

DATE 09/05/2006

Columbia County Building Permit

PERMIT
000024947

This Permit Expires One Year From the Date of Issue

APPLICANT FRANK LEMASTER PHONE 904.723.2700
ADDRESS 819 TOWNSEND BLVD., STE 6 JACKSONVILLE FL 32211
OWNER ARTHUR BYRNE PHONE 386.961.8933
ADDRESS 3013 NW NASH ROAD LAKE CITY FL 32211
CONTRACTOR PATRICIA PATTON PHONE 904.723.2700
LOCATION OF PROPERTY 90-W TO BROWN RD,TR TO NASH RD,TL AND IT'S 1/4 MILE DOWN
ON THE L @ MAILBOX.
TYPE DEVELOPMENT ALUM. GLASSROOM ESTIMATED COST OF CONSTRUCTION 13900.00
HEATED FLOOR AREA TOTAL AREA HEIGHT STORIES 1
FOUNDATION CONC WALLS ROOF PITCH FLOOR
LAND USE & ZONING A-3 MAX. HEIGHT 35
Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00
NO. EX.D.U. 1 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 17-3S-16-02169-003 SUBDIVISION
LOT BLOCK PHASE UNIT TOTAL ACRES 5.00

CRC058128
Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor
EXISTING 06-0703-E BLK JTH N
Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident

COMMENTS: LETTER OF AUTHORIZATION GIVEN.

Check # or Cash 5052

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power Foundation Monolithic
date/app. by date/app. by date/app. by
Under slab rough-in plumbing Slab Sheathing/Nailing
date/app. by date/app. by date/app. by
Framing Rough-in plumbing above slab and below wood floor
date/app. by date/app. by
Electrical rough-in Heat & Air Duct Peri. beam (Lintel)
date/app. by date/app. by date/app. by
Permanent power C.O. Final Culvert
date/app. by date/app. by date/app. by
M/H tie downs, blocking, electricity and plumbing Pool
date/app. by date/app. by
Reconnection Pump pole Utility Pole
date/app. by date/app. by date/app. by
M/H Pole Travel Trailer Re-roof
date/app. by date/app. by date/app. by

BUILDING PERMIT FEE \$ 70.00 CERTIFICATION FEE \$ 0.00 SURCHARGE FEE \$ 0.00
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$
FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ TOTAL FEE 145.00
INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVENIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Left Message 8-31-06 JH

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0608-78 Date Received 8/22/06 By GT Permit # 24947
Application Approved by - Zoning Official BLK Date 30-08-06 Plans Examiner OK JTH Date 8-28-06
Flood Zone X Development Permit N/A Zoning A-3 Land Use Plan Map Category A-3
Comments _____

(CNC)
Applicants Name Frank Lemaster Fax: 273-3055
Address 819 Townsend BLVD #6 Jacksonville, FL 32211 Phone 35-4796
Owners Name Arthur Byrne Phone 386-961-8983
911 Address 3013 NW Nash Rd. Lake City FL 32055
Contractors Name Patricia Patton Phone (904) 723-2700
Address 819 Townsend Blvd #6 Jacksonville, FL 32211
Fee Simple Owner Name & Address Same as above
Bonding Co. Name & Address N/A
Architect/Engineer Name & Address _____
Mortgage Lenders Name & Address N/A

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 17-35-14-02149-003 Estimated Cost of Construction \$13,900

Subdivision Name _____ Lot _____ Block _____ Unit _____ Phase _____

Driving Directions 90 W, (R) Brown, (R) Nash Rd

then
ON (R) 1/2 mile Down see mailbox

Type of Construction Aluminum Glass Room Number of Existing Dwellings on Property 1

Total Acreage 5 Lot Size _____ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 314' Side (31) Side 43' Rear 950'

Total Building Height _____ Number of Stories 1 Heated Floor Area 216 Roof Pitch _____
TOTAL 216

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Arthur Byrne
Owner Builder or Agent (Including Contractor)

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me

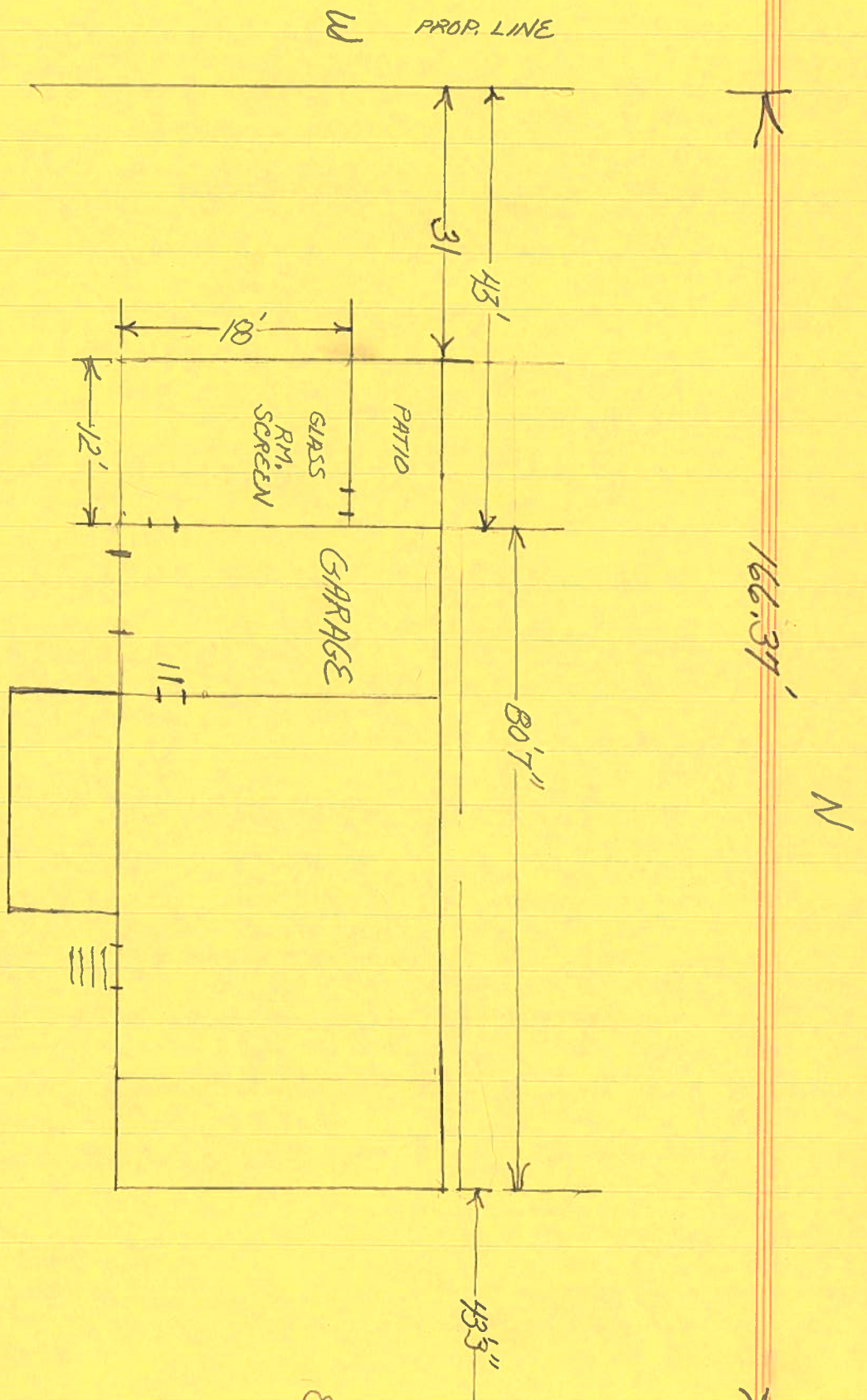
this 15 day of August 2006

Personally known _____ or Produced Identification Yes

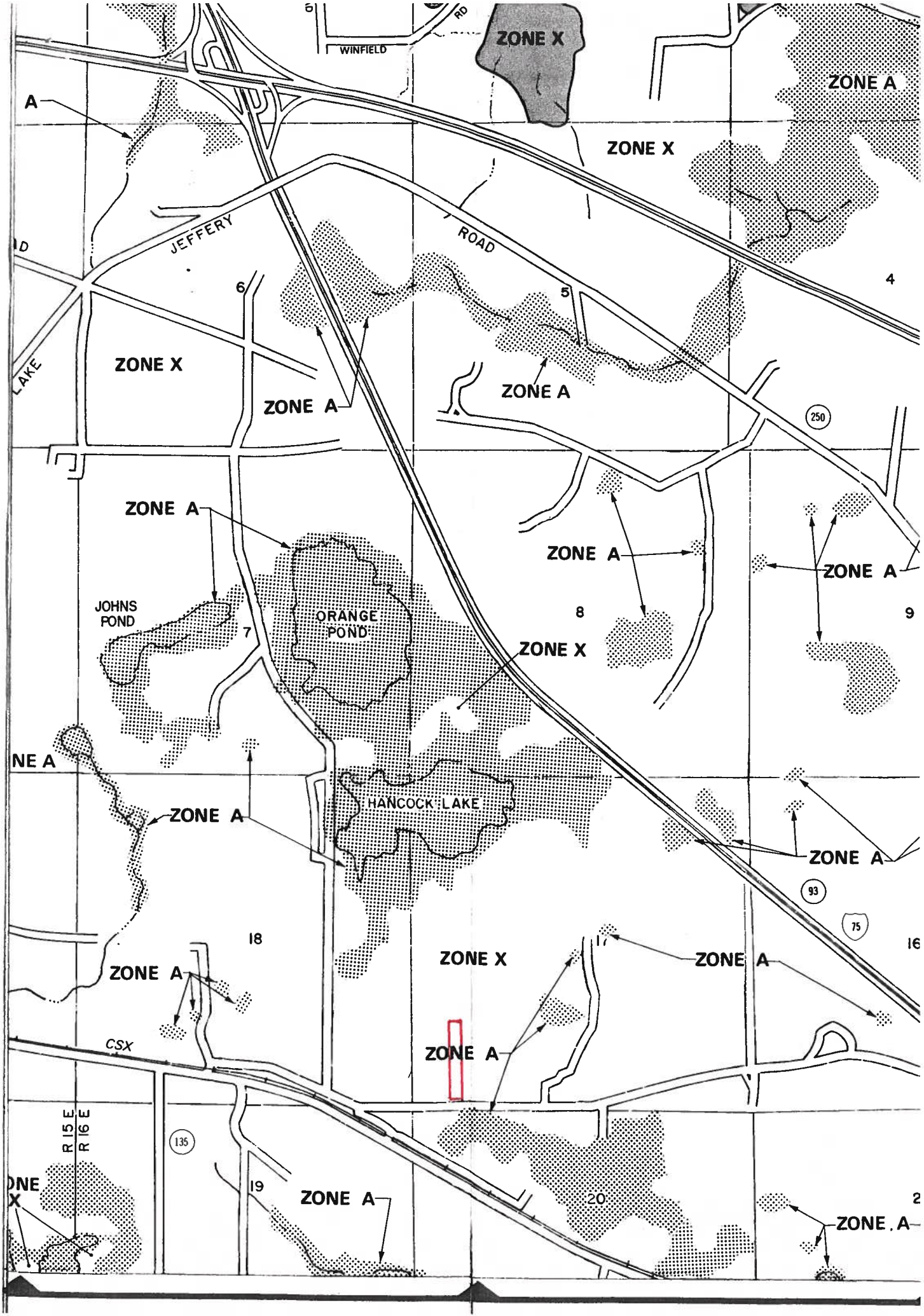
Patricia Patton
Contractor Signature
Contractors License Number CRC058128
Competency Card Number _____
NOTARY STAMP/SEAL

[Signature]
Notary Signature
FRANK D. LEMASTER
MY COMMISSION # DD 463666
EXPIRES: December 19, 2009
Bonded Thru Budget Notary Services
5052

New Site Plan



0608-78



@ CAM112M01 S CamaUSA Appraisal System
 8/22/2006 11:46 Legal Description Maintenance
 Year T Property Sel
 2006 R 17-3S-16-02169-003

Columbia County
 42000 Land 002
 AG 000
 Bldg 000
 Xfea 000
 42000 TOTAL B*

BYRNE ARTHUR F & CORINNE A

1	COMM NW COR OF SW1/4 OF SW1/4,,	RUN E 608.26 FT FOR POB,, CONT	2
3	E 166.39 FT,, S 1307.96 FT TO	N R/W NASH RD,, W ALONG R/W	4
5	166.36 FT,, N 1310.37 FT TO	POB,, ORB 1064-1691	6
7			8
9			10
11			12
13			14
15			16
17			18
19			20
21			22
23			24
25			26
27			28

Mnt 12/20/2005 LARRY

F1=Task F3=Exit F4=Prompt F10=GoTo PgUp/PgDn F24=More

SOUTHERN HOME EXTERIOR DESIGNS, INC.

September 3, 2006

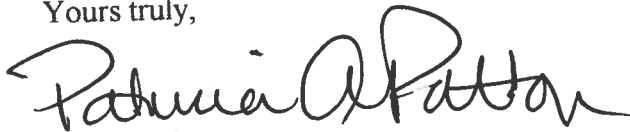
Columbia County Building Department

Re: Southern Home Exterior Designs, Inc.

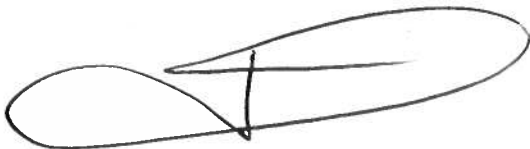
To Whom It May Concern:

I give authorization for Frank D. Lemaster to pull permits for Southern Home Exterior Designs, Inc.

Yours truly,



Patricia A. Patton
President



FRANK D. LEMASTER
MY COMMISSION # DD 483666
EXPIRES: December 19, 2009
Bonded Thru Budget Notary Services

January 01, 2006

LAWRENCE E. BENNETT, P.E.
P.O. BOX 214368
SOUTH DAYTONA, FL 32121
386-767-4774

TO ALL BUILDING DEPARTMENTS

Re: Master File Engineering
"ALUMINUM STRUCTURES DESIGN MANUAL"
2004 edition & 2006 edition

Dear Building Official/Plans Examiner,

This is to certify that the following contractor/company is hereby authorized to use my 2004 ed "ALUMINUM STRUCTURES DESIGN MANUAL" during the year 2006. When we publish and distribute the 2006 ed of the "ALUMINUM STRUCTURES DESIGN MANUAL", they will be authorized to use that manual for the remainder of 2006.

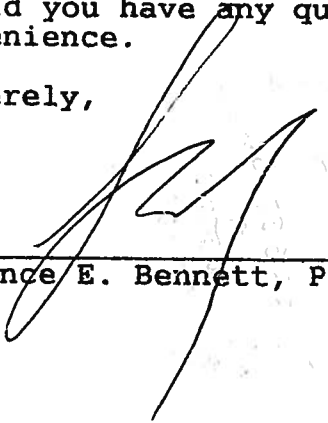
Our authorization is based on a January to January basis regardless of the edition of the manual. This authorization also applies to contractor master file drawings, "ONE PERMIT ONLY" drawings or any "site specific" drawings that I may furnish the contractor.

Patricia Patton
CRC058128
Southern Home Exterior Designs
819 Townsend Blvd #6
Jacksonville, FL 32211

They are hereby added to my 2006 MASTERFILE LIST

Should you have any questions please contact me at your convenience.

Sincerely,



Lawrence E. Bennett, P.E. #16644

Design Check List for Screen / Vinyl Room (page 1 of 2)

1. Design Statement

These plans have been designed in accordance with the Aluminum Structures Design Manual by Lawrence E. Bennett and comply with the Florida Building Code, 2004 Edition, Chapter 20 and ASCE 7-98, Table 6-2; Importance Factor 0.77; Exposure 'B' ☒ or 'C' ☐; 120 MPH or 110 MPH for 3 second wind gust velocity load; Open Design pressures can be found on page 3-ii:

a. 'B' exposure = 11 PSF for Roofs & 11 PSF for Walls

b. 'C' exposure = PSF for Roofs & PSF for Walls

Negative I.P.C. 0.00

For 'C' exposure design loads, multiply 'B' exposure loads by 1.4.

2. Host Structure Adequacy Statement:

I certify that I have inspected the host structure and it is in good repair and attachments made to the structure will be solid.

Frank D. [Signature] Phone: 904-723-2700
Contractor / Authorized Rep** Name (please print)

[Signature] Date: 01/18/08
Contractor / Authorized Rep** Signature

Note: Projection of room from host structure shall not exceed 16'.

3. Building Permit Application Package contains the following:

	Yes	No
A. Project name & address on plans	☒	☐
B. Site plan or survey with enclosure location	☐	☐
C. Contractor's / Designer's name, address, phone number, & signature on plans	☐	☐
D. Notice of consumer rights attached and initialed by consumer	☐	☐
E. Proposed project layout drawing @ 1/8" or 1/10" scale with the following:	☒	☐
1. Plan view with host structure area of attachment, enclosure length, and projection from host structure	☒	☐
2. Front and side elevation views with all dimensions & heights	☒	☐
3. Beam span, spacing, & size (i.e. 2" x 8" x 0.072" x 0.224")	☒	☐
4. Upright height, spacing, & size (i.e. 2" x 8" x 0.072" x 0.224")	☒	☐
5. Chair rail or girts size, length, & spacing (i.e. 2" x 2" x 0.044" x 5'-0" @ 6'-0" O.C.)	☒	☐
6. Knee braces length, location, & size (i.e. 2" x 3" x 0.045" for 2" x 8" x 0.072" x 0.224" s.m.b.)	☐	☒

4. Highlight details from Aluminum Structures Design Manual:

	Yes	No
A. Beam & purlin tables w/ sizes, thickness, spacing, & spans / lengths. Indicate Section 3 tables used:	☒	☐

Beam allowable span conversions from 120 MPH wind zone, 'B' Exposure to MPH wind zone and/or 'C' Exposure for load width :

Look up span on 120 MPH table and apply the following formula:

Span / Height @ 120 MPH Required Span @ MPH

$$\frac{0.00}{(b \text{ or } d)} \times \frac{1.00}{(b \text{ or } d)} \times \frac{1.00}{(b \text{ or } d)} = \underline{\hspace{2cm}}$$

Wind Zone Multiplier * Exposure Multiplier *

B. Upright tables w/ sizes, thickness, spacing, & heights (Tables 3A.2.1, 3A.2.2, or 3A.2.3)	☒	☐
--	-------------------------------------	--------------------------

Upright or wall member allowable height / span conversions from 120 MPH wind zone, 'B' Exposure to MPH wind zone and/or 'C' Exposure for load width :

Look up span on 120 MPH table and apply the following formula:

Span / Height @ 120 MPH Required Span @ MPH

$$\frac{0.00}{(b \text{ or } d)} \times \frac{1.00}{(b \text{ or } d)} \times \frac{1.00}{(b \text{ or } d)} = \underline{\hspace{2cm}}$$

Wind Zone Multiplier * Exposure Multiplier *

* Appropriate multiplier from page 3-ii.

** Must have attended Engineer's Continuing Education Class with in past two years.

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

*****THIS DOCUMENT MUST BE RECORDED AT THE COUNTY
CLERK'S OFFICE BEFORE YOUR FIRST REPLETION*****

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided to this Notice of Commencement:

Tax Parcel ID Number 7-35-16-02167-1003 PERMIT NUMBER 000024947

1. Description of property: (legal description of the property and street address or 911 address)

Legal Description - see attached
3013 NW Nash Rd Lake City, FL 32055

2. General description of improvement: Glass Room

3. Owner Name & Address Arthur Byrne - 3013 NW Nash Rd Lake City
FL 32055 Interest in Property Fee Simple

4. Name & Address of Fee Simple Owner (if other than owner): N/A

5. Contractor Name Fairway Builders Phone Number 723-2700 (904)
Address 819 Townsend Blvd Jacksonville, FL 32211 (Suite 6)

6. Surety Holders Name _____ Phone Number _____
Address _____
Amount of Bond _____ Inst: 2006022108 Date: 09/15/2006 Time: 11:21
DC, P. Dewitt Cason, Columbia County B: 1086 P: 599

7. Lender Name _____
Address _____

8. Persons within the State of Florida designated by the owner upon whom process or other documents may be served as provided by section 713.13 (1)(a) 7, Florida Statutes:

Name _____ Phone Number _____
Address _____

9. In addition to himself/herself the owner designates _____ of _____
to receive a copy of the Lessor's Notice as provided in Section 713.13 (1) -
(a) 7. Phone Number of the designee _____

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording.
(Unless a different date is specified) _____

NOTICE AS PER CHAPTER 713, Florida Statutes:

The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Arthur F. Byrne
Signature of Owner

Sworn to (or affirmed) and subscribed before
day of 4/5, 2006

NOTARY STAMP/SEAL

Frank D. Lemaster
Signature of Notary



FRANK D. LEMASTER
BY COMMISSION # 0046300
EXPIRES - December 19, 2009
Bonded Time Based Money Services

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

~~THIS DOCUMENT MUST BE RECORDED AT THE COUNTY
CLERK'S OFFICE BEFORE YOUR FIRST INSPECTION~~

THE UNDERSIGNED hereby give notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided to this Notice of Commencement.

Tax Parcel ID Number 7-35-16-02167-103 PERMIT NUMBER 000024947

1. Description of property: (legal description of the property and street address or 911 address)

Legal Description - See attached
3013 NW Nash Rd Lake City, FL 32055

2. General description of improvement: Glass Room

3. Owner Name & Address Arthur Byrne - 3013 NW Nash Rd Lake City,
FL 32055 Interest in Property Fee Simple

4. Name & Address of Fee Simple Owner (if other than owner): N/A

5. Contractor Name Patricia Ritten Phone Number 123-2700 (914)
Address 819 Townsend Blvd Jacksonville, FL 32211 (Suite 6)

6. Surety Holders Name _____ Phone Number _____
Address _____
Inst: 2006022108 Date: 09/15/2006 Time: 11:21
Amount of Bond 15 DC, P. DeWitt Cason, Columbia County B: 1086 P: 589

7. Lender Name _____
Address _____

8. Persons within the State of Florida designated by the owner upon whom notices or other documents may be served as provided by section 713.13 (1)(a) 7, Florida Statutes:

Name _____ Phone Number _____
Address _____

9. In addition to himself/herself the owner designates _____ of
_____ to receive a copy of the Lender's Notice as provided in Section 713.13 (1) -
(a) 7. Phone Number of the designee _____

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording.
(Unless a different date is specified) _____

NOTICE AS PER CHAPTER 713, Florida Statutes:

The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Sworn to (or affirmed) and subscribed before
day of 9/15, 2006

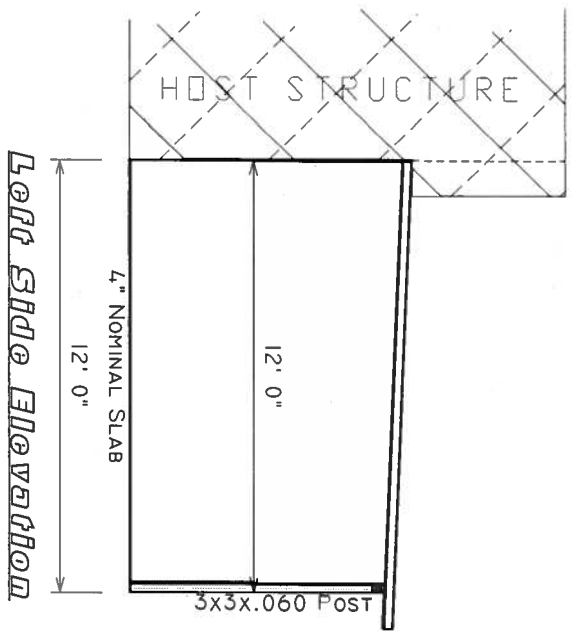
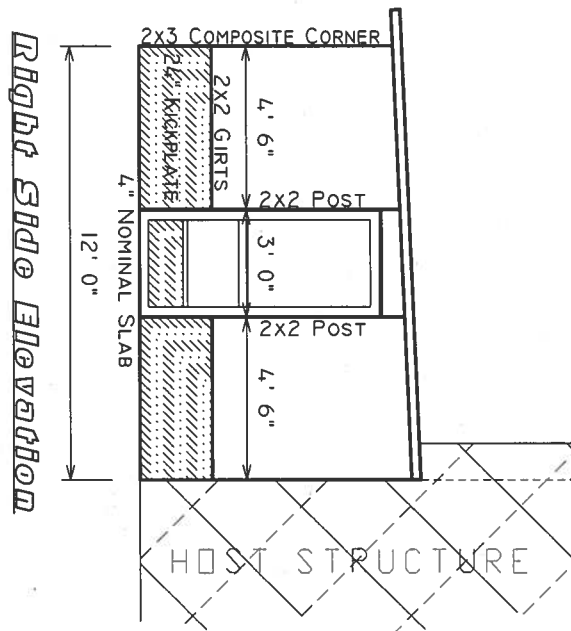
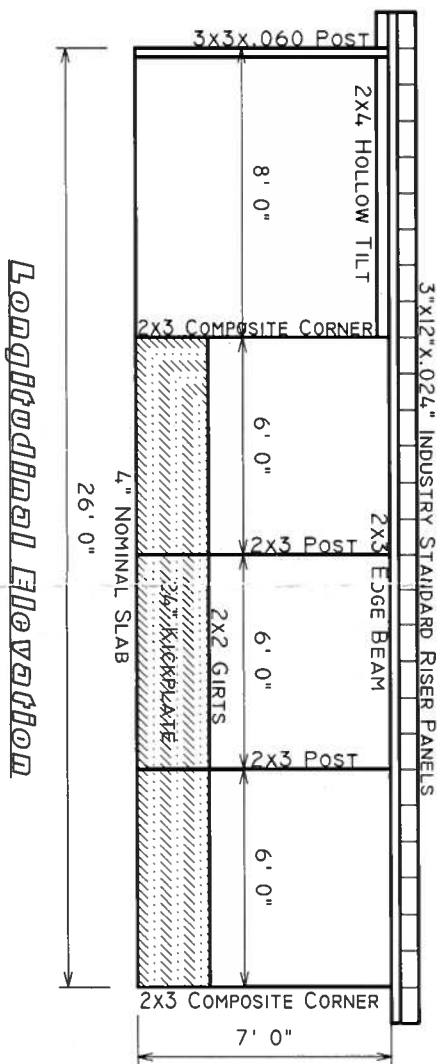
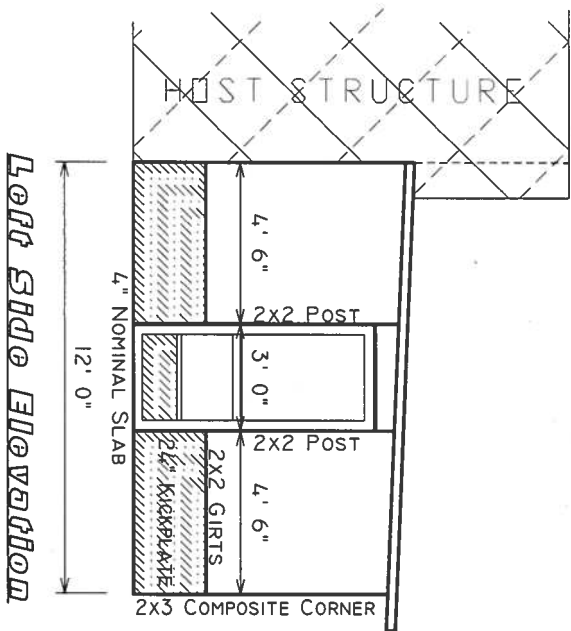
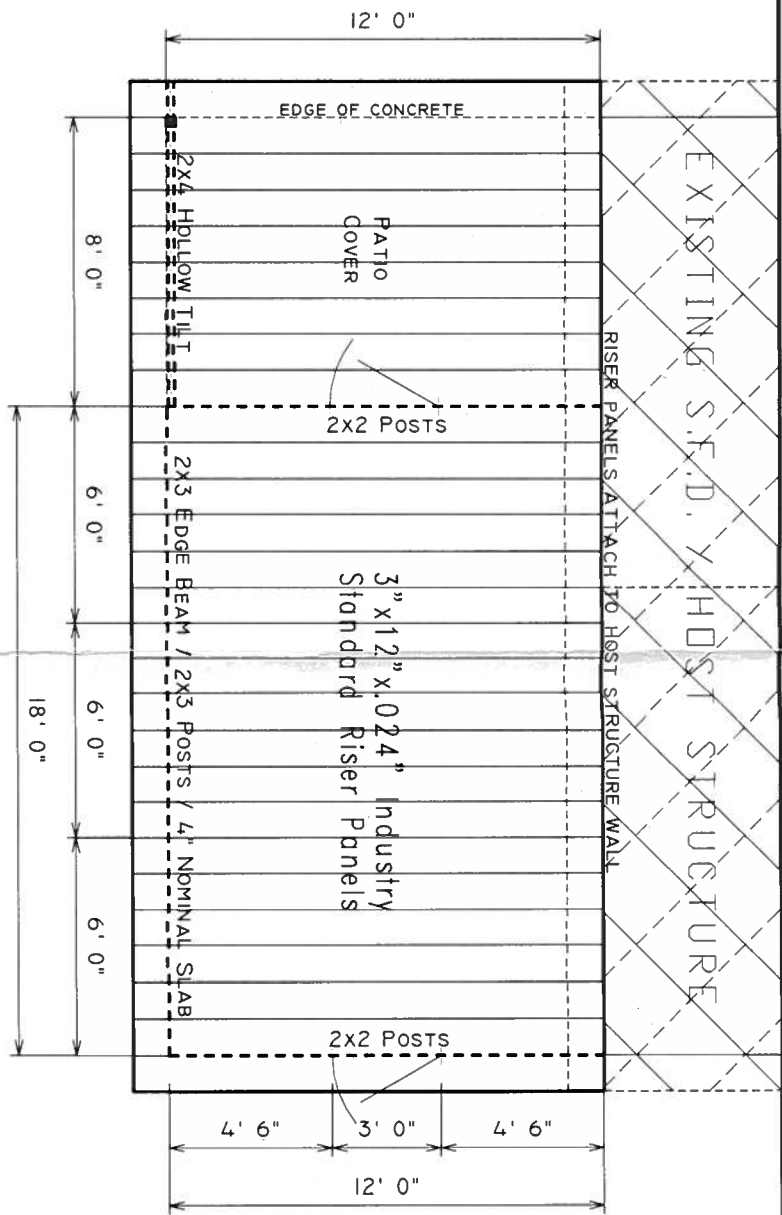
NOTARY STAMP/SEAL

Arthur Byrne
Signature of Owner

Frank D. Lemaster
Signature of Notary



FRANK D. LEMASTER
MY COMMISSION # CD 453888
EXPIRES: December 18, 2008
Send The Notary Today Service



Copyright Notice: Copyright© 2006 - All Rights Reserved by Specialty Structure Design and/ or David W. Miller

LAWRENCE E. BENNETT, P.E.
CIVIL ENGINEER-DEVELOPMENT CONSULTANT
P.O.BOX 4638, SOUTH DAYTONA, FL 32121

Drafting by:
Specialty Structure Design, Inc.
8455 Beverly Lane
St. Augustine, Florida 32092

SOUTHERN HOME EXTERIOR DESIGN
HOME OWNER: BYRNE
SITE ADDRESS: 3013 NW NASH RD.

SHEET #



DESIGN CRITERIA: VELOCITY ZONE: 110 MPH EXPOSURE B
BUILDING CATEGORY: SCREEN STRUCTURE DESIGN PRESSURES (TABLE 2002.4, FBC)
SCREEN WALLS: II PSF ~ ROOF: II PSF / IMPORTANCE FACTOR .77

1. The following structures are designed to be married to block and wood frame structures of adequate structural capacity. The contractor / home owner shall verify that the host structure is in good condition and of sufficient strength to hold the proposed addition.
2. If there is a question about the host structure, the owner (at his own expense) shall hire an architect, engineer, or a certified home inspection company to verify host structure capacity.
3. The structures designed using this section shall be limited to a maximum projection of 16' from the host structure. Freestanding structures shall be limited to the maximum spans and size limits of component parts. Larger than these limits shall have site specific engineering.
4. The following rules apply to attachments involving mobile and manufactured homes:
 - a. Structures to be placed adjacent to a mobile / manufactured home built prior to 1994 shall use "fourth wall construction" or the contractor / home owner shall provide detailed plans of the mobile / manufactured home and inspection report along with addition plans for site specific review and seal by the engineer. This applies to all screen / glass rooms and / or structures to be attached.
 - b. "Fourth wall construction" means the addition shall be free standing with only the roof flashing of the two units being attached. The most common "fourth wall construction" is a post & beam frame adjacent to the mobile / manufactured home. The same span tables can be used as for the front wall beam. For fourth wall beam use the carrier beam table. The post shall be sized according to this manual and/or as a minimum be a 2" x 3" x 0.050" with an 18" x 2" x 0.044" knee brace at each end of the beam.
 - c. For mobile / manufactured homes built after 1994, structures may be attached, provided the project follows the plan provided in this manual. The contractor / owner shall provide verification that the structural system of the host structure is adequate for the addition to be attached.
 - d. If the mobile / manufactured home manufacturer certifies in writing that the mobile home may be attached to, then a "fourth wall" is NOT required.
 5. Glass and modular walls between existing walls, floors, and ceilings shall be allowed and spans shall be figured the same as other glass and modular walls.
 6. When using TEK screws in lieu of S.M.S., longer screws must be used to compensated for drill head.
 7. For high velocity hurricane zones the minimum roof live load / applied load shall be 30 PSF.
 8. The framing of existing screen room walls to receive windows or solid sheathing shall be accomplished with the details of this chapter
 9. Loads, coefficients, spans and heights may be interpolated between values but not extrapolated outside values.

The structures designed for Section 3 are solid roofs with screen or solid walls and are considered part of an enclosed or partially enclosed structural system since they are designed to be married to an existing structure.

The design wind loads used for glass rooms are from ASCE 7-98 Section 6.5, Analytical Procedure and are in compliance with the 2004 Florida Building Code. The loads assume a mean roof height of less than 30'; roof slope of 20° to 30° (+/- 10°), $I = 0.77$. All pressures shown in the table below are in PSF (#/SF). Negative internal pressure coefficient is 0.18 for enclosed and 0.55 for partially enclosed structures.

Anchors for composite panel roof systems were computed on a load width of 10' and 16' projection with a 2' overhang. Any greater load width shall be site specific.

Section 3B Design Loads for Screen, Glass, & Modular Rooms

Note 1: Framing systems of glass rooms are considered to be main frame resistance components. Wind loads are listed as minus loads for roofs and plus loads for walls. To convert above wind loads to "C" Exposure loads multiply by factors listed in Table 2B-C.

Wind Zone Conversions for Glass & Modular Rooms

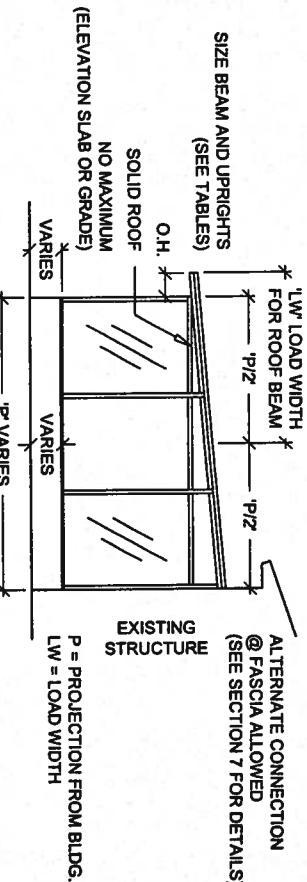
Walls	Bending	Deflection
(b)	(d)	
1.21	1.14	
1.11	1.07	
1.00	1.00	
0.98	0.99	
0.94	0.96	
0.87	0.91	
0.80	0.86	

Wind Zone Conversions for Over Hangs

Mean Host Structure Height		Span Multiplier	
	Load Multiplier	Pans	Composite Panels
0-15'	1.21	0.94	0.91
15-20'	1.29	0.92	0.88
20'-25'	1.34	0.91	0.86
25'-30'	1.40	0.89	0.85

for Glass & Modular Rooms

	Roof	Walls
bending (b)	0.92	0.90
deflection (d)	0.90	0.85



- NOTES:**
1. ANCHOR 1" x 2" OPEN BACK EXTRUSION W/ 1/4" x 2-1/4" CONCRETE FASTENER MAX. OF 2'-0" O.C. AND W/ IN 6" EACH SIDE OF UPRIGHT ANCHOR 1" x 2" TO WOOD WALL W/ #10 x 2-1/2" S.A.S. W/ WASHERS OR #10 x 2-1/2" WASHER HEADED SCREW 2'-0" O.C.. ANCHOR BEAM AND COLUMN INTERNALLY OR W/ ANCHOR CLIPS AND (2) #8 SCREWS W/ WASHERS @ EACH POINT OF CONNECTION.
 2. SELECT FRONT WALL BEAM FROM TABLE USING LARGER LOAD WIDTH VALUE OF P2 OR P2 + 0.1
 3. SELECT SCREEN ROOM FORTH WALL BEAM FROM TABLE 3.1,3 AND GLASS ROOM FOURTH WALL BEAMS FROM TABLE 3B.1,4 USING P2
 4. ANCHORS BASED ON 120 MPH WIND VELOCITY. FOR HIGHER WIND ZONES USE THE FOLLOWING CONVERSION:
- | | | | |
|---------|-----|-----|-----|
| 100-123 | 130 | 140 | 150 |
|---------|-----|-----|-----|

100-123	130	140	150
#8	#10	#12	#12

SCALE: 1/8" = 1'-0"

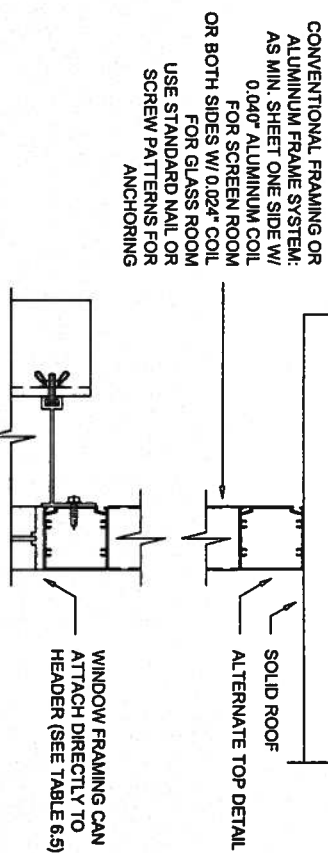
PURSUANT TO PROVISIONS OF THE FLORIDA DEPARTMENT OF
HIGHWAY SAFETY & MOTOR VEHICLES DIVISION OF MOTOR
VEHICLES RULE 15C-2, THE SPAN TABLES, CONNECTION
DETAILS, ANCHORING AND OTHER SPECIFICATIONS ARE
DESIGNED TO BE MARRIED TO CONVENTIONALLY
CONSTRUCTED HOMES AND / OR MANUFACTURED HOMES AND
MOBILE HOMES CONSTRUCTED AFTER 1984.

THE DESIGNS AND SPANS SHOWN ON THESE DRAWINGS ARE
BASED ON THE LOAD REQUIREMENTS FOR THE FLORIDA
BUILDING CODE 2001 EDITION.

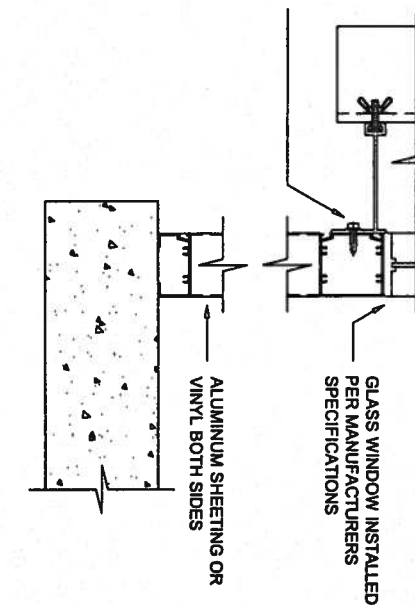
JOB NAME: _____
ADDRESS: _____

DRAWING FOR ONE PERMIT ONLY

2004



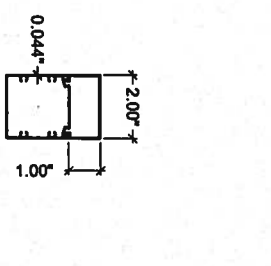
SCALE: 1/4" = 1'-0"



SCALE: 1/4" = 1'-0"

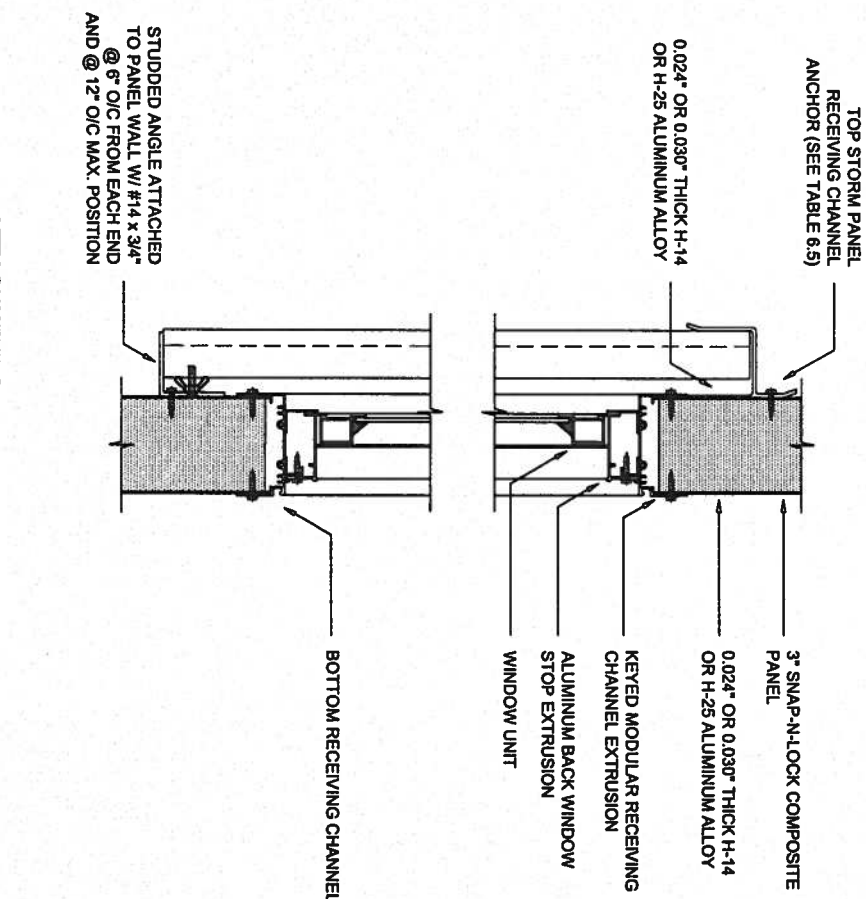
SCALE: 2" = 1'-0"

SCALE: 2" = 1'-0"



1" x 2" x 0.044"
SNAP CAP SECTION
WITH 2" x 2" x 0.044"
PATIO SECTION

SCALE: 2" = 1'-0"



SCALE: 1/4" = 1'-0"

SCALE: 3" = 1'-0"

SCALE: 2" = 1'-0"

Anchor Specs for the Attachment of Storm Panels to Composite Panel Walls
Exposure "B" or "C"

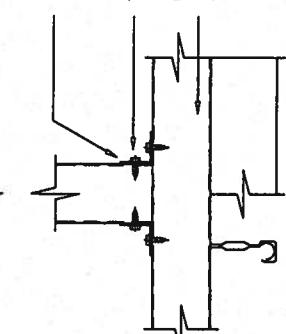
Attachment of Window Header / Studded Angle (Max. Hgt. 5'-1/2" Storm Panels)		
Anchor Size & Spacing		Anchor Size & Spacing
Wind Spd. (MPH)	26 ga. Steel or 0.030" Aluminum	0.024" Aluminum
130	#10 / 6" from ea. end & 12" O.C.	#10 / 4" from ea. end & 12" O.C.
130	#10 / 6" from ea. end & 12" O.C.	#12 / 4" from ea. end & 8" O.C.
140	#10 / 6" from ea. end & 12" O.C.	#12 / 4" from ea. end & 8" O.C.
150	#12 / 4" from ea. end & 8" O.C.	#12 / 4" from ea. end & 8" O.C.

Attachment of Door Header / Studded Angle (Max. Hgt. 7" Storm Panels)		
Anchor Size & Spacing		Anchor Size & Spacing
Wind Spd. (MPH)	26 ga. Steel or 0.030" Aluminum	0.024" Aluminum
130	#10 / 6" from ea. end & 12" O.C.	#10 / 4" from ea. end & 8" O.C.
130	#10 / 6" from ea. end & 12" O.C.	#12 / 4" from ea. end & 8" O.C.
140	#12 / 4" from ea. end & 8" O.C.	#14 / 4" from ea. end & 8" O.C.
150	#14 / 4" from ea. end & 8" O.C.	#14 / 3d" from ea. end & 8" O.C.

Notes:
For anchoring studded angle to concrete use 1/4" drop in machines spaced @ 6" from ea. end and 12" O.C. for all wind zones and exposure categories w/ a min. anchor to edge of concrete distance of 8d.
For anchoring studded angle to wood use 1/4" lag screws @ the same spacing as for 26 ga. steel panels listed above for the appropriate wind zone.
1. The exposure "B" spacing is for mean roof heights 0-20'. For mean roof heights greater than these consult engineer.
2. Space all header & studded angle anchors to fall within the panel face area.

PAN ROOF, COMPOSITE
PANEL OR HOST STRUCTURAL
FRAMING

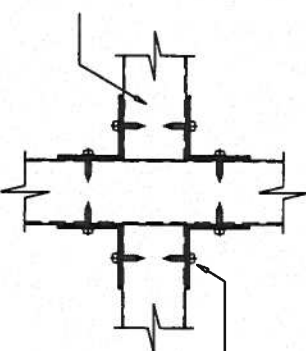
(4) #8 x 1/2" S.M.S. EACH SIDE
OF POST
1 x 2 TOP RAIL FOR SIDE
WALLS ONLY, OR MIN. FRONT
WALL 2 x 2 ATTACHED TO
POST W/ 1" x 1" x 2" ANGLE
CLIPS EACH SIDE OF POST



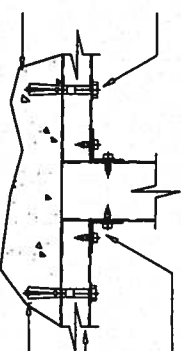
WIND ZONE	MAX. UPRIGHT SPACING
100	7'-0"
110	6'-7"
120	6'-3"
123	6'-1"
130	6'-8"
140	5'-1"
150	4'-11"

1" x 2" TOP RAILS FOR SIDE WALLS
WITH MAX. 3.5' LOAD WIDTH SHALL
HAVE A MAXIMUM UPRIGHT
SPACING AS FOLLOWS

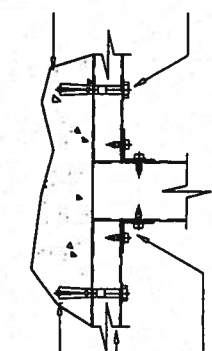
INTERNAL OR EXTERNAL
1" CLIP OR U-CHANNEL CHAIR
RAIL ATTACHED TO POST W/
MIN. (4) #10 S.M.S.



ANCHOR 1 x 2 PLATE TO
CONCRETE WITH 1/4" x 2-1/2"
CONCRETE ANCHORS WITHIN
6" OF EACH SIDE OF EACH
POST AT 24" O.C. MAX. OR
THROUGH ANGLE AT 24" O.C.
MAX.



1 x 2 OR 2 x 2 ATTACHED TO
BOTTOM W/ 1" x 1" x 2" x 1/16"
0.045" ANGLE CLIPS EACH
SIDE AND MIN. (4) #10 x 1/2"
S.M.S.
1" x 2" x 0.032" MIN. OPEN BACK
EXTRUSION
1-1/8" MIN. IN CONCRETE
VAPOR BARRIER UNDER
CONCRETE



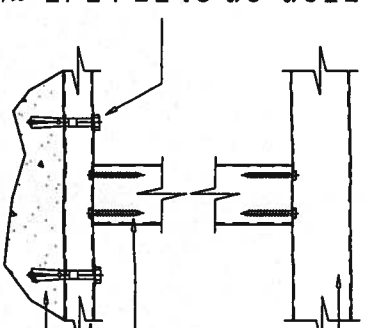
POST TO BASE, GIRT AND POST TO BEAM DETAIL

SCALE: 2" = 1'-0"

ALTERNATE CONNECTION
DETAIL 1" x 2" WITH
(3) #10 x 1-1/2" S.M.S. INTO
SCREW BOSS

(2) #10 x 1 1/2" S. M. S. INTO
SCREW BOSS
ANCHOR 1" x 2" PLATE TO
CONCRETE W/ 1/4" x 2-1/2"
6" OF EACH SIDE OF EACH
POST AND 24" O.C. MAX.

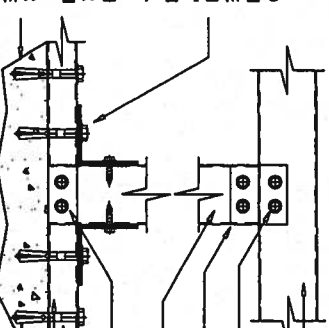
MIN. 3-1/2" SLAB 2500 PSI
CONC. 6 x 6 - 10 x 10 W.W.M.
OR FIBER MESH
VAPOR BARRIER UNDER
CONCRETE



ALTERNATE HOLLOW UPRIGHT TO BASE AND
HOLLOW UPRIGHT TO BEAM DETAIL

SCALE: 2" = 1'-0"

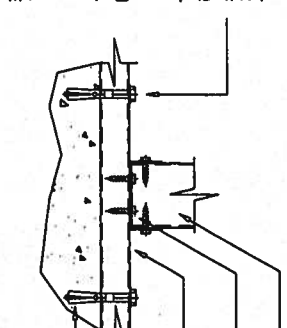
ANCHOR 1" x 2" CHANNEL TO
CONCRETE WITH
1/4" x 2-1/4" CONCRETE
ANCHORS WITHIN 6" OF EACH
SIDE OF EACH POST AT 24"
O.C. MAX. OR THROUGH
ANGLE AT 24" O.C. MAX.
MIN. 3-1/2" SLAB 2500 PSI
CONC. 6x6-10x10 W.W.M. OR
FIBER MESH
VAPOR BARRIER UNDER
CONCRETE



ALTERNATE PATIO SECTION TO UPRIGHT AND
PATIO SECTION TO BEAM DETAIL

SCALE: 2" = 1'-0"

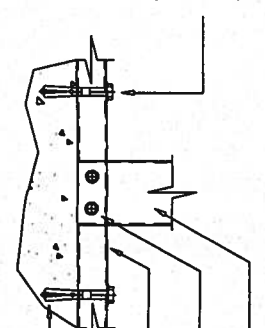
ANCHOR RECEIVING CHANNEL
TO CONCRETE W/ FASTENER
(PER TABLE) WITHIN 6" OF
EACH SIDE OF EACH POST @
24" O.C. MAX.
MIN. 3-1/2" SLAB 2500 PSI
CONC. 6 x 6 - 10 x 10 W.W.M.
OR FIBER MESH
VAPOR BARRIER UNDER
CONCRETE



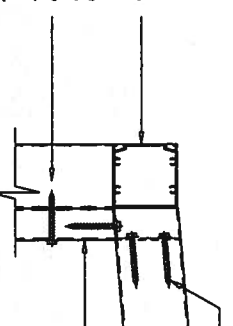
ALTERNATE POST TO BASE CONNECTION - DETAIL 1

SCALE: 2" = 1'-0"

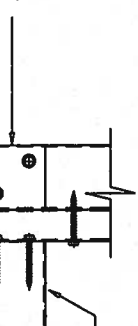
ANCHOR RECEIVING CHANNEL
TO CONCRETE W/ FASTENER
(PER TABLE) WITHIN 6" OF
EACH SIDE OF EACH POST @
24" O.C. MAX.
MIN. 3-1/2" SLAB 2500 PSI
CONC. 6 x 6 - 10 x 10 W.W.M.
OR FIBER MESH
VAPOR BARRIER UNDER
CONCRETE



EDGE BEAM
1" x 2" OPEN BACK ATTACHED
TO FRONT POST W/
#10 x 1-1/2" S.M.S. MAX. 6"
FROM EACH END OF POST
AND 24" O.C.



FRONT WALL GIRT



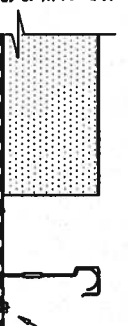
1" x 2" OPEN BACK ATTACHED
TO FRONT POST W/
#10 x 1-1/2" S.M.S. MAX. 6"
FROM EACH END OF POST
AND 24" O.C.



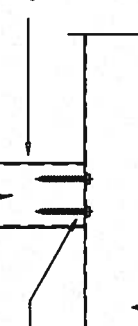
TYPICAL & ALTERNATE CORNER DETAIL

SCALE: 2" = 1'-0"

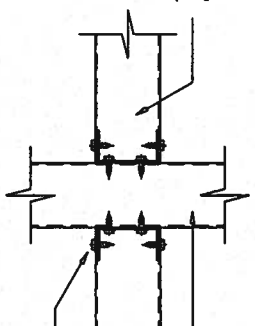
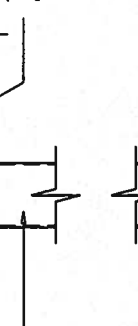
COMPOSITE ROOF PANELS:
(4) 1/4" x 4" LAG BOLTS W/
1-1/4" FENDER WASHERS PER
4'-0" PANEL ACROSS THE
FRONT AND 24" O.C. ALONG
SIDES



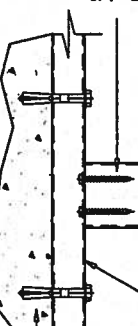
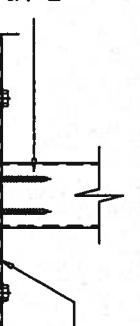
2" x 2" OR 2" x 3" HOLLOW



GIRT AND KICK PLATE 2" x 2"
HOLLOW RAIL



POST ATTACHED TO BOTTOM
W/ MIN. (3) #10 x 1-1/2" S.M.S.
IN SCREW BOSSES



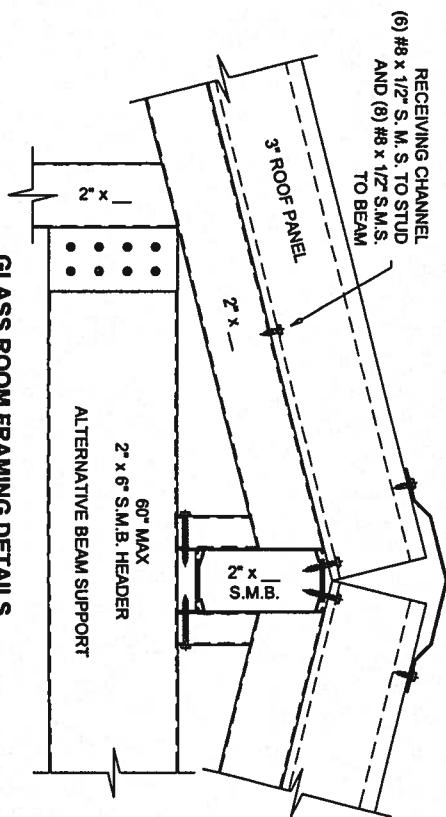
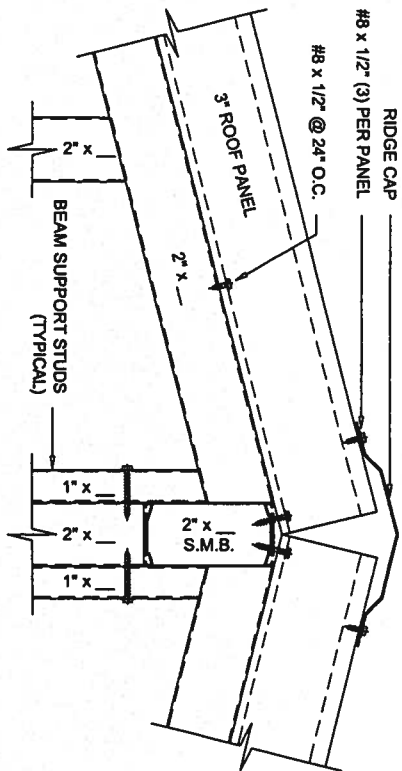
TYPICAL UPRIGHT DETAIL

SCALE: 2" = 1'-0"

07-08-2004

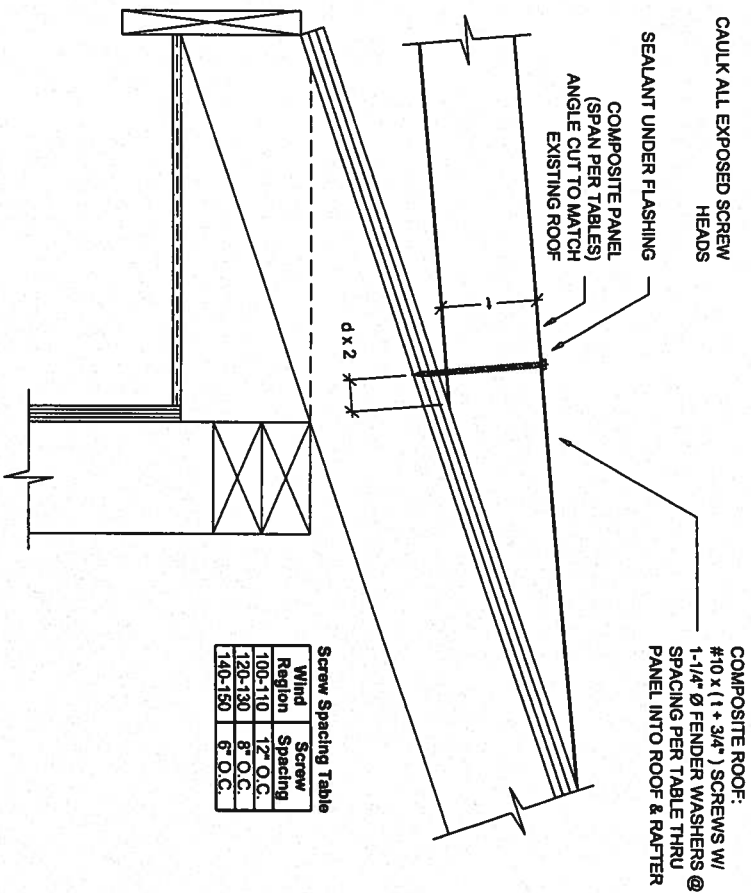
OF

10



GLASS ROOM FRAMING DETAILS

SCALE: 2" = 1'-0"

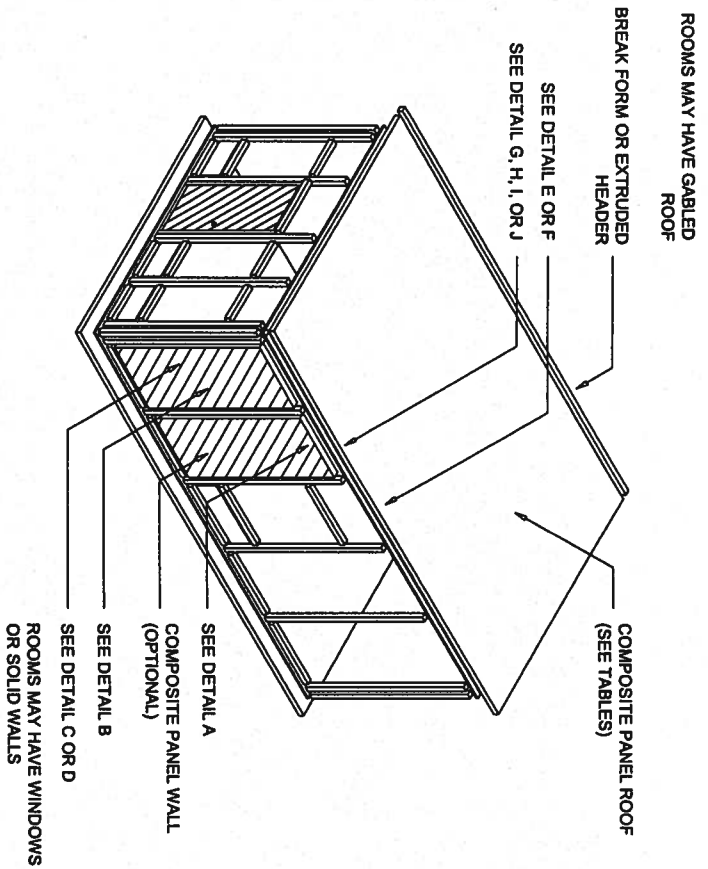


Screw Spacing Table

Wind Region	Screw Spacing
100-110	12" O.C.
120-130	8" O.C.
140-150	6" O.C.

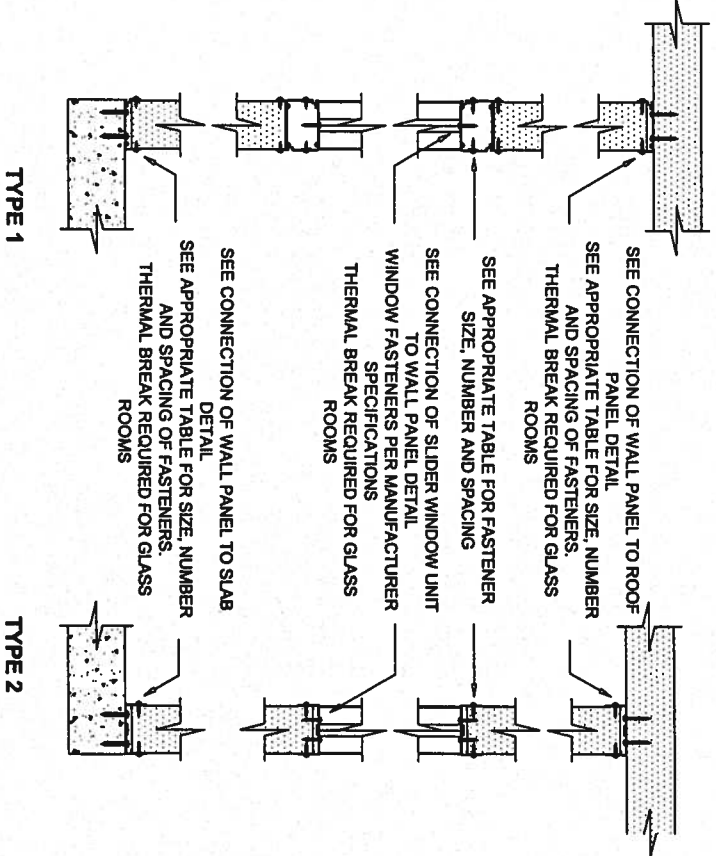
WEDGE ROOF CONNECTION DETAIL

SCALE: 2" = 1'-0"



3" MODULAR ROOM

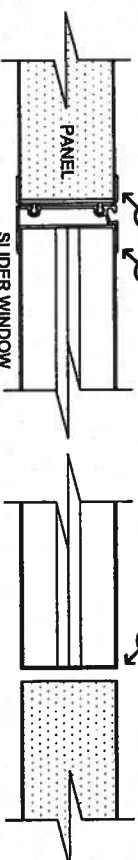
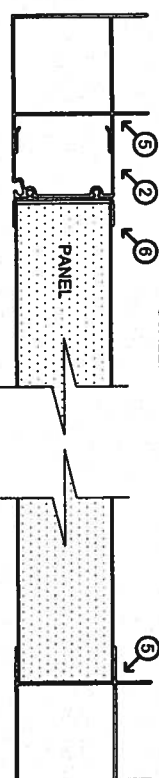
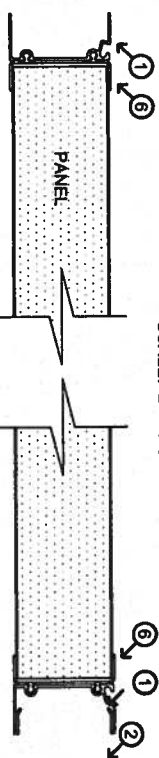
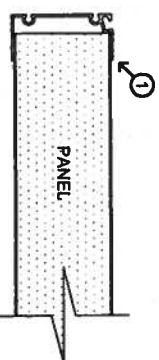
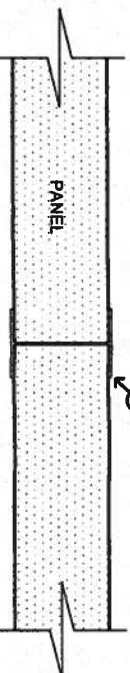
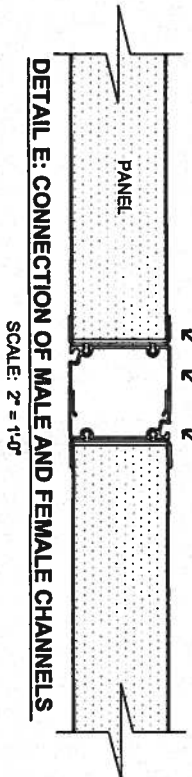
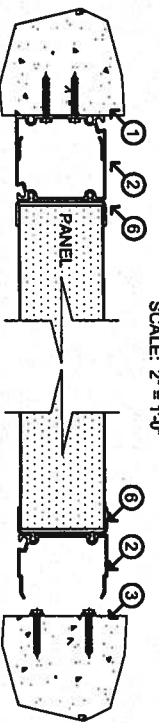
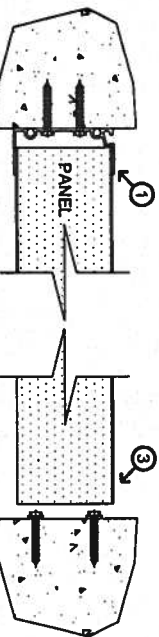
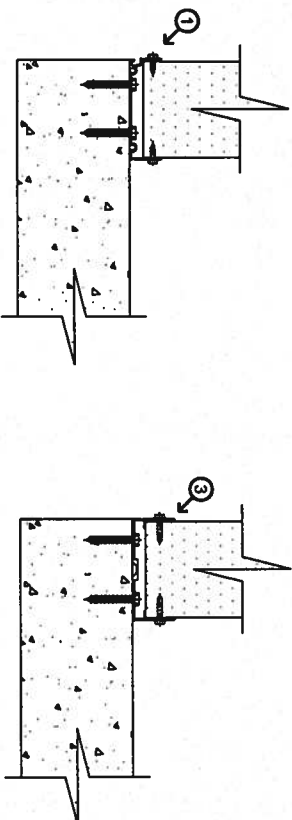
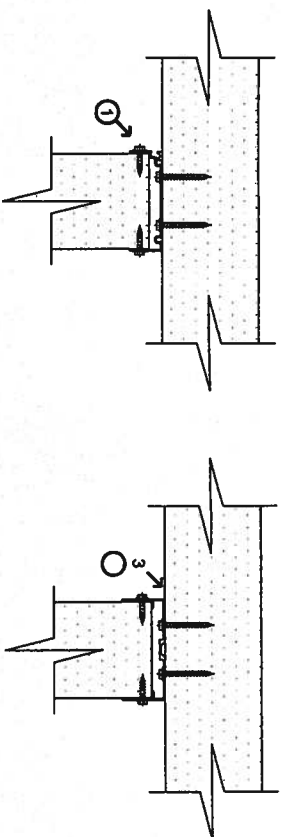
SCALE: N.T.S.



3" MODULAR ROOM WALL SECTION

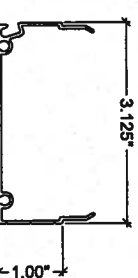
SCALE: 1" = 1'-0"

NOTE: STORM PANEL UNITS MAY BE ATTACHED TO ALUMINUM FRAME MEMBERS OR DIRECTLY TO COMPOSITE PANEL WALL MEMBERS



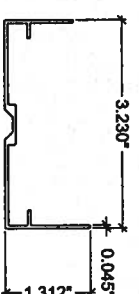
① 3.00" FEMALE CHANNEL

SCALE: 4" = 1'-0"



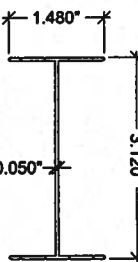
② 3.00" MALE CHANNEL

SCALE: 4" = 1'-0"



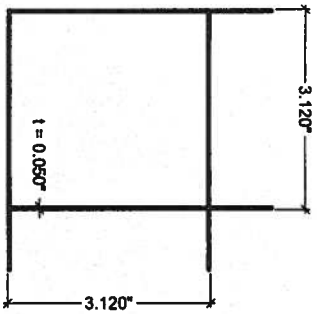
③ TOP AND BOTTOM CHANNEL

SCALE: 4" = 1'-0"



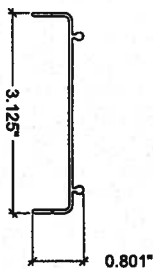
④ 3.00" 'H' CHANNEL

SCALE: 4" = 1'-0"



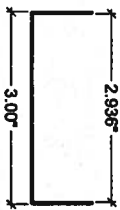
PROPERTIES:
 $A = 0.564 \text{ in.}^2$
 $I_x - y = 0.8756 \text{ in.}^4$
 $S_x - y = 0.703 \text{ in.}^3$
 $r_y - y = 1.246 \text{ in.}$

⑤ CORNER POST
 SCALE: 4" = 1'-0"



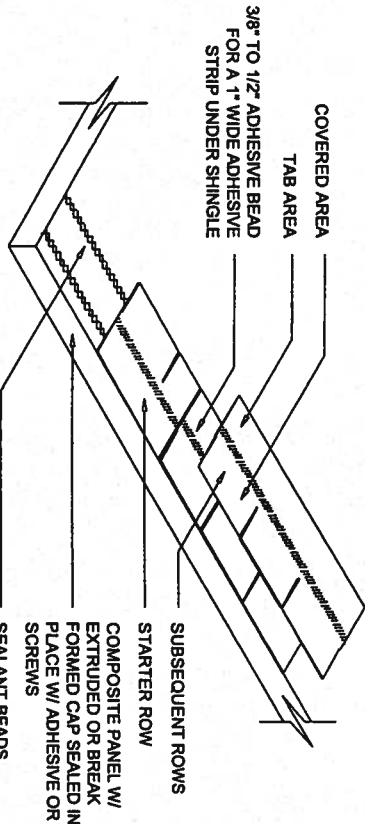
PROPERTIES:
 $A = 0.164 \text{ in.}^2$
 $I_x - y = 0.102 \text{ in.}^4$
 $S_x - y = 0.129 \text{ in.}^3$
 $r_y - y = 1.493 \text{ in.}$

⑥ 3.00" PANEL ADAPTER
 SCALE: 4" = 1'-0"



PROPERTIES:
 $A = 0.138 \text{ in.}^2$
 $I_x - y = 0.165 \text{ in.}^4$
 $S_x - y = 0.110 \text{ in.}^3$
 $r_y - y = 1.092 \text{ in.}$

⑦ 3.00" INSERT CHANNEL (D.R.C.)
 SCALE: 4" = 1'-0"



COMPOSITE ROOF PANEL WITH ALTERNATE SHINGLE FINISH DETAIL
 SCALE: N.T.S.

ATTACH SHINGLES TO COMPOSITE ROOF PANELS WITH INDUSTRIAL ADHESIVE*.
 APPLY ADHESIVE IN A CONTINUOUS BEAD 3/8" TO 1/2" DIAMETER SO THAT THERE IS A 1" WIDE STRIP OF ADHESIVE WHEN THE SHINGLE IS PUT IN PLACE.

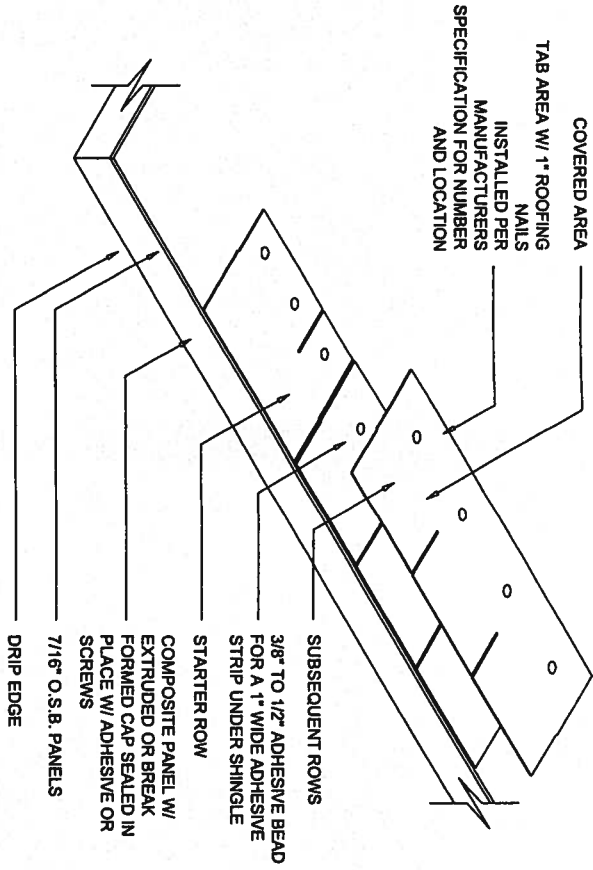
FOR AREAS UP TO 120 M.P.H. WIND ZONE:

- 1) STARTER ROWS OF SHINGLES SHALL HAVE ONE STRIP OF ADHESIVE UNDER THE SHINGLE AT MID COVERED AREA AND ONE UNDER THE SHINGLE AT MID TAB AREA. STARTER SHINGLE ROW INSTALLED WITH THE TABS FACING IN THE UPWARD DIRECTION OF THE ROOF SLOPE.
- 2) SUBSEQUENT ROWS OF SHINGLES INSTALLED WITH THE TABS FACING IN THE DOWNWARD DIRECTION OF THE ROOF SLOPE WITH ONE STRIP OF ADHESIVE UNDER THE SHINGLE AT MID COVERED AREA.

FOR AREAS ABOVE 120 M.P.H. WIND ZONE:

- 1) STARTER ROWS OF SHINGLES SHALL HAVE TWO STRIPS OF ADHESIVE UNDER THE SHINGLE AT MID COVERED AREA AND TWO STRIPS AT MID TAB AREA. SHINGLE ROW INSTALLED WITH THE TABS FACING IN THE UPWARD DIRECTION OF THE ROOF SLOPE.
- 2) SUBSEQUENT ROWS OF SHINGLES INSTALLED PER PREVIOUS SPECIFICATION WITH TWO STRIPS OF ADHESIVE AT MID COVERED AREA.

* ADHESIVE: CHEM REX - PL. PREMIUM 948 URETHANE ADHESIVE OR OSI - RF-140
 MINIMUM ROOF SLOPE: 2" IN 12"

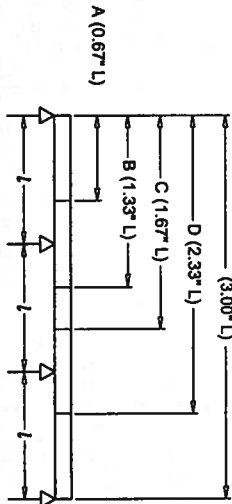


COMPOSITE ROOF PANEL WITH ALTERNATE O.S.B. AND STANDARD SHINGLE FINISH DETAIL
 SCALE: N.T.S.

SPECIFICATIONS FOR APPLYING O.S.B. AND SHINGLES FOR ROOF SLOPES OF 2:12 TO 4:12

NOTES:

1. INSTALL PRO-FAB PANELS IN ACCORDANCE WITH MANUFACTURERS INSTRUCTIONS.
2. SEAL ALL SEAMS WITH PRO 2000 CHEMREX 948 URETHANE AND CLEAN THE ROOF OF ANY DIRT, GREASE, WATER OR OIL.
3. APPLY 12 - 16 MILS OF CONTACT CEMENT TO THE PANELS AND INSTALL 7/16" O.S.B. OVER THE GLUE AND PANELS. ALTERNATE FASTENING: USE #8 X 1-1/4" GALVANIZED DECK SCREWS @ 8" O.C. EDGES & 16" O.C. FIELD.
4. INSTALL 15# FELT PAPER IN ACCORDANCE WITH STANDARD BUILDING CODE SECTION 1507.38.1.
5. INSTALL SHINGLES IN ACCORDANCE WITH STANDARD BUILDING CODE SECTION 1507.3.7. THROUGH 1507.3.7.



ALLOWABLE BEAM SPLICE LOCATIONS
 SCALE: 1/8" = 1'-0"

SINGLE SPAN BEAM SPLICE @ 1/4 POINT OF BEAM SPAN ALL SPLICES SHALL BE STAGGERED ON EACH SIDE OF SELF MATING BEAM

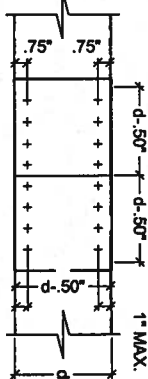
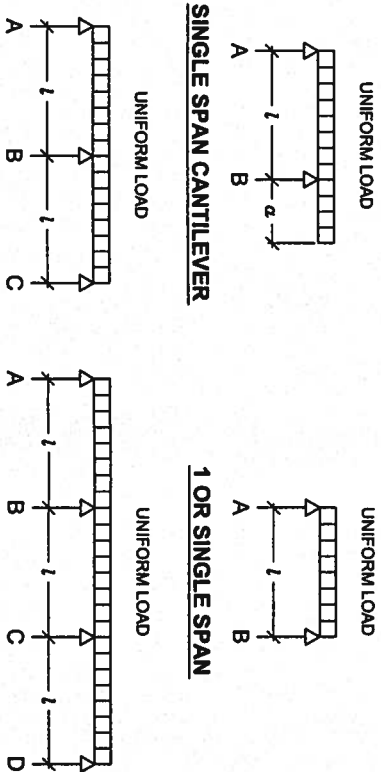


PLATE TO BE SAME THICKNESS AS BEAM WEB PLATE CAN BE INSIDE OR OUTSIDE BEAM OR LAP CUT DENOTES SCREW PATTERN NOT NUMBER OF SCREWS

Screw Size	d _s (in.)	Edge To Center 2d _s (in.)	Center To Center 2-(1/2)d _s (in.)	Gusset Plate Thickness	
				Beam Size	Thickness
#8	0.16	3/8	7/16	2" x 7" x 0.055" x 0.120"	1/16" = 0.063"
#10	0.19	3/8	1/2	2" x 8" x 0.072" x 0.224"	1/8" = 0.125"
#12	0.21	7/16	9/16	2" x 8" x 0.072" x 0.224"	1/8" = 0.125"
#14 or 1/4"	0.25	1/2	5/8	2" x 8" x 0.082" x 0.308"	1/8" = 0.125"
5/16"	0.313	5/8	3/4	2" x 10" x 0.082" x 0.368"	1/4" = 0.25"

* Refers to each side of splice.
 ** Use for 2" x 4" and 2" x 6" also.
 Note:
 1. All gusset plates shall be a minimum 5052 H-32 Alloy or have a minimum yield of 23 ksi.

TYPICAL BEAM SPLICE DETAIL
 SCALE: 1" = 1'-0"



SINGLE SPAN CANTILEVER

1 OR SINGLE SPAN

2 SPAN

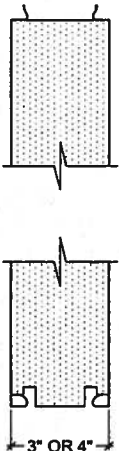
3 SPAN

4 SPAN

NOTES:

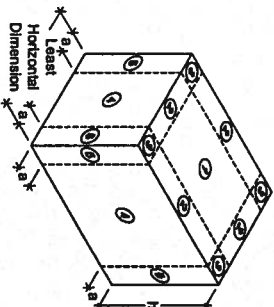
- 1) L = Span Length
 a = Overhang Length
- 2) All spans listed in the tables are for equally spaced distances between supports or anchor points.
- 3) Hollow extrusions shall not be spliced.
- 4) Single span beams shall only be spliced at the quarter points and splices shall be staggered.

SPAN EXAMPLES FOR SECTION 3 TABLES
 SCALE: N.T.S.



COMPOSITE ROOF OR WALL PANEL

SCALE: 2" = 1'-0"



WINDOW & DOOR LOADINGS

NOTES:
1. FOR PARTIALLY ENCLOSED DESIGN LOADS MULTIPLY PRESSURES LISTED BELOW BY 1.31.
2. FOR "C" EXPOSURE MULTIPLY PRESSURES LISTED BELOW BY LOAD MULTIPLIERS LISTED IN TABLE 3B-C.

6-3C Enclosed Structures
ASCE 7-98 Section 9.5 Method 2 - Analytical Procedure Results

Table 0 to 10°																		
Location		Design Pressure (psf)																
Zone	Effective Wind Area	Basic Wind Speed V (mph)																
(SF)	80	100	110	120	130	140	150	160	170	180	190	200	210	220	230	240		
Roof	1	10	6	-15	7	-18	8	-22	11	-26	12	-30	14	-35	16	-40	19	-44
	20	5	-14	7	-17	8	-21	9	-24	11	-26	12	-33	14	-38	16	-43	
	80	5	-14	6	-17	7	-20	9	-24	11	-29	12	-33	14	-38	16	-43	
	10	6	-24	7	-30	9	-36	11	-43	12	-51	14	-59	16	-68	19	-77	
	20	5	-22	7	-27	8	-32	9	-39	11	-45	13	-55	15	-60	17	-69	
3	20	5	-18	6	-23	7	-27	9	-32	10	-38	12	-44	14	-51	16	-58	
	10	6	-37	7	-45	9	-55	11	-65	12	-72	14	-89	16	-102	19	-118	
	5	5	-31	7	-38	8	-46	9	-54	11	-64	13	-74	15	-85	17	-97	
	5	5	-22	6	-27	7	-33	8	-39	10	-46	12	-53	14	-61	16	-69	
	80	100	110	120	130	140	150	160										
Roof	1	10	6	-13	10	-16	13	-20	15	-24	18	-28	20	-32	23	-37	27	-42
	20	8	-13	10	-16	13	-20	15	-24	18	-28	20	-32	23	-35	25	-40	
	50	7	-12	8	-15	10	-18	12	-22	14	-26	18	-30	18	-34	21	-39	
	10	8	-28	10	-35	13	-42	15	-50	16	-59	20	-68	23	-78	27	-89	
	20	8	-26	10	-32	12	-38	14	-46	16	-54	19	-62	22	-71	25	-81	
3	20	7	-22	8	-27	10	-33	12	-38	14	-46	16	-53	18	-61	21	-69	
	10	6	-28	10	-35	13	-42	15	-50	18	-59	20	-68	23	-78	27	-89	
	20	8	-28	10	-32	12	-38	14	-46	16	-54	19	-62	22	-71	25	-81	
	5	7	-22	8	-27	10	-33	12	-39	14	-46	16	-53	18	-61	21	-69	
	80	100	110	120	130	140	150	160										
h < 60'	10	15	-16	18	-18	22	-24	26	-26	30	-33	35	-38	40	-44	46	-50	54
	20	14	-15	17	-18	21	-23	25	-27	29	-32	34	-37	39	-42	44	-48	51
	50	13	-14	16	-18	20	-21	23	-25	27	-30	32	-35	38	-40	41	-45	48
	100	12	-14	15	-17	18	-20	22	-24	26	-28	30	-33	34	-38	39	-43	45
	20	15	-19	16	-24	22	-29	28	-35	30	-41	35	-47	40	-54	46	-66	56
Wall	10	14	-16	17	-23	21	-27	25	-32	29	-34	34	-44	39	-51	44	-58	52
	20	15	-18	16	-20	23	-29	27	-34	32	-40	36	-42	41	-48	41	-52	48
	50	13	-16	16	-20	23	-29	27	-34	32	-40	36	-42	41	-48	41	-52	48
	100	12	-15	16	-18	23	-22	27	-26	32	-30	37	-34	42	-39	48	-43	58
	20	15	-18	16	-20	23	-29	27	-34	32	-40	36	-42	41	-48	41	-52	48

**Table 3B.1.1-110 Allowable Edge Beam Spans - Hollow Extrusions
Gases & Modular Rooms**
For 3 second wind gust at 110 MPH velocity, using design load of 23 #/SF (36 #/SF for Max. Cantilever,
Aluminum Alloy 6063 T-6

Aluminum Alloy 6063 T-6

$2^2 \times 2^2 \times 0.044^*$					$2^2 \times 2^2 \times 0.055^*$				
Load Width (ft.)	Max. Span L_v / (bending v. or deflection δ_f)	3 Span	4 Span	Max. Cantilever	Load Width (ft.)	Max. Span L_v / (bending v. or deflection δ_f)	3 Span	4 Span	Max. Cantilever
5	4'-2" d	5'-1" b	4'-10" b	1'-1" d	5	4'-5" d	5'-6" b	5'-4" b	1'-2" d
6	3'-11" d	4'-7" b	4'-5" b	1'-0" d	6	4'-2" d	5'-0" b	4'-10" b	1'-1" d
7	3'-9" d	4'-5" b	4'-1" b	0'-11" d	7	3'-11" d	4'-8" b	4'-5" b	1'-0" d
8	3'-7" d	3'-11" b	3'-10" b	0'-11" d	8	3'-9" d	4'-4" b	4'-2" b	0'-11" d
9	3'-4" b	3'-9" b	3'-6" b	0'-10" d	9	3'-8" d	4'-1" b	3'-11" b	0'-11" d
10	3'-2" b	3'-7" b	3'-5" b	0'-10" d	10	3'-6" b	3'-11" b	3'-9" b	0'-11" d
11	3'-0" b	3'-5" b	3'-3" b	0'-10" d	11	3'-4" b	3'-9" b	3'-7" b	0'-10" d
12	2'-11" b	3'-3" b	3'-2" b	0'-10" d	12	3'-2" b	3'-7" b	3'-5" b	0'-10" d
$2^2 \times 2^2 \times 0.045^*$					$2^2 \times 2^2 \times 0.070^*$				
Load Width (ft.)	Max. Span L_v / (bending v. or deflection δ_f)	3 Span	4 Span	Max. Cantilever	Load Width (ft.)	Max. Span L_v / (bending v. or deflection δ_f)	3 Span	4 Span	Max. Cantilever
5	4'-5" d	5'-10" d	5'-10" b	1'-2" d	5	5'-4" d	6'-7" d	6'-8" d	1'-4" d
6	4'-5" d	5'-6" d	5'-4" b	1'-2" d	6	4'-11" d	6'-2" d	6'-3" d	1'-3" d
7	4'-3" d	5'-1" b	4'-11" b	1'-1" d	7	4'-9" d	5'-10" d	5'-11" b	1'-2" d
8	4'-0" d	4'-9" b	4'-7" b	1'-0" d	8	4'-6" d	5'-7" d	5'-6" b	1'-3" d
9	3'-11" d	4'-6" b	4'-4" b	0'-11" d	9	4'-4" d	5'-4" b	5'-2" b	1'-1" d
10	3'-9" d	4'-3" b	4'-2" b	0'-11" d	10	4'-3" d	5'-1" b	4'-11" b	1'-1" d
11	3'-8" d	4'-1" b	3'-11" b	0'-11" d	11	4'-1" d	4'-10" b	4'-8" b	1'-0" d
12	3'-6" b	3'-1" b	3'-9" b	0'-11" d	12	3'-11" d	4'-6" b	4'-6" b	1'-0" d
$2^2 \times 3 \times 0.045^*$					$2^2 \times 4 \times 0.050^*$				
Load Width (ft.)	Max. Span L_v / (bending v. or deflection δ_f)	3 Span	4 Span	Max. Cantilever	Load Width (ft.)	Max. Span L_v / (bending v. or deflection δ_f)	3 Span	4 Span	Max. Cantilever
5	5'-10" d	6'-10" b	6'-7" b	1'-6" d	5	7'-7" d	8'-8" b	8'-5" b	1'-11" d
6	5'-6" d	6'-3" b	6'-0" b	1'-5" d	6	7'-4" b	7'-11" b	7'-8" b	1'-10" d
7	5'-2" b	5'-9" b	5'-7" b	1'-4" d	7	6'-7" b	7'-4" b	7'-1" b	1'-8" d
8	4'-10" b	5'-5" b	5'-2" b	1'-3" d	8	6'-2" b	6'-11" b	6'-8" b	1'-6" d
9	4'-7" b	5'-1" b	4'-11" b	1'-3" d	9	5'-10" b	6'-6" b	6'-3" b	1'-7" d
10	4'-4" b	4'-10" b	4'-8" b	1'-2" d	10	5'-6" d	6'-2" b	5'-11" b	1'-6" d
11	4'-1" b	4'-7" b	4'-5" b	1'-2" d	11	5'-3" b	6'-10" b	5'-8" b	1'-5" d
12	3'-11" b	4'-5" b	4'-3" b	1'-1" d	12	5'-0" b	5'-7" b	5'-5" b	1'-5" d

Notes:

1. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.

2. Spans may be interpolated

Table 3B.1.1-120 Allowable Edge Beam Spans - Hollow Extrusions Glass & Modular Rooms

©

For 3 second wind gust at 120 MPH velocity, using design load of 27 #/SF (43 #/SF for Max. Cantilever Aluminum Alloy 6063 T-6

2 ³ x 2 ² x 0.045 ³											
Load	Max. Span	L / (bending τ or deflection δ_T)				Load	Max. Span	L / (bending τ or deflection δ_T)			
Width (ft.)	1 & 2 Span	3 Span	4 Span	Max. Cantilever	Width (ft.)	1 & 2 Span	3 Span	4 Span	Max. Cantilever		
5	3'-11" d	4'-6" b	4'-6" b	1'-0" d	6	4'-2" d	5'-1" b	4'-11" b	1'-1" d		
6	3'-9" d	4'-3" b	4'-1" b	0'-11" d	5	3'-11" d	4'-6" b	4'-6" b	1'-0" d		
7	3'-6" d	3'-11" b	3'-10" b	0'-11" d	7	3'-9" d	4'-4" b	4'-2" b	0'-11" d		
8	3'-3" b	3'-6" b	3'-7" b	0'-10" d	8	3'-7" d	4'-0" b	3'-11" b	0'-10" d		
8	3'-1" b	3'-6" b	3'-4" d	0'-10" d	9	3'-5" d	3'-8" b	3'-6" b	0'-10" d		
10	2'-11" b	3'-2" b	3'-2" d	0'-10" d	10	3'-3" d	3'-7" b	3'-6" b	0'-10" d		
11	2'-10" b	3'-2" b	3'-0" d	0'-9" d	11	3'-1" b	3'-5" b	3'-4" b	0'-10" d		
12	2'-6" b	3'-0" b	2'-11" b	0'-9" d	12	2'-11" b	3'-3" b	3'-2" b	0'-10" d		
3 ³ x 2 ² x 0.045 ³											
Load	Max. Span	L / (bending τ or deflection δ_T)				Load	Max. Span	L / (bending τ or deflection δ_T)			
Width (ft.)	1 & 2 Span	3 Span	4 Span	Max. Cantilever	Width (ft.)	1 & 2 Span	3 Span	4 Span	Max. Cantilever		
5	4'-6" d	5'-6" b	5'-5" b	1'-2" d	5	5'-0" d	6'-3" b	6'-4" d	1'-3" d		
6	4'-3" d	5'-1" b	4'-11" b	1'-1" d	6	4'-9" d	5'-10" d	5'-10" b	1'-2" d		
7	3'-11" d	4'-9" b	4'-7" b	1'-0" d	7	4'-6" d	5'-7" d	5'-5" b	1'-2" d		
8	3'-10" d	4'-5" b	4'-3" b	0'-11" d	8	4'-4" d	5'-3" b	5'-1" b	1'-1" d		
9	3'-6" d	4'-2" b	4'-0" b	0'-11" d	9	4'-2" d	4'-11" b	4'-8" b	1'-1" d		
10	3'-6" b	3'-11" b	3'-10" b	0'-11" d	10	3'-11" d	4'-8" b	4'-6" b	1'-0" d		
11	3'-4" b	3'-9" b	3'-8" b	0'-10" d	11	3'-10" d	4'-6" b	4'-4" b	0'-11" d		
12	3'-3" b	3'-7" b	3'-6" b	0'-10" d	12	3'-9" d	4'-3" b	4'-2" b	0'-11" d		
2 ³ x 3 x 0.045 ³											
Load	Max. Span	L / (bending τ or deflection δ_T)				Load	Max. Span	L / (bending τ or deflection δ_T)			
Width (ft.)	1 & 2 Span	3 Span	4 Span	Max. Cantilever	Width (ft.)	1 & 2 Span	3 Span	4 Span	Max. Cantilever		
5	5'-7" d	6'-3" b	6'-1" b	1'-5" d	5	7'-2" b	8'-0" b	7'-9" b	1'-10" d		
6	5'-2" b	5'-9" b	5'-7" b	1'-4" d	6	6'-7" b	7'-4" b	7'-1" b	1'-9" d		
7	4'-8" b	5'-4" b	5'-2" b	1'-3" d	7	6'-1" b	6'-9" b	6'-7" b	1'-8" d		
8	4'-5" b	4'-11" b	4'-10" b	1'-2" d	8	5'-8" b	6'-4" b	6'-2" b	1'-7" d		
8	4'-2" b	4'-8" b	4'-6" b	1'-2" d	9	5'-4" b	6'-1" b	5'-9" b	1'-6" d		
10	3'-11" b	4'-5" b	4'-4" b	1'-1" d	10	5'-1" b	5'-8" b	5'-6" b	1'-5" d		
11	3'-10" b	4'-4" b	4'-1" b	1'-1" d	11	4'-10" b	5'-5" b	5'-3" b	1'-5" d		
12	3'-6" b	4'-1" b	3'-11" d	1'-1" d	12	4'-5" b	5'-0" b	5'-0" b	1'-4" d		
2 ³ x 2 ² x 0.055 ³											
Load	Max. Span	L / (bending τ or deflection δ_T)				Load	Max. Span	L / (bending τ or deflection δ_T)			
Width (ft.)	1 & 2 Span	3 Span	4 Span	Max. Cantilever	Width (ft.)	1 & 2 Span	3 Span	4 Span	Max. Cantilever		
5	5'-7" d	6'-3" b	6'-1" b	1'-5" d	5	7'-2" b	8'-0" b	7'-9" b	1'-10" d		
6	5'-2" b	5'-9" b	5'-7" b	1'-4" d	6	6'-7" b	7'-4" b	7'-1" b	1'-9" d		
7	4'-8" b	5'-4" b	5'-2" b	1'-3" d	7	6'-1" b	6'-9" b	6'-7" b	1'-8" d		
8	4'-5" b	4'-11" b	4'-10" b	1'-2" d	8	5'-8" b	6'-4" b	6'-2" b	1'-7" d		
8	4'-2" b	4'-8" b	4'-6" b	1'-2" d	9	5'-4" b	6'-1" b	5'-9" b	1'-6" d		
10	3'-11" b	4'-5" b	4'-4" b	1'-1" d	10	5'-1" b	5'-8" b	5'-6" b	1'-5" d		
11	3'-10" b	4'-4" b	4'-1" b	1'-1" d	11	4'-10" b	5'-5" b	5'-3" b	1'-5" d		
12	3'-6" b	4'-1" b	3'-11" d	1'-1" d	12	4'-5" b	5'-0" b	5'-0" b	1'-4" d		

Notes:

1. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.

2. Spans may be interpolated

Table 3B1.1-130 Allowable Edge Beam Spans - Hollow Extrusions Glass & Modular Rooms
For 3 second wind gust at 130 MPH velocity, using design load of 32 #/SF (50 #/SF for Max. Cantilever)
Aluminum Alloy 6063 T-6

Aluminum Alloy 6063 T-6

3.1.2.2.0.044					3.1.2.2.0.045				
Load	Max. Span T_L / bending T_b or deflection T_f	3 Span	4 Span	Max. Cantilever	Load	Max. Span T_L / bending T_b or deflection T_f	3 Span	4 Span	Max. Cantilever
Width (ft.)	1 & 2 Span				Width (ft.)	1 & 2 Span			
5	3-5 ⁺ d	4-3 ⁺ b	4-2 ⁺ b	0-11 ⁺ d	5	3-11 ⁺ d	4-5 ⁺ b	4-5 ⁺ b	1-0 ⁺ d
6	3-6 ⁺ b	4-11 ⁺ b	3-8 ⁺ b	0-11 ⁺ d	6	3-9 ⁺ d	4-3 ⁺ b	4-11 ⁺ b	0-11 ⁺ d
7	3-5 ⁺ b	3-7 ⁺ b	3-6 ⁺ b	0-10 ⁺ d	7	3-6 ⁺ b	3-11 ⁺ b	3-10 ⁺ b	0-11 ⁺ d
8	3-6 ⁺ b	3-5 ⁺ b	3-3 ⁺ b	0-10 ⁺ d	8	3-4 ⁺ b	3-8 ⁺ b	3-7 ⁺ b	0-10 ⁺ d
9	2-10 ⁺ b	3-2 ⁺ b	3-1 ⁺ b	0-9 ⁺ d	9	3-1 ⁺ b	3-6 ⁺ b	3-4 ⁺ b	0-10 ⁺ d
10	2-5 ⁺ b	3-0 ⁺ b	2-11 ⁺ b	0-9 ⁺ d	10	2-11 ⁺ b	3-4 ⁺ b	3-2 ⁺ b	0-10 ⁺ d
11	2-7 ⁺ b	2-11 ⁺ b	2-9 ⁺ b	0-9 ⁺ d	11	2-10 ⁺ b	3-2 ⁺ b	3-0 ⁺ b	0-9 ⁺ d
12	2-6 ⁺ b	2-9 ⁺ b	2-8 ⁺ b	0-9 ⁺ d	12	2-8 ⁺ b	3-0 ⁺ b	2-11 ⁺ b	0-9 ⁺ d

3.1.2.2.0.046					3.1.2.2.0.047				
Load	Max. Span T_L / bending T_b or deflection T_f	3 Span	4 Span	Max. Cantilever	Load	Max. Span T_L / bending T_b or deflection T_f	3 Span	4 Span	Max. Cantilever
Width (ft.)	1 & 2 Span				Width (ft.)	1 & 2 Span			
5	4-3⁺ d	5-1⁺ b	4-11⁺ b	1-1⁺ d	5	4-6⁺ d	5-10⁺ b	5-11⁺ b	1-3⁺ d
6	3-11⁺ d	4-8⁺ b	4-5⁺ b	1-0⁺ d	6	4-6⁺ d	5-8⁺ d	5-5⁺ b	1-2⁺ d
7	3-9⁺ d	4-4⁺ b	4-2⁺ b	0-11⁺ d	7	4-3⁺ d	5-2⁺ b	4-11⁺ b	1-1⁺ d
8	3-7⁺ d	4-1⁺ b	3-11⁺ b	0-11⁺ d	8	4-1⁺ d	4-10⁺ b	4-8⁺ b	1-0⁺ d
9	3-5⁺ b	3-10⁺ b	3-8⁺ b	0-11⁺ d	9	3-11⁺ d	4-7⁺ b	4-5⁺ b	0-11⁺ d
10	3-3⁺ b	3-7⁺ b	3-6⁺ b	0-10⁺ d	10	3-9⁺ d	4-4⁺ b	4-2⁺ b	0-11⁺ d
11	3-1⁺ b	3-5⁺ b	3-4⁺ b	0-10⁺ d	11	3-8⁺ d	4-1⁺ b	3-11⁺ b	0-11⁺ d
12	2-11⁺ b	3-4⁺ b	3-2⁺ b	0-10⁺ d	12	3-6⁺ b	3-11⁺ b	3-10⁺ b	0-11⁺ d
3.1.2.2.0.048					3.1.2.2.0.049				
Load	Max. Span T_L / bending T_b or deflection T_f	3 Span	4 Span	Max. Cantilever	Load	Max. Span T_L / bending T_b or deflection T_f	3 Span	4 Span	Max. Cantilever
Width (ft.)	1 & 2 Span				Width (ft.)	1 & 2 Span			
5	5-2⁺ b	5-9⁺ b	5-7⁺ b	1-4⁺ d	5	6-7⁺ b	7-5⁺ b	7-2⁺ b	1-9⁺ d
6	4-4⁺ b	5-1⁺ b	5-1⁺ b	1-3⁺ d	6	5-7⁺ b	6-8⁺ b	6-6⁺ b	1-8⁺ d
7	4-4⁺ b	4-11⁺ b	4-9⁺ b	1-2⁺ d	7	5-7⁺ b	6-3⁺ b	6-0⁺ b	1-7⁺ d
8	4-1⁺ b	4-7⁺ b	4-5⁺ b	1-2⁺ d	8	5-13⁺ b	5-10⁺ b	5-5⁺ b	1-6⁺ d
9	3-10⁺ b	4-4⁺ b	4-2⁺ b	1-1⁺ d	9	4-11⁺ b	5-6⁺ b	5-4⁺ b	1-5⁺ d
10	3-6⁺ b	4-1⁺ b	3-11⁺ b	1-1⁺ d	10	4-8⁺ b	5-3⁺ b	5-1⁺ b	1-5⁺ d
11	3-8⁺ b	3-11⁺ b	3-9⁺ b	1-0⁺ d	11	4-6⁺ b	4-11⁺ b	4-10⁺ b	1-4⁺ d
12	3-4⁺ b	3-9⁺ b	3-7⁺ b	0-11⁺ d	12	4-3⁺ b	4-9⁺ b	4-7⁺ b	1-4⁺ d

Notes:

1. Above spans do not include hinged or knee braces. And horizontal distances from upright to center of brace to obtain connection to the above spans for total beam spans.

2. Spans may be interpolated

Table 3B1.1-140A Allowable Edge Beam Spans - Hollow Extrusions
Glass & Modular Rooms

③

For 3 second wind gust at 140A MPH velocity; using design load of 3 / #/SF (58 #/SF for max. cantilever)

$2^2 \times 3^2 \times 0.004^2$					
Load Width (ft.)	Max. Span (ft.)	1" / (bending γ or deflection δ_T)			Max. Cantilever
	1 & 2 Span	3 Span	4 Span		
5	3-7' b	3-11' b	3-10' b	0-11' d	5
6	3-3' b	3-8' b	3-6' b	0-10' d	6
7	3-0' b	3-4' b	3-3' b	0-10' d	7
8	2-10' b	3-2' b	3-0' b	0-9' d	8
9	2-8' b	2-11' b	2-10' b	0-9' d	9
10	2-6' b	2-10' b	2-8' b	0-9' d	10
11	2-5' b	2-8' b	2-7' b	0-9' d	11
12	2-4' b	2-7' b	2-6' b	0-9' d	12
$3^2 \times 2^2 \times 0.004^2$					
Load Width (ft.)	Max. Span (ft.)	1" / (bending γ or deflection δ_T)			Max. Cantilever
	1 & 2 Span	3 Span	4 Span		
5	4-0' d	4-9' b	4-7' b	1-0' d	5
6	3-9' d	4-4' b	4-2' b	0-11' d	6
7	3-7' d	4-0' b	3-11' b	0-11' d	7
8	3-4' b	3-6' b	3-6' b	0-11' d	8
9	3-2' b	3-4' b	3-5' b	0-10' d	9
10	3-0' b	3-4' b	3-3' b	0-10' d	10
11	2-10' b	3-3' b	3-1' b	0-9' d	11
12	2-8' b	3-1' b	2-11' b	0-9' d	12
$2^2 \times 3^2 \times 0.005^2$					
Load Width (ft.)	Max. Span (ft.)	1" / (bending γ or deflection δ_T)			Max. Cantilever
	1 & 2 Span	3 Span	4 Span		
5	4-10' b	5-4' b	5-2' b	1-3' d	5
6	4-5' b	4-11' b	4-9' b	1-2' d	6
7	4-1' b	4-6' b	4-5' b	1-2' d	7
8	3-10' b	4-3' b	4-1' b	1-1' d	8
9	3-7' b	4-0' b	3-10' b	1-1' d	9
10	3-5' b	3-10' b	3-8' b	1-0' d	10
11	3-3' b	3-7' b	3-6' b	0-11' d	11
12	3-1' b	3-6' b	3-4' b	0-11' d	12
$3^2 \times 2^2 \times 0.005^2$					
Load Width (ft.)	Max. Span (ft.)	1" / (bending γ or deflection δ_T)			Max. Cantilever
	1 & 2 Span	3 Span	4 Span		
5	4-6' d	5-7' d	5-5' b	1-2' d	5
6	4-3' d	5-2' b	5-0' b	1-1' d	6
7	4-1' d	4-10' b	4-8' b	1-0' d	7
8	3-10' d	4-10' b	4-4' b	0-11' d	8
9	3-6' d	4-3' b	4-1' b	0-11' d	9
10	3-7' d	4-0' b	3-11' b	0-11' d	10
11	3-5' b	3-10' b	3-8' b	0-11' d	11
12	3-3' b	3-9' b	3-6' b	0-10' d	12
$2^2 \times 3^2 \times 0.006^2$					
Load Width (ft.)	Max. Span (ft.)	1" / (bending γ or deflection δ_T)			Max. Cantilever
	1 & 2 Span	3 Span	4 Span		
5	4-10' b	5-4' b	5-2' b	1-3' d	5
6	4-5' b	4-11' b	4-9' b	1-2' d	6
7	4-1' b	4-6' b	4-5' b	1-2' d	7
8	3-10' b	4-3' b	4-1' b	1-1' d	8
9	3-7' b	4-0' b	3-10' b	1-1' d	9
10	3-5' b	3-10' b	3-8' b	1-0' d	10
11	3-3' b	3-7' b	3-6' b	0-11' d	11
12	3-1' b	3-6' b	3-4' b	0-11' d	12
$3^2 \times 2^2 \times 0.006^2$					
Load Width (ft.)	Max. Span (ft.)	1" / (bending γ or deflection δ_T)			Max. Cantilever
	1 & 2 Span	3 Span	4 Span		
5	4-6' d	5-7' d	5-5' b	1-2' d	5
6	4-3' d	5-2' b	5-0' b	1-1' d	6
7	4-1' d	4-10' b	4-8' b	1-0' d	7
8	3-10' d	4-10' b	4-4' b	0-11' d	8
9	3-6' d	4-3' b	4-1' b	0-11' d	9
10	3-7' d	4-0' b	3-11' b	0-11' d	10
11	3-5' b	3-10' b	3-8' b	0-11' d	11
12	3-3' b	3-9' b	3-6' b	0-10' d	12

Notes:

1. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.

2. Spans may be interpolated

Table 3B.1.2-110 Allowable Edge Beam Spans - Snap Sections
Glass & Modular Rooms
For 3 second wind gust at 110 MPH velocity; using design load of 23 #/SF (36 #/SF for Max. Cantilever)

2" x 2" x 0.044" Snap Extrusion						2" x 3" x 0.045" Snap Extrusion					
Load	Max. Span L / (bending "v" or deflection "δ")	1 & 2 Span	3 Span	4 Span	Max. Cantilever	Load	Max. Span L / (bending "v" or deflection "δ")	1 & 2 Span	3 Span	4 Span	Max. Cantilever
Width (ft.)						Width (ft.)					
5	4'-9" d	5'-6" d	5'-10" b	1'-2" d	1'-2" d	5	6'-4" d	7'-9" b	7'-5" b	1'-7" d	1'-7" d
6	4'-5" d	5'-0" d	5'-4" b	1'-2" d	1'-2" d	6	5'-11" d	7'-0" b	6'-9" b	1'-6" d	1'-6" d
7	4'-2" d	5'-1" b	4'-11" b	1'-1" d	1'-1" d	7	5'-8" d	6'-6" b	6'-3" b	1'-5" d	1'-5" d
8	4'-0" d	4'-9" b	4'-7" b	1'-0" d	1'-0" d	8	5'-5" d	6'-1" b	5'-11" b	1'-5" d	1'-5" d
9	3'-10" d	4'-6" b	4'-4" b	0'-11" d	0'-11" d	9	5'-2" d	5'-9" b	5'-7" b	1'-4" d	1'-4" d
10	3'-9" d	4'-3" b	4'-1" b	0'-11" d	0'-11" d	10	4'-10" b	5'-5" b	5'-3" b	1'-3" d	1'-3" d
11	3'-7" d	4'-1" b	3'-11" b	0'-11" d	0'-11" d	11	4'-6" b	5'-2" b	5'-0" b	1'-3" d	1'-3" d
12	3'-6" b	3'-11" b	3'-9" b	0'-11" d	0'-11" d	12	4'-5" b	4'-11" b	4'-10" b	1'-2" d	1'-2" d

2" x 4" x 0.045" Snap Extrusion					
Load	Max. Span L / (bending "v" or deflection "δ")	1 & 2 Span	3 Span	4 Span	Max. Cantilever
Width (ft.)					
5	7'-11" d	9'-5" d	9'-1" b	2'-0" d	2'-0" d
6	7'-6" d	8'-7" b	8'-4" b	1'-11" d	1'-11" d
7	7'-1" b	7'-11" b	7'-8" b	1'-10" d	1'-10" d
8	6'-8" b	7'-5" b	7'-2" b	1'-9" d	1'-9" d
9	6'-3" b	7'-0" b	6'-9" b	1'-8" d	1'-8" d
10	5'-11" b	6'-5" b	6'-3" b	1'-7" d	1'-7" d
11	5'-8" b	6'-4" b	6'-2" b	1'-7" d	1'-7" d
12	5'-5" b	6'-1" b	5'-10" b	1'-6" d	1'-6" d

Notes:
1. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
2. Spans may be interpolated.

Table 3B.1.2-120 Allowable Edge Beam Spans - Snap Sections
Glass & Modular Rooms
For 3 second wind gust at 120 MPH velocity; using design load of 27 #/SF (43 #/SF for Max. Cantilever)

2" x 2" x 0.044" Snap Extrusion						2" x 3" x 0.045" Snap Extrusion					
Load	Max. Span L / (bending v ^o or deflection v ^o)					Load	Max. Span L / (bending v ^o or deflection v ^o)				
Width (ft.)	1 & 2 Span	3 Span	4 Span	Max. Cantilever	Width (ft.)	1 & 2 Span	3 Span	4 Span	Max. Cantilever		
5	4'-5" d	5'-6" d	5'-4" d	1'-2" d	5	6'-0" d	7'-1" d	6'-10" d	1'-9" d		
6	4'-2" d	5'-1" d	4'-11" d	1'-1" d	6	5'-8" d	6'-6" d	6'-3" d	1'-8" d		
7	3'-11" d	4'-8" d	4'-6" d	1'-0" d	7	5'-4" d	6'-0" d	5'-10" d	1'-4" d		
8	3'-10" d	4'-5" d	4'-3" d	0'-11" d	8	5'-0" d	5'-6" d	5'-5" d	1'-3" d		
9	3'-8" d	4'-2" d	3'-11" d	0'-11" d	9	4'-9" d	5'-4" d	5'-1" d	1'-3" d		
10	3'-6" d	3'-11" d	3'-9" d	0'-11" d	10	4'-6" d	5'-0" d	4'-10" d	1'-2" d		
11	3'-4" d	3'-8" d	3'-7" d	0'-10" d	11	4'-3" d	4'-9" d	4'-8" d	1'-2" d		
12	3'-2" d	3'-7" d	3'-5" d	0'-10" d	12	4'-1" d	4'-7" d	4'-5" d	1'-2" d		

Notes:
1. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
2. Spans may be interpolated.

Table 3B.1.2-130 Allowable Edge Beam Spans - Snap Sections
Glass & Modular Rooms
For 3 second wind gust at 130 MPH velocity; using design load of 32 #/SF (50 #/SF for Max. Cantilever)

2" x 4" x 0.045" Snap Extrusion						2" x 3" x 0.045" Snap Extrusion					
Load	Max. Span L / (bending ∇ or deflection ∇')	1 & 2 Span	3 Span	4 Span	Max. Cantilever	Load	Max. Span L / (bending ∇ or deflection ∇')	1 & 2 Span	3 Span	4 Span	Max. Cantilever
Width (ft.)						Width (ft.)					
5	4'-3" d	5'-1" d	4'-11" d	1'-1" d	1'-1" d	5	5'-5" d	6'-5" d	6'-4" d	1'-5" d	1'-5" d
6	3'-11" d	4'-8" d	4'-6" d	1'-0" d	1'-0" d	6	5'-4" d	6'-4" d	6'-3" d	1'-4" d	1'-4" d
7	3'-9" d	4'-4" d	4'-2" d	0'-11" d	0'-11" d	7	4'-11" d	5'-6" d	5'-4" d	1'-4" d	1'-4" d
8	3'-7" d	4'-0" d	3'-11" d	0'-11" d	0'-11" d	8	4'-7" d	5'-2" d	4'-11" d	1'-3" d	1'-3" d
9	3'-5" d	3'-10" d	3'-8" d	0'-11" d	0'-11" d	9	4'-4" d	4'-10" d	4'-8" d	1'-2" d	1'-2" d
10	3'-3" d	3'-7" d	3'-6" d	0'-10" d	0'-10" d	10	4'-2" d	4'-7" d	4'-6" d	1'-2" d	1'-2" d
11	3'-1" d	3'-5" d	3'-4" d	0'-10" d	0'-10" d	11	3'-11" d	4'-5" d	4'-3" d	1'-1" d	1'-1" d
12	2'-11" d	3'-3" d	3'-2" d	0'-10" d	0'-10" d	12	3'-9" d	4'-3" d	4'-1" d	1'-1" d	1'-1" d

Notes:
1. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
2. Spans may be interpolated.

Table 3B.1.2-140A Allowable Edge Beam Spans - Snap Sections
Glass & Modular Rooms
For 3 second wind gust at 140A MPH velocity; using design load of 37 #/SF (58 #/SF for Max. Cantilever)

2" x 2" x 0.044" Snap Extrusion						2" x 3" x 0.045" Snap Extrusion						
Load	Max. Span L / (bending v _r or deflection v _f)					Load	Max. Span L / (bending v _r or deflection v _f)					
Width (ft.)	1 & 2 Span	3 Span	4 Span	Cantilever	Max.	Width (ft.)	1 & 2 Span	3 Span	4 Span	Cantilever	Max.	
5	4'-0"	4'-5"	4'-2"	1'-0"	d	5	5'-5"	d	6'-5"	d	1'-5"	d
6	3'-9"	4'-4"	4'-2"	0'-11"	d	6	5'-1"	d	5'-9"	d	1'-4"	d
7	3'-7"	4'-0"	3'-10"	0'-11"	d	7	4'-10"	d	5'-6"	d	1'-3"	d
8	3'-4"	3'-9"	3'-7"	0'-10"	d	8	4'-7"	d	5'-2"	d	1'-2"	d
9	3'-2"	3'-6"	3'-5"	0'-10"	d	9	4'-4"	d	4'-10"	d	1'-2"	d
10	2'-11"	3'-4"	3'-3"	0'-10"	d	10	4'-2"	d	4'-7"	d	1'-1"	d
11	2'-10"	3'-2"	3'-1"	0'-9"	d	11	3'-11"	d	4'-5"	d	1'-1"	d
12	2'-9"	3'-1"	2'-11"	0'-9"	d	12	3'-9"	d	4'-3"	d	1'-0"	d

2" x 4" x 0.045" Snap Extrusion						
Load	Max. Span L / (bending v _r or deflection v _f)					
Width (ft.)	1 & 2 Span	3 Span	4 Span	Cantilever	Max.	
5	6'-5"	d	7'-2"	d	1'-8"	d
6	6'-1"	d	6'-8"	d	1'-6"	d
7	5'-7"	d	6'-3"	d	1'-7"	d
8	5'-3"	d	5'-10"	d	1'-6"	d
9	4'-11"	d	5'-6"	d	1'-5"	d
10	4'-8"	d	5'-3"	d	1'-5"	d
11	4'-6"	d	5'-0"	d	1'-4"	d
12	4'-3"	d	4'-10"	d	1'-4"	d

Notes:
1. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
2. Spans may be interpolated.

Table 3B.1.3-110 Allowable Edge Beam Spans - Hollow Extrusions
For Screen Rooms Converted to or Typical Construction of Glass / Enclosed Rooms
For 3 second wind gust at 110 MPH velocity; using design load of 23 #/SF (36 #/SF for Max. Cantilever)

Load Width (ft.)	2" x 2" Span					Load Width (ft.)	1 & 2" Span				
	Max. Span L / (bending ψ or deflection δ)	3 Span	4 Span	Max. Cantilever	Max. Span L / (bending ψ or deflection δ)		3 Span	4 Span	Max. Cantilever		
5	5'-3" d	6'-3" d	6'-0" d	1'-4" d	5	6'-0" d	7'-1" d	6'-11" d	1'-6" d		
6	4'-11" d	5'-6" d	5'-6" d	1'-3" d	6	5'-8" d	6'-6" d	6'-3" d	1'-5" d		
7	4'-8" d	5'-3" d	5'-1" d	1'-2" d	7	5'-4" d	6'-0" d	5'-10" d	1'-4" d		
8	4'-5" d	4'-11" d	4'-9" d	1'-2" d	8	5'-0" d	5'-6" d	5'-5" d	1'-4" d		
9	4'-2" d	4'-8" d	4'-6" d	1'-1" d	9	4'-8" d	5'-4" d	5'-2" d	1'-3" d		
10	3'-11" d	4'-5" d	4'-3" d	1'-1" d	10	4'-6" d	5'-0" d	4'-10" d	1'-3" d		
11	3'-8" d	4'-2" d	4'-1" d	1'-0" d	11	4'-4" d	4'-10" d	4'-8" d	1'-2" d		
12	3'-7" d	4'-0" d	3'-10" d	0'-11" d	12	4'-1" d	4'-7" d	4'-5" d	1'-2" d		

Load Width (ft.)	Max. Span L / (bending v or deflection v)			
	1 & 2 Span	3 Span	4 Span	Max. Cantilever
5	7'-5" d	8'-5" b	8'-2" b	1'-11" d
6	6'-11" b	7'-4" b	7'-5" b	1'-9" d
7	6'-4" b	7'-2" b	6'-11" b	1'-8" d
8	5'-11" b	6'-4" b	6'-5" b	1'-7" d
9	5'-7" b	6'-3" b	6'-1" b	1'-7" d
10	5'-4" b	5'-11" b	5'-9" b	1'-6" d
11	5'-1" b	5'-8" b	5'-6" b	1'-5" d
12	4'-10" b	5'-5" b	5'-3" b	1'-5" d

Notes:
1. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.

Table 3B.1.3-120 Allowable Edge Beam Spans - Hollow Extrusions
For Screen Rooms Converted to or Typical Construction of Glass / Enclosed Rooms
For 3 second wind gust at 120 MPH velocity; using design load of 27 #/SF (43 #/SF for Max. Cantilever)

2" x 2" x 0.044" w/ Snap Cap Attached						1" x 2" x 0.044" to 2" x 2" x Existing					
Load	Max. Span L / (bending v or deflection v)					Load	Max. Span L / (bending v or deflection v)				
Width (ft.)	1 & 2 Span	3 Span	4 Span	Max. Cantilever	Width (ft.)	1 & 2 Span	3 Span	4 Span	Max. Cantilever		
5	4'-11" d	5'-8" d	5'-6" b	1'-3" d	5	5'-6" d	6'-7" b	6'-4" b	1'-5" d		
6	4'-8" d	5'-3" d	5'-1" b	1'-2" d	6	5'-4" d	6'-11" b	6'-10" b	1'-4" d		
7	4'-4" b	4'-10" b	4'-8" b	1'-1" d	7	4'-11" b	5'-7" b	5'-4" b	1'-4" d		
8	4'-1" b	4'-6" b	4'-5" b	1'-1" d	8	4'-8" b	5'-2" b	5'-0" b	1'-3" d		
9	3'-10" b	4'-3" b	4'-2" d	1'-0" d	9	4'-5" d	4'-11" b	4'-9" b	1'-2" d		
10	3'-8" b	4'-1" b	3'-11" b	0'-11" d	10	4'-2" b	4'-8" b	4'-6" b	1'-2" d		
11	3'-5" d	3'-10" b	3'-8" b	0'-11" d	11	3'-11" b	4'-5" b	4'-3" b	1'-1" d		
12	3'-4" b	3'-8" b	3'-7" b	0'-11" d	12	3'-10" b	4'-3" b	4'-1" b	1'-1" d		

Load Width (ft.)	Max. Span L / (bending v or deflection v)				
	1 & 2 Span	3 Span	4 Span	Max. Cantilever	
5	6'-11" d	7'-6" d	7'-6" d	1'-9" d	
6	6'-4" d	7'-1" d	6'-10" d	1'-8" d	
7	5'-11" d	6'-7" d	6'-4" d	1'-7" d	
8	5'-6" d	6'-2" d	5'-11" d	1'-6" d	
9	5'-2" d	5'-10" d	5'-7" d	1'-5" d	
10	4'-11" d	5'-6" d	5'-4" d	1'-5" d	
11	4'-8" d	5'-3" d	5'-1" d	1'-4" d	
12	4'-6" d	5'-0" d	4'-10" d	1'-4" d	

Notes:
1. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.

Table 3B.1.3-130 Allowable Edge Beam Spans - Hollow Extrusions
For Screen Rooms Converted to or Typical Construction of Glass / Enclosed Rooms
For 3 second wind gust at 130 MPH velocity; using design load of 32 #/SF (50 #/SF for Max. Cantilever)

2" x 2" x 0.044" w/ Snap Cap Attached						1" x 2" x 0.044" to 2" x 2" x Existing					
Load	Max. Span L / (bending v or deflection v)	1 & 2 Span	3 Span	4 Span	Max. Cantilever	Load	Max. Span L / (bending v or deflection v)	1 & 2 Span	3 Span	4 Span	Max. Cantilever
Width (ft.)						Width (ft.)					
5	4'-6" d	5'-3" d	5'-1" d	1'-2" d	1'-2" d	5	5'-5" d	6'-0" d	5'-10" d	1'-4" d	1'-4" d
6	4'-4" d	4'-10" d	4'-8" d	1'-2" d		6	4'-11" d	5'-6" d	5'-4" d	1'-4" d	1'-4" d
7	3'-11" d	4'-5" d	4'-4" d	1'-1" d		7	4'-7" d	5'-1" d	4'-11" d	1'-3" d	
8	3'-6" d	4'-2" d	4'-0" d	1'-0" d		8	4'-3" d	4'-9" d	4'-7" d	1'-2" d	
9	3'-0" d	3'-11" d	3'-10" d	0'-11" d		9	4'-0" d	4'-6" d	4'-4" d	1'-2" d	
10	3'-4" d	3'-8" d	3'-7" d	0'-11" d		10	3'-10" d	4'-3" d	4'-4" d	1'-1" d	
11	3'-2" d	3'-7" d	3'-5" d	0'-11" d		11	3'-6" d	4'-1" d	3'-11" d	1'-1" d	
12	3'-0" d	3'-5" d	3'-3" d	0'-11" d		12	3'-6" d	3'-11" d	3'-9" d	1'-0" d	

2" x 2" x 0.044" to 2" x 2" x Existing					
Load	Max. Span L / (bending v or deflection v)	1 & 2 Span	3 Span	4 Span	Max. Cantilever
Width (ft.)					
5	6'-5" d	7'-2" d	6'-11" d	1'-8" d	
6	5'-10" d	6'-6" d	6'-4" d	1'-7" d	
7	6'-5" d	6'-0" d	5'-10" d	1'-6" d	
8	6'-1" d	5'-8" d	5'-6" d	1'-5" d	
9	4'-9" d	5'-4" d	5'-2" d	1'-5" d	
10	4'-6" d	5'-1" d	4'-11" d	1'-4" d	
11	4'-4" d	4'-10" d	4'-8" d	1'-4" d	
12	4'-2" d	4'-7" d	4'-6" d	1'-3" d	

Table 3B.1.4-110
Allowable Spans For Fourth Wall or Miscellaneous Framing Beams for Glass & Modular Rooms
 For 3 second wind gust at 110 MPH velocity; using design load of 23 #/SF
 Aluminum Alloy 6063 T-6

Single Self-Mating Beams	4'-0"	6'-0"	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"	18'-0"	20'-0"	22'-0"	24'-0"	26'-0"
2" x 4" x 0.044" x 0.100"	8'-1" d	7'-11" d	7'-3" b	6'-5" b	5'-11" b	5'-5" b	4'-10" b	4'-7" b	4'-4" b	4'-2" b	4'-0" b	3'-8" b
2" x 5" x 0.050" x 0.100"	11'-4" d	8'-10" d	8'-11" b	7'-11" b	7'-4" b	6'-8" b	5'-11" b	5'-8" b	5'-5" b	5'-2" b	4'-11" b	4'-8" b
2" x 6" x 0.060" x 0.120"	13'-3" d	11'-7" d	10'-4" b	9'-5" b	7'-10" b	7'-4" b	6'-11" b	6'-8" b	6'-3" b	5'-11" b	5'-8" b	5'-5" b
2" x 7" x 0.065" x 0.120"	15'-1" d	13'-2" d	11'-8" b	10'-5" b	9'-8" b	8'-10" b	6'-3" b	7'-9" b	7'-4" b	7'-0" b	6'-8" b	6'-5" b
2" x 7" x 0.085" w/ Insulat	18'-2" d	15'-11" d	14'-0" b	12'-7" d	12'-7" d	11'-8" b	10'-11" b	10'-4" b	9'-9" b	9'-4" b	8'-11" b	8'-7" b
2" x 8" x 0.072" x 0.224"	18'-6" d	16'-4" d	14'-10" d	13'-5" b	12'-3" b	11'-4" b	10'-11" b	10'-4" b	9'-6" b	9'-1" b	8'-11" b	8'-7" b
2" x 8" x 0.072" x 0.224"	20'-3" d	17'-11" d	14'-7" b	13'-4" b	12'-4" b	11'-6" b	10'-11" b	10'-4" b	9'-6" b	9'-1" b	8'-11" b	8'-7" b
2" x 8" x 0.082" x 0.306"	21'-3" d	18'-6" d	16'-10" d	15'-8" d	14'-7" b	13'-6" b	12'-4" b	11'-4" b	10'-9" b	10'-4" b	9'-5" b	9'-0" b
2" x 10" x 0.092" x 0.369"	25'-6" d	22'-4" d	20'-3" d	18'-10" d	17'-7" b	16'-3" b	15'-2" b	14'-4" b	13'-7" b	12'-11" b	12'-5" b	11'-11" b

Double Self-Mating Beams	4'-0"	6'-0"	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"	18'-0"	20'-0"	22'-0"	24'-0"	26'-0"
2" x 8" x 0.072" x 0.224"	23'-6" d	20'-6" d	18'-6" d	17'-4" d	16'-4" d	15'-6" d	14'-10" d	14'-2" b	13'-5" b	12'-10" b	12'-3" b	11'-9" b
2" x 8" x 0.072" x 0.224"	25'-10" d	22'-6" d	20'-6" d	19'-0" d	17'-11" d	16'-11" d	15'-3" d	15'-4" b	14'-7" b	13'-11" b	13'-4" b	12'-8" b
2" x 9" x 0.082" x 0.306"	27'-5" d	23'-11" d	21'-9" d	20'-3" d	19'-0" d	18'-1" d	17'-3" d	16'-7" d	15'-11" b	15'-3" b	14'-7" b	14'-0" b
2" x 10" x 0.092" x 0.369"	32'-2" d	28'-1" d	25'-6" d	23'-8" d	22'-4" d	21'-2" d	20'-3" d	19'-6" d	18'-10" d	18'-3" d	17'-7" b	16'-10" b

Note:
 1. It is recommended that the engineer be consulted on any miscellaneous framing beam that spans more than 40'.
 2. Spans are based on 110 M.P.H. wind load plus dead load for framing.
 3. Span is measured from center of connection to fascia or wall connection.
 4. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
 5. Spans may be interpolated.

Table 3B.1.4-120
Allowable Spans For Fourth Wall or Miscellaneous Framing Beams for Glass & Modular Rooms
 For 3 second wind gust at 120 MPH velocity; using design load of 27 #/SF
 Aluminum Alloy 6063 T-6

Single Self-Mating Beams	4'-0"	6'-0"	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"	18'-0"	20'-0"	22'-0"	24'-0"	26'-0"
2" x 4" x 0.044" x 0.100"	8'-6" d	7'-7" d	6'-8" b	5'-11" b	5'-0" b	4'-9" b	4'-5" b	4'-3" b	4'-0" b	3'-10" b	3'-8" b	3'-6" b
2" x 5" x 0.050" x 0.100"	10'-8" d	9'-4" d	8'-5" b	7'-4" b	6'-8" b	6'-3" b	5'-10" b	5'-6" b	5'-3" b	4'-11" b	4'-8" b	4'-7" b
2" x 6" x 0.060" x 0.120"	12'-6" d	10'-11" d	9'-8" b	8'-6" b	7'-9" b	7'-3" b	6'-8" b	6'-4" b	6'-0" b	5'-9" b	5'-6" b	5'-3" b
2" x 7" x 0.065" x 0.120"	14'-3" d	12'-5" b	10'-8" b	9'-7" b	8'-9" b	8'-1" b	7'-7" b	7'-2" b	6'-9" b	6'-6" b	6'-2" b	5'-11" b
2" x 7" x 0.085" w/ Insulat	17'-3" d	15'-1" d	13'-8" d	12'-8" d	11'-8" b	10'-10" b	10'-1" b	9'-8" b	8'-7" b	8'-3" b	7'-11" b	7'-8" b
2" x 8" x 0.072" x 0.224"	17'-8" d	15'-5" d	13'-10" b	12'-8" b	11'-4" b	10'-8" b	9'-10" b	8'-3" b	8'-0" b	7'-11" b	7'-8" b	7'-5" b
2" x 8" x 0.072" x 0.224"	19'-5" d	16'-11" d	14'-9" b	13'-5" b	12'-3" b	11'-4" b	10'-6" b	10'-0" b	9'-5" b	9'-1" b	8'-6" b	8'-4" b
2" x 9" x 0.082" x 0.306"	20'-1" d	17'-7" d	15'-11" d	14'-8" b	13'-6" b	12'-6" b	11'-8" b	11'-4" b	10'-5" b	9'-11" b	8'-9" b	8'-7" b
2" x 10" x 0.092" x 0.369"	24'-2" d	21'-2" d	19'-2" d	17'-9" b	16'-2" b	14'-11" b	14'-0" b	13'-3" b	12'-7" b	11'-11" b	11'-5" b	11'-0" b

Double Self-Mating Beams	4'-0"	6'-0"	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"	18'-0"	20'-0"	22'-0"	24'-0"	26'-0"
2" x 8" x 0.072" x 0.224"	22'-3" d	19'-6" d	17'-8" d	16'-5" d	15'-5" d	14'-8" d	13'-10" b	13'-4" b	12'-5" b	11'-10" b	11'-4" b	10'-11" b
2" x 8" x 0.072" x 0.224"	24'-5" d	21'-4" d	19'-5" d	18'-0" d	16'-11" d	16'-1" b	15'-0" b	14'-2" b	13'-5" b	12'-10" b	12'-3" b	11'-10" b
2" x 9" x 0.082" x 0.306"	26'-0" d	22'-9" d	20'-8" d	19'-2" d	18'-0" d	17'-2" d	16'-5" d	15'-7" d	14'-9" b	14'-1" b	13'-6" b	12'-11" b
2" x 10" x 0.092" x 0.369"	30'-6" d	26'-6" d	24'-2" d	22'-5" d	21'-2" d	20'-1" d	19'-2" d	18'-6" d	17'-9" b	16'-11" b	16'-2" b	15'-7" b

Note:
 1. It is recommended that the engineer be consulted on any miscellaneous framing beam that spans more than 40'.
 2. Spans are based on 120 M.P.H. wind load plus dead load for framing.
 3. Span is measured from center of connection to fascia or wall connection.
 4. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
 5. Spans may be interpolated.

Table 3B.1.4-130
Allowable Spans For Fourth Wall or Miscellaneous Framing Beams for Glass & Modular Rooms
 For 3 second wind gust at 130 MPH velocity; using design load of 32 #/SF
 Aluminum Alloy 6063 T-6

Single Self-Mating Beams	4'-0"	6'-0"	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"	18'-0"	20'-0"	22'-0"	24'-0"	26'-0"
2" x 4" x 0.044" x 0.100"	8'-2" d	7'-1" b	6'-1" b	5'-6" b	4'-11" b	4'-8" b	4'-4" b	4'-1" b	3'-10" b	3'-8" b	3'-6" b	3'-5" b
2" x 5" x 0.050" x 0.100"	10'-1" d	8'-8" b	7'-7" b	6'-8" b	6'-2" b	5'-8" b	5'-4" b	5'-1" b	4'-9" b	4'-7" b	4'-4" b	4'-2" b
2" x 6" x 0.060" x 0.120"	11'-10" d	10'-1" b	8'-9" b	7'-10" b	7'-2" b	6'-7" b	6'-2" b	5'-10" b	5'-6" b	5'-3" b	5'-1" b	4'-10" b
2" x 7" x 0.065" x 0.120"	13'-6" d	11'-5" b	9'-10" b	8'-10" b	8'-1" b	7'-5" b	6'-11" b	6'-7" b	6'-3" b	5'-11" b	5'-8" b	5'-6" b
2" x 7" x 0.085" w/ Insulat	16'-4" d	14'-3" d	12'-11" d	11'-9" b	10'-8" b	9'-11" b	8'-9" b	8'-3" b	7'-11" b	7'-7" b	7'-3" b	7'-3" b
2" x 8" x 0.072" x 0.224"	16'-8" d	14'-7" d	12'-9" b	11'-5" b	10'-5" b	9'-8" b	8'-9" b	8'-1" b	7'-8" b	7'-4" b	7'-4" b	7'-1" b
2" x 8" x 0.072" x 0.224"	18'-4" d	15'-11" b	13'-10" b	12'-4" b	11'-3" b	10'-5" b	9'-8" b	8'-3" b	8'-1" b	7'-8" b	7'-4" b	7'-8" b
2" x 9" x 0.082" x 0.306"	18'-0" d	16'-7" d	15'-1" d	13'-7" d	12'-5" b	11'-6" b	10'-8" b	10'-1" b	9'-7" b	8'-2" b	8'-6" b	8'-5" b
2" x 10" x 0.092" x 0.369"	22'-10" d	19'-11" d	18'-2" d	16'-4" b	14'-11" b	13'-8" b	12'-11" b	12'-2" b	11'-6" b	10'-11" b	10'-6" b	10'-1" b

Double Self-Mating Beams	4'-0"	6'-0"	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"	18'-0"	20'-0"	22'-0"	24'-0"	26'-0"
2" x 8" x 0.072" x 0.224"	21'-1" d	18'-5" d	16'-8" d	15'-6" d	14'-7" d	13'-7" b	12'-8" b	12'-0" b	11'-5" b	10'-10" b	10'-5" b	9'-11" b
2" x 8" x 0.072" x 0.224"	23'-1" d	20'-2" d	18'-4" d	17'-0" d	15'-11" b	14'-9" b	13'-10" b	13'-0" b	12'-4" b	11'-8" b	11'-3" b	10'-10" b
2" x 9" x 0.082" x 0.306"	24'-7" d	21'-8" d	19'-5" d	18'-1" d	16'-2" d	15'-2" b	14'-4" b	14'-4" b	13'-7" b	12'-11" b	12'-5" b	11'-11" b
2" x 10" x 0.092" x 0.369"	28'-10" d	25'-2" d	22'-10" d	21'-3" d	19'-11" d	18'-11" d	18'-2" d	17'-2" b	16'-4" b	15'-7" b	14'-11" b	14'-4" b

Note:
 1. It is recommended that the engineer be consulted on any miscellaneous framing beam that spans more than 40'.
 2. Spans are based on 130 M.P.H. wind load plus dead load for framing.
 3. Span is measured from center of connection to fascia or wall connection.
 4. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
 5. Spans may be interpolated.

Table 3B.1.4-140A
Allowable Spans For Fourth Wall or Miscellaneous Framing Beams for Glass & Modular Rooms
 For 3 second wind gust at 140A MPH velocity; using design load of 37 #/SF
 Aluminum Alloy 6063 T-6

Single Self-Mating Beams	4'-0"	6'-0"	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"	18'-0"	20'-0"	22'-0"	24'-0"	26'-0"
2" x 4" x 0.044" x 0.100"	7'-9" d	6'-7" b	5'-8" b	5'-1" b	4'-9" b	4'-4" b	4'-0" b	3'-10" b	3'-7" b	3'-5" b	3'-3" b	3'-2" b
2" x 5" x 0.050" x 0.100"	9'-8" d	8'-2" b	7'-1" b	6'-4" b	5'-9" b	5'-4" b	4'-11" b	4'-8" b	4'-5" b	4'-3" b	4'-1" b	3'-11" b
2" x 6" x 0.060" x 0.120"	11'-3" d	9'-5" b	8'-2" b	7'-3" b	6'-8" b	6'-2" b	5'-9" b	5'-6" b	5'-2" b	4'-11" b	4'-8" b	4'-6" b
2" x 7" x 0.065" x 0.120"	12'-10" d	10'-7" b	9'-2" b	8'-2" b	7'-6" b	6'-11" b	6'-6" b	6'-1" b	5'-10" b	5'-6" b	5'-4" b	5'-1" b
2" x 7" x 0.085" w/ Insulat	15'-8" d	13'-7" d	12'-2" d	10'-11" b	9'-11" b	8'-3" b	8'-5" b	7'-11" b	7'-6" b	7'-2" b	6'-10" b	6'-7" b
2" x 8" x 0.072" x 0.224"	15'-11" d	13'-9" d	12'-0" b	10'-7" b	9'-8" b	8'-11" b	8'-5" b	7'-11" b	7'-6" b	7'-2" b	6'-10" b	6'-7" b
2" x 8" x 0.072" x 0.224"	17'-6" d	14'-10" d	12'-10" b	11'-6" b	10'-6" b	9'-9" b	9'-1" b	8'-7" b	8'-1" b	7'-6" b	7'-5" b	7'-2" b
2" x 9" x 0.082" x 0.306"	18'-1" d	15'-10" d	14'-1" b	12'-7" b	11'-6" b	10'-8" b	9'-11" b	9'-5" b	8'-6" b	8'-2" b	7'-10" b	7'-10" b
2" x 10" x 0.092" x 0.369"	21'-9" d	19'-0" d	16'-11" b	15'-2" b	13'-10" b	12'-10" b	11'-11" b	11'-4" b	10'-8" b	10'-3" b	9'-9" b	9'-5" b

Double Self-Mating Beams	4'-0"	6'-0"	8'-0"	10'-0"	12'-0"	14'-0"	16'-0"	18'-0"	20'-0"	22'-0"	24'-0"	26'-0"
2" x 8" x 0.072" x 0.224"	20'-1" d	17'-6" d	15'-11" d	14'-9" d	13'-6" d	12'-6" b	11'-10" b	11'-2" b	10'-7" b	10'-1" b	9'-6" b	9'-4" b
2" x 8" x 0.072" x 0.224"	22'-0" d	19'-3" d	16'-5" d	14'-10" b	13'-9" b	12'-10" b	12'-0" b	11'-6" b	10'-11" b	10'-6" b	10'-1" b	10'-1" b
2" x 9" x 0.082" x 0.306"	23'-5" d	20'-5" d	18'-7" d	17'-3" d	16'-3" d	15'-1" b	14'-1" b	13'-4" b	12'-7" b	12'-0" b	11'-6" b	11'-1" b
2" x 10" x 0.092" x 0.369"	27'-5" d	23'-11" d	21'-9" d	20'-3" d	18'-0" d	16'-1" d	15'-11" b	15'-1" b	14'-5" b	13'-10" b	13'-4" b	13'-4" b

Note:
 1. It is recommended that the engineer be consulted on any miscellaneous framing beam that spans more than 40'.
 2. Spans are based on 140A M.P.H. wind load plus dead load for framing.
 3. Span is measured from center of connection to fascia or wall connection.
 4. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
 5. Spans may be interpolated.

Table 3B.1.5-110
Allowable Spans for Ridge Beams with Self Mating Beams or Square Tubes
 For 3 second wind gust at 110 MPH velocity; using design load of 23 #/SF

Self Mating Sections	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"	10'-0"	11'-0"	12'-0"
2" x 4" x 0.044 x 0.100"	10'-3" b	8'-4" b	6'-8" b	5'-1" b	4'-7" b	4'-3" b	6'-11" b	6'-7" b
2" x 5" x 0.050 x 0.100"	12'-8" b	11'-6" b	10'-8" b	8'-11" b	7'-3" b	6'-5" b	8'-6" b	8'-2" b
2" x 6" x 0.060 x 0.120"	14'-7" b	13'-4" b	12'-4" b	11'-7" b	10'-11" b	10'-4" b	8'-10" b	8'-6" b
2" x 7" x 0.065 w/ Insulat	16'-5" b	15'-0" b	13'-11" b	13'-0" b	12'-3" b	11'-6" b	11'-1" b	10'-7" b
2" x 7" x 0.085 w/ Insulat	21'-11" b	19'-5" b	18'-6" b	17'-4" b	16'-4" b	15'-6" b	14'-8" b	14'-2" b
2" x 8" x 0.072 x 0.224"	21'-3" b	19'-5" b	17'-11" b	16'-10" b	15'-10" b	14'-9" b	14'-4" b	13'-9" b
2" x 8" x 0.072 x 0.224"	23'-1" b	21'-0" b	19'-6" b	18'-3" b	17'-2" b	16'-4" b	15'-6" b	14'-10" b
2" x 9" x 0.082 x 0.310"	25'-4" b	23'-1" b	21'-5" b	19'-11" b	18'-10" b	17'-11" b	17'-1" b	16'-4" b
2" x 10" x 0.092 x 0.369"	30'-5" b	27'-9" b	25'-8" b	23'-9" b	22'-8" b	21'-6" b	20'-6" b	19'-6" b

Square Tubes	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"	10'-0"	11'-0"	12'-0"
	Allowable Span 1' / bending b or deflection d							
2" x 4" x 0.044 x 0.100"	10'-6" d	8'-7" b	6'-11" b	5'-4" b	4'-7" b	4'-3" b	7'-5" b	6'-9" b
2" x 5" x 0.050 x 0.100"	11'-5" d	10'-9" d	10'-2" b	8'-5" b	6'-11" b	5'-8" b	8'-1" b	7'-9" b

Notes:

1. Glass Joists: The addition of aluminum frame wyndows with glass panes that are designed to 110 M.P.H. wind load increases the above upright spans. Increases the straight run but additional framing is not required.

Table 3B.2.1 Allowable Upright Heights, Chair Rail Spans or Header Spans For Glass & Modular Rooms

Sections	Thickened Load Width W_L = Parlin Spacing											
	3'-0"	3'-6"	4'-0"	4'-6"	5'-0"	5'-6"	6'-0"	6'-6"	7'-0"	7'-6"		
	Allowable Height H_A / bending M or deflection δ^d											
2' x 2' x 0.044" Hollow	6'-10"	6'-6"	6'-2"	5'-11"	5'-8"	5'-5"	5'-2"	4'-11"	4'-8"	4'-5"	4'-2"	4'-0"
2' x 2' x 0.055" Hollow	7'-3"	6'-10"	6'-7"	6'-4"	6'-1"	5'-11"	5'-8"	5'-5"	5'-3"	5'-1"	5'-0"	
3' x 2' x 0.045" Hollow	7'-3"	7'-4"	7'-0"	6'-7"	6'-4"	6'-1"	5'-11"	5'-8"	5'-5"	5'-3"	5'-1"	
3' x 2' x 0.070" Hollow	8'-3"	8'-3"	7'-11"	7'-7"	7'-4"	7'-1"	6'-11"	6'-8"	6'-7"	6'-5"	6'-3"	
2' x 3' x 0.045" Hollow	9'-7"	9'-1"	8'-7"	8'-1"	7'-8"	7'-4"	7'-0"	6'-7"	6'-5"	6'-3"	6'-1"	
2' x 2' x 0.050" Hollow	12'-5"	11'-9"	11'-0"	11'-0"	10'-7"	9'-8"	9'-4"	8'-11"	8'-8"	8'-4"	8'-0"	
2' x 3' x 0.050" S.A.B.	17'-2"	16'-4"	15'-7"	15'-0"	14'-3"	13'-7"	13'-0"	12'-6"	12'-3"	11'-8"	11'-4"	
2' x 6' x 0.050" S.A.B.	20'-1"	19'-1"	18'-3"	17'-5"	16'-5"	15'-9"	15'-1"	14'-6"	13'-11"	13'-6"	13'-2"	
2' x 6' x 0.060" S.A.B.	20'-1"	19'-1"	18'-3"	17'-5"	16'-5"	15'-9"	15'-1"	14'-6"	13'-11"	13'-6"	13'-2"	
2' x 2' x 0.044" Slump	13'-4"	9'-0"	8'-11"	8'-5"	8'-1"	7'-11"	7'-8"	7'-4"	7'-1"	6'-11"	6'-7"	
2' x 3' x 0.045" Slump	10'-1"	12'-5"	12'-0"	11'-11"	11'-3"	10'-8"	10'-2"	9'-9"	9'-4"	8'-11"	8'-6"	
2' x 2' x 0.045" Slump	13'-4"	9'-0"	8'-11"	8'-5"	8'-1"	7'-11"	7'-8"	7'-4"	7'-1"	6'-11"	6'-7"	
2' x 2' x 0.045" Slump	13'-4"	9'-0"	8'-11"	8'-5"	8'-1"	7'-11"	7'-8"	7'-4"	7'-1"	6'-11"	6'-7"	

For 3 second wind gust at 120 MPH velocity; using design load of 22 MSF

Sections	Thickened Load Width W_L = Parlin Spacing											
	3'-0"	3'-6"	4'-0"	4'-6"	5'-0"	5'-6"	6'-0"	6'-6"	7'-0"	7'-6"		
	Allowable Height H_A / bending M or deflection δ^d											
2' x 2' x 0.044" Hollow	6'-5"	6'-1"	5'-9"	5'-5"	5'-2"	4'-11"	4'-8"	4'-5"	4'-2"	4'-0"	4'-0"	
2' x 2' x 0.055" Hollow	6'-9"	6'-5"	6'-2"	5'-11"	5'-8"	5'-4"	5'-2"	4'-11"	4'-8"	4'-7"	4'-7"	
3' x 2' x 0.045" Hollow	7'-3"	6'-10"	6'-7"	6'-4"	6'-1"	5'-11"	5'-8"	5'-5"	5'-3"	5'-1"	5'-1"	
3' x 2' x 0.070" Hollow	8'-3"	7'-9"	7'-5"	7'-1"	6'-10"	6'-8"	6'-5"	6'-3"	6'-1"	5'-11"	5'-11"	
2' x 3' x 0.045" Hollow	6'-11"	6'-2"	5'-7"	5'-4"	5'-1"	6'-5"	6'-4"	6'-1"	5'-8"	5'-6"	5'-6"	
2' x 2' x 0.050" S.A.B.	16'-1"	15'-3"	14'-5"	13'-7"	12'-11"	12'-4"	11'-9"	11'-4"	10'-11"	10'-7"	10'-7"	
2' x 6' x 0.050" S.A.B.	18'-10"	17'-10"	16'-8"	15'-9"	14'-11"	14'-3"	13'-8"	13'-1"	12'-6"	12'-2"	12'-2"	
2' x 2' x 0.044" Slump	7'-2"	6'-10"	6'-4"	6'-3"	6'-1"	5'-10"	5'-7"	5'-5"	5'-2"	5'-0"	5'-0"	
2' x 3' x 0.045" Slump	12'-3"	11'-6"	10'-9"	10'-2"	9'-7"	7'-10"	7'-6"	7'-2"	6'-8"	6'-5"	6'-5"	
2' x 2' x 0.045" Slump	12'-3"	11'-6"	10'-9"	10'-2"	9'-7"	7'-10"	7'-6"	7'-2"	6'-8"	6'-5"	6'-5"	
2' x 2' x 0.045" Slump	12'-3"	11'-6"	10'-9"	10'-2"	9'-7"	7'-10"	7'-6"	7'-2"	6'-8"	6'-5"	6'-5"	

Table 3B.2.2 Allowable Upright Heights, Chair Rail Spans or Header Spans For Glass & Modular Rooms

Sections	Tributary Load Width 4° Purlin Spacing										
	3-0°	3-6°	4-0°	4-6°	5-0°	5-6°	6-0°	6-6°	7-0°	7-6°	
2" x 2" x 0.044"	Hollow	6-1°	5-10°	5-7°	5-4°	5-2°	5-0°	4-10°	4-7°	4-6°	
2" x 2" x 0.055"	Hollow	6-5°	6-2°	5-11°	5-8°	5-6°	5-4°	5-2°	5-0°	4-8°	
3" x 2" x 0.045"	Hollow	6-11°	6°	5-4°	6-1°	5-10°	5-6°	5-4°	5-3°	5-1°	
3" x 2" x 0.070"	Hollow	7-5°	7-5°	7-1°	6-1°	5-10°	6-4°	5-6°	5-10°	5-8°	
2" x 2" x 0.045"	Hollow	6-2°	6-2°	7-10°	7-6°	6-7°	6-10°	6-5°	6-5°	6-3°	
2" x 2" x 0.050"	Hollow	11-2°	10-7°	10-3°	9-9°	9-5°	9-1°	8-10°	8-7°	8-4°	
2" x 2" x 0.050"	S.M.B.	15-0°	14-7°	13-11°	13-5°	12-11°	12-7°	11-11°	11-7°	11-4°	
2" x 2" x 0.050"	S.M.B.	18-5°	17-1°	16-4°	15-6°	15-2°	14-9°	14-4°	13-11°	13-3°	
2" x 2" x 0.044"	Shap	6-11°	6-7°	6-5°	6-1°	5-10°	6-5°	5-8°	5-1°	5-2°	
2" x 2" x 0.045"	Shap	9-4°	8-10°	8-5°	8-1°	7-10°	7-7°	7-5°	7-2°	6-10°	
2" x 4" x 0.045"	Shap	11-9°	11-2°	10-8°	10-3°	9-11°	9-7°	9-4°	9-1°	8-10°	

Table 3B.3.1-110 Allowable Upright Heights, Chair Rail Spans or Header Spans - Hollow Extrusions

Sections	Trussway Load Width "w" = Eutim Spacing									
	Allowable Height "H" Landing Is or deflection "x"									
	3'-0"	3'-6"	4'-0"	4'-6"	5'-0"	5'-6"	6'-0"	7'-0"	7'-6"	
2" x 2" x 0.044" with Snap Cap Attached	8'-7" d	8'-2" d	7'-8" d	7'-5" b	7'-0" b	6'-8" b	6'-5" b	6'-2" b	5'-9" b	
1" x 2" x 0.044" Existing	8'-10" d	8'-4" d	8'-11" d	8'-6" b	8'-1" b	7'-8" b	7'-4" b	7'-1" b	6'-7" b	
2" x 2" x 0.044" Existing	12'-1" d	11'-5" b	10'-8" b	10'-1" b	9'-6" b	9'-1" b	8'-6" b	8'-4" b	8'-1" b	
2" x 2" x Existing (Lead to 2" x 2" Existing)	12'-1" d	12'-2" d	11'-7" d	11'-2" d	10'-6" d	10'-5" d	10'-2" d	9'-11" d	9'-8" d	
3" x 3" x 0.039" sq. Tube (Lead to 3" Existing)	13'-9" d	13'-1" d	12'-6" d	12'-0" d	11'-7" d	11'-3" d	10'-11" d	10'-7" d	10'-4" d	
1" x 3" x 0.044" Open Back (Lead to 3" Direction) ¹	13'-11" d	13'-3" d	12'-6" d	12'-2" d	11'-8" d	11'-5" d	11'-1" d	10'-10" d	10'-5" d	
3" x 3" x 0.039" sq. Tube (Lead to 3" Direction) ²	14'-7" d	13'-10" d	13'-5" d	12'-9" d	12'-4" d	11'-11" d	11'-7" d	11'-3" d	10'-9" d	
3" x 3" x 0.044" Open Back (Lead to 4" Direction) ³	15'-5" d	14'-7" d	13'-11" d	13'-5" d	12'-11" d	12'-7" d	12'-3" d	11'-11" d	11'-6" b	
3" x 3" x 0.039" sq. Tube (Lead to 4" Direction) ⁴	18'-10" d	17'-11" d	17'-1" d	16'-5" d	15'-10" d	15'-5" d	14'-11" d	14'-7" d	14'-2" d	
3" x 3" x 0.044" Open Back (Lead to 4" Direction) ⁵	18'-10" d	17'-11" d	17'-1" d	16'-5" d	15'-10" d	15'-5" d	14'-11" d	14'-7" d	14'-2" d	

Table 3B.3.1-120 Allowable Upright Heights, Chair Rail Spans or Header Spans - Hollow Extrusions For Screen Rooms Converted to Glass / Enclosed Rooms

Sections	Tribrachy Load Width w/ a Purlin Sizing									
	3'-0"	3'-6"	4'-0"	4'-6"	5'-0"	6'-0"	6'-6"	7'-0"	7'-6"	
$2" \times 2" \times 0.044$ with Snap Cap attached	8'-4" b	7'-9" b	7'-3" b	Allowable Height w/ Landing w/ or without deflection, d'	6'-10" b	6'-5" b	6'-2" b	5'-11" b	5'-8" b	
$1" \times 2" \times 0.044$	8'-2" d	8'-5" b	8'-2" b	7'-6" b	7'-3" b	6'-11" b	6'-8" b	6'-5" b	6'-2" b	
to $2" \times 2" \times$ Existing	11'-2" b	10'-4" b	9'-8" b	8'-1" b	8'-7" b	8'-3" b	7'-10" b	7'-7" b	7'-3" b	
$2" \times 2" \times 0.044$	11'-2" b	10'-4" b	9'-8" b	8'-1" b	8'-7" b	8'-3" b	7'-10" b	7'-7" b	7'-3" b	
to $2" \times 2" \times$ Existing	11'-11" d	11'-4" d	10'-10" d	10'-5" d	10'-1" d	9'-6" d	9'-3" d	8'-9" d	8'-5" d	
$3" \times 3" \times 0.093$ Sq. Tube	12'-10" d	12'-3" d	11'-8" d	11'-3" d	10'-10" d	10'-6" d	10'-2" d	9'-11" d	9'-8" d	
$1" \times 3" \times 0.044$ Open Beek (Lead to 3' Direction)	13'-1" d	12'-5" d	11'-10" d	11'-5" d	11'-0" d	10'-6" d	10'-4" d	10'-1" d	9'-7" d	
$3" \times 3" \times 0.093$ Sq. Tube	13'-8" d	12'-11" d	12'-6" d	11'-11" d	11'-6" d	11'-2" d	10'-10" d	10'-7" d	10'-3" d	
$1" \times 3" \times 0.044$ Open Beek (Lead to 4' Direction)	14'-5" d	13'-8" d	13'-1" d	12'-7" d	12'-2" d	11'-9" d	11'-5" b	10'-9" b	10'-4" b	
$1" \times 3" \times 0.044$ Open Beek (Lead to 4' Direction)	17'-7" d	16'-9" d	15'-11" d	15'-5" d	14'-10" d	14'-5" d	13'-11" d	13'-7" d	13'-3" d	

Table 3B.3.1-130 Allowable Upright Heights, Chair Rail Spans or Header Spans - Hollow Extrusions

Sections	Tributary Load Width "W" = Purlin Spacing									
	Allowable Height "H" Landing LV or deflection "d"									
	3'-0"	3'-6"	4'-0"	4'-6"	5'-0"	5'-6"	6'-0"	6'-6"	7'-0"	7'-6"
2" x 2" x 0.044" - With Snap Cap Attached	8'-2" d	7'-7" b	7'-1" b	6'-8" b	6'-4" b	6'-0" b	5'-6" b	5'-2" b	5'-4" f	5'-2" f
1" x 2" x 0.044" - to 2" x 2" x Existing	8'-5" b	7'-10" b	7'-3" b	6'-10" b	6'-6" b	6'-3" b	5'-11" b	5'-9" b	5'-6" b	5'-4" f
2" x 2" x 0.044" - to 2" x 2" x Existing	8'-2" b	8'-5" b	7'-11" b	7'-6" b	7'-1" b	6'-8" b	6'-2" b	5'-11" b	5'-8" f	5'-6" f
3" x 3" x 0.093" Sq. Tube (Load to 3" Direction) ¹	11'-1" d	10'-6" d	10'-1" d	9'-8" d	9'-4" d	8'-9" d	8'-5" b	8'-1" b	7'-10" f	7'-8" f
1" x 3" x 0.044" Open Beek (Load to 3" Direction) ²	11'-11" d	11'-4" d	10'-10" d	10'-5" d	10'-1" d	9'-8" d	9'-3" d	8'-9" d	8'-5" f	8'-1" f
3" x 3" x 0.125" Sq. Tube (Load to 4" Direction) ¹	12'-2" d	11'-7" d	11'-1" d	10'-7" d	10'-3" d	9'-11" d	9'-6" d	9'-2" d	8'-11" f	8'-9" f
1" x 3" x 0.044" Open Beek (Load to 4" Direction) ²	12'-8" d	12'-1" d	11'-6" d	11'-1" d	10'-8" d	10'-4" d	10'-1" d	9'-10" d	9'-7" d	9'-4" f
1" x 3" x 0.083" to (1) 1" x 3" x 0.044" Open Beek (Load to 4" Direction) ²	13'-5" d	12'-9" d	12'-2" d	11'-7" b	11'-0" b	10'-6" b	10'-1" b	9'-6" b	8'-4" b	8'-0" f
16'-4" d	15'-6" d	14'-10" d	14'-3" d	13'-10" d	13'-4" d	12'-11" d	12'-8" d	12'-4" d	12'-1" f	

Table 3B.3.1-140A Allowable Upright Heights, Chair Rail Spans or Header Spans - Hollow Extrusions

Sections	Tributary Load Width "w" = Purlin Spacing									
	3'-0"	3'-6"	4'-0"	4'-6"	5'-0"	5'-6"	6'-0"	6'-6"	7'-0"	7'-6"
	Allowable Height, "h" Landing to or below eave, "d"									
2" x 2" x 0.044" with Snap cap attached	7'-2" b	6'-7" b	6'-2" b	5'-10" b	5'-6" b	5'-3" b	5'-1" b	4'-10" b	4'-6" b	4'-6" b
1" x 2" x 0.044"	8'-2" b	7'-7" b	7'-1" b	6'-8" b	6'-4" b	6'-1" b	5'-9" b	5'-7" b	5'-4" b	5'-2" b
to 2" x 2" Existing										
2" x 2" x 0.044"	8'-8" b	8'-1" b	8'-5" b	7'-11" b	7'-6" b	7'-2" b	6'-10" b	6'-7" b	6'-4" b	6'-2" b
to 2" x 2" Existing										
3" x 3" x 0.083" Sq. Tube	10'-11" d	10'-4" d	9'-11" d	9'-6" d	9'-2" d	8'-11" d	8'-6" b	8'-2" b	7'-11" b	7'-8" b
to 3" x 3" x 0.083" Sq. Tube										
1" x 3" x 0.044" Open Beek (Load to 3" Direction) ¹	11'-9" d	11'-2" d	10'-8" d	10'-3" d	9'-11" d	9'-7" d	9'-4" d	9'-1" d	8'-10" b	8'-7" b
3" x 3" x 0.075" Sq. Tube (Load to 3" Direction) ²	11'-11" d	11'-4" d	10'-10" d	10'-5" d	10'-1" d	9'-9" d	9'-5" d	9'-2" d	8'-11" d	8'-8" d
1" x 3" x 0.044" Open Beek (Load to 4" Direction) ³	12'-5" d	11'-10" d	11'-4" d	10'-10" d	10'-6" d	10'-2" d	9'-11" d	8'-7" d	8'-5" d	8'-2" d
1" x 3" x 0.044" Open Beek (Load to 4" Direction) ⁴	13'-2" d	12'-6" d	11'-11" d	11'-5" b	10'-8" b	10'-2" b	9'-6" b	9'-5" b	9'-1" b	8'-8" b
16'-1" d	15'-3" d	14'-7" d	14'-0" d	13'-6" d	13'-1" d	12'-9" d	12'-5" d	12'-1" d	11'-10" d	

Table 3B.4 Schedule of Post to Beam Size
Aluminum Alloy 6063 T-6

Beam Size	Minimum Post Size	Alternate Post Size	# Thrust-Bolts @ L=1-1/2" Min	Minimum Knee Brace ^a	Min. # Knee Brace Screws
2" x 4" x 0.050" Hollow	3" x 3" x 0.085"	2" x 3" x 0.050"	1/4" 3/8" ø	2" x 3" x 0.050"	(3) #8
Self-Mating Beams					
2" x 4" x 0.040" x 0.100"	3" x 3" x 0.093"	2" x 3" x 0.050"	2	2" x 3" x 0.050"	(3) #8
2" x 5" x 0.050" x 0.100"	3" x 3" x 0.093"	2" x 3" x 0.050"	2	2" x 3" x 0.050"	(3) #8
2" x 6" x 0.050" x 0.120"	3" x 3" x 0.093"	2" x 3" x 0.050"	(3) #10	2" x 3" x 0.050"	(3) #10
2" x 7" x 0.055" x 0.120"	3" x 3" x 0.093"	2" x 3" x 0.050"	2	2" x 3" x 0.050"	(3) #10
2" x 7" x 0.055" w/insert	3" x 3" x 0.093"	2" x 3" x 0.050"	2	2" x 3" x 0.050"	(3) #10
2" x 8" x 0.070" x 0.120"	3" x 3" x 0.093"	2" x 4" x 0.050"	3	2" x 4" x 0.050"	(3) #12
2" x 8" x 0.070" x 0.124"	3" x 3" x 0.093"	2" x 4" x 0.050" x 0.100"	3	2" x 5" x 0.050" x 0.100"	(3) #14
2" x 9" x 0.082" x 0.136"	3" x 3" x 0.125"	2" x 5" x 0.050" x 0.120"	4	2" x 6" x 0.050" x 0.120"	(4) #14
2" x 10" x 0.092" x 0.136"	3" x 3" x 0.125"	2" x 7" x 0.055" x 0.120"	5	2" x 7" x 0.055" x 0.120"	(5) #14
Double Self-Mating Beams					
2" x 3" x 0.075" x 0.124"	2" x 5" x 0.050" x 0.100"	-	6	2" x 4" x 0.044" x 0.100"	(6) #14
2" x 3" x 0.077" x 0.124"	2" x 6" x 0.050" x 0.120"	-	6	2" x 6" x 0.050" x 0.120"	(6) #14
2" x 3" x 0.082" x 0.136"	2" x 7" x 0.055" x 0.120"	-	8	2" x 6" x 0.050" x 0.120"	(8) #14
2" x 10" x 0.092" x 0.136"	2" x 8" x 0.077" x 0.124"	-	10	2" x 7" x 0.055" x 0.120"	(10) #14

Table 3B.5.1 Allowable Spans for Industry Standard Composite Roof Panels for Various Loads for 3" Modular Rooms

Aluminum Alloy 3105 H-14 or H-25 1.0 EFS Core Density Foam 3" x 48" x 0.024" Panels									
Wind Region	Enclosed			Overhang/ Cantilever	All Region	3" x 48" x 0.030" Panels			
	162	3	4			162	3	4	Overhang/ Cantilever
100 MPH	11'-6"	13'-6"	13'-3"	4'-0"	100 MPH	14'-6"	16'-2"	15'-8"	4'-0"
110 MPH	10'-6"	12'-6"	11'-5"	4'-0"	110 MPH	13'-3"	14'-3"	14'-3"	4'-0"
120 MPH	9'-6"	10'-10"	10'-8"	4'-1"	120 MPH	11'-5"	13'-5"	13'-0"	4'-0"
130 MPH	9'-0"	10'-1"	10'-3"	3'-11"	130 MPH	11'-2"	13'-1"	12'-8"	4'-0"
140 MPH	8'-4"	9'-4"	9'-0"	2'-11"	140 MPH	10'-8"	12'-5"	11'-6"	4'-0"
150 MPH	7'-5"	8'-6"	8'-4"	2'-10"	150 MPH	9'-11"	11'-1"	10'-8"	4'-1"

Note: Total roof panel width = room width + wall width + overhang.

Table 3B.5.2 Allowable Spans for Industry Standard Composite Wall Panels for Various Loads for 3" Modular Rooms

Aluminum Alloy 3105 H-14 or H-25 1.0 EFS Core Density Foam Aluminum Alloy 3105 H-14 or H-25 Statewide Product Approval #P1.1049									
Wind Condition (MPH)	Upright Height for Various Load Conditions					3" x 48" x 0.030" Panels			
	100	110	120	130	140 A & B	162	3	4	Overhang/ Cantilever
Applied Load	15 #/Sq. Ft. 18 #/Sq. Ft. 22 #/Sq. Ft. 25 #/Sq. Ft. 28 #/Sq. Ft. 34 #/Sq. Ft.	18 #/Sq. Ft. 22 #/Sq. Ft. 25 #/Sq. Ft. 28 #/Sq. Ft. 34 #/Sq. Ft.	22 #/Sq. Ft. 25 #/Sq. Ft. 28 #/Sq. Ft. 34 #/Sq. Ft.	25 #/Sq. Ft. 28 #/Sq. Ft. 34 #/Sq. Ft.	28 #/Sq. Ft. 34 #/Sq. Ft.	15 #/Sq. Ft. 18 #/Sq. Ft. 22 #/Sq. Ft. 25 #/Sq. Ft. 28 #/Sq. Ft.	18 #/Sq. Ft. 22 #/Sq. Ft. 25 #/Sq. Ft. 28 #/Sq. Ft.	22 #/Sq. Ft. 25 #/Sq. Ft. 28 #/Sq. Ft.	25 #/Sq. Ft. 28 #/Sq. Ft.
Per Linear Foot of Wall *	14'-2"	12'-11"	11'-5"	10'-11"	10'-2"	14'-2"	12'-11"	11'-5"	10'-2"
16'-6"	15'-3"	13'-10"	12'-11"	12'-0"	11'-1"	16'-6"	15'-3"	13'-10"	12'-0"

* Engineering properties of panel are in linear feet of wall.

Table 3B.5.3 Allowable Wall / Upright Heights for Components of Complete Standard Room 3" Panel System for Enclosed Rooms

Upright Height for Various Load Conditions									
Wind Condition (MPH)	100	110	120	130	140 A & B	150	162	3	4
Applied Load	15 #/Sq. Ft. 18 #/Sq. Ft. 22 #/Sq. Ft. 25 #/Sq. Ft. 28 #/Sq. Ft. 34 #/Sq. Ft.	18 #/Sq. Ft. 22 #/Sq. Ft. 25 #/Sq. Ft. 28 #/Sq. Ft. 34 #/Sq. Ft.	22 #/Sq. Ft. 25 #/Sq. Ft. 28 #/Sq. Ft. 34 #/Sq. Ft.	25 #/Sq. Ft. 28 #/Sq. Ft. 34 #/Sq. Ft.	28 #/Sq. Ft. 34 #/Sq. Ft.	15 #/Sq. Ft. 18 #/Sq. Ft. 22 #/Sq. Ft. 25 #/Sq. Ft. 28 #/Sq. Ft.	18 #/Sq. Ft. 22 #/Sq. Ft. 25 #/Sq. Ft. 28 #/Sq. Ft.	22 #/Sq. Ft. 25 #/Sq. Ft. 28 #/Sq. Ft.	25 #/Sq. Ft. 28 #/Sq. Ft.
Load Width	13'-11"	12'-8"	11'-5"	10'-9"	10'-0"	9'-3"	13'-11"	12'-8"	11'-5"
12"	8'-10"	8'-11"	8'-2"	7'-6"	7'-1"	6'-7"	8'-10"	8'-11"	8'-2"
24"	6'-11"	6'-4"	5'-9"	5'-5"	5'-0"	4'-8"	6'-11"	6'-4"	5'-9"
72"	5'-8"	5'-2"	4'-5"	4'-5"	4'-1"	3'-9"	5'-8"	5'-2"	4'-5"
Load Width	3" x 3" x 0.050" Extruded Corner Post 6063 T-6 Alloy	19'-0"	17'-2"	16'-1"	14'-11"	13'-10"	3" x 3" x 0.050" Extruded Corner Post 6063 T-6 Alloy	19'-0"	17'-2"
12"	20'-10"	19'-0"	17'-2"	16'-1"	14'-11"	13'-10"	20'-10"	19'-0"	17'-2"
24"	14'-9"	13'-5"	12'-2"	11'-5"	10'-7"	9'-8"	14'-9"	13'-5"	12'-2"
48"	10'-5"	9'-6"	8'-7"	8'-1"	7'-6"	6'-11"	10'-5"	9'-6"	8'-7"
72"	8'-9"	7'-9"	7'-0"	6'-7"	6'-1"	5'-8"	8'-9"	7'-9"	7'-0"
Load Width	2" x 3" x 0.050" Matted Extrusion 6063 T-6 Alloy	19'-2"	17'-11"	16'-2"	14'-1"	13'-0"	2" x 3" x 0.050" Matted Extrusion 6063 T-6 Alloy	19'-2"	17'-11"
12"	19'-7"	17'-11"	16'-2"	15'-2"	14'-1"	13'-0"	19'-7"	17'-11"	16'-2"
24"	13'-10"	12'-8"	11'-5"	10'-9"	9'-11"	9'-3"	13'-10"	12'-8"	11'-5"
48"	9'-10"	8'-11"	8'-1"	7'-7"	7'-1"	6'-5"	9'-10"	8'-11"	8'-1"
72"	8'-0"	7'-4"	6'-7"	6'-2"	5'-9"	5'-4"	8'-0"	7'-4"	6'-7"
Load Width	2" x 3" x 0.050" Matted Extrusion with Male Adaptor (1) Side 6063 T-6 Alloy	22'-7"	20'-8"	18'-5"	16'-3"	15'-0"	2" x 3" x 0.050" Matted Extrusion with Male Adaptor (1) Side 6063 T-6 Alloy	22'-7"	20'-8"
12"	18'-11"	14'-7"	13'-2"	12'-5"	11'-6"	10'-7"	18'-11"	14'-7"	13'-2"
24"	11'-4"	10'-4"	9'-4"	8'-9"	7'-6"	7'-6"	11'-4"	10'-4"	9'-4"
72"	8'-3"	8'-5"	7'-7"	7'-2"	6'-5"	6'-2"	8'-3"	8'-5"	7'-7"
Load Width	2" x 3" x 0.050" Matted Extrusion with Male Adaptor (2) Side 6063 T-6 Alloy	25'-3"	23'-1"	20'-10"	19'-7"	18'-2"	2" x 3" x 0.050" Matted Extrusion with Male Adaptor (2) Side 6063 T-6 Alloy	25'-3"	23'-1"
12"	17'-10"	16'-4"	14'-8"	13'-10"	12'-10"	11'-10"	17'-10"	16'-4"	14'-8"
24"	12'-5"	11'-6"	10'-5"	9'-9"	9'-1"	8'-5"	12'-5"	11'-6"	10'-5"
72"	10'-4"	9'-5"	8'-5"	7'-11"	7'-5"	6'-10"	10'-4"	9'-5"	8'-5"

Note: Room projection may not exceed 16" from host structure.

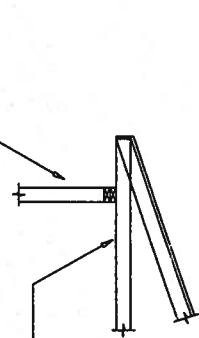
Example: For 2" x 3" x 0.050" Matted Extrusions; Load Width = Upright Spacing, use Load Width to enter table. For upright spacing of Load Width = 48" enter table on left @ 48" and read span under appropriate load. For corner post use 1/2 upright spacing. Thus: For 100 M.P.H. Wind Load or Live Load of 15 #/Sq. Ft. and Load Width = 48"; Allowable Upright Height for a 2" x 3" x 0.050" Matted Extrusion: 9'-10"

3B.8 Anchor Schedule for Composite Panel Room Components

Connection Description	80 - 100 MPH	110 - 130 MPH	140 - 150 MPH
Receiving channel to roof panel at front wall or at the receiving channel. 0.024" or 0.030" metal	#10 x (T*+1/2") SMS 1 @ 6" from each side of the panel and 1 @ 12" O.C.	#10 x (T*+1/2") SMS 1 @ 6" from each side of the panel and 1 @ 8" O.C.	#10 x (T*+1/2") SMS 1 @ 6" from each side of the panel and 1 @ 6" O.C.
Receiving channel to wood deck at front wall. #2 pine framing	1 @ 6" from each end of receiving channel and 1 @ 24" O.C.	2 @ 6" from each end of receiving channel and 2 @ 24" O.C.	2 @ 6" from each end of receiving channel and 2 @ 24" O.C.
Receiving channel to concrete deck at front wall. 2,500 psi concrete headers and other wall connections	1 @ 6" from each end of receiving channel and 1 @ 32" O.C.	1 @ 6" from each end of receiving channel and 1 @ 24" O.C.	2 @ 6" from each end of receiving channel and 2 @ 24" O.C.
Receiving channel to existing concrete beam, masonry wall, slab, foundation, host structure, or infill connected to concrete.	#8 x 3/4" SMS 1 @ 6" from each end of component and 1 @ 48" O.C.	#10 x 3/4" SMS 1 @ 4" from each end of component and 1 @ 24" O.C.	3/8" x 1-1/2" Tapcon 1 @ 3" from each end of component and 1 @ 24" O.C.
Roof Panel to top of wall	1 @ 6" from each end of component & 1 @ 12" O.C.	1 @ 4" from each end of component & 1 @ 8" O.C.	1 @ 3" from each end of component & 1 @ 6" O.C.
a. To wood	#10 x T* + 1-1/2"	#10 x T* + 1-1/2"	#10 x T* + 1-1/2"
b. To 0.05" aluminum	#10 x T* + 1/2"	#10 x T* + 1/2"	#10 x T* + 1/2"

Notes:
1. The anchor schedule above is for mean roof height of 0-20', enclosed structure, exposure "B", 1 = 1.0, maximum from wall projection from host structure of 16", with maximum overhang of 2', and 10' wall height. There is no restriction on room length. For structures exceeding this criteria consult the engineer.
2. Anchors through receiving channel into roof panels, wood, or concrete / masonry shall be staggered side to side at the required spacing.
3. Wood deck materials are assumed to be #2 pressure treated pine. For spruce, pine or fir decrease spacing of anchors by 0.75. Reduce spacing of anchors for "C" exposure by 0.83.
4. Concrete is assumed to be 2,500 psi @ 7 days minimum. For concrete strength other than 2,500 psi consult the engineer. Reduce anchor spacing for "C" exposure by 0.83.
5. Tapcon or equal masonry anchor may be used, allowable rating (not ultimate) must meet or exceed 4,114 for 1-1/2" embedment at minimum 5d spacing from concrete edge to center of anchor. Roof anchors shall require 1-1/4" binder washer.

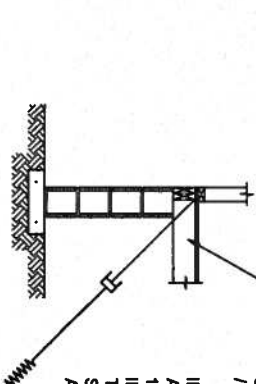
REMOVE VINYL SIDING AND SOFFIT ON THE WALL AND INSTALL SIMPSON CS-16 COIL STRAP OR EQUAL FROM TRUSS / RAFTER TO BOTTOM OF DOUBLE TOP PLATE JOIST @ EACH TRUSS / RAFTER



THE FLOOR, WALL, AND ROOF SYSTEM ARE THAT OF MOBILE / MANUFACTURED HOME

THE FLOOR, WALL, AND ROOF SYSTEM ARE THAT OF MOBILE / MANUFACTURED HOME

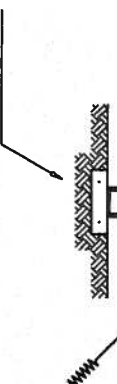
NAIL STRAP W/ 16d COMMON @ TRUSS RAFTER AND PERIMETER JOIST SCREW COIL STRAP TO SHEATHING W/ #8 x 1" DECK SCREWS @ 16" O.C. VERTICALLY REPLACE VINYL SIDING



THE FLOOR, WALL, AND ROOF SYSTEM ARE THAT OF MOBILE / MANUFACTURED HOME

INSTALL NEW 48" OR 60" AUGER ANCHOR PER RULE 15C @ EACH NEW PIER. INSTALL 1/2" CARRIAGE BOLT THRU PERIMETER JOIST AND STRAP TO NEW AUGER ANCHOR

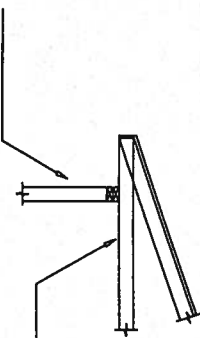
PROVIDE NEW 4" x 6" OR 8" x 16" CMU PIER AND SOLID FOUNDATION BLOCK @ 5'-0" MAX. O.C. ALONG ATTACHMENT WALL



TYPICAL WALL SECTION FOR ATTACHMENT TO MOBILE / MANUFACTURED HOME

SCALE: 1/4" = 1'-0"

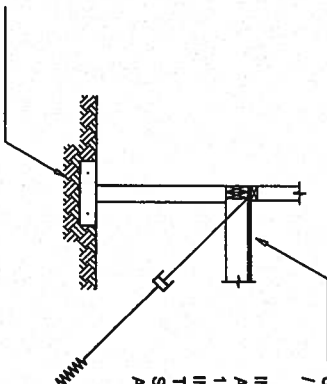
REMOVE VINYL SIDING AND SOFFIT ON THE WALL AND INSTALL SIMPSON CS-16 COIL STRAP OR EQUAL FROM TRUSS / RAFTER TO BOTTOM OF DOUBLE TOP PLATE JOIST @ EACH TRUSS / RAFTER



THE FLOOR, WALL, AND ROOF SYSTEM ARE THAT OF MOBILE / MANUFACTURED HOME

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INSTALL NEW 48" OR 60" AUGER ANCHOR PER RULE 15C @ EACH NEW PIER. INSTALL 1/2" CARRIAGE BOLT THRU PERIMETER JOIST AND STRAP TO NEW AUGER ANCHOR

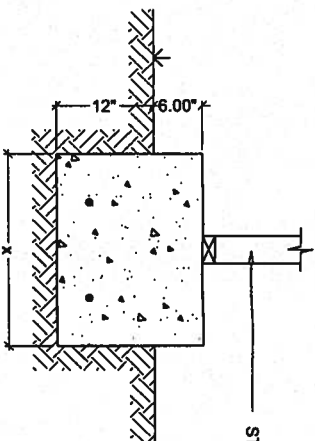
ALTERNATE:
4" x 4" P.T.P. POST W/ SIMPSON 4" x 4" POST BUCKET INSTALLED PER MANUFACTURERS SPECIFICATIONS TOP & BOTTOM



ALTERNATE WALL SECTION FOR ATTACHMENT TO MOBILE / MANUFACTURED HOME

SCALE: 1/4" = 1'-0"

STUD WALL OR POST



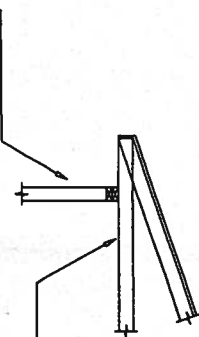
RIBBON FOOTING

SCALE: 1/2" = 1'-0"

Wind Zone	# / Sq. Ft.	x	Post Anchor @ 48" O.C.	Stud Anchors
100 - 123	+ 20 - 23	1.1"	ABU 44	SP1 @ 16" O.C.
130 - 140A	+ 30 - 37	1.4"	ABU 44	SP1 @ 16" O.C.
140B - 150	+ 30 - 33	1.5"	CBQ44SDS2	SP1 @ 16" O.C.

* Maximum 16" projection from host structure.
* For stud walls use 1/2" x 8" L-bolts @ 48" O.C. and 2" square washers to attach sole plate to footing. Stud anchors shall be at the sole plate only and coil strap shall lap over the top plate on to the studs anchors and straps shall be per manufacturers specifications.

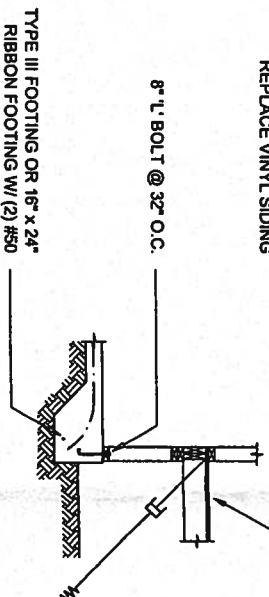
REMOVE VINYL SIDING AND SOFFIT ON THE WALL AND INSTALL SIMPSON CS-16 COIL STRAP OR EQUAL FROM TRUSS / RAFTER TO BOTTOM OF DOUBLE TOP PLATE JOIST @ EACH TRUSS / RAFTER



THE FLOOR, WALL, AND ROOF SYSTEM ARE THAT OF MOBILE / MANUFACTURED HOME

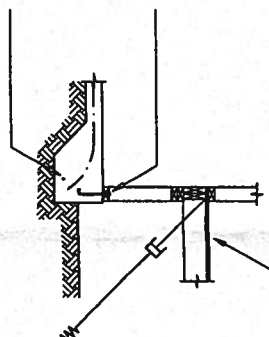
THE FLOOR, WALL, AND ROOF SYSTEM ARE THAT OF MOBILE / MANUFACTURED HOME

NAIL STRAP W/ 16d COMMON @ TRUSS RAFTER AND PERIMETER JOIST SCREW COIL STRAP TO SHEATHING W/ #8 x 1" DECK SCREWS @ 16" O.C. VERTICALLY REPLACE VINYL SIDING



THE FLOOR, WALL, AND ROOF SYSTEM ARE THAT OF MOBILE / MANUFACTURED HOME

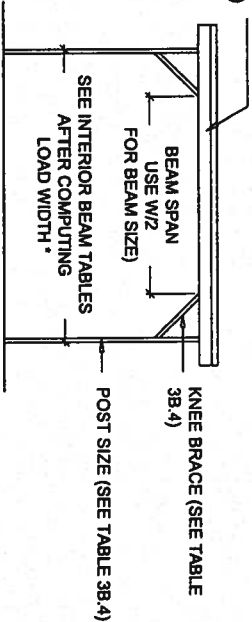
TYPE III FOOTING OR 16" x 24" RIBBON FOOTING W/ (2) #30 BARS, 2,500 PSI CONCRETE



ALTERNATE WALL SECTION FOR ATTACHMENT TO MOBILE / MANUFACTURED HOME

SCALE: 1/4" = 1'-0"

INTERIOR BEAM (SEE TABLE 3B.1.4)



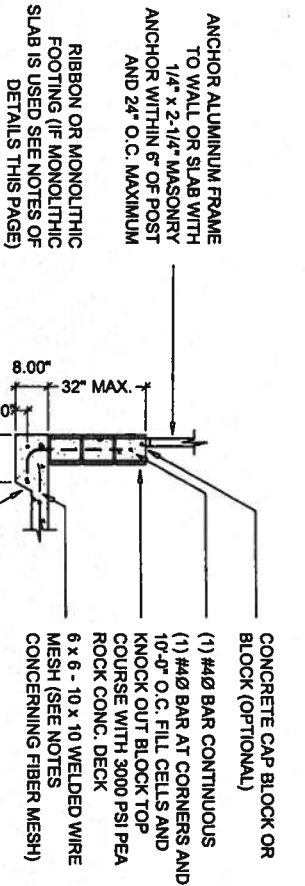
* LOAD WIDTH IS 1/2 THE DISTANCE BETWEEN SUPPORTS ON EITHER SIDE OF THE BEAM OR SUPPORT BEING CONSIDERED

TYPICAL SECTION "FOURTH" WALL FOR ADDITIONS ADJACENT TO A MOBILE/MANUFACTURED HOME

SCALE: 1/8" = 1'-0"

Extrusion Sizing Table:

Upright Size	Max. Beam Size	Knee Brace
2" x 2" x 0.036"	2" x 4"	2" x 2" x 0.036"
3" x 2" x 0.036"	2" x 4"	2" x 3" x 0.036"
3" x 3" x 0.036"	2" x 6" S.M.B.	2" x 3" x 0.036"
3" x 3" x 0.036"	2" x 6" S.M.B.	2" x 3" x 0.036"
3" x 3" x 0.125"	2" x 6" S.M.B.	3" x 3" x 0.036"

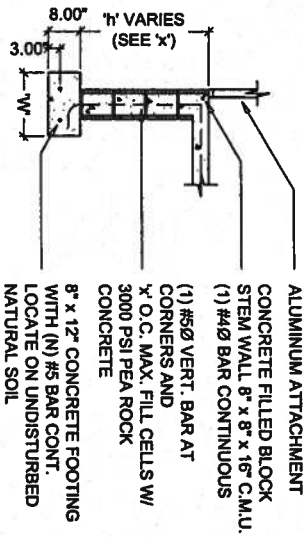


8" x 8" x 16" BLOCK WALL (MAX. 32")

KNEE WALL FOOTING FOR SCREENED OR GLASS ROOMS

SCALE: 1/4" = 1'-0"

h'	w'	N	N'
32"	12"	2	10'-0"
36"	12"	2	8'-0"
48"	18"	3	4'-0"



ALL MASONRY KNEE WALLS SHALL HAVE A FILLED CELL AND VERTICAL BAR @ ALL CORNERS

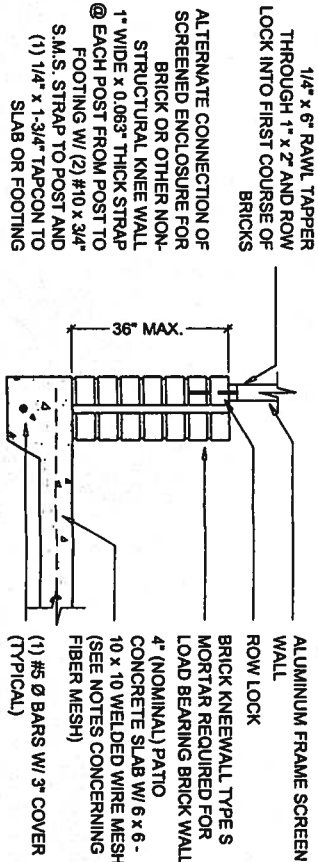
Notes:

- 3-1/2" concrete slab with 6 x 6 - 10 x 10 welded wire mesh or crack control fiber mesh. Fibermesh @ Mesh, Inforce™ #3™ (Formerly Fibermesh MD) per manufacturer's specification may be used in lieu of wire mesh. Visqueen vapor barrier under slabs having structures above compacted clean fill over (scarified) natural soil 90% density.
- Local code footing requirements shall be used in of the minimum footings shown. Orange County footings shall be a minimum of 12" x 16" with (2) #5 Ø continuous bars for structures / buildings over 400 sq. ft..

RAISED PATIO FOOTING

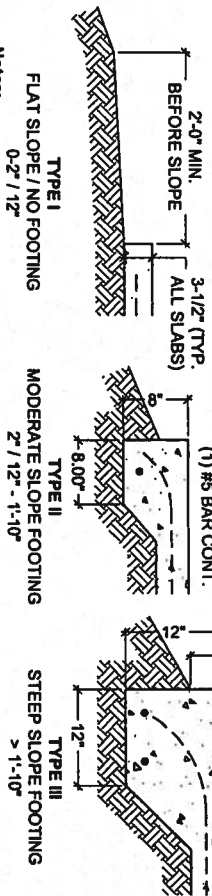
KNEE WALL FOOTING FOR SCREENED OR GLASS ROOMS

SCALE: 1/4" = 1'-0"



BRICK KNEE WALL AND FOUNDATION FOR SCREEN WALLS

SCALE: 1/2" = 1'-0"



Notes:

- The foundations shown are based on a minimum soil bearing pressure of 1,500 psf. Bearing capacity of soil shall be verified, prior to placing the slab, by field soil test or a soil testing lab.
- The slab / foundation shall be cleared of debris, roots, and compacted prior to placement of concrete.
- No footing other than 3-1/2" (4" nominal) slab is required except when addressing erosion until the projection from the host structure of the carport or patio cover exceeds 20'-0". Then a minimum of a Type II footing is required. All slabs shall be 3-1/2" (4" nominal) thick.
- Monolithic slabs and footings shall be minimum 2,500 psi concrete with 6 x 6 - 10 x 10 welded wire mesh or crack control fiber mesh. Fibermesh @ Mesh, Inforce™ #3™ (Formerly Fibermesh MD) per manufacturer's specification may be used in lieu of wire mesh.
- if local building codes require a minimum footing use Type II footing or footing section required by local code. Local code governs. (See additional detail for structures located in Orange County, FL.)
- If a carrier beam or fourth wall frame is required use a Type II footing minimum.

SLAB-FOOTING DETAILS

SCALE: 1/2" = 1'-0"