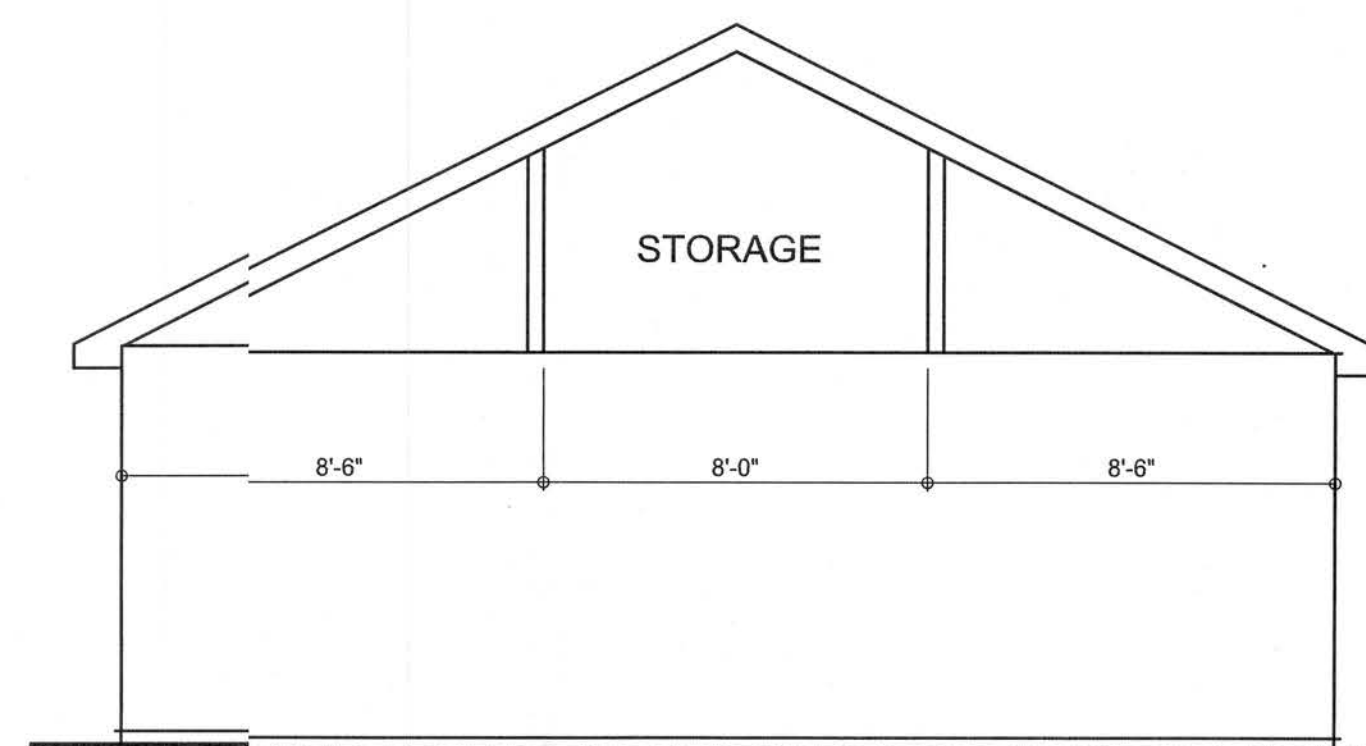


REVISIONS		

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



FRONT ELEVATION
SCALE: 1/4" = 1'-0"



REQUIRED ROOF VENTILATION: AS PER FLORIDA BUILDING CODE 2309.7

RIDGE VENT
MIN. 50% TOTAL VENT AREA
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3" ABOVE EAVE)
722 S.F. / 300 x 50% = 2 S.F. RIDGE VENT AREA REQUIRED
18 FEET OF RIDGE VENT REQUIRED

SOFFIT VENT
722 S.F. / 300 x 50% = 2 S.F. SOFFIT VENT AREA REQUIRED
67 FEET OF SOFFIT VENT REQUIRED

BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

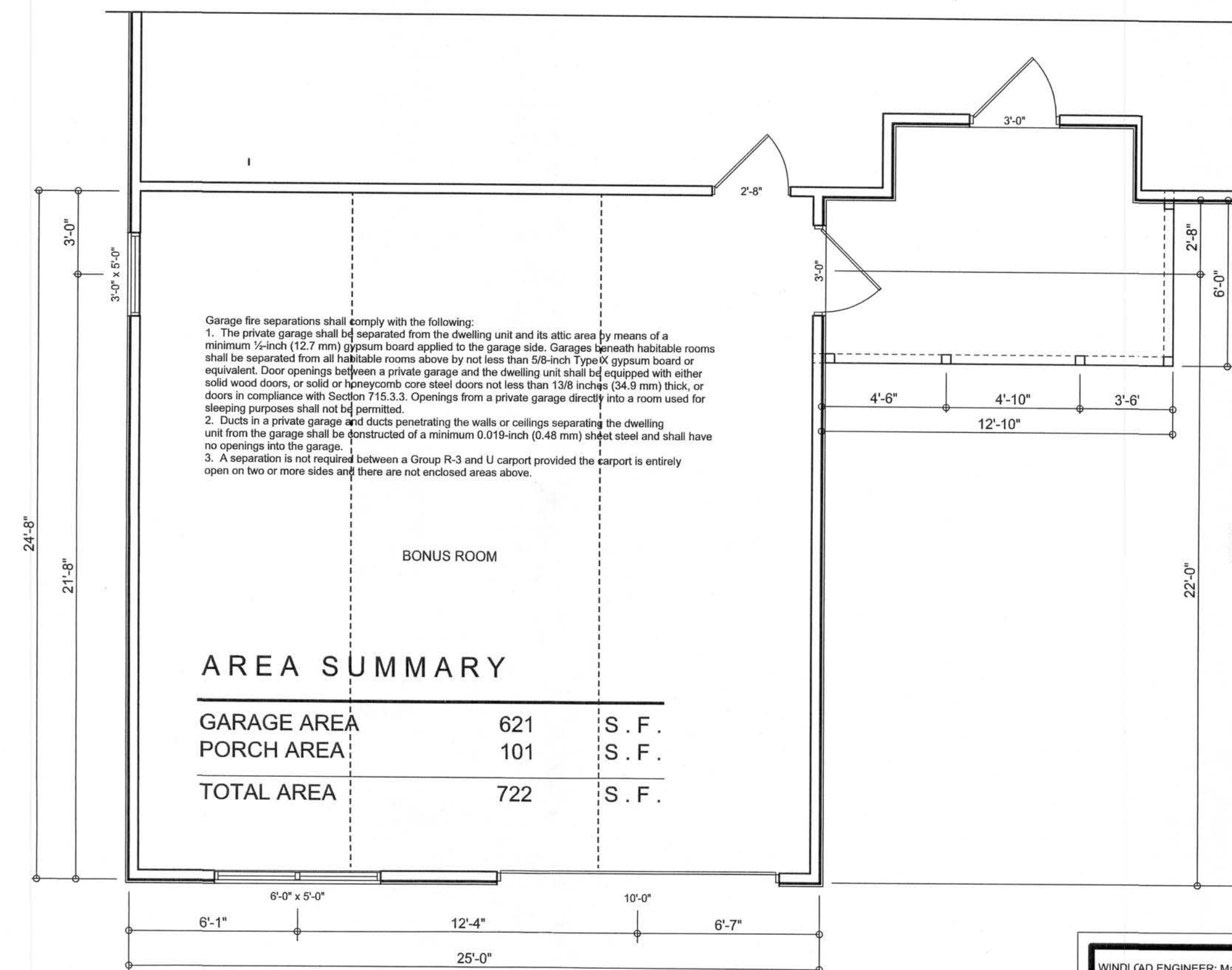
1. RIDGE VENTS = 16 IN²/FT (11 FT²/FT)
2. OFF-RIDGE VENTS = .70 FT² PER 4' UNIT
3. SOFFIT VENTS = 4.3 IN²/FT (.03 FT²/FT)



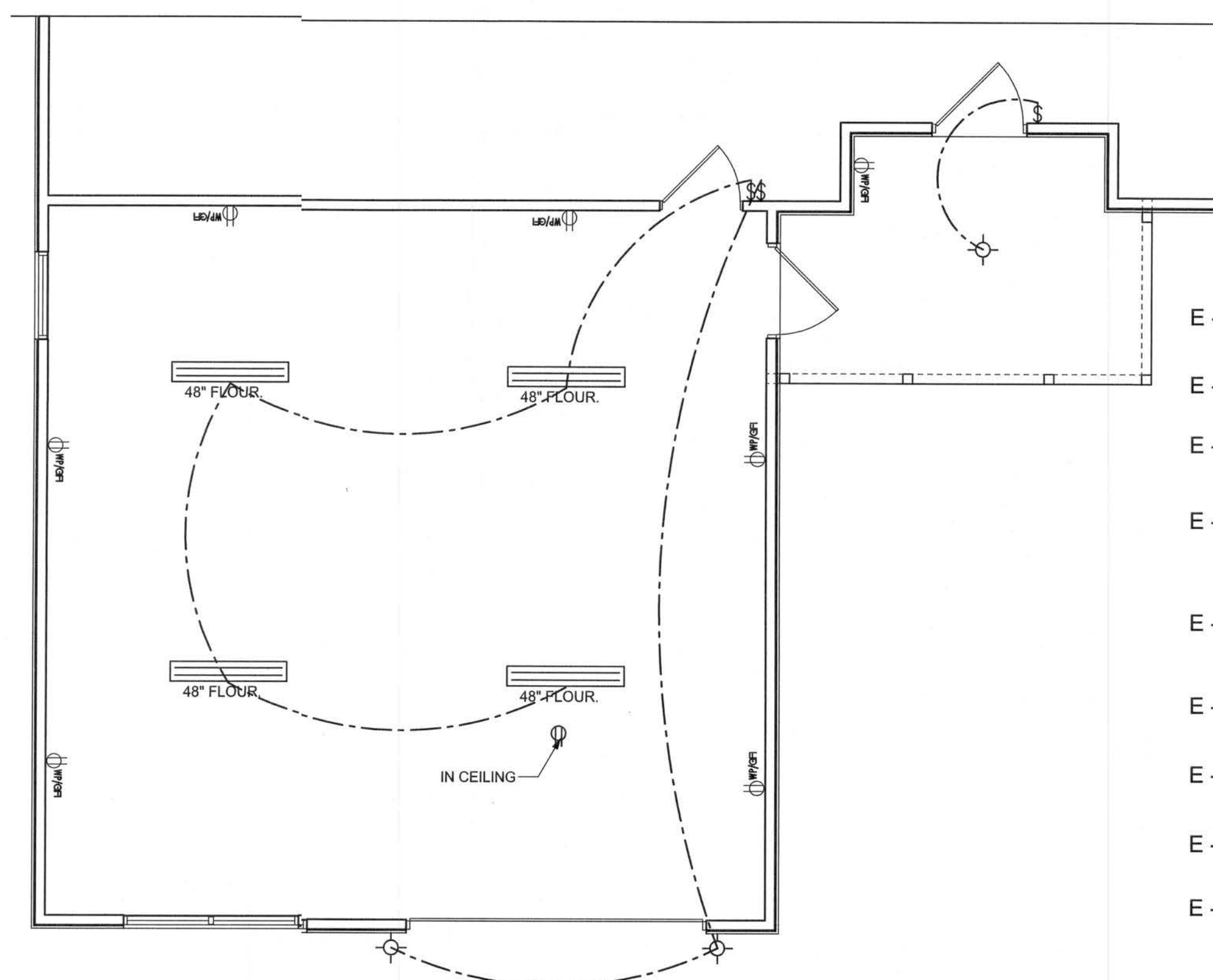
LEFT ELEVATION
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"



FLOOR PLAN
SCALE: 1/4" = 1'-0"



ELECTRICAL PLAN NOTES

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPARATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
- E -10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXHAUST FAN WITH LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	SMOKE DETECTOR
	WALL SWITCH
	3 WAY WALL SWITCH
	4 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	PHONE JACK
	TELEVISION JACK
	GARAGE DOOR OPENER
	WALL HEATER

WINDLOAD ENGINEER: Mark Disosway,
PE No. 5815, PCB 868, Lake City, FL
32056, 36-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building at specified location.

MARK DISOSWAY
P.E. 5815
09/09/06
SEAL

CHAD WRIGHT

P. DENIS WESTON
RESIDENCE

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PRINTED DATE:
November 09, 2006

DRAWN BY: Ben Sanks

CHECKED BY:

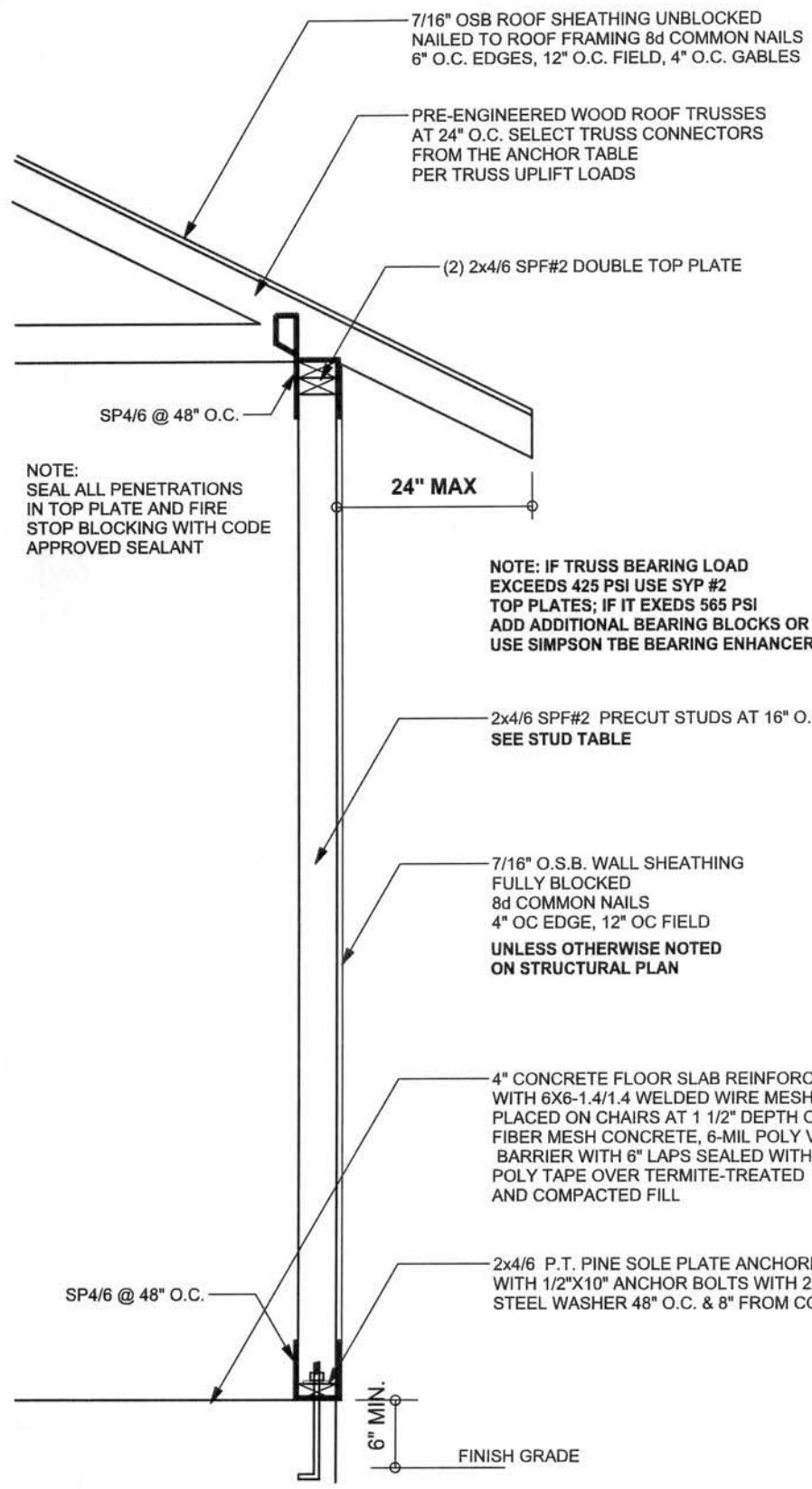
FINALS DATE:
06 / 10v / 06

JOB NUMBER:
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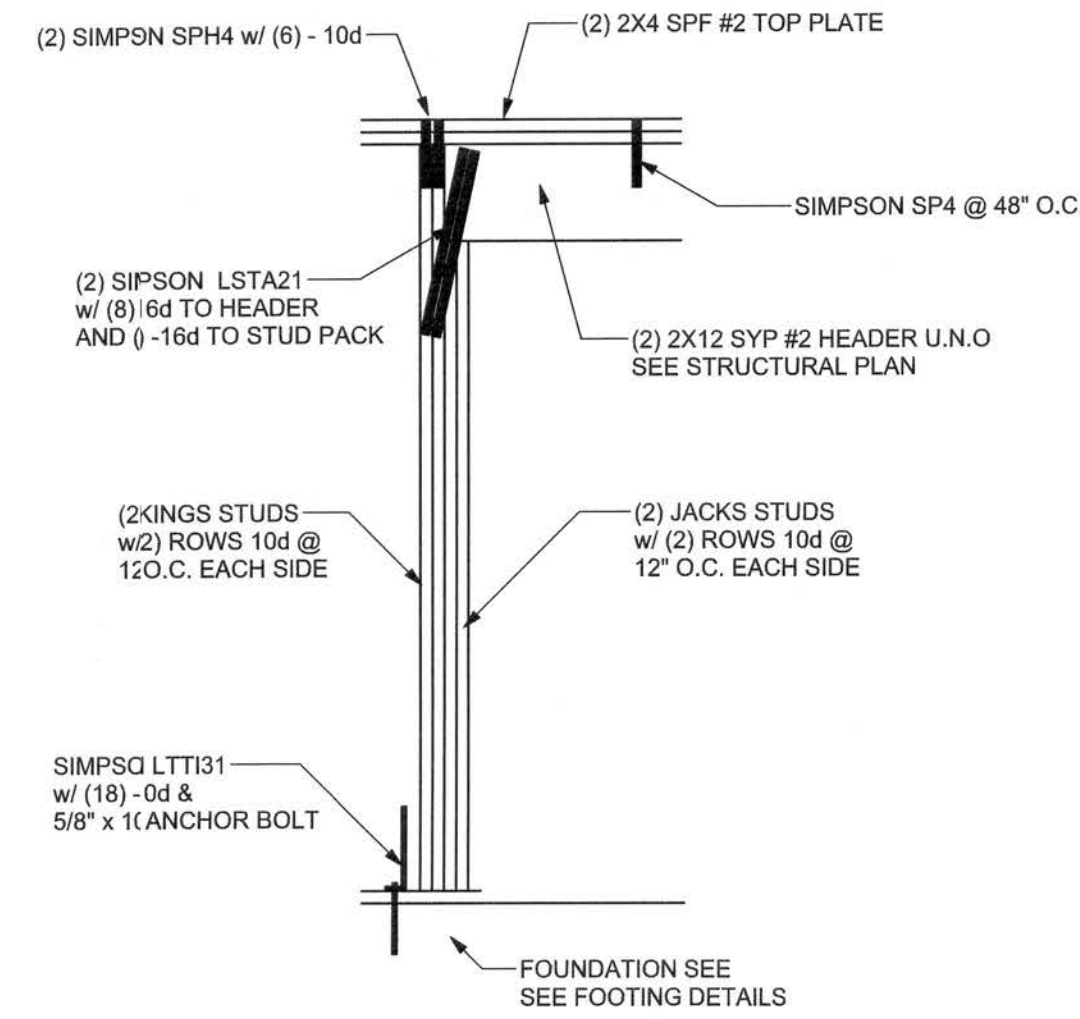
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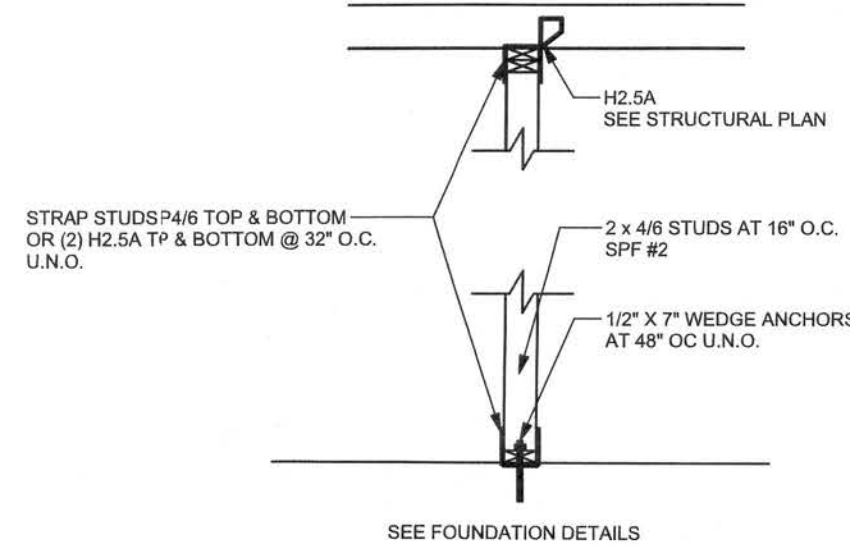
OF 3 SHEETS



ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"



TYPICAL GARAGE DOOR HEADER STRAPING DETAIL
SCALE: 1/2" = 1'-0"



INTERIOR BEARING WALL
SCALE: 1/2" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

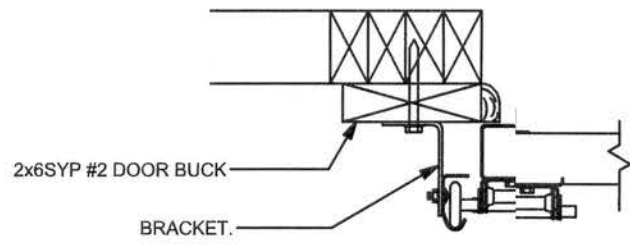
(1) 2x4 @ 16" O.C.	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" O.C.	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" O.C.	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" O.C.	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20R.
EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS
RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B.
STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING
LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.
EXAMPLE: 16" O.C. x 0.85 = 13.6" O.C.

2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT

ATTACH GARAGE DOOR BUCK TO STUD U.P. PACK AT
EACH SIDE OF DOOR OPENING WITH 1 3/8"x4" LAG
SCREWS w/ 1" WASHER LAG SCREWS MAY BE
COUNTERSUNK. HORIZONTAL JAMBS DO NOT
TRANSFER LOAD. CENTER LAG SCREWS OR
STAGGER 1/4" NAILS OR (2) ROWS OFF. 131 x 3 1/4"
ON PER TABLE BELOW:

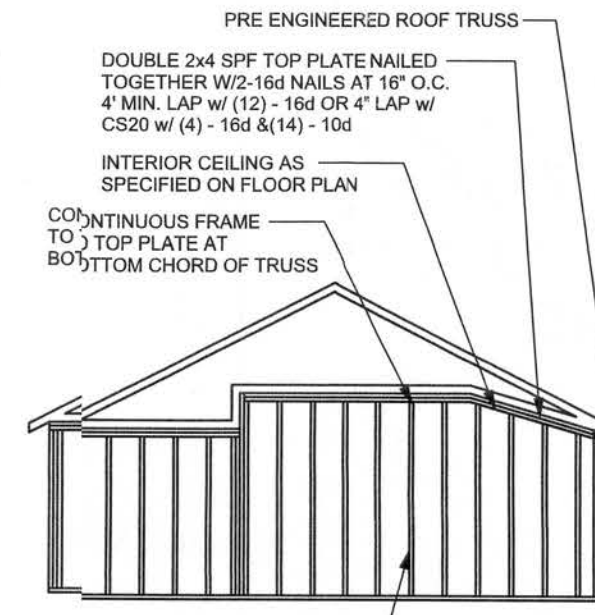
DOOR WIDTH	3/8" x 4" LAG	1 3/8" LAG STAGGER	(2) ROWS OF 131 x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	18" O.C.	3" O.C.	3" O.C.



GARAGE DOOR BUCK INST/ALLATION DETAIL
SCALE: N.T.S.

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



**CONTINUOUS FRAME TO
CEILING DIAPHRAGM DETAIL**
SCALE: N.T.S.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION
R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN
TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS
THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE
COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS
MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN
PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED
LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO
REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF
SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL
BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF
DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT
RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE
TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES
RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED
TRUSS SHEETS.

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE
FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND
PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR
ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER
AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S
RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO
SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR
INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR
REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT
CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET
GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS
VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, F_y = 80KSI, WELDED WIRE REINFORCEMENT FABRIC
(W.W.R.) CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED
MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT.
FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD
PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER
TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN
ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH /
WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT
CUT W/ W/ OR REINFORCEMENT. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO
OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS
BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F_y = 60 KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS);
UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-86, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F_b = 2400, E = 1800ksi, UNO. SUPPLIER MAY SUPPLY AN
ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED,
APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES
STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6" O.C. PANEL EDGES, 12" O.C. INTERMEDIATE
MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" O.C. UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS,
AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE
SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE
TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION
INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO
LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE OR 10" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH
3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16". UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST
REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE
SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND
BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004
REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION, IF YOU
BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL
THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS
DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS,
TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL
BEARING LOCATIONS.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS
MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-4d	3-4d	
< 455	< 265	H5	4-4d	4-4d	
< 360	< 235	H4	4-4d	4-4d	
< 455	< 320	H3	4-4d	4-4d	
< 415	< 365	H2.5	5-4d	5-4d	
< 600	< 535	H2.5A	5-4d	5-4d	
< 950	< 820	H6	8-4d	8-4d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-4d	12-4d, 1 1/2"	
< 1465	< 1050	H14-2	15-4d	12-4d, 1 1/2"	
< 990	< 850	H10-1	8-4d, 1 1/2"	8-4d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1285	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1285	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LG2T	14 -16d	14 -16d	
HEAVY GIRDER TIEDOWNS*					TO FOUNDATION
< 3965	< 3330	MG7		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-4d		
< 1705	< 1705	CS16	28-4d		
STUD ANCHORS*					TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTT31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	H02A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 -16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 -16d		2-5/8" AB

DESIGN DATA

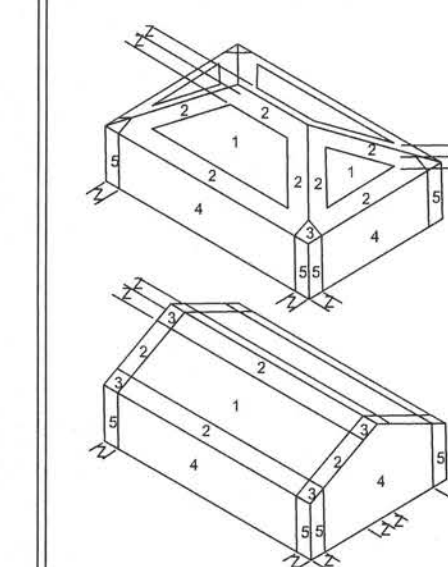
WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS;
MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT
ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10%
SLOPE AND UNOBSERVED UPWARD FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- 1) BASIC WIND SPEED = 130 MPH
- 2) WIND EXPOSURE = B
- 3) WIND IMPORTANCE FACTOR = 1.0
- 4) BUILDING CATEGORY = II
- 5) ROOF ANGLE = 10-45 DEGREES
- 6) MEAN ROOF HEIGHT = <30 FT
- 7) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- 8) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



Zone	Effective Wind Area (ft ²)		
	10	100	
1	19.9	21.8	18.1 -18.1
2	19.9	25.5	18.1 -21.8
2.0th		40.6	-40.6
3	19.9	25.5	18.1 -21.8
3.0th		48.3	-42.4
4	21.8	23.6	18.5 -20.4
5	21.8	29.1	18.5 -22.6
Doors & Windows		21.8	-29.1
Wind Case			
(Zone 5, 10 ft2)			
8x7 Garage Door		19.5	-22.9
16x7 Garage Door		18.5	-21.0

DESIGN LOADS

- FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)
- 30 PSF (SLEEPING ROOMS)
- 30 PSF (ATTICS WITH STORAGE)
- 10 PSF (ATTICS WITHOUT STORAGE, <3.12)
- ROOF 20 PSF (FLAT OR <4:12)
- 16 PSF (4:12 TO <12:12)
- 12 PSF (12:12 AND GREATER)
- STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)
- SOIL BEARING CAPACITY 1000PSF
- NOT IN FLOOD ZONE (BUILDER TO VERIFY)

LW44 - ALUMINUM PORCH POST & HEADER
ANCHORS

SCALE: N.T.S.

BEAM CORNER CONNECTION DETAIL

SCALE: N.T.S.

SUPPORTIVE CENTER POST TO BEAM DETAIL

SCALE: N.T.S.

TYPICAL HEADER STRAPING DETAIL

SCALE: 1/2" = 1'-0"

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway,
P.E. NO.5315, P.O.B. 868, Lake City, FL
32056, 352-754-5419

DIMENSIONS:
Scaled dimensions supersede scaled
dimensions. Refer all questions to
Mark Disoway, P.E. for resolution.
Do not proceed without clarification.

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form or manner without first the express written
permission and consent of Mark Disoway.

CERTIFICATION: I hereby certify that I have
examined this plan, and that the applicable
portions of the plan, relating to wind engineering
comply with section R301.2.1, Florida building
code residential 2004, to the best of my
knowledge.

LIMITATION: This design is valid for one
building, 4 specified location.

MARK DISOWAY
P.E. 5315
Mark Disoway
09 Nov 06
SEAL

CHAD WRIGHT

**P. DENIS WESTON
RESIDENCE**

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PRINTED DATE:
November 09, 2006

DRAWN BY:
Ben Sparks

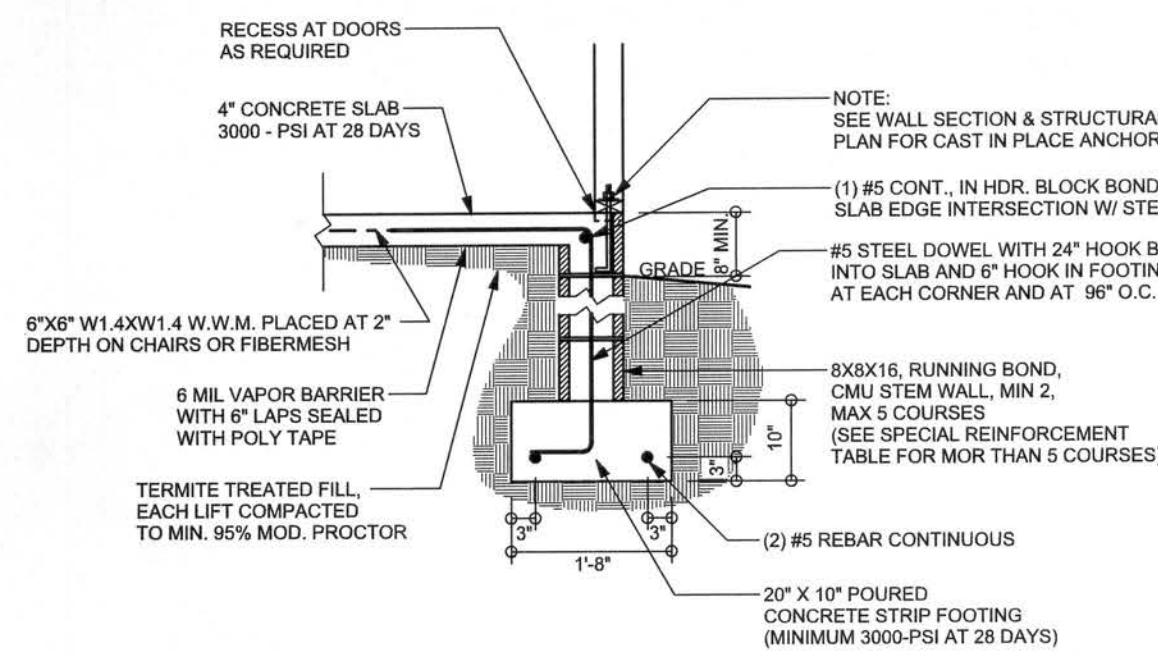
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FINAL DATE:
06 / Nov / 06

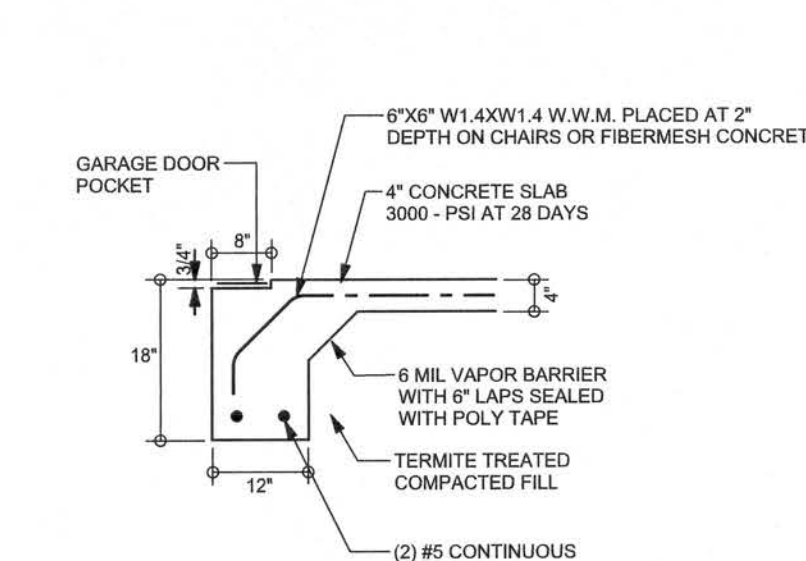
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DRAWING NUMBER

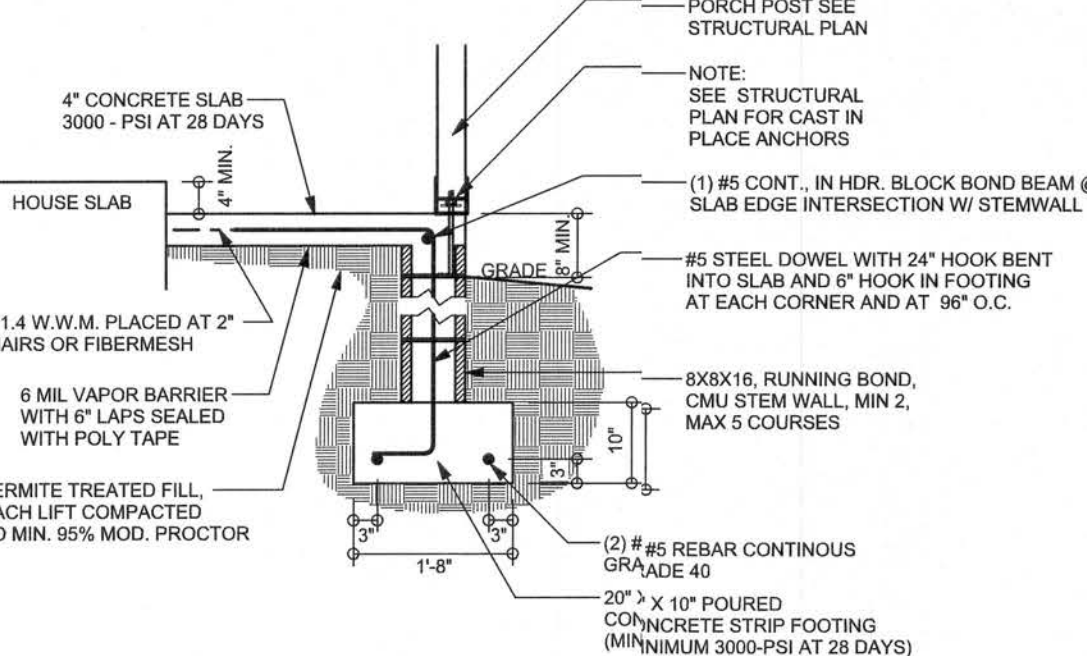
S-1
OF 3 SHEETS



F9 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



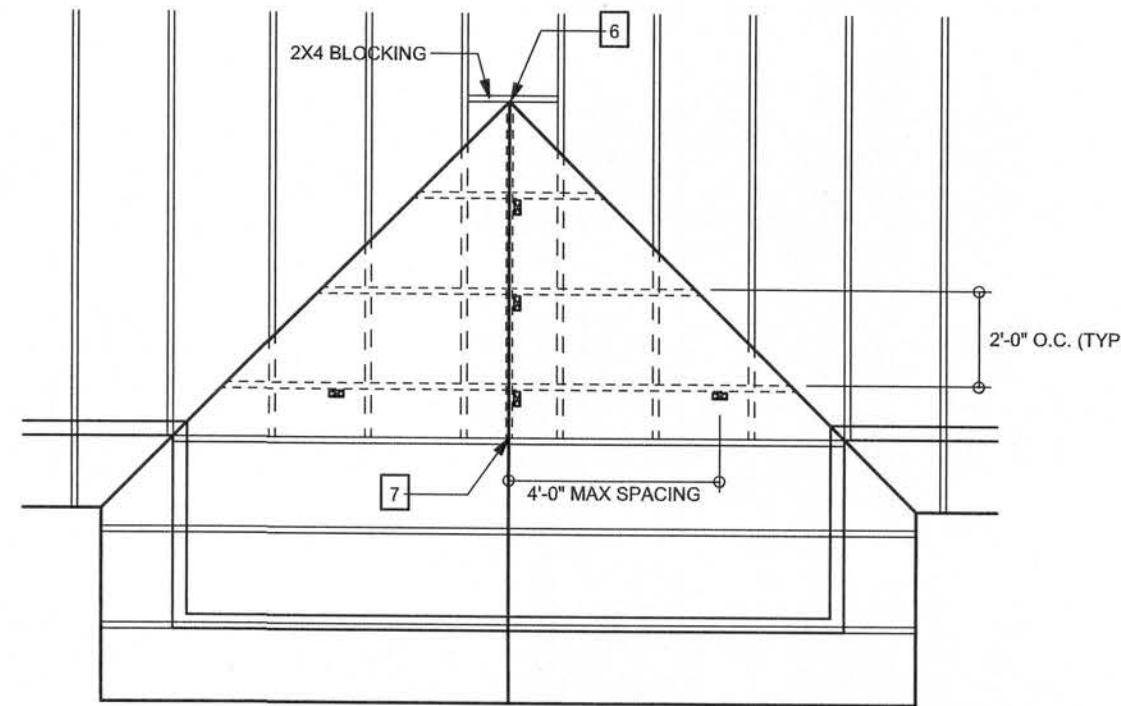
F4 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"



F12 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

LUMBER SIZE & GRADE MINIMUM REQUIREMENTS

RIDGE BOARD	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
PURLINS / LATERAL BRACING	2X4 SPF #2
SLEEPERS	2X (WIDTH OF RAFTER SEAT CUT) SPF #3 OR 2 PARALLEL 2X4 SPF #3
CRIPPLES & BLOCKING	2X4 SPF #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



VALLEY ROOF PLAN MEMBER LEGEND

—	TRUSS
—	TRUSS UNDER VALLEY FRAMING
—	VALLEY RAFTER OR RIDGE
—	CRIPPLE

CONNECTION REQUIREMENT NOTES

1. 2X4 RAFTERS TO RIDGE	3-16d OR 6-131 x 3" TOE NAILS
2. CRIPPLE TO RIDGE	3-16d OR 6-131 x 3" FACE NAILS
3. CRIPPLE TO RAFTERS	3-16d OR 6-131 x 3" FACE NAILS
4. RAFTER TO SLEEPER OR BLOCKING	6-16d OR 12-131 x 3" TOE NAILS
5. SLEEPER TO TRUSS	4-16d OR 8-131 x 3" FACE NAILS EACH TRUSS
6. RIDGE BOARD TO ROOF BLOCK	3-16d OR 6-131 x 3" TOE NAIL
7. RIDGE BOARD TO TRUSS	3-16d OR 6-131 x 3" TOE NAIL
8. PURLIN TO TRUSS (TYP.)	3-16d OR 6-131 x 3" NAILS
9. PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4-16d OR 8-131 x 3" NAILS
10. TRUSS TO BLOCKING	3-16d OR 6-131 x 3" END NAIL
11. CRIPPLE TO TRUSS	3-16d OR 6-131 x 3" FACE NAIL
12. CRIPPLE TO PURLIN	3-16d OR 6-131 x 3" FACE NAIL

GENERAL NOTES

MAXIMUM RAFTER SPAN: 6'-0" FOR 2X4, 8'-0" FOR 2X6 SPF #2 OR SYP #2.

MAXIMUM ROOF AREA PER SUPPORT: 1600 IN ZONES 2 & 3, 2400 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN = 1600 OR 2'-0" X 8'-0" SPAN = 1600)

PURLINS REQUIRED 24" O.C. IF EXISTING SHEATHING IS REMOVED. PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM. IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 6" AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 8-3d COMMON WIRE NAILS.

THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:

- SPANS (DISTANCES BETWEEN HEELS) 40'-0" OR LESS
- MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS
- MAXIMUM WIND SPEED: 120 MPH
- MAXIMUM MEAN ROOF HEIGHT: 30 FEET
- MAXIMUM TOTAL LOADING: 40 psf
- MEETS FBC 2001/ASCE 7-98 WIND REQUIREMENTS
- EXPOSURE CATEGORY "B", I = 1.0, K_z = 1.0
- ENCLOSED BUILDING

CRIPPLE, BRACING & BLOCKING NOTES

2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 8'-0" TO 10'-0" LONG NAILED w/ 2-10d NAILS OR 2X4 "T" OR SCAB BRACE NAILED TO PLAT EDGE OF CRIPPLE WITH 8d NAILS @ 6" O.C. "T" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO CLBS OR BOTH FACES w/ "T" OR SCAB. USE STRESS GRADED LUMBER & BOX OR COMMON NAILS.

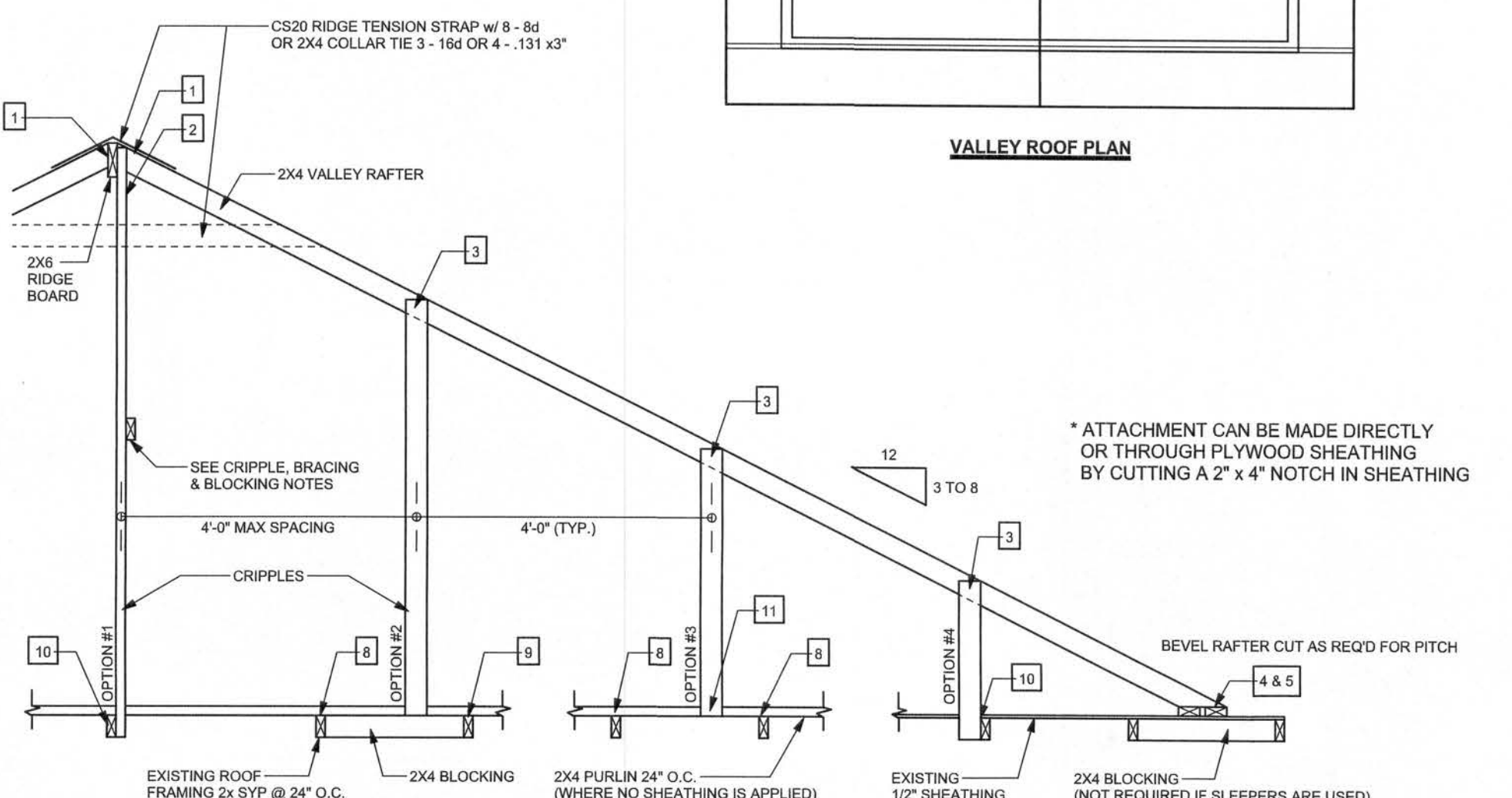
NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD.

INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.

INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN SLEEPERS.

LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.

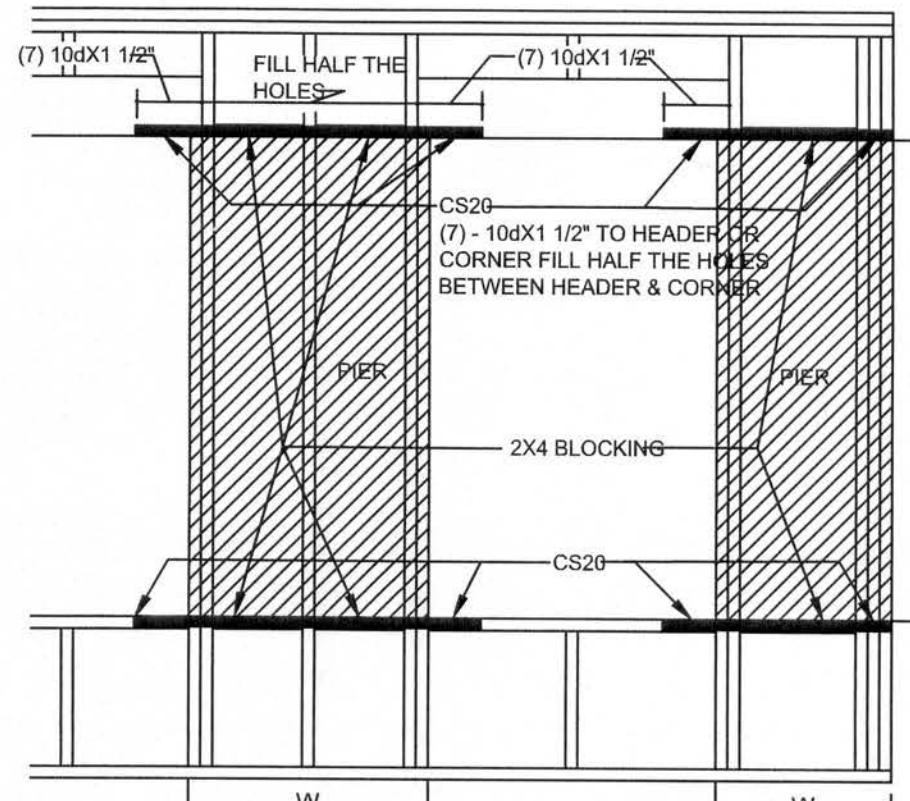
APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.



SECTION CUT PARALLEL TO VALLEY RAFTER

RETROFIT ROOF OVER FRAMING & BRACING DETAIL

SCALE: N.T.S.

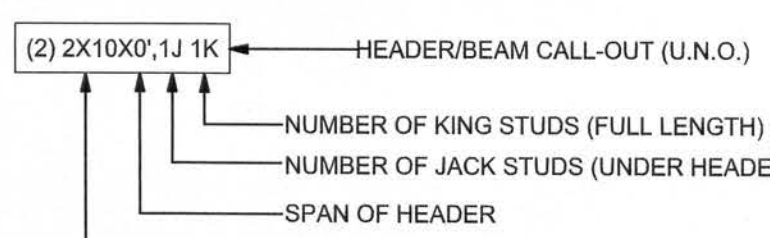


NOTE: THIS DETAIL IS INTENDED TO BE USED ONLY FOR NARROW SHEARWALL SEGMENTS AS SPECIFIED ON THE PLAN. THE PIER BEHIND THE OPENING MUST MEET THE ASPECT RATIO REQUIREMENT HW < 3.5:1 WHERE H IS THE PIER HEIGHT. FOR WINDOWS NOT GREATER THAN 2' HIGH OR 5' WIDE THE WIDTH OF THE OPENING MAY BE INCLUDED AS FULL HEIGHT SHEARWALL IN ADDITION TO THE PIER WIDTH WHEN STRAPPED ACCORDING TO THIS DETAIL.

W68 - OPENING FORCE TRANSFER

SCALE: 1/2" = 1'-0"

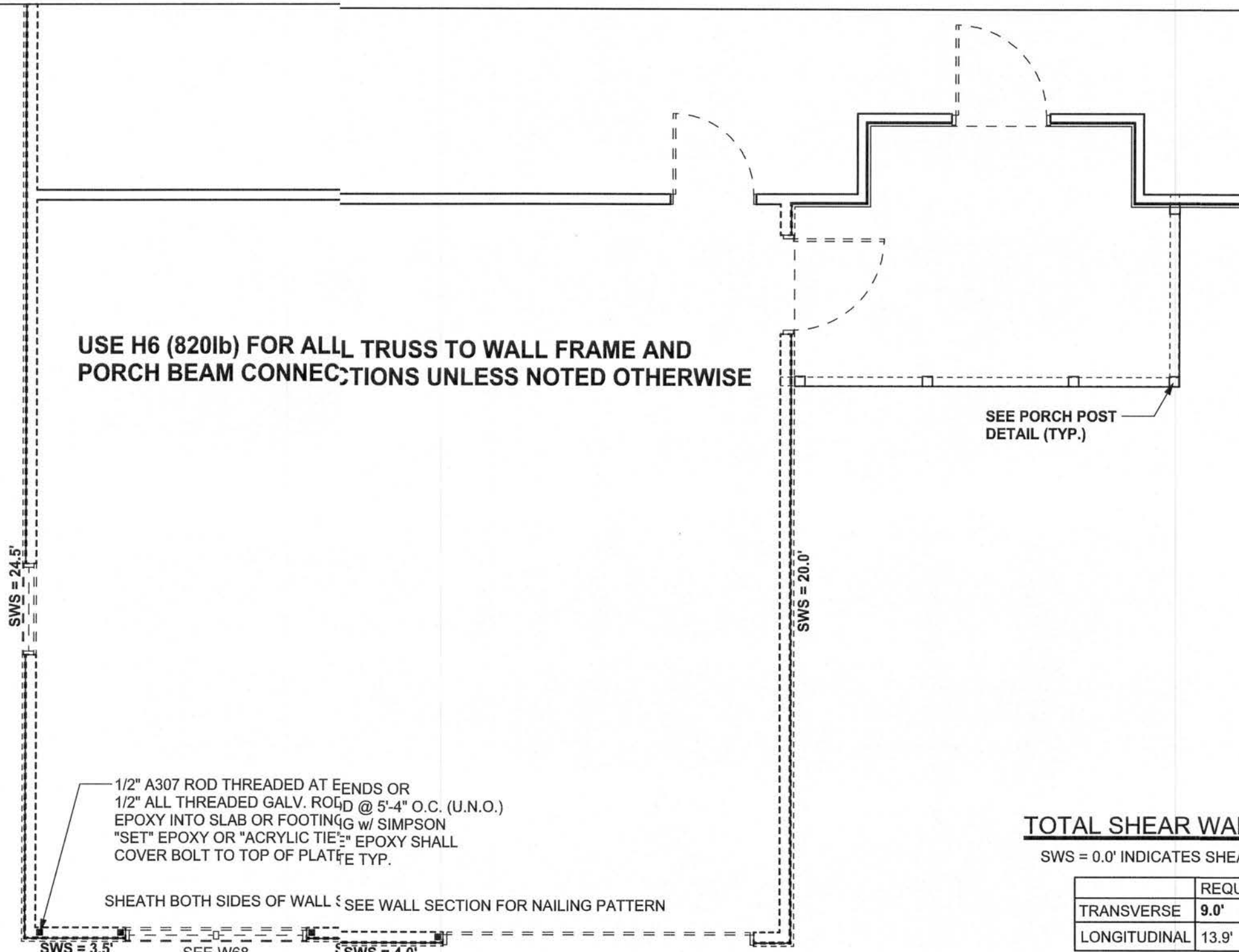
HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	9.0'	28.0'
LONGITUDINAL	13.9'	44.5'



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

USE H6 (820lb) FOR ALL TRUSS TO WALL FRAME AND PORCH BEAM CONNECTIONS UNLESS NOTED OTHERWISE

1/2" A307 ROD THREADED AT ENDS OR 1/2" ALL THREADED GALV. ROD @ 5'-4" O.C. (U.N.O.) EPOXY INTO SLAB OR FOOTING w/ SIMPSON "SET" EPOXY OR "ACRYLIC TIE" EPOXY SHALL COVER BOLT TO TOP OF PLATE TYP.

SHEATH BOTH SIDES OF WALL - SEE WALL SECTION FOR NAILING PATTERN

SEE GARAGE DOOR HEADER STRAPPING DETAIL (TYP.)

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
SWS = 0.0'	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

SIZE F HEADER MATERIAL
NUMBER OF PLIES IN HEADER

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 5385, P.O. Box 868, Lake City, FL 32056, 386-754-9419

DIMENSIONS:
Stated dimensions supersede scaled dimension. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R501.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, specified location.

MARK DISOWAY
P.E. 05915
Mark Disoway
ENR0006
SEAL

CHAD WRIGHT

P. DENIS WESTON RESIDENCE

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Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
November 09, 2006

DRAWN BY:
Ben Sparks

CHECKED BY:

FINALS DATE:
06 / Nov / 06

JOB NUMBER:
610271

DRAWING NUMBER
S-2

OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ?
JOB #?

REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



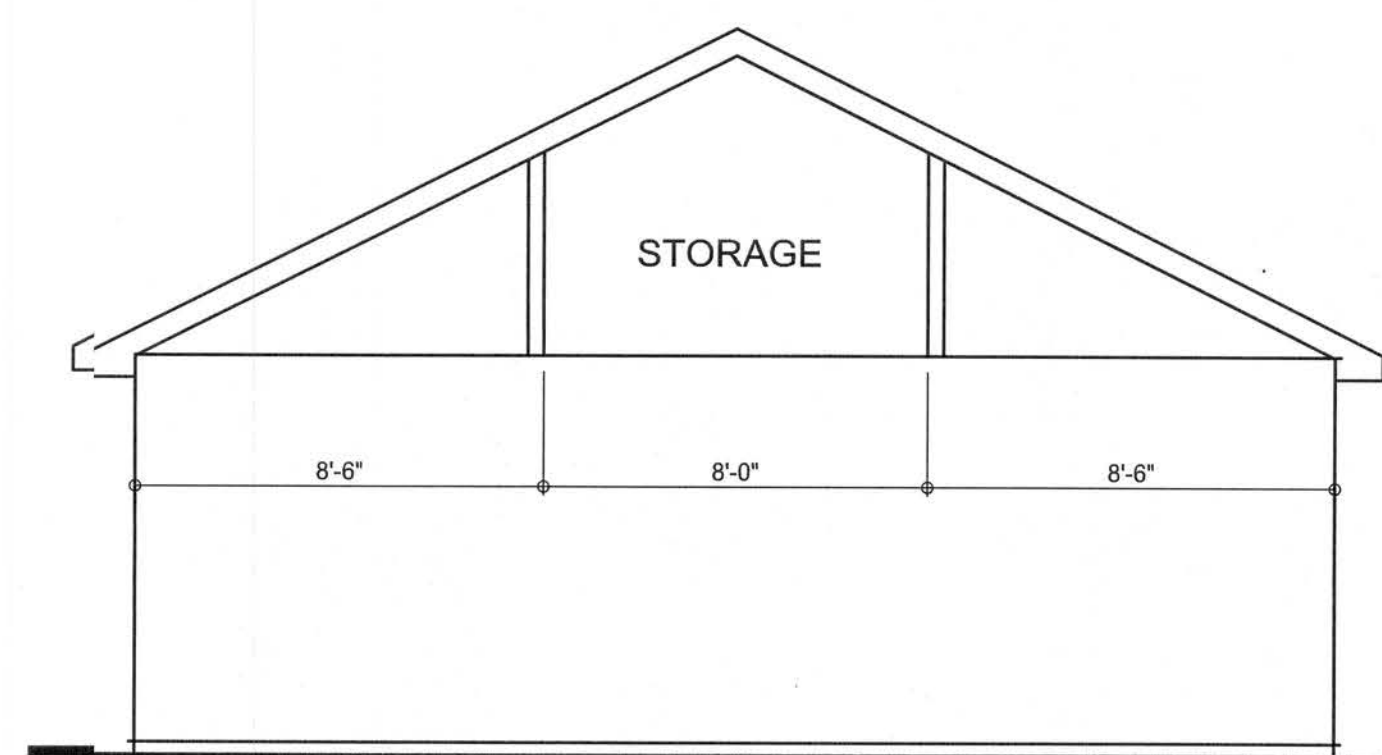
FRONT ELEVATION
SCALE: 1/4" = 1'-0"



LEFT ELEVATION
SCALE: 1/4" = 1'-0"

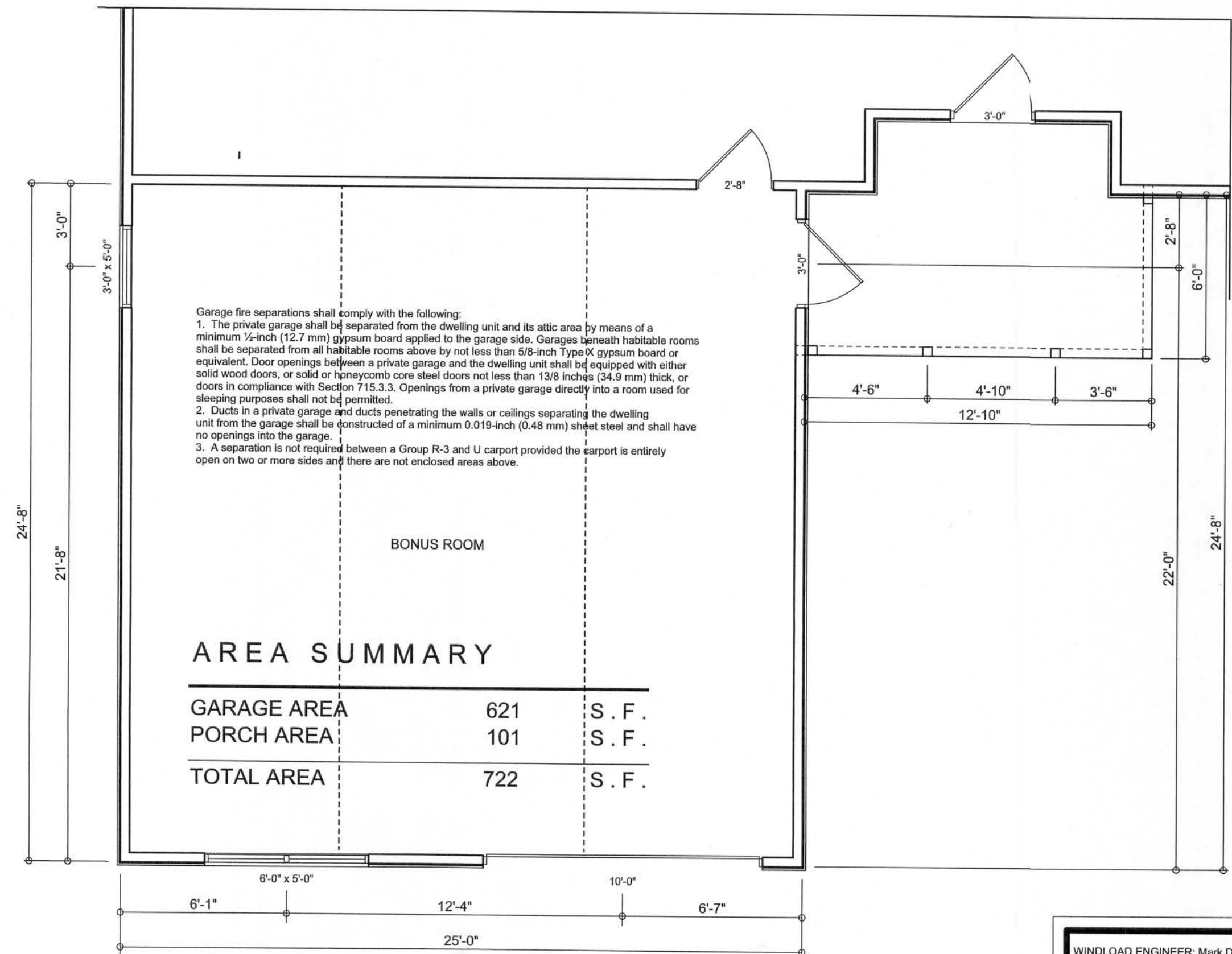


RIGHT ELEVATION
SCALE: 1/4" = 1'-0"



REQUIRED ROOF VENTILATION:
AS PER FLORIDA BUILDING CODE 2309.7

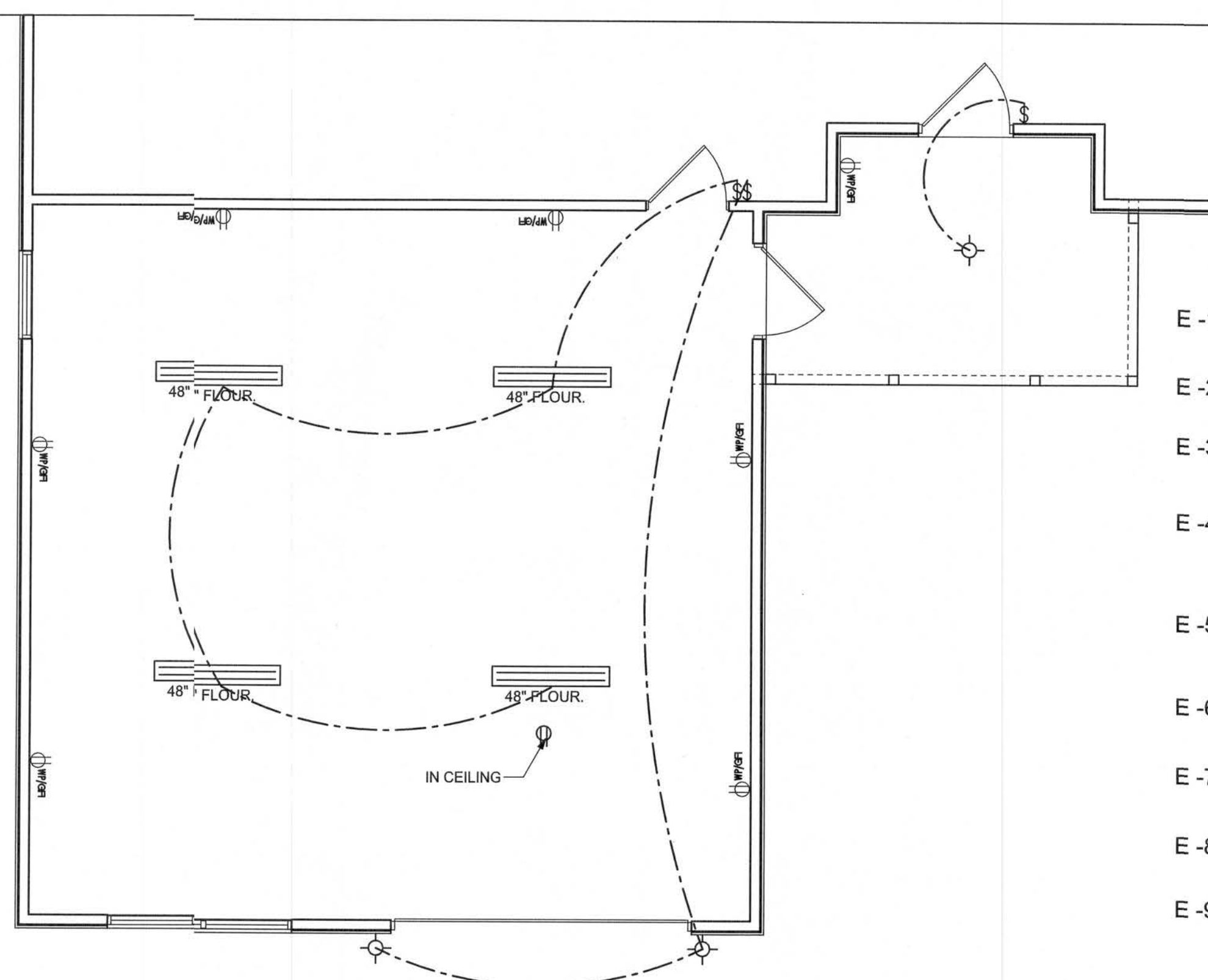
- RIDGE VENT**
MIN. 50% TOTAL VENT AREA
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3" ABOVE EAVE)
722 S.F. / 300 x 50% = 2 S.F. RIDGE VENT AREA REQUIRED
18 FEET OF RIDGE VENT REQUIRED
- SOFFIT VENT**
722 S.F. / 300 x 50% = 2 S.F. SOFFIT VENT AREA REQUIRED
67 FEET OF SOFFIT VENT REQUIRED
- BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:**
1. RIDGE VENTS = 16 IN2/FT (.11 FT2/FT)
 2. OFF-RIDGE VENTS = .70 FT2 PER 4' UNIT
 3. SOFFIT VENTS = 4.3 IN2/FT (.03 FT2/FT)



AREA SUMMARY

GARAGE AREA	621	S . F .
PORCH AREA	101	S . F .
TOTAL AREA	722	S . F .

FLOOR PLAN
SCALE: 1/4" = 1'-0"



ELECTRICAL PLAN NOTES

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E -5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
- E -10 A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING. SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL.

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXHAUST FAN WITH LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	SMOKE DETECTOR
	WALL SWITCH
	3 WAY WALL SWITCH
	4 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	PHONE JACK
	TELEVISION JACK
	GARAGE DOOR OPENER
	WALL HEATER

WINDLOAD ENGINEER: Mark Disosway,
P.E. No. 53915, FOR 866, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

SEAL

CHAD WRIGHT

P. DENIS WESTON
RESIDENCE

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PRINTED DATE:
November 09, 2006

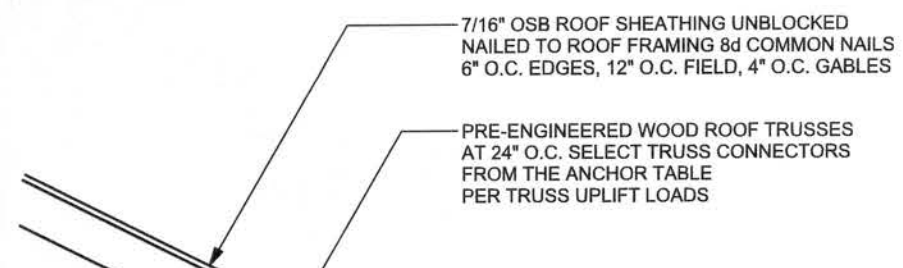
DRAWN BY:
Ben Sparks

CHECKED BY:

FINALS DATE:
06 / Nov / 06

JOB NUMBER:
610271

DRAWING NUMBER
A-1
OF 3 SHEETS

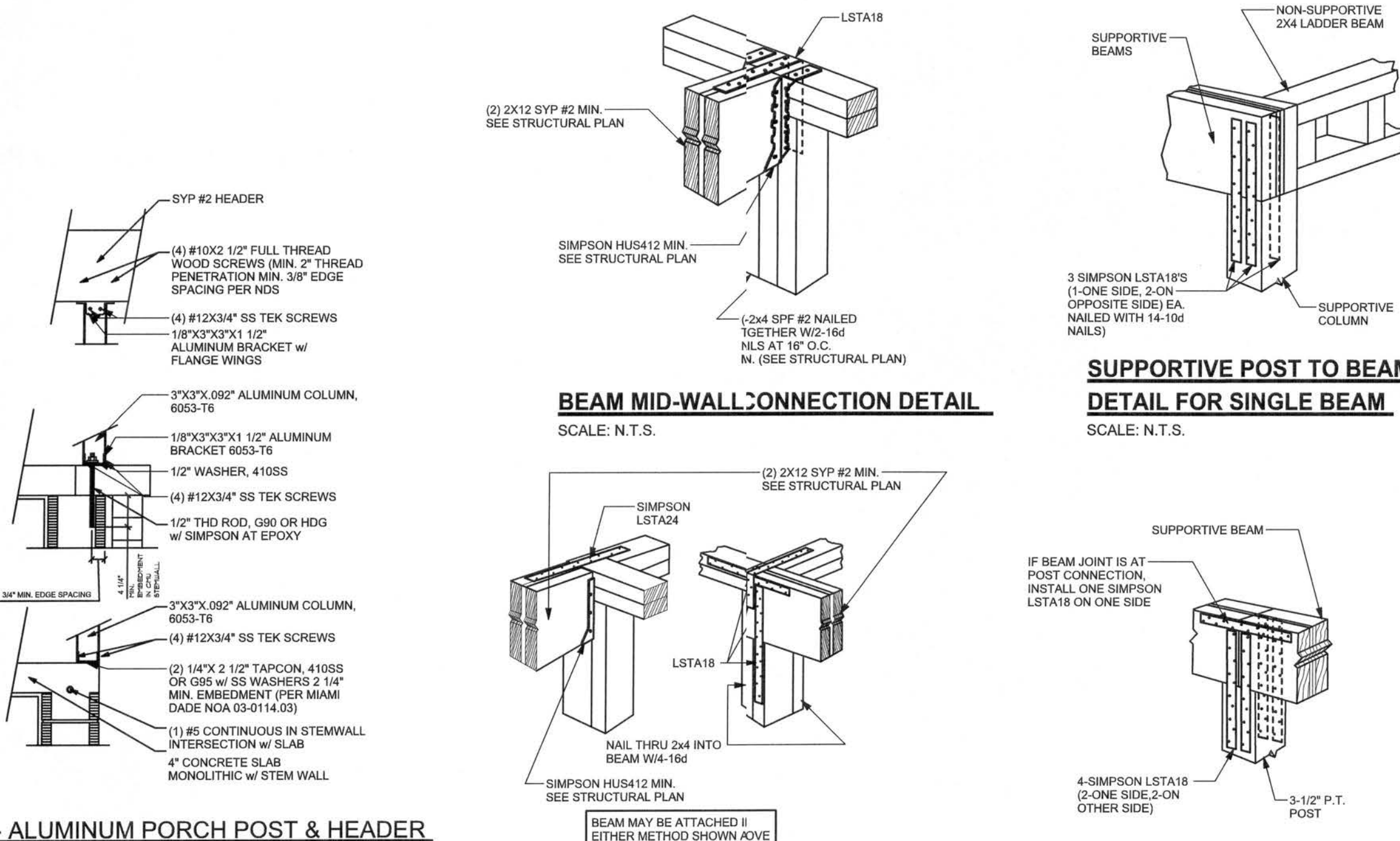


ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

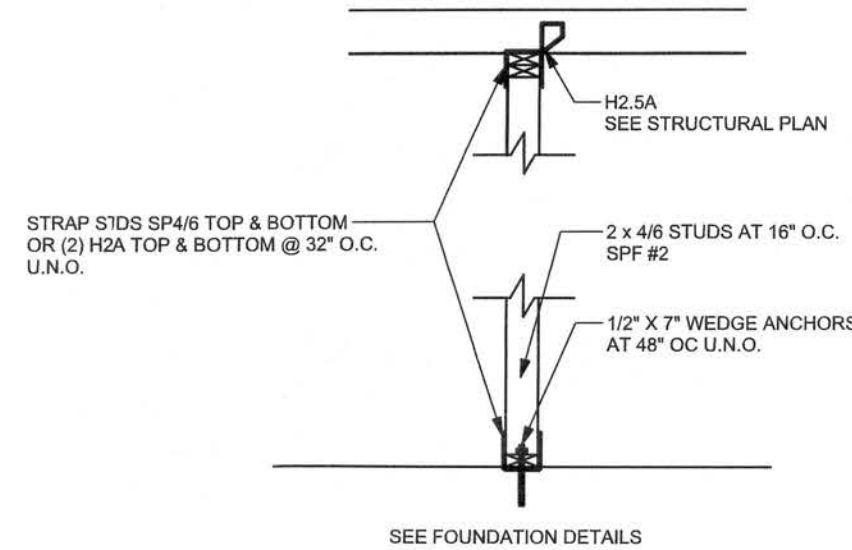
(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B.
EXTERIOR LOAD BEARING A NON LOAD BEARING STUD LENGTHS
RESISTING INTERIOR ZONE WIND LOADS 110 MPH EXPOSURE B.
STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING
LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.
EXAMPLE: 16" O.C. x 0.85 = 13.6" O.C.



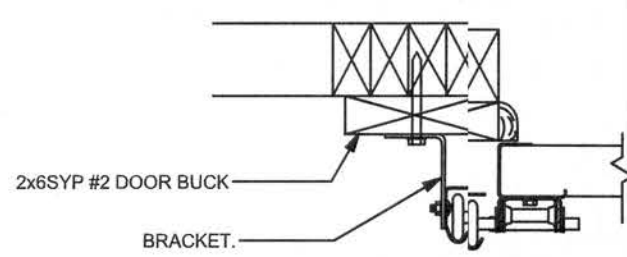
**LLW44 - ALUMINUM PORCH POST & HEADER
ANCHORS**
SCALE: N.T.S. (REVISED)

INTERIOR BEARING WALL
SCALE: 1/2" = 1'-0"



2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT
ATTACH GARAGE DOOR BUCK TO STUD PACK AT
EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG
SCREWS w/ 1" WASHER LAG SCREWS MAY BE
COUNTERSUNK. HORIZONTAL JAMBS MAY NOT
TRANSFER LOAD. CENTER LAG SCREWS OR
STAGGER 16d NAILS OR (2) ROWS OF 1/2" x 3 1/4"
ON PER TABLE BELOW:

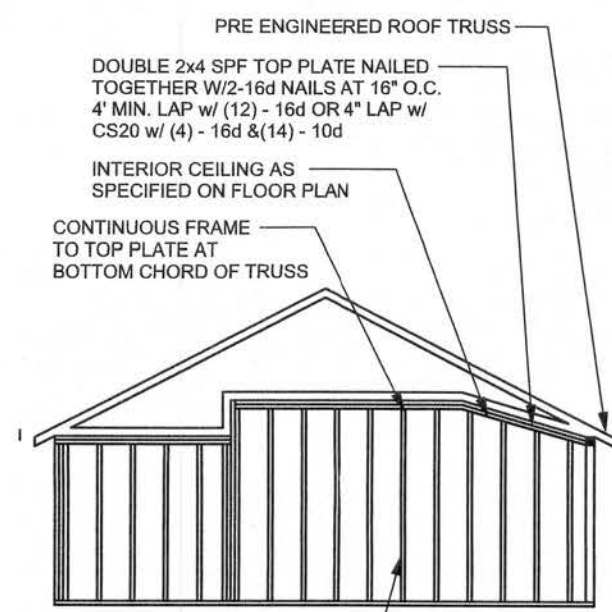
DOOR WIDTH	3/8" x 4" LAG	16d STAGGER	(2) ROWS OF 1/2" x 3 1/4" GN
8' - 10'	24" O.C.	5" O.C.	5" O.C.
11' - 15'	18" O.C.	4" O.C.	4" O.C.
16' - 18'	16" O.C.	3" O.C.	3" O.C.



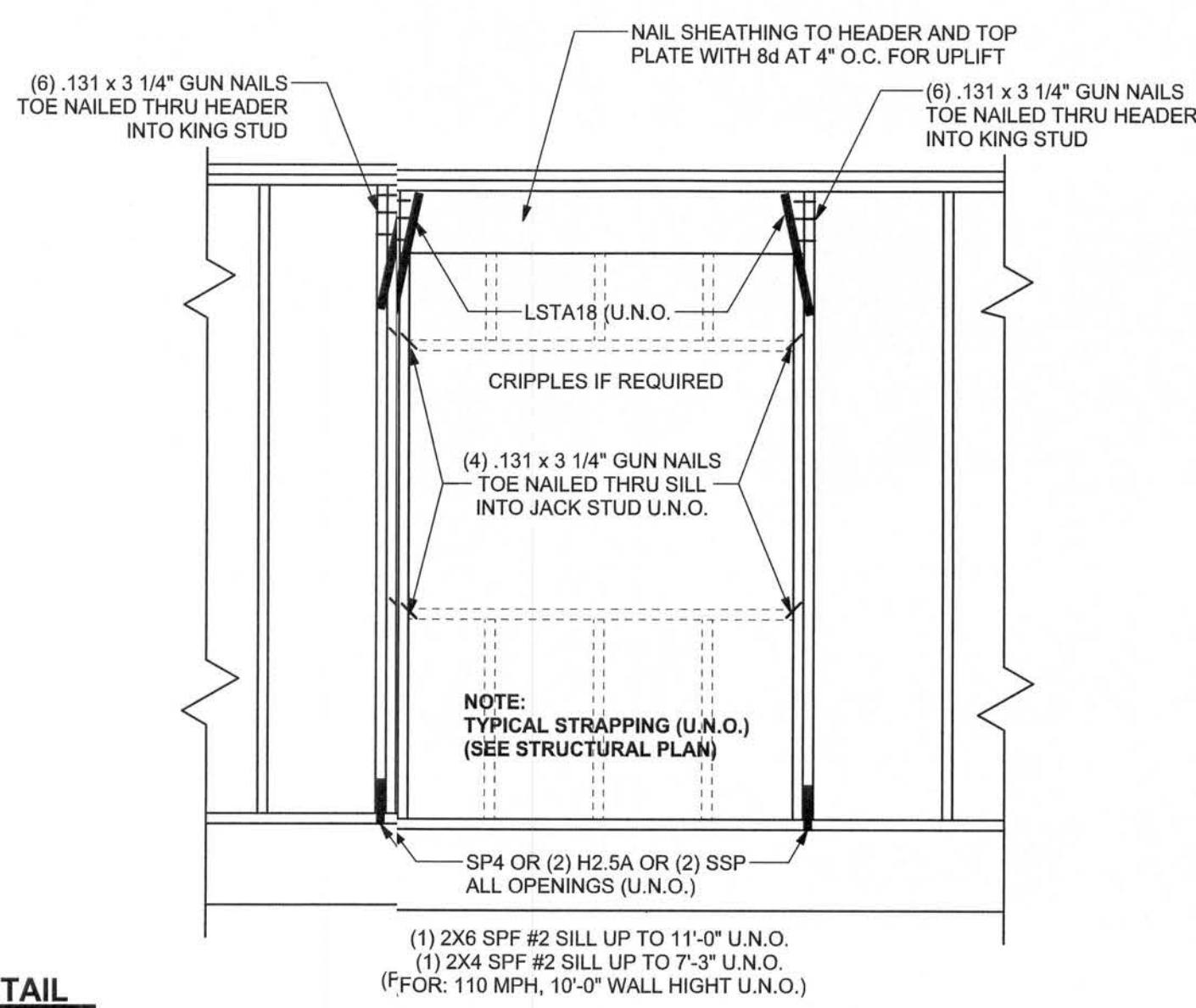
GARAGE DOOR BUCK INSTALLATION DETAIL
SCALE: N.T.S.

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



**CONTINUOUS FRAME TO
CEILING DIAPHRAGM DETAIL**
SCALE: N.T.S.



GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE
FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND
PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR
ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER
AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S
RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO
SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR
INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR
REVIEW OF TRUSS REACTION ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT
CONNECTION #15.8 EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET
GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS
VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, F_y = 85KSI, WELDED WIRE REINFORCEMENT FABRIC
(W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED
MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT.
FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD
PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER
TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

REBAR: ASTM A615, GRADE 60, DEFORMED BARS, F_y = 60 KSI. ALL LAP SPLICES 40" OR (25" FOR #5 BARS);
UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-86, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F_b = 2400 ksi, E = 1800 ksi. UNO. SUPPLIER MAY SUPPLY AN
ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, 7/16" OSB SHEATHING UNLOCKED,
APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES
STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6" OC PANEL EDGES, 12"OC INTERMEDIATE
MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS,
AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE
SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE
TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION
INSTRUCTIONS TO ACHIEVE THE FOLLOWING UPLIFT LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO
LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH
3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST
REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

**THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE
SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.**

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND
BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004
REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU
BELIEVE THE PLAN OMITTS A CONTINUOUS LOAD PATH CONNECTION, CALL
THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS
DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS,
TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL
BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION
R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN
TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS
THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE
COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS
MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN
PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED
LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO
REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF
SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL
BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF
DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT
RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE
TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES
RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED
TRUSS SHEETS.

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL
CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY
STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON
MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF
ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.
ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER
IN WRITING.

ACI/530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F _m = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 270, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 60, F _y = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class C60, 0.60 oz/ft ² or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft ² or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS
MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 565	< 520	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 760	< 650	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 1470	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1050	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1465	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LTG2	14-16d	14-16d	
HEAVY GIRDER TIEDOWNS*					
< 3965	< 3330	MG7		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
STUD STRAP CONNECTOR*					
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	6-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP8			6-10d, 1 1/2"
< 1240	< 1065	SPH8			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
STUD ANCHORS*					
< 1350	< 1305	LT119	8-16d		1/2" AB
< 2310	< 2310	LT131	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18-16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18-16d		2-5/8" AB

DESIGN DATA

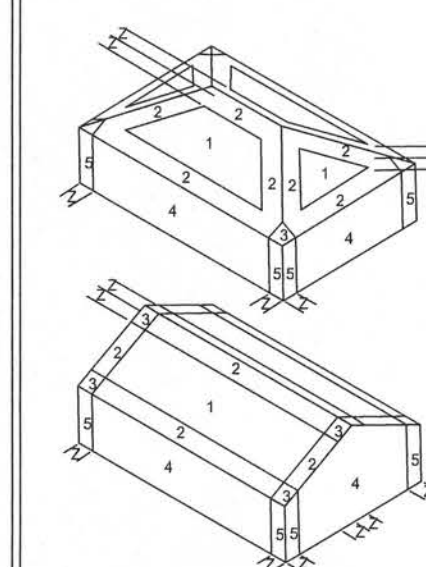
WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS;
MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 80 FT; NOT
ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 30 FT IN EXP. C AND 10%
SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- BASIC WIND SPEED = 130 MPH
- WIND EXPOSURE = B
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = <30 FT
- INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



Zone	Effective Wind Area (ft ²)	10	20	100
1	19.9	21.8	18.1	-18.1
2	19.9	25.5	18.1	-21.8
2 O'Hg		40.6		-40.6
3	19.9	25.5	18.1	-21.8
3 O'Hg		48.3		-42.4
4	21.8	23.6	19.5	-20.4
5	21.8	29.1	18.5	-22.6
Doors & Windows Worst Case (Zone 5, 10 ft ²)		21.8	-29.1	
8x7 Garage Door		19.5	-22.9	
16x7 Garage Door		18.5	-21.0	

DESIGN LOADS

FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
	SOIL BEARING CAPACITY 1000PSF
	NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WIND LOAD ENGINEER: Mark Disoway,
P.E. No. 53915, POB 866, Lake City, FL
3206, 386-754-5419

DIMENSIONS:
Sheet dimensions supersede scaled
dimensions. Refer all questions to
Mark Disoway, P.E. for resolution.
Do not proceed without clarification.

REVISIONS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

VALLEY ROOF PLAN MEMBER LEGEND

===== TRUSS
===== TRUSS UNDER VALLEY FRAMING
===== VALLEY RAFTER OR RIDGE
===== CRIPPLE

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAY

CONNECTION REQUIREMENT NOTES

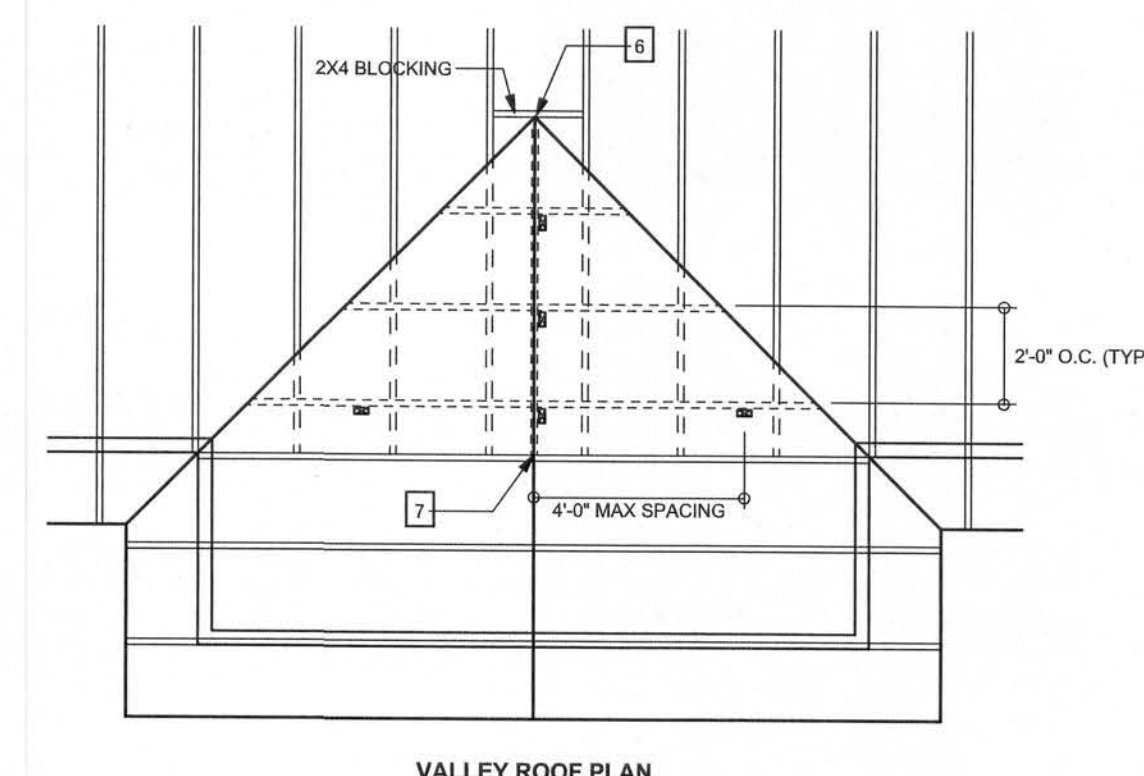
1	2X4 RAFTERS TO RIDGE	3-16d OR 6-131 x 3" TOE NAILS
2	CRIPPLE TO RIDGE	3-16d OR 6-131 x 3" FACE NAILS
3	CRIPPLE TO RAFTERS	3-16d OR 6-131 x 3" FACE NAILS
4	RAFTER TO SLEEPER OR BLOCKING	6-16d OR 12-131 x 3" TOE NAILS
5	SLEEPER TO TRUSS	4-16d OR 8-131 x 3" FACE NAILSEACH TRUSS
6	RIDGE BOARD TO ROOF BLOCK	3-16d OR 6-131 x 3" TOE NAILS
7	RIDGE BOARD TO TRUSS	3-16d OR 6-131 x 3" TOE NAILS
8	PURLIN TO TRUSS (TYP.)	3-16d OR 6-131 x 3" NAILS
9	TRUSS TO BLOCKING	4-16d OR 8-131 x 3" NAILS
10	CRIPPLE TO TRUSS	3-16d OR 6-131 x 3" FACE NAILS
11	CRIPPLE TO PURLIN	3-16d OR 6-131 x 3" FACE NAILS

GENERAL NOTES

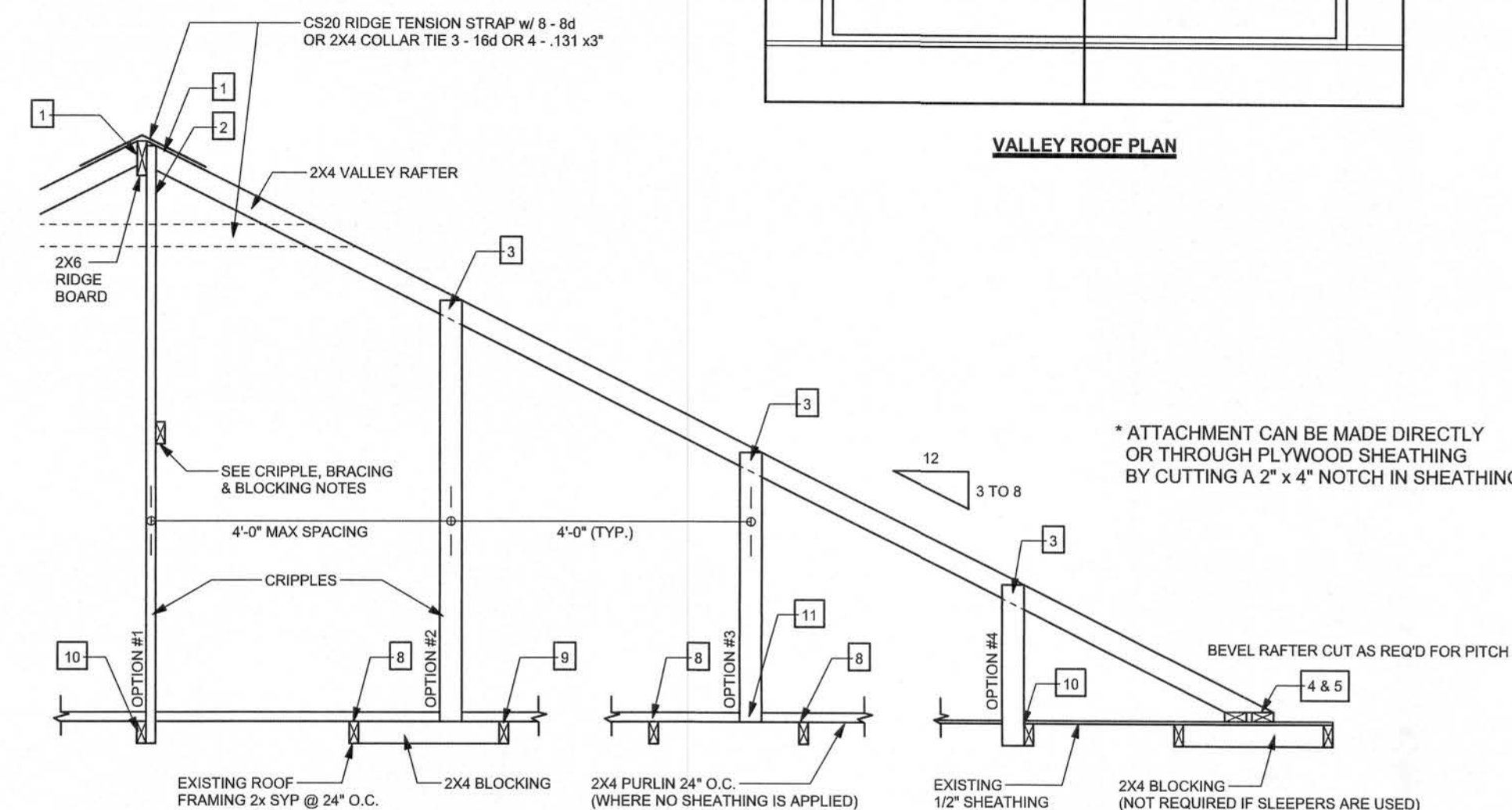
MAXIMUM RAFTER SPANS:
6'-0" FOR 2X4, 9'-0" FOR 2X6 SYP #2 OR SYP #2
MAXIMUM ROOF AREA PER SUPPORT
1602 IN ZONES 2 & 3, 2402 IN ZONE 1, (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN = 1602 OR 2'-0" X 8'-0" SPAN = 1602)
PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.
PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM.
IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 6", AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 8-8d COMMON WIRE NAILS.
THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
- SPANS (DISTANCES BETWEEN HEELS) 40'-0" OR LESS
- MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS
- MAXIMUM WIND SPEED: 120 MPH
- MAXIMUM MEAN ROOF HEIGHT: 30 FEET
- MAXIMUM TOTAL LOADING: 40 psf
- MEETS FBC 2001/ASCE 7-98 WIND REQUIREMENTS
- EXPOSURE CATEGORY "B", I = 1.0, Ks = 1.0
- ENCLOSED BUILDING

CRIPPLE, BRACING, & BLOCKING NOTES

- 2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG NAILLED W/ 2-10d NAILS OR 2X4 1" OR SCAB BRACE NAILD TO FLAT EDGE OF CRIPPLE WITH 8d NAILS @ 8" O.C. 1" OR SCAB MUST BE 80% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO CLBs OR BOTH FACES W/ 1" OR SCAB. USE ITRESS GRADED LUMBER & BOX OR COMMON NAILS.
- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD.
- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.
- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.
- APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.



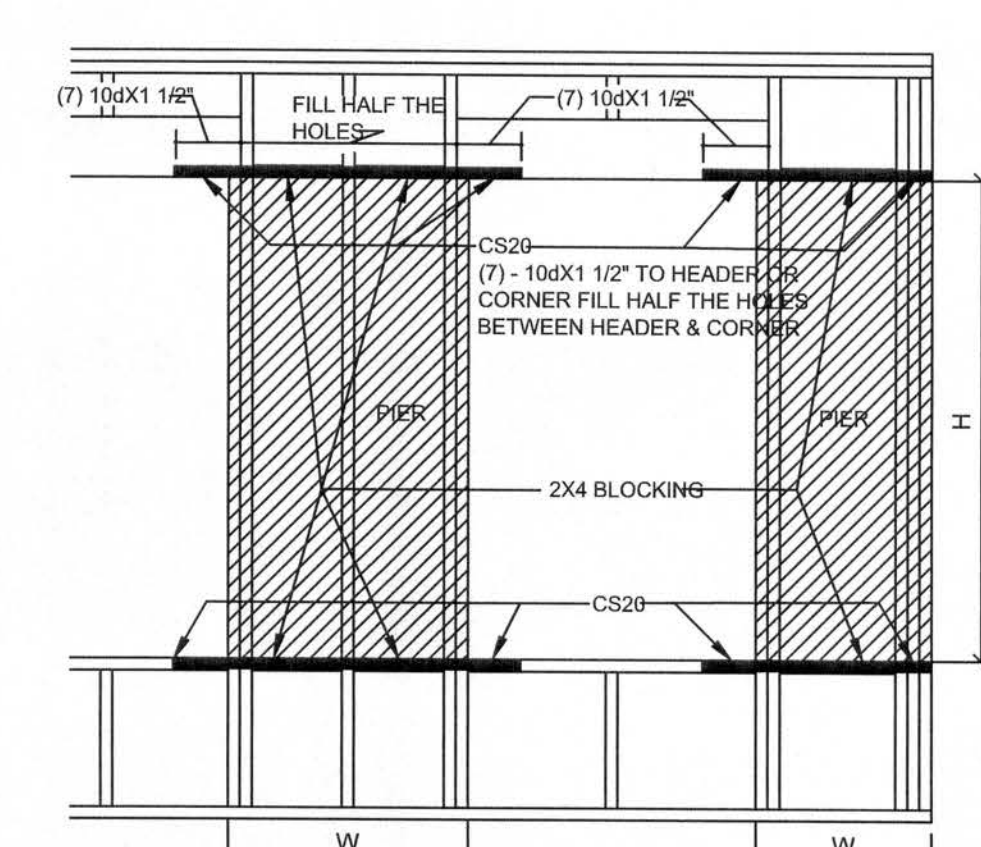
LUMBER SIZE & GRADE MINIMUM REQUIREMENTS	
RIDGE BOARD	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
PURLINS / LATERAL BRACING	2X4 SYP #2
SLEEPERS	2X (WIDTH OF RAFTER SEAT CUT) SYP #3 OR 2 PARALLEL 2X4 SYP #3
CRIPPLES & BLOCKING	2X4 SYP #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



SECTION CUT PARALLEL TO VALLEY RAFTER

RETROFIT ROOF OVER FRAMING & BRACING DETAIL

SCALE: N.T.S.

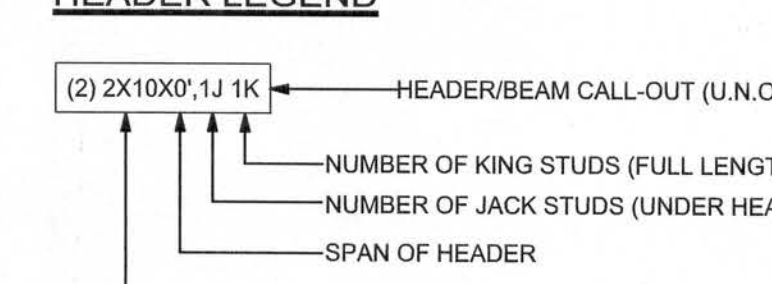


NOTE: THIS DETAIL IS INTENDED TO BE USED ONLY FOR NARROW SHEARWALL SEGMENTS AS SPECIFIED ON THE PLAN. THE PIER BESIDE THE OPENING MUST MEET THE ASPECT RATIO REQUIREMENT HW < 3.5:1 WHERE H IS THE PIER HEIGHT. FOR WINDOWS NOT GREATER THAN 2' HEIGHT OR 5' WIDE THE WIDTH OF THE OPENING MAY BE INCLUDED AS FULL HEIGHT SHEARWALL IN ADDITION TO THE PIER WIDTH WHEN STRAPPED ACCORDING TO THIS DETAIL.

W68 - OPENING FORCE TRANSFER

SCALE: 1/2"=1'-0"

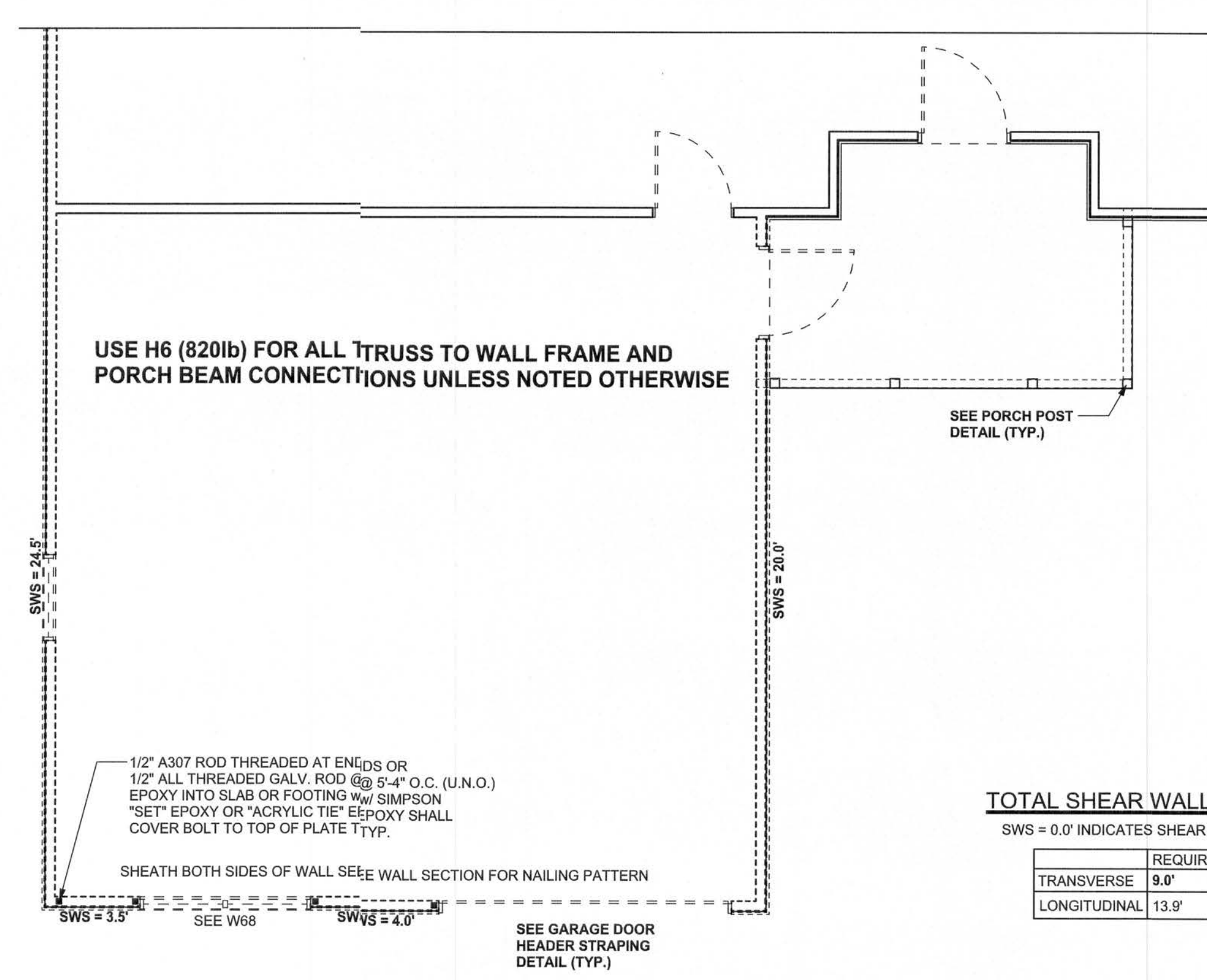
HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	9.0'	28.0'
LONGITUDINAL	13.9'	44.5'

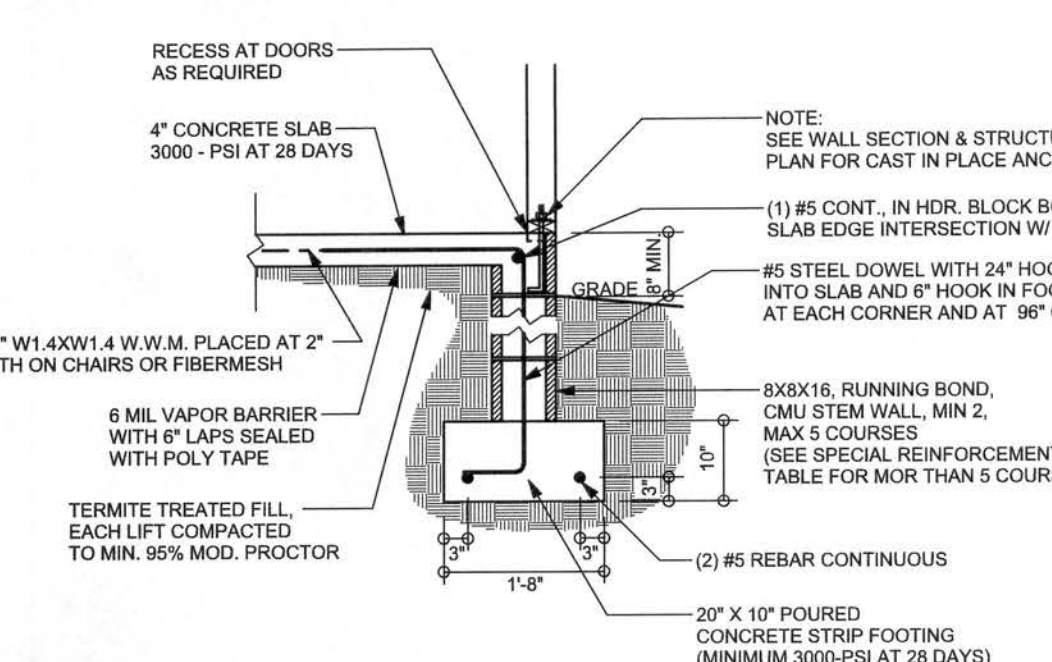


STRUCTURAL PLAN

SCALE: 1/4"=1'-0"

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
SWS = 0.0'	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1



STEM WALL FOOTING

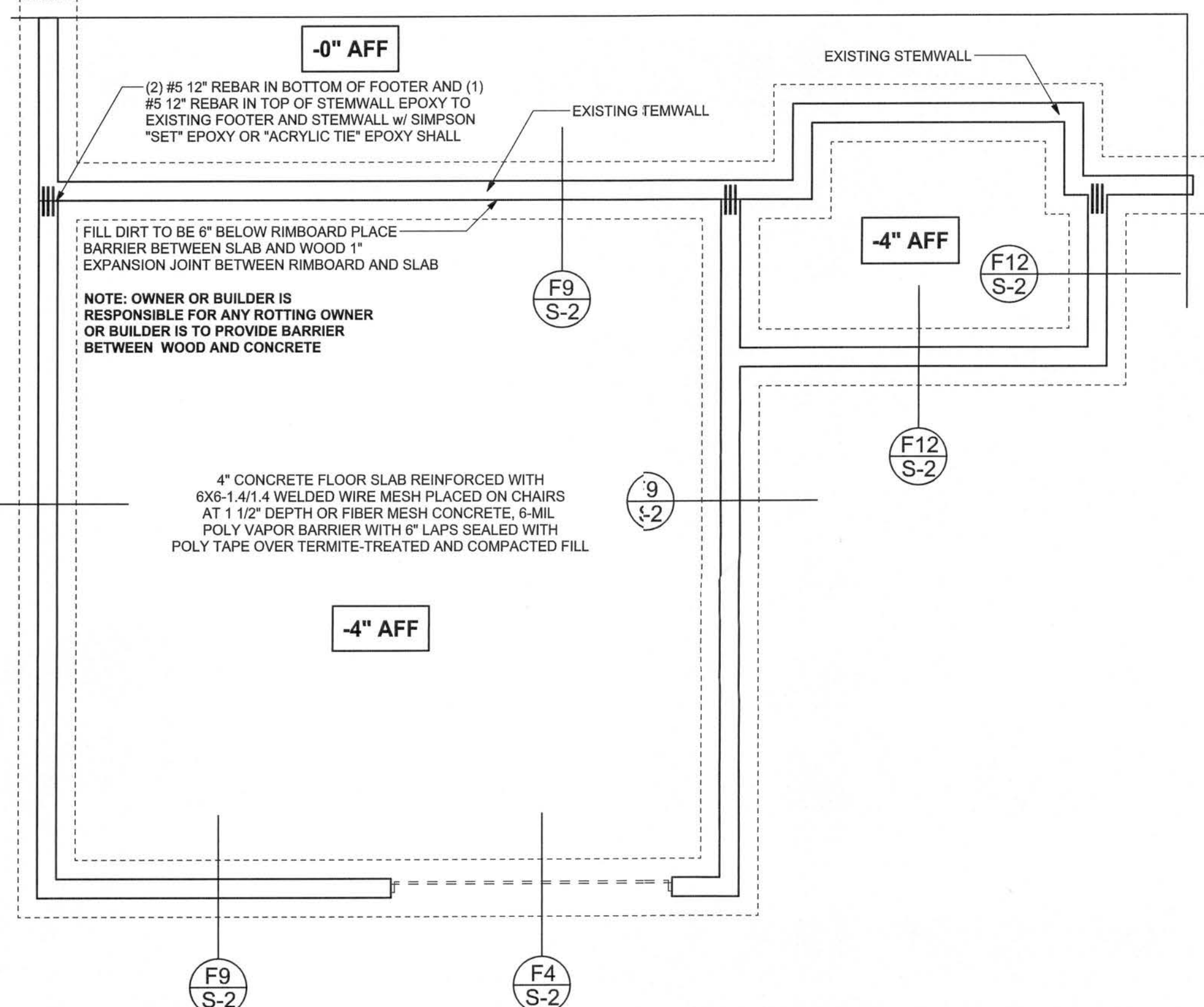
SCALE: 1/2"=1'-0"

GARAGE DOOR FOOTING

SCALE: 1/2"=1'-0"

ALT. STEM WALL PORCH FOOTING

SCALE: 1/2"=1'-0"



FOUNDATION PLAN

SCALE: 1/4"=1'-0"

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, 708 868, Lake City, FL 32056, 386-75-5415

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section 6301.2.1, Florida building code (residential) 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915

SEAL

CHAD WRIGHT

P. DENIS WESTON RESIDENCE

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PRINTED DATE:

November 09, 2006

DRAWN BY: Ben Sparks

CHECKED BY:

FINALS DATE:
06 / Nov / 06

JO3 NUMBER:
610271

DRAWING NUMBER

S-2

OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ?
JOB #?