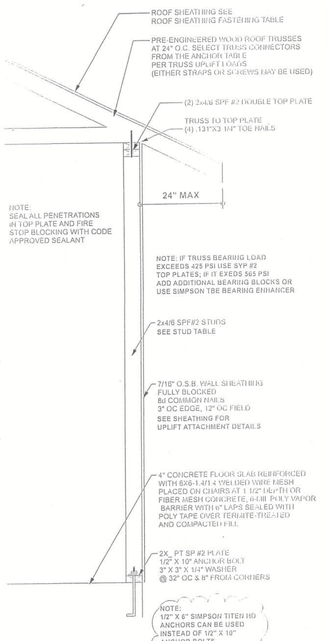
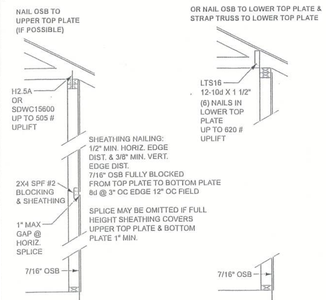


ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

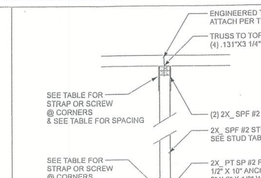
ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)				
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RSRS-01 (2 3/8" x 0.113")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RSRS-01 (2 3/8" x 0.113")	6" oc	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RSRS-03 (2 1/2" x 0.131") or ASTM F1667 RSRS-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RSRS-01 (2 3/8" x 0.113")	6" oc	6" oc
130 mph Exp. C	15/32"	ASTM F1667 RSRS-01 (2 3/8" x 0.113")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RSRS-03 (2 1/2" x 0.131") or ASTM F1667 RSRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RSRS-01 (2 3/8" x 0.113")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RSRS-03 (2 1/2" x 0.131") or ASTM F1667 RSRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	10/32"	ASTM F1667 RSRS-03 (2 1/2" x 0.131") or ASTM F1667 RSRS-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RSRS-03 (2 1/2" x 0.131") or ASTM F1667 RSRS-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RSRS-03 (2 1/2" x 0.131") or ASTM F1667 RSRS-04 (3" x 0.120")	4" oc	4" oc

Note: For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field. Note: This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



SHEATHING FOR UPLIFT ATTACHMENT DETAILS

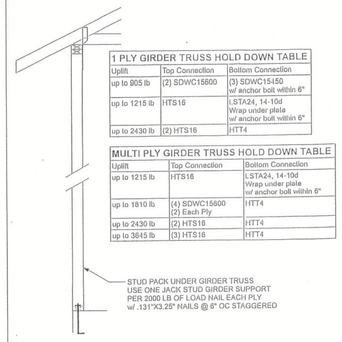
ONE STORY WOOD FRAME



INTERIOR BEARING WALL UPLIFT STRAP OR SCREW TABLE			
Uplift on wall	Top Connection	Bottom Connection	Anchor Bolt Spacing
227 plf	SP2 @ 32" OC	SP1 @ 32" OC	48" OC
454 plf	SP2 @ 16" OC	SP1 @ 16" OC	32" OC
223 plf	(2) SDWC15600 @ 48" OC	(3) SDWC15450 @ 48" OC	48" OC
336 plf	(2) SDWC15600 @ 32" OC	(3) SDWC15450 @ 32" OC	48" OC
257 plf	SPH4/6, (12) .148" x 1 1/2" @ 48" OC	SPH4/6, (12) .148" x 1 1/2" @ 48" OC	48" OC
387 plf	SPH4/6, (12) .148" x 1 1/2" @ 32" OC	SPH4/6, (12) .148" x 1 1/2" @ 32" OC	32" OC
309 plf	LSTA24, (14) .148" x 1 1/2" @ 48" OC Wrap Under Plate	LSTA24, (14) .148" x 1 1/2" @ 48" OC Wrap Over Plate	48" OC
465 plf	LSTA24, (14) .148" x 1 1/2" @ 32" OC Wrap Under Plate	LSTA24, (14) .148" x 1 1/2" @ 32" OC Wrap Over Plate	32" OC

(TYP.) INTERIOR BEARING WALL

ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



1 PLY GIRDER TRUSS HOLD DOWN TABLE		
Uplift	Top Connection	Bottom Connection
up to 905 lb	(2) SDWC15600	(3) SDWC15450 w/ anchor bolt within 6"
up to 1215 lb	HTS16	LSTA24, 14-10d Wrap under plate w/ anchor bolt within 6"
up to 2430 lb	(2) HTS16	HTT4

MULTI PLY GIRDER TRUSS HOLD DOWN TABLE		
Uplift	Top Connection	Bottom Connection
up to 1215 lb	HTS16	LSTA24, 14-10d Wrap under plate w/ anchor bolt within 6"
up to 1810 lb	(4) SDWC15600 (2) Each Ply	HTT4
up to 2430 lb	(2) HTS16	HTT4
up to 3645 lb	(3) HTS16	HTT4

(TYP.) GIRDER TRUSS HOLD DOWN DETAIL

WOOD FRAME w/ STRAPS & ANCHORS

CONNECTOR TABLE				
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
805	505	SDWC15600	-	-
400	290	H3	4-.131"x1 1/2"	4-.131"x1 1/2"
625	540	H2.5A	5-.131"x1 1/2"	5-.131"x1 1/2"
1040	1015	H10A	9-.148"x1 1/2"	9-.148"x1 1/2"
845	515	LTS12-20	6-.148"x1 1/2"	6-.148"x1 1/2"
990	850	MTS12-30	7-.148"x1 1/2"	7-.148"x1 1/2"
1415	1215	HTS20-30	8-.148"x1 1/2"	8-.148"x1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-.148"x1 1/2"	8-.148"x1 1/2"
1640	1460	MSTA24	9-.148"x1 1/2"	9-.148"x1 1/2"
1030	1030	CS20	7-.148"x1 1/2"	7-.148"x1 1/2"
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
555	535	SP1	4-.148"x3"	4-.148"x3"
1010	605	SP2	6-.148"x3"	6-.148"x3"
1280	1100	SPH4/6	12-.148"x1 1/2"	wrap under or over plate
771	771	LSTA24	10-.148"x1 1/2"	wrap under or over plate
1235	1235	LSTA24	14-.148"x1 1/2"	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stemwall	To Stud / Post	Anchor
2145	1835	DTT2Z	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-.162"x2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
2145	1835	DTT2Z	8-SDS 1/4"x1 1/2"	1/2"x8" Titen HD
4235	3640	HTT4	18-.162"x2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stemwall	To Post	Anchor
1900		ABU44Z	12-.162"x3 1/2"	5/8"x12" Drill & Epoxy
2475		ABU66Z	12-.162"x3 1/2"	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
1900		ABU44Z	12-.162"x3 1/2"	5/8"x7" Drill & Epoxy
2475		ABU66Z	12-.162"x3 1/2"	5/8"x7" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:
THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE			
		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 13FT. DO NOT CUT WWM OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-98, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

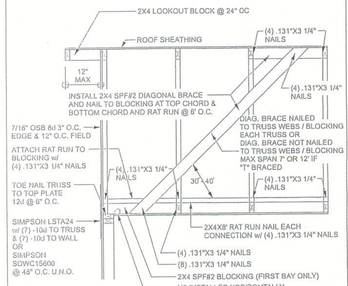
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

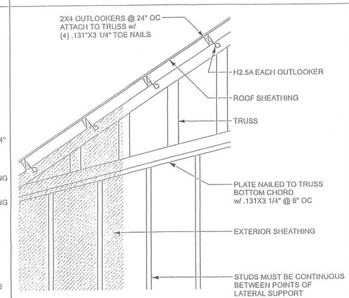
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C. FOR GABLE HEIGHT UP TO 25'-0" 130 MPH, EXP. C, ENCLOSED

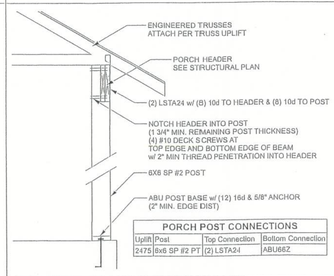
(TYP.) GABLE BRACING DETAIL

WOOD FRAME



(TYP.) GABLE WALL w/ VAULTED CEILING

WOOD FRAME



PORCH POST CONNECTIONS			
Uplift	Post	Top Connection	Bottom Connection
2475	6x6 SP #2 PT	(2) LSTA24	ABU66Z

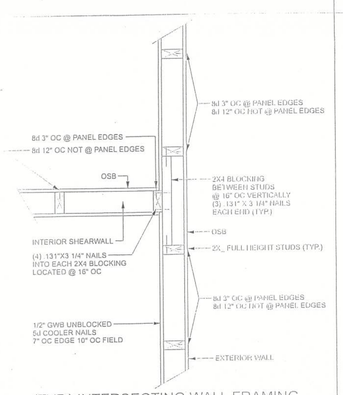
POSTS CUT FLUSH WITHOUT NOTCH - BRACE TO PREVENT ROTATION:

EAVE / HIP ROOF:
- STRAP TRUSS w/ H2.5A OR EQUAL TO EACH SIDE OF HEADER (FRONT & BACK) USE LONGER STRAP AS NEEDED IF TOP PLATES ARE INSTALLED
- (2) LSTA24 16-10d OR (2) MTS20 14-10d ON FRONT AND REAR OF POST TO HEADER
- ATTACH HEADER TO POST w/ (8) .131" x 3.25" TOE-NAIL

GABLE END:
- EXTEND GABLE SHEATHING TO BOTTOM OF HEADER NAILED WITH 8d NAILS @ 6" MIN. INTO TRUSS BOTTOM CHORD AND INTO BEAM AT 1 1/2" FROM TOP & BOTTOM EDGE
- ATTACH HEADER TO POST w/ (8) .131" x 3.25" TOE-NAIL

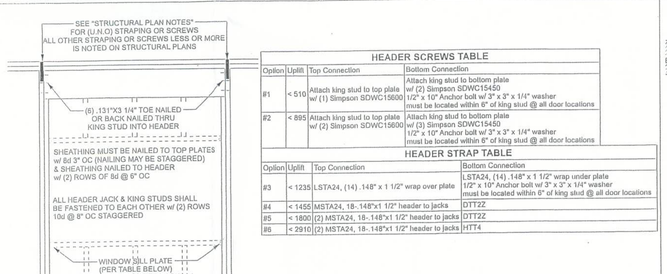
(TYP.) PORCH POST

ONE STORY WOOD



(TYP.) INTERSECTING WALL FRAMING

WOOD FRAME



HEADER SCREWS TABLE			
Option	Uplift	Top Connection	Bottom Connection
#1	< 510	Attach king stud to top plate w/ (1) Simpson SDWC15600	Attach king stud to bottom plate w/ (2) Simpson SDWC15450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations
#2	< 895	Attach king stud to top plate w/ (2) Simpson SDWC15600	Attach king stud to bottom plate w/ (3) Simpson SDWC15450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations

HEADER STRAP TABLE			
Option	Uplift	Top Connection	Bottom Connection
#3	< 1235	LSTA24, (14) .148" x 1 1/2" wrap over plate	LSTA24, (14) .148" x 1 1/2" wrap under plate 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations
#4	< 1455	MSTA24, 18-.148"x1 1/2" header to jacks	DTT2Z
#5	< 1800	(2) MSTA24, 18-.148"x1 1/2" header to jacks	DTT2Z
#6	< 2910	(2) MSTA24, 18-.148"x1 1/2" header to jacks	HTT4

SILL PLATE SPANS FOR 10'-0" WALL HEIGHT					
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2				BASED ON WFCM TABLE A-3.23B
	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6	
130 MPH EXP. C	5'-2"	7'-9"	7'-7"	11'-3"	FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)

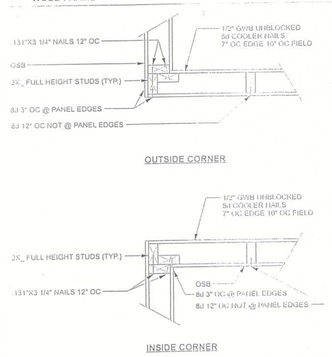
TYPICAL HEADER STRAPING OR SCREWS DETAIL

ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

ALLOWABLE UPLIFT: 1235 LB

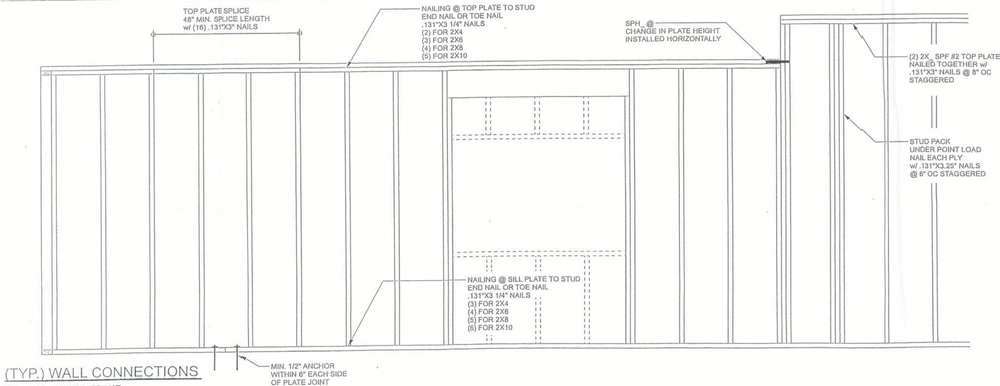
(TYP.) BEAM TO WALL

WOOD FRAME w/ STRAPS & ANCHORS



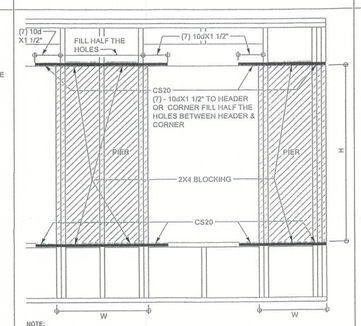
(TYP.) CORNER FRAMING

WOOD FRAME



(TYP.) WALL CONNECTIONS

ONE STORY WOOD FRAME



NOTE:
THIS DETAIL IS INTENDED TO BE USED ONLY FOR NAROW SHEARWALL SEGMENTS AS SPECIFIED ON THE PLAN. THE PIER BESIDE THE OPENING MUST MEET THE ASPECT RATIO REQUIREMENT H/W < 3.5:1 WHERE H IS THE PIER HEIGHT. FOR WINDOWS NOT GREATER THAN 2' HIGHT OR 5' WIDE THE WIDTH OF THE OPENING MAY BE INCLUDED AS FULL HEIGHT SHEARWALL IN ADDITION TO THE PIER WIDTH WHEN STRAPPED ACCORDING TO THIS DETAIL.

OPENING FORCE TRANSFER

WOOD FRAME

DESIGN CRITERIA & LOADS:	
BUILDING CODE	8TH EDITION (FLORIDA BUILDING CODE RESIDENTIAL (2023)
CODE FOR DESIGN LOADS	ASCE 7-22
WINDLOADS	
BASIC WIND SPEED (ASCE 7-16, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
ROOF LOADING	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

| COMPONENT & CLADING