Eagle Metal Distributors, Inc.
Aluminum Alloy 6061 T-6
For 110 MPH Wind Zones, Exposure "B" and Latitudes Below 30° 30°-00" North (Jacksonville, FL)

			Tri	buta	ry Load	Wid	th 'W' =	Bear	n Spacin	g				
Hollow Sections	3'-0	*	4'-0'	•	5'-0'	*	6*-0		7'-0"		8'-0"		9'-0'	•
	Allowa	able :	Span 'L'	/ P	oint Loa	d (P)	or Unif	orm	Load (U)	, bei	nding (b)	, de	flection	(d)
2" x 2" x 0.043"	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd	5'-9"		5'-9"	Pd
3" x 2" x 0.045"	6'-9"	Pb	6'-9"	Pb	6'-9"	Pb	6'-9"	Pb	6'-9"	Pb	6'-9"	Pb	6'-9"	Pb
3" x 2" x 0.070"	8'-3"	Pd	8'-3"	Pd	8'-3"	Pd	8'-3"	Pd	8'-3"	Pd	8:-3*	Pd	8'-3"	Pd
2" x 3" x 0.045"	9'-9"	Pd	9'-9"	Pd	9-9	Pd	99.	Pd	9'-9"	Pd	9'-9"	Pd	9'-9"	Pd
2" x 4" x 0.050"	14'-8"	Pd	14'-8"	Pd	14'-8"	Pd	14'-8"	Pd	14'-8"	Pd	14'-8"	Pd	14'-3"	Ud
2" x 5" x 0.060"	21'-1"	Pd	21'-1"	Pd	21'-1"	Pd	20'-9"	Ud	19'-8"	Ud	18'-10"	Ud	18'-1"	Ud
			Tri	buta	ry Load	Wid	th W =	Bear	n Spacin	g				

			Tri	buta	ry Load	Wid	th W = 1	Bear	n Spacin	g				
Self Mating Sections	3*-0"		4'-0"		5'-0"	9-8	6'-0"		7'-0")	8'-0"		9'-0"	
	Allowa	ble 5	Span 'L'	/ P	oint Load	d (P)	or Unifo	rm	Load (U)	be	nding (b)	, de	flection	(d)
2" x 4" x 0.048" x 0.109"	18'-11"	Pd	18'-11"	Pd	18'-11"	Pd	18'-11"	Pd	18'-4"	Ud	17'-6*	Ud	16'-10"	Ud
2" x 5" x 0.050" x 0.131"	22'-10"	Pd	22'-10"	Pd	22'-10°	Pd	21'-10"	Ud	20'-9"	Ud	19'-10"	Ud	19'-1"	Ud
2" x 6" x 0.050" x 0.135"	29'-1"	Pd	29'-1"	Pd	27'-3"	Ud	25'-8"	Ud	24'-4"	Ud	23'-4"	Ud	22'-5"	Ud
2" x 7" x 0.055" x 0.135")	35'-4"	Pd	33'-5"	Ud	31'-0"	Ud	29'-2"	Ud	27'-9")	Ud	26'-6"	Ud	25'-6"	Ud
2" x 8" x 0.070" x 0.239"	45'-5"	Ud	41'-3"	Ud	38'-3"	Ud	36'-0"	Ud	34-3*	Ud	32-9	Ud	31'-6"	Ud
2" x 9" x 0.070" x 0.219"	49'-3"	Ud	44'-9"	Ud	41'-6"	Ud	39'-1"	Ud	37'-2"	Ud	35'-6*	Ud	34'-2"	Ud
2" x 9" x 0.082" x 0.321"	53'-1"	Ud	48'-3"	Ud	14'-10"	Ud	42'-2"	Ud	40'-1"	Ud	38'-4"	Ud	B6'-10"	Ud
2" x 10" x 0.090" x 0.389"	57'-5"	Ld	56'-2"	Ud	52'-2"	Ud	49'-1"	Ud	46'-7"	Ud	44'-7"	Ud	12'-10"	Ud

- Note:

 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".

 2. The structures designed using this section shall be limited to a maximum combined span and upright height of 50' and a maximum upright height of 16'. Structures larger than these limits shall have site specific engineering.

 3. Span is measured from center of beam and upright connection to fascia or wall connection.

 4. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.

 5. Tables are based on a maximum wall height of 16' including a 4' max. mansard or gable.
- 6. Soans may be interpolated.
- To convert spans to "C" and "D" exposure categories see exposure multipliers and example on Table 1B Page 3.

Table 1.2 110

Allowable Purlin Spans

Lagre metal Distributors, Inc.
Aluminum Alloy 6061 7-6
For 110 MPH Wind Zones, Exposure "B" and Latitudes Below 30*-30'-00" North (Jacksonville, FL)
Uniform Load = 4 #/SF, a Point Load of 300 # over (1) linear ft. is also considered
A. Sections Fastened With Clips

20		Tri	ibuta	ry Load	Wid	th 'W' =	Purli	n Spaci	ng				
3'-6	*	4'-0	**	4'-6	16	5'-0		5'-6		6'-0'		6'-8'	
Allow	able	Span 'L	. / P	oint Lo	ad (P	or Uni	form	Load (L	J), be	nding (b), de	flection	(d)
5'-9"	Pd	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd
6'-9"	Pb	6'-9"	Pb	6'-9"	Pb	6'-9"	Pb	6'-9"	Pb	6,-9,	Pb	6'-9"	Pb
8'-3"	Pd	8'-3"	Pd	8'-3"	Pd	8'-3"	Pd	8'-3"	Pd	8'-3"	Pd	8'-3"	Pd
9,-9,	Pd	9'-9"	Pd	9'-9"	Pd	9,-9,	Pd	9'-9"	Pd	9,-9,	Pd	9'-9"	Pd
14'-8"	Pd	14'-8"	Pd	14'-8"	Pd	14'-8"	Pd	14'-8"	Pd	14'-8"	Pd	14'-8"	Pd
21'-1"	Pd	21'-1"	Pd	21'-1"	Pd	21'-1"	Pd	21'-1"	Pd	20'-9"	Ud	20'-0"	Ud
	Allow 5'-9" 6'-9" 8'-3" 9'-9"	5'-9" Pd 6'-9" Pb 8'-3" Pd 9'-9" Pd 14'-8" Pd	3'-6" 4'-0 Allowable Span 'L 5'-9" Pd 5'-9" 6'-9" Pb 6'-9" 8'-3" Pd 8'-3" 9'-9" Pd 9'-9" 14'-8" Pd 14'-8"	3'.6" 4'.0" Allowable Span 'L.' / F 5'.9" Pd 5'.9" Pd 6'.9" Pb 6'.9" Pb 8'.3" Pd 8'.3" Pd 9'.9" Pd 9'.9" Pd 14'.8" Pd 14'.8" Pd	3'-6" 4'-0" 4'-6 Allowable Span 'L' / Point Lo. 5'-9" Pd 5'-9" Pd 5'-9" 6'-9" Pb 6'-9" Pb 6'-9" 8'-3" Pd 8'-3" Pd 8'-3" 9'-9" Pd 9'-9" Pd 9'-9" 14'-6" Pd 14'-8" Pd 14'-8"	3'-6"	3'-6"	3'-6" 4'-0" 4'-6" 5'-0" Allowable Span 'L' / Point Load (P) or Uniform 5'-9" Pd 5'-9" Pd 5'-9" Pd 6'-9" Pb 6'-9" Pb 6'-9" Pb 6'-9" Pb 6'-3" Pd 8'-3" Pd 8'-3" Pd 8'-3" Pd 9'-9" Pd 9'-9" Pd 9'-9" Pd 9'-9" Pd 14'-8" Pd 14'-8" Pd 14'-8" Pd 14'-8" Pd	3'.6" 4'.0" 4'.6" 5'.0" 5'.5 Allowable Span 'L' / Point Load (P) or Uniform Load (S.5') Pd 5'.9" Pd 6'.9" Pb 6'.9" Pd 8'.3" Pd 8	Allowable Span 'L' / Point Load (P) or Uniform Load (U), be 5:9° Pd 6:9° Pb 6:9° Pb 6:9° Pb 6:9° Pb 6:9° Pb 6:9° Pd 5:3° Pd 5:3° Pd 9:3° Pd 8:3° Pd 8:	3'.6"	3'.6" 4'.0" 4'.6" 5'.0" 5'.5" 60.0" Allowable Span 'L' / Point Load (P) roll form Load (U), bending (b), do fi.9" Pd 5'.9" Pd 6'.9" Pd 6'	3'.6" 4'.0" 4'.6" 5'.0" 5'.6" 6'.0"

7.1			Tril	buta	ry Load	Widt	th 'W' = F	urli	n Spacin	ng			0	-
Hollow Sections	3'-6"		4'-0"		4'-6"	100	5'-0"		5'-6"		6'-0"		(6,-8,	7
	Allowa	able	Span 'L'	/ P	oint Loa	d (P	or Unif	orm	Load (U), be	nding (b), de	faction	(d)
2" x 2" x 0.043"	8'-2"	Pb	8'-2"	Pb	8'-2"	Pb	8'-2"	Pb	8'-2"	Pb	8'-2"	Pb	8'-2	Pb
3" x 2" x 0.045"	8'-5"	Pb	8'-5"	Pb	8'-5"	Pb	8'-5"	Pb	8'-5"	Pb	8*-5*	Pb	8-5	Pb
3" x 2" x 0.070"	12'-4"	Pd	12'-4"	Pd	12'-4"	Pd	12'-4"	Pd	12'-4"	Pd	12'-4"	Pd	12'-4"	Pd
2" x 3" x 0.045"	14'-8"	Pd	14'-8"	Pd	14'-8"	Pd	14'-8"	Pd	14'-8"	Pd	14'-8"	Pd	14'-8"	Pd
2" x 4" x 0.050"	21'-11"	Pd	21'-11"	Pd	21'-11"	Pd	21'-11"	Pd	21'-6"	Ub	20'-4"	Ub	19'-1"	Ub
2" x 5" x 0.060"	31'-7"	Pd	31'-7"	Pd	30'-7"	Ud	29'-6"	Ud	28'-3"	Ub	26'-11"	Ub	25'-4"	Ub

Note:

- 1, Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
- 2. Span is measured from center of beam and upright connection to fascia or wall connection Tables are based on a maximum wall height of 16' including a 4' max. mansard or gable.

- 5. 2" x 4" & 2" x 5" Hollow Girts shall be connected w/ an internal or external 1-1/2" x 1-1/2" x 0.044" angle.
- 6. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on Table 1B Page 3. CHECK TABLE 1.6 FOR MINIMUM PURLIN SIZE FOR BEAMS.

Allowable Upright Heights Eagle Metal Distributors, Inc. Table 1.3 110 E 6061

Aluminum Alloy 6061 T-6
For 3 second wind gust at a velocity of 110 MPH, Exposure "B" or an applied load of 13 #/sq. ft.

			Tributar	yΙ	oad Wic	ith	'W' = Up	rig	ht opac	99.9				
Hollow Sections	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"	"	8'-0"		9'-0"	
		Alla	owable h	lei	ght "H" /	b	ending (b),	demectic	on	(d)	11/2		
2" x 2" x 0.043"	7'-5"	d	6'-8"	d	6'-3"	d	5'-10"	d	5'-7"	d	5'-2*	b	4'-10"	b
3" x 2" x 0.045"	8'-4"	d	7'-7"	d	7'-1"	d	6'-5"	ь	5'-10"	Ь	5'-5"	b	5'-0"	b
3" x 2" x 0.070"	9'-5"	d	8'-7"	d	7'-11"	d	7'-6"	d	7'-1"	d	6'-10"	d	6'-6"	d
2" x 3" x 0:045"	10'-7"	d	9'-7"	d	8'-11"	d	8'-5"	d	Z	ь	7'-3"	Ь	6'-9"	t
2" x 4" x 0.050"	13'-10"	d	12'-7"	d	11'-8"	d	10'-11"	d	10'-4"	ь	9'-7"	Ь	8'-11"	l
2" x 5 x 0.000"	17'-8"	d	16'-0"	d	14'-11"	d	14'-0"	d	13'-4"	d	12'-7"	b	11'-9"	t
		=	Tributar	уl	oad Wid	ith	'W' = Up	riç	ht Spac	ing				
Self Mating Sections	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9,-0,,	
		Alle	owable h	lei	ght "H" /	b	ending (b),	deflection	on	(d)			
2" x 4" x 0.048" x 0.109"	16'-5"	d	14'-11"	d	13'-10"	d	12'-10"	b	11'-10"	Ь	11'-1"	b	10'-5"	l
2" x 5" x 0.050" x 0.131"	19'-5"	d	17'-7"	d	16'-4"	d	15'-5"	b	14'-7"	b	13'-11"	b	13'-0"	l
2" x 6" x 0.050" x 0.135"	22'-10"	d	20'-7"	b	19'-1"	b	17'-11"	b	17'-1"	Ь	16'-0"	b	15'-1"	ı
2" x 7" x 0.055" x 0.135"	25-8*	d	23-4"	b	21'-8"	b	20'-4"	b	18'-9"	ь	17'-6"	b	16-6*	l
2" x 8" x 0.070" x 0.239"	30'-8"	ď	27'-10"	đ	25'-10"	d	24'-4"	d	23'-1"	d	22'-1"	d	21'-0"	1
2" x 9" x 0.070" x 0.219"	33'-3"	d	30'-3"	d	28'-1"	d	26'-5"	d	24'-7"	b	23'-0"	b	21'-8"	l
2" x 9" x 0.082" x 0.321"	35'-10"	d	32'-7"	d	30,-3,	d	28'-6"	d	27'-0"	d	25'-10"	d	24'-10"	0
2" x 10" x 0.090" x 0.389"	41'-9"	d	37'-11"	d	35'-2"	d	33'-2"	d	31'-6"	ld	30'-1"	d	28'-11"	10

- Note:

 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".

 2. Using screen panel width W select upright length "H".

 3. Above heights do not include length of knee brace. Add vertical distance from upright to center of brace to beam

- 3. Above heights do not include length of knee brace. Add vertical distance from upright to center of brace to beam connection to the above spans for total beam spans.

 4. Site specific engineering required for pool enclosures over 30° in mean roof height.

 5. Height is to be measured from center of beam and upright connection to fascia or wall connection.

 6. Chair rails of 2" x 2" x 0.044" min. and set @ 35" in height are designed to be residential guardrails provided they are attached with min. (3) #10 x 1-1/2" S.M.S. into the screw bosses and do not exceed 8:0" in span.

 7. Max. beam size for 2" x 5" is 2" x 7" x 0.055" x 0.120"

 8. Spans may be interpolated.

- Spans may be interpolated.

 To convert spans to "C" and "D" exposure categories see exposure multipliers and example on Table 1B Page 3.

 To patio decks 30" above grade and less than 1 story in height, screen meets the criteria for use as pickets.

Table 1.4 110 E 6061 Allowable Post / Girt / Chair Rail / Header Spans & Upright Heights Eagle Metal Distributors, Inc. Aluminum Alloy 6061 T-6

For 3 second wind gust at a velocity of 110 MPH, Exposure "B" or an applied load of 13 # / sq. ft.

			Tributa	y L	oad Wid	ith	'W' = Me	mb	er Spac	ing			1	
Hollow Sections	3'-6"		W4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	2
2	All	bw	ablantoi	ght	"H" or	Spa	n "L" /	ber	nding (b), d	eflection	1 (d		
2" x 2" x 0.043")	7'-0"	di	6'-9"	d	6'-5"	d	6'-3"	d	6'-0"	d	5'-10"	d	5'-8"	d
3" x 2" x 0.045"	7'-11"	d	7-7	d	7'-3"	d	7'-1"	d	6'-9"	b	6'-5"	b		b
3" x 2" x 0 070"	9'-0"	d	8'-8"	d	8'-3"	d	8'-0"	ď	7'-9"	d	7'-7"	ď	Z-3.	d
2" x 3" x 0.045")	10'-0"	d	9'-7"	d	9'-2"	d	8'-11"	d	8'-8"	d	8'-5"	1	8'-1"	d
3" x 3" x 0.062"	10'-9"	d	10'-4"	d	9'-10"	d	9'-7"	d	9'-3"	d	9'-0"	d	8-6	Ъ
2" x 4" x 0.050"	13'-2"	d	12'-7"	d	12'-0"	d	11'-8"	d	11'-4"	d	10'-11"	d	10'-7"	d
3" x 3" x 0.090"	13'-2"	d	12'-7"	d	12'-1"	d	11'-9"	d	11'-4"	d	11'-0°	ď	10'-8"	d
3" x 3" x 0.090"	14'-6"	d	13'-10"	d	13'-3"	d	12'-10"	d	12'-5"	d	12'-1"	d	11'-8"	d
2" x 5" x 0.060"	16'-9"	d	16'-0"	d	15'-4"	d	14'-11"	d	14'-5"	d	14'-0"	d	13'-6"	d
4" x 4" x 0.125"	19'-4"	d	18'-6"	d	17'-8"	d	17'-2"	d	16'-7"	d	16'-2"	d	15'-7"	d
3. Sections As Horizon	ntals Faster	ned	To Post	3 7	hrough	Sic	le Into S	сге	w Boss	es			15.0	_
			Tributar	y L	oad Wid	ith	W = Ma	mt	er Spac	ing	1		150	Т
Hollow Sections	3'-6"		4'-0"	_	4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	All	ow	able Hei	ght	"H" or	Spa	n "L" /	ber	nding (b), d	eflection	1 (d)	
2" x 2" x 0.043"	9'-5"	d	8'-11"	b	8'-4"	Ь	7'-11"	b	7'-6"	Ь	7'-2"	b	6'-9"	b
3" x 2" x 0.045"	9'-9"	Ь	9'-1"	b	8'-5"	Ь	7'-11"	Ь	7'-7"	b	7'-2"	b	6'-9"	b
3" x 2" x 0.070"	12'-1"	d	11'-7"	d	11'-1"	d	10'-9"	d	10'-5"	d	10'-1"	d	9'-9"	d
2" x 3" x 0.045"	13'-4"	b	12'-6"	b	11'-8"	Ь	11'-2"	b	10'-8"	b	10'-2"	Ь	9'-7"	b
3" x 3" x 0.062"	13'-7"	Ь	12-7*	Ь	11'-8"	Ь	11'-2"	Ь	10'-7"	Ь	10'-1"	b	9'-6"	b
2" x 4" x 0.050"	17'-3"	b	16'-1"	Ь	14'-11"	Ь	14'-3"	Ь	13'-6"	Ь	12'-11"	Ь	12'-2"	Ь
3" x 3" x 0.090"	17'-8"	d	16'-11"	d	16'-2"	d	15'-8"	đ	15'-2"	d	14'-9"	d	14'-3"	b
3" x 3" x 0.125"	19'-5"	d	18'-7"	d	17'-9"	d	17'-3"	d	16'-8"	d	16'-3"	d	15'-8"	d
2" x 5" x 0.060"	22'-4"			Ь	19'-6"	b	18'-9"	b	17'-10"	Ь	17'-1"	Ь	16'-2"	Ь
4" x 4" x 0.125"	25'-10"	+-	24'-9"	۲÷	23'-7"	1	22'-11"	1.	22'-3"	14	21'-7"	77	20'-10"	ta

- 1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".

 2. Using screen panel width "W select girt lengths.

 3. Site specific engineering required for pool enclosures over 30" in mean roof height.

 4. Spartheight is to be measured from center of beam and upright connection to fascia or wall connection.

 5. Chair rails of 2" x 2" x 0.044" min. and set @ 35" in height are designed to be residential gardrails provided they are attached with min. (3) #10 x 1-1/2" s.m.s. into the screw bosses and do not exceed 8"-0" o.c.

 6. Girt spacing shall not exceed 6"-8".

 7. Max. beam size for 2" x 5" is 2" x 7" x 0.055" x 0.120"

 8. 2" x 4" & 2" x 5" is 10" in the connected whan internal or external 1-1/2" x 1-1/2" x 0.044" angle.

- 8. 2" x 4" & 2" x 5" hollow girts shall be connected w/ an internal or external 1-1/2" x 1-1/2" x 0.044" angle.
- Spans/heights may be interpolated.
 To convert spans to "C" and "D" exposure categories see exposure multipliers and example on Table 1B Page 3.

Table 1.5.2 110 110 E 6061

Allowable Spans for Miscellaneous Framing Beams as Supporting Screen Roof Frame Members Both Ends of Beam Attached to Host Structure (Not Axially Loaded)

Eagle Metal Distributors, Inc.

(110 MPH) Aluminum Alloy 6061 T-6

for Areas with Wind Loads up to 110 M.P.H., Exposure "B" and Latitudes Below 30*-30*-00" North (Jacksonville, FL)
Uniform Load = 4 #/ft., a Point Load of 300 # over (1) linear ft. is also considered

									1	Γri	butary	Lo	ad Wid	th										
Single Self-Mating	10'-0'	•	14'-0"		18'-0"		22'-0"		26'-0"	17.1	30'-0"		34'-0"		38'-0"		42'-0"		46'-0"		50'-0"		54'-0"	
Beams			Allowa	ble	Span	'L'	/ Poin	tL	oad (P)	or	Unifor	m	Load (L	J),	bendin	9 (b), defl	ect	ion (d)	_		_		
2" x 4" x 0.044" x 0.100" SMB	16'-3"	d	14'-6"	ď	13'-4"	d	12'-6"	D G	11'-10"	d	11'-3"	Ь	10'-7"	O.b	10'-0"	b	9*-6*	b	9'-1"	Ь	8*-9*	Ь	8'-5"	Р
2" x 5" x 0.050" x 0.116" SMB	18'-5°	d	16'-5"	ď	15'-2"	U	14'-2"	ď	13'-5"	d	12'-6"	b	11'-9"	b	11'-1"	b	10'-6"	D	10'-1"	U	9"-8"	b	9'-4"	b
2" x 6" x 0.050" x 0.120" SMB	21'-8"	U	19'-4"	U d	17"-9"	d	16'-8"	D D	15'-7"	U	14'-6"	U	13'-8"	D	12'-11"	D	12'-3"	U	11'-9"	Db	11'-3"	D	10'-10'	b
2" x 7" x 0.055" x 0.120" SMB	24'-8"	U	22'-0"	D U	20'-3"	d U	18'-11"	D	17"-7"	b	16'-4"	U	15-4*	DP	14'-6"	D	13'-10"	D	13'-2"	D	12'-8"	DР	12'-2"	b
2" x 8" x 0.072" x 0.224" SMB	30'-5"	D	27'-2"	D	24'-11'	ď	23'-4"	D G	22'-1"	d	20'-11"	U	19'-8"	b	18'-7"	b	17'-8"	U	16'-11"	D	16'-3"	D	15'-7"	Ь
2" x 9" x 0.072" x 0.224" SMB	32'-11'	U	29'-6"	U	27'-1"	U	25'-4"	D	23'-11"	b	22'-4"	U	20'-11"	P	19'-10"	b	18'-10"	P	18'-0"	b	17'-3"	P	16'-8"	Ь
2" x 9" x 0.082" x 0.306" SMB	35'-7"	d	31'-9"	d	29'-3"	D	27'-4"	d	25'-10"	U	24'-8"	D	23'-6"	U	22'-2"	b	21'-1"	Ub	20'-2"	D	19'-4"	P	18'-8"	Ь
2" x 10" x 0.092" x 0.374" SMB	41'-5"	ď	36'-11'	d	34'-0"	d	31'-10"	d d	30'-1"	U	28'-8"	d	27'-6"	d	26'-5"	Ь	25'-2"	Ь	24'-1"	Ub	23'-1"	P	22'-2"	b
		_		=		=			-	Tri	butary	Lo	ad Wid	th				_				_		
Double Self-Mating	10'-0'	•	14'-0"	•	18'-0"	•	22'-0"		26'-0"		30'-0"		34'-0"		38'-0"		42'-0"	•	46'-0"		50'-0'		54'-0'	
Beams		All	owable	S	an 'L'	11	Point Lo	oac	(P) or	Un	iform L	.02	id (U), b	er	nding (b),	deflecti	ion	(d)					
(2) 2" x 8" x 0.072" x 0.224"	38'-3"	U	34'-3"	D d	31'-6"	D	29'-5"	D d	27'-10"	Dd	26'-6"	D G	25'-5"	D	24'-6"	PC	23'-9"	D G	23'-0"	D	22'-5"	P	21'-10'	d
(2) 2" x 9" x 0.072" x 0.224"	41'-6"	U	37'-2"	D	34'-2"	U	31'-11"	D	30'-3"	U	28'-10"	d	27'-7"	d	26'-7"	D	25'-9"	P	24'-11"	D	24'-4"	Dd	23'-6"	Ub
(2) 2" x 9" x 0.082" x 0.206"	44'-10	U	40'-1"	U	36'-10"	U	34'-5"	U	32'-7"	U	31'-1"	ď	29'-9"	ď	28'-8"	ų U	27'-9"	U	26'-11"	D.C	26'-2"	D d	25'-6"	D d

(2) 2" x 10" x 0.092" x 0.374" 52-2" d 46-7" d 42-10 d 40-1" d 37-11 d 36-2" d 34-8" d 33-5" d 32-4" d 31-4" d 30-6" d 29-9" d

- It is recommended that the engineer be consulted on any carrier beam that spans more than 50°
- Span is measured from center of connection to fascia or wall connection.
 Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans
- for total beam spans.

 4. Spans may be interpolated.

 5. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on Table 1B Page 3.

Allowable Spans for Miscellaneous Framing Beams as Supporting Screen Roof Frame Members One End of Beam Attached to Host Structure (Axially Loaded) Table 1.5.1 110 110 E 6061 Eagle Metal Distributors, Inc. Aluminum Alloy 6061 T-6

for Areas with Wind Loads up to 110 M.P.H., Exposure "B" and Latitudes Below 30"-30"-00" North (Jacksonville, FL)
Uniform Load = 4 #/ft., a Point Load of 300 # over (1) linear ft. is also considered

	$\overline{}$									Tri	butary l	-0	ad Wid	th									
Single Self-Mating	10'-0'	•	14'-0		18'-0		22'-0'	10	26'-0"		30'-0"		34'-0"		38'-0"	15	42'-0"	46'-0	_	50'-0'	•	54'-0'	•
Beams		Alle	owable	S	oan 'L'	/ F	oint L	oac	(P) or	Ur	iform L	oa	id (U), b	oen	iding (b), (deflection	(d)	_		_		_
x 4" x 0.044" x 0.100" SMB	16'-3"	d	14'-6'	U d	13'-4'	D d	11'-11'	D	10'-9"	D	9'-10"	b	90.	Ub	8'-4"	Ub	7'-9" U	7'-3"	b	6*-9*	Ь	6'-4"	Ь
x 5" x 0.050" x 0.116" SMB	18'-5"	D U	16'-5'	U d	14'-11	Ub	13'-4"	U	12'-0"	Ub	10'-11"	b	10'-2"	Ub	9'-5"	U	8-9° D	8'-2"	b	7'-8"	D	7'-2"	b
x 6" x 0.050" x 0.120" SMB	21'-8"	D U	19'-4'	U	17'-8'	Ub	15'-9"	Ub	14'-3"	Ub	13'-0"	b	12'-1"	Ub	11'-3"	U	10'-6" U	9'-10'	. U	9'-3"	Ь	8'-9"	b
7" x 0.055" x 0.120" SMB	24'-8"	P	22'-0"	U d	19'-11	U _b	17'-10	P	16'-3"	Ub	14'-11"	b	13'-10"	UЬ	12'-10"	UЬ	12'-1" U	11'-4'	. U	10'-9"	Ь	10'-2"	b
x 8" x 0.072" x 0.224" SMB	30-5	U	27'-2'	U d	24'-11	U	23'-4"	Ud	21'-4"	Ub	19'-8"	b	18'-4"	Ub	17"-2"	D	16'-2" U	15'-4'	b U	14'-7"	U	13'-10'	b
c 9" x 0.072" x 0.224" SMB	32'-11'	U	29'-6'	· d	27'-1"	U	24'-11'	P	22'-10"	b	21'-1"	b	19'-7"	Ub	18'-5"	υb	17'-4" U	16'-5'	. U	15'-7"	D	14'-11'	Ь
9" x 0.082" x 0.306" SMB	35'-7"	U	31'-9'	. U	29'-3'	Ud	27'-4"	U	25'-9*	U	23'-10"	b	22'-3"	U	20'-11"	P	19'-9" U	18'-9'	. U	17'-10'	D	17'-0"	b
x 10" x 0.092" x 0.374" SMB	41'-5"	D	36'-11	-U	34'-0"	U d	31'-10	U	30'-1"	d	28'-8"	U	26'-10"	D	25'-3"	Ub	23'-10" U	22'-8'	. U	21'-8"	D	20'-8"	Ь
	1			=		=		=		Tri	butary I	Lo	ad Wid	th		_				27/2-282-2		in visions	
Double Self-Mating	10'-0		14'-0	*	18'-0	*	22'-0	**	26'-0"		30'-0"		34'-0"	•	38'-0"		42'-0"	46'-0	ř.	50'-0'	-	54'-0'	•
Beams		All	owable	e S	pan 'L'	11	oint L	oad	d (P) or	Ur	iform L	oa	ad (U), I	ber	nding (b), (deflection	n (d)					
	$\overline{}$	111		100		111		TIT	1	нΤ		11		HIT		HIL			- 111		101		10

2" x 9" x 0.082" x 0.306" SMB	35-1	d	31-9	d	29-3	d	21-4	d	23-5	b	23-10	b	22.3	b	20-11	b	10-0	Ь	10-0	Þ	11 -10	b	17-0
2" x 10" x 0.092" x 0.374" SMB	41'-5"	D	36'-11'	ď	34'-0°	D	31'-10"	d	30'-1"	d	28'-8"	b	26'-10"	b	25'-3"	Ь	23'-10'	Ь	22'-8"	Ь	21'-8"	ь	20'-8"
										Tri	butary	Lo	ad Widt	h							V/2-215-2		raskes:
Double Self-Mating	10'-0	*	14'-0		18'-0"	•	22'-0'	•	26'-0	•	30'-0'	•	34'-0"		38'-0"		42'-0'		46'-0"		50'-0'		54'-0"
Beams		All	owable	S	pan 'L'	11	Point L	oad	d (P) or	Ur	niform I	Loa	ad (U), b	en	ding (b),	deflect	ion	(d)				
(2) 2" x 8" x 0.072" x 0.224"	38'-3"	U	34'-3"	D d	31'-6"	d	29'-5"	ď	27'-10	Q U	26'-6"	DO	25'-5"	d	24'-6"	d	23'-9"	d	22'-10"	U	21'-9"	Ь	20'-10'
(2) 2" x 9" x 0.072" x 0.224"	41'-6"	U	37'-2"	U	34'-2"	D d	31'-11'	g	30,-3,	D	28'-10'	U	27-7*	U	26'-7"	ď	25'-7"	b	24'-5"	D	23'-4"	P	22'-4"
(2) 2" x 9" x 0.082" x 0.206"	44'-10	U	40'-1"	U	36'-10'	U	34'-5"	d	32'-7"	D	31'-1"	U	29'-9"	d	28'-8"	U d	27'-9"	D D	26'-11"	q	26'-2"	d	25'-3"
(2) 2" x 10" x 0.092" x 0.374"	52'-2"	U	46'-7"	U	42'-10	U	40'-1"	d	37'-11	U	36'-2"	U	34'-8"	d	33'-5"	U	32'-4"	d	31'-4"	d	30'-6"	d	29'-9"
Note:							·						FOI										

- 1. It is recommended that the engineer be consulted on any carrier beam that spans more than 50° Span is measured from center of connection to fascia or wall connection.
- 3. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
- Spans may be interpolated.
 To convert spans to "C" and "D" exposure categories see exposure multipliers and example on Table 1B Page 3.

EAGLE 6061 ALLOY IDENTIFIER™ INSTRUCTIONS FOR PERMIT PURPOSES

To: Plans Examiners and Inspectors,

These identification instructions are provided to contractors for permit purposes. The detail below illustrates our unique "raised" external identification mark (Eagle 6061™) and its location next to the spline groove, to signify our 6061 alloy extrusions. It is ultimately the purchaser's / contractor's responsibility to ensure that the proper alloy is used in conjunction with the engineering selected for construction. We are providing this identification mark to simplify identification when using our 6061 Alloy products.

A separate signed and sealed certification letter from Eagle Metals will be provided once the metal is purchased. This should be displayed on site for review at final inspection

The inspector should look for the identification mark as specified below to validate the use of 6061 engineering.

EAGLE 6061 I.D. DIE MARK

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21

Screw Size	Minimum Distance a	and Spacing of Screws
	Edge To Center	Center To Center
#8	5/16"	5/8*
#10	3/8"	3/4*
#12	1/2*	1"
#14 or 1/4"	3/4"	1-1/2"
5/16"	7/8"	1-3/4"
3/8"	1*	2*

Gusset Plate Thic	kness
Beam Size	Thickness
2" x 7" x 0.055" x 0.120" SMB	0.063*
2" x 8" x 0.082" x 0.306" SMB	0.125*
2" x 9" x 0.072" x 0.224" SMB	0.125*
2" x 9" x 0.082" x 0.306" SMB	0.190*
2" x 10" x 0.092" x 0.374" SMB	0.250*

Connection Example

2" x 7" Beam & 2" x 4" at beam & gusset plate, (14) #8 x 1/2" sms & upright & gusset plate (14) #8 x 1/2" sms ea. side of beam & upright.

1. Connection of 2* x 6* to 2* x 4* shall use a full lap cut or 1/16* gusset plate.

2. For beam splice connections the number of screws shown is the total for each splice with 1/2 the screws on each side of the cut.

3. The number of deck anchors is based on RAWL R Tapper allowable load data for 2,500 psi concrete and / or equal anchors may be used.

The number shown is the total use 1/2 per side.

4. Hollow splice connections can be made provided the connection is approved by the engineer.

4. Hollow spice connections can be made provided the connection is approved by the engineer.
5. If a larger than minimum upright is used the number of screws is the same for each splice with 1/2 the screws on each side of the cut.
6. The side wall upright shall have a minimum beam size as shown above, ie., a 2" x 4" upright shall have a 2" x 3" beam.
7. For minimum girt size read upright size as a beam and purlin size is minimum girt size. (i.e. 2" x 9" x 0.072" x 0.219" s.m.b. w/
2" x 5" x 0.050 x 0.135" s.m.b. upright requires a 2" x 3" x 0.045" girt / chair rail.)
8. All connections shall use a full lap cut.

Minimum Size Screen Enclosure Knee Braces and Anchoring Required Table 1.7 Aluminum 6063 T-6

Brace Length*	Extrusion	Anchoring System
0' - 2'-0"	2" x 2" x 0.044"	2" H-Channel With (3) #10 x 1/2" each leg of channel
To 3'-0"	2" x 3" x 0.045"	2" H-Channel With (3) #10 x 1/2" each leg of channel
Up to 6'-0"	2" x 4" x 0.044" x 0.100"	2" H-Channel With (4) #10 x 1/2" each leg of channel

Knee brace length shall be the horizontal and vertical length @ a 45 angle from the center of the connection to the face of the beam or upright.

For required knee braces greater than 4'-6" contact engineer for specifications and details

Cantilever beam detail shall be used for transom wall to host structure attachment when knee brace length exceeds 6-0°.

Table 1.8 K-Bracing Fastening Schedule For 120 MPH "C" Exposure

Maximum Corner Post Diagg Wall Width = @ Top ps 20'-0" 2	Number of #10 x 3/4" S.M.S. Required												
		Diagonals (K) per End	Intermediate Post @ Chair Rail	Corner Post @ Bottom	Plate to Sole Plate								
	2	4	2	2									
30'-0"	2	2	4	2	2								
40'-0"	3	4	6	2	2								
50'-0"	4	5	8	3	3								
60'-0"	6	7	12	3	3								

* Use screw sizes specified in the table below Use front wall width when determining number of s.m.s. for the side wall K-bracing.

on number of s.m.s. for the front and / or back wall K-bracing

Wind Zone, Exposure "C"	Screw Siz
90-130 MPH	#10
130 MPH	#12
140 - 150 MPH	#14

Table 1.11 Maximum Overhang for Rafter / Truss Tails when Connected to Screen Roof

20' Max. Encl	losure Spart	Ra	fter.	Truss T	aiı#	Span /	ben	ding (b) o	r de	flection (d)				
Wind Zone ("B" Exp.)	Wind Pressure (#/SF)	2x4		2x6		2x8		2x10		2x12					
100-110	4	2'-2"	Ь	5'-4"	b	9:-3"	b	15'-0"	b	22'-3"	b				
120	4	2'-2"	Ь	5'-4"	b	9'-3"	Ь	15'-0"	b	22'-3"	b				
123	4.3	2'-0"	b	4'-11"	b	8'-7"	b	13'-11"	b	20'-8"	b				
130	5	1'-9"	b	4'-3"	b	7-5*	b	12'-0"	b	17'-10"	b				
140	6	1'-5"	Ь	3'-7"	b	6'-2"	b	10'-0"	b	14'-10"	b				
150	7	1'-3"	Ь	3'-0"	b	5'-3"	b 8'-7"		b	12'-9"	b				
30' Max. Enclosure Span		Ra	Rafter / Truss Tail #2 Span / bending (b) or deflection (
Wind Zone ("B" Exp.)	Wind Pressure (#/SF)	2x4		2x6		2x8		2x10)	2x12					
100-110	4	1'-5"	b	3'-7"	b	6'-2"	b	10'-0"	b	14'-10"	b				
120	4	1'-5"	b	3'-7"	b	6'-2"	b	10'-0"	b	14'-10"	b				
123	4.3	1'-4"	Ь	3'-4"	b	5'-9"	b	9'-4"	b	13'-10"	b				
130	5	1'-2"	b	2'-10"	b	4'-11"	Ь	8,-0,	b	11'-10"	b				
140	6	0'-11"	Ь	2'-4"	b	4'-1"	b	6"-8"	b	9'-11"	b				
150	7	0'-10"	Ь	2'-0"	b	3'-6"	Ь	5'-9"	b	8'-6"	b				
40' Max. Enc	losure Span	Ra	flection (d)											
Wind Zona ("B" Exp.)	Wind Pressure (#/SF)	2x4		2x6		2x8		2x10)	2x12					
100-110	4	1'-1"	Ь	2'-8"	b	4'-7"	b	7'-6"	b	11'-1"	b				
120	4	1'-1"	b	2'-8"	b	4'-7"	b	7'-6"	b	11'-1"	b				
123	4.3	1'-0"	Ь	2'-6"	b	4'-4"	b	6'-11"	b	10'-4"	b				
130	5	0'-10"	Ь	2'-2"	b	3'-8"	b	6'-0"	b	8'-11"	b				
140	6	0"-9"	b	1'-9"	ь	3'-1"	b	5'-0"	b	7'-5"	b				
150	7	0'-7"	Ь	1'-6"	b	2'-8"	Ь	4'-4"	Ь	6'-4"	ь				

 For overhangs with spans that exceed those listed above site specific engineering is required. If truss bottom cord extends more than 24" over the wall site specific engineering is required.
 To convert from exposure "B" spans to "C" or "D" exposure spans see multipliers and example Table 1B on page3

Example:

For a pool inclosure with 30' max. beam span, in a 123 MPH wind zone, "B" exposure. For 2 x 6 rafter / truss the max overhand from the wail of the host structure to the sub-fascia is 3'-4".

LATITUDES NORTH 30 - 30' - 00" NORTH (JACKSONVILLE, FL)

Table 1.9.1 E 6061

Allowable Beam Spans

Eagle Metal Distributors, Inc.

Aluminum Alloy 6061 T-6
sure "B" and Latitudes North of 30"-30"-00" North (Jacksonville, FL) Uniform Load = 15 #/SF, a Point Load of 300 #/SF over (1) linear ft. is also considered

e Span 'l e Span 'l ed 5'-9" ed 8'-2" Jd 11'-0' Jd 11'-11 Jd 15'-3'	Pd Pb Ud	5'-0' oint Load 5'-9" 6'-5" 7'-7"	Pd Ub Ud	6'-0" or Unife 5'-7" 5'-9" 7'-2"	Ud Ub	5'-3" 5'-2"	, ber	8'-0" ding (b) 4'-10" 4'-9"	, def	9'-0" lection 4'-7" 4'-4"	(d) Ub		
Pd 5'-9" Pb 6'-9" Pd 8'-2" Jd 11'-0" Jd 11'-11	Pd Pb Ud Ud	5'-9" 6'-5" 7'-7"	Pd Ub Ud	5'-7* 5'-9*	Ud Ub	5'-3" 5'-2"	Ub	4'-10"	Ub	4'-7"	Ub		
b 6'-9" d 8'-2" Jd 11'-0' Jd 11'-11	Pb Ud	6'-5" 7'-7"	Ub	5'-9"	Ub	5'-2"	1	_	-	COMPAND OF THE PARTY OF THE PAR			
Pd 8'-2" Jd 11'-0" Jd 11'-11	Ud	7'-7"	Ud		-	-	Ub	4'-9"	Ub	4'-4"	11:15-		
Jd 11'-0'	Ud		_	7'-2"	114						Ub		
Jd 11'-11		10'-3"			Ud	6'-9"	Ud	6'-6"		6'-2"	Ud		
	- IIId		Ud	9'-7"	Ud	9'-2"	Ud	8'-8"	Ub	8'-7"	Ub		
Jd 15'-3'		11'-2"	Ud	10'-5"	Ub	9'-6"	Ub	8'-9"	UЪ	8'-6"	Ub		
	Ud	14'-2"	Ud	13'-4"	Ud	12'-8"	Ub	11'-9"	Ub	10'-4"	Ub		
Tributary Load Width 'W' = Beam Spacing													
3'-0" 4'-0"		5'-0'	6'-0"		7'-0"		8"-0"		9'-0"				
e Span '	' / P	oint Loa	d (P)	or Unife	orm l	oad (U)	, ber	nding (b)	, def	lection	(d)		
Jd 14'-2'	Ud	12'-11"	Ub	11'-10"	Ub	10'-11"	Ub	10'-2"	Ub	9'-5"	Ub		
Jd 16'-1'	Ud	14'-7"	Ub	13'-3"	Ub	12'-3"	Ub	11'-5"	Ub	10'-1"	Ub		
Jd 18'-4'	Ub	16'-4"	Ub	14'-11"	Ub	13'-9"	Ub	12'-10"	Ub	12'-6"	Ub		
Jb 20'-3'	Ub	18'-1"	Ub	16'-5"	UЪ	15'-2"	Ub	14'-2"	Ub	13'-10"	Ub		
Jd 26'-6'	Ud	24'-8"	Ud	23'-2"	Ud	22'-0"	Ud	20'-8"	Ub	19'-2"	Ub		
Jd 28'-10	* Ud	26'-9"	Ud	24'-8"	Ub	22'-9"	Ub	21'-3"	UЪ	20'-2"	Ub		
Jd 31'-1'	Ud	28'-10"	Ud	27'-2"	Ud	25'-9"	Ud	24'-8"	Ud	23'-2"	Ub		
	Ud	33'-7"	Ud	31'-7"	Ud	30'-0"	Ud	28'-8"	Ud	27'-7"	Ud		
	Id 16'-1' Id 18'-4' Ib 20'-3' Id 26'-6' Id 28'-10 Id 31'-1'	Jd 16'-1" Ud Jd 18'-4" Ub Jb 20'-3" Ub Jd 26'-6" Ud Jd 28'-10" Ud Jd 31'-1" Ud	Jd 16'-1" Ud 14'-7" Jd 18'-4" Ub 16'-4" Jb 20'-3" Ub 18'-1" Jd 26'-6" Ud 24'-8" Jd 28'-10" Ud 26'-9" Jd 31'-1" Ud 28'-10"	Id 16'-1" Ud 14'-7" Ub Id 18'-4" Ub 16'-4" Ub Ib 20'-3" Ub 18'-1" Ub Id 26'-6" Ud 24'-8" Ud Id 28'-10" Ud 26'-9" Ud Id 31'-1" Ud 28'-10" Ud	Id 16:-1" Ud 14'-7" Ub 13:-3" Id 18'-4" Ub 16'-4" Ub 14'-11" Ib 20'-3" Ub 18'-1" Ub 16'-5" Id 26'-6" Ud 24'-8" Ud 23:-2" Id 28'-10" Ud 28'-9" Ud 24'-8" Id 31'-1" Ud 28'-10" Ud 27:-2"	Id 16-1" Ud 14-7" Ub 13-3" Ub Id 18-4" Ub 16-4" Ub 14-11" Ub Ib 20-3" Ub 18-1" Ub 16-5" Ub Id 28-6" Ud 24-8" Ud 23-2" Ud Id 28-10" Ud 28-9" Ud 24-8" Ud Id 31-1" Ud 28-10" Ud 27-2" Ud Id 31-1" Ud 28-10" Ud 27-2" Ud	Id 16-1" Ud 14-7" Ub 13-3" Ub 12-3" Id 18-4" Ub 16-4" Ub 14-11" Ub 13-9" Ib 20-3" Ub 18-1" Ub 16-5" Ub 15-2 Id 26-6" Ud 24-8" Ud 23-2" Ud 22-0" Id 28-10" Ud 28-9" Ud 24-8" Ub 22-9" Id 31-1" Ud 28-10" Ud 27-2" Ud 22-9" Id 31-1" Ud 28-10" Ud 27-2" Ud 25-9"	Id 16-1" Ud 14-7" Ub 13-3" Ub 12-3" Ub 16 18-4" Ub 16-4" Ub 18-4" Ub 18-4" Ub 18-6"	Id 16-1" Ud 14-7" Ub 13-3" Ub 12-3" Ub 11-5" Id 18-4" Ub 16-4" Ub 14-11" Ub 13-2" Ub 12-10" Ib 20-3" Ub 18-1" Ub 18-5" Ub 12-20" Ub 12-10" Id 26-6" Ud 24-8" Ud 23-2" Ud 22-0" Ud 20-8" Id 28-10" Ud 26-9" Ud 24-8" Ub 22-2" Ud 21-3" Id 31-1" Ud 28-10" Ud 22-2" Ud 22-9" Ud 21-3"				

Note:

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".

2. The structures designed using this section shall be limited to a maximum combined span and upright height of 50' and a maximum upright height of 16. Structures larger than these limits shall have site specific engineering.

3. Span is measured from center of beam and upright connection to fascia or wall connection.

4. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.

5. Tables are based on a maximum wall height of 16' including a 4' max. mensard or gable. Other conditions may offer better spans w enclosure site specific engineering.

6. Spans may be interpolated.

7. To convert spans to "O" and "D" exposure categories see exposure multipliers and example on page 1.15".

7. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

Table 1.9.2

Allowable Purlin Spans Eagle Metal Distributors, Inc. Aluminum Alloy 6061 T-6

For 130 MPH Wind Zones, Exposure "B" and Latitudes North of 30°-30°-00" North (Jacksonville, FL) Uniform Load = 15 #/SF, a Point Load of 300 #/SF over (1) linear ft. is also considered

A. Sections Fastened With Clips

	Tributary Load Width "W" = Purlin Spacing													
Hollow Sections	3'-6"		4'-0'	4'-0"		4'-6"		5'-0"		5'-6"			6'-8"	
	Allowa	ble \$	Span 'L'	/ Po	int Loa	d (P)	or Unif	orm	Load (U)	, ber	nding (b)	, de	flection	(d)
2" x 2" x 0.043"	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd	5'-9"	Pd	5'-7"	Ud	5'-5"	Ud
3" x 2" x 0.045"	6'-9"	Pb	6'-9"	Pb	6'-9"	Pb	6'-5"	Ub	6'-1"	Ub	5'-9"	Ub	5'-4"	Ub
3" x 2" x 0.070"	8'-3"	Pd	8'-2"	Ud	7'-10"	Ud	7'-7"	Ud	7'-4"	Ud	7'-2"	Ud	6'-11"	Ud
2" x 3" x 0.045"	11'-6"	Ud	11'-0"	Ud	10'-7"	Ud	10'-3"	Ud	9'-11"	Ud	9'-7"	Ud	9'-3"	Ud
2" x 4" x 0.050"	12'-6°	Ud	11'-11"	Ud	11'-6"	Ud	11'-2"	Ud	10'-9"	Ud	10'-5"	Ub	9'-10"	Ub
2" x 5" x 0.060"	15'-11"	Ud	15'-3"	Ud	14'-8"	Ud	14'-2"	Ud	13'-9"	Ud	13'-4"	Ud	12'-11"	Ud
B. Sections Fastened Throu	gh Beam We	bs I	nto Scre	w Bo	osses									
Contract Contract	Tributary Load Width 'W' = Purlin Spacing													
Hollow Sections	3'-6'	6" 4'-0		" 4'-6"			5'-0"		5'-6"		6'-0"		6'-8"	
	Allowa	ble !	Span 'L'	/ Po	oint Loa	d (P)	or Unif	orm	Load (U)	, bei	nding (b	, de	flection	(d)
2" x 2" x 0.043"	7'-5"	Pb	7'-4"	Ub	6'-10"	Ub	6'-5"	Ub	6'-1"	Ub	5'-9"	Ub	5'-5"	Ub
3" x 2" x 0.045"	7'-11"	Ub	7'-4"	Ub	6'-10"	Ub	6'-5"	Ub	6'-1"	Ub	5'-9"	Ub	5'-4"	Ub
2" ~ 2" ~ 0.070"	1441.47	IIII	1101.78	II II	01.11	Lib	0'.6"	II II	0,0,	LIB	8'-7"	LIB	8'-1"	1 lb

Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
 Span is measured from center of beam and upright connection to fascia or wall connection.
 Tables are based on a maximum wall height of 16' including a 4' max. mansard or gable. Other conditions may offer better spans will

3. Tables are based on a maximum wan neight of 16 including a 4 max. mansard of gable. Other conditions enclosure site specific engineering.

4. Spans may be interpolated.

5. 2° x 4° 8.2° x 5° Holliow Girst shall be connected w/ an internal or external 1-1/2° x 1-1/2° x 0.044° angle.

6. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii. CHECK TABLE 1.6 FOR MINIMUM PURLIN SIZE FOR BEAMS.

AND METAL DISTRIBUTORS, INC.

◆ THE FREEDOM OF CHOICE ◆ W. LANDSTREET ROAD, ORLANDO, FL 32824 TRL: 407-367-0688 ◆ FAX: 407-367-0684

MANUAL ALUMINUM STRUCTURES DESIGN SCREEN ENCLOSURES SECTION 1 TABLES

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NOT TO BE REPRODUCED IN WHOLE OR IN PART

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