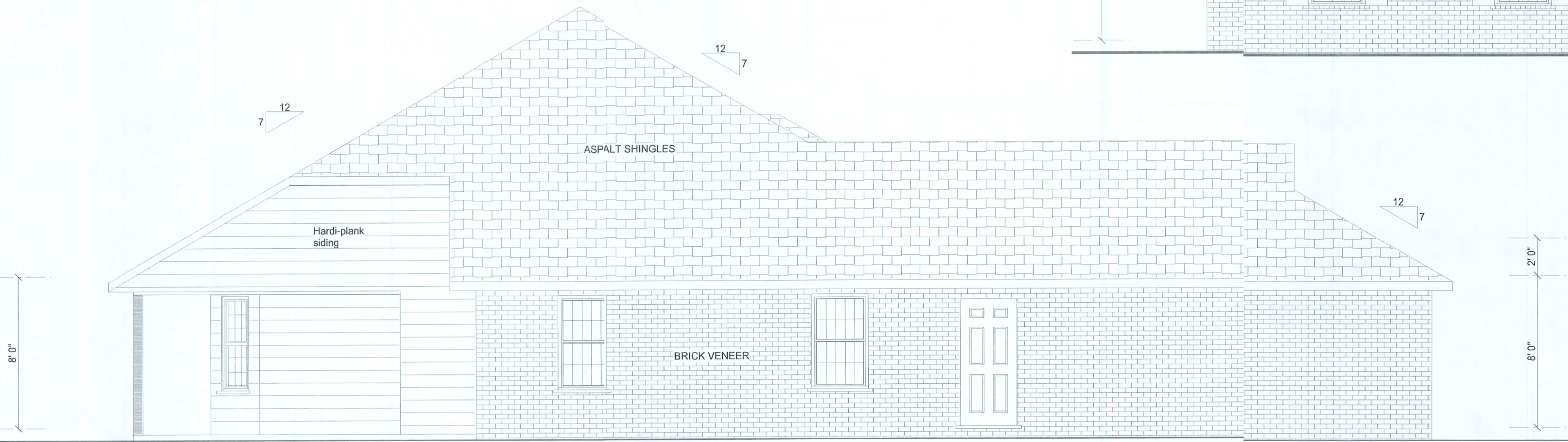
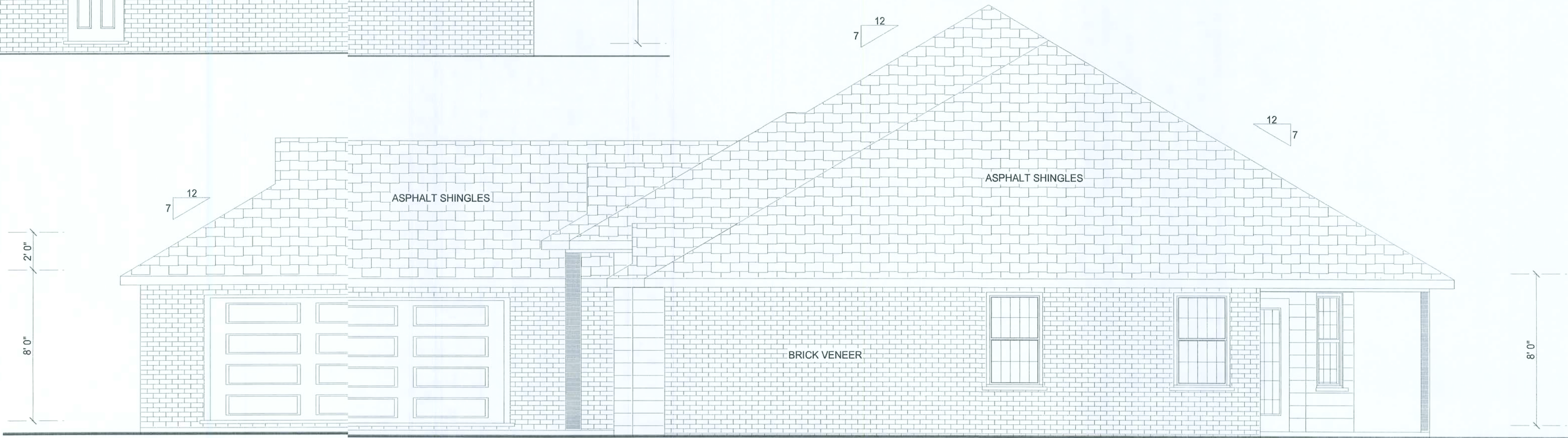


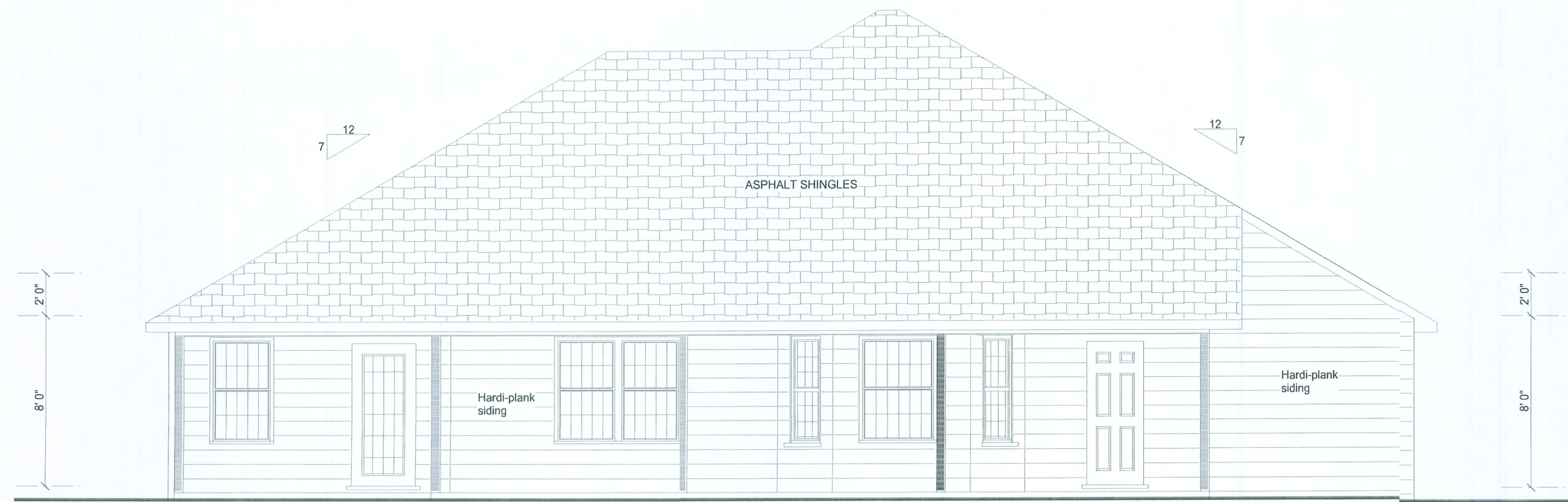
FRONT ELEVATION



LEFT ELEVATION



RIGHT ELEVATION



REAR ELEVATION

NOTES:

ALL WORK SHALL COMPLY WITH
THE STANDARD BUILDING CODE
AND ALL OTHER APPLICABLE
LOCAL CODES AND ORDINANCES

AREA SUMMARY
Living Area 1777 SF
Garage 543 SF
Porches 585 SF
TOTAL AREA 2905 SF

Stephen Crawford Construction, Inc. (Mason City Spec)

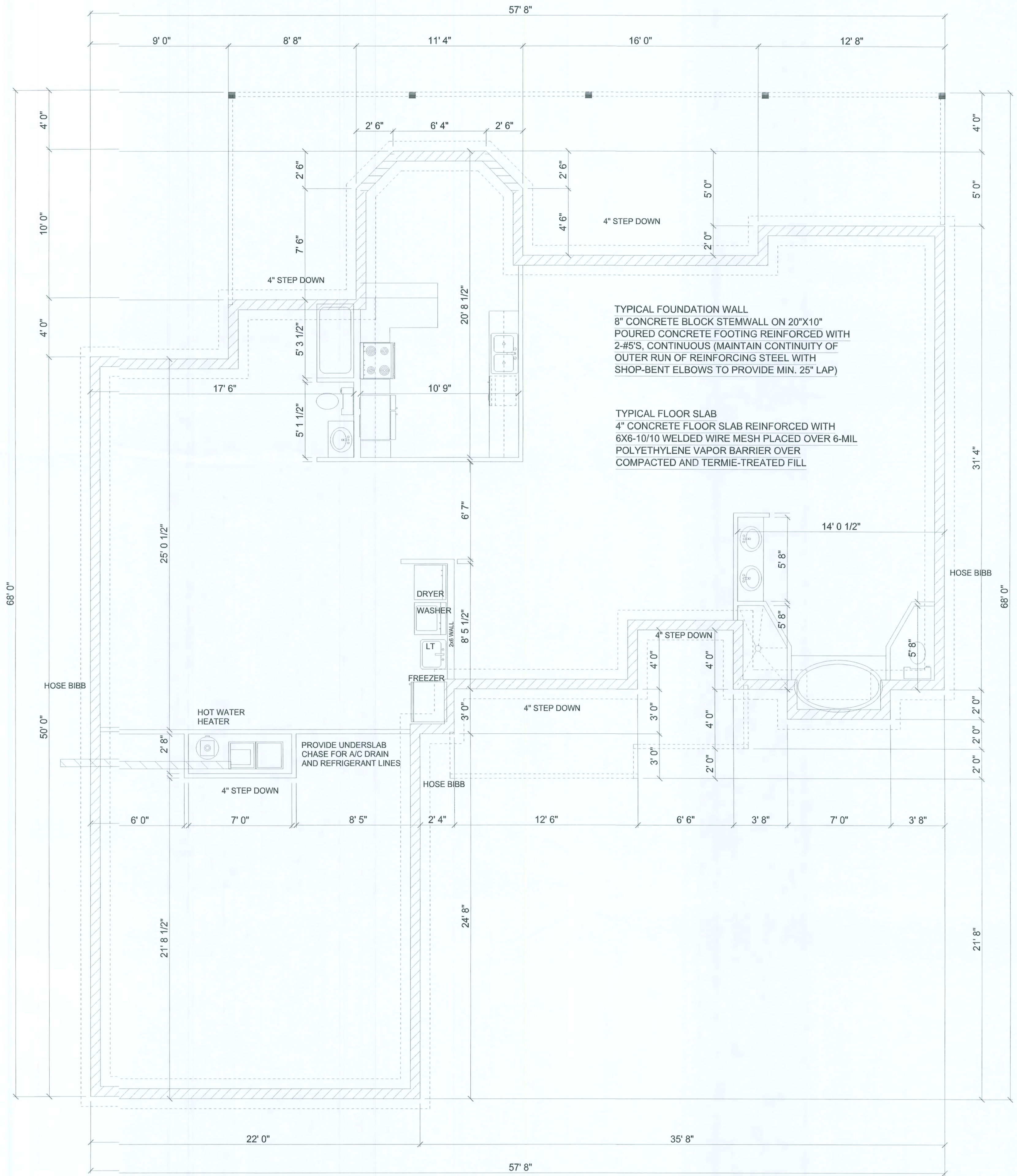
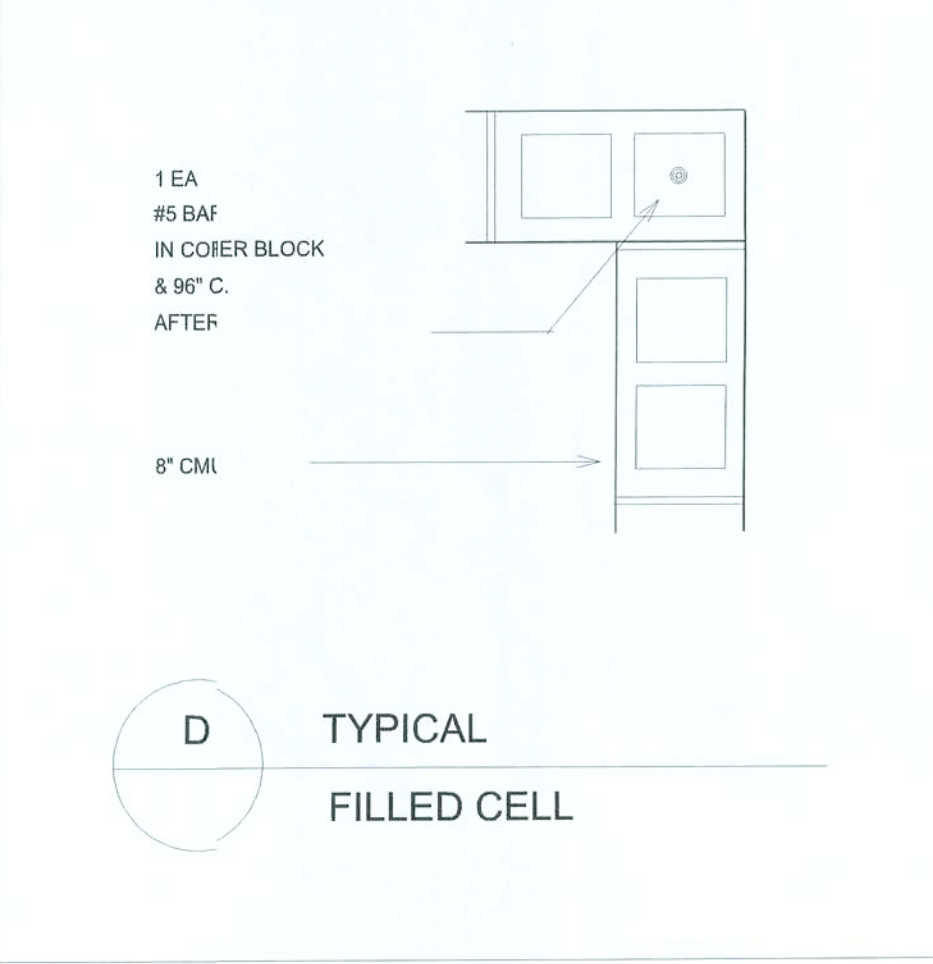
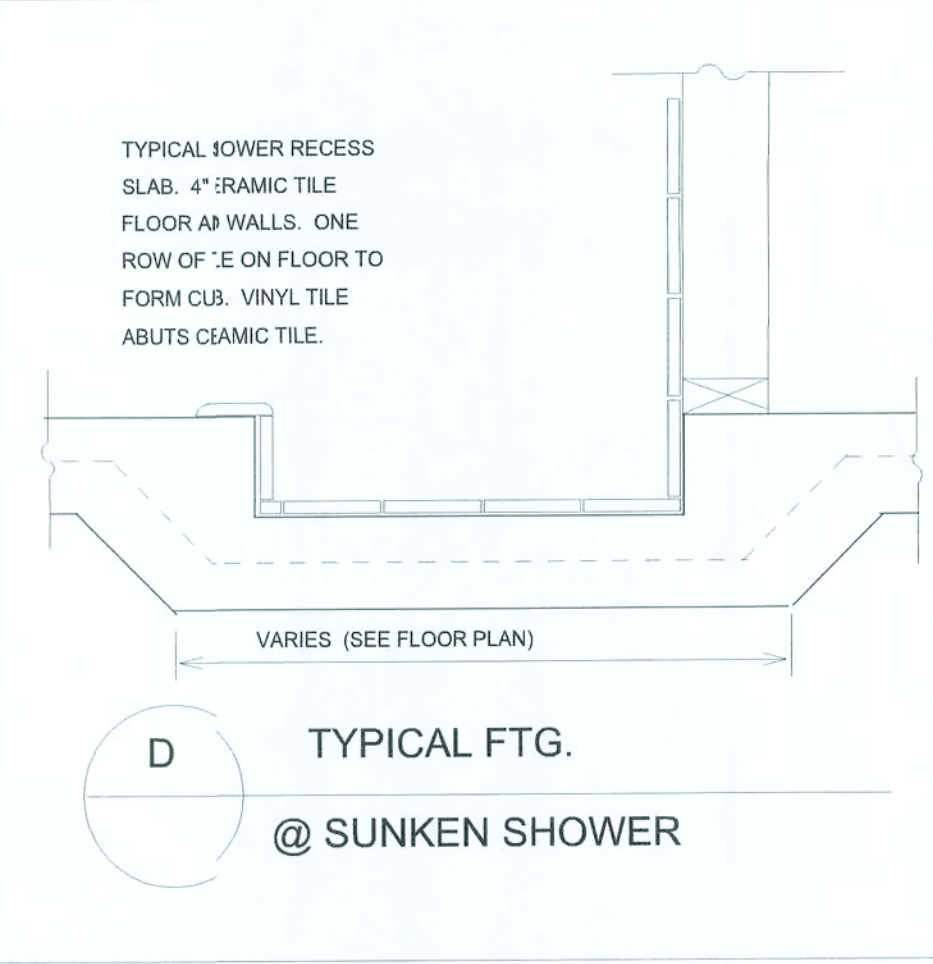
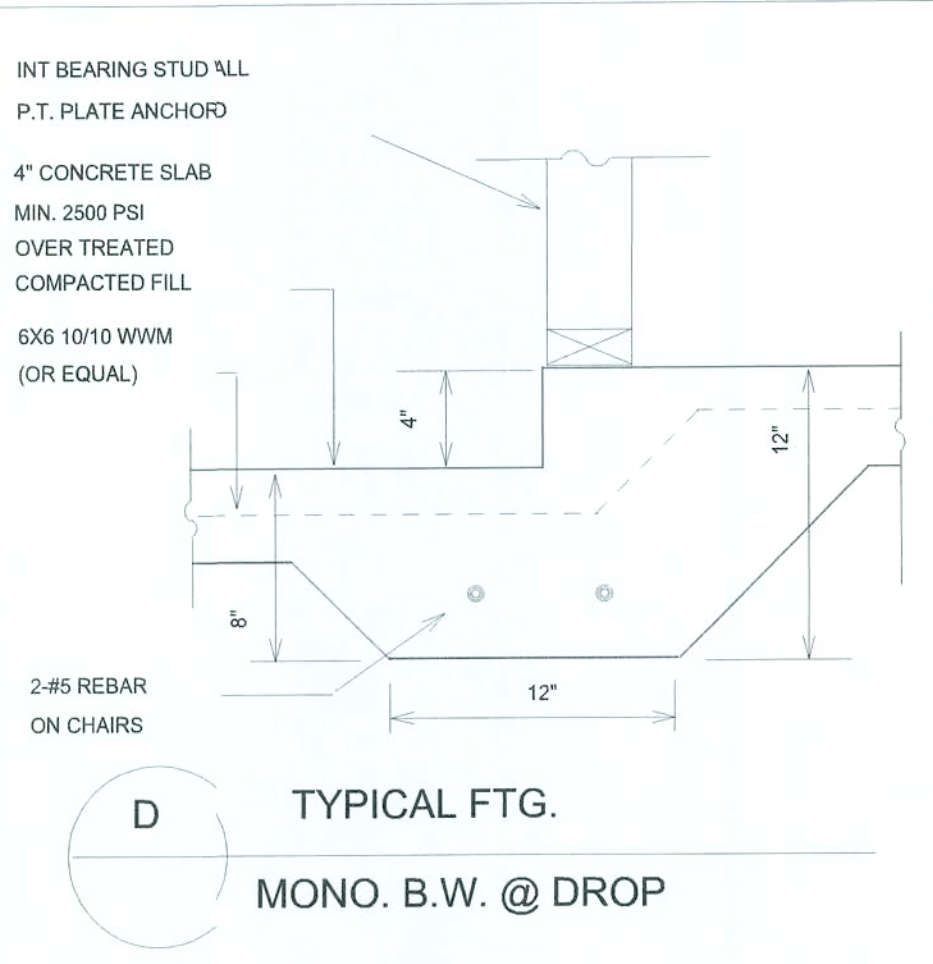
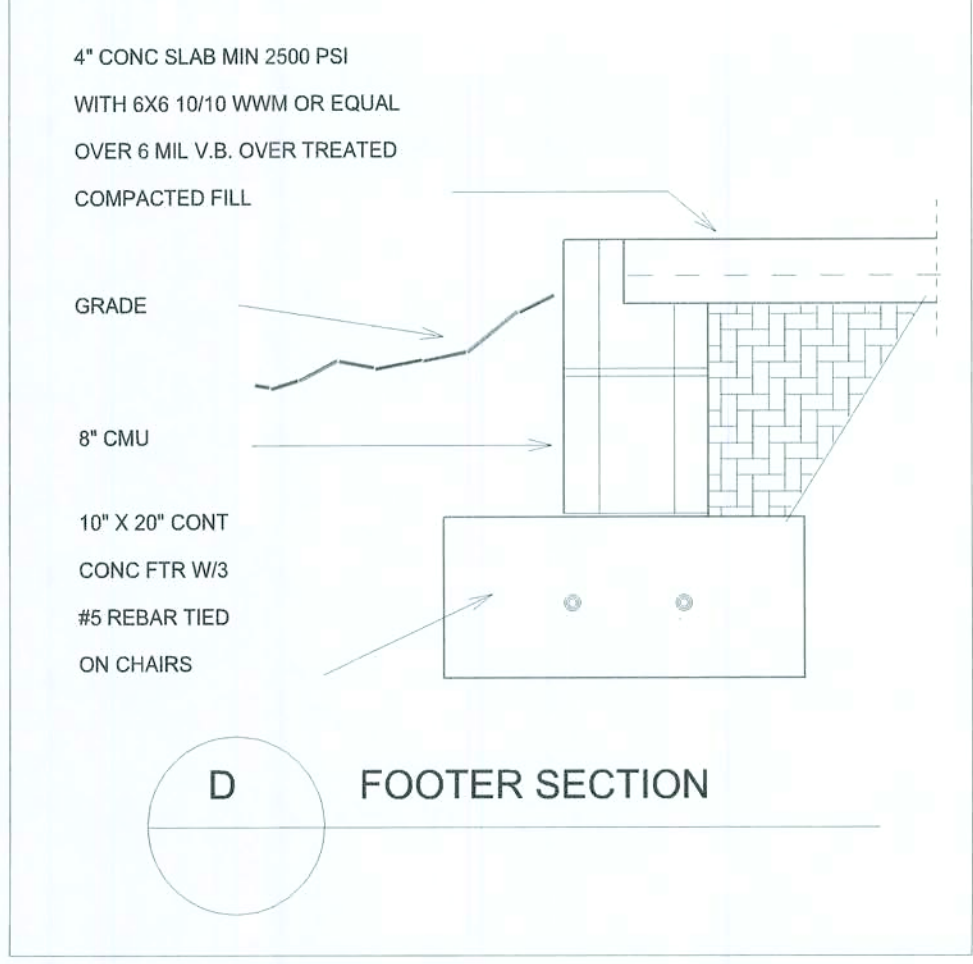
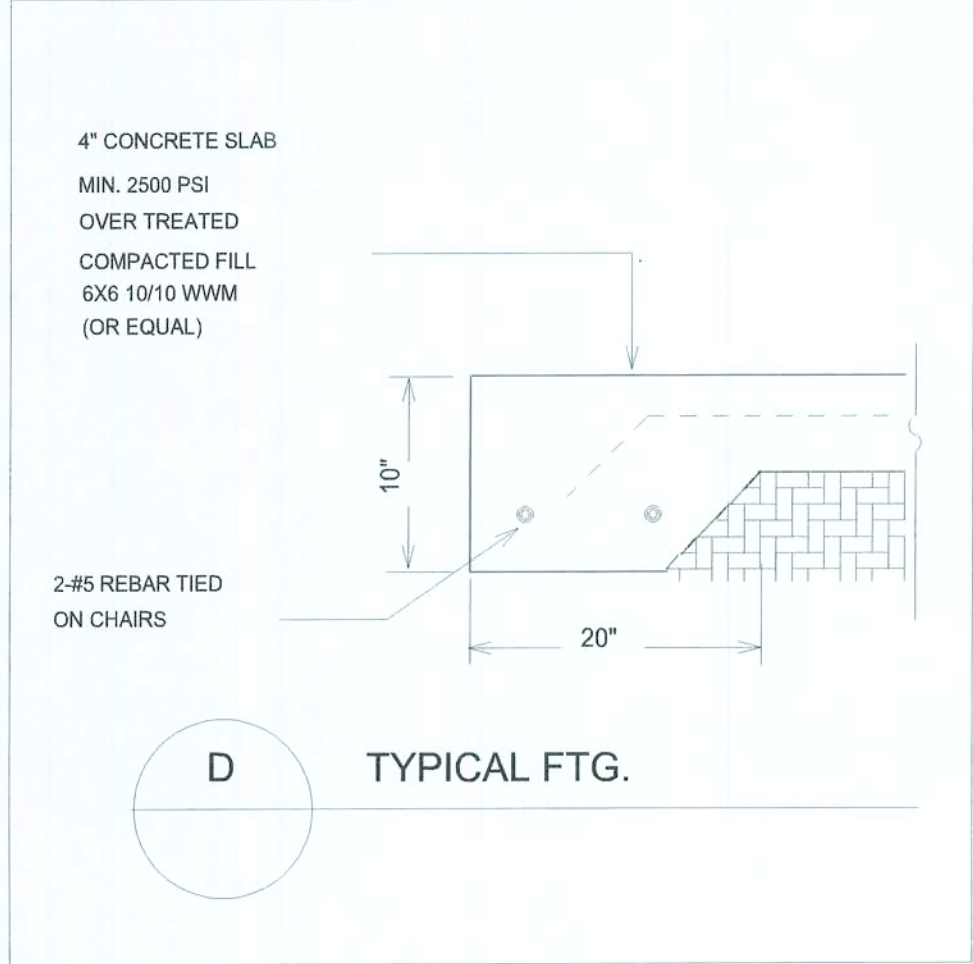
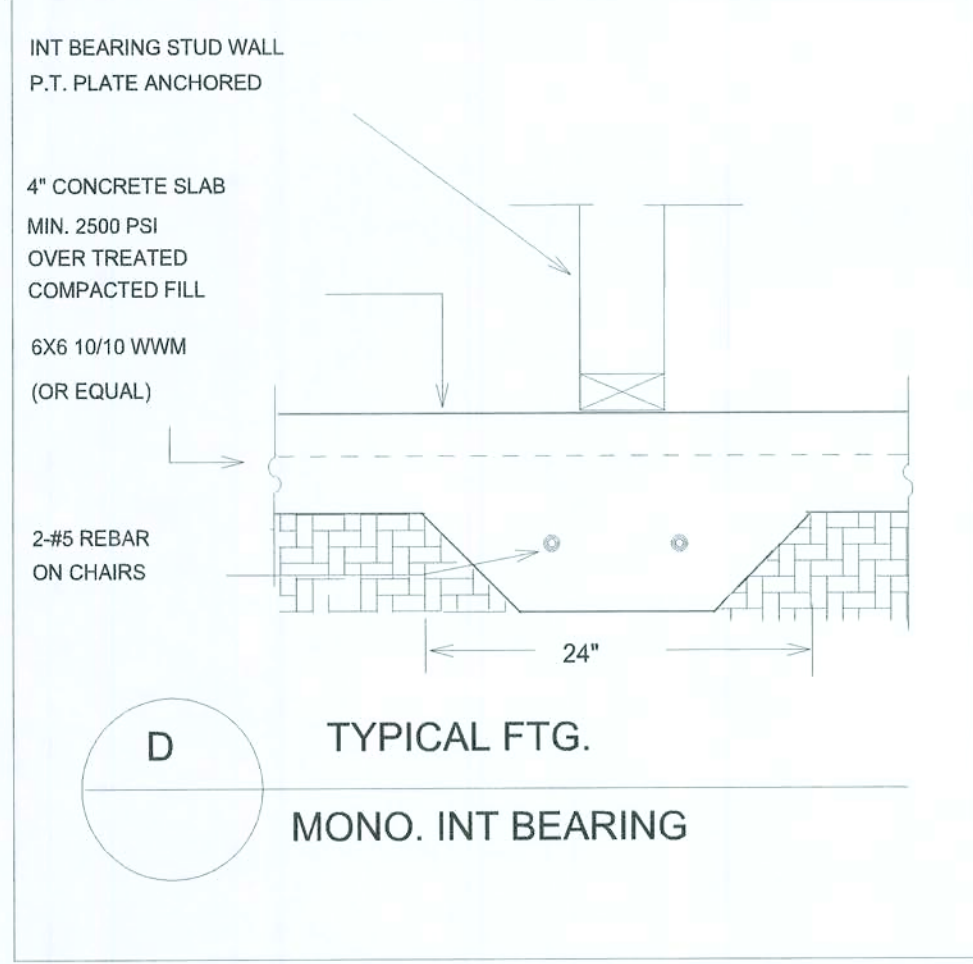
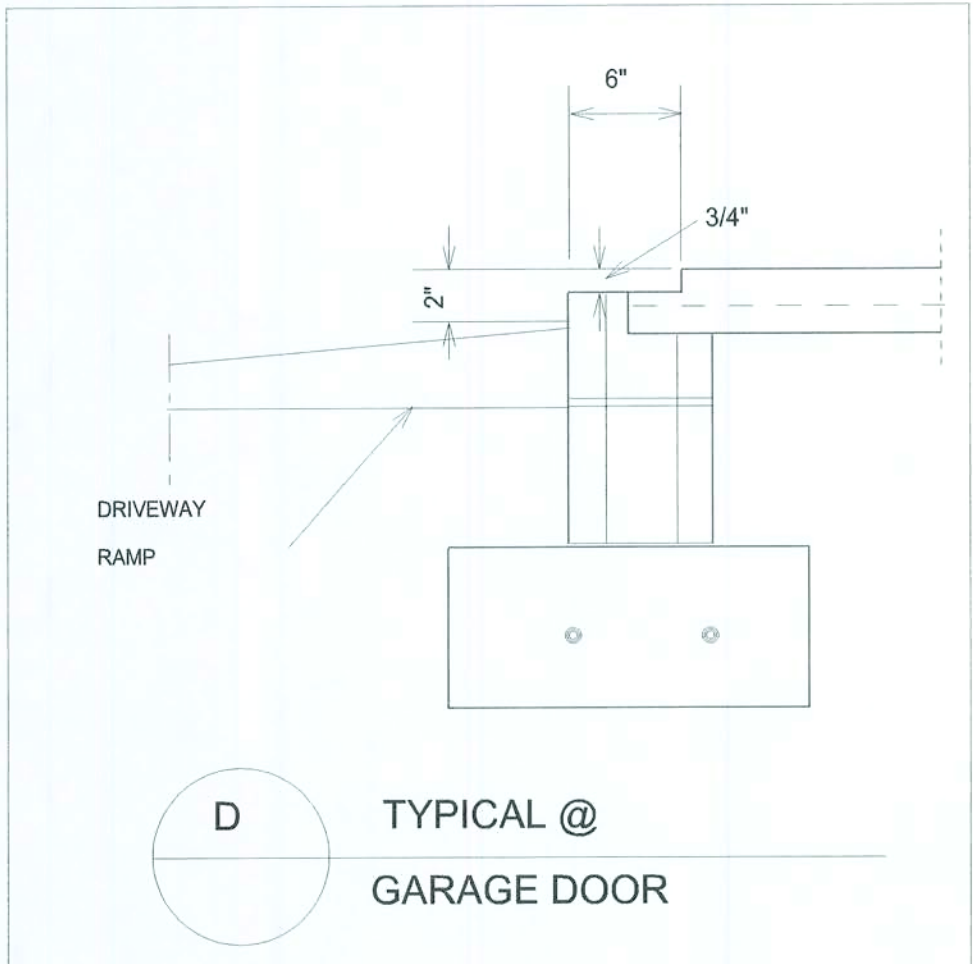
Stephen Crawford Construction, Inc. Lake City, Florida.
PLANS BY: Stephen Crawford

EXTERIOR ELEVATIONS
SCALE 1/4" = 1'-0"

@ DATE
@ TIME

SHEET #

1



NOTES:

ALL WORK SHALL COMPLY WITH
THE STANDARD BUILDING CODE
AND ALL OTHER APPLICABLE
LOCAL CODES AND ORDINANCES

AREA SUMMARY
Living Area 1777 SF
Garage 543 SF
Porches 585 SF
TOTAL AREA 2905 SF

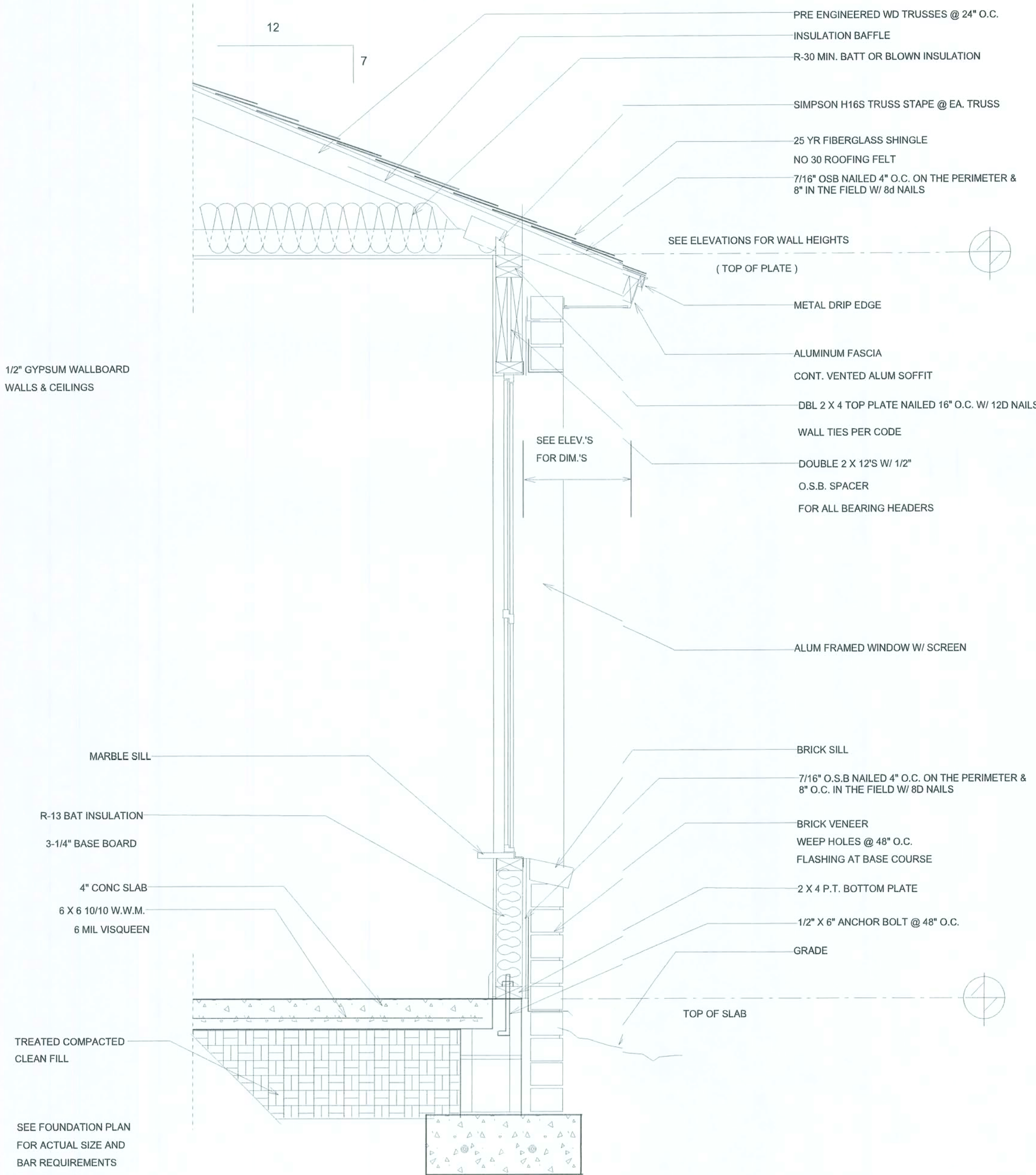
Stephen Crawford Construction, Inc. (Mason City Spec)
Stephen Crawford Construction, Inc. Lake City, Florida.
PLANS BY: Stephen Crawford

FOUNDATION PLAN
SCALE 1/4" = 1'-0"

@ DATE
@ TIME

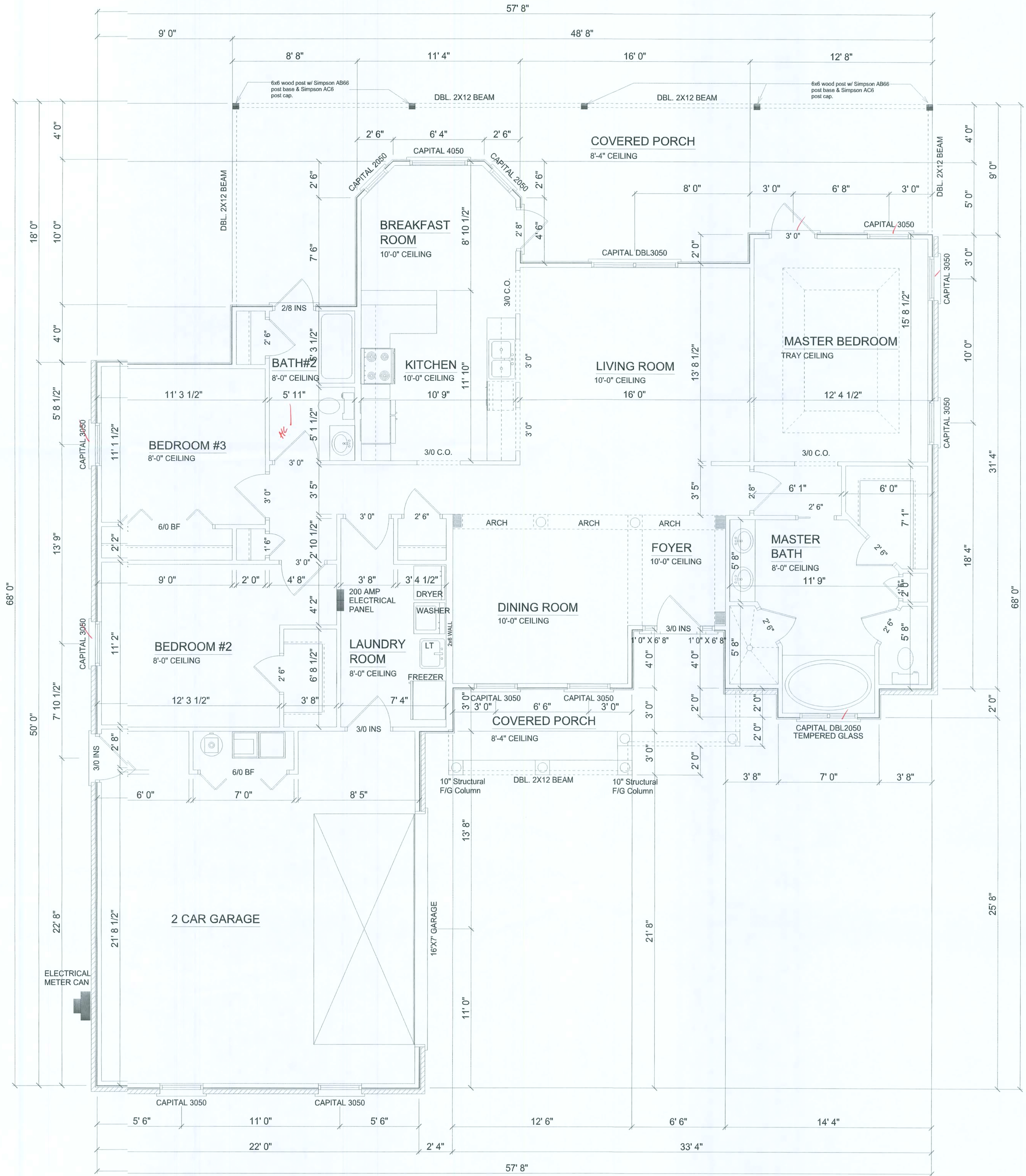
SHEET #

2



TYPICAL WALL SECTION
2 X 4 STUD WALL W/ BRICK VENEER

SCALE: 1"=1'-0"



NOTES:	
ALL WORK SHALL COMPLY WITH THE STANDARD BUILDING CODE AND ALL OTHER APPLICABLE LOCAL CODES AND ORDINANCES	

AREA SUMMARY
Living Area 1777 SF
Garage 543 SF
Porches 585 SF
TOTAL AREA 2905 SF

Stephen Crawford Construction, Inc. (Mason City Spec)

Stephen Crawford Construction, Inc. Lake City, Florida.
PLANS BY: Stephen Crawford

MAIN FLOOR PLAN
SCALE 1/4" = 1'-0"

@ DATE
@ TIME

SHEET #

3

ELECTRICAL PLAN NOTES
ALL RECEPTICALS IN ALL BEDROOMS
SHALL BE AFCI CIRCUITS

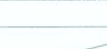
WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT
PER MANUFACTOR SPECIFICATIONS

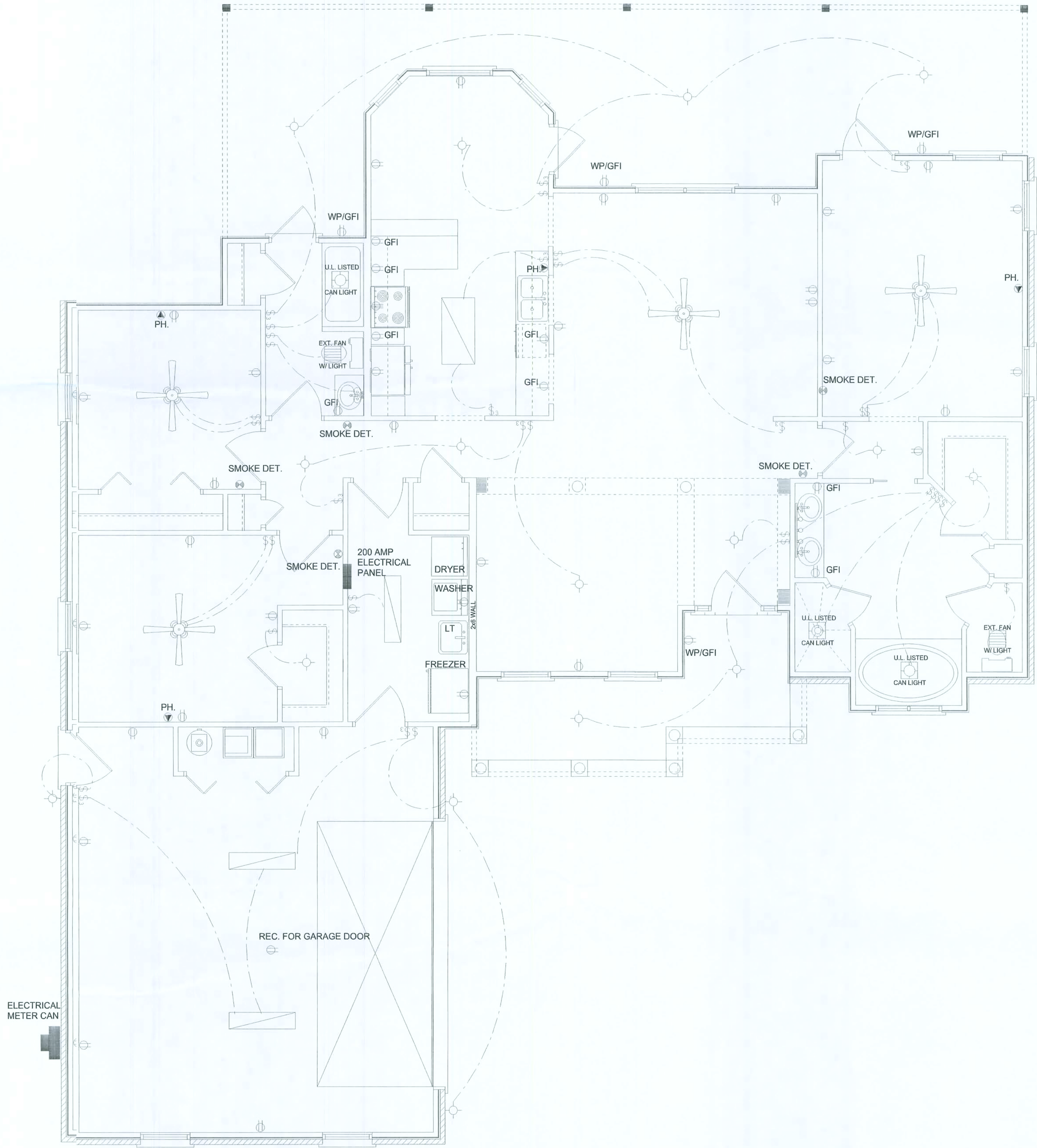
INSTALLATION SHALL BE PER NATIONAL ELECTRIC CODE.

ALL SMOKE DETECTORS SHALL BE 10V W/ BATTERY
BACKUP OF THE PHOTOELECTRIC TYPE AND SHALL BE
INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL
BEDROOMS.

TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE
DEVICES OR OUTLETS SHALL BE AS PER THE OWNERS
DIRECTIONS & IN ACCORDANCE W/ APPLICABLE
SECTIONS OF NEC-LATEST EDITIONS

ELECTRICAL CONTRACTOR SHALL PREPARE "AS-BUILT" SHOP DRAWINGS
INDICATING ALL ELECTRICAL WORK INCLUDING ANY CHANGES TO ELEC. PLAN,
ADDITIONS TO THE ELEC. PLAN, RISER DIAGRAM, AS-BUILT PANEL SCHEDULE
W/ ALL CKTS IDENTIFIED W/ CKT Nr., DESCRIPTION AND BRKR, SERVICE ENT.
& ALL UNDERGROUD WIRE LOCATIONS/ROUTING/DEPTH.
RISER DIA. SHALL INCLUDE WIRE SIZES/TYPE & EQUIPMENT
TYPE W/ RATINGS & LOADS.
CONTRACTOR SHALL PROVIDE 1 COPY OF AS-BUILT DWGS.
TO OWNER & 1-COPY TO THE PERMIT ISSUING AUTHORITY.

ELECTRICAL	COUNT	SYMBOL
ELECTRICAL PANEL	1	
METER	1	
220V OUTLET	2	
120V OUTLET	26	
SWITCH	25	
3-WAY SWITCH	6	
LIGHT	13	
CHANDALIER	1	
CAN LIGHT	2	
EXT. FAN	3	
FLOOR FIXTURE	3	
DBL. SPOTLIGHT	3	
GFI OUTLET	19	
SMOKE DETECTOR	5	
CEILING FAN	4	



NOTES:
ALL WORK SHALL COMPLY WITH
THE STANDARD BUILDING CODE
AND ALL OTHER APPLICABLE
LOCAL CODES AND ORDINANCES

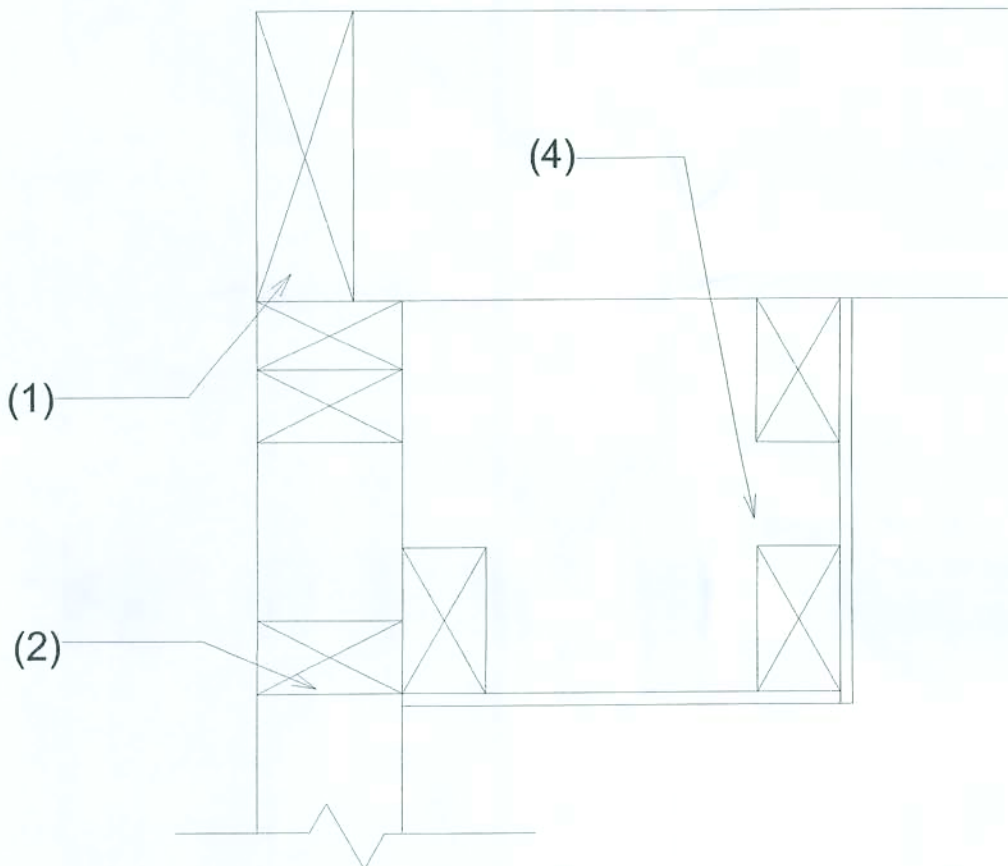
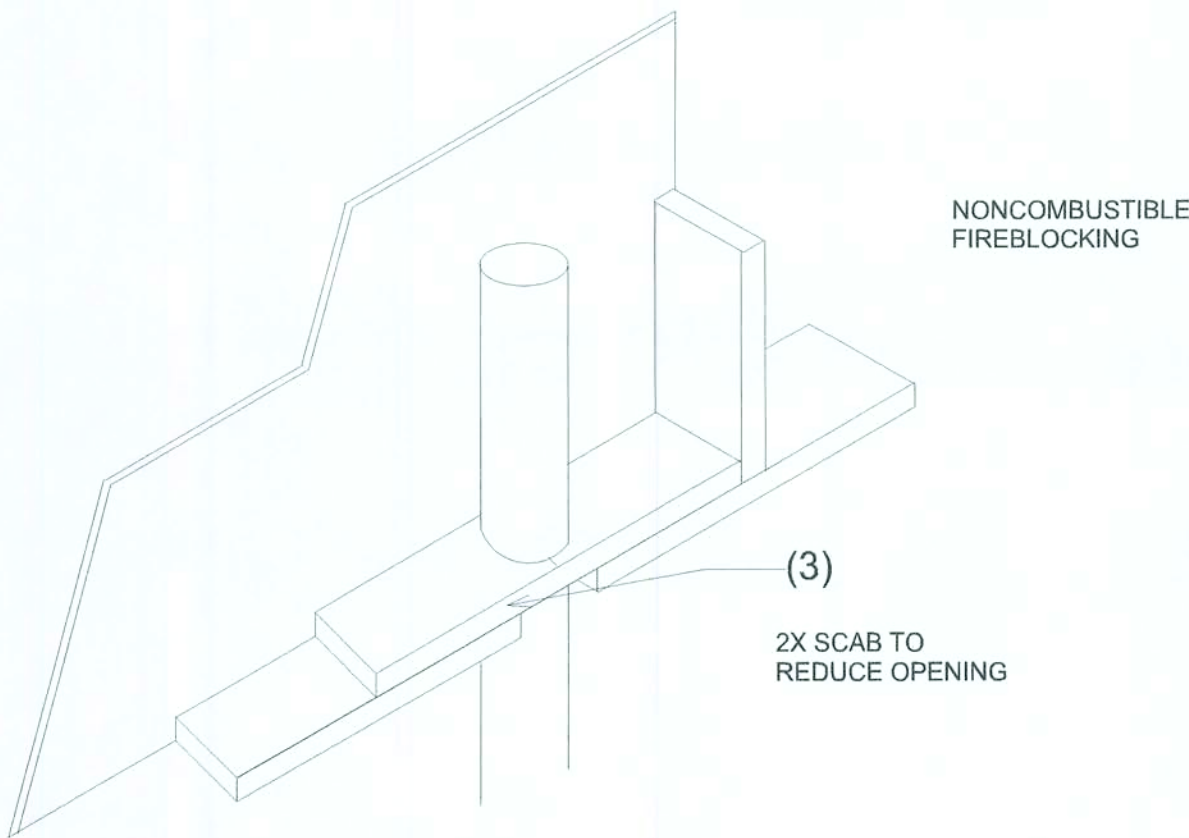
AREA SUMMARY
Living Area 1777 SF
Garage 543 SF
Porches 585 SF
TOTAL AREA 2905 SF

Stephen Crawford Construction, Inc. (Mason City Spec)

Stephen Crawford Construction, Inc. Lake City, Florida.

PLANS BY: Stephen Crawford

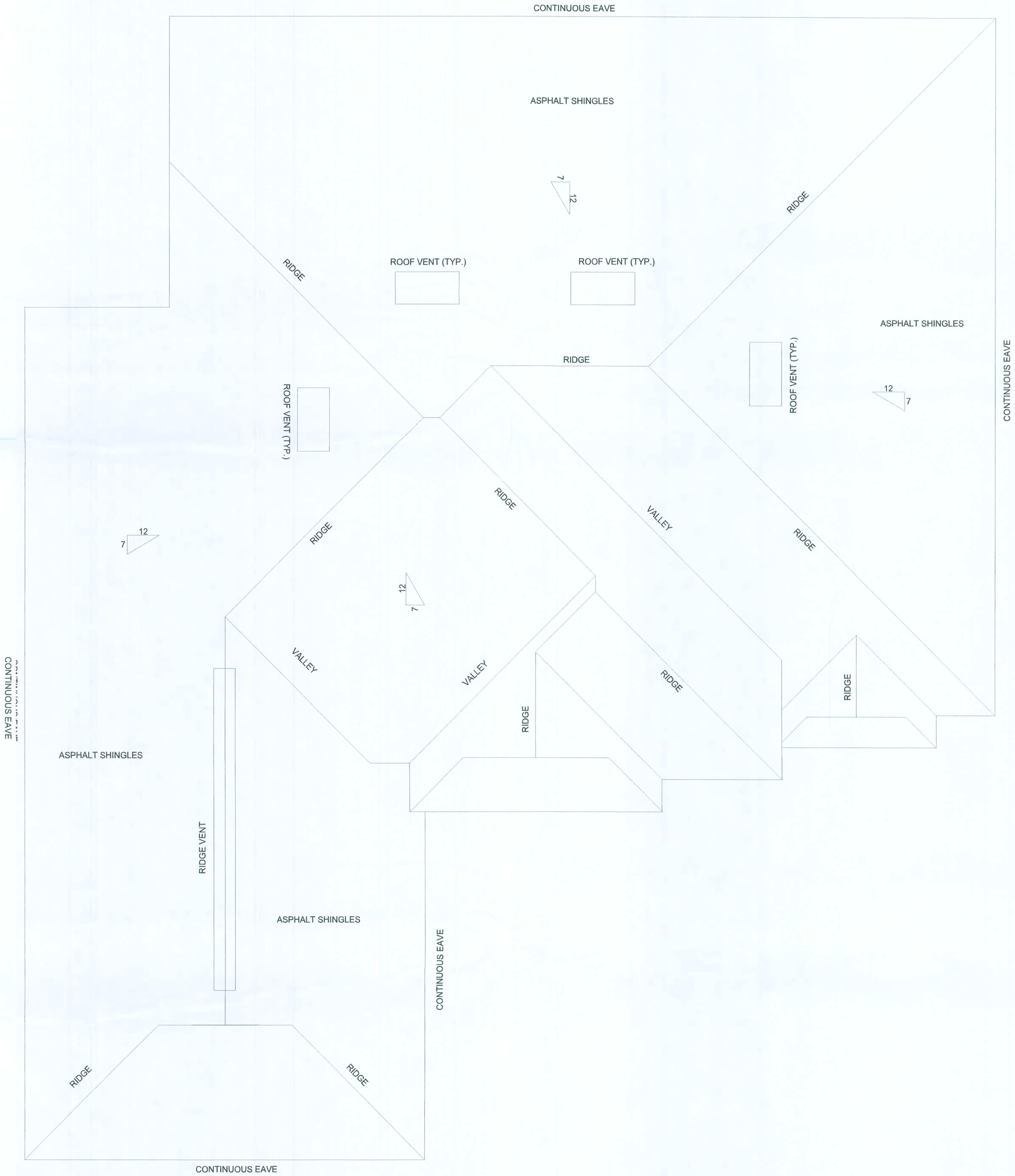
ELECTRICAL PLAN
SCALE 1/4" = 1'-0"
@ DATE
@ TIME
SHEET #
4



FIRE STOPPING DETAILS

SCALE: NONE

- FIRE BLOCKING NOTES:
- FIRE BLOCKING SHALL BE INSTALLED IN WOOD FRAME CONSTRUCTION IN THE FOLLOWING LOCATIONS:
1. IN CONCEALED SPACES OF THE STUD WALLS AND PARTITIONS INCLUDING FURRED SPACES AT CEILING AND FLOOR LEVELS.
 2. AT ALL INTERSECTIONS BETWEEN CONCEALED VERTICAL AND HORIZONTAL SPACES SUCH AS OCCUR AT SOFFITS, DROP CEILINGS, COVE CEILINGS, ETC.
 3. AT OPENINGS AROUND VENTS, PIPES, DUCTS, CHIMEYS, AND FIREPLACES AT CEILING AND FLOOR LEVELS WITH "PYROPANEL MULTIFLEXX SEALANT"
 4. AT ALL INTERSECTIONS BETWEEN CONCEALED VERTICAL STUD WALL OR PARTITIONS SPACES AND CONCEALED SPACES CREATED BY AN ASSEMBLY OF FLOOR JOISTS, FIREBLOCKING SHALL BE PROVIDED FOR THE FULL DEPTH OF THE JOISTS AT THE ENDS AND OVER THE SUPPORTS.



NOTES:				
ALL WORK SHALL COMPLY WITH THE STANDARD BUILDING CODE AND ALL OTHER APPLICABLE LOCAL CODES AND ORDINANCES				

AREA SUMMARY		Stephen Crawford Construction, Inc. (Mason City Spec)	
Living Area	1777 SF	Stephen Crawford Construction, Inc. Lake City, Florida. PLANS BY: Stephen Crawford	
Garage	543 SF		
Porches	585 SF		
TOTAL AREA		2905 SF	

ROOF PLAN		@ DATE	
SCALE 1/4" = 1'-0"		@ TIME	
SHEET #		5	

REVISIONS



ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	HSA	3-8d	3-8d	
< 455	< 285	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2800	< 2400	2 - HTS24			
< 2050	< 1785	LG2	14 - 16d	14 - 16d	
HEAVY GIRDER TIEDOWNS*					TO FOUNDATION
< 3965	< 3330	MG7		22 - 10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10630	< 9035	HGT-3		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 - 10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 - 10d		4 - 10d
< 455	< 420	SSP SINGLE SILL PLATE	1 - 10d		4 - 10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 - 10d		8 - 10d
< 825	< 600	DSP SINGLE SILL PLATE	2 - 10d		8 - 10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14 - 10d		
< 1235	< 1235	LSTA21	16 - 10d		
< 1030	< 1030	CS20	18 - 8d		
< 1705	< 1705	CS16	28 - 8d		
STUD ANCHORS*				TO STUDS	TO FOUNDATION
< 1350	< 1305	LTT19		8 - 16d	1 1/2" AB
< 2310	< 2310	LTT31	18 - 10d, 1 1/2"		1 1/2" AB
< 2775	< 2570	H20A	2-5/8" BOLTS		5/8" AB
< 4175	< 3995	HTT16	16 - 16d		5/8" AB
< 1400	< 1400	PMAH2	16 - 16d		
< 3335	< 3335	HPAH22	16 - 16d		
< 2200	< 2200	ABU44	12 - 16d		1 1/2" AB
< 2300	< 2300	ABU68	12 - 16d		1 1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED AND SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS RECTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, Fc = 3000 PSI

WELDED WIRE REINFORCED SLAB: 6" x 6" W1 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 308.2. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWW OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO, ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi, UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (13d), 6" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES, MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/16", WITH 5/8" BOLTS TO BE 3" x 3" x 9/16", WITH 3/4" BOLTS TO BE 3" x 3" x 9/16", WITH 7/8" BOLTS TO BE 3" x 3" x 5/16", UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBCT TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCT 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

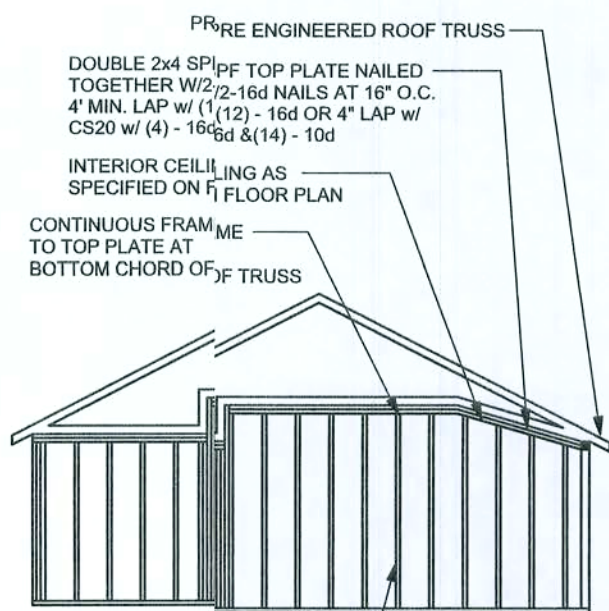
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCT 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS. TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCT 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

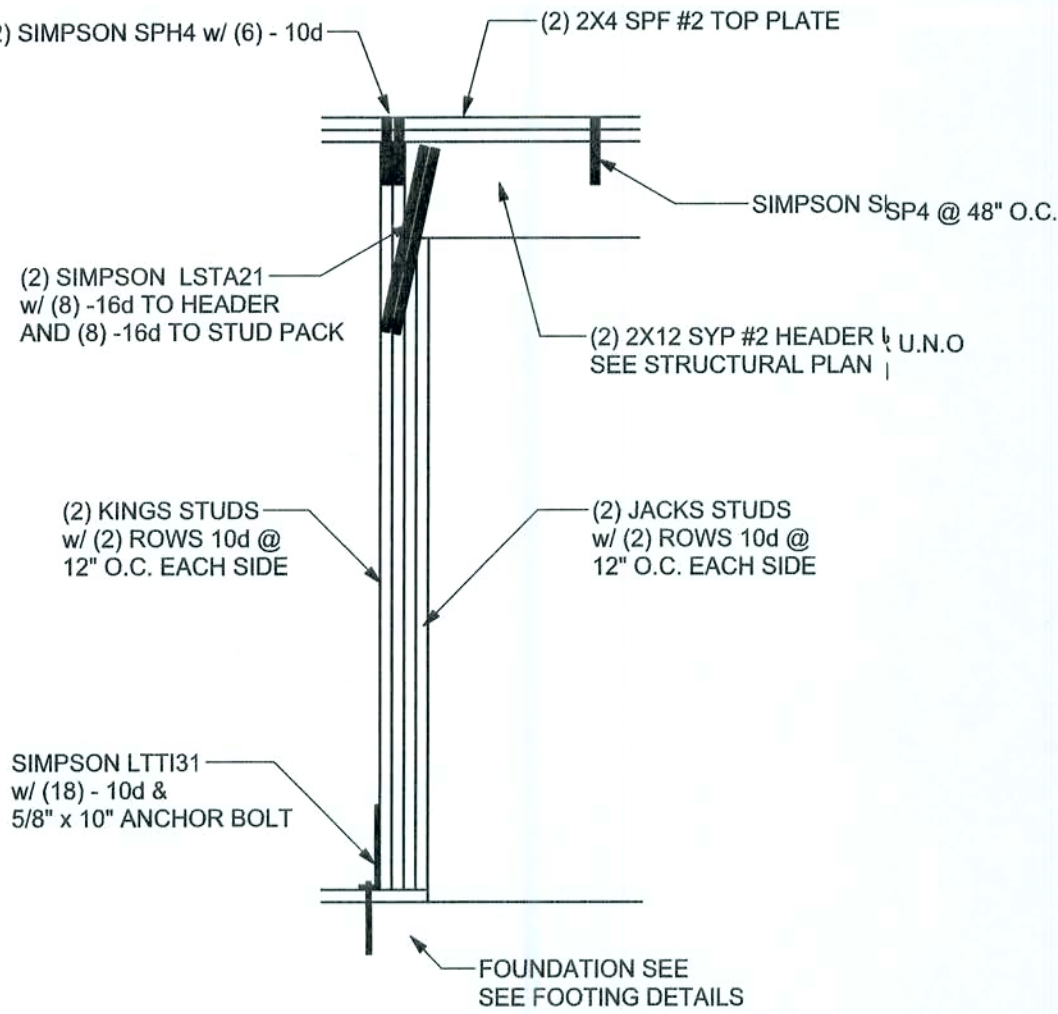
GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



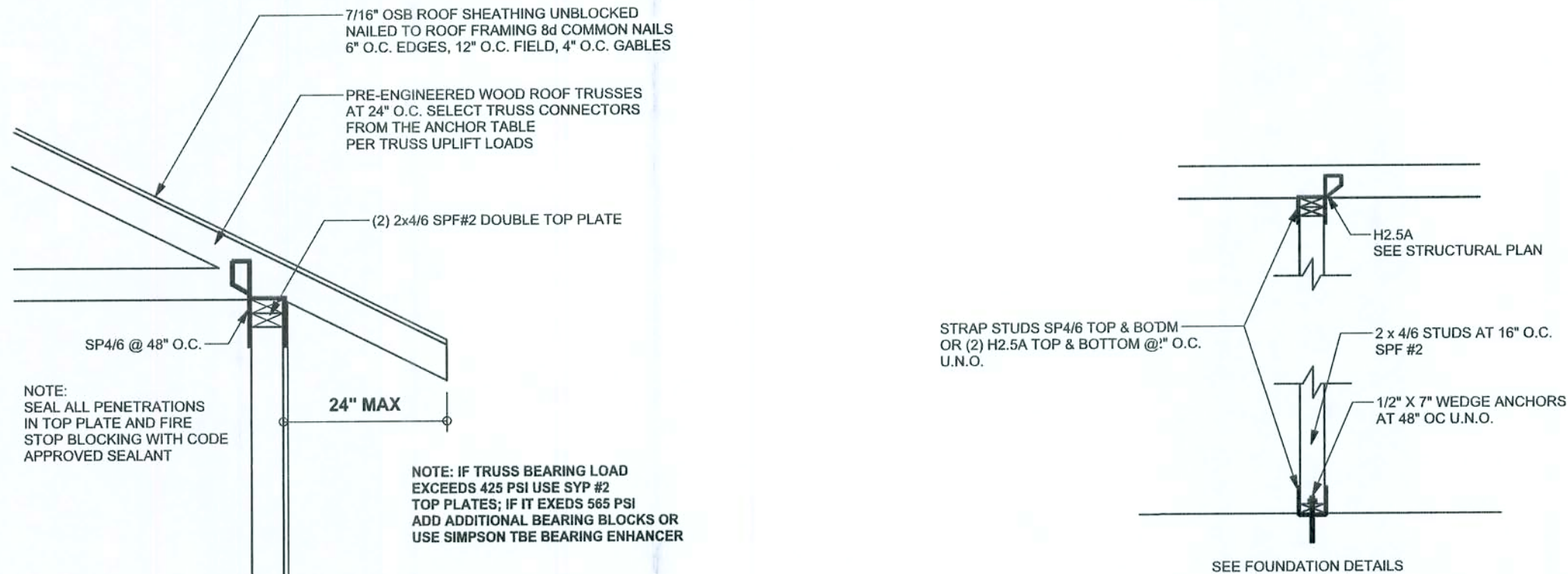
CONTINUOUS FRAME TO CEILING: DIAPHRAGM DETAIL

SCALE: N.T.S.



TYPICAL GARAGE DOOR HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"



ONE STORY WALL SECTION

SCALE: 3/4" = 1'-0"

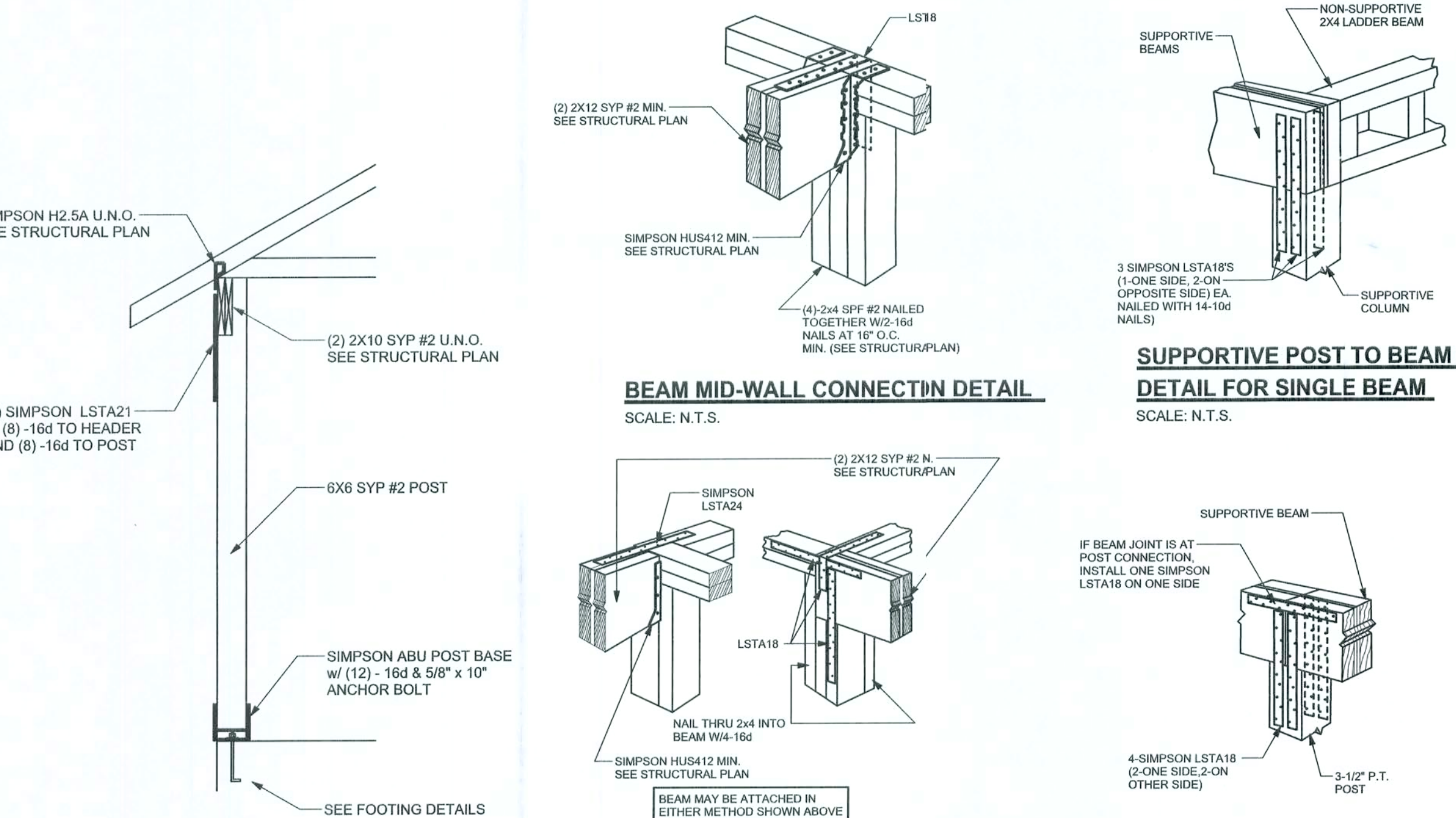
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLES OF 16" FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

TYPICAL GABLE END (X-BRACING)

ALL MEMBERS SHALL BE SYP



TYPICAL PORCH POST DETAIL

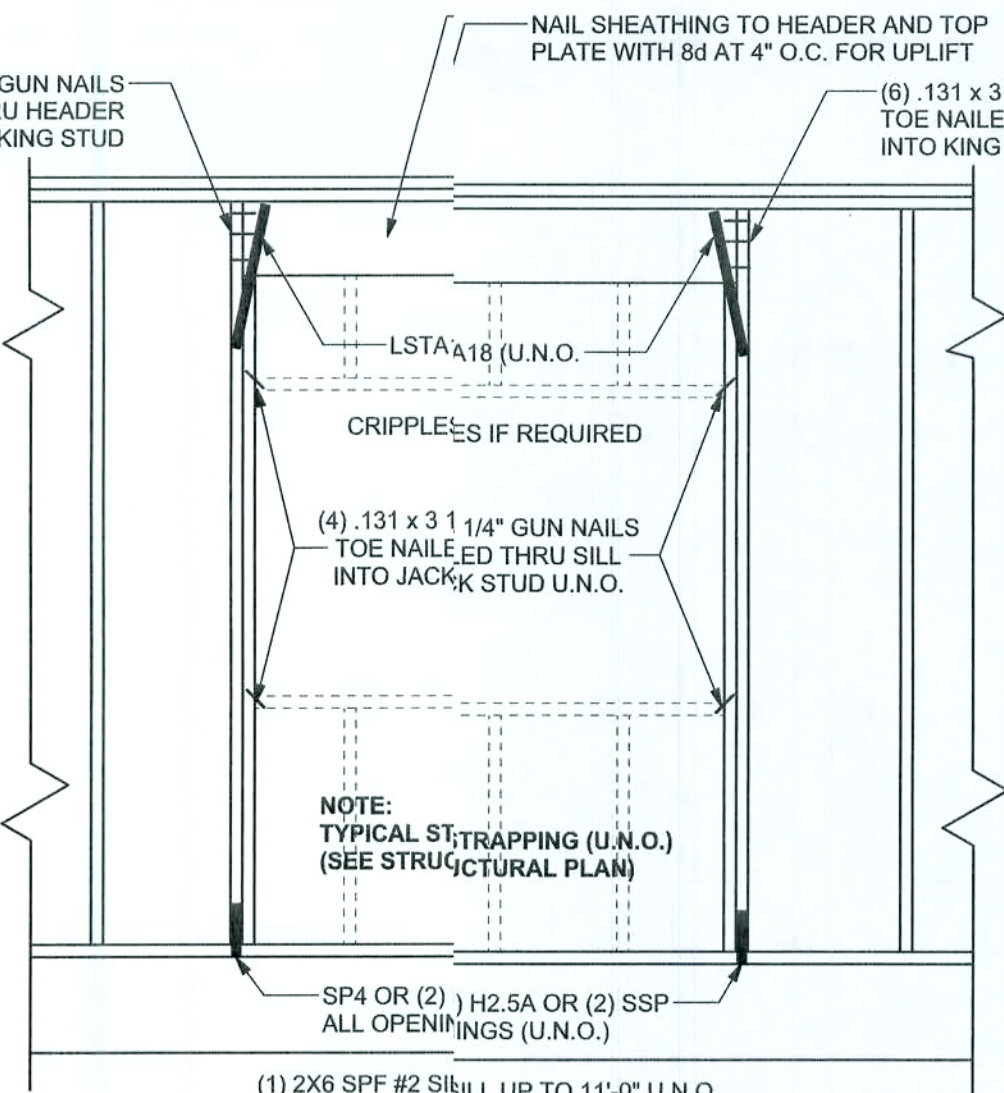
SCALE: 1/2" = 1'-0"

BEAM CORNER CONNECTION DETAIL

SCALE: N.T.S.

SUPPORTIVE CENTER POST TO BEAM DETAIL

SCALE: N.T.S.



TYPICAL HEADER STRAPPING DETAIL

SCALE: 1/2" = 1'-0"

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F'm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 475, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft ² or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft ² or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

DESIGN DATA

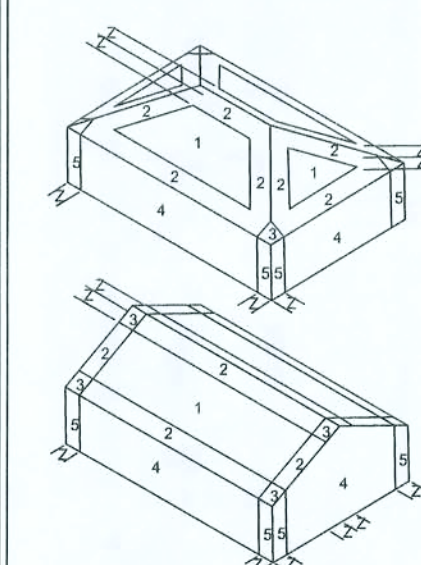
WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 30 FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = B
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = <30 FT
- INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



Zone	Effective Wind Area (ft ²)	10	100
1	19.9	-21.8	-18.1
2	19.9	-25.5	-21.8
2 Other	40.6	-40.6	-40.6
3	19.9	-25.5	-21.8
3 Other	48.3	-42.4	-42.4
4	21.8	-23.6	-20.4
5	21.8	-29.1	-22.6
Doors & Windows	21.8	-29.1	-29.1
Worst Case (Zone 5, 10 ft ²)			
8x7 Garage Door	19.5	-22.9	-22.9
16x7 Garage Door	18.5	-21.0	-21.0

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (SLEEPING ROOMS)

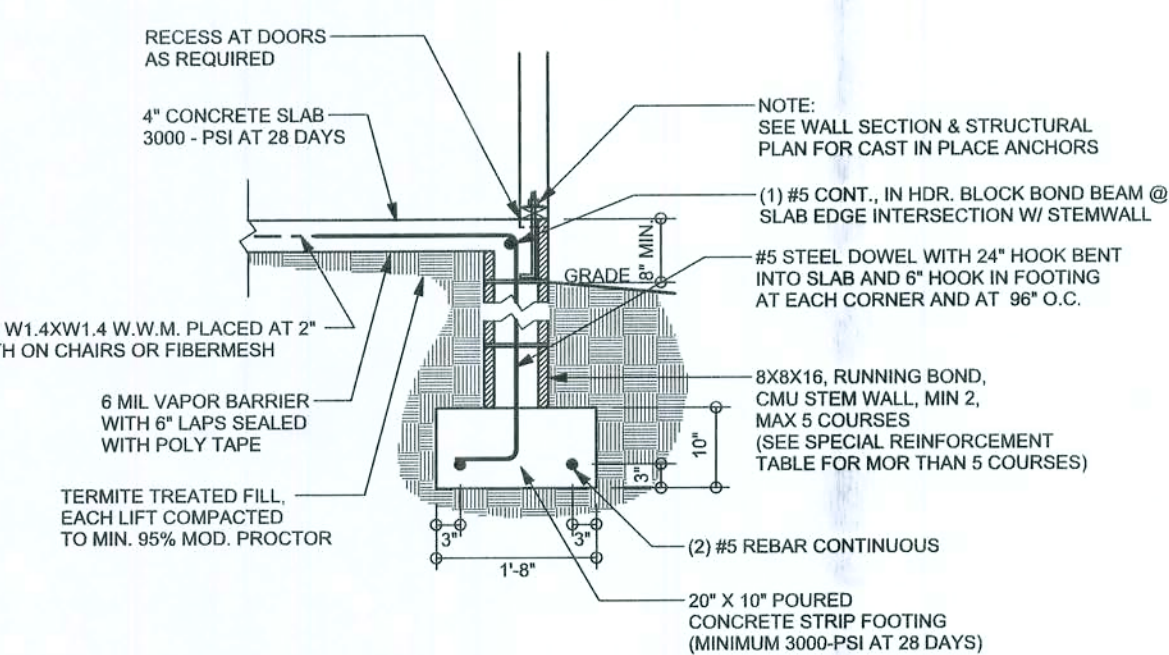
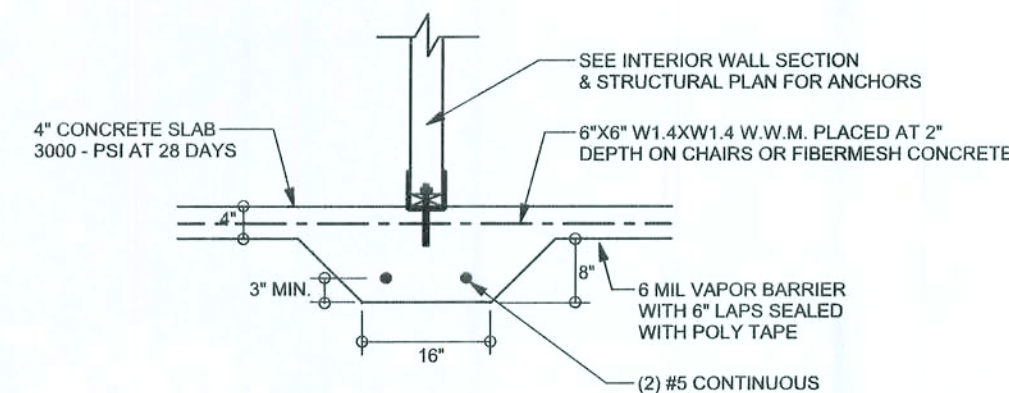
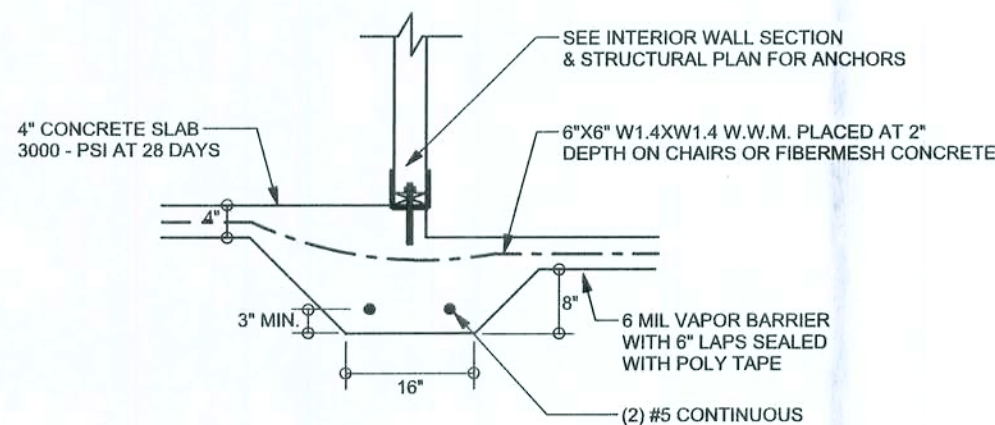
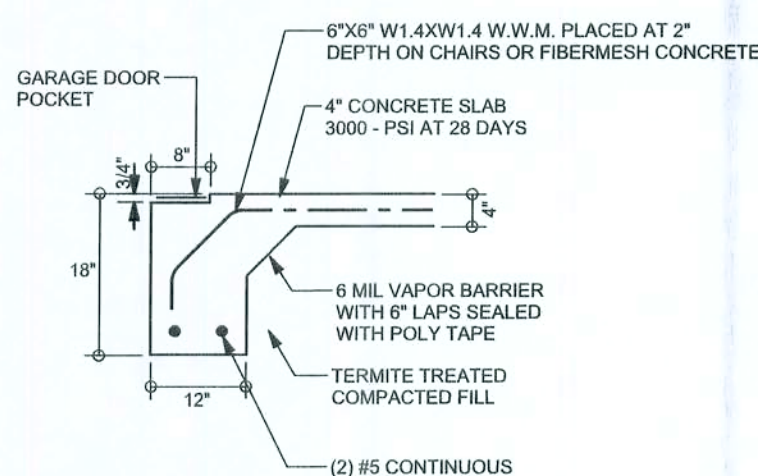
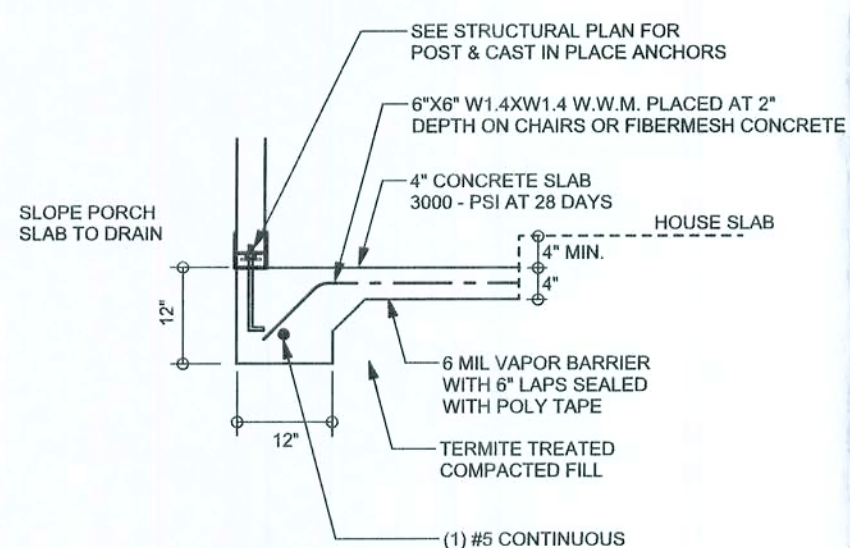
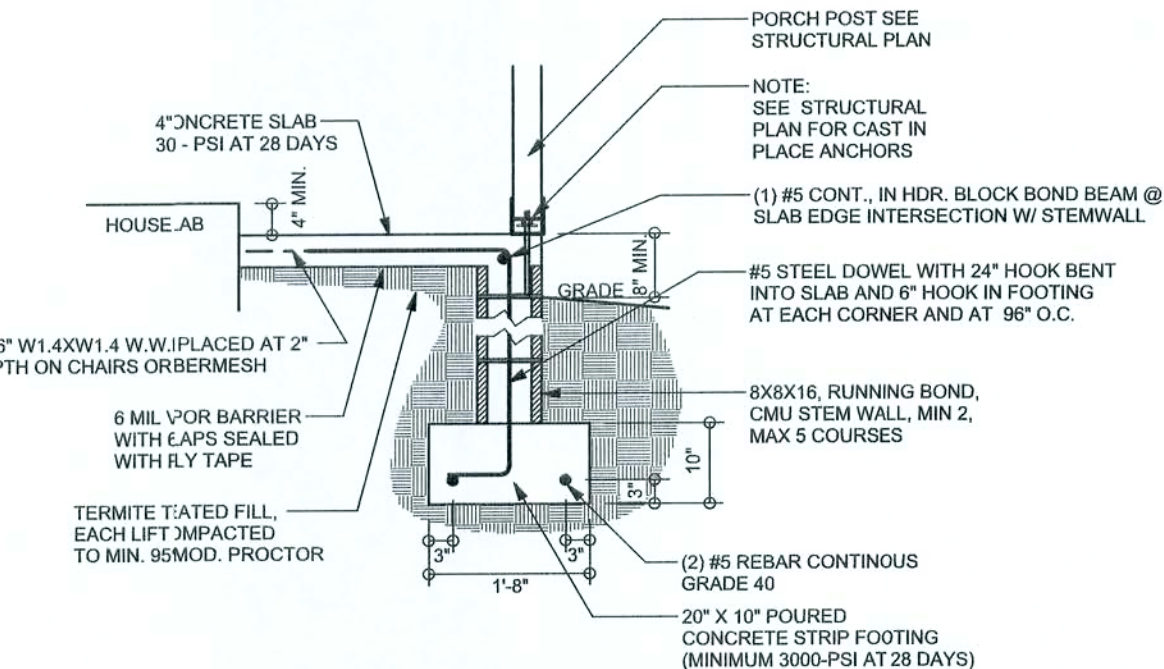
30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3.12)

ROOF 20 PSF (FLAT OR <4.12)

16 PSF (4.12 TO <12.

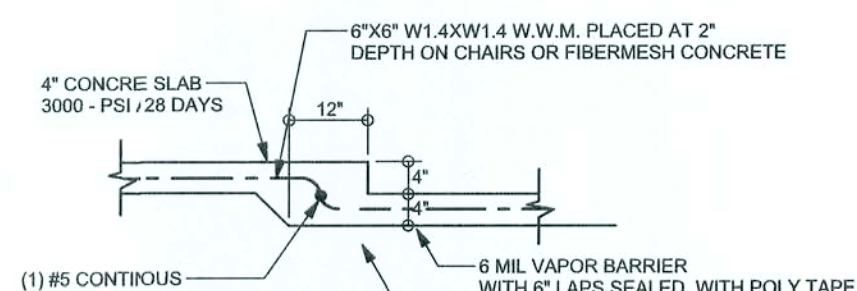
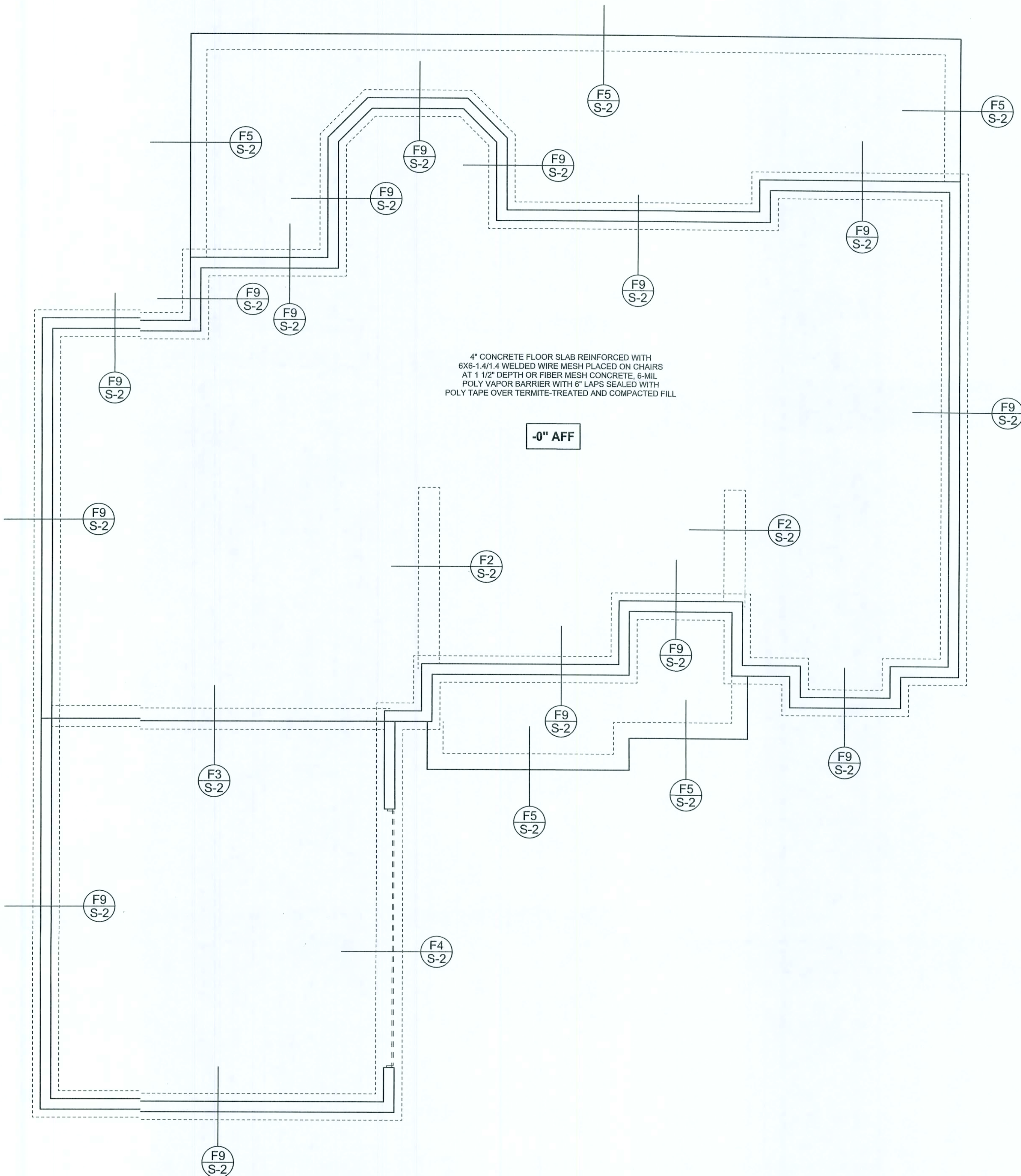
REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

F9
S-2
STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

F2
S-2
INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"

F3
S-2
INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"

F4
S-2
GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"

F5
S-2
PORCH FOOTING
SCALE: 1/2" = 1'-0"

F12
S-2
OPTIONAL STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

TALL STM WALL TABLE

The table assumes 8 #5 reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add overall ladder reinforcement at 18" O.C. vertically or a horizontal bond beam with 1#5 corners at mid height. For higher parts of the wall 12" CMU may be used with reinforcement shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED HEIGHT (FEET)	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48


F6
S-2
TYPICAL NON-BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"

FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

WINDLOAD ENGINEER: Mark Dicosway,
PE No. 53815, PCB 86, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled
dimensions. Refer all questions to
Mark Dicosway, P.E. for resolution.
Do not proceed without clarification.

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permission and consent of Mark Dicosway.

CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
portions of the plan, relating to wind engineering
comply with section 601.2.1, Florida building
code residential 2004 to the best of my
knowledge.

LIMITATION: This design is valid for one
building, at specified location.

MARK DICOSWAY
P.E. 53815

Stephen Crawford
Construction

441 Spc House

ADDRESS:
Columbia County, Florida
Parcel ID: R2255-17-09328-008

Mark Dicosway P.E.
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Lake City, Florida 32056
Phone: (386) 754 - 5419
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PRINTED DATE:
February 13, 2007

DRAWN BY: David Dicosway
CHECKED BY:

FINAL DATE:
13 / Feb / 07

JOB NUMBER:
70-252

DRAWING NUMBER

S-2

OF 3 SHEETS

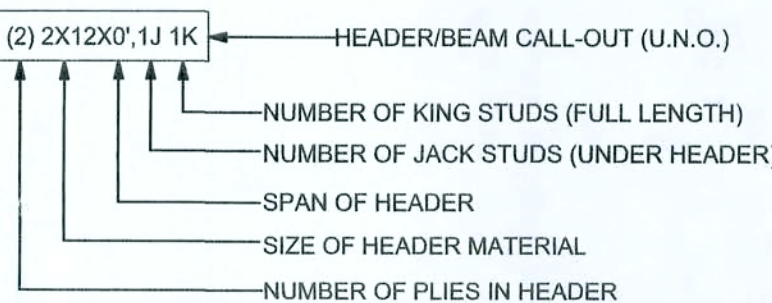
REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND



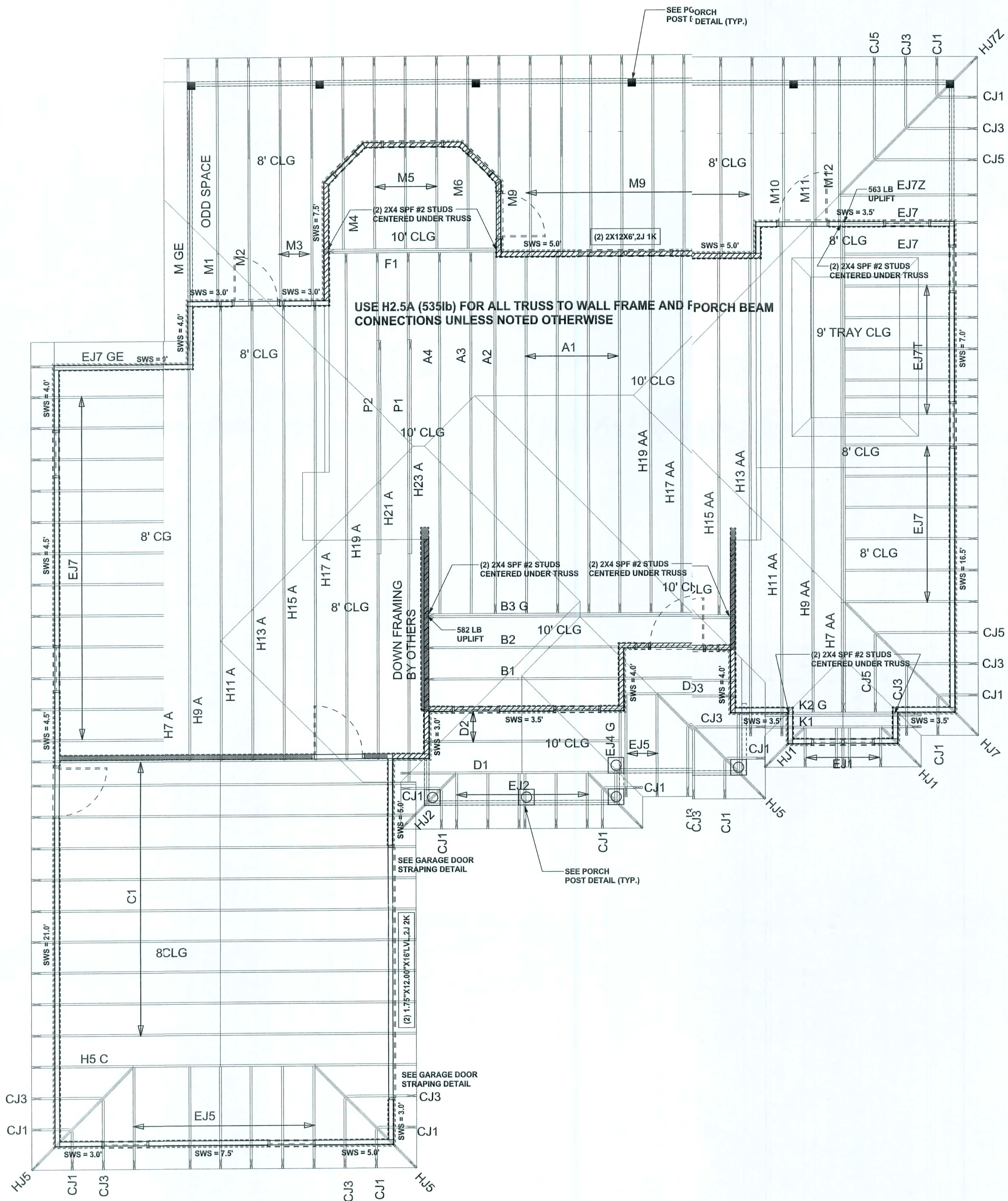
WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR WALL
IBW	1ST FLOOR INTERIOR BEARING WALLS
IBW	2ND FLOOR INTERIOR BEARING

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	39.3'	88.0'
LONGITUDINAL	33.6'	54.5'



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, ANDERSON TRUSS JOB #7-049

WINDLOAD ENGINEER Mark Discoway,
P.E. No. 53915, PCB 860, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Discoway, P.E. for resolution. Do not proceed without verification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section F303.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISCOWAY
P.E. 9915

Stephen Crawford
Construction

441 Spe: House

ADDRESS:
Columbia County, Florida
Parcel ID: R22-5-17-09328-008

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DRAWN BY: David Discoway
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FINALS DATE:
13 / Feb / 07

JOB NUMBER:
701252

DRAWING NUMBER

S3

OF 3 SHEETS