BUILDING PROFILE

Width (ft)	= 40	Eave Height (ft)	= 12
Length (ft)	= 80	Roof Slope (Rise/12)	= 4.0:12

BUILDING LOADS

A) THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY FBC 23 / 8TH EDITION

B) THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT. ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PANELS, VENTILATORS ARE NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE BUILDING MANUFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL EQUIPMENT AND THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHOULD BE ERECTED ON A PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN MANUAL, THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES. THE END USER AND/OR ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF THE LOCAL BUILDING DEPT.

OCCUPANCY/RISK CATEGORY: II – Normal Is 1.0000 Ie 1.00

WIND LOAD ULTIMATE 121 MPH NOMINAL 93.73 MPH WIND EXPOSURE B

CLOSURE TYPE: Enclosed INTERNAL WIND COEF. −0.18 / 0.18

GROUND SNOW LOAD: 3.00 PSF ROOF SNOW LOAD 2.17 PSF Ce 1.0000 Ct 1.2000

SNOW BANKING LOADS: PER CODE

COLLATERAL DEAD LOAD: 2.00 PSF

ROOF LIVE LOAD: 20.00 PSF (REDUCIBLE Yes)

DEAD LOAD: 2.00 PSF (FOR ROOF PANELS AND PURLINS)

SEISMIC

SPECTRAL RESPONSE Ss 0.1200 S1 0.0560 Sds 0.1000 Sd1 0.0800

SITE CLASS D DESIGN RISK CATEGORY B Cs 0.0334

RESPONSE MODIFICATION FACTOR, R 3.000* FRAMES 3.000* BRACING

BASIC SEISMIC FORCE RESISTING SYSTEM (LATERAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES

BASIC SEISMIC FORCE RESISTING SYSTEM (ENDWALLS) = ORDINARY STEEL MOMENT FRAMES

BASIC SEISMIC FORCE RESISTING SYSTEM (BSW) = ORDINARY STEEL CONC. BRACED FRAMES

BASIC SEISMIC FORCE RESISTING SYSTEM (FSW) = ORDINARY STEEL MOMENT FRAMES

ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE PROCEDURE

* STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.

SERVICEABILITY CRITERIA

MINIMUM DESIGN DEFLECTIONS

Endwall Column	= 180	Roof Panel (Live)	= 60
Endwall Rafter (Live)	= 180	Roof Panel (Wind)	= 60
Endwall Rafter (Wind)	= 180	Rigid Frame (Horz)	= 100
Wall Girt	= 90	Rigid Frame (Vert)	= 180
Roof Purlin (Live)	= 150	Rigid Frame (Seismic)	= 50
Roof Purlin (Wind)	= 150		
Wall Panel	= 60		

GENERAL NOTES

A) THE STRUCTURE UNDER THIS CONTRACT HAS BEEN DESIGNED AND DETAILED FOR THE LOADS AND CONDITIONS STIPULATED IN THE CONTRACT AND SHOWN ON THESE DRAWINGS. ANY ALTERATIONS TO THE STRUCTURAL SYSTEM OR REMOVAL OF ANY COMPONENT PARTS, OR THE ADDITION OF OTHER CONSTRUCTION MATERIALS OR LOADS MUST BE DONE UNDER THE ADVICE AND DIRECTION OF A REGISTERED ARCHITECT, CIVIL OR STRUCTURAL ENGINEER. THE BUILDING MANUFACTURER WILL ASSUME NO RESPONSIBILITY FOR ANY LOADS NOT INDICATED.

B) THIS METAL BUILDING IS DESIGNED WITH THE BUILDING MANUFACTURER'S STANDARD PRACTICES WHICH ARE BASED ON PERTINENT PROCEDURES AND RECOMMENDATIONS OF THE FOLLOWING ORGANIZATIONS AND CODES.

1. AMERICAN INSTITUTE OF STEEL CONSTRUCTION: " AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS– ALLOWABLE STRESS DESIGN" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.
2. AMERICAN IRON AND STEEL INSTITUTE: "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.
3. AMERICAN WELDING SOCIETY: "STRUCTURAL WELDING CODE" AWS D1.1. AS ADOPTED BY THE BUIDLING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.
4. METAL BUILDING MANUFACTURER'S ASSOCIATION: "LOW RISE BUILDING SYSTEMS MANUAl" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE.

C) 1) MATERIAL PROPERTIES OF STEEL PLATE USED IN THE FABRICATION OF PRIMARY RIGID FRAMES, AND OTHER PRIMARY STRUCTURAL EXCLUSIVE OF COLD–FORMED SECTIONS, CONFORM TO ASTM–A529 OR A572 . FLANGES AND WEB MATERIAL CONFORMS TO ASTM–A529 OR A572 GRADE 55 WITH A MINIMUM YIELD POINT OF 55,000 psi.

2) MATERIAL PROPERTIES OF HSS ROUND SECTIONS CONFORM TO ASTM–A500, GRADE B OR C WITH A MINIMUM YIELD POINT OF 42,000 psi.

3) MATERIAL PROPERTIES OF HSS RECT. OR SQUARE SECTIONS CONFORM TO ASTM–A500, GRADE B OR C WITH A MINIMUM YIELD POINT OF 46,000 psi.

4) MATERIAL PROPERTIES OF HOT ROLLED CHANNEL AND ANGLE MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM–A992 WITHMINIMUM YIELD POINT OF 50,000 PSI. HOT ROLLED W–SHAPED MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM–A992WITH MINIMUM YIELD POINT OF 50,000 PSI.

5) MATERIAL PROPERTIES OF COLD FORMED LIGHT GAGE STEEL MEMBERS CONFORM TO EITHER ASTM A653–06 GR 55 OR A1011–04 HSLAS GRADE 55 WITH YIELD OF 55,000 psi.

6) MATERIAL PROPERTIES OF ROOF/WALL SHEETING, BASE METAL CONFORM TO ASTM–A792 GRADES 80 CLASS 1, 2 OR 3 WITH A MINIMUM YIELD STRENGTH OF 80,000 PSI. COATING OF BASE MATERIAL IS 55% ALUMINUM–ZINC ALLOY IN ACCORDANCE WITH AZ55 SPECIFICATIONS.

7) CABLE UTILIZED FOR BRACING CONFORMS TO ASTM A475. CABLE BRACING IS TO BE INSTALLED TO A TAUT CONDITION.

8) ROD UTILIZED FOR BRACING MEMBERS CONFORM TO ASTM–A36 WITH MINIMUM YIELD POINT OF 36,000 PSI.

9) IT IS THE RESPONSIBILITY OF ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE "RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A–325 OR A–490 BOLTS". ALL A–325 BOLTS IN PRIMARY FRAMING MUST BE "SNUG–TIGHT", EXCEPT AS FOLLOWS:
"FULLY–PRETENSION" A–325 BOLTS IF:
   a) BUILDING LOCATED IN A HIGH SEISMIC AREA. FOR IBC–BASED CODE, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC DESIGN CATEGORY" OF "D", "E" OR "F".
   b) BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5.00 TONS.
   c) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR STRESS – REVERSALS ON THE CONNECTIONS.
   d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A–325 – SC".

10) SECONDARY MEMBERS AND FLANGE BRACE CONNECTIONS SHALL ALWAYS BE SNUG TIGHT, UNO.

11) ANCHOR BOLTS 3/4" IN DIAMETER THRU 1 1/4" IN DIAMETER CONFORM TO A.S.T.M. F1554 GR. 36. ANCHOR BOLTS 1/2" IN DIAMETER CONFORM TO A.S.T.M. A–307.

D) UNLESS NOTED OTHERWISE ON FRAMING COLOR CHART: ALL STEEL MEMBERS EXCEPT BOLTS, FASTENERS, CABLE AND RODS SHALL RECEIVE ONE COAT OF STANDARD RED OXIDE SHOP PRIMER.

E) SHOP AND FIELD INSPECTIONS AND ASSOCIATED FEES ARE THE RESPONSIBILITY OF THE CONTRACTOR, UNLESS STIPULATED OTHERWISE IN THE CONTRACT.

APPROVAL NOTES

THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS APPROVAL DRAWINGS:

A) IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS:
   1) BE MADE IN CONTRASTING INK.
   2) HAVE ALL INSTANCES OF CHANGE CLEARLY INDICATED.
   3) BE LEGIBLE AND UNAMBIGUOUS.

B) DATED SIGNATURE IS REQUIRED ON ALL PAGES.

C) MANUFACTURER RESERVES THE RIGHT TO RESUBMIT DRAWINGS WITH EXTENSIVE OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION. THIS MAY IMPACT THE DELIVERY SCHEDULE.

D) APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE MANUFACTURER HAS CORRECTLY INTERPRETED THE CONTRACT REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN, OR AS DRAWN WITH INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED BY MANUFACTURER.

E) ANY CHANGES NOTED ON THE DRAWINGS NOT IN CONFORMANCE WITH THE TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION. MANUFACTURER RECOGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED. HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTUAL TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILAR INDICATION OF APPROVAL, DISAPPROVAL, ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS UNACCEPTABLE ALTERATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS CUSTOMER.

SAFETY COMMITMENT

A) THE BUILDING MANUFACTURER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF THE BUILDING MANUFACTURER.

B) IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE.

C) LOCAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKER SAFETY.

D) MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES.

E) DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED.

ERECTOR / CONTRACTOR RESPONSIBILITIES

A) IT IS THE RESPONSIBILITY OF THE ERECTOR/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE BUILDING MANUFACTURER OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT.

B) THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS REQUIRED.

C) APPROVAL OF THE MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE BUILDING MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.4.1 AISC CODE OF STANDARD PRACTICES, LATEST ED.)

D) WHERE DISCREPANCIES EXIST BETWEEN THE MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE LATEST ED.)

E) DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY THE BUILDING MANUFACTURER ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN THE BUILDING MANUFACTURER'S ENGINEERS UNLESS SPECIFICALLY INDICATED.

F) THE ERECTOR/CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK IN COMPLIANCE WITH THE BUILDING MANUFACTURER'S "FOR CONSTRUCTION" DRAWINGS.

G) PRODUCTS SHIPPED TO ERECTOR/CONTRACTOR OR HIS CUSTOMER SHALL BE INSPECTED BY ERECTOR/CONTRACTOR IMMEDIATELY UPON ARRIVAL. CLAIMS FOR SHORTAGES OR DEFECTIVE MATERIAL IF NOT PACKAGED MUST BE SENT TO THE MANUFACTURER IN WRITING WITHIN FIVE (5) DAYS AFTER RECEIPT OF THE SHIPMENT. HOWEVER, IF A DEFECT IS OF SUCH A NATURE THAT REASONABLE VISUAL INSPECTION WOULD FAIL TO DISCLOSE IT, THEN THE CLAIM MUST BE MADE WITHIN FIVE (5) DAYS AFTER THE ERECTOR/CONTRACTOR LEARNS OF THE DEFECT. THE MANUFACTURER WILL NOT BE LIABLE FOR ANY DEFECT UNLESS CLAIM IS MADE WITHIN ONE (1) YEAR AFTER DATE OF THE ORIGINAL SHIPMENT BY THE MANUFACTURER TO CONTRACTOR OR HIS CUSTOMER. THE MANUFACTURER WILL BE GIVEN A REASONABLE OPPORTUNITY TO INSPECT DEFECTIVE MATERIALS UPON RECEIPT OF CLAIM BY CONTRACTOR.
IF A DEFECT IS OF SUCH NATURE THAT IT CAN BE REMEDIED BY A FIELD OPERATION AT THE JOB SITE WITHOUT THE NECESSITY OF RETURNING THE MATERIAL TO THE MANUFACTURER, THEN UPON WRITTEN AUTHORIZATION OF THE MANUFACTURER THE CONTRACTOR MAY REPAIR OR CAUSE THE MATERIAL TO BE REPAIRED AND THE MANUFACTURER WILL REIMBURSE THE CONTRACTOR FOR THE COST OF THE REPAIR IN ACCORDANCE WITH THE WRITTEN AUTHORIZATION.
THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS IN TO LINE, MODERATE AMOUNTS OF REAMING,CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM.

H) ALL BRACING AS SHOWN AND PROVIDED BY THE MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE.

I) TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION OR COLLISION. (SECT. 7.10.3 AISC CODE OF STANDARD PRACTICE, LATEST ED.)

J) METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE DESIGN,MATERIAL AND WORKMANSHIP OF FOUNDATION. ANCHOR BOLT PLANS PREPARED BY MBM ARE INTENDED TO SHOW ONLY LOCATION, DIAMETER AND PROJECTION OF THE ANCHOR RODS REQUIRED TO ATTACH THE METAL BUILDING SYSTEM TO FOUNDATION. IT IS RESPONSIBILITY OF THE END CUSTOMER TO ENSURE THAT ADEQUATE PROVISIONS ARE MADE FOR SPECIFYING ROD EMBEDMENT, BEARING VALUES, TIE RODS AND OTHER ASSOCIATED ITEMS EMBEDDED IN THE CONCRETE FOUNDATION, AS WELL AS FOUNDATION DESIGN FOR THE LOADS IMPOSED BY MB SYSTEM, OTHER IMPOSED LOAD, AND THE BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE (MBMA 06 SECTIONS 3.2.2 AND A3)

K) METAL BUILDING MANUFACTURER DOES NOT PROVIDE ANY FIELD SUPERVISION FOR THE ERECTION, NOR DOES MBM PERFORM ANY INSPECTIONS DURING OR AFTER ERECTION.

COMPONENTS & CLADDING (unfactored)

Wall Field Values	= 21.497	psf	/ −23.319	psf
Wall Edge Values	= 21.497	psf	/ −28.784	psf

HORNET STEEL BUILDINGS

Pre-Engineered Metal Buildings

FLORIDA PRODUCT APPROVAL NUMBER

PBR ROOF PANEL	36875.1
PBR WALL PANEL	36876.1

IT IS THE RESPONSIBILITY OF THE CUSTOMER TO PROVIDE ALL DOCUMENTATION REQUIRED FOR ANY ACCESSORIES NOT PROVIDED BY MBM TO THEIR LOCAL PERMITTING OFFICE. ALL ACCESSORIES MUST COMPLY AND MEET ALL DESIGN REQUIREMENTS PER LOCAL CODES.

ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA. THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING REINFORCEMENT.

FRAMING COLORS

Rigid Frame: RO Flange brace: RO Angle: RO

RO = Red Oxide
CP = Grey Primer
GZ = Galvanized

	Grt	Pur	EvSt	Jmb	BB	Endwall Col	Endwall Raf
U SECTION:	RO	RO	RO	RO	RO	RO	RO
C SECTION:	RO	RO	RO	RO	RO	RO	RO
D SECTION:	RO	RO	RO	RO	RO	RO	RO
Z SECTION:	RO	RO	RO	RO	RO	RO	RO
E SECTION:	RO	RO	RO	RO	RO	RO	RO
R SECTION:	RO	RO	RO	RO	RO	RO	RO
W SECTION:	RO	RO	RO	RO	RO	RO	RO

WHEN GALVANIZED PROVIDED: ALL FINISHED PRIMARY BUILT–UP AND HOT ROLL MEMBERS ARE HOT DIPPED GALVANIZED. ALL SECONDARY COLD FORMED MEMBERS ARE PRE–GALVANIZED.

IAS ACCREDITED Metal Building Systems AC 472

BUILDING DESIGNED & MANUFACTURED BY AN IAS ACCREDITED FACILITY.

DRAWING INDEX

REV.	PAGE	DESCRIPTION
	0	COVER PAGE
	1	ANCHOR BOLT LAYOUT
	1.1	ANCHOR BOLT DETAILS
	1.2	ANCHOR BOLT REACTIONS
	2	ROOF FRAMING LAYOUT
	2.1–2.5	RIGID FRAME CROSS SECTION
	3	SIDEWALL FRAMING LAYOUT
	4	ENDWALL FRAMING LAYOUT
	5–5.3	FRAMING DETAILS
	6	ROOF PANELS & TRIM
	6.1	ROOF PANEL DETAILS
	7	SIDEWALL PANELS & TRIM
	7.1	SIDEWALL PANEL DETAILS
	8	ENDWALL PANELS & TRIM
	8.1	ENDWALL PANEL DETAILS
	9	SPECIAL DETAILS

BUILDING CODE SPECIFICATIONS REQUIRE CONSIDERATION OF SNOW SURCHARGES FOR ANY LOWER ROOF OF A STRUCTURE LOCATED WITHIN 20ft. OF A HIGHER STRUCTURE. INFORMATION SUPPLIED TO THE METAL BUILDING SUPPLIER DOES NOT INDICATE PRESENCE OF A SHADOWING STRUCTURE WITHIN THIS 20ft. ENVELOPE, AND AS SUCH, SNOW SURCHARGES HAVE NOT BEEN CONSIDERED IN THE DESIGN OF THE BUILDING(S) SHOWN ON THESE PLANS. [2]

THIS PROJECT IS DESIGNED AS AN ENCLOSED BUILDING. ACCESSORIES (DOORS, WINDOWS, ETC.) BY OTHERS MUST BE DESIGNED AS "COMPONENTS AND CLADDING" IN ACCORDANCE TO SPECIFIC WIND PROVISIONS OF REFERENCED BUILDING CODE. [3]

FOR OCCUPANCY (RISK) CATEGORY I OR II, IBC PROVISIONS INDICATE THAT SINGLE–STORY BUILDINGS SHALL HAVE "NO DRIFT LIMIT" PROVIDED THAT INTERIOR WALLS, PARTITIONS, CEILINGS AND EXTERIOR WALL SYSTEMS HAVE BEEN DESIGNED TO ACCOMMODATE THE SEISMIC STORY DRIFTS. INTERIOR WALLS, PARTITIONS, CEILINGS OR EXTERIOR SYSTEMS NOT PROVIDED BY MBM SHALL BE DESIGNED AND DETAILED BY OTHERS TO ACCOMODATE THE SEISMIC STORY DRIFTS. [4]

RICHARD T SMITH LICENSE No 43547 STATE OF FLORIDA PROFESSIONAL ENGINEER

Richard T Smith

Digitally signed by Richard T Smith
DN: c=US, st=Alabama, l=Salem, cn=Richard T Smith, email=rtsmith@smitheng.net
Date: 2024.04.09 18:25:07 -04'00'

This item has been digitally signed and sealed by Richard T Smith on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

COLORS:

ROOF:	COLOR
WALLS:	COLOR
GABLE:	COLOR
EAVE:	COLOR
CORNER:	COLOR
FRAMED OPENINGS:	COLOR
GUTTER:	COLOR
DOWNSPOUTS:	COLOR
BASE:	COLOR

DRAWING STATUS

☐ FOR APPROVAL: THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.

☒ FOR PERMIT: THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL IN THAT, AS A MINIMUM, PIECE MARKINGS ARE NOT IDENTIFIED. ONLY DRAWINGS "ISSUED FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.

☐ FOR CONSTRUCTION: THESE DRAWINGS ARE FINAL AND ISSUED FOR FIELD USE FOR BUILDING ERECTION

ISSUE	DATE	CHK	DET

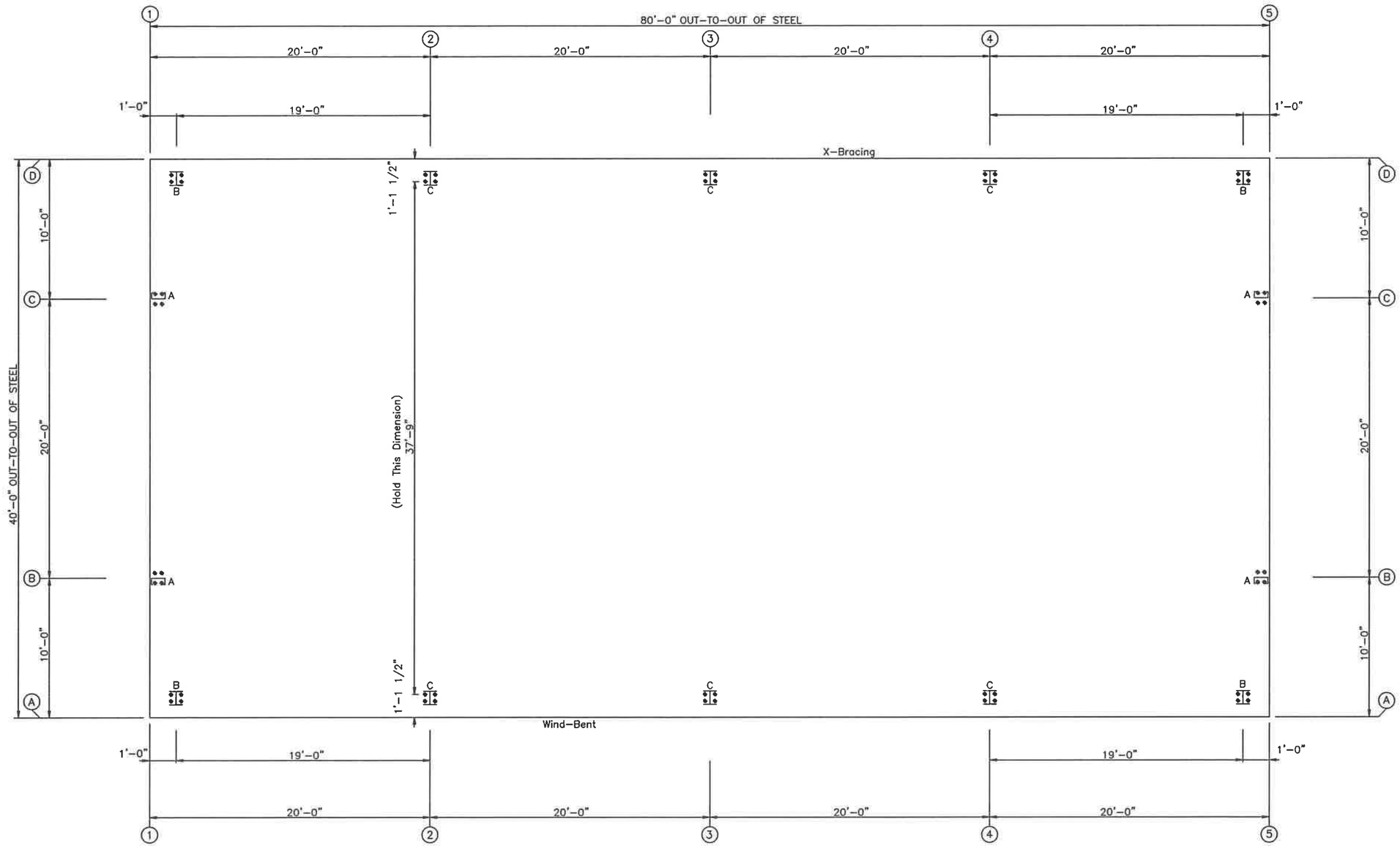
FOR: BETHLEHEM PARK ASSEMBLY BLDG.
157 SW BETHLEHEM AVE,
FT. WHITE FL, 32038

JOBSITE: FT. WHITE FL, 32038

FROM: LMC STEEL
22061 FLETCHER RD
OBRIEN, FL 32071

JOB NO : 8546
DATE : 4/ 7/24
BY : CTW
SCALE : NONE
TITLE : COVER PAGE
NUMBER : PAGE 0

Dia= 3/4"



ANCHOR BOLT PLAN

NOTE: All Base Plates @ 100'-0" (Unless Noted)

NOTE: ALL FIELD LOCATED FRAMED OPENING LOCATIONS SHALL BE AT THE DISCRETION OF THE ERECTOR/CUSTOMER. IT IS RECOMMENDED THAT THESE ANCHORS BE LOCATED AT TIME OF ERECTION.

FIELD LOCATE:
(9) 4'-0" x 4'-0" FRAMED OPENINGS
(2) 3'-0 x 7'-0" FRAMED OPENING
(3) 6'-0" x 7'-0" FRAMED OPENING

ISSUE	DET	CHK	DATE

LMC STEEL

CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

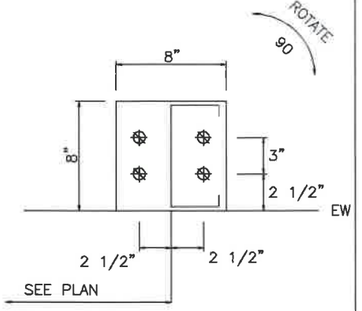
JOB NO: 8546 | DATE: 4/ 7/24

LOCATION: FT. WHITE FL, 32038

DRAWING NAME: ANCHOR BOLT LAYOUT

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 1	CTW	SPW	NONE

Dia= 3/4"



DETAIL A

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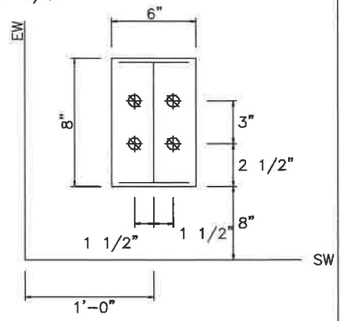

DETAIL B

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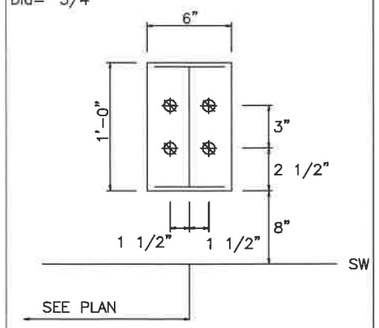
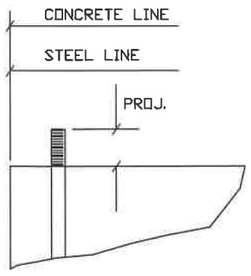


DETAIL C



ANCHOR BOLT PROJECTION

ISSUE	DET	CHK	DATE

LMC STEEL

CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

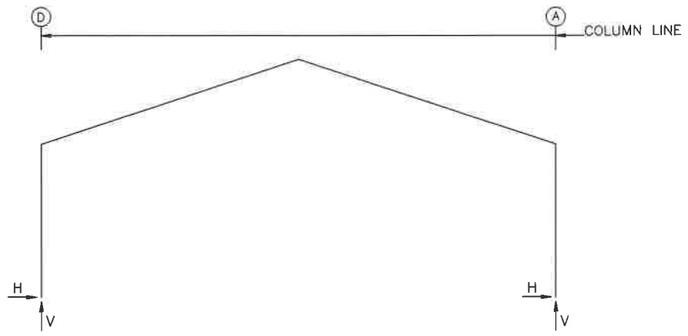
JOB NO: 8546 | DATE: 4/ 7/24

LOCATION: FT. WHITE FL, 32038

DRAWING NAME: ANCHOR BOLT DETAILS

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 1.1	CTW	SPW	NONE

FRAME LINES: 1 2 3 4 5



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
1	D	4	0.750	6.000	8.000	0.375	0.0
1	A	4	0.750	6.000	8.000	0.375	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
2*	D	4	0.750	6.000	12.00	0.375	0.0
2*	A	4	0.750	6.000	12.00	0.375	0.0

2* Frame lines: 2 3

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
4	D	4	0.750	6.000	12.00	0.375	0.0
4	A	4	0.750	6.000	12.00	0.375	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
5	D	4	0.750	6.000	8.000	0.375	0.0
5	A	4	0.750	6.000	8.000	0.375	0.0

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Wind Press Horz	Wind Suct Horz	Seis Long Vert
1	C	0.1	−1.9	2.1	0.0
1	B	0.1	−1.9	2.1	0.0
5	B	0.1	−1.9	2.1	0.0
5	C	0.1	−1.9	2.1	0.0

ENDWALL COLUMN: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Dia	Base_Plate (in) Width	Length	Thick	Grout (in)
1	C	4	0.750	8.000	8.000	0.250	0.0
1	B	4	0.750	8.000	8.000	0.250	0.0
5	B	4	0.750	8.000	8.000	0.250	0.0
5	C	4	0.750	8.000	8.000	0.250	0.0

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

1

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Snow Horz	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert
1	D	0.3	0.8	0.2	0.4	1.4	3.4	0.2	0.5	−2.9	−5.1	0.0	−3.6
1	A	−0.3	0.8	−0.2	0.4	−1.4	3.4	−0.2	0.5	0.0	−3.6	2.9	−5.1

Frame Line	Column Line	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seismic_Left Horz	Seismic_Left Vert	Seismic_Right Horz	Seismic_Right Vert
1	D	−2.9	−3.4	0.0	−1.9	0.0	−3.5	−0.5	−3.1	−0.1	0.0	0.1	0.0
1	A	0.0	−1.9	2.9	−3.4	0.5	−3.1	0.0	−3.5	−0.1	0.0	0.1	0.0

Frame Line	Column Line	MIN_SNOW Horz	MIN_SNOW Vert	F1UNB_SL_L Horz	F1UNB_SL_L Vert	F1UNB_SL_R Horz	F1UNB_SL_R Vert
1	D	0.3	0.6	0.2	0.4	0.2	0.3
1	A	−0.3	0.6	−0.2	0.3	−0.2	0.4

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Snow Horz	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert
2*	D	0.4	1.2	0.3	0.8	2.0	4.8	0.4	0.9	−4.6	−7.9	0.4	−5.5
2*	A	−0.4	1.2	−0.3	0.8	−2.0	4.8	−0.4	0.9	−0.4	−5.5	4.6	−7.9

Frame Line	Column Line	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seismic_Left Horz	Seismic_Left Vert	Seismic_Right Horz	Seismic_Right Vert
2*	D	−4.6	−4.7	0.3	−2.4	0.0	−6.7	−1.0	−5.9	−0.1	−0.1	0.1	0.1
2*	A	−0.3	−2.4	4.6	−4.7	1.0	−5.9	0.0	−6.7	−0.1	0.1	0.1	−0.1

Frame Line	Column Line	MIN_SNOW Horz	MIN_SNOW Vert	F2UNB_SL_L Horz	F2UNB_SL_L Vert	F2UNB_SL_R Horz	F2UNB_SL_R Vert
2*	D	0.5	1.2	0.3	0.8	0.3	0.5
2*	A	−0.5	1.2	−0.3	0.5	−0.3	0.8

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Snow Horz	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert
4	D	0.4	1.2	0.3	0.8	2.0	4.8	0.4	0.9	−4.6	−7.9	0.4	−5.5
4	A	−0.4	1.2	−0.3	0.8	−2.0	4.8	−0.4	0.9	−0.4	−5.5	4.6	−7.9

Frame Line	Column Line	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seismic_Left Horz	Seismic_Left Vert	Seismic_Right Horz	Seismic_Right Vert
4	D	−4.6	−4.7	0.3	−2.4	0.0	−6.7	−1.0	−5.9	−0.1	−0.1	0.1	0.1
4	A	−0.3	−2.4	4.6	−4.7	1.0	−5.9	0.0	−6.7	−0.1	0.1	0.1	−0.1

Frame Line	Column Line	MIN_SNOW Horz	MIN_SNOW Vert	F3UNB_SL_L Horz	F3UNB_SL_L Vert	F3UNB_SL_R Horz	F3UNB_SL_R Vert
4	D	0.5	1.2	0.3	0.8	0.3	0.5
4	A	−0.5	1.2	−0.3	0.5	−0.3	0.8

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Snow Horz	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert
5	D	0.3	0.8	0.2	0.4	1.4	3.4	0.2	0.5	−2.9	−5.1	0.0	−3.6
5	A	−0.3	0.8	−0.2	0.4	−1.4	3.4	−0.2	0.5	0.0	−3.6	2.9	−5.1

Frame Line	Column Line	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seismic_Left Horz	Seismic_Left Vert	Seismic_Right Horz	Seismic_Right Vert
5	D	−2.9	−3.4	0.0	−1.9	0.0	−3.5	−0.5	−3.1	−0.1	0.0	0.1	0.0
5	A	0.0	−1.9	2.9	−3.4	0.5	−3.1	0.0	−3.5	−0.1	0.0	0.1	0.0

Frame Line	Column Line	MIN_SNOW Horz	MIN_SNOW Vert	F4UNB_SL_L Horz	F4UNB_SL_L Vert	F4UNB_SL_R Horz	F4UNB_SL_R Vert
5	D	0.3	0.6	0.2	0.4	0.2	0.3
5	A	−0.3	0.6	−0.2	0.3	−0.2	0.4

2* Frame lines: 2 3

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width	(ft)	=	40.0
Length	(ft)	=	80.0
Eave Height	(ft)	=	12.0/ 12.0
Roof Slope	(rise/12)	=	4.00/ 4.00
Roof Dead Load	(psf)	=	2.0
Wall Dead Load			
Left Endwall	(psf)	=	2.0
Right Endwall	(psf)	=	2.0
Front Sidewall	(psf)	=	2.0
Back Sidewall	(psf)	=	2.0
Roof Live Load	(psf)	=	20.0
Frame Live Load	(psf)	=	16.0
Collateral Load	(psf)	=	2.0
Snow Load	(psf)	=	2.2
Minimum Snow	(psf)	=	3.0
Wind Speed	(mph)	=	121.0
Wind Code		=	FBC 23 (8TH EDITION)
Exposure		=	B
Closure		=	Enclosed
Internal Wind Coeff		=	−0.18, +0.18
Risk Category		=	II − Normal
Importance − Wind		=	1.00
Importance − Seismic		=	1.00
Seismic Design Category		=	B
Seismic Coeff	(Sms)	=	0.15

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Projection (in)
16	Endwall	3/4"	GR36	1.50
40	Frame	3/4"	GR36	2.50

GENERAL NOTES

1. FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
2. ALL REACTIONS ARE UNFACTORED.
3. ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
4. ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/− 1/8" IN BOTH ELEVATION AND LOCATION.
5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

BUILDING BRACING REACTIONS

Wall Loc	Col Line	Col Line	± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Seis	Note
L_EW	1								(h)
F_SW	A	2,3	1.3	1.3	0.2	0.2			(b)
R_EW	5								(h)
B_SW	D	4,3	2.6	1.3	0.4	0.2			

(b)Wind bent in bay, base above finish floor
(h)Rigid frame at endwall

Reactions for seismic represent shear force, Eh
Reaction values shown are unfactored

ISSUE	DET	CHK	DATE

LMC STEEL

CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

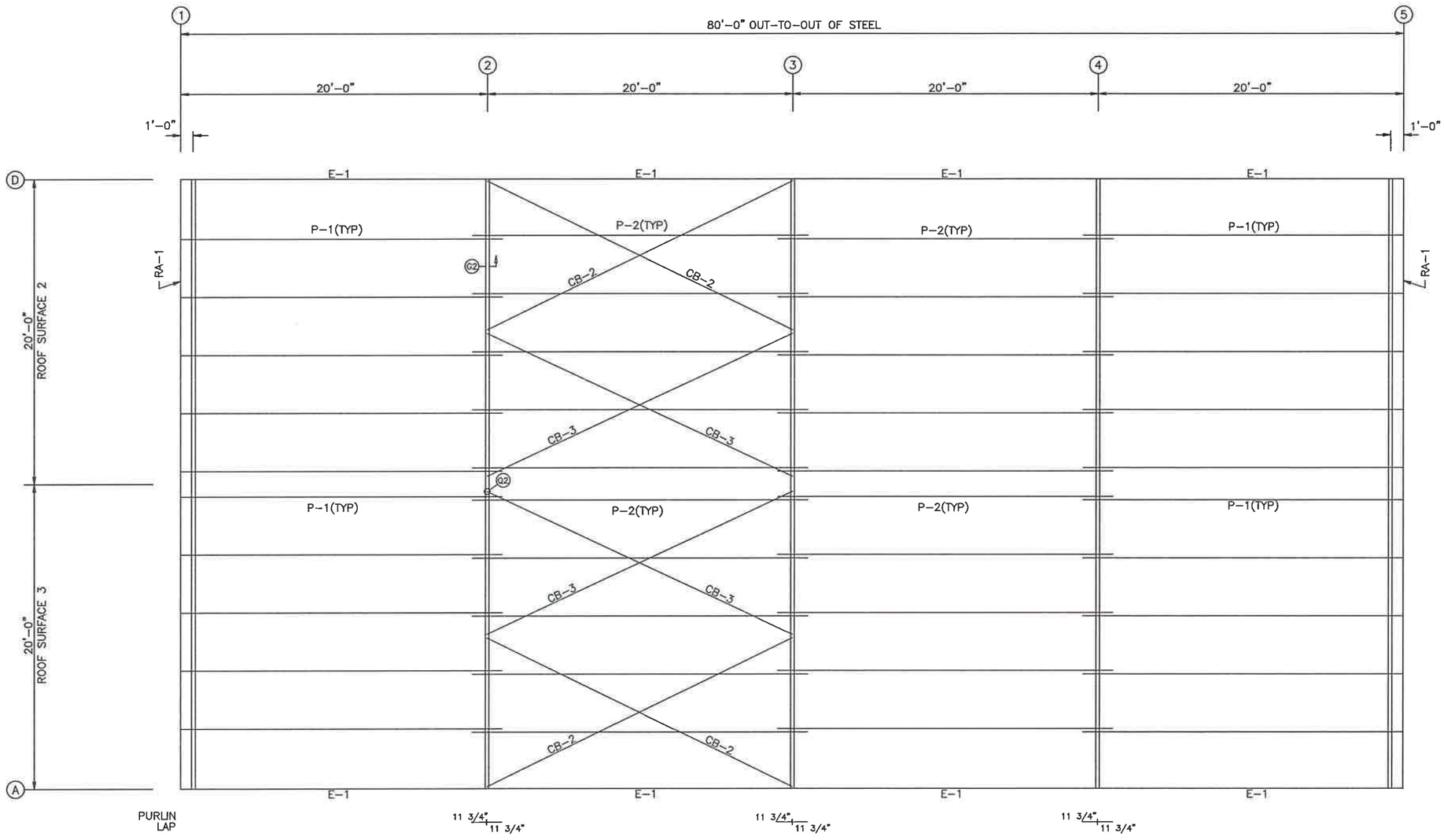
JOB NO: 8546 DATE: 4/ 7/24

LOCATION: FT. WHITE FL, 32038

DRAWING NAME: ANCHOR BOLT REACTIONS

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 1.2	CTW	SPW	NONE

MEMBER TABLE ROOF PLAN		
MARK	PART	LENGTH
P-1	8x25Z16	20'-11 1/2"
P-2	8x25Z16	21'-11 1/2"
E-1	8LE14@4	19'-11 1/2"
CB-2	1/4 CBL	22'-1"
CB-3	1/4 CBL	22'-10"



ROOF FRAMING PLAN

ISSUE	DET	CHK	DATE

LMC STEEL

CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

JOB NO: 8546 | DATE: 4/ 7/24

LOCATION: FT. WHITE FL, 32038

DRAWING NAME: ROOF FRAMING LAYOUT

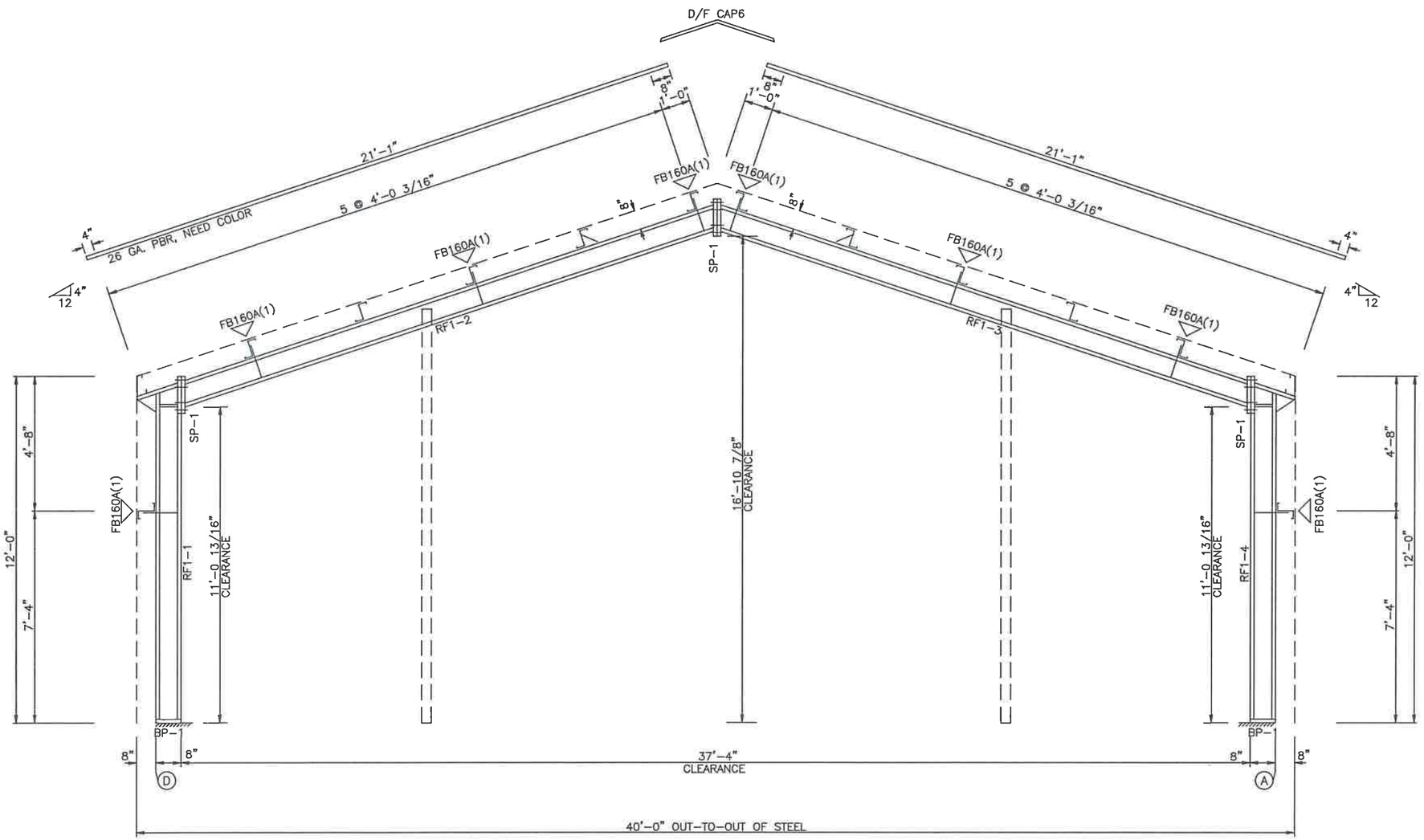
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2	CTW	SPW	NONE

SPLICE BOLT TABLE						
MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP-1	4	4	0	A325	5/8"	2"

BASE PLATE TABLE			
COL MARK	PLATE SIZE Width	THICK	Length
BP-1	6"	3/8"	8"

▽FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A – L2x2x14

MEMBER SIZE TABLE			
MARK	MEMBER	LENGTH	WEIGHT
RF1-1	W8X13	11'-6 1/4"	182
RF1-2	W8X10	19'-7 7/8"	234
RF1-3	W8X10	19'-7 7/8"	234
RF1-4	W8X13	11'-6 1/4"	182



RIGID FRAME ELEVATION: FRAME LINE 1

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

6

ISSUE	DET	CHK	DATE

LMC STEEL

CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

JOB NO: 8546 | DATE: 4/ 7/24

LOCATION: FT. WHITE FL, 32038

DRAWING NAME: RIGID FRAME CROSS SECTION

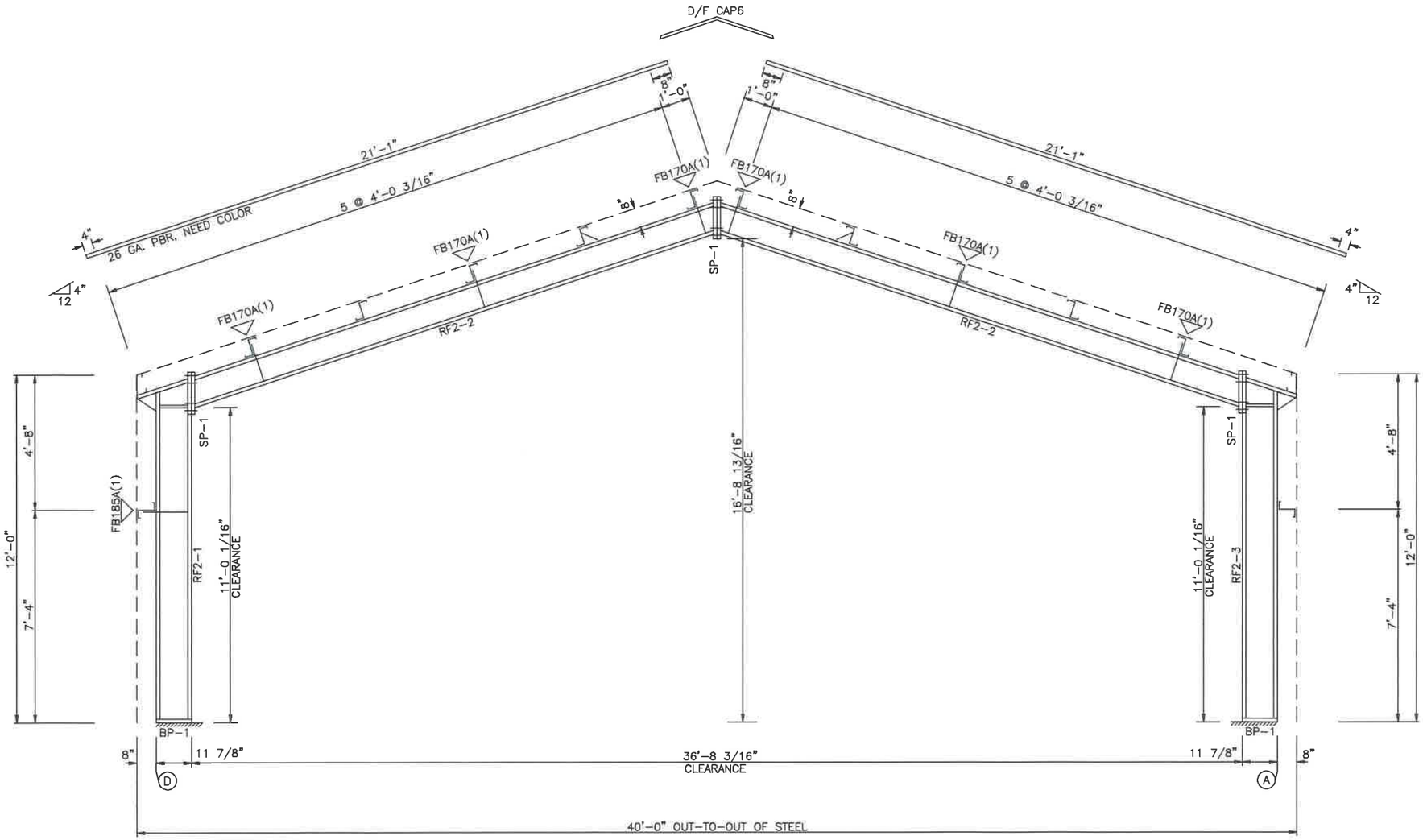
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.1	CTW	SPW	NONE

SPLICE BOLT TABLE						
MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP-1	4	4	0	A325	5/8"	2"

BASE PLATE TABLE			
COL MARK	PLATE SIZE Width	THICK	Length
BP-1	6"	3/8"	1'-0"

▽FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A - L2x2x14

MEMBER SIZE TABLE			
MARK	MEMBER	LENGTH	WEIGHT
RF2-1	W12X14	11'-6 1/4"	196
RF2-2	W10X12	19'-3 11/16"	270
RF2-3	W12X14	11'-6 1/4"	196



RIGID FRAME ELEVATION: FRAME LINE 2 3

ISSUE	DET	CHK	DATE

LMC STEEL

CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

JOB NO: 8546 | DATE: 4/ 7/24

LOCATION: FT. WHITE FL, 32038

DRAWING NAME: RIGID FRAME CROSS SECTION

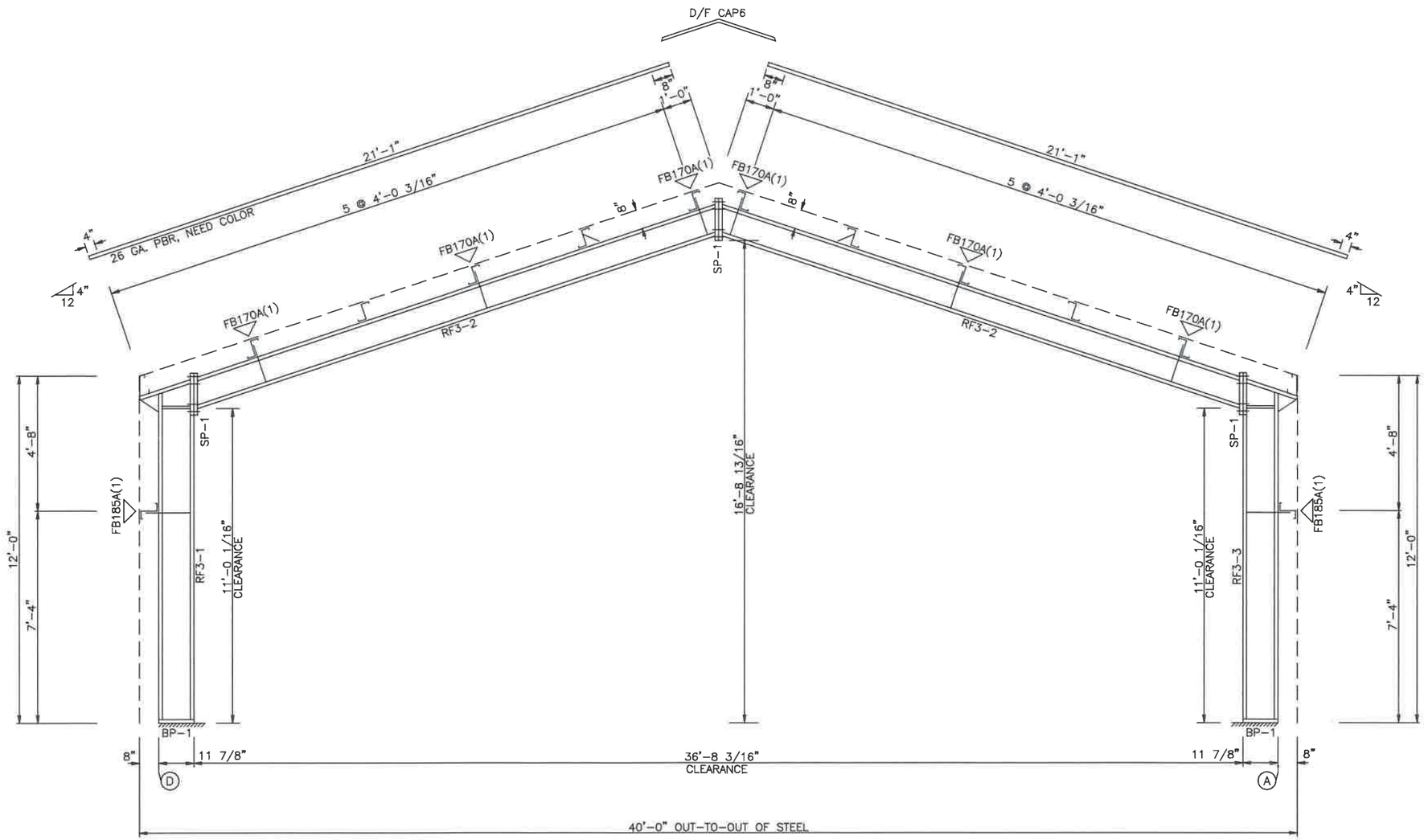
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.2	CTW	SPW	NONE

SPLICE BOLT TABLE						
MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP-1	4	4	0	A325	5/8"	2"

BASE PLATE TABLE			
COL MARK	PLATE SIZE Width	THICK	Length
BP-1	6"	3/8"	1'-0"

▽FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A - L2x2x14

MEMBER SIZE TABLE			
MARK	MEMBER	LENGTH	WEIGHT
RF3-1	W12X14	11'-6 1/4"	196
RF3-2	W10X12	19'-3 11/16"	270
RF3-3	W12X14	11'-6 1/4"	196



RIGID FRAME ELEVATION: FRAME LINE 4

ISSUE	DET	CHK	DATE

LMC STEEL

CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

JOB NO:	DATE:
8546	4/ 7/24

LOCATION: FT. WHITE FL, 32038

DRAWING NAME: RIGID FRAME CROSS SECTION

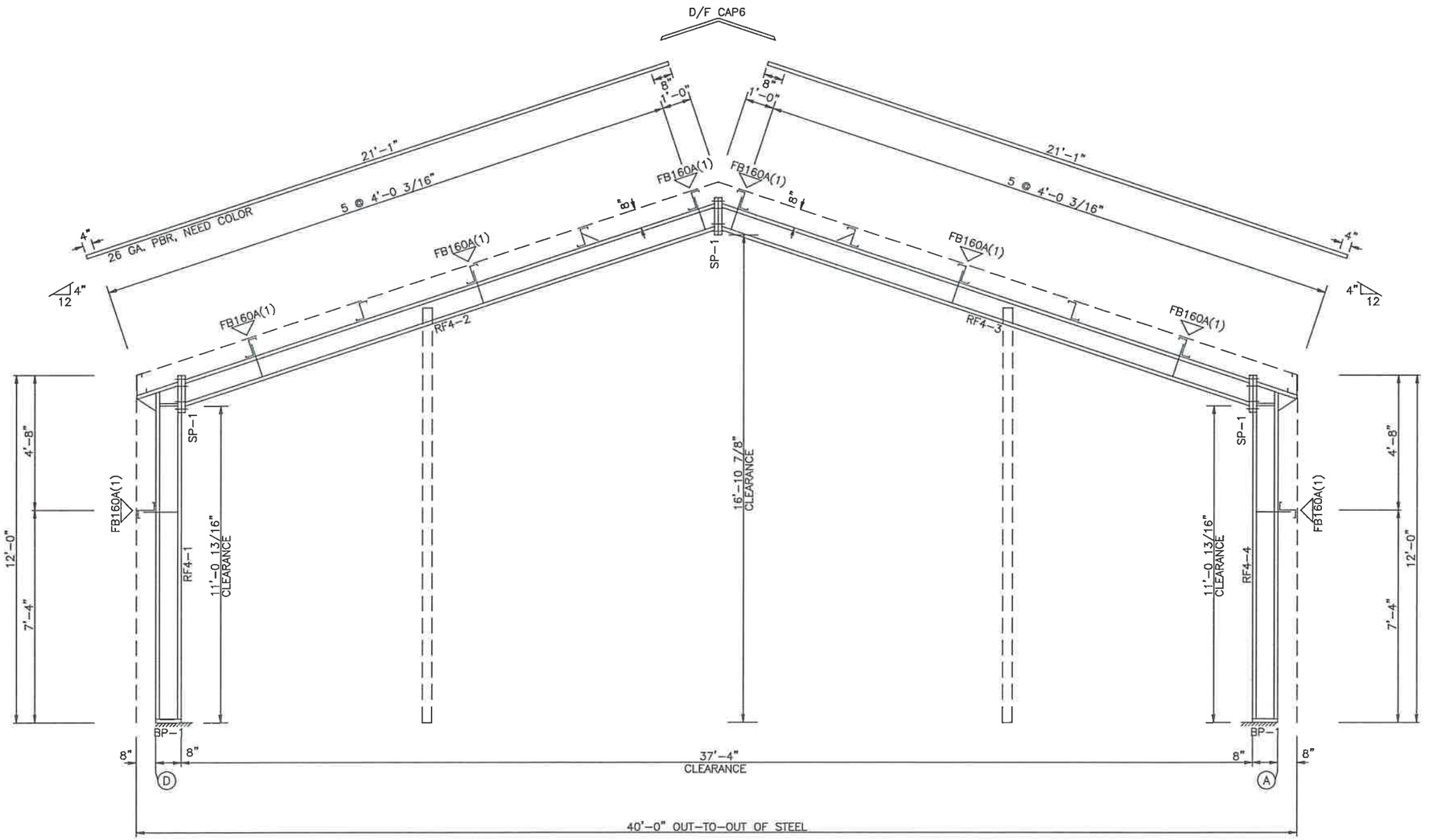
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.3	CTW	SPW	NONE

SPLICE BOLT TABLE						
MARK	Qty Top	Bot	Int	TYPE	DIA	Length
SP-1	4	4	0	A325	5/8"	2"

BASE PLATE TABLE			
COL MARK	PLATE SIZE Width	THICK	Length
BP-1	6"	3/8"	8"

▽FLANGE BRACES: (1) One Side; (2) Two Sides
FBxxA(1): xx=length(in)
A – L2x2x14

MEMBER SIZE TABLE			
MARK	MEMBER	LENGTH	WEIGHT
RF4-1	W8X13	11'-6 1/4"	182
RF4-2	W8X10	19'-7 7/8"	234
RF4-3	W8X10	19'-7 7/8"	234
RF4-4	W8X13	11'-6 1/4"	182



RIGID FRAME ELEVATION: FRAME LINE 5

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

ISSUE	DET	CHK	DATE

LMC STEEL

CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

JOB NO: 8546 | DATE: 4/ 7/24

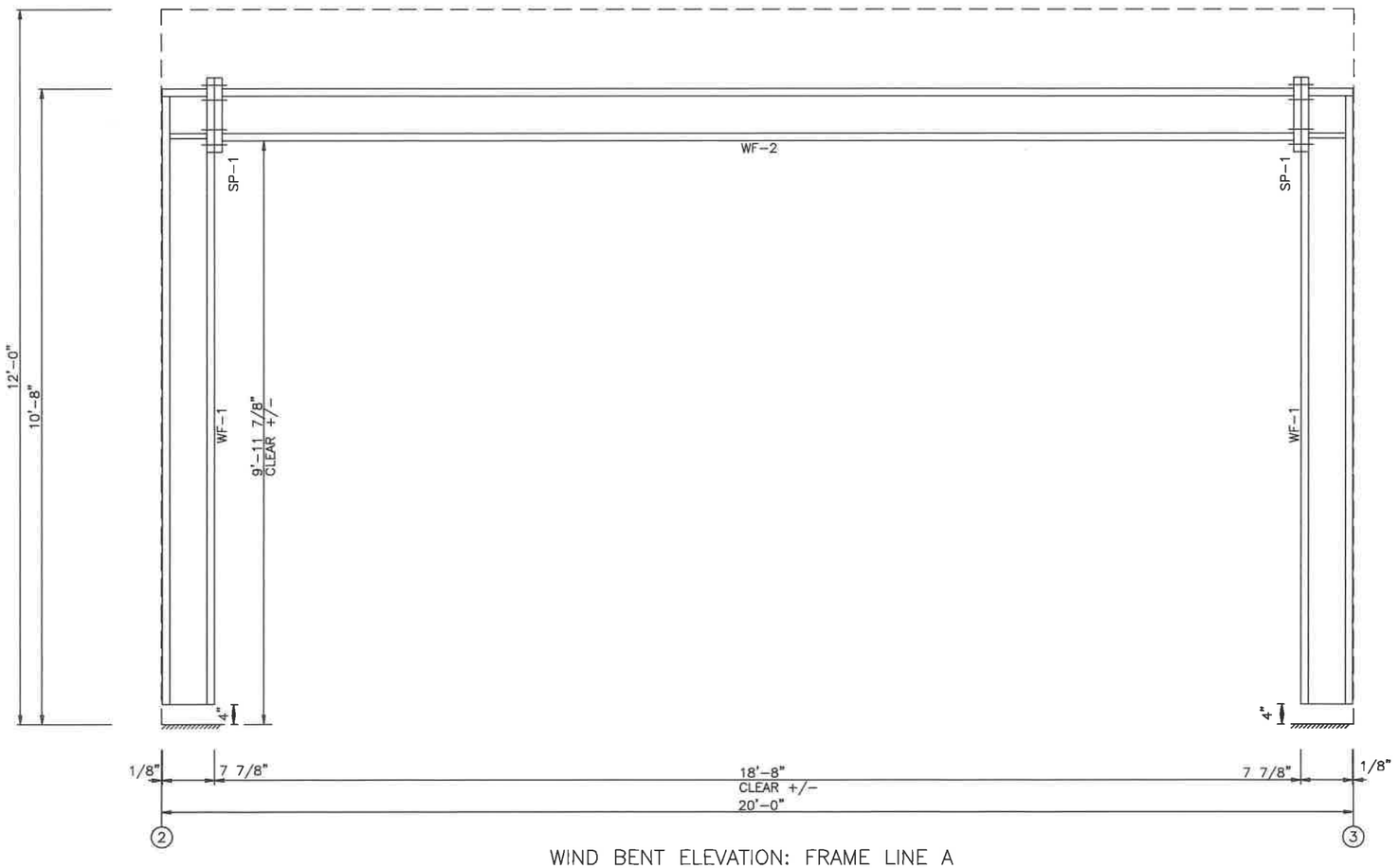
LOCATION: FT. WHITE FL, 32038

DRAWING NAME: RIGID FRAME CROSS SECTION

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.4	CTW	SPW	NONE

SPLICE BOLTS					
Splice Mark	Quan Top/ Bot		Bolt Type	Dia	Length
SP- 1	4	4	A325	5/8"	2"

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
WF-2	W8X18	18'-7 5/8"
WF-1	W8X10	10'-4"



WIND BENT ELEVATION: FRAME LINE A

ISSUE	DET	CHK	DATE

LMC STEEL

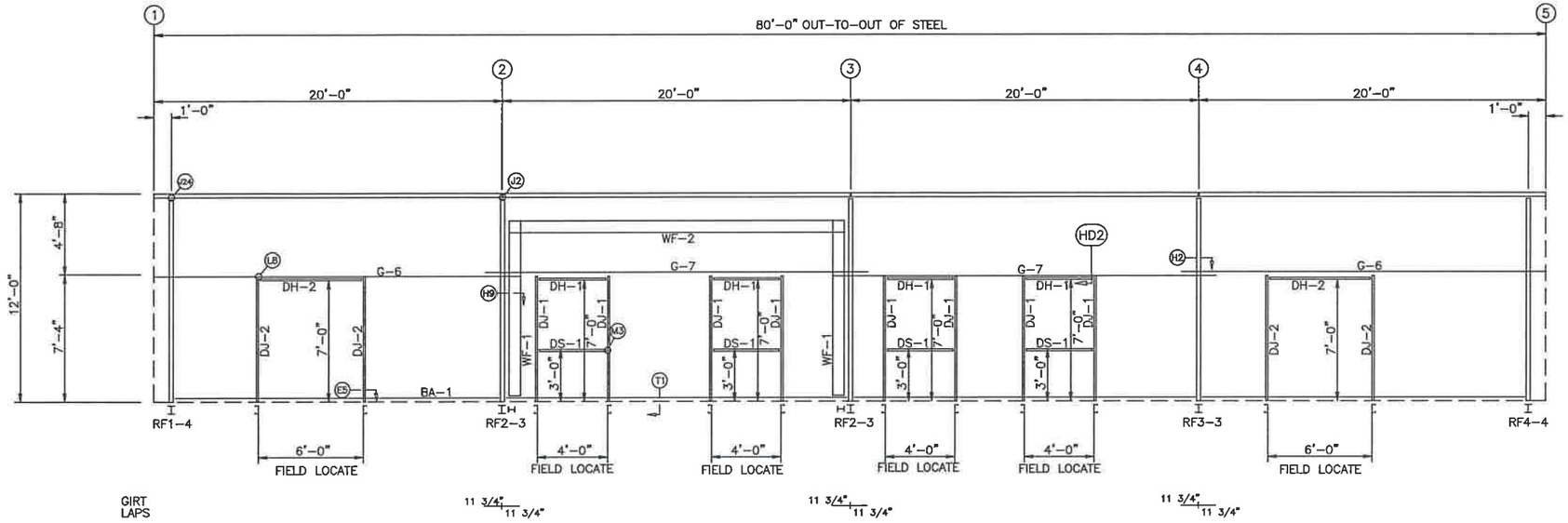
CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

JOB NO: 8546 | DATE: 4/ 7/24

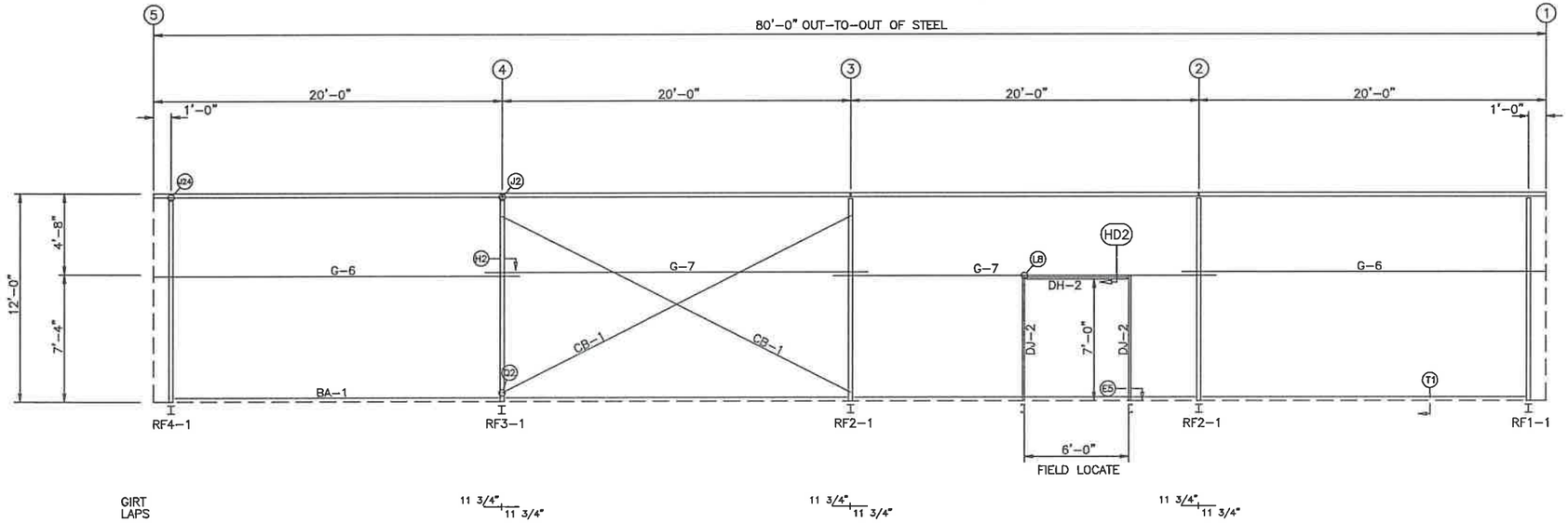
LOCATION: FT. WHITE FL, 32038

DRAWING NAME: RIGID FRAME CROSS SECTION

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 2.5	CTW	SPW	NONE



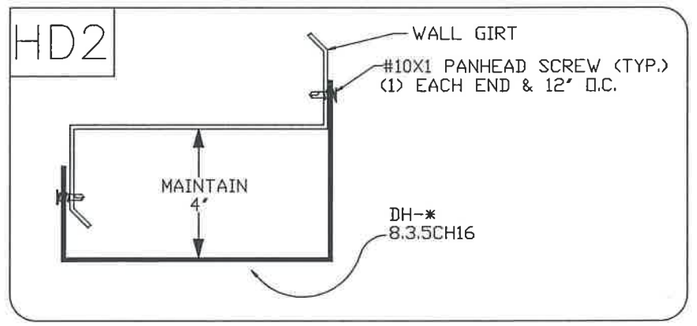
SIDEWALL FRAMING: FRAME LINE A



SIDEWALL FRAMING: FRAME LINE D

BOLT TABLE FRAME LINE A & D				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 – WF-2	8	A325	5/8"	2"
WF-1 – RF2-3	6	A325	5/8"	2"

MEMBER TABLE FRAME LINE A & D		
MARK	PART	LENGTH
WF-1	W8X10	10'-4"
WF-2	W8X18	18'-7 5/8"
DJ-1	8X25C16	7'-4"
DJ-2	8X25C16	7'-4"
DH-1	8.3.5CH6	4'-0"
DH-2	8X25C16	6'-0"
DS-1	8X25C16	4'-0"
G-6	8x25Z16	20'-11 1/2"
G-7	8x25Z16	21'-11 1/2"
CB-1	1/4 CBL	23'-4"



ISSUE	DET	CHK	DATE

LMC STEEL

CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

JOB NO: 8546 | DATE: 4/ 7/24

LOCATION: FT. WHITE FL, 32038

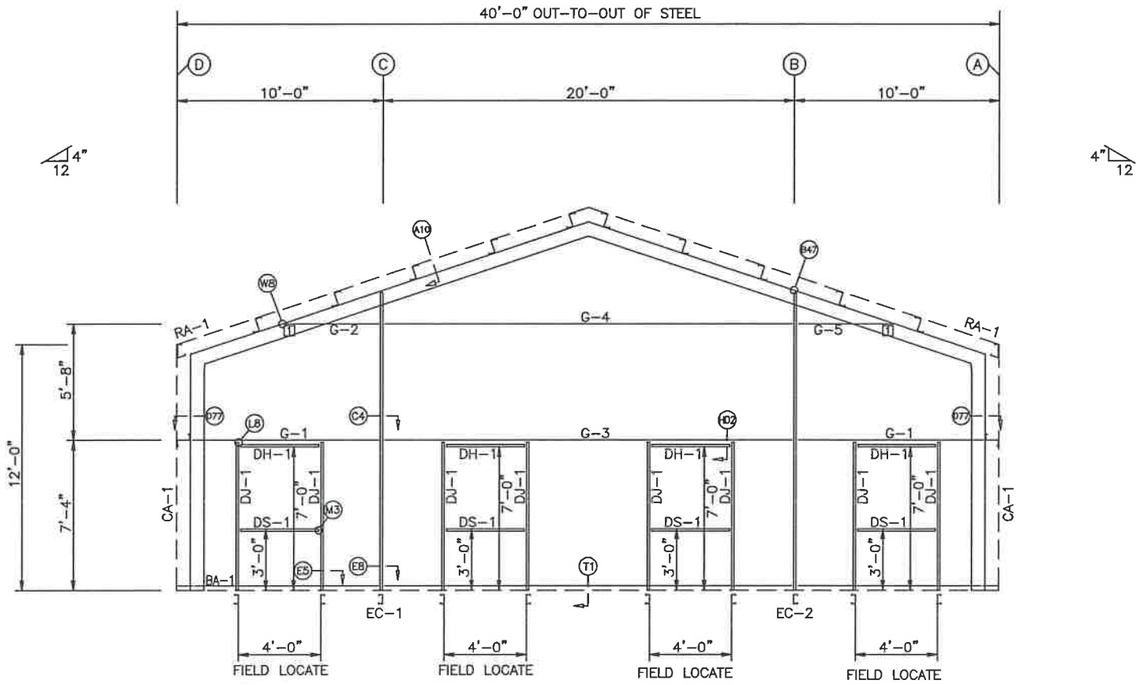
DRAWING NAME: SIDEWALL FRAMING LAYOUT

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 3	CTW	SPW	NONE

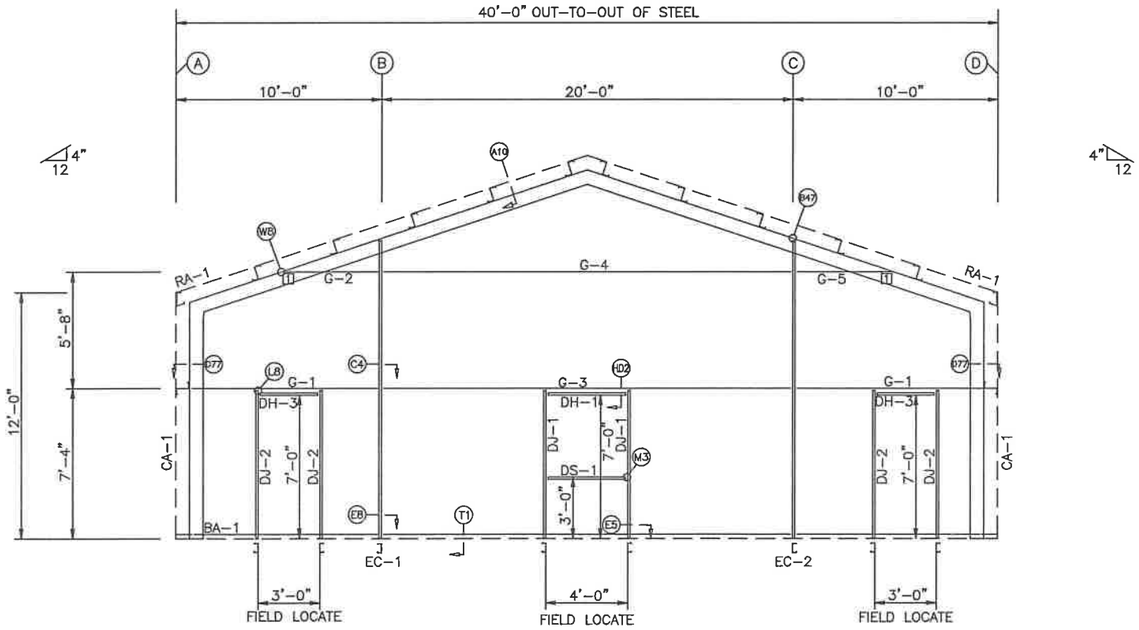
BOLT TABLE FRAME LINE 1 & 5				
LOCATION	QUAN	TYPE	DIA	LENGTH
Columns/Raf	2	A325	5/8"	2"

MEMBER TABLE FRAME LINE 1 & 5		
MARK	PART	LENGTH
EC-1	8X35C14	14'-6"
EC-2	8X35C14	14'-6"
DJ-1	8X25C16	7'-4"
DJ-2	8X25C16	7'-4"
DH-1	8.3.5CH6	4'-0"
DH-3	8X25C16	3'-0"
DS-1	8X25C16	4'-0"
G-1	8x25Z16	8'-11 1/2"
G-2	8x25Z16	4'-6 7/16"
G-3	8x25Z14	19'-11 1/2"
G-4	8x25Z16	19'-11 1/2"
G-5	8x25Z16	4'-6 7/16"

CONNECTION PLATES FRAME LINE 1 & 5	
☐ID	MARK/PART
1	SGC-1



ENDWALL FRAMING: FRAME LINE 1



ENDWALL FRAMING: FRAME LINE 5

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

ISSUE	DET	CHK	DATE

LMC STEEL

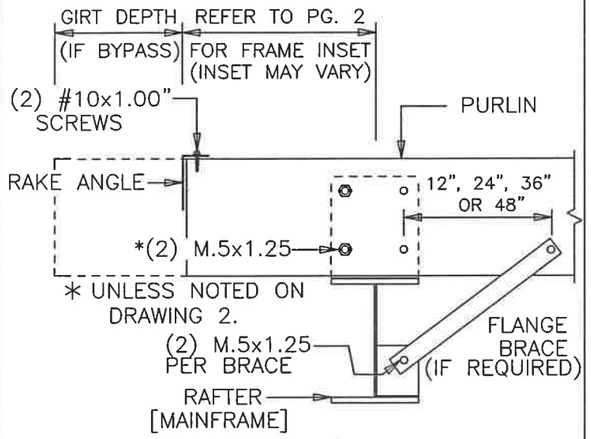
CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

JOB NO: 8546 | DATE: 4/ 7/24

LOCATION: FT. WHITE FL, 32038

DRAWING NAME: ENDWALL FRAMING LAYOUT

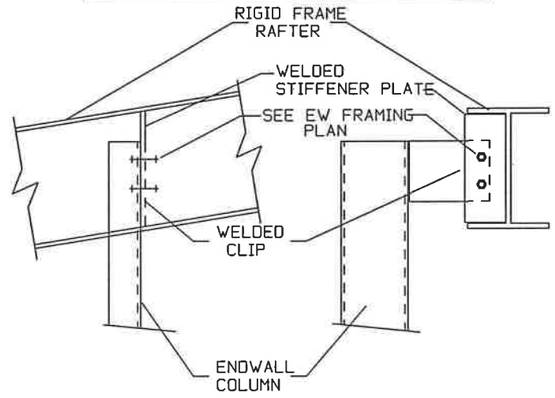
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 4	CTW	SPW	NONE



DETAIL A10

PURLIN TO ENDWALL RAFTER

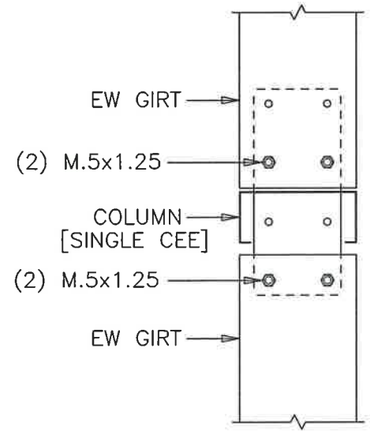
NOTE: HAND TIGHTEN EW COLUMN/RAFTER CONNECTION BOLTS, THEN INTERRUPT THREADS.



B47

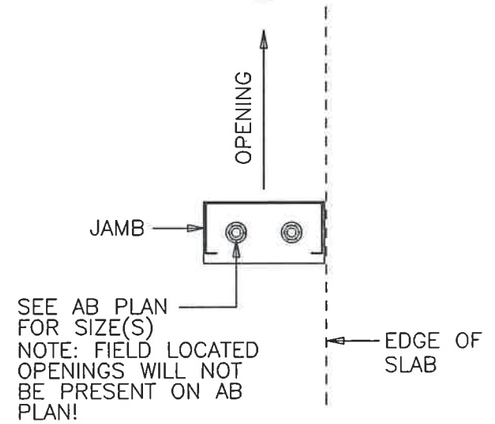
CEE COLUMN TO RIGID FRAME RAFTER

DETAIL C4

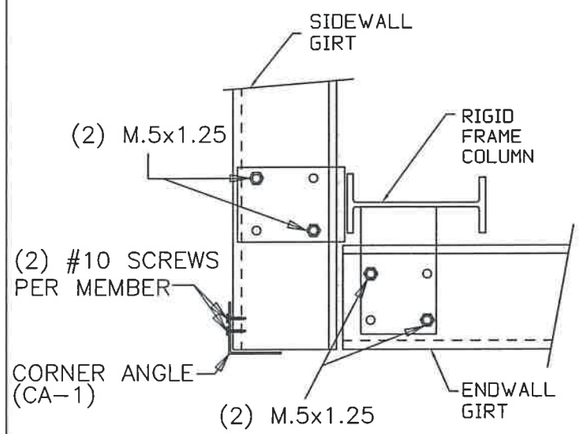


ENDWALL GIRTS TO INTERIOR COLUMN

DETAIL E5

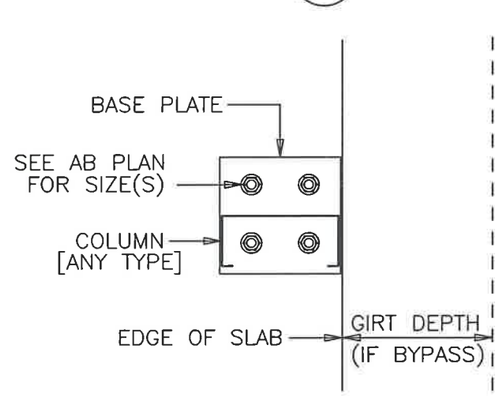


FRAMED OPENING JAMB BASE DETAIL



D77 CORNER COLUMN TO WALL GIRT

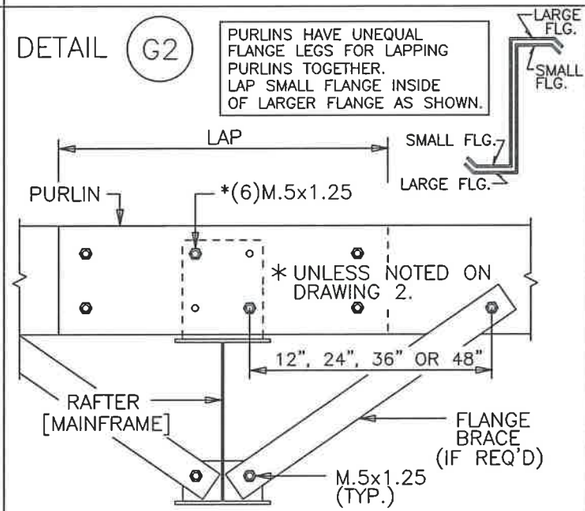
DETAIL E8



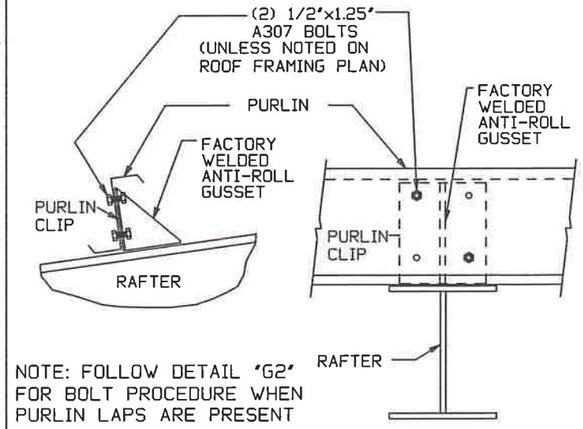
ENDWALL COLUMN BASE DETAIL

DETAIL G2

PURLINS HAVE UNEQUAL FLANGE LEGS FOR LAPPING PURLINS TOGETHER. LAP SMALL FLANGE INSIDE OF LARGER FLANGE AS SHOWN.



PURLIN TO MAINFRAME RAFTER



NOTE: FOLLOW DETAIL 'G2' FOR BOLT PROCEDURE WHEN PURLIN LAPS ARE PRESENT

G26 WELDED ANTI-ROLL CLIP

ISSUE	DET	CHK	DATE

LMC STEEL

CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

JOB NO: 8546 | DATE: 4/ 7/24

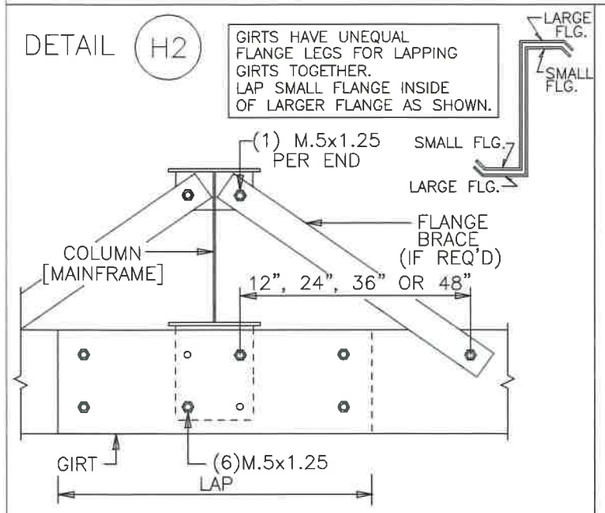
LOCATION: FT. WHITE FL, 32038

DRAWING NAME: FRAMING DETAILS

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 5	CTW	SPW	NONE

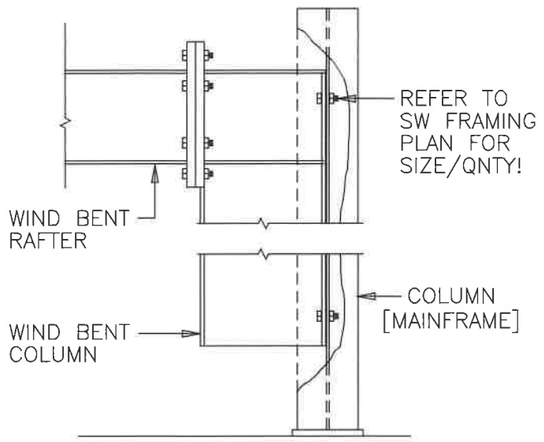
DETAIL H2

GIRTS HAVE UNEQUAL FLANGE LEGS FOR LAPPING GIRTS TOGETHER.
LAP SMALL FLANGE INSIDE OF LARGER FLANGE AS SHOWN.

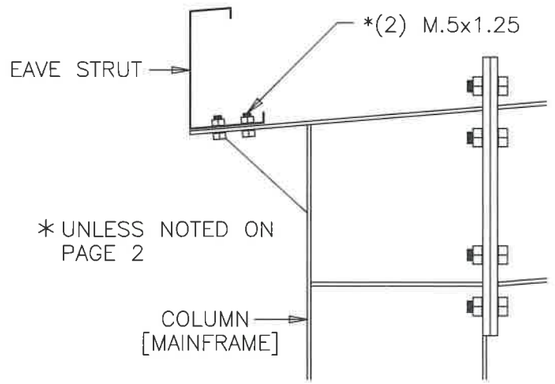


GIRT TO MAINFRAME COLUMN

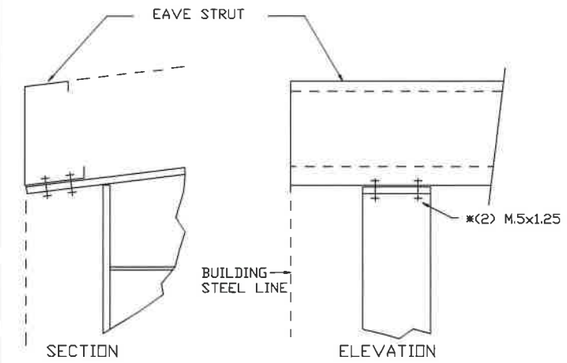
DETAIL H9



WIND BENT DETAIL



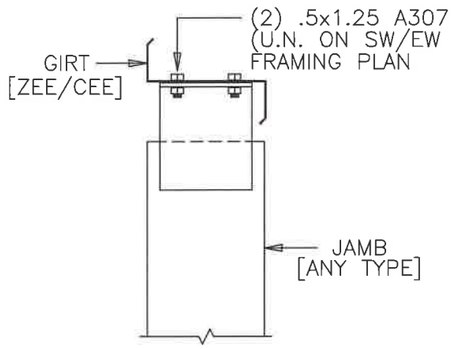
J2 EAVE STRUT CONNECTION AT MAINFRAME



* = UNLESS NOTED ON ROOF FRAMING PLAN

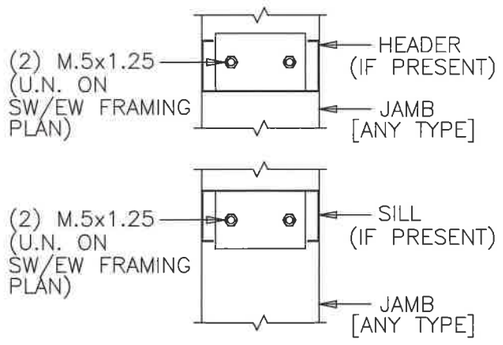
J24 EAVE STRUT TO RIGID FRAME

DETAIL L8



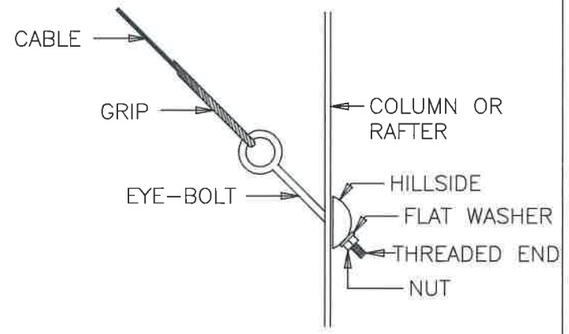
FRAMED OPENING JAMB TO GIRT

DETAIL M3



FRAMED OPENING HEADER/SILL TO JAMB

Q2 CABLE INSTALLATION DETAIL



NOTE: WHEN FLUSH GIRTS/PURLINS ARE USED, FIELD SLOT GIRT OR PURLIN AS REQ'D FOR CABLE/ROD PASSAGE THROUGH PURLIN/GIRT.

ISSUE	DET	CHK	DATE

LMC STEEL

CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

JOB NO: 8546 | DATE: 4/ 7/24

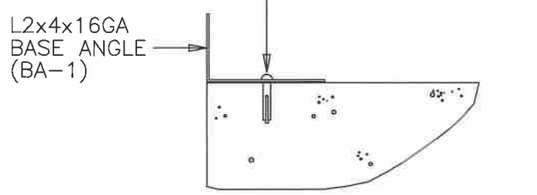
LOCATION: FT. WHITE FL, 32038

DRAWING NAME: FRAMING DETAILS

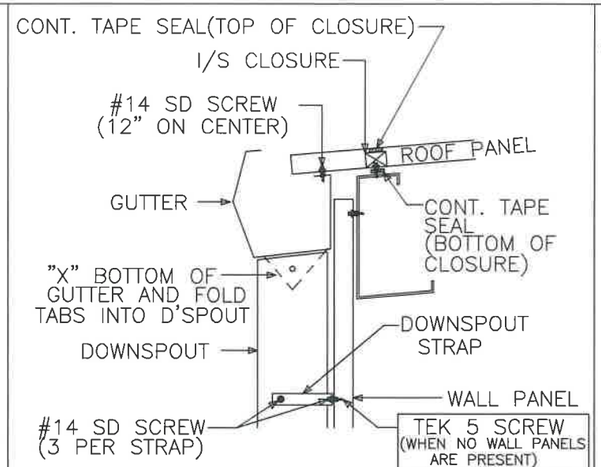
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 5.1	CTW	SPW	NONE

DETAIL (T1)

1/4" x 1 1/4" ZINC HAMMER DRIVES
ZAMAK ALLOY (ASTM B633, SC1, TYPE III)
(24" ON CENTER)

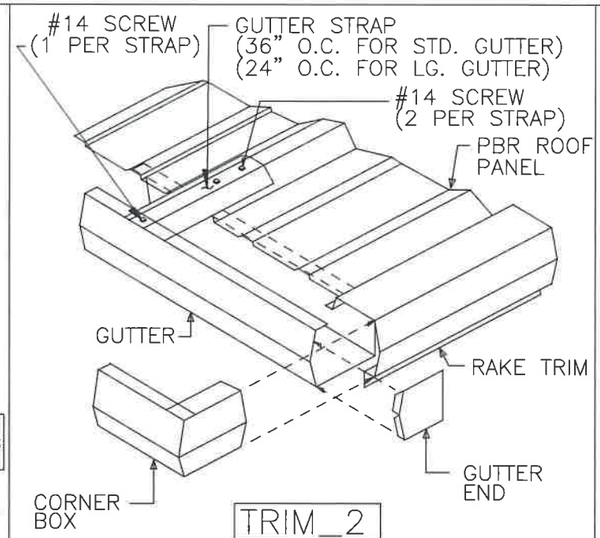


BASE ANGLE DETAIL

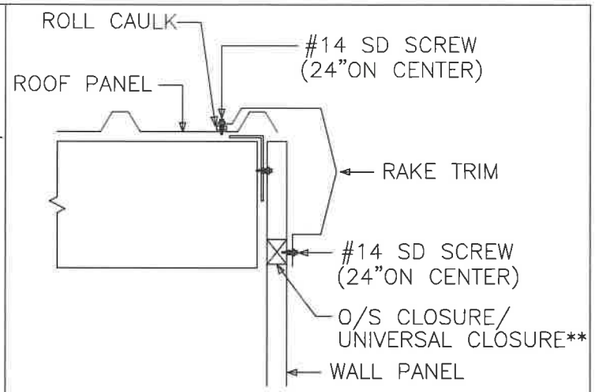


NOTE: INSTALL GUTTER STRAPS 3'-0" ON CENTER.
NOTE: INSTALL D'SPOUT STRAPS 5'-0" ON CENTER.

TRIM_1
GUTTER DETAIL



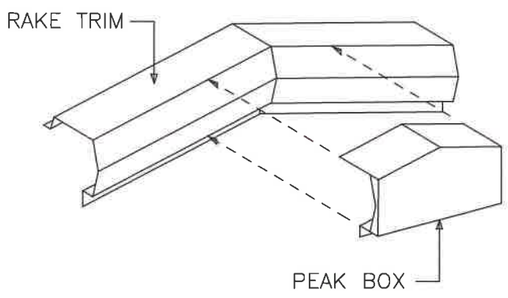
TRIM_2
GUTTER END DETAIL



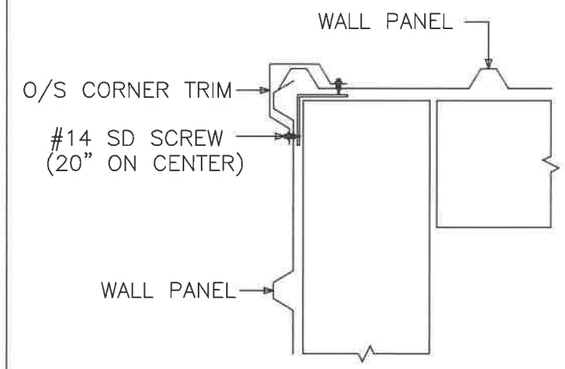
** - O/S CLOSURE FOR ROOF PITCHES UP TO 2:12"
UNIVERSAL CLOSURE FOR ROOF PITCHES ABOVE 2:12"

NOTE: FIELD CUT UNIVERSAL CLOSURES TO MATCH WALL PANEL PROFILE (ON PITCH).

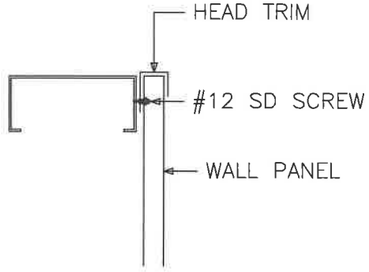
TRIM_3
RAKE TRIM DETAIL



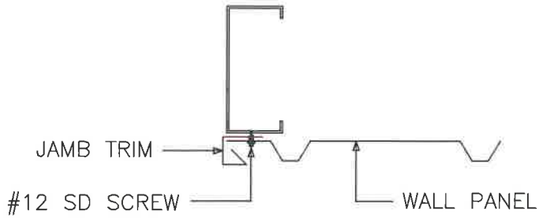
TRIM_4
PEAK BOX DETAIL



TRIM_5
O/S CORNER DETAIL



TRIM_7
HEAD TRIM DETAIL AT SILL



TRIM_8
JAMB TRIM DETAIL AT JAMB

ISSUE	DET	CHK	DATE

LMC STEEL

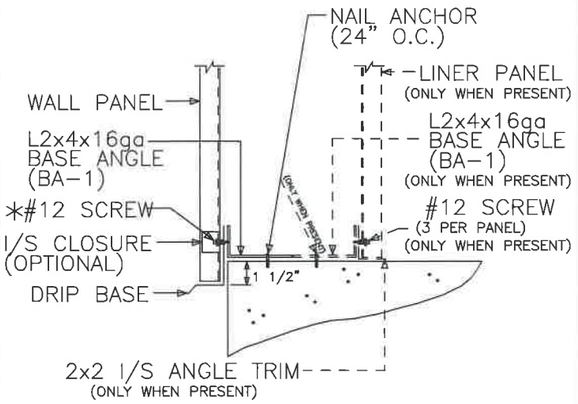
CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

JOB NO: 8546 | DATE: 4/ 7/24

LOCATION: FT. WHITE FL, 32038

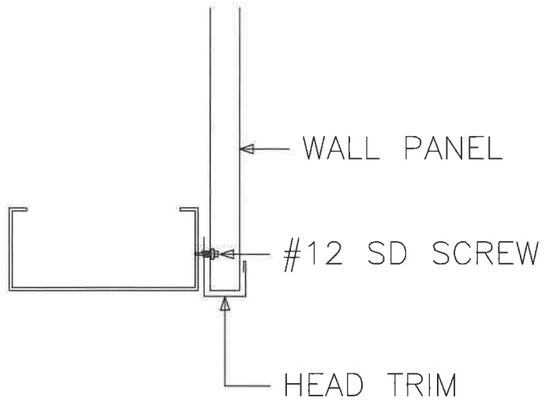
DRAWING NAME: FRAMING DETAILS

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 5.2	CTW	SPW	NONE



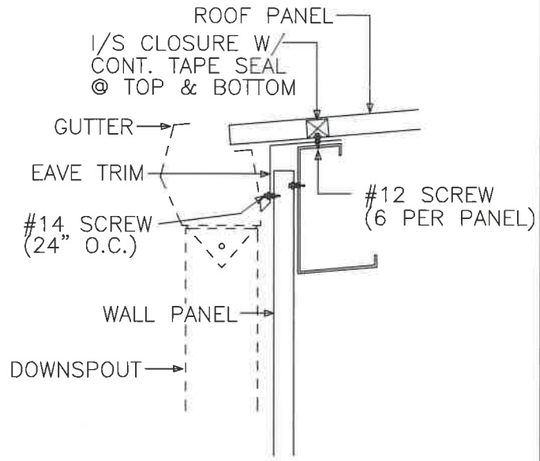
*= 6 PER PANEL FOR STANDARD PBR
3 PER PANEL FOR REV. ROLLED PBR

TRIM_16 BASE TRIM DETAIL



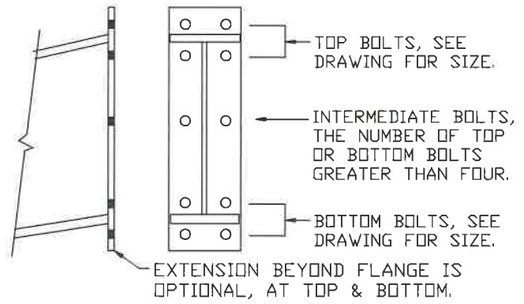
TRIM_61

HEAD TRIM DETAIL AT HEADER



TRIM_120

EAVE/GUTTER TRIM DETAIL



BOLTED END PLATE CONNECTION

STRUCTURAL BOLTED CONNNECTIONS

REFER TO COVER PAGE "GENERAL NOTES" PARAGRAPH "C", SECTION "9" FOR INSTRUCTIONS ON TIGHTENING ALL A325 AND A490 CONNECTION BOLTS.

TRIM NOTES:

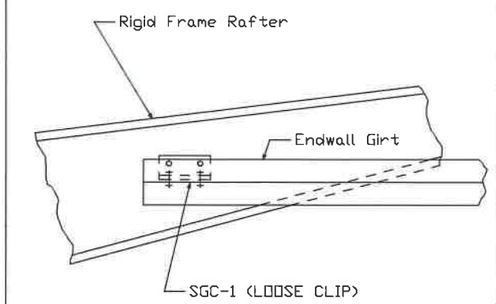
[1] SEAL TRIM SPLICES WITH TUBE CAULK.
[2] SECURE GUTTER SPLICES AND END PLUGS WITH RIVETS.
[3] SECURE ALL OTHER ROOF TRIM SPLICES WITH TRIM SCREWS UNLESS NOTED OTHERWISE.
[4] TRIM SCREWS ARE LOCATED 24" ON CENTER UNLESS NOTED OTHERWISE.
[5] STD. TRIM SPLICES ARE 3" TOTAL UNLESS NOTED OTHERWISE.

MORTISE PREPPED PERSONNEL DOORS

ALL MORTISE PREPPED PERSONNEL DOORS COME AS RIGHTHAND REVERSED SWING.

(i.e. STANDING ON THE OUTSIDE OF THE BUILDING FACING THE DOOR, THE LOCK WILL BE ON THE LEFTHAND SIDE OF THE DOOR AND THE DOOR WILL SWING OUTWARD FROM THE BUILDING.)

ANY FIELD MODIFICATIONS ARE THE RE-SPONSIBILITY OF THE ERECTOR AND MBM IS NOT LIABLE FOR LABOR CHARGES NOR DAMAGES DUE TO ERROR.



ATTACH WITH:
CLIP TO RAFTER (2) 1/2"x1.25" A307 BOLT/NUT
CLIP TO GIRT (2) 1/2"x1.25" A307 BOLT/NUT

SECTION OF ENDWALL GIRT TO RAFTER

W8

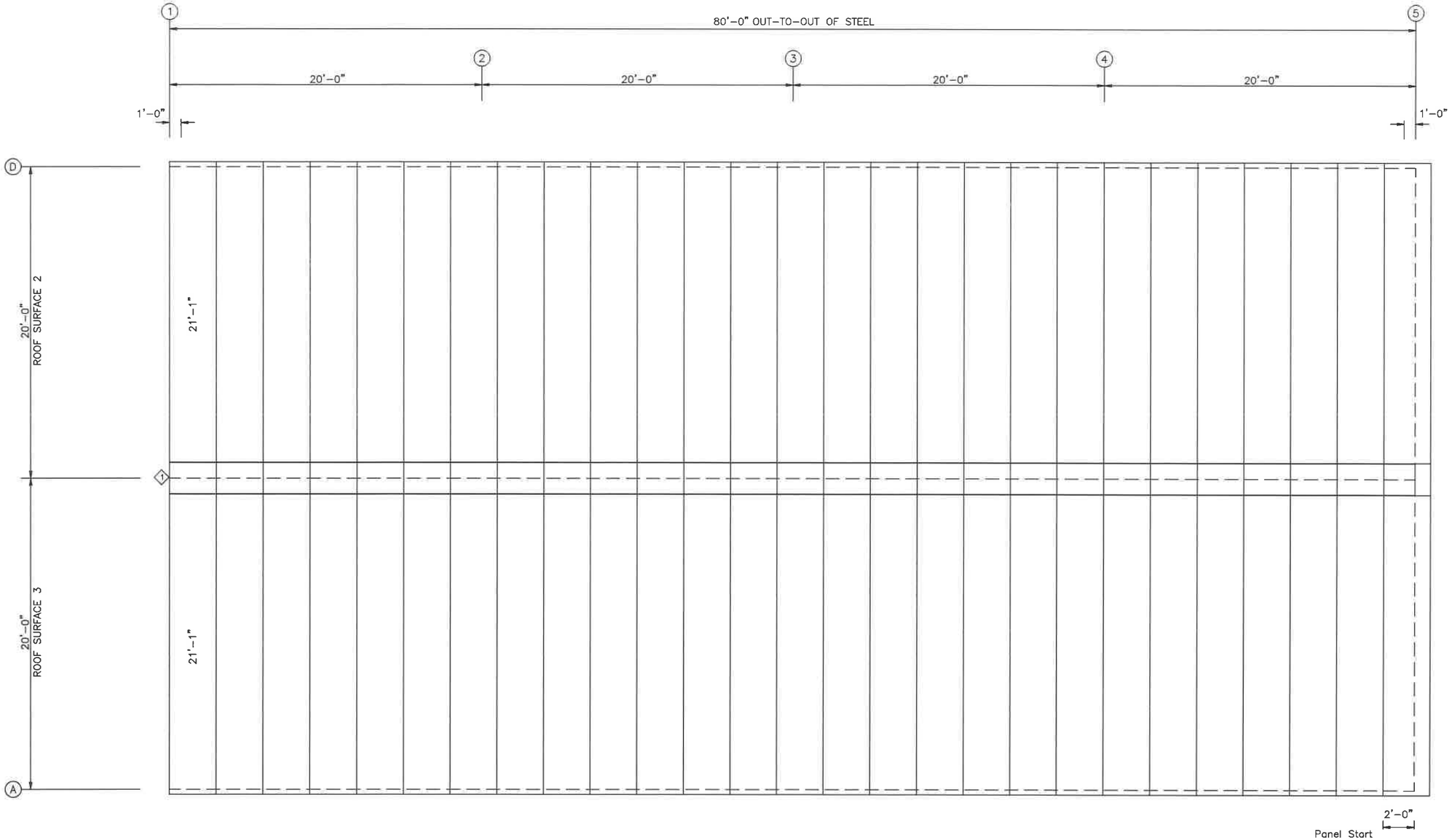
BUILT-UP MEMBER LEGEND

BEAM TYPE	BEAM DEPTH	FLANGE WIDTH	FLANGE THK.	WEB THK.
B	08	5	4	1
B= BUILT-UP	08= 8" 10= 10" 12= 12" 14= 14" ETC.	5,6,8,10 OR 12 (INCHES)	MEASURED IN 16ths. (4= 1/4", 5= 5/16" ETC.)	1= 10ga 3= 3/16" ETC.

ISSUE	DET	CHK	DATE

LMC STEEL

CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.
JOB NO: 8546 DATE: 4/ 7/24
LOCATION: FT. WHITE FL, 32038
DRAWING NAME: FRAMING DETAILS
DRAWING NO: PAGE 5.3 DRAWN BY: CTW CHECKED BY: SPW SCALE: NONE



ROOF SHEETING PLAN

PANELS: 26 GA. PBR – NEED COLOR

TRIM TABLE ROOF PLAN		
◇ID	PART	LENGTH
1	D/F CAP6	3'-0"

ISSUE	DET	CHK	DATE

LMC STEEL

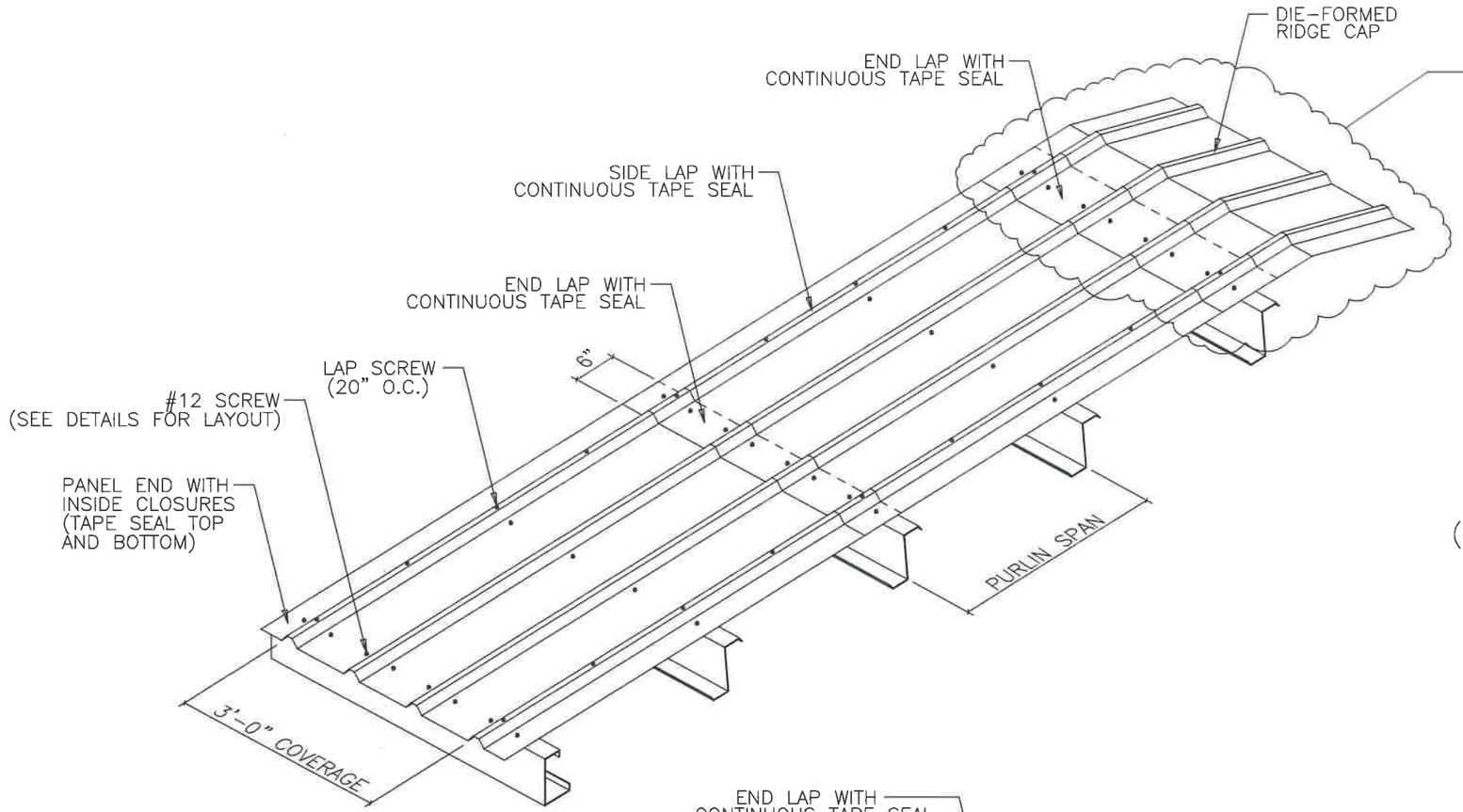
CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

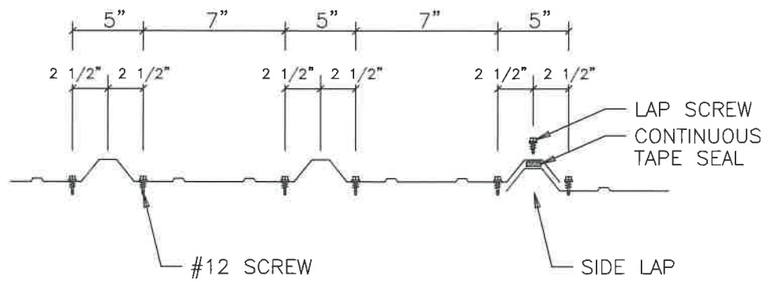
JOB NO: 8546 | DATE: 4/ 7/24

LOCATION: FT. WHITE FL, 32038

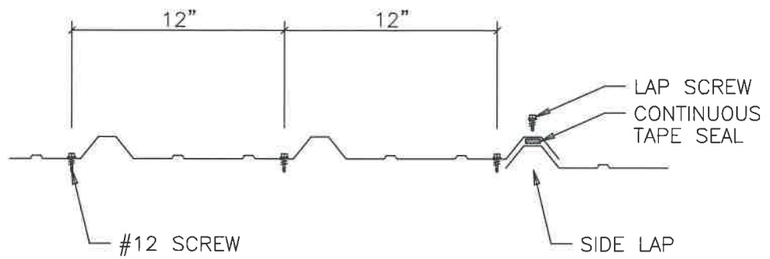
DRAWING NAME: ROOF PANELS & TRIM

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 6	CTW	SPW	NONE

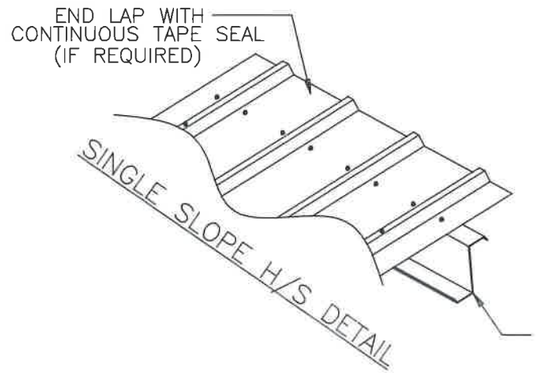




PANEL ATTACHMENT AT PANEL END
(PEAK PURLIN, EAVE STRUT, AND PANEL END LAPS)



PANEL ATTACHMENT AT INTERMEDIATE MEMBERS



NOTES:

[1] ALL END LAPS MUST BE A MINIMUM OF 6".

[2] METAL SHAVINGS MUST BE SWEPT FROM THE ROOF EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.

[3] TAPE SEAL MUST BE APPLIED WITH NO GAPS OR BREAKS.

[4] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE PURLINS. #14 LAP SCREWS ARE USED AT THE PANEL-TO-PANEL ATTACHMENTS. ALL FASTENERS ARE SELF-DRILLING.

ISSUE	DET	CHK	DATE

LMC STEEL

CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

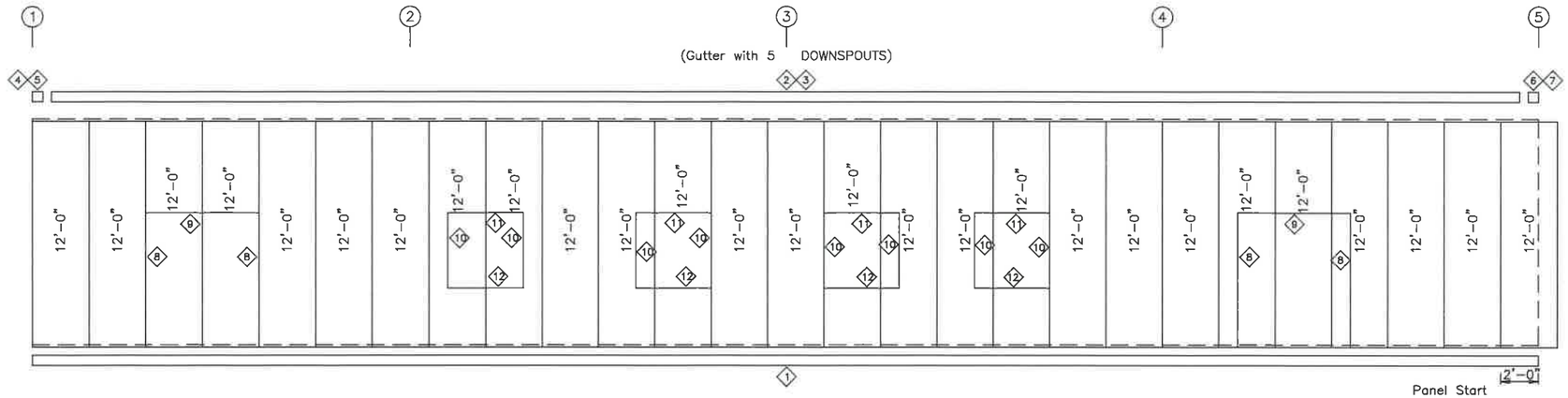
JOB NO: 8546 | DATE: 4/ 7/24

LOCATION: FT. WHITE FL, 32038

DRAWING NAME: ROOF PANEL DETAILS

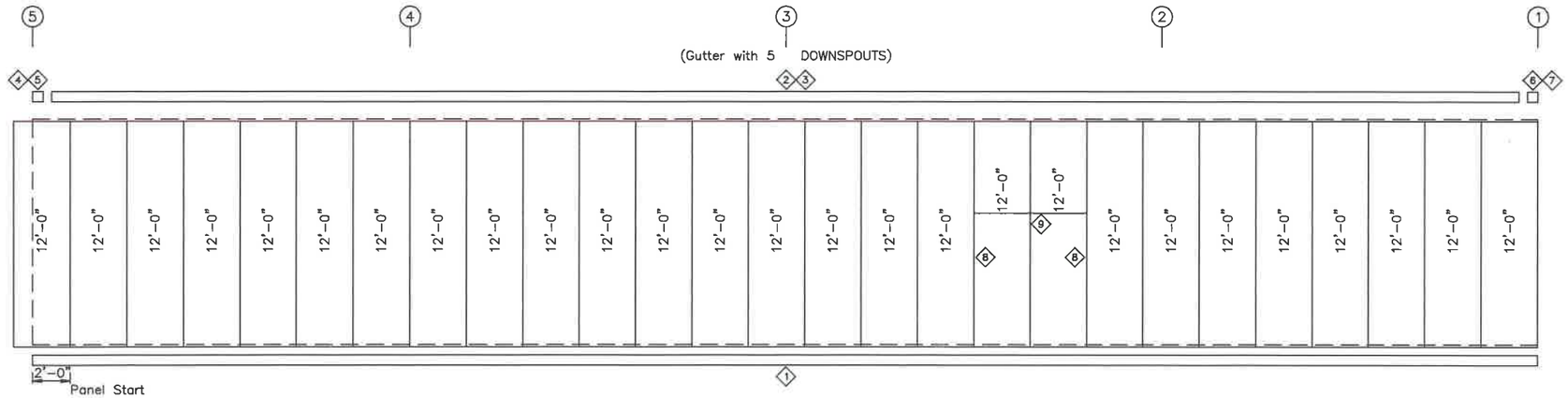
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 6.1	CTW	SPW	NONE

TRIM TABLE FRAME LINE A & D			
◇ID	PART	LENGTH	DETAIL
1	DRIP BASE	20'-3"	TRIM_16
2	GUTTER	20'-3"	TRIM_1
3	EAVE TRM	20'-3"	TRIM_120
4	GUTEND L	1"	TRIM_2
5	CORBOX L	1'-0"	TRIM_2
6	GUTEND R	1"	TRIM_2
7	CORBOX R	1'-0"	TRIM_2
8	R JAMB	7'-3"	TRIM_8
9	R HEAD	6'-3"	TRIM_61
10	R JAMB	4'-3"	TRIM_8
11	R HEAD	4'-3"	TRIM_61
12	R HEAD	4'-3"	TRIM_7



SIDEWALL SHEETING & TRIM: FRAME LINE A

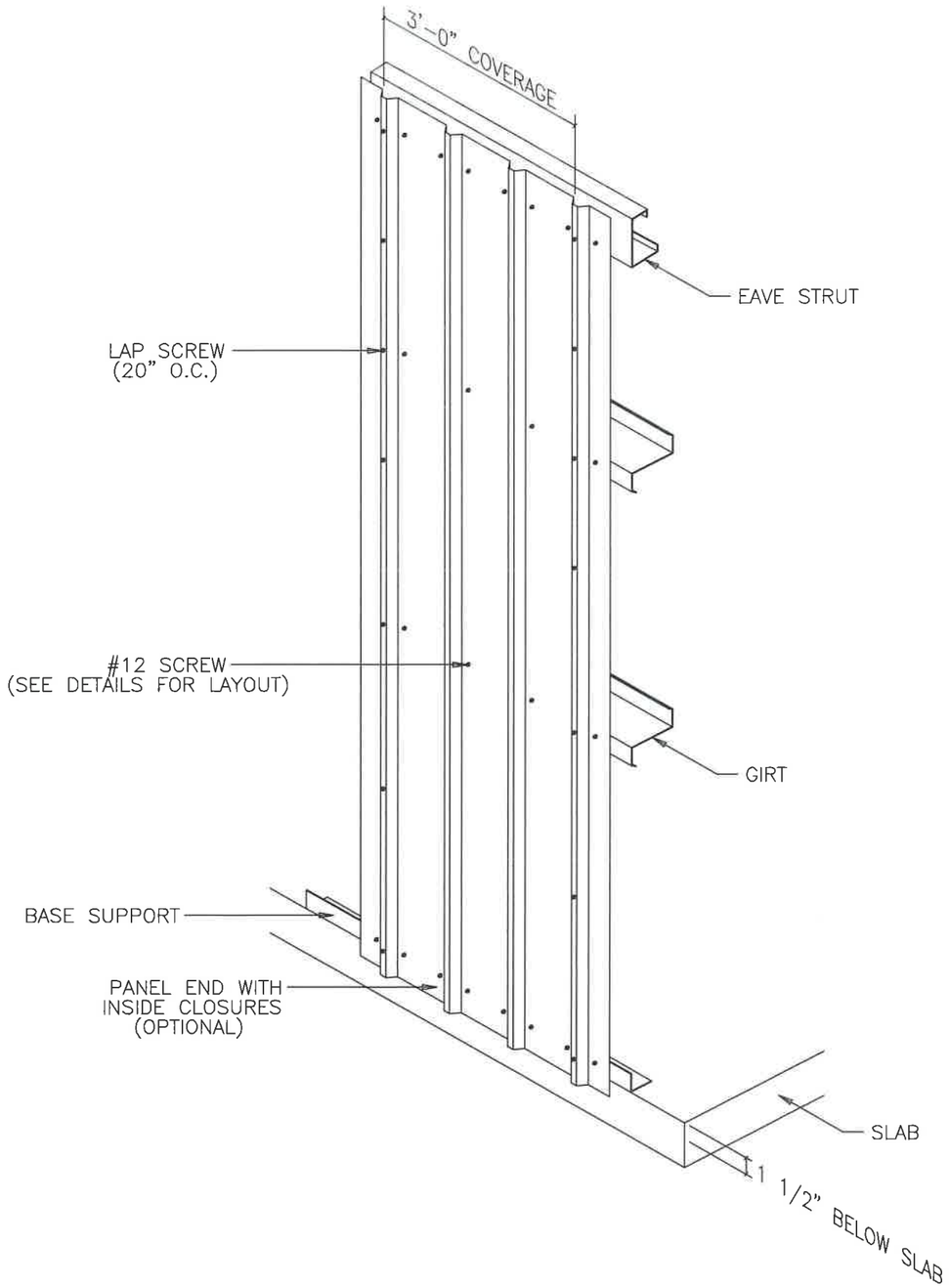
PANELS: 26 GA. PBR – COLOR

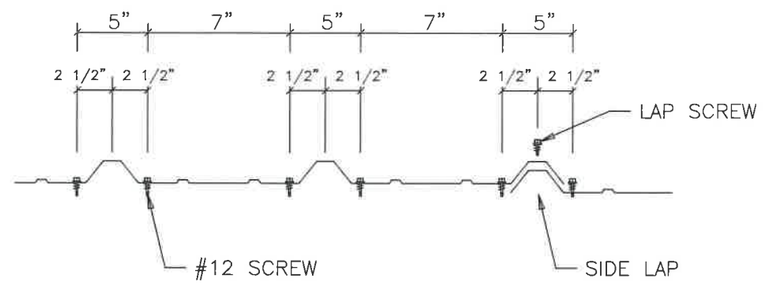


SIDEWALL SHEETING & TRIM: FRAME LINE D

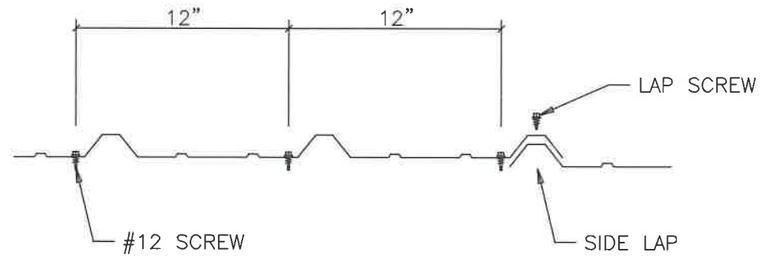
PANELS: 26 GA. PBR – COLOR

ISSUE	DET	CHK	DATE
LMC STEEL			
CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.			
JOB NO: 8546	DATE: 4/ 7/24		
LOCATION: FT. WHITE FL, 32038			
DRAWING NAME: SIDEWALL PANELS & TRIM			
DRAWING NO: PAGE 7	DRAWN BY: CTW	CHECKED BY: SPW	SCALE: NONE

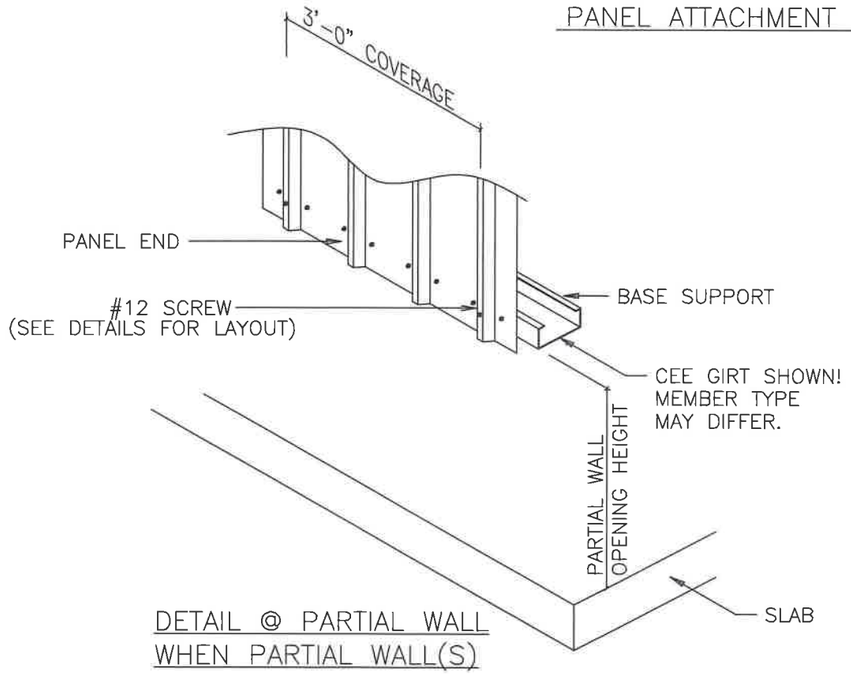




PANEL ATTACHMENT AT PANEL END
(BASE, EAVE STRUT, HEADER, SILL, AND PANEL END LAPS)



PANEL ATTACHMENT AT INTERMEDIATE MEMBERS



DETAIL @ PARTIAL WALL
WHEN PARTIAL WALL(S)
ARE PRESENT

NOTES:

[1] METAL SHAVINGS MUST BE SWEPT FROM THE WALL EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.

[2] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE GIRTS. #14 LAP SCREWS ARE USED AT THE PANEL-TO-PANEL ATTACHMENTS. ALL FASTENERS ARE SELF-DRILLING.

ISSUE	DET	CHK	DATE

LMC STEEL

CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

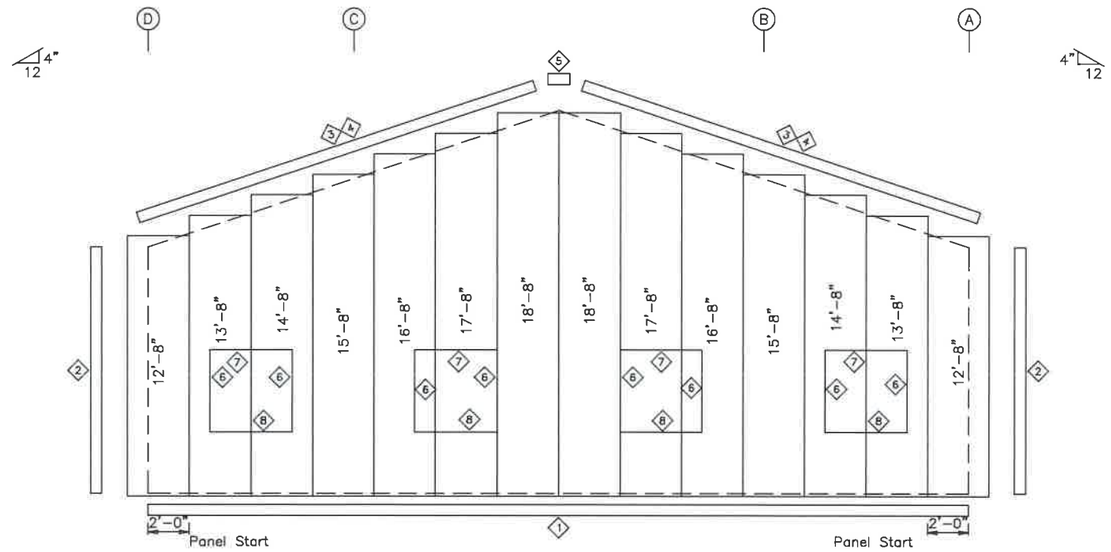
JOB NO: 8546 | DATE: 4/ 7/24

LOCATION: FT. WHITE FL, 32038

DRAWING NAME: SIDEWALL PANEL DETAILS

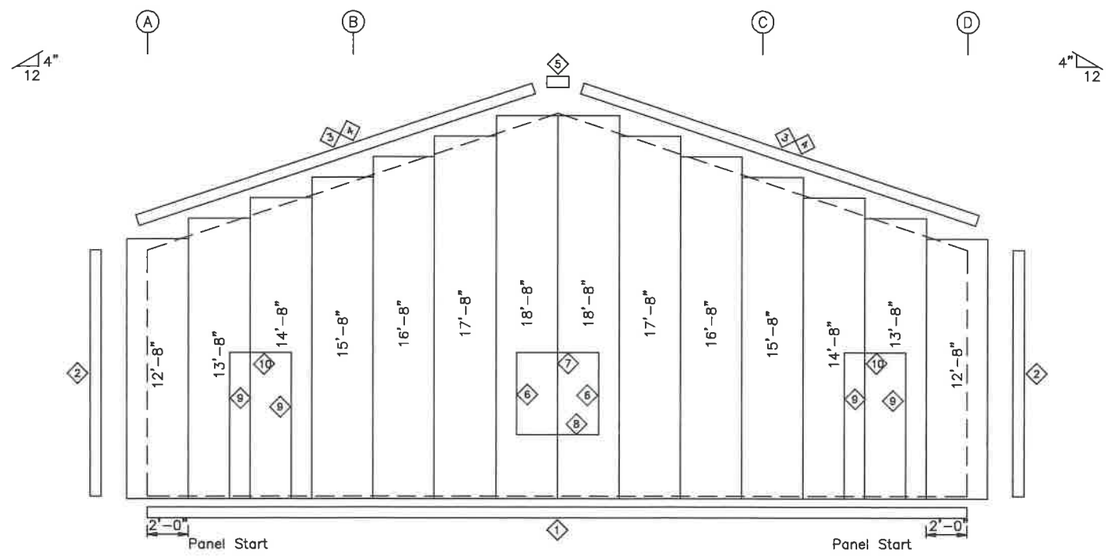
DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 7.1	CTW	SPW	NONE

TRIM TABLE FRAME LINE 1 & 5			
◇ID	PART	LENGTH	DETAIL
1	DRIP BASE	20'-3"	TRIM_16
2	O/S CORN	12'-2"	TRIM_5
3	RAKE TRM	20'-3"	TRIM_3
4	RAKE TRM	1'-1"	TRIM_3
5	PEAK BOX	1'-4"	TRIM_4
6	R JAMB	4'-3"	TRIM_8
7	R HEAD	4'-3"	TRIM_61
8	R HEAD	4'-3"	TRIM_7
9	R JAMB	7'-3"	TRIM_8
10	R HEAD	3'-3"	TRIM_61



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 GA. PBR – COLOR



ENDWALL SHEETING & TRIM: FRAME LINE 5

PANELS: 26 GA. PBR – COLOR

NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

ISSUE	DET	CHK	DATE

LMC STEEL

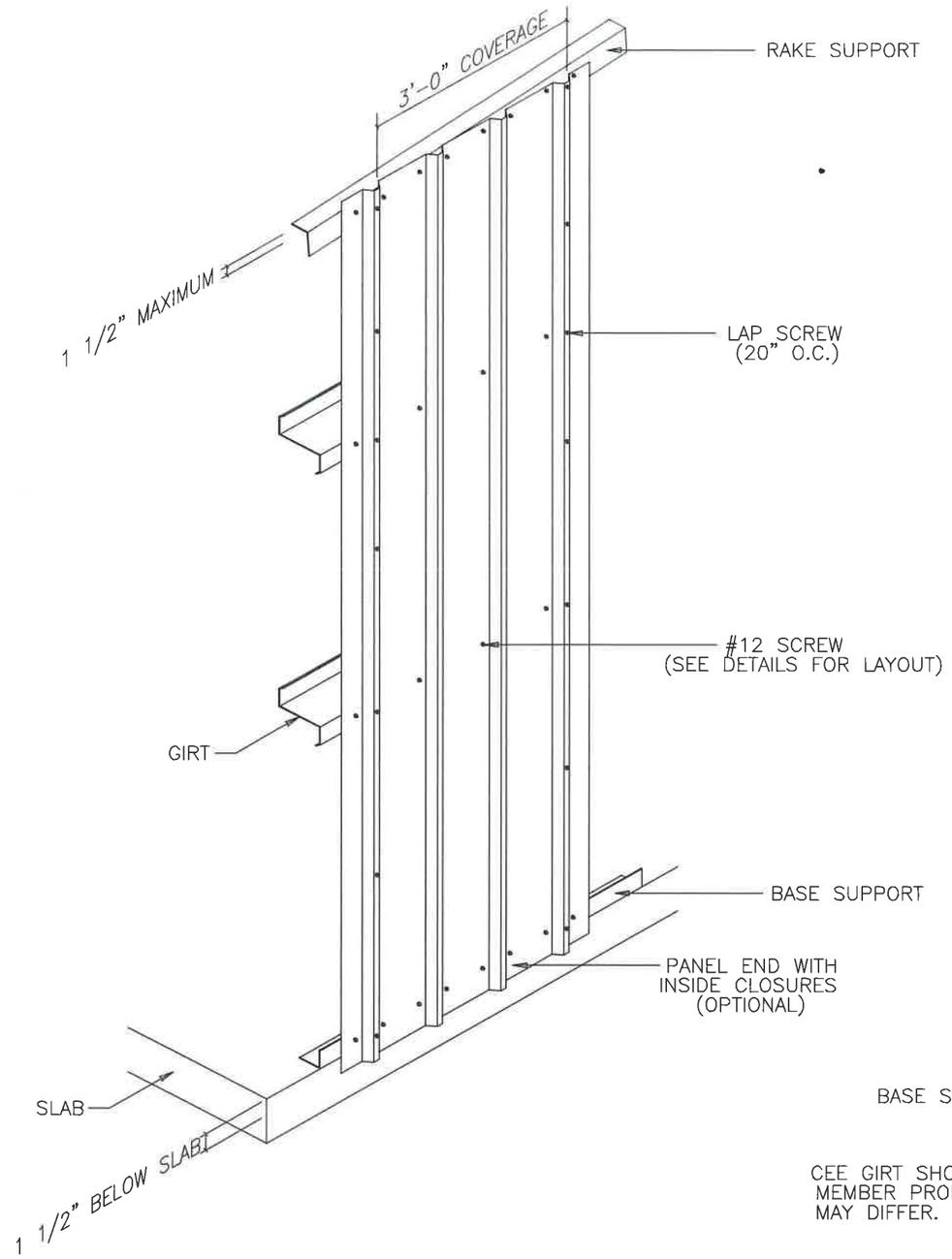
CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

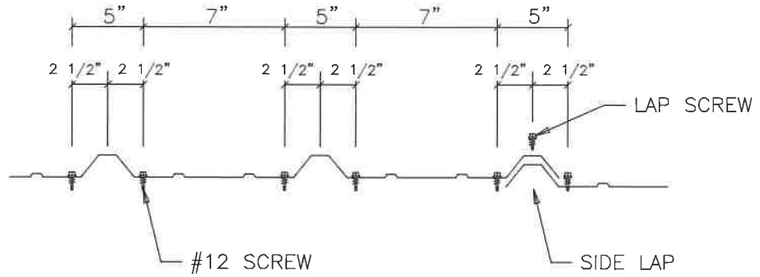
JOB NO: 8546 | DATE: 4/ 7/24

LOCATION: FT. WHITE FL, 32038

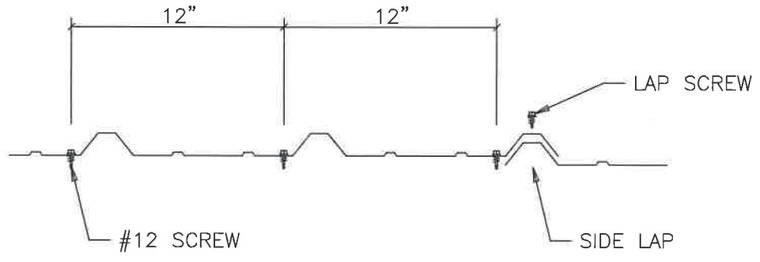
DRAWING NAME: ENDWALL PANELS & TRIM

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 8	CTW	SPW	NONE

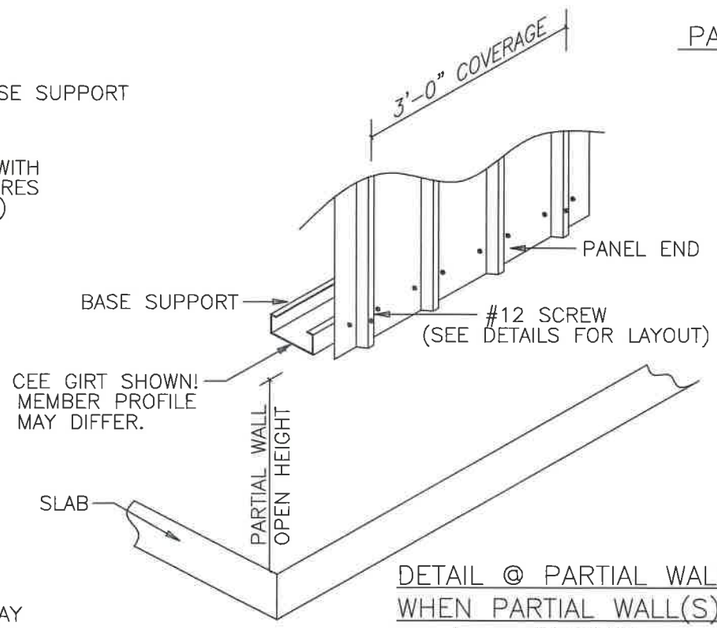




PANEL ATTACHMENT AT PANEL END
(BASE, EAVE STRUT, HEADER, SILL, AND PANEL END LAPS)



PANEL ATTACHMENT AT INTERMEDIATE MEMBERS



DETAIL @ PARTIAL WALL
WHEN PARTIAL WALL(S)
ARE PRESENT

NOTES:

[1] METAL SHAVINGS MUST BE SWEPT FROM THE WALL EACH DAY DURING ERECTION TO PREVENT SURFACE RUSTING.

[2] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE GIRTS. #14 LAP SCREWS ARE USED AT THE PANEL-TO-PANEL ATTACHMENTS. ALL FASTENERS ARE SELF-DRILLING.

ISSUE	DET	CHK	DATE

LMC STEEL

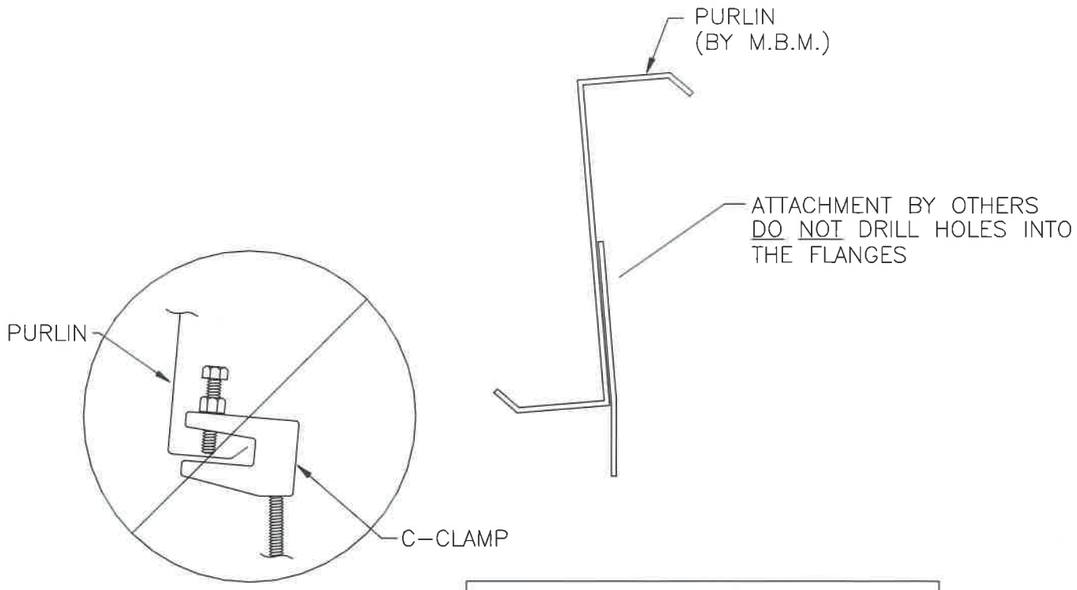
CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

JOB NO: 8546 | DATE: 4/ 7/24

LOCATION: FT. WHITE FL, 32038

DRAWING NAME: ENDWALL PANEL DETAILS

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 8.1	CTW	SPW	NONE



Flange C-Clamp is not an acceptable connection

NOTE: M.B.M. only provides the roof purlin. All other material and hardware is by others.

Recommended Connection Detail

NOTE

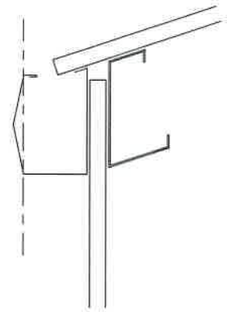
MANY FACTORS BEYOND THE CONTROL OF THE METAL BUILDING SUPPLIER AFFECT THE ABILITY OF A PURLIN TO SAFELY SUPPORT HANGING LOADS COMBINED WITH OTHER REQUIRED ROOF LOADS. DUE TO THE VARIABLES INVOLVED IN HANGING LOADS AND THEIR ATTACHMENTS TO THE PURLINS, THE METAL BUILDING SUPPLIER CANNOT ASSURE THAT THE PURLINS FOR A PARTICULAR BUILDING PROJECT CAN SAFELY SUPPORT THE MAXIMUM ALLOWABLE HANGING LOADS IN COMBINATION WITH OTHER ROOF LOADS.

IT IS THE RESPONSIBILITY OF THE HANGER SYSTEM INSTALLER TO COORDINATE WITH THE ENGINEER OF RECORD FOR THE OVERALL PROJECT TO ENSURE A SAFE HANGING LOAD INSTALLATION. THE METAL BUILDING ENGINEER IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT. WITHOUT SPECIFIC CERTIFICATION FOR INDIVIDUAL HANGING LOADS, THE NET EFFECTS OF APPLIED HANGER LOADS INSTALLED ON A PARTICULAR PURLIN SHALL NOT EXCEED THE NET EFFECTS OF THE CERTIFIED UNIFORMLY APPLIED DESIGN COLLATERAL LOAD.

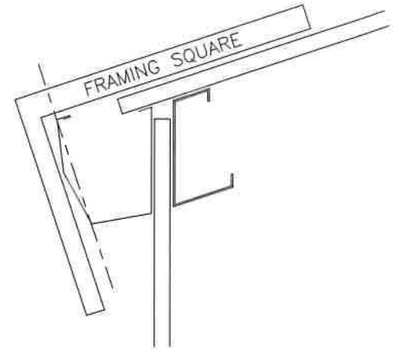
HANGING LOADS SHOULD NOT BE APPLIED TO THE PURLIN LIP. WHERE PERMISSIBLE, THE BEST PRACTICE FOR HANGING LOADS IS TO ATTACH TO THE PURLIN WEB USING A BOLT AND NUT, OR SELF-DRILLING SCREWS.

HANGING UNIFORM LOADS SUCH AS SPRINKLER MAINS OR HVAC EQUIPMENT SHOULD BE DISTRIBUTED OVER SEVERAL PURLINS, AND SHOULD NEVER EXCEED THE COLLATERAL LOAD ALLOWANCE FOR THE ROOF SYSTEM. FOR UNIFORM LOADS THAT RUN PARALLEL TO THE PURLINS, IT MAY BE NECESSARY TO USE TRANSVERSE SUPPORT CHANNELS(A.KA. TRAPEZE BEAMS) ATTACHED TO THE WEBS OR FLANGES OF ADJACENT PURLINS TO SPREAD THE LOAD BETWEEN TWO OR MORE PURLINS. IN SUCH CASES, CONTACT THE BUILDING MANUFACTURER OR A LOCAL PROFESSIONAL ENGINEER PRIOR TO ATTEMPTING TO HANG LOADS FROM THE PURLINS

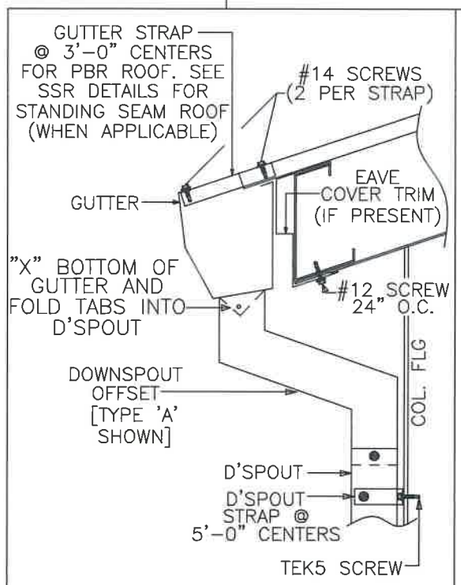
GUTTER INSTALLATION DETAIL (ONLY IF PROVIDED)



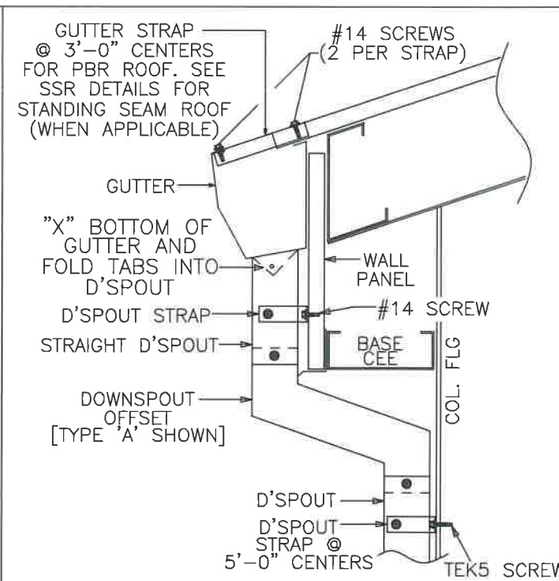
DO NOT INSTALL GUTTER WITH OUTSIDE FACE PERPENDICULAR TO THE GROUND.



INSTALL GUTTER WITH OUTSIDE FACE PERPENDICULAR TO THE ROOF.



@ OPEN WALLS W/O WALL PANELS



@ PARTIAL WALLS W/ WALL PANELS

NOTE: REGARDLESS OF DOWNSPOUT OFFSET SCENARIO, TEK5 SCREWS MUST BE USED TO ATTACH DOWNSPOUT STRAPS TO PEMB FRAMING. WHEN WALL PANELS SPAN FROM GROUND TO EAVE (FULL SPAN), #14 SCREWS WILL BE USED TO ATTACH DOWNSPOUT STRAPS TO WALL PANELS.

ISSUE	DET	CHK	DATE

LMC STEEL

CUSTOMER: BETHLEHEM PARK ASSEMBLY BLDG.

JOB NO: 8546 | DATE: 4/ 7/24

LOCATION: FT. WHITE FL, 32038

DRAWING NAME: SPECIAL DETAILS

DRAWING NO:	DRAWN BY:	CHECKED BY:	SCALE:
PAGE 9	CTW	SPW	NONE