2008 Page 2

NOTES

- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1317 lb uplift at joint 1 and 1001 lb uplift at joint 5.

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-54, 1-10=-465(B=-455), 5-10=-10
Concentrated Loads (lb)
Vert: 11=-2151(F)

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Truss Design Engineer
Florida P.E. No. 21888
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T18	HIP	1	1	J1973111
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:27 2008 Page 1

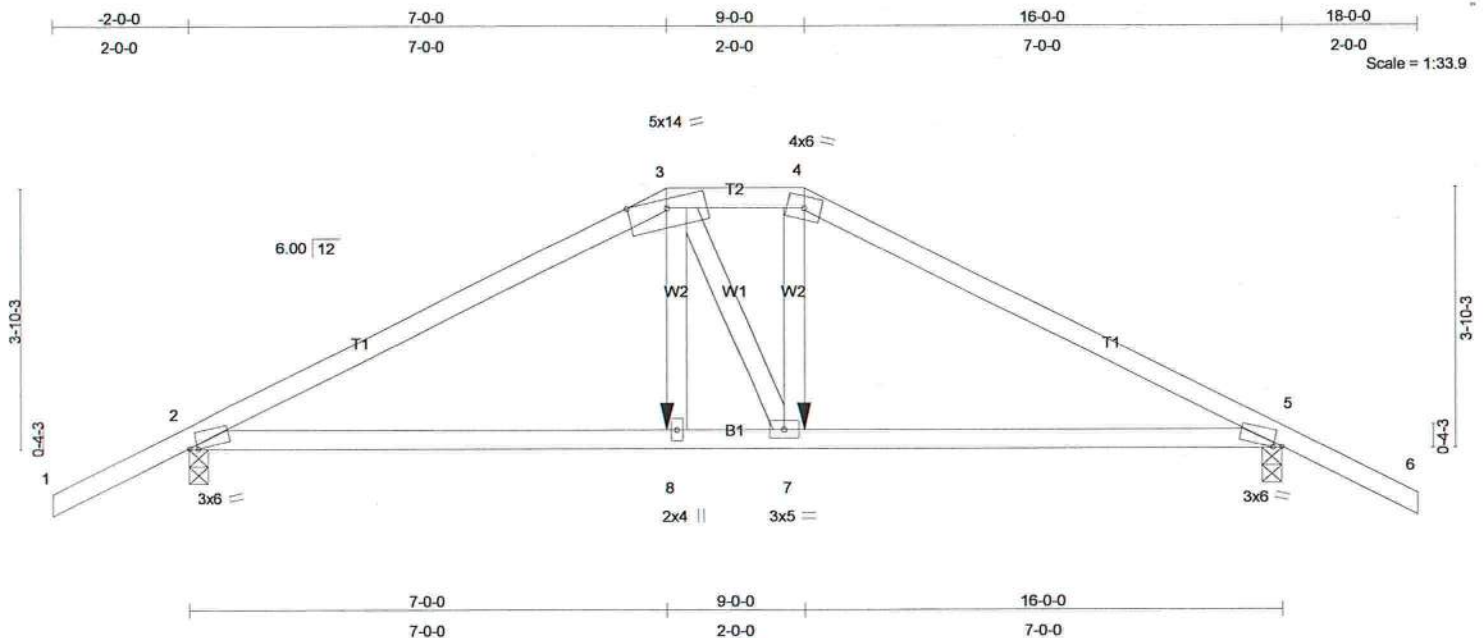


Plate Offsets (X,Y): [2:0-1-8,0-0-7], [5:0-1-8,0-0-7]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.40	Vert(LL)	0.12	2-8	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.42	Vert(TL)	-0.14	2-8	>999	240		
BCLL 10.0	* Rep Stress Incr	NO	WB 0.19	Horz(TL)	0.04	5	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 72 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-4-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-5-6 oc bracing.

REACTIONS (lb/size) 2=1103/0-3-8, 5=1103/0-3-8
Max Horz 2=77(load case 5)
Max Uplift 2=-595(load case 5), 5=-595(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1778/804, 3-4=-1526/770, 4-5=-1781/805, 5-6=0/47
BOT CHORD 2-8=-674/1503, 7-8=-684/1523, 5-7=-658/1506
WEBS 3-8=-262/480, 3-7=-146/159, 4-7=-303/592

JOINT STRESS INDEX

2 = 0.77, 3 = 0.87, 4 = 0.76, 5 = 0.77, 7 = 0.43 and 8 = 0.34

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

Julian Lee
Truss Design Engineer
Florida P.E. No. 24888
1109 Coastal Bay Blvd
Boynton Beach, FL 33435

June 13, 2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T18	HIP	1	1	J1973111
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 595 lb uplift at joint 2 and 595 lb uplift at joint 5.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-8=-10, 7-8=-22(F=-12), 5-7=-10

Concentrated Loads (lb)

Vert: 8=-411(F) 7=-411(F)

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Truss Design Engineer
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June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T19	QUEENPOST	2	1	J1973112
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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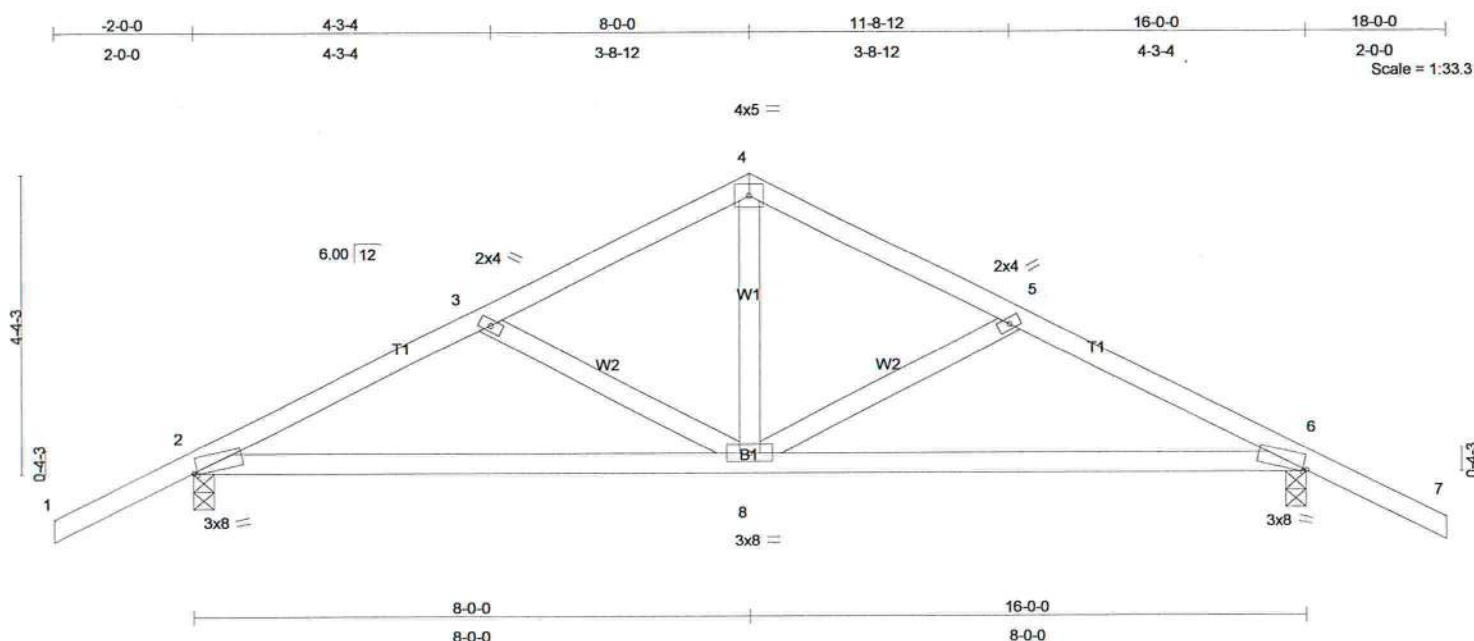


Plate Offsets (X,Y): [2:0-0-10,Edge], [6:0-0-10,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.29	Vert(LL)	0.19	2-8	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.32	Vert(TL)	-0.12	2-8	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.20	Horz(TL)	-0.02	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 75 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-7-15 oc bracing.

REACTIONS (lb/size) 2=619/0-3-8, 6=619/0-3-8
Max Horz 2=83(load case 6)
Max Uplift 2=-404(load case 6), 6=-404(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-802/1064, 3-4=-590/942, 4-5=-590/942, 5-6=-802/1064, 6-7=0/47
BOT CHORD 2-8=-810/663, 6-8=-810/663
WEBS 3-8=-217/243, 4-8=-670/331, 5-8=-217/243

JOINT STRESS INDEX

2 = 0.74, 3 = 0.13, 4 = 0.43, 5 = 0.13, 6 = 0.74 and 8 = 0.17

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Law
Truss Design Engineer
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June 13,2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T19	QUEENPOST	2	1	J1973112
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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NOTES

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 404 lb uplift at joint 2 and 404 lb uplift at joint 6.

LOAD CASE(S) Standard

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Truss Design Engineer
Florida PE No. 2-18889
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Boynton Beach, FL 33435

June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T20	COMMON	1	2	J1973113
Builders FirstSource, Lake City, FL 32055					6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:29 2008 Page 1
					Job Reference (optional)

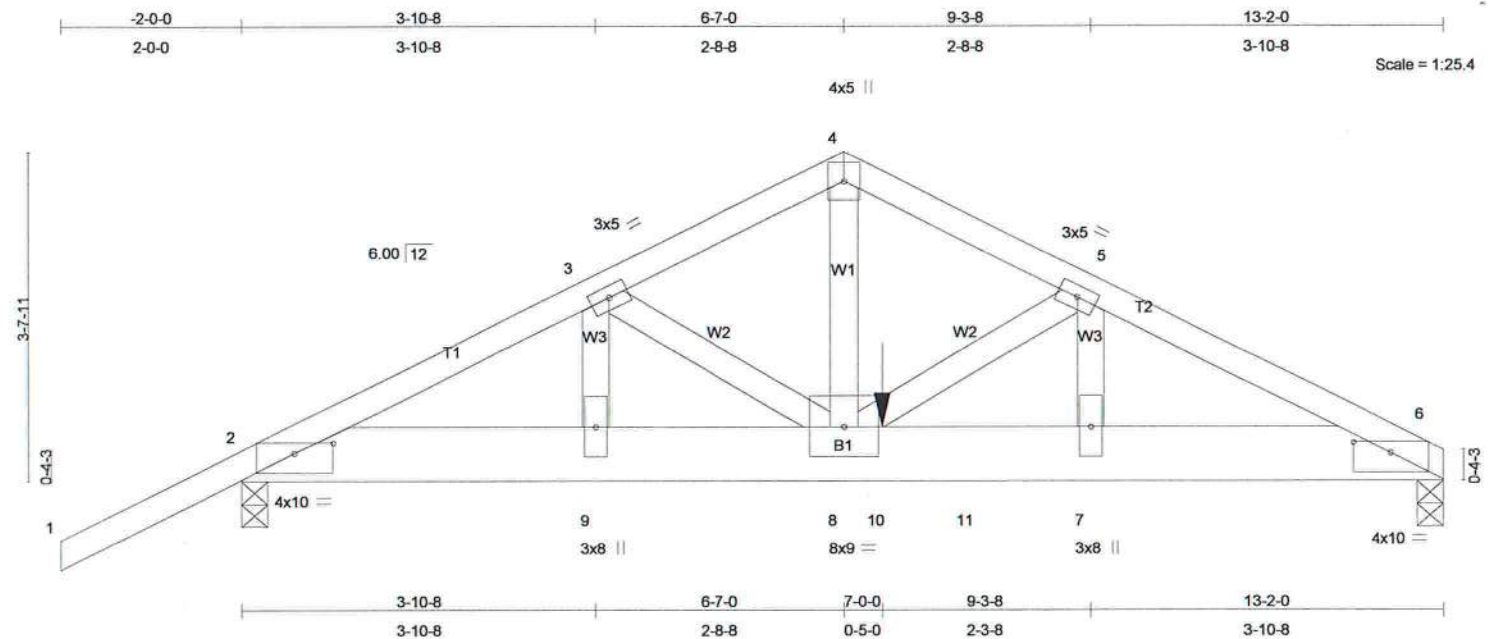


Plate Offsets (X,Y): [2:0-5-0,0-1-7], [6:0-5-0,0-1-7]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.22	Vert(LL)	-0.06	7-8	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.28	Vert(TL)	-0.11	7-8	>999	240		
BCLL 10.0	* Rep Stress Incr	NO	WB 0.35	Horz(TL)	0.02	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 166 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 8 SYP 2400F 2.0E
 WEBS 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-2-10 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=3999/0-3-8, 2=2281/0-3-8
 Max Horz 2=94(load case 5)
 Max Uplift 6=-1083(load case 6), 2=-673(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/54, 2-3=-4266/1115, 3-4=-4536/1241, 4-5=-4533/1234, 5-6=-6364/1715
 BOT CHORD 2-9=-969/3785, 8-9=-969/3785, 8-10=-1506/5684, 10-11=-1506/5684,
 7-11=-1506/5684, 6-7=-1506/5684
 WEBS 3-9=-469/184, 3-8=-113/411, 4-8=-1041/3829, 5-8=-1989/583, 5-7=-457/1734

JOINT STRESS INDEX

2 = 0.70, 3 = 0.74, 4 = 0.54, 5 = 0.74, 6 = 0.70, 7 = 0.27, 8 = 0.35 and 9 = 0.27

NOTES

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

Continued on page 2

Julius Lee
 Truss Design Engineer
 Florida PE No. 34858
 1100 Coastal Bay Blvd.
 Boynton Beach, FL 33435

June 13,2008

Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.	J1973113
L280515	T20	COMMON	1	2	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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NOTES

- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 5) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1083 lb uplift at joint 6 and 673 lb uplift at joint 2.

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 4-6=-54, 2-11=-10, 6-11=-561(F=-551)

Concentrated Loads (lb)

Vert: 10=-2575(F)

Julius Lee
Truss Design Engineer
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1109 Coastal Bay Blvd.
Boynton Beach, FL 33426

June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T20G	GABLE	1	1	J1973114
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

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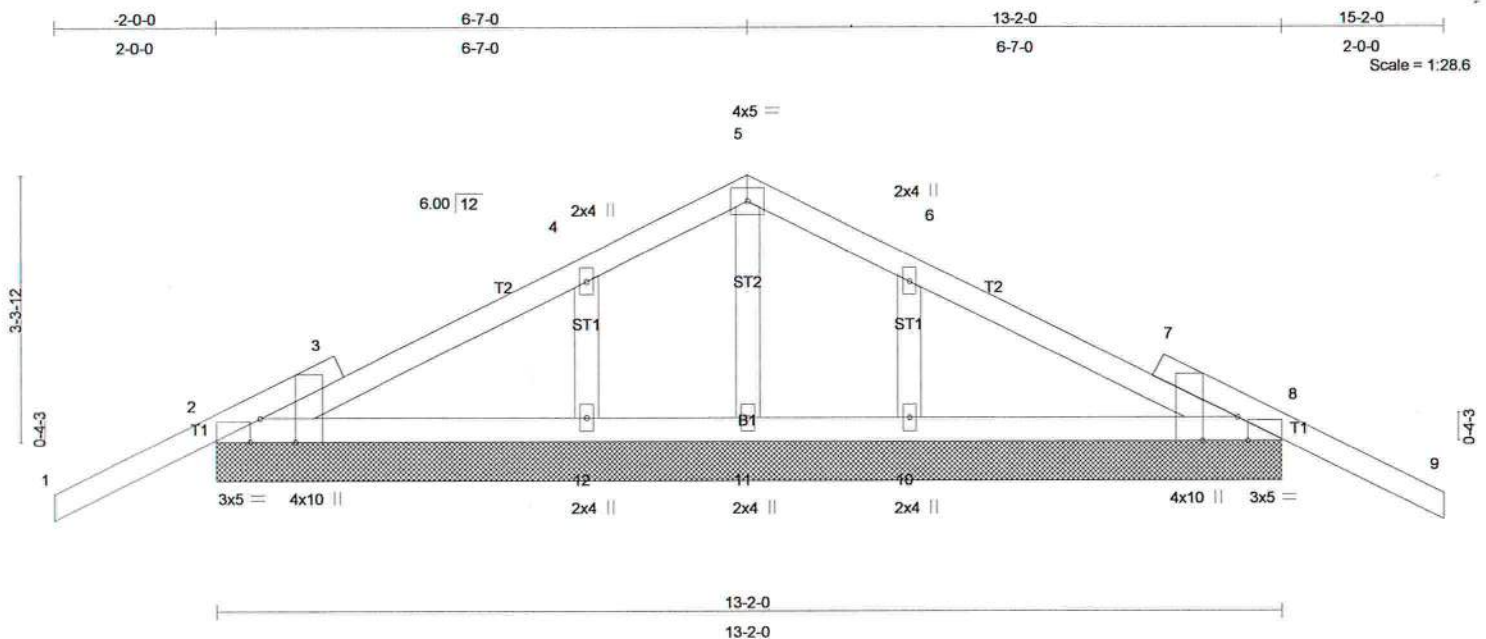


Plate Offsets (X,Y): [2:0-3-8,Edge], [2:0-1-8,Edge], [8:0-3-8,Edge], [8:0-1-8,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.26	Vert(LL)	-0.01	9	n/r	120	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.09	Vert(TL)	-0.01	9	n/r	90		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.05	Horz(TL)	0.00	8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 61 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=223/13-2-0, 8=223/13-2-0, 11=117/13-2-0, 12=247/13-2-0, 10=247/13-2-0
Max Horz 2=80(load case 6)
Max Uplift 2=-188(load case 6), 8=-199(load case 7), 12=-125(load case 6), 10=-129(load case 7)
Max Grav 2=235(load case 10), 8=235(load case 11), 11=117(load case 1), 12=249(load case 10), 10=249(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-64/59, 3-4=-47/142, 4-5=0/97, 5-6=0/97, 6-7=-23/142, 7-8=-35/59, 8-9=0/47
BOT CHORD 2-12=-84/143, 11-12=-84/143, 10-11=-84/143, 8-10=-84/143
WEBS 5-11=-123/5, 4-12=-198/186, 6-10=-198/186

Julius Lee
Truss Design Engineer
Florida PE No. 34868
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

JOINT STRESS INDEX

2 = 0.49, 2 = 0.00, 3 = 0.00, 3 = 0.00, 4 = 0.10, 5 = 0.05, 6 = 0.10, 7 = 0.00, 7 = 0.00, 8 = 0.49, 8 = 0.00, 10 = 0.10, 11 = 0.04 and 12 = 0.10

NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

June 13, 2008

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Job	Truss	Truss Type	Qty	Ply	JOHN NORRIS - HOWELL RES.
L280515	T20G	GABLE	1	1	J1973114
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Fri Jun 13 15:58:30 2008 Page 2

NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 188 lb uplift at joint 2, 199 lb uplift at joint 8, 125 lb uplift at joint 12 and 129 lb uplift at joint 10.

LOAD CASE(S) Standard

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June 13, 2008

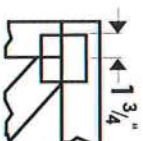
Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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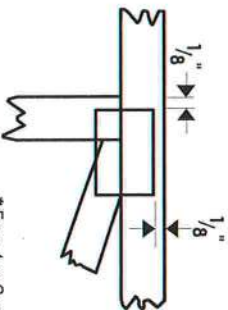


Symbols

PLATE LOCATION AND ORIENTATION



* Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



* For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



* This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

4 X 4

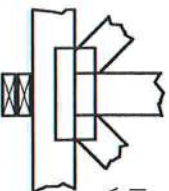
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



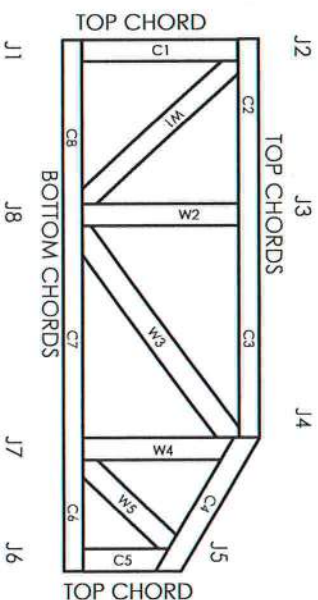
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561

General Safety Notes

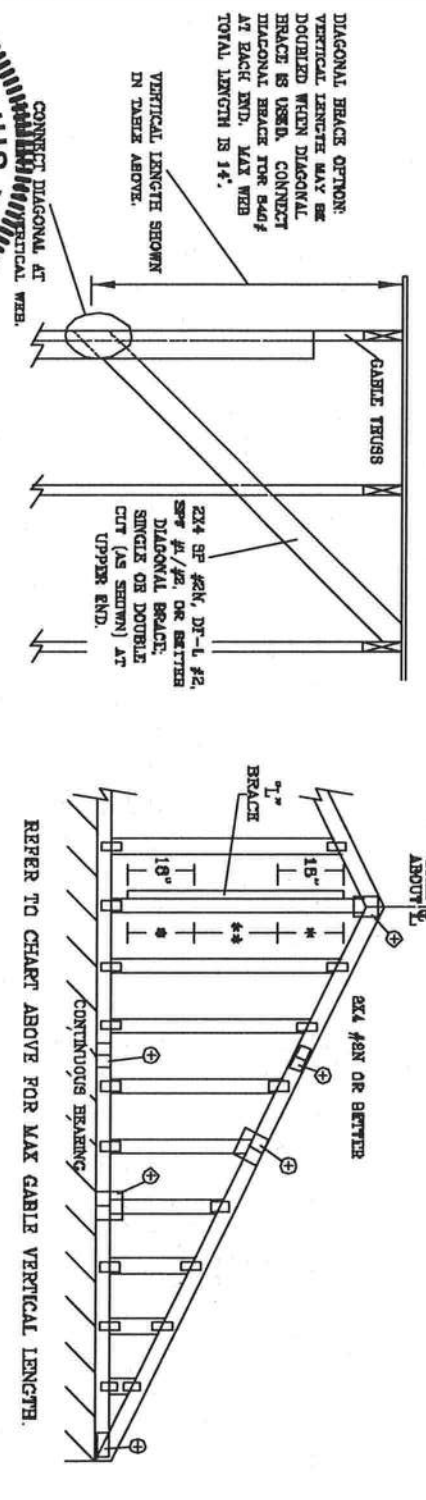
Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at 1/4 panel length ($\pm 6"$ from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.



MAX GABLE VERTICAL LENGTH		2x4		BRACE		NO		(1) 1x4 T" BRACE *		(1) 2x4 T" BRACE *		(2) 2x4 T" BRACE **		(1) 2x6 T" BRACE *		(2) 2x6 T" BRACE *		(2) 2x8 T" BRACE **	
CABLE VERTICAL SPACING	SPECIES	GRADE	BRACES	SP	HF	SPF	DFL	SP	HF	SPF	DFL	SP	HF	SPF	DFL	SP	HF	SPF	DFL
12" O.C.	SPF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"	13' 3"	13' 3"	13' 3"	13' 3"	13' 3"
				4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	8' 3"	10' 10"	10' 10"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"
				4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	8' 3"	10' 10"	10' 10"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"
				4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	8' 3"	10' 10"	10' 10"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"
16" O.C.	SPF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"	13' 3"	13' 3"	13' 3"	13' 3"	13' 3"
				4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	8' 3"	10' 10"	10' 10"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"
				4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	8' 3"	10' 10"	10' 10"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"
				4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	8' 3"	10' 10"	10' 10"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"
24" O.C.	SPF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"	13' 3"	13' 3"	13' 3"	13' 3"	13' 3"
				4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	8' 3"	10' 10"	10' 10"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"
				4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	8' 3"	10' 10"	10' 10"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"
				4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	8' 3"	10' 10"	10' 10"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"	12' 11"

BRACING GROUP SPECIES AND GRADES:			
GROUP A:		GROUP B:	
SPRUCE-PINE-TYR	HEM-FIR	SOUTHERN PINE	HEM-FIR
#1 / #2	#2	#3	#1 & BTR
STANDARD	STANDARD	STUD	STUD
#3	#3	STANDARD	STANDARD
STUD			
DOUGLAS FIR-LARCH			
#3			
STUD			
STANDARD			



REVIEWED
By Julius Lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
MIRAMAR BEACH, FL 33444-8161

REF: ASCE 7-02-CAB13015
DATE: 11/26/03
DRWG: MIXX STD CABLE IS I ET
-ENG

MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

2X4 CABLE VERTICAL SPECIES		BRACE NO.	BRACE		(1) 1X4 1" BRACE *		(1) 2X4 1" BRACE *		(2) 2X4 1" BRACE *		(1) 2X6 1" BRACE *		(2) 2X8 1" BRACE *		(2) 2X8 1" BRACE **	
GRADE	BRACE		GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
SPF	#1 / #2	3' 2"	5' 6"	6' 8"	6' 6"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"	12' 3"	12' 7"	12' 3"	12' 7"
SPF	#3	3' 1"	4' 5"	4' 5"	6' 10"	7' 10"	7' 10"	8' 1"	9' 1"	9' 1"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"
HF	STANDARD	3' 1"	4' 6"	4' 5"	5' 10"	6' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"	12' 3"
SP	#1	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"	12' 3"	13' 2"	12' 3"	13' 2"
SP	#2	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"	12' 3"	13' 2"	12' 3"	13' 2"
DFL	#3	3' 3"	4' 8"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 6"	12' 3"	12' 6"	12' 3"	12' 6"
DFL	STANDARD	3' 0"	3' 10"	3' 10"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 6"	12' 3"	12' 6"	12' 3"	12' 6"
SPF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
SPF	#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	9' 3"	11' 1"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
HF	STANDARD	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	9' 3"	11' 1"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
SP	#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
SP	#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
DFL	#3	3' 8"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
DFL	STANDARD	3' 8"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
SPF	#1 / #2	4' 0"	6' 11"	7' 2"	8' 3"	8' 3"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
SPF	#3	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
HF	STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	8' 3"	9' 6"	11' 1"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
SP	#1	4' 4"	6' 11"	7' 6"	8' 3"	8' 3"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
SP	#2	4' 4"	6' 11"	7' 6"	8' 3"	8' 3"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
DFL	#3	4' 2"	6' 4"	6' 4"	8' 3"	8' 3"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
DFL	STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	8' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

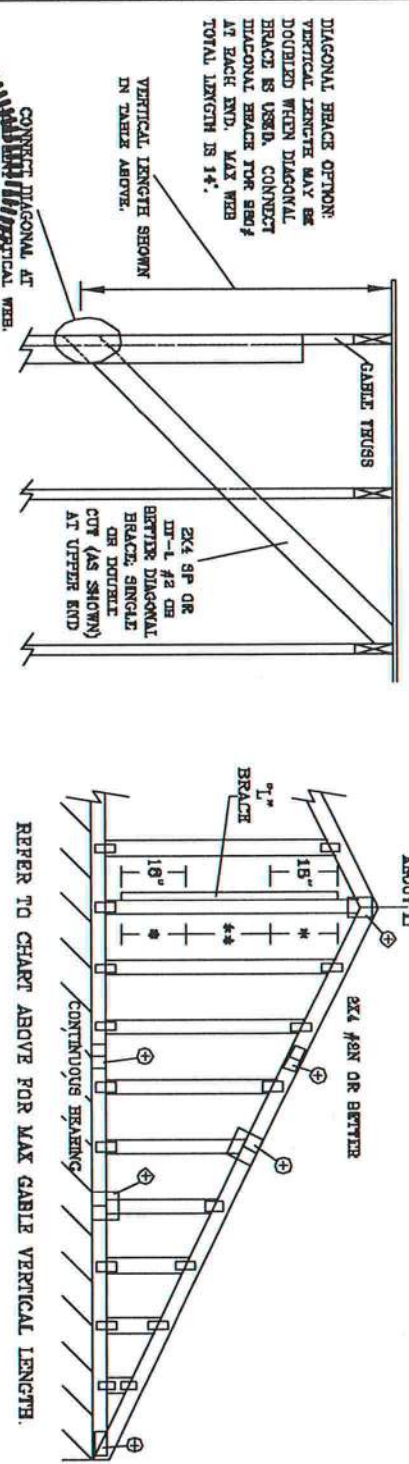
BRACING GROUP SPECIES AND GRADES:			
GROUP A:		GROUP B:	
SPRUCES-PINE-2B	HEM-FIR	SPRUCES-PINE-2B	HEM-FIR
#1 / #2	STANDARD	#1 / #2	STANDARD
#3	STUD	#3	STUD
STANDARD	STANDARD	STANDARD	STANDARD

CABLE TRUSS DETAIL NOTES:

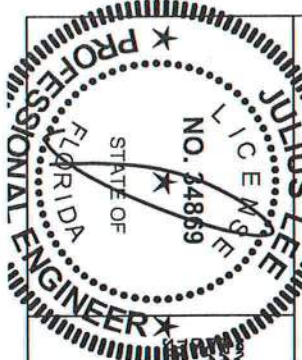
LIVE LOAD DEFLECTION CRITERIA IS $L/240$.
 PROVIDE UPLIFT CONNECTIONS FOR 160 PSF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).
 CABLE END SUPPORTS LOAD FROM 4" O" OUTLOOKERS WITH 2" O" OVERHANG, OR 12" PLYWOOD OVERHANG.
 ATTACH EACH 1" BRACE WITH 104 NAILS.
 * FOR (1) 1" BRACE, SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
 ** FOR (2) 1" BRACES, SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.
 1" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO. SPICES
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0" BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.



CONTRACTOR: TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING, AND MAINTAINING. REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ROOF CEILING.

REVIEWED
 By Julius Lee at 12:00 pm, Jun 11, 2008

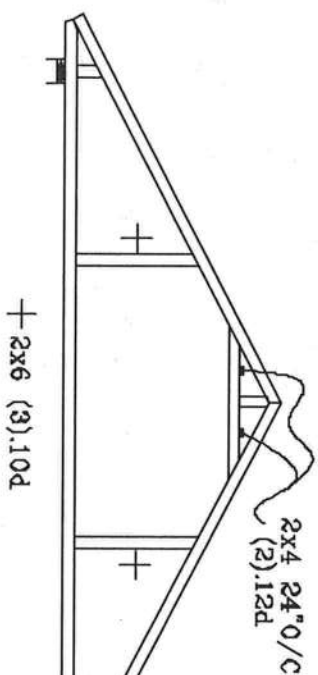
JULIUS LEE'S
 CONS. ENGINEERS P.A.
 1456 SW 4th Avenue
 DEERFIELD BEACH, FL 33444-6211

No. 34869
 STATE OF FLORIDA

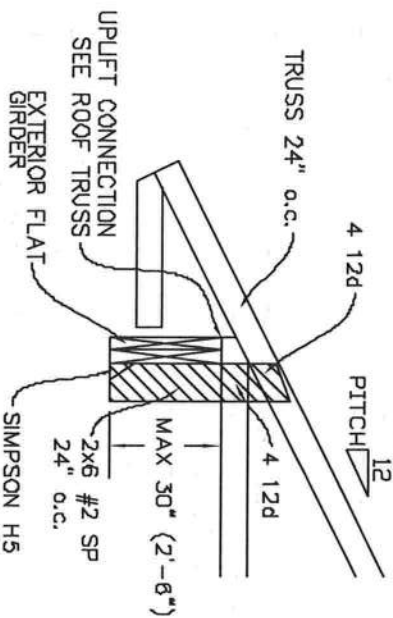
MAX. TOT. LD. 60 PSF
 MAX. SPACING 24.0"

REF ASCE7-02-GAB13030
 DATE 11/26/03
 DWG DATE STD GABLE 90' x 17'
 -ENG

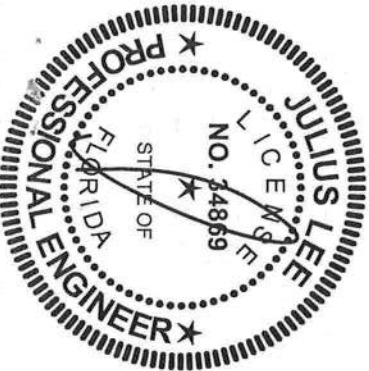
TYPICAL ATTIC TRUSS BRACING



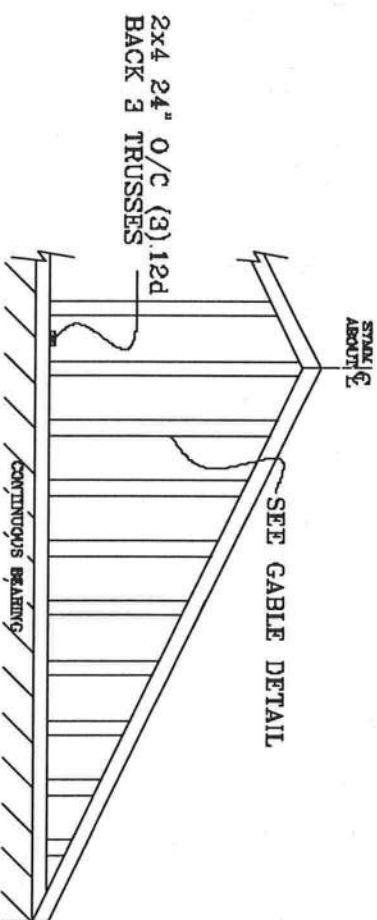
TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

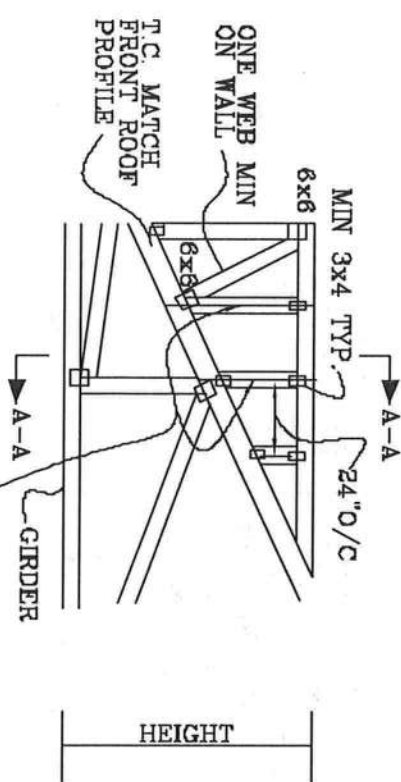


CABLE END TRUSS DETAIL



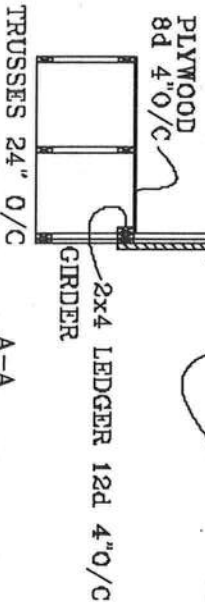
MINIMUM BRACING ON GABLE TRUSSES. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



SEE ROOF TRUSSES FOR UPLIFT
ROOF 24" O/C

SEE CABLE END DETAIL FOR T-BRACE BEHIND EACH VERTICAL



JULIUS LEE'S
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No. 34869
STATE OF FLORIDA

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #8 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

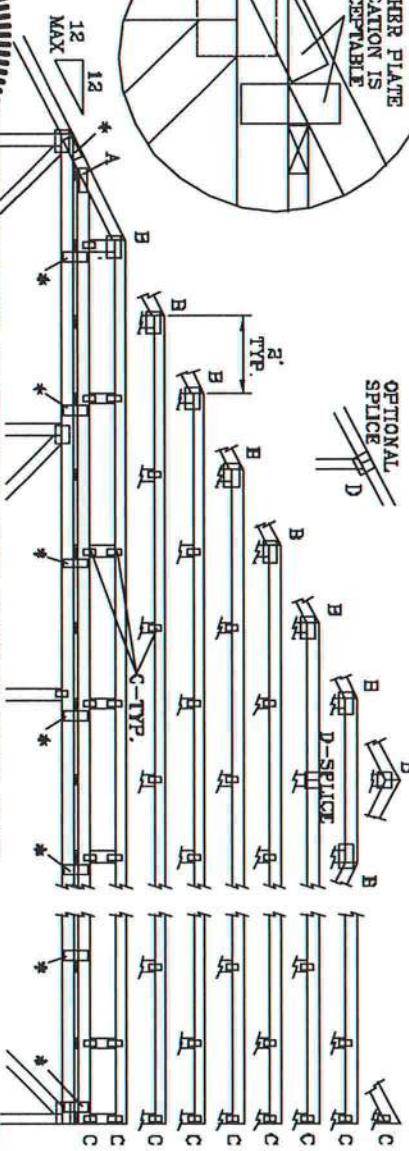
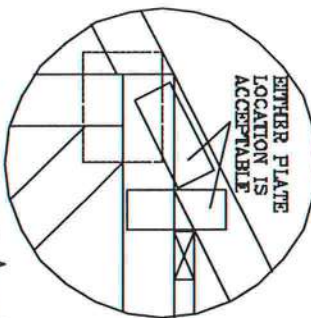
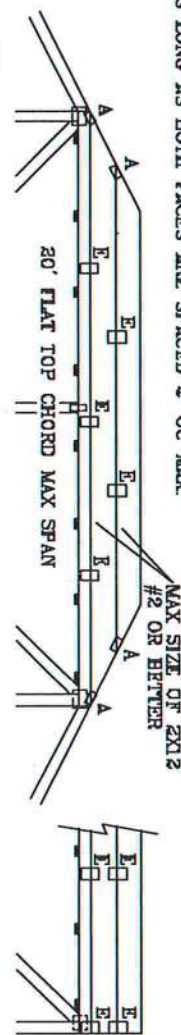
ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST
CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF
110 MPH WIND, 30' MEAN HGT, ENG ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL=5 PSF, WIND BC DL=5 PSF

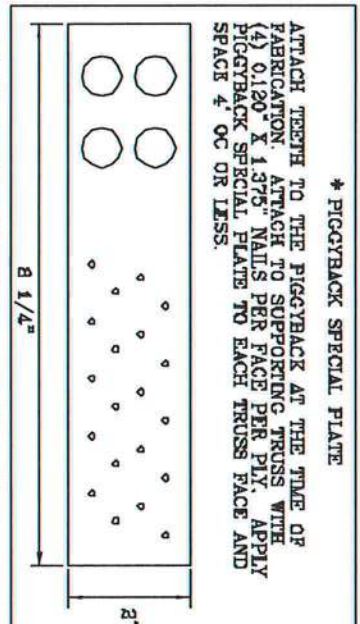
FRONT FACE (E*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.



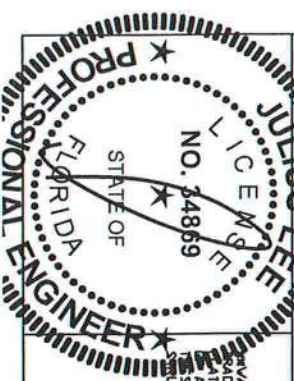
JOINT TYPE	SPANS UP TO		
	30'	36'	62'
A	2X4	2.6X4	2.6X4
B	4X6	6X6	6X6
C	1.5X3	1.6X4	1.5X4
D	5X4	6X6	6X6
E	4X8 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY		

ATTACH TRUSS PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

WEB LENGTH	WEB BRACING CHART
0' TO 7'9"	NO BRACING
7'9" TO 10'	1X4 "T" BRACE SAME GRADE SPECIES AS WEB MEMBER OR BETTER AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 6d NAILS AT 4' OC.
10' TO 14'	2X4 "T" BRACE SAME GRADE SPECIES AS WEB MEMBER OR BETTER AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4' OC.



* PIGGYBACK SPECIAL PLATE
ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



PROFESSIONAL ENGINEER
STATE OF FLORIDA
NO. 34869
JULIUS LEE
1440 SW 4th AVENUE
DIKRAY BEACH, FL 33444-2611

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1440 SW 4th AVENUE
DIKRAY BEACH, FL 33444-2611

No. 34869
STATE OF FLORIDA

MAX LOADING	REF	PIGGYBACK
55 PSF AT	DATE	09/12/07
1.33 DUR. FAC.	DRWG/ITEK	STD PIGGY
50 PSF AT	ENG	JL
1.25 DUR. FAC.		
47 PSF AT		
1.15 DUR. FAC.		
SPACING	24.0"	

THIS DRAWING REPLACES DRAWINGS 634.016 634.017 & 647.045

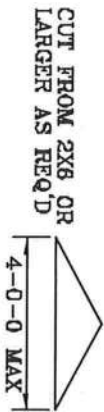
VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

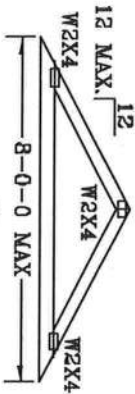
* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:

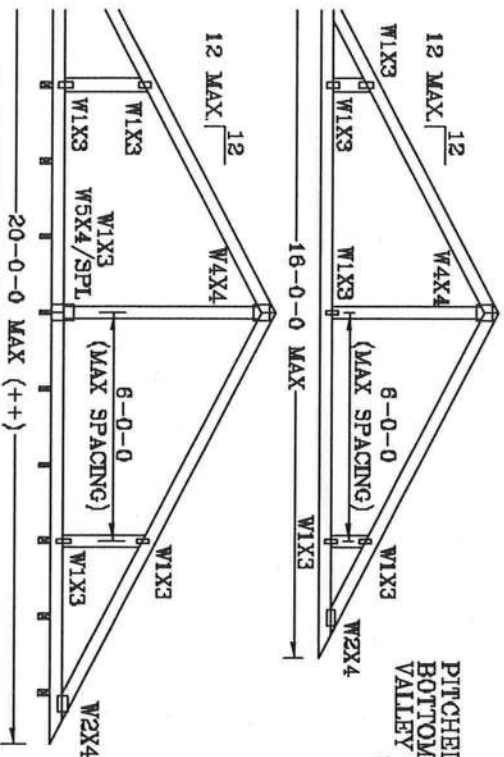
(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR
ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED
BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF.



CUT FROM 2X6 OR
LARGER AS REQ'D



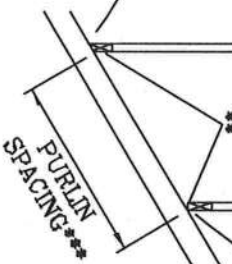
PITCHED CUT
BOTTOM CHORD
VALLEY



20'-0" MAX (++)

VALLEY
SPACING

TOE-NAILED

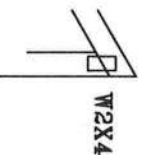


SQUARE CUT
BOTTOM CHORD
VALLEY

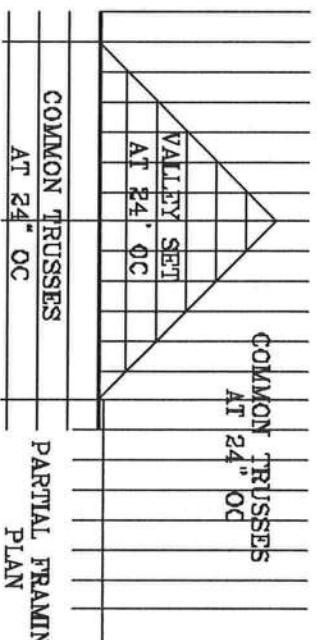
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS
BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES
NOT EXCEED 12'0".
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80%
LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED
WITH 8d BOX (0.135" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING,
EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".
TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS
INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON
ENGINEERS' SEALED DESIGN.

OPTIONAL STUB
END DETAIL



COMMON TRUSSES
AT 24" OC



COMMON TRUSSES
AT 24" OC

PARTIAL FRAMING
PLAN

THIS DRAWING REPLACES DRAWING A105

JULIUS LEE'S
CONS. ENGINEERS P.A.

1455 SW 4th AVENUE
DEALY BEACH, FL 33444-8161

TC LL	20	20	PSF	REF	VALLEY DETAIL
TC DL	7	15	PSF	DATE	11/26/03
BC DL	5	5	PSF	DRWG	VALTRUSS1103
BC LL	0	0	PSF	ENG	JL
TOT. LD.	32	40	PSF		
DURFAC	1.25	1.25			
SPACING	24"				

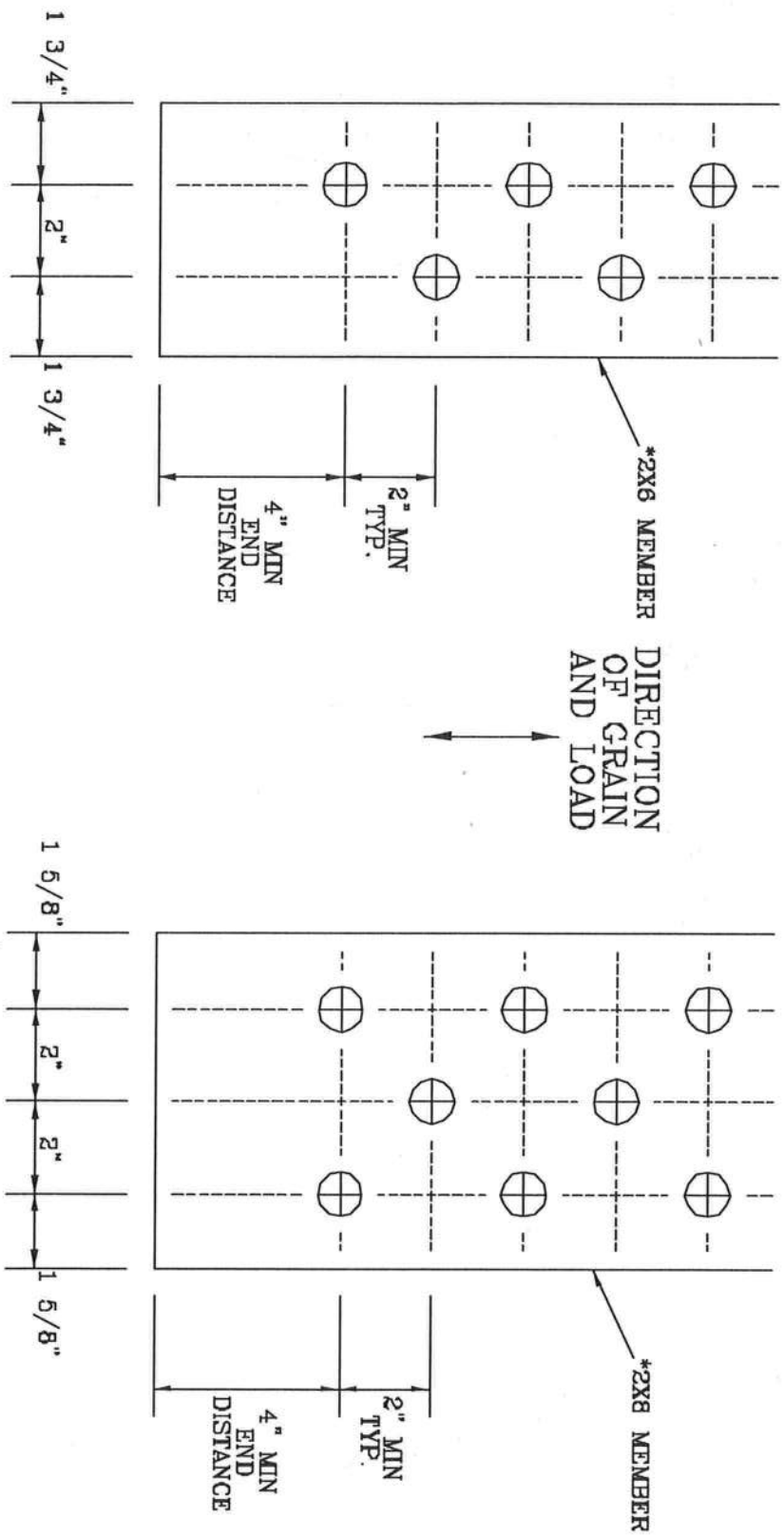
REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008



1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

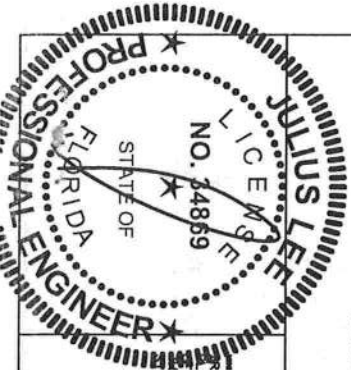
TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL

2X8 DETAIL

THIS DRAWING REPLACES DRAWING A626.016



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO 2001 I-BO BUILDING DEPARTMENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS LITE INSTITUTE, 583 ODESSA DR., SUITE 204, MADISON, VA, 22729 AND VITA CLOUD TRUSS COUNCIL, 6300 ENTERPRISE LN, MADISON, VA 22750 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

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1400 ST 4TH AVENUE
DELRIST GULCH, FL 33444-2161

No. 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTS P1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

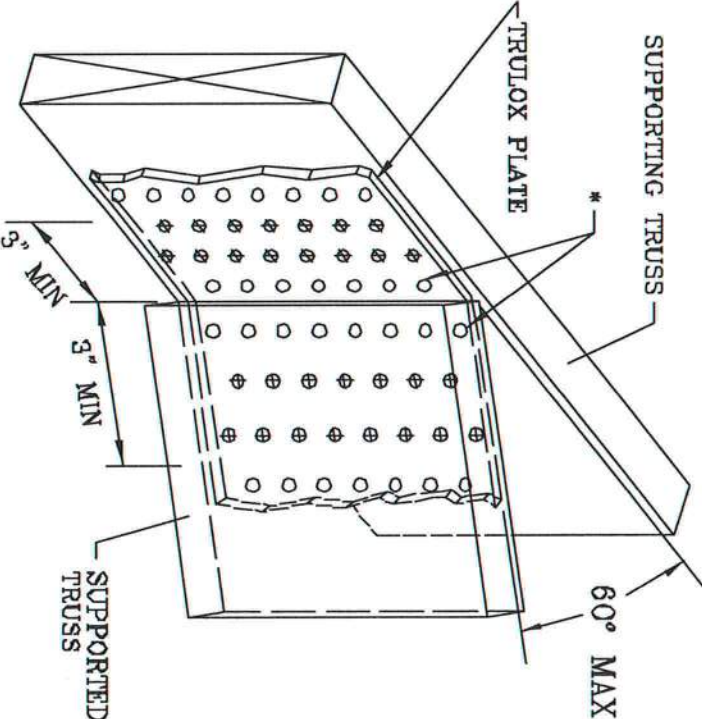
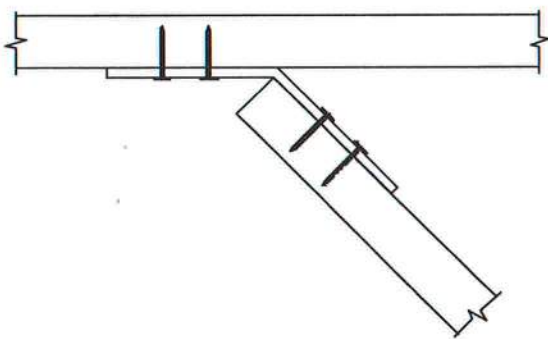
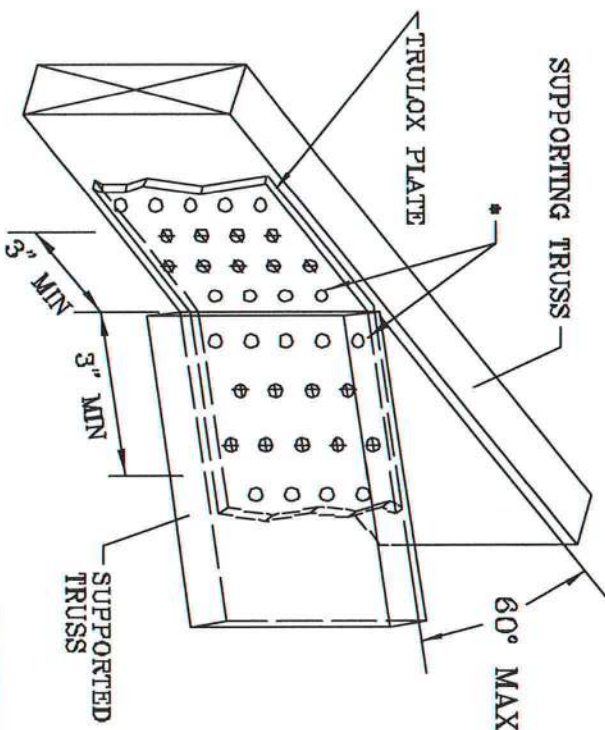
TRULOX CONNECTION DETAIL

11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO, PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.
REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

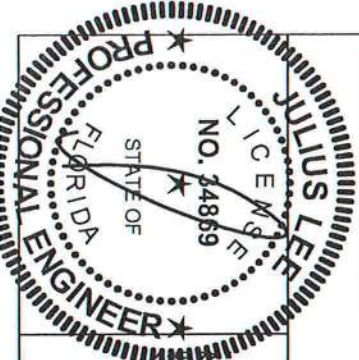


MINIMUM 3X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
6X6	15	990#

MINIMUM 5X6 TRULOX PLATE

THIS DRAWING REPLACES DRAWINGS 1,158,986 1,158,988/R 1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO SECT 1003 (BUILDING CODE) AND SECT 1004 (LUMBER) FOR ADDITIONAL TRUSS CONSTRUCTION REQUIREMENTS. THE DESIGNER IS RESPONSIBLE FOR THE DESIGN AND THE TRUSS CONSTRUCTION. THE TRUSS CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE DESIGN AND THE TRUSS CONSTRUCTION. THE TRUSS CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE DESIGN AND THE TRUSS CONSTRUCTION. THE TRUSS CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE DESIGN AND THE TRUSS CONSTRUCTION.

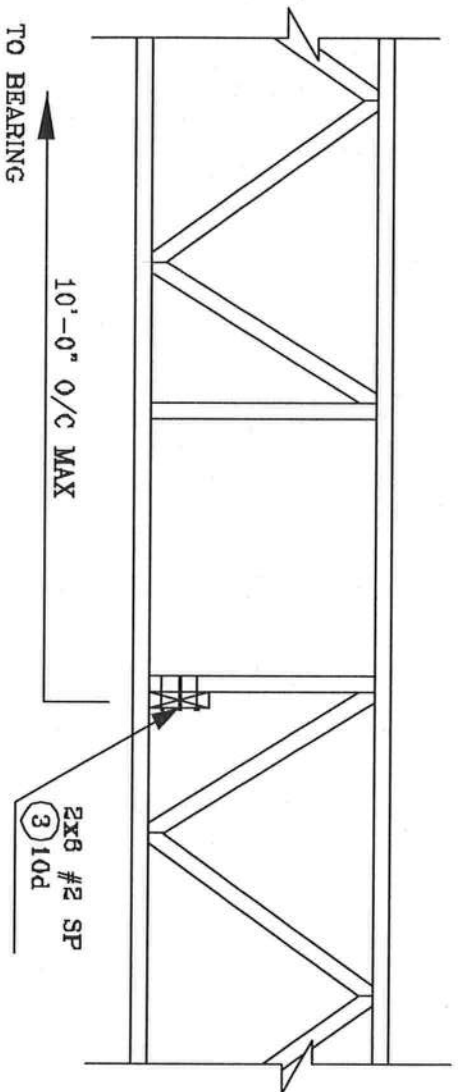
JULIUS LEE'S
CONS. ENGINEERS P.A.

1455 SW 4th AVENUE
DELAWARE BEACH, FL 33444-2161

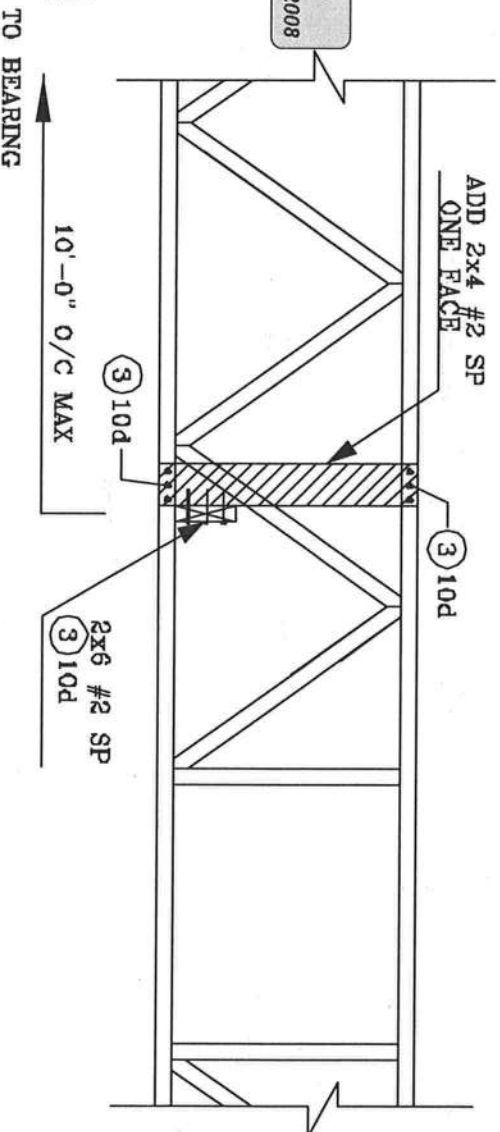
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

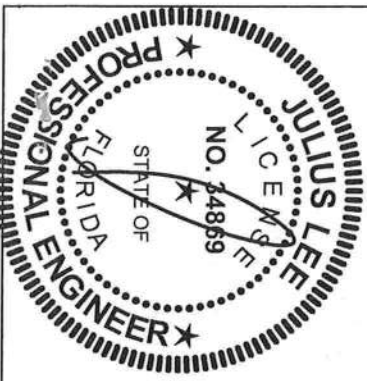
STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008


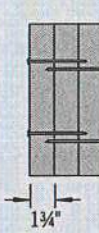

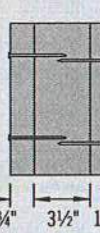
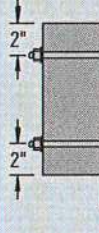



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CONS. ENGINEERS P.A.
1425 SW 4th AVENUE
DIKSHIT BUILDING, FL 33411-2161

No. 34869
STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
								
			3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	280	245		
	3	12"	555	415	415	370		
1/2" A307 Through Bolts ⁽²⁾⁽⁴⁾	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS 1/4" x 3/2" ⁽⁴⁾	2	24"	680	510	510	455		
		19.2"	850	640	640	565		
		16"	1,020	765	765	680		
SDS 1/4" x 6" ⁽³⁾⁽⁴⁾	2	24"				455	465	455
		19.2"				565	580	565
		16"				680	695	680
USP WS35 ⁽⁴⁾	2	24"	480	360	360	320		
		19.2"	600	450	450	400		
		16"	715	540	540	480		
USP WS6 ⁽³⁾⁽⁴⁾	2	24"				350	525	350
		19.2"				440	660	440
		16"				525	790	525
3 3/4" TrussLok ⁽⁴⁾	2	24"	635	475	475	425		
		19.2"	795	595	595	530		
		16"	955	715	715	635		
5" TrussLok ⁽⁴⁾	2	24"		500	500	445	480	445
		19.2"		625	625	555	600	555
		16"		750	750	665	725	665
6 3/4" TrussLok ⁽⁴⁾	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 1/16" maximum.

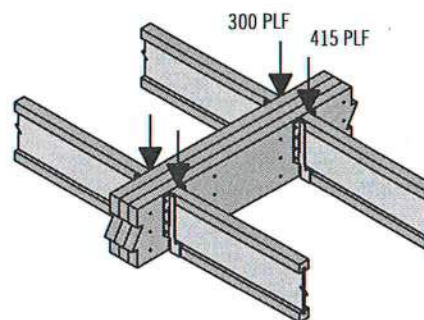
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



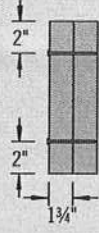
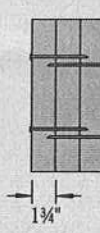
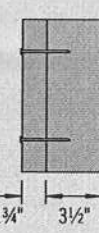

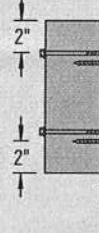
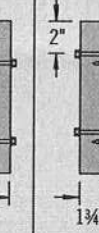
First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 3/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
							
		3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	835	835	740		
	12	2,225	1,670	1,670	1,485		
	18	3,335	2,505	2,505	2,225		
	24	4,450	3,335	3,335	2,965		
SDS Screws 1/4" x 3 1/2" or WS35 1/4" x 6" or WS6 ⁽¹⁾	4	1,915	1,435 ⁽⁴⁾	1,435	1,275	1,860 ⁽²⁾	1,405 ⁽²⁾
	6	2,870	2,150 ⁽⁴⁾	2,150	1,915	2,785 ⁽²⁾	2,110 ⁽²⁾
	8	3,825	2,870 ⁽⁴⁾	2,870	2,550	3,715 ⁽²⁾	2,810 ⁽²⁾
3 3/8" or 5" TrussLok™	4	2,545	1,910 ⁽⁴⁾	1,910	1,695	1,925 ⁽³⁾	1,775 ⁽³⁾
	6	3,815	2,860 ⁽⁴⁾	2,860	2,545	2,890 ⁽³⁾	2,665 ⁽³⁾
	8	5,090	3,815 ⁽⁴⁾	3,815	3,390	3,855 ⁽³⁾	3,550 ⁽³⁾

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

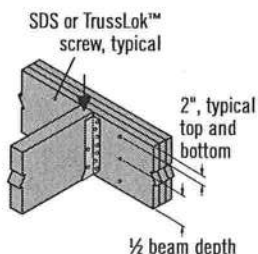
(2) 6" long screws required.

(3) 5" long screws required.

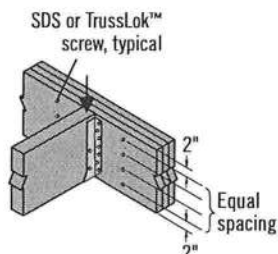
(4) 3 1/2" and 3 3/8" long screws must be installed on both sides.

Connections

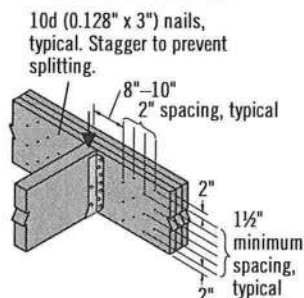
4 or 6 or Screw Connection



8 Screw Connection

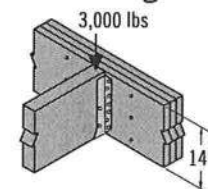


Nail Connection



There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1 1/4" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1 1/4" assembly, eight 3 3/8" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1 3/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3 3/8" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed

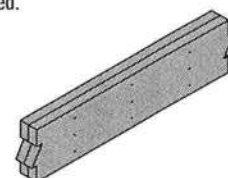
on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3 1/2" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of 1/2" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

L6

9'-0"

 $2^i - 0^{i-1}$ **ROOF PITCH(S)**

6/12

REFER TO HD 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED)

- [illegible]

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JASON HOWELL, RES.

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5-13-08	K.L.H.	L280515
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