



Project Information for: L235554F1

Builder: Lipscomb and Eagle Development Inc.
Lot : 112-1
Subdivision: Preserve at Laurel Lake
County: Columbia
Truss Count: 2
Design Program: MiTek 20/20 6.3

Truss Design Load Information:

Gravity: **Wind:**

Building Code:FBC2004/TPI2002

Roof (psf):42.0 Wind Standard:ASCE 7-02
Floor (psf):55.0 Wind Speed (mph):110

Note: See the individual truss drawings for special loading conditions.

Engineer of Record: James M. Lipscomb Florida P.E. License No. CBC1253543

Address: 255 Southwest Woods Terrace Lake City, Florida 32025

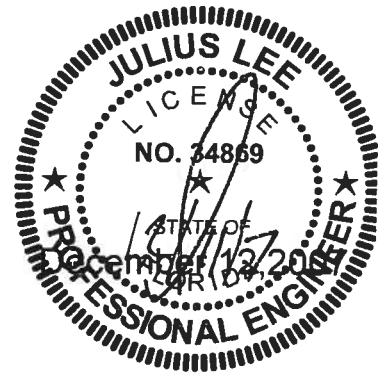
Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elements in the web and chord planes. See Florida Administrative Code 61G15-31.003 section 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

#	Truss ID	Dwg. #	Seal Date
1	F01	J1917170	12/12/07
2	F02KW	J1917171	12/12/07



Job	Truss	Truss Type	Qty	Ply	0 0	J1917170
L235554F	F01	FLOOR	1	3	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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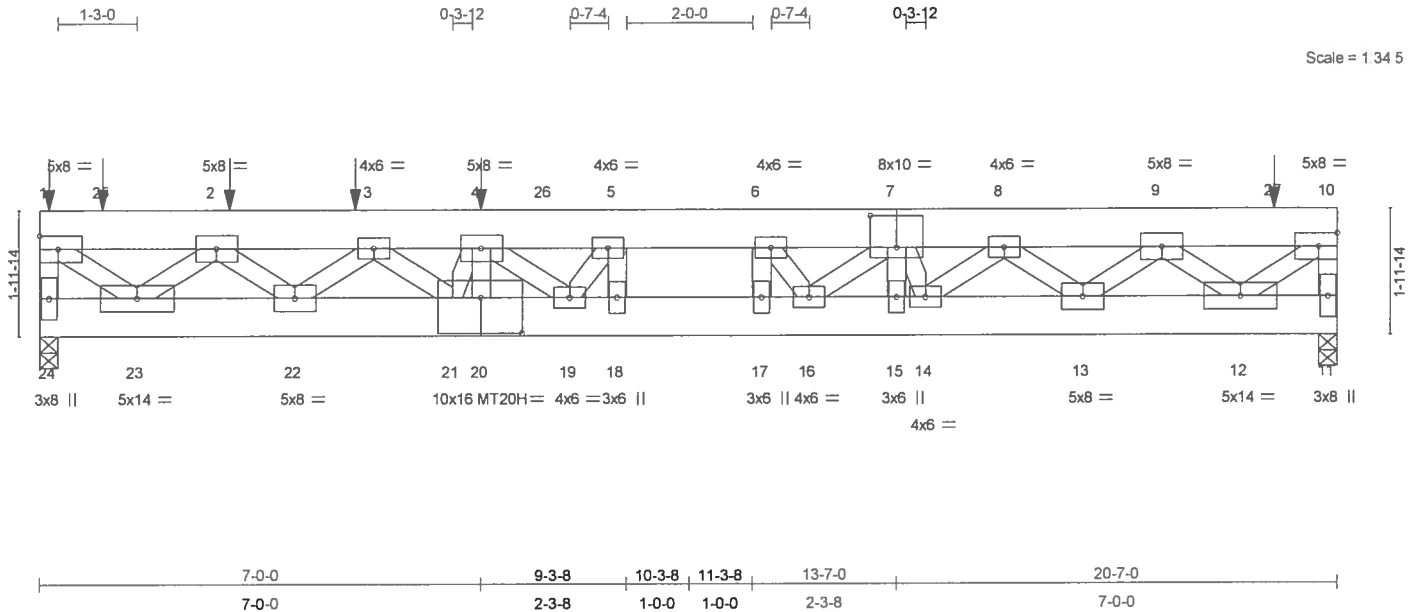


Plate Offsets (X,Y): [7:0-5-0,0-6-0], [20:0-7-12,0-6-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCCL 40.0	Plates Increase	1.00	TC 0.82	Vert(LL)	-0.28 17-18	>869	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.74	Vert(TL)	-0.52 17-18	>467	240	MT20H	187/143
BCLL 0.0	Rep Stress Incr	NO	WB 0.76	Horz(TL)	0.09 11	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 496 lb

LUMBER

TOP CHORD 2 X 8 SYP 2400F 2.0E
 BOT CHORD 2 X 8 SYP 2400F 2.0E
 WEBS 2 X 4 SYP No.2 *Except*
 4-20 2 X 4 SYP No.3, 5-18 2 X 4 SYP No.3
 6-17 2 X 4 SYP No.3, 7-15 2 X 4 SYP No.3
 4-21 2 X 4 SYP No.3, 5-19 2 X 4 SYP No.3
 6-16 2 X 4 SYP No.3, 7-14 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 24=8670/0-3-8, 11=10403/0-3-8

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-24=-7585/0, 10-11=-9040/0, 1-25=-7026/0, 2-25=-7026/0, 2-3=-18913/0, 3-4=-28209/0, 4-26=-31857/0, 5-26=-31857/0, 5-6=-32875/0, 6-7=-31950/0, 7-8=-28759/0, 8-9=-20627/0, 9-27=-8108/0, 10-27=-8108/0
 BOT CHORD 23-24=0/887, 22-23=0/13478, 21-22=0/24139, 20-21=0/29491, 19-20=0/29491, 18-19=0/32875, 17-18=0/32875, 16-17=0/32875, 15-16=0/29794, 14-15=0/29794, 13-14=0/25519, 12-13=0/15569, 11-12=0/1143
 WEBS 4-20=-1008/0, 5-18=-948/707, 6-17=-1001/663, 7-15=-688/0, 1-23=0/8649, 2-23=-9631/0, 2-22=0/8115, 3-22=-7800/0, 3-21=0/6076, 4-21=-4083/0, 4-19=0/3332, 5-19=-2666/304, 7-16=0/3037, 6-16=-2553/440, 10-12=0/9814, 9-12=-11138/0, 9-13=0/7550, 8-13=-7302/0, 8-14=0/4838, 7-14=-3294/0

JOINT STRESS INDEX

1 = 0.79, 2 = 0.91, 3 = 0.94, 4 = 0.58, 5 = 0.25, 6 = 0.25, 7 = 0.80, 8 = 0.94, 9 = 0.91, 10 = 0.79, 11 = 0.74, 12 = 0.87, 13 = 0.79, 14 = 0.94, 15 = 0.14, 16 = 0.51, 17 = 0.14, 18 = 0.14, 19 = 0.51, 20 = 0.96, 21 = 0.00, 22 = 0.79, 23 = 0.87 and 24 = 0.74

December 12, 2007

Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE
 This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	0 0	J1917170
L235554F	F01	FLOOR	1	3	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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NOTES

- 1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 8 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced floor live loads have been considered for this design.
- 4) All plates are MT20 plates unless otherwise indicated.
- 5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S) Standard Except:

- 1) Floor: Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 11-24=-10, 1-26=-421(F=-201), 26-27=-936(F=-201), 10-27=-421(F=-201)

Concentrated Loads (lb)

Vert: 1=-407 4=-2028 2=-204 3=-257 25=-179 27=-1292

Justin Lane
Truss Design Engineer
Builders FirstSource
11650 Coastal Hwy Blvd
Lakeland, FL 33550

December 12, 2007

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Job	Truss	Truss Type	Qty	Ply	0 0	J1917171
L235554F	F02KW	GABLE	1	1		
Job Reference (optional)						

Builders FirstSource, Lake City, FL 32055

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0-1-8



0-1-8
Scale = 1 216

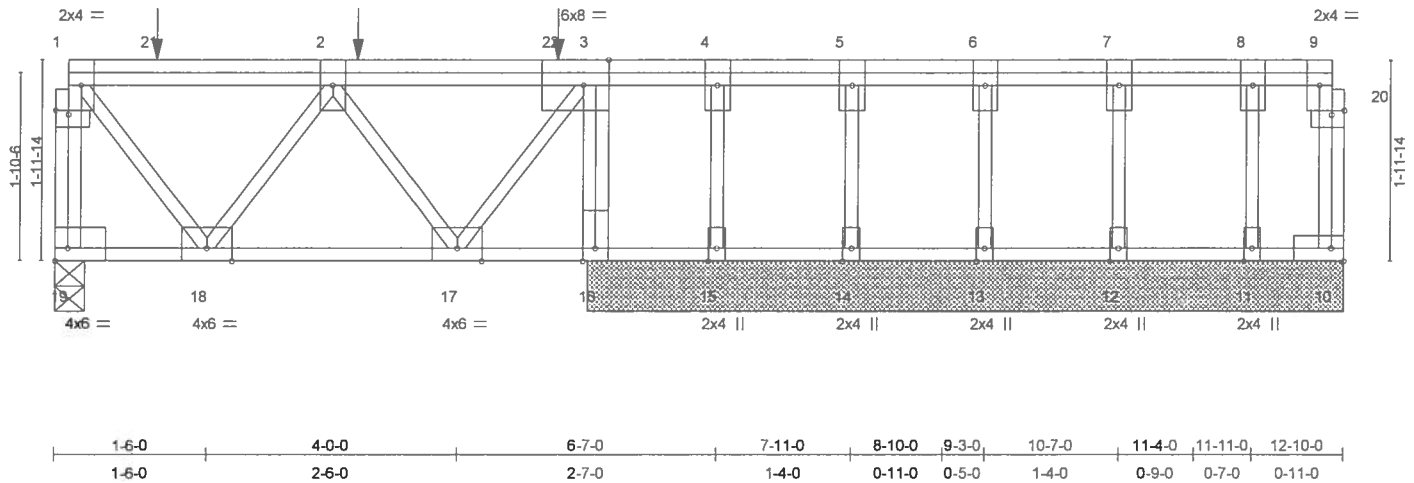


Plate Offsets (X,Y): [1:0-1-8,0-0-8], [3:0-3-0,Edge], [19:Edge,0-1-8], [20:0-1-8,0-0-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plates Increase	1.00	TC 0.94	Vert(LL)	-0.01 17-18	>999	360	MT20	244/190
TCDL 10.0	Lumber Increase	1.00	BC 0.26	Vert(TL)	-0.02 17-18	>999	240		
BCLL 0.0	Rep Stress Incr	NO	WB 0.34	Horz(TL)	0.00 10	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)					Weight: 95 lb	

LUMBER

TOP CHORD 4 X 2 SYP No.2
BOT CHORD 4 X 2 SYP No.2
WEBS 4 X 2 SYP No.3
OTHERS 4 X 2 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 19=1167/0-3-8, 10=24/7-6-8, 16=1432/7-6-8, 15=-5/7-6-8, 14=178/7-6-8, 13=139/7-6-8, 12=153/7-6-8, 11=126/7-6-8
Max Uplift 15=-5(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-19=-1162/0, 10-20=-21/0, 9-20=-21/0, 1-21=-525/0, 2-21=-520/0, 2-22=-515/0, 3-22=-515/0, 3-4=-1/0, 4-5=-1/0, 5-6=-1/0, 6-7=-1/0, 7-8=-1/0, 8-9=-1/0
BOT CHORD 18-19=0/0, 17-18=0/974, 16-17=0/1, 15-16=0/1, 14-15=0/1, 13-14=0/1, 12-13=0/1, 11-12=0/1, 10-11=0/1
WEBS 1-18=0/853, 2-18=-795/0, 2-17=-806/0, 3-17=0/848, 3-16=-1405/0, 4-15=0/8, 5-14=-162/0, 6-13=-126/0, 7-12=-139/0, 8-11=-114/0

JOINT STRESS INDEX

1 = 0.85, 1 = 0.00, 2 = 0.57, 3 = 0.52, 4 = 0.00, 5 = 0.07, 6 = 0.05, 7 = 0.06, 8 = 0.05, 9 = 0.01, 10 = 0.02, 11 = 0.07, 12 = 0.09, 13 = 0.08, 14 = 0.10, 15 = 0.01, 16 = 0.61, 17 = 0.64, 18 = 0.65, 19 = 0.54, 20 = 0.00 and 20 = 0.00

NOTES

- All plates are 3x6 MT20 unless otherwise indicated.
- Gable studs spaced at 1-4-0 oc.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 5 lb uplift at joint 15.

Builders FirstSource
Truss Connectors
Engineered for use with
MiTek Truss Connectors
Copyright 2007, MiTek Industries, Inc.

December 12, 2007

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	0 0	J1917171
L235554F	F02KW	GABLE	1	1		
						Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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NOTES

- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

LOAD CASE(S)

- 1) Floor: Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 10-19=-10, 1-3=-220, 3-9=-100

Concentrated Loads (lb)

Vert: 2=-380 21=-446 22=-380

Justin A. Loefer
Truss Design Engineer
Florida PE# 110,385,000
1102 Chestnut Way NW
Covington, GA 30045

December 12, 2007

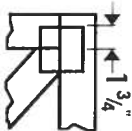
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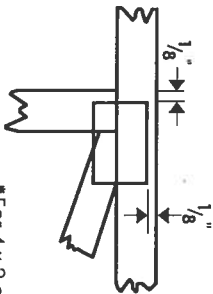


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

4 X 4

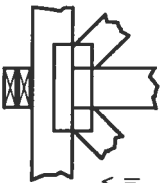
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



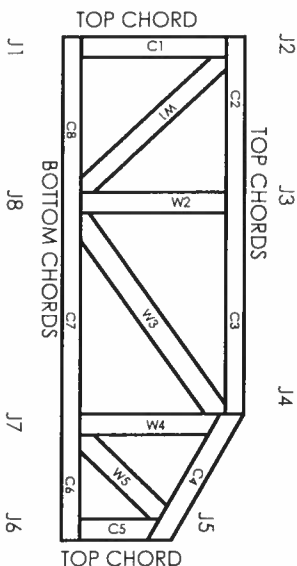
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DLHR	960022-W, 970036-N
NER	561



MITek Engineering Reference Sheet: MIT-7473



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at 1/4 panel length ($\pm 6"$ from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

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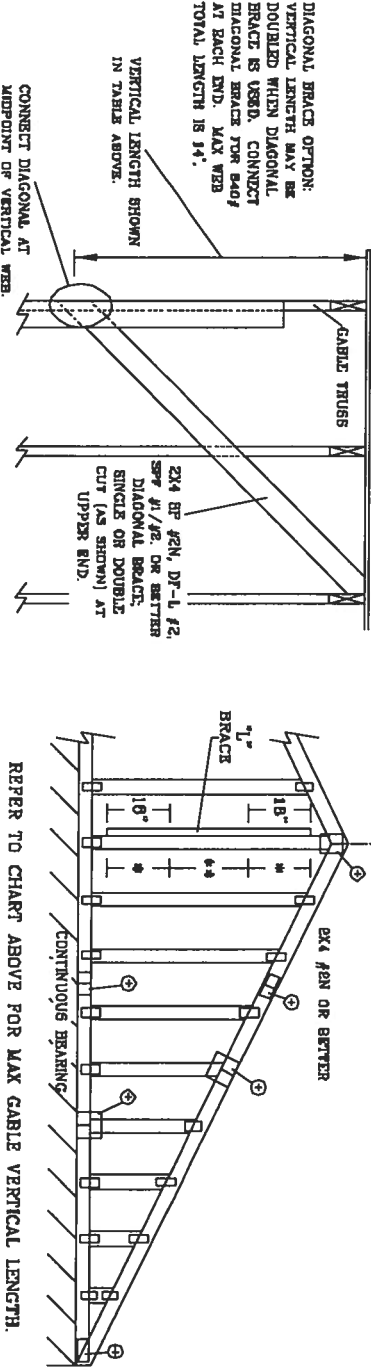
BRACING GROUP SPECIES AND GRADES:			
GROUP A:		HDM-GIR	
SPRUCE-PINE-TIR		#1 / #2	
STUD		STUD	
#3		#3	
STUD		STUD	
STANDARD		STANDARD	
DOUGLAS FIR-LARCH		SOUTHERN PINE	
#3		#3	
STUD		STUD	
STANDARD		STANDARD	
GROUP B:			
HDM-GIR		HDM-GIR	
#1 & BTR		#1 & BTR	
#1		#1	
SOUTHERN PINE		DOUGLAS FIR-LARCH	
#1		#1	
#2		#2	

LIVE LOAD DEPLETION CRITERIA IS $L/240$.
 PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER
 CONTINUOUS BRACING (6 PSP TC DEAD LOAD).
 CABLE END SUPPORTS LOAD FROM 4' 0"

ATTACH EACH T¹ BRACE WITH 10d NAILS.
 * FOR (1) T¹ BRACE: SPACE NAILS AT 8" O.C.
 * IN 18" END ZONES AND 4" O.C. BETWEEN ZONES
 * FOR (2) T¹ BRACES: SPACE NAILS AT 3" O.C.
 IN 18" END ZONES AND 8" O.C. BETWEEN ZONES.
 T¹ BRACING MUST BE A MINIMUM OF 80% OF WEB
 MEMBER LENGTH.

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO. SPICES
LESS THAN 4" 0"	1X4 OR 2X3
GREATER THAN 4" 0", BUT LESS THAN 11" 8"	2X4
GREATER THAN 11" 8"	2, 5X4

+ REFER TO COLUMN TRUSS DESIGN FOR
PEAK, SPUR, AND HEBL PLATES.



****WARNING**** RECOVER EXISTING CABLE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BLS-1-CO (BUILDING CONCURRENT SAFETY INFORMATION), PUBLISHED BY ITI TRUSS PLATE INSTITUTE, 503 THOMPSON RD., SUITE 200, MADISON, WI 53719 AND VICA LEADERS TRUSS COUNCIL OF AMERICA, 6300 EVERETT LN, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE TACKLING. DATES OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CLIPPING.

JULIUS LEE'S
CONS. ENGINEERS P.A.

1455 6th AVENUE
DELRAY BEACH, FL 33441-2161

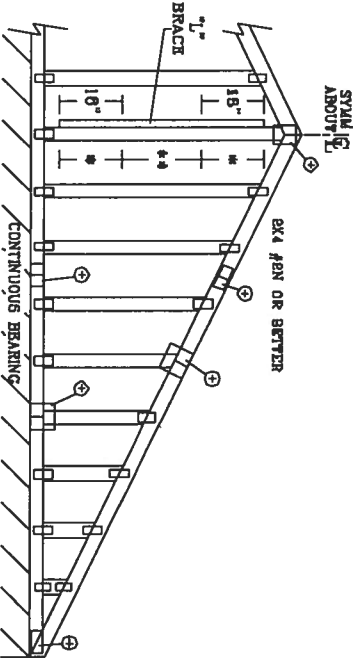
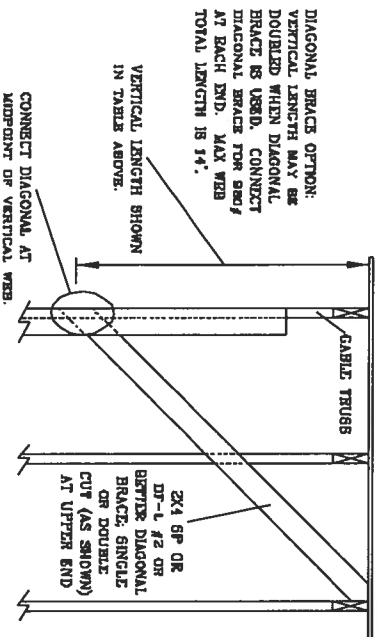
REF	ASCB7-02-CAB13015
DATE	11/26/03
DRWG	ATTX STD CABLE 16 E HT
-ENG	

-ENG

MAX. TOT. LD. 60 PSF

No: 34869
STATE OF FLORIDA

MAX. SPACING 24.0"



REFER TO CHART ABOVE FOR MAX CABLE VERTICAL LENGTH.

DIAGONAL BRACE OPTION:
VERTICAL LENGTH MAY BE
DOUBLED WHEN DIAGONAL
BRACE IS USED. CONNECT
DIAGONAL BRACE TO JOINT
AT EACH END. MAX WEB
TOTAL LENGTH IS 14'.

VERTICAL LENGTH SHOWN
IN TABLE ABOVE.

CONNECT DIAGONAL AT
MIDPOINT OF VERTICAL WEB.

MANUFACTURER'S RESIDUE. EXTREME CARE, FORTIFICATION, HANDLING, SHIPPING, INSTALLING, AND MAINTENANCE. REFER TO BEST-1-800 BUILDING CONCRETE SOCIETY (INCORPORATED), PUBLISHED BY THE CONCRETE PAVING INSTITUTE, 135 DUNDAS ST. W., SUITE 400, MISSISSAUGA, ONT. L4V 1N3 AND LYCA LABORATORY, CENTRAL OF AMERICA, 6680 ENTERPRISE LANE, MORTON, VI 53179 FOR SPECIFIC PRACTICES PRIOR TO PROCEEDING WITH THESE FINDINGS. UNLESS OTHERWISE INDICATED, TOP COARD SHALL HAVE PROPERLY ATTACHED RIGID FIBERGLASS PANELS AND BOTTOM COARD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.

1456 SW 4th AVENUE
DELRAY BEACH, FL. 33444-2161

CABLE VERTICAL PLATE SIZES		
VERTICAL LENGTH	NO SPICE	
LESS THAN 4" 0"	1X1 DR. EX3	
GREATER THAN 4" 0" BUT LESS THAN 11" 6"	2X4	
GREATER THAN 11" 6"	2,3X4	

+ REFER TO COMMON TRUSS DESIGN FOR
PEAK, SPICE, AND HEEL PLATES.

REF. ASSC7-02-CAB313

DATE 11/26/03

DWG. APPROV. STD. CABLE 30 Y

-ENG

CABLE END SUPPORTS LOAD FROM $\pm 0^{\circ}$
 OUTDOCKERS WITH $\pm 0^{\circ}$ OVERHANG, OR 12°
 PLYWOOD OVERHANG.
 ATTACH EACH T¹ BRACE WITH 10d NAILS.
 * FOR (1) T¹ BRACE: SPACE NAILS AT 2° O.C.
 IN 18° END ZONES AND AT O.C. BETWEEN ZONES
 * FOR (2) T¹ BRACES: SPACE NAILS AT 3° O.C.
 IN 18° END ZONES AND AT O.C. BETWEEN ZONES.
 T¹ BRACING MUST BE A MINIMUM OF 80% OF WEB
 DECKING LENGTH.

CABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS $L/240$.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER
CONTINUOUS BEARING (6 PSP TC DEAD LOAD).

CABLE END SUPPORTS LOAD FROM 4' 0"

OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS

* FOR (1) T-BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

* FOR (2) " BRACES: SPACE NAILS AT 3" O.C.
IN 18" END ZONES AND 6" O.C. BETWEEN ZONES

7. BRACING MUST BE A MINIMUM OF 80% OF WRB

MEMBER LENGTH.

CABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR EQV

LESS THAN 4' 0"	1X4 DR BR3
GREATER THAN 4' 0", BUT LESS THAN 11' 0"	2X4

LESS THAN 11' 6"	
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR
BRAC, SPLICE, AND HEEL PLATES.

PEAN, SPICE, AND HERB PLANTS.

REF	ASCB7-02-CAB13030
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DATE	11/26/03
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DWG MITEK STD CABLE 30' E MT

-ENG

ONE	
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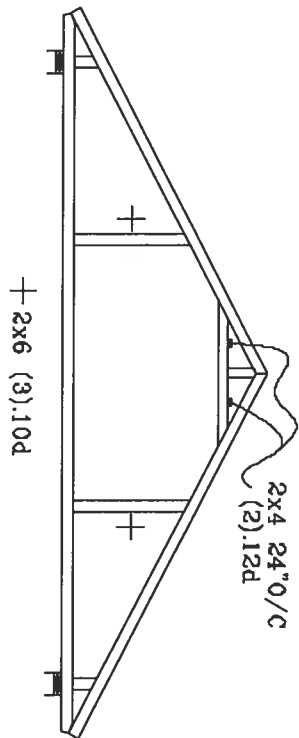
LD. 60 PSF

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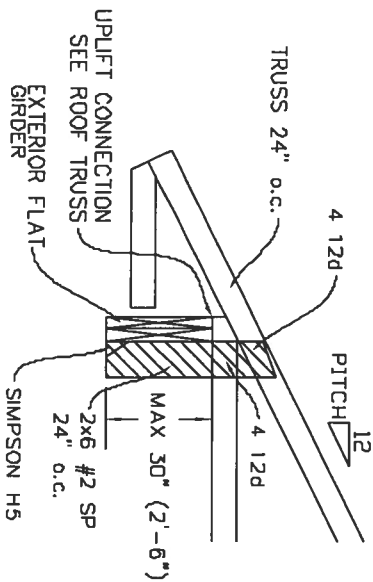
ING 24.0"	
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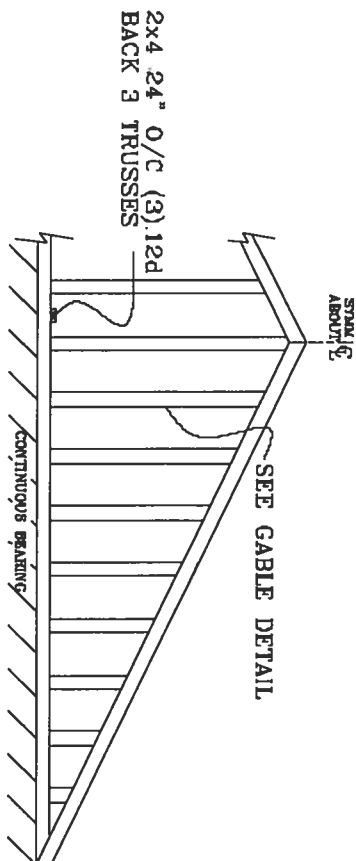
TYPICAL ATTIC TRUSS BRACING



TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS

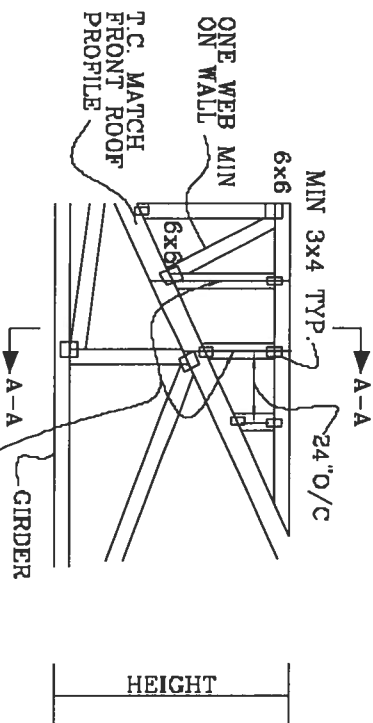


GABLE END TRUSS DETAIL



MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR BOB

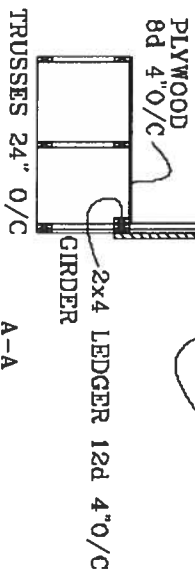
TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



SEE ROOF TRUSSES FOR UPLIFT

ROOF 24" O/C

SEE GABLE END DETAIL FOR T-BRACE BEHIND EACH VERTICAL



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STATE OF FLORIDA

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG. LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST

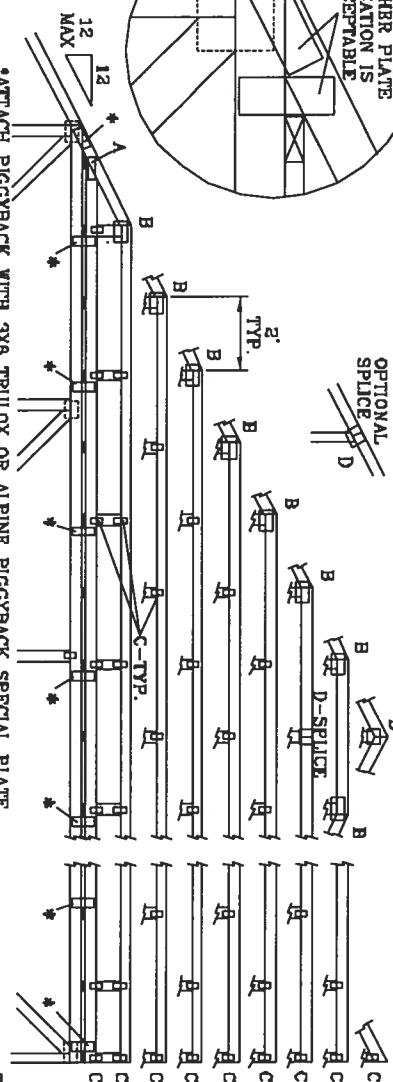
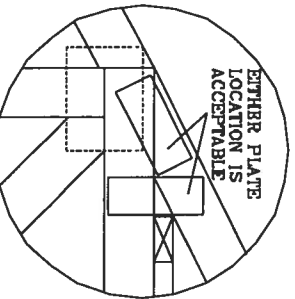
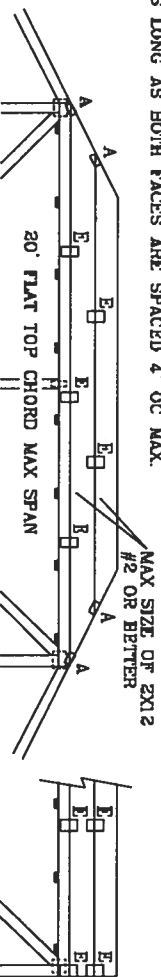
CAT 1, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, FBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF

WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (B,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT 1, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF



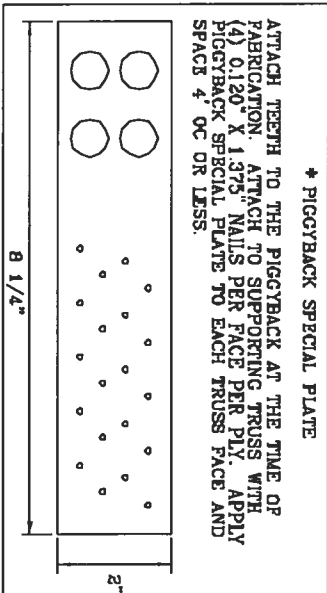
*ATTACH PIGGYBACK WITH 3X8 TRUSS OR ALPINE PIGGYBACK SPECIAL PLATE.

REMARKS: TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING, AND BRACING. REFER TO BEST PRACTICES BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 3600 BROAD RD DR, SUITE 200, HANOVER, VA 22979 AND VTRC CHORD TRUSS CHORD TRUSS FUNCTIONAL. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED BIRD CEILING.

JOINT TYPE	SPANS UP TO			
	30'	34'	38'	62'
A	2X4	2.5X4	2.5X4	3X6
B	4X6	5X6	6X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	6X6	6X5	5X6
E	4X6 OR 3X8 TRUSS AT 4' OC, ROTATED VERTICALLY			

ATTACH TRUSS PLATES WITH (B) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4' OC.
10' TO 14'	2X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4' OC.



* PIGGYBACK SPECIAL PLATE
ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.

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No. 34888
STATE OF FLORIDA

MAX LOADING		REF PIGGYBACK	
55 PSF AT	DATE 09/12/07	DRG/MITEK STD PIGGY	
1.33 DUR. FAC.		-ENG JL	
50 PSF AT			
1.25 DUR. FAC.			
47 PSF AT			
1.15 DUR. FAC.			
SPACING 24.0"			

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

- * 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:
(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR
ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED
BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "I"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.6") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 120'.

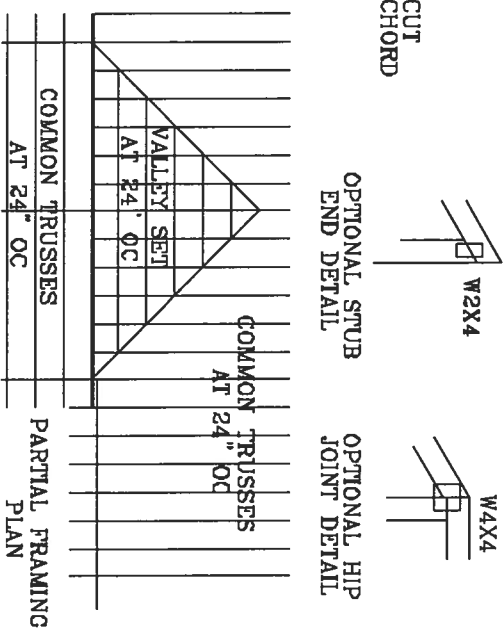
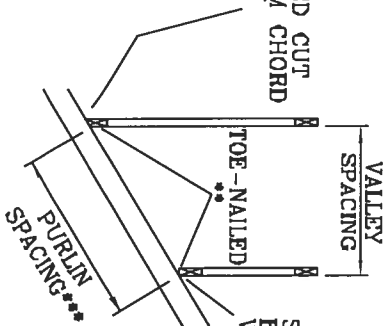
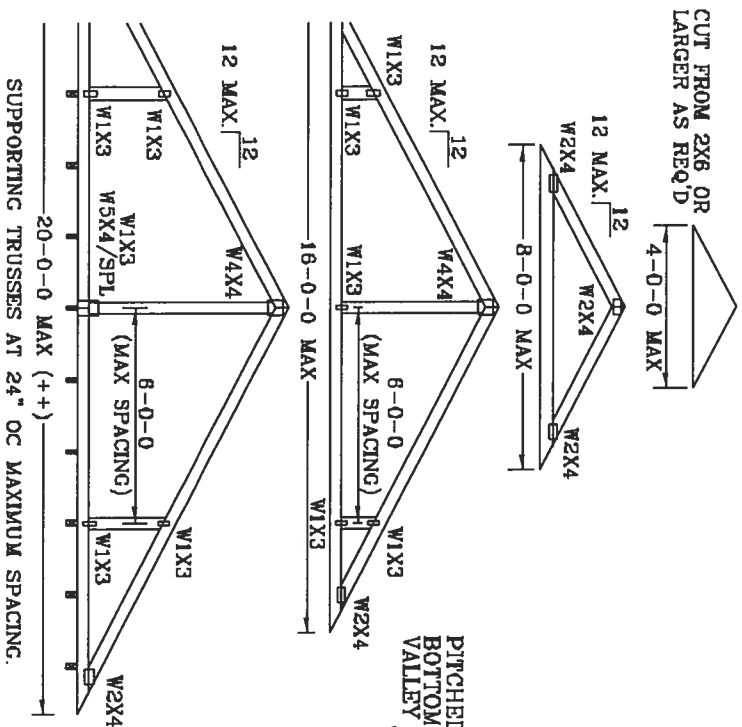
TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS
INSTALLATION

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN OR BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



THIS DRAWING REPLACES DRAWING A105

MANUFACTURE TO THESE RECORD, EXTENSIVE CASE FABRICATING, HANDING, SHIPPING, INSTALLING AND BRACING. REFER TO LIST-1 (C) BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE STEEL PLATE INSTITUTE, 560 DOWNEY RD., SUITE 200, WILMISTON, VA. 51779 AND WILCO COVERED TRUSS COMPANY OF AMERICA, 6300 WESTERN LN, WILMISTON, VA. 51779 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. (LATEST EDITIONS INDICATED). TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND JOINTED CHORD SHALL HAVE A PROPERLY ATTACHED BRIDG CEILING.

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STATE OF FLORIDA

		REF	VALLEY DETAIL
TC LL	20	PSF	
TC DL	7	PSF	DATE 11/26/03
BC DL	5	PSF	DRWG VALTRUSS1103
BC LL	0	PSF	-ENG JL

TOT. LD.	32	40	PSF
DUR.FAC.	1.25	1.25	
SPACING	24"		

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

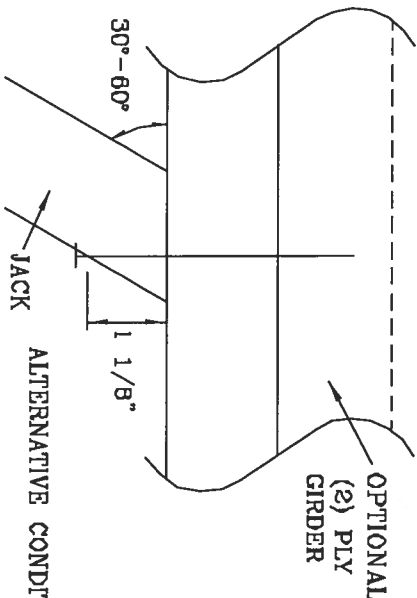
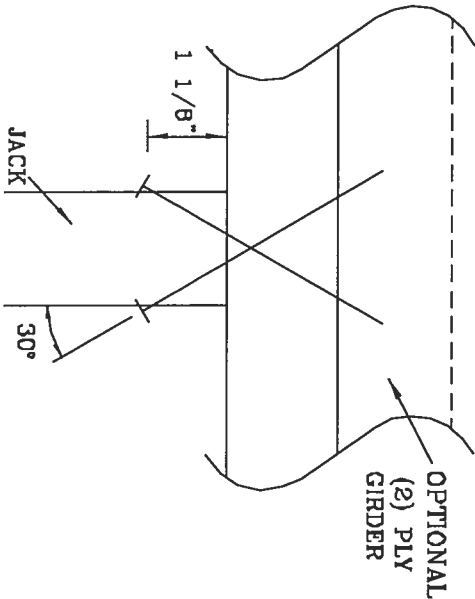
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	363#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 764040

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. ALWAYS FOLLOW THE MANUFACTURER'S INSTRUCTIONS. FOR MORE INFORMATION, CONTACT THE PLATE INSTITUTE, 283 TONGERIDGE DR., SUITE 200, MADISON, VT 05719, AND VICA (WOOD TRUSS COUNCIL OF AMERICA, 6900 ENTERPRISE LN., MADISON, VT 05719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		

DUR. FAC. 1.00

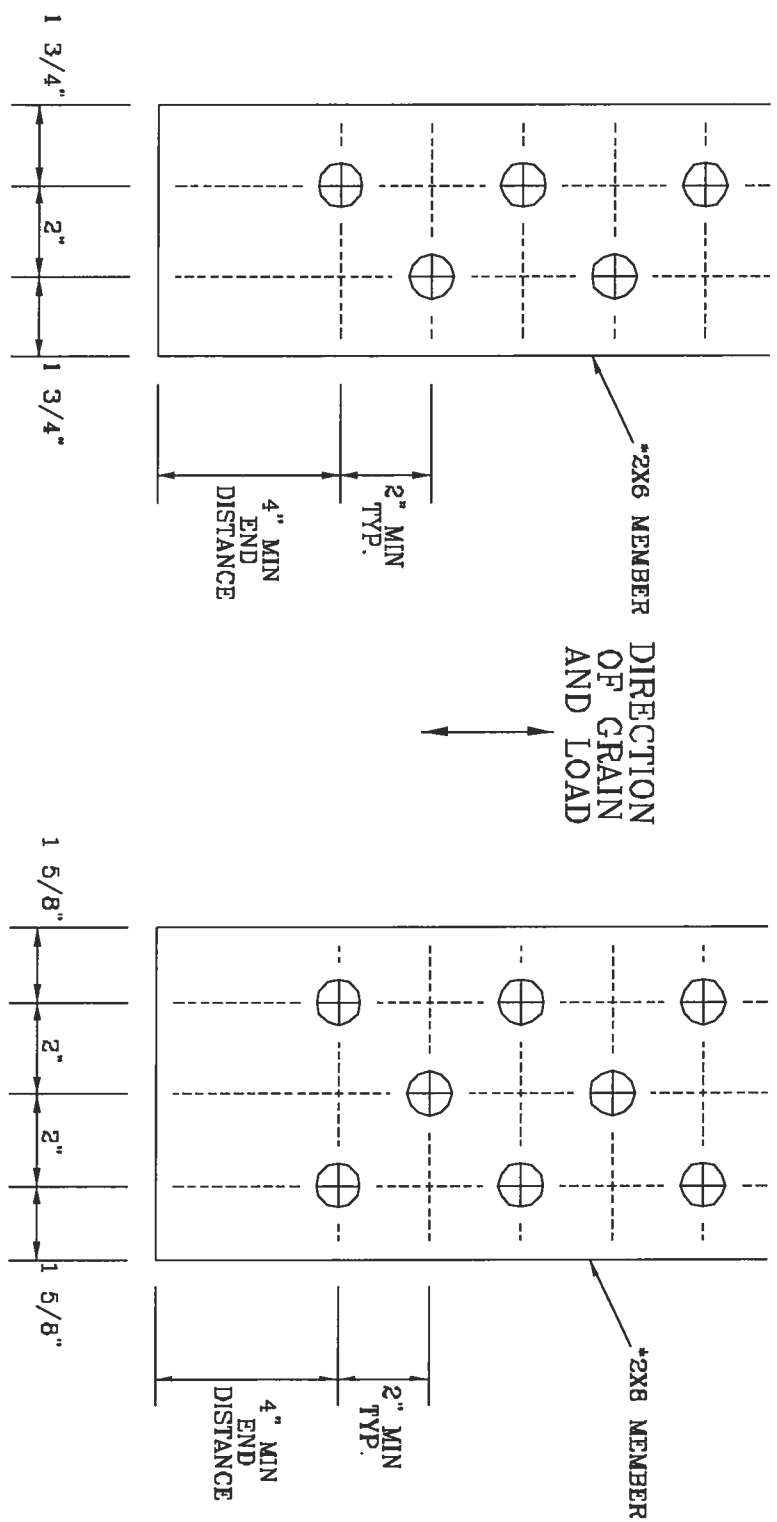
SPACING

1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

- GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
- BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL

2X8 DETAIL

THIS DRAWING REPLACES DRAWING A828.016

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-80 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS COUNCIL OF AMERICA, 6000 CENTRE DRIVE, SUITE 200, HANSON, VA 22079 AND VICA CYCLED TRUSS COUNCIL OF AMERICA, 10000 WILSON ROAD, SUITE 100, WILSON, VA 22197 FOR SAFETY PRACTICES PRIOR TO PERFORMING STRUCTURAL PANELS AND SECTION CHORDS SHALL HAVE A PROPERLY ATTACHED RIBBON CEILING.

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No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTS P1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TRULOX CONNECTION DETAIL

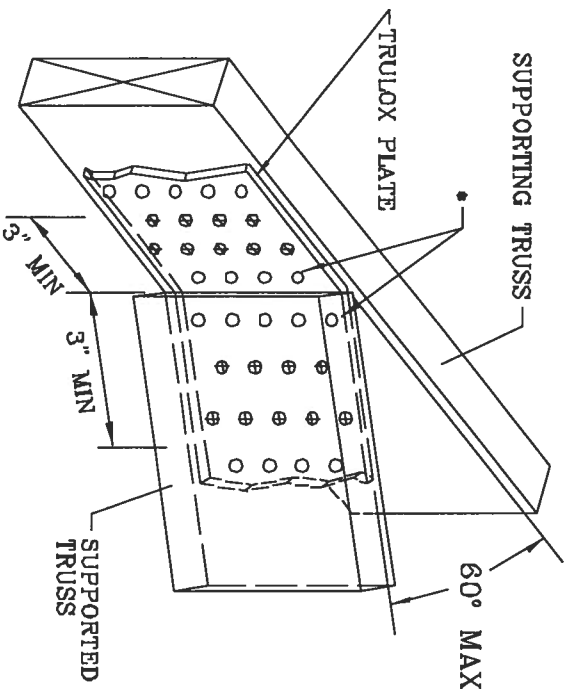
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

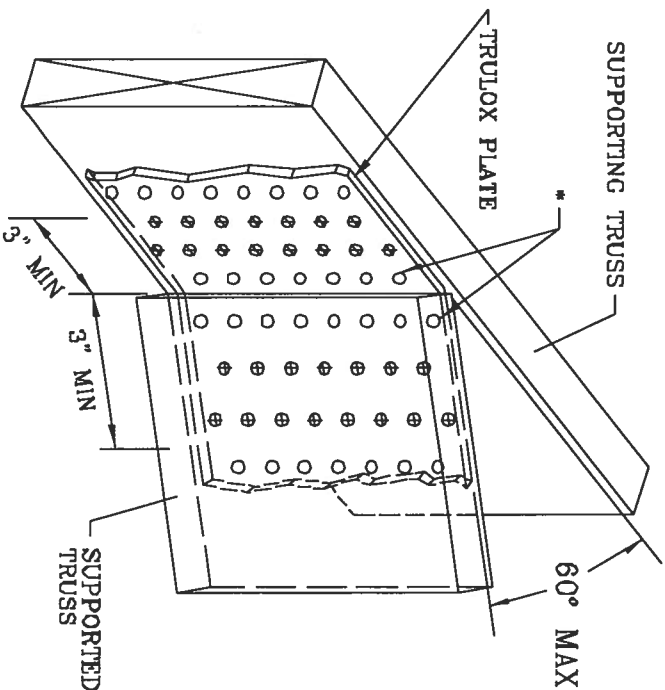
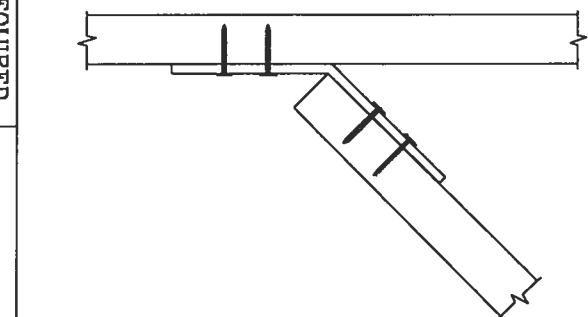
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



MINIMUM 3X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
6X6	15	990#



MINIMUM 6X6 TRULOX PLATE

THIS DRAWING REPLACES DRAWINGS 1,158,988 1,158,988/R
1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

WARNING TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO AISC 1-10 BUILDING CONSTRUCTION SAFETY INFORMATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 583 TOWNSEND DR., SUITE 200, MADISON, VI. 53719 AND VTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 CANTERBURY LN, MADISON, VI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID GIRDING.

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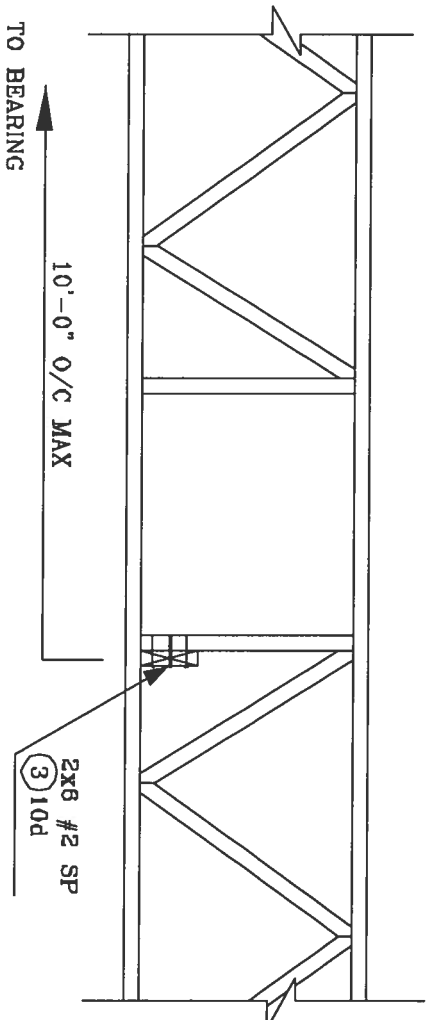
REF TRULOX

DATE 11/26/03

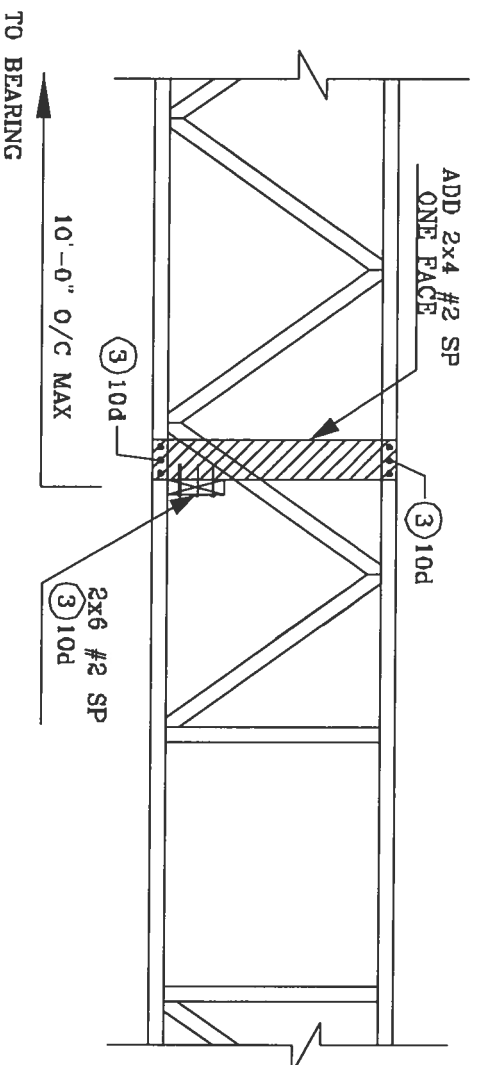
DRWG CNTRULOX1103

-ENG JL

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



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