

BUILDING LOAD SUMMARY

BUILDING CODE: FLORIDA BUILDING CODE

ED: 7TH EDITION

JOB LOCATION: LAKE CITY, FL

>>> ROOF LOADS:

Dead Load: 0.73 psf Live Load: 20 psf Tributary Reduction: X Y N

Collateral Load: 0 psf Sprinkler Load: 0 psf

Ground Snow Load: N/A psf Flat roof Snow load: N/A psf

Snow exposure: N/A Snow Importance: N/A Thermal Factor: N/A

>>> WIND LOADS:

Wind Velocity (ULT): 120 mph Fastest Mile: N/A 3-sec Gust: X

Wind velocity (ASD): 93 mph Risk Category: II Wind Exposure: C

Enclosure: ☒ Encl ☐ Partial ☐ Open Internal pressure coefficient: ± 0.18

Components and Cladding (Design):

Zone1 +9.60 / -23.16 psf Zone 4 +19.54 / -21.17 psf

Zone2 +9.60 / -32.21 psf Zone 5 +19.54 / -26.06 psf

Zone3 +9.60 / -50.30 psf

>>> EARTHQUAKE LOADS:

Base Seismic Force Resisting System: N/A

Design Base Shear: N/A Analysis Used: N/A Seismic Importance: N/A

Risk Category: N/A Site Class: N/A Seismic Design Category: N/A

S(S): N/A q S(1): N/A q C(S): N/A R: N/A

>>> MEZZANINE LOADS:

Dead Load: N/A psf Live Load: N/A psf Reduction: N/A %

>>> OTHER LOADS:

NONE



	SHEETING				FRAMING SCREW			STITCH SCREW		
	PANEL TYPE	APPROVAL #	MATL.	COLOR	MARK	PART #	SPACING	MARK	PART #	SPACING
ROOF	RIB-12	FL3774.1	26 GAGE	GL2	◇	HW5510	12" O.C.	◇	HW5502	20" O.C.
WALLS	RIB-12	FL3775.1	26 GAGE	???	◇	HW5515	12" O.C.	◇	HW5511	30" O.C.

FLASHING		
	MATL.	COLOR
EAVE FLASHING	26 GAGE	??
RAKE	26 GAGE	??
CORNERS	26 GAGE	??
OTHER	26 GAGE	??

ALTHOUGH WE TRY TO MAKE ALL DRAWINGS AS ACCURATE AS POSSIBLE, ANY DRAWING MARKED AS PERMIT OR APPROVAL HAS NOT BEEN FINAL DETAILED OR CHECKED. THE CUSTOMER ACCEPTS ALL RESPONSIBILITY FOR USING PERMIT OR APPROVAL DRAWINGS FOR ANY PURPOSE OTHER THAN OBTAINING A BUILDING PERMIT OR REVIEWING THE DETAILS FOR CONTRACT COMPLIANCE.

NOTE:
ALL ZEES, CHANNELS AND EAVE STRUTS
ARE PRE-PAINTED RED COLOR.

IMPORTANT --- PLEASE READ THE FOLLOWING:

>>> General
Dean Steel Buildings, Inc. standard product specifications apply, unless stipulated otherwise on the Dean Steel Buildings' purchase order. Dean Steel Buildings design practices, manufacturing processes, accessories and warranties will govern the work. A Dean Steel Buildings Standard Specifications Brochure is available upon request.

The builder/contractor is responsible for the setting of the anchor bolts and the erection of the steel building components in accordance with Dean Steel Buildings, Inc. "For Construction" drawings.

>>> Unloading Check-out and Shortages:
It is essential, while unloading your Dean Steel Building, to verify that all components listed on the shipping papers were actually shipped. The actual quantities of the items on each truck are circled on the shipping papers. Any items found to be shipped short, or in direct conflict with the shipping papers should be noted on the driver's report. This serves two purposes: 1) You, as the erector, will be aware that a particular item is missing and can temporarily work around it; 2) it lets us know that we have a problem and allow us to react quickly. This system eliminates the discovery that something is missing at the time you need it to erect the building. Taking the time to properly unload the job and check it allows for proper placing of the parts around the job site, which should expedite the erection process. Dean will do its utmost to fill any reported shortages as quickly as possible. Once the erector, or owner, signs that he has received the goods and accepted them as being complete, we can only assume that shortages after this point are missing due to job site theft. All claims for damage or shortage must be presented, in writing, to the carrier—either Dean Steel Buildings, Inc., or common carrier, within seven days after receipt of materials by purchaser. Failure to do so voids any claim.

>>> Storage and Protection of Materials:
A Galvanic action known as "white rust" may result when aluminum, galvanized, or the galvanized pre-painted coating on piled flat sheets or nested formed sheets becomes wet from rain, condensation, or other causes. Under certain weather conditions this "white rust" can happen in as little as 24-48 hours. Galvalume coated sheet is susceptible, as is galvanized sheet, to wet storage staining. However, due to the composition of the coating, the sheet surface will develop a dark gray discoloration as opposed to the white oxide that develops on galvanized. Formed pre-painted sheets must be protected from moisture, in the same manner as plain galvanized or Galvalume sheets, if they are in contact with other sheets. The sheets must be properly packaged and stored. It is important upon receipt of material to examine packages for damage. Builders should take prompt action where cuts, tears, or other damage is evident. If moisture is present the panel should be dried at once.
Panels that cannot be stored out of the elements should be re-stacked individually and spacers put between the panels, so that individual panels can have air circulated around them (non-metallic spacers; i.e., wood, cardboard, etc.). Bundled panels should be off the ground sufficiently to prevent rising water from coming in contact with the panels. Bundled panels should also be staked so that any condensation may be drained off. All bundled panels should be thoroughly covered with a waterproof canvas tarp. Do not use non-breathing materials such as plastic because they prevent air passage and tend to trap moisture in the bundle. Roof and side panels should be erected as soon as possible after their arrival at the job site. If prolonged job site storage will be required, the builder is advised to seek storage of the panels out of the elements.

>>> Proper Erection Practices:
Dean requires that erection of its products be done by experienced pre-engineered metal building assemblers. Erecting and construction methods should be performed as outlined in the "American Institute of Steel Construction Code of Standard Practices for Steel Buildings and Bridges, Part 5." In addition, the erection policies and practices of both the SBA Independent Erectors Division, and MBMA erection practices must be understood and adhered to. The quality of erection has a direct bearing on the quality of the end product. If there are any questions as to these drawings on the Dean Steel Building system, please do not hesitate to contact our Customer Service Department (239) 334-1051.

Temporary supports or bracing required for the building erection is the responsibility of the erector to determine, furnish and install.

>>> Back Charge Claim Procedure:
Dean Steel Buildings, Inc., follows the back charge claim procedure adopted by MBMA and as outlined in the MBMA Low Rise Building Systems Manual, Common Industry Practices Section 6.10. The Customer Service Manager must be notified at once when a condition becomes apparent, which may result in back charge by the builder or erector. Notification by phone must be confirmed in writing. Some approximation of the amount of the back charge must be established at this time, and a written authorization from the Customer Service Manager must be secured before the work is started.

Dean will not honor any field corrections or back charges unless prior notice has been given and agreed upon. All discrepancies must be agreed upon, in writing, and accompanied with a Dean purchase order number before Dean will honor any back charges. Dean will then pay this agreed amount upon presentation of a final claim. Payment will be by credit memo to the Builder's account.

Any work which is undertaken without such notification and authorization, for which the builder expects to back charge Dean, will not be honored as a back charge.

Should a discrepancy exist, Dean may elect to do one of the following:

- 1) Ship material from its plant for field correction (freight allowed)
- 2) Purchase material locally (or allow builder to do so) for field correction
- 3) Modify existing materials previously shipped to conform to requirements
- 4) Return material to Dean's plant for exchange or modification requirements

When delivery is contracted by Dean, it is our carrier's intent to arrive on the job site at a pre-designated time and every effort will be made to do so. However, Dean will not accept any back charges due to late arrivals.

Dean Steel Buildings, Inc., shall not, however, be liable to builder in any way or for any reason on account of any change in Dean Steel Buildings, Inc., product lines. Dean will not pay any back charges for delays that may be incurred due to shortages. Dean will not pay any claims on account of improper unloading of material, improper storage of material, or delays or damages caused by improper erection techniques. Dean Steel Buildings, Inc., may make changes from time to time in their product lines by discontinuing, altering, or modifying any or all of the products included therein and by adding new and additional products thereto.

Genesis Construction has reviewed the material shown and marked in this submittal for general compliance with construction drawing and specifications.
Reviewed by:
Date: 11/01/2022

P	PERMIT	10/11/22	YAN
ISSUE	REVISION	DATE	BY

THIS SET OF DRAWINGS IS FOR

☒ PERMIT

☐ CONSTRUCTION

☐

DEAN

STEEL BUILDINGS, INC.

2929 INDUSTRIAL AVE.

FORT MYERS, FLORIDA

33901

JOB NUMBER

TV01633

DRAWN

YAN

DATE

10/11/22

CHECKED

YAN

DATE

10/11/22

SHEET NUMBER

1 OF 7

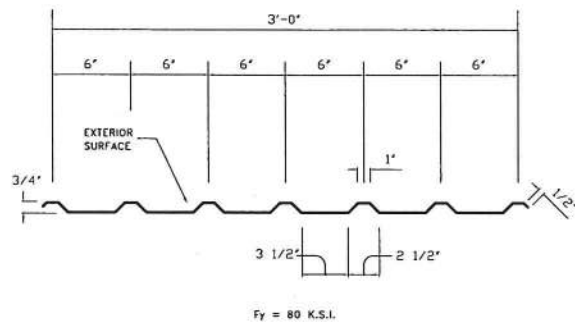
CUSTOMER - GENESIS CONST. & MANAGEMENT

PROJECT - TOWNHOMES SHED 1

DESCRIPTION -

JOB INFORMATION

RIB - 6 PANELS



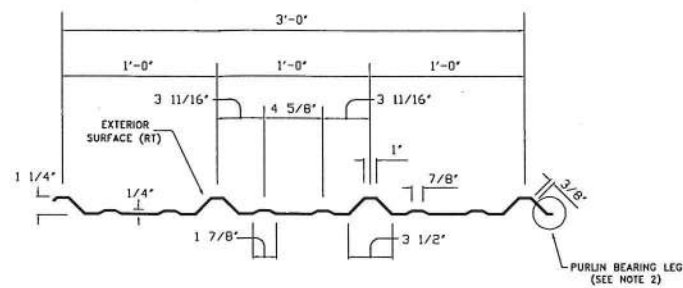
MATERIAL GAUGE	NOMINAL THICKNESS	LOAD IN		LOAD OUT	
		IX (IN. 4/FT.)	SX (IN. 3/FT.)	IX (IN. 4/FT.)	SX (IN. 3/FT.)
26 GA	0.0198	0.0182	0.0331	0.0127	0.0314
24 GA	0.0258	0.0258	0.0486	0.0179	0.0464

THE PROFILE AND MATERIAL THICKNESS OF ALL PANELS ARE DENOTED BY THE FIRST, SECOND, AND FOURTH CHARACTERS OF THE PART NUMBER.

RS A 6 - 27 07 00
 RIB-6 END CUT CODE LENGTH IN SIXTEENTHS
 A = SQUARE END B = LEFT SLOPE C = RIGHT SLOPE
 LENGTH IN FEET
 END CUT SLOPE CODE
 4 = 24GA 6 = 26GA

NOTE:
 1) SECTION PROPERTIES ARE CALCULATED IN ACCORDANCE WITH THE AISI S100-16 NORTH AMERICAN SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS STANDARD, 2016 EDITION
 2) ALL STEEL SUBSTRATES ARE MANUFACTURED IN ACCORDANCE TO ASTM A446 SPECIFICATIONS. PROTECTIVE COATINGS SHALL BE EITHER GALVANIZED, G90 1.25 OZ./PER SF. MEETING ASTM A525 OR GALVALUME™ AZ55, 0.55 OZ./PER SF. ASTM A792 SPECIFICATIONS.

RIB - 12 PANELS SHADOW PANELS (Reverse Rib)



MATERIAL GAUGE	NOMINAL THICKNESS	LOAD IN		LOAD OUT	
		IX (IN. 4/FT.)	SX (IN. 3/FT.)	IX (IN. 4/FT.)	SX (IN. 3/FT.)
26 GA	0.0198	0.0294	0.0283	0.0371	0.0500
24 GA	0.0258	0.0465	0.0481	0.0520	0.0671

MATERIAL GAUGE	NOMINAL THICKNESS	LOAD IN		LOAD OUT	
		IX (IN. 4/FT.)	SX (IN. 3/FT.)	IX (IN. 4/FT.)	SX (IN. 3/FT.)
24 GA	0.0258	0.0547	0.0589	0.0542	0.0677

THE PROFILE AND MATERIAL THICKNESS OF ALL PANELS ARE DENOTED BY THE FIRST, SECOND, AND FOURTH CHARACTERS OF THE PART NUMBER.

RT A 6 - 27 07 00
 RIB-12 END CUT CODE LENGTH IN SIXTEENTHS
 A = SQUARE END B = LEFT SLOPE C = RIGHT SLOPE
 LENGTH IN FEET
 END CUT SLOPE CODE
 4 = 24GA 6 = 26GA

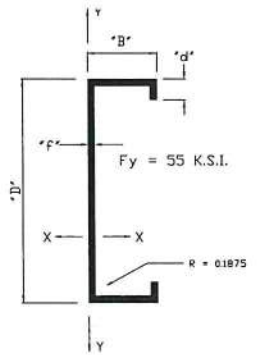
NOTE:
 1) SECTION PROPERTIES ARE CALCULATED IN ACCORDANCE WITH THE AISI S100-16 NORTH AMERICAN SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS STANDARD, 2016 EDITION
 2) PANELS WITH PURLIN BEARING LEGS ARE AVAILABLE IN RIB-12 GALVALUME ONLY
 3) ALL STEEL SUBSTRATES ARE MANUFACTURED IN ACCORDANCE TO ASTM A446 SPECIFICATIONS. PROTECTIVE COATINGS SHALL BE EITHER GALVANIZED, G90 1.25 OZ./PER SF. MEETING ASTM A525 OR GALVALUME™ AZ55, 0.55 OZ./PER SF. ASTM A792 SPECIFICATIONS.
 4) SHADOW PANEL IS INSTALLED AS A REVERSE RIB, THEREFORE THE LOAD-IN AND LOAD-OUT PROPERTIES ARE REVERSED.

D	*B*	MATERIAL	*F*	*d*	IX in. ⁴	SX in. ³
0'-8"	2 1/2"	14 GA	0.0747	3/4"	9.716	2.429
0'-8"	2 1/2"	12 GA	0.1046	3/4"	13.311	3.328
0'-10"	3 1/2"	14 GA	0.0747	3/4"	20.863	4.173
0'-10"	4"	12 GA	0.1046	3/4"	29.949	5.810

THE MATERIAL SIZE AND THICKNESS OF ALL PURLINS, AND GIRTS ARE DENOTED BY THE SECOND AND SIXTH CHARACTER OF THE PART NUMBER

NOTE:
 1) SECTION PROPERTIES ARE CALCULATED IN ACCORDANCE WITH THE AISI S100-16 NORTH AMERICAN SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS STANDARD, 2016 EDITION
 2) MINIMUM YIELD STRENGTH IS 55,000 P.S.I. (GALVANIZED MEMBERS ARE 50,000 P.S.I. MINIMUM)

CEE C H A - A 4 - 11 11 12
 E OR H = 0'-8" DEEP, T = 0'-10" DEEP
 PUNCH PATTERN
 LENGTH IN SIXTEENTHS
 LENGTH IN INCHES
 LENGTH IN FEET
 4 = 14 GA, 2 = 12 GA

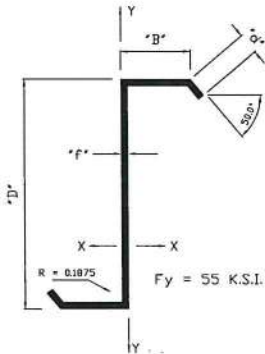


D	*B*	MATERIAL	*F*	*d*	IX in. ⁴	SX in. ³
0'-8"	2 1/2"	16 GA	0.0598	3/4"	8.120	2.030
0'-8"	2 1/2"	15 GA	0.0673	3/4"	9.099	2.275
0'-8"	2 1/2"	14 GA	0.0747	3/4"	10.058	2.514
0'-8"	2 1/2"	12 GA	0.1046	3/4"	13.847	3.462
0'-10"	3 1/2"	14 GA	0.0747	3/4"	21.960	4.390
0'-10"	3 1/2"	12 GA	0.1046	3/4"	30.520	6.100

THE MATERIAL SIZE AND THICKNESS OF ALL PURLINS, AND GIRTS ARE DENOTED BY THE SECOND AND SIXTH CHARACTER OF THE PART NUMBER

NOTE:
 1) SECTION PROPERTIES ARE CALCULATED IN ACCORDANCE WITH THE AISI S100-16 NORTH AMERICAN SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS STANDARD, 2016 EDITION
 2) MINIMUM YIELD STRENGTH IS 55,000 P.S.I. (GALVANIZED MEMBERS ARE 50,000 P.S.I. MINIMUM)

ZEE Z E A - A 4 - 27 07 00
 E = 0'-8" DEEP, T = 0'-10" DEEP
 PUNCH PATTERN
 LENGTH IN SIXTEENTHS
 LENGTH IN INCHES
 LENGTH IN FEET
 6 = 16 GA, 5 = 15 GA, 4 = 14 GA, 2 = 12 GA

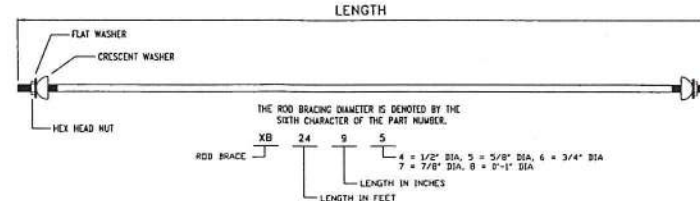
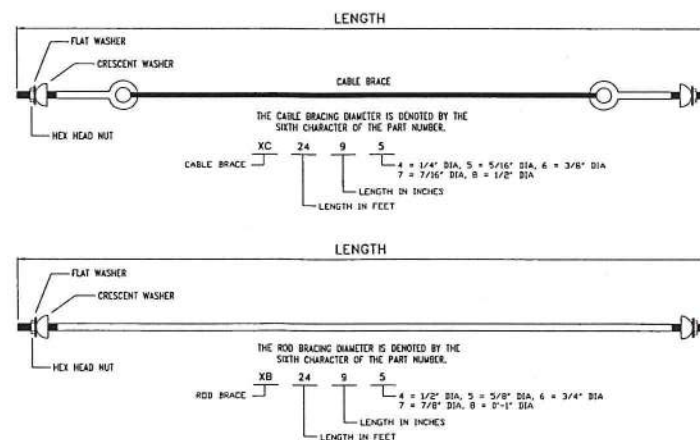
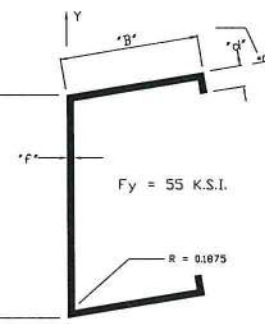


D	*B*	MATERIAL	*F*	*d*	IX in. ⁴	SX in. ³	IX in. ⁴	SX in. ³
0'-8"	0'-5"	14 GA	0.0747	3/4"	15.581	3.895	6.624	1.351
0'-8"	0'-5"	12 GA	0.1046	3/4"	21.335	4.979	9.341	1.970

THE SLOPE AND MATERIAL THICKNESS OF ALL EAVE STRUTS ARE DENOTED BY THE THIRD, FIFTH, AND SIXTH CHARACTER OF THE PART NUMBER.

NOTE:
 1) SECTION PROPERTIES ARE CALCULATED IN ACCORDANCE WITH THE AISI S100-16 NORTH AMERICAN SPECIFICATION FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS STANDARD, 2016 EDITION
 2) MINIMUM YIELD STRENGTH IS 55,000 P.S.I. (GALVANIZED MEMBERS ARE 50,000 P.S.I. MINIMUM)

EAVE STRUT E A - A 4 - 24 11 12
 EAVE STRUT
 LENGTH IN SIXTEENTHS
 LENGTH IN INCHES
 LENGTH IN FEET
 SLOPE CODE

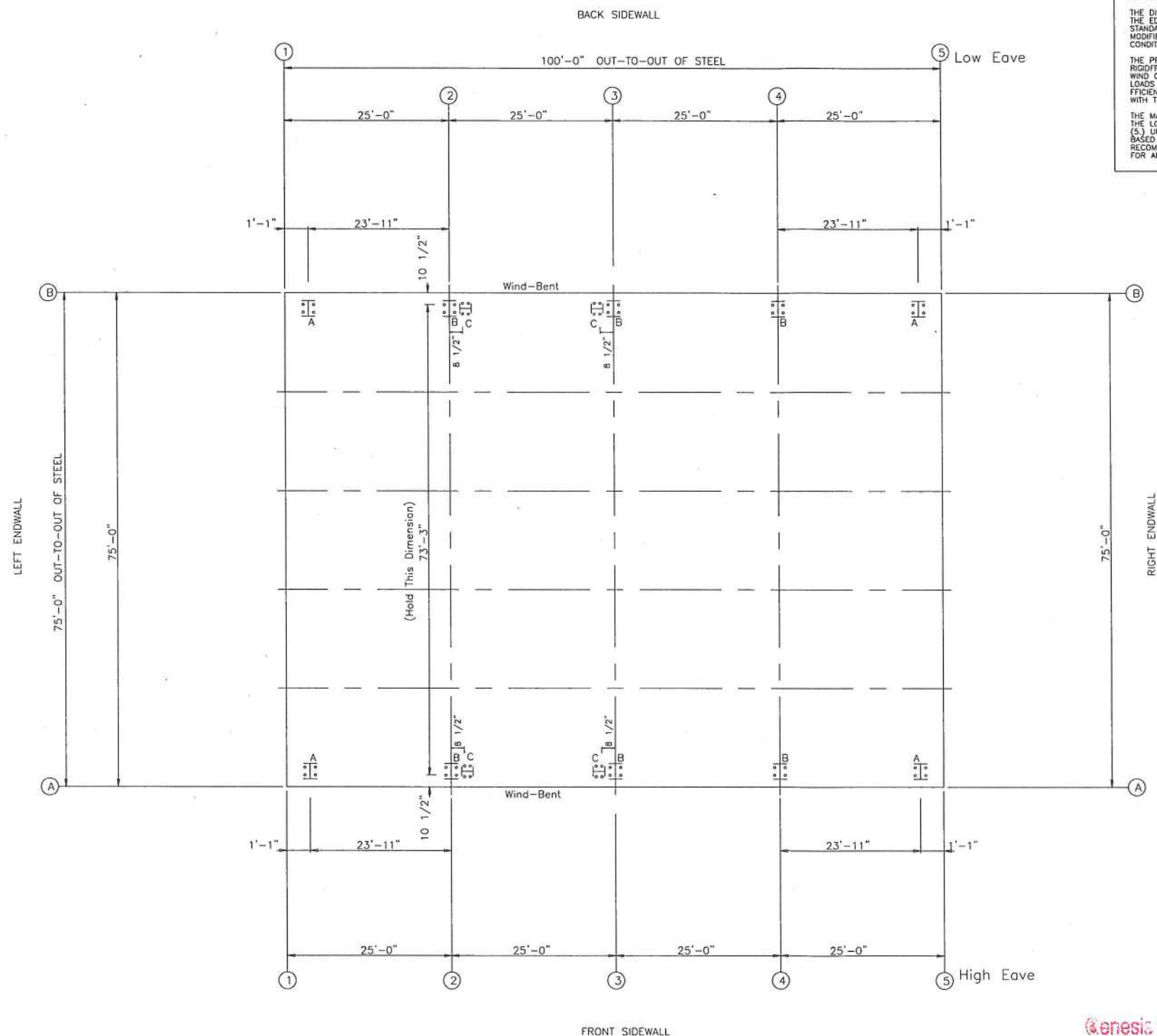
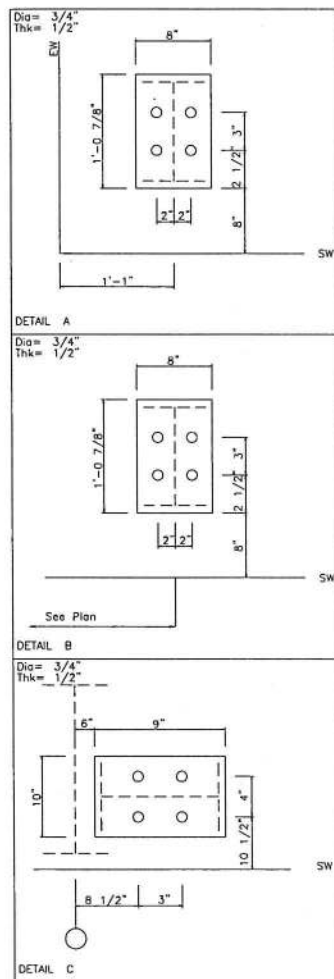


Genesis Construction Inc. reviewed the material shown and marked in this submittal for general compliance with construction drawing and specifications.
 Reviewed by:
 Date: 11/01/2022

A = 8.012	H = 3.512	O = 7.012
B = 0.512	J = 4.012	R = 7.512
C = 1.012	K = 4.512	S = 8.012
D = 1.512	L = 5.012	T = 8.512
E = 2.012	M = 5.512	U = 9.012
F = 2.512	N = 6.012	V = 9.512
G = 3.012	P = 6.512	W = 10.012

2929 INDUSTRIAL AVE. FORT MYERS, FLORIDA 33901	DRAWN YAN 10/11/22 CHECKED YAN 10/11/22 SHEET NUMBER 2 OF 7
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CUSTOMER	GENESIS CONST. & MANAGEMENT
PROJECT NAME	TOWNHOMES SHED 1
DESCRIPTION	MATERIAL PROPERTIES
ISSUE	PERMIT
REVISION	10/11/22
DATE	YAN
BY	



ANCHOR BOLT PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)

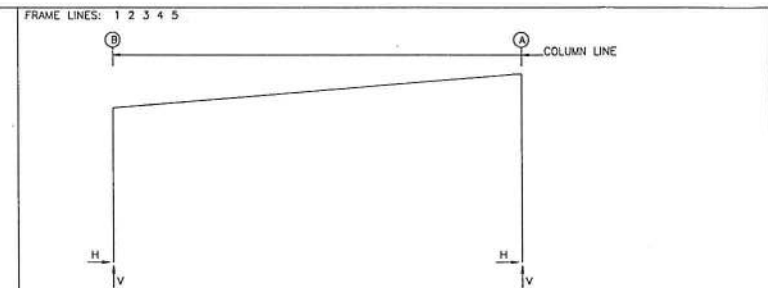
GENERAL NOTES

DEAN STEEL BUILDINGS, INC. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIALS AND CONSTRUCTION OF THE FOUNDATION OR FOUNDATION EMBEDMENTS. ADEQUATE PROVISIONS SHALL BE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING AND ANY OTHER IMPOSED LOADS, ACCOUNTING FOR THE BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE.

THE DISTANCE OF THE ANCHOR BOLTS FROM THE EDGE OF THE SLAB IS BASED ON DSB STANDARDS. THE ACTUAL SLAB SIZE MAY BE MODIFIED TO ACCOMMODATE ACTUAL SITE CONDITIONS AND FOUNDATION DESIGN.

THE PROVIDED BASIC COLUMN REACTIONS OF RIGID FRAMES, ENDWALLS, AND WIND BENTS OR WIND COLUMNS ARE UNFACTORED ELEMENTAL LOADS PRIOR TO ANY LOAD COMBINATION COEFFICIENTS OR LOAD FACTORS IN ACCORDANCE WITH THE BUILDING CODE USED.

THE MAXIMUM REACTIONS ARE CALCULATED BY THE LOAD COMBINATIONS PROVIDED IN SECTION (5) UNDER THE NOTES FOR REACTIONS AND ARE BASED ON STEEL DESIGN STANDARDS. DSB RECOMMENDS USING THE BASIC COLUMN REACTIONS FOR ADEQUATE FOUNDATION DESIGN.



RIGID FRAME: MAXIMUM REACTIONS

Frame Line	Col Line	Load Id	Hmax	Hmin	Vmax	Vmin
1*	B	1	6.0	14.7	2	-6.5 -13.1
1*	A	3	5.8	-9.0	1	-6.0 15.0
1*		1	-6.0	15.0	4	5.4 -12.8

1* Frame lines: 1 5

RIGID FRAME: MAXIMUM REACTIONS

Frame Line	Col Line	Load Id	Hmax	Hmin	Vmax	Vmin
2*	B	1	6.0	14.7	2	-6.5 -13.1
2*	A	3	5.8	-9.0	1	-6.0 15.0
2*		1	-6.0	15.0	4	5.4 -12.8

2* Frame lines: 2 3 4

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead	Live	Wind_Left1	Wind_Right1	Wind_Left2	Wind_Right2
1*	B	-1.1	3.7	-4.9	11.3	8.8	-23.7
1*	A	-1.1	3.7	4.9	11.3	-11.9	-25.3
2*	B	-1.1	3.7	-4.9	11.3	8.8	-23.7
2*	A	-1.1	3.7	4.9	11.3	-11.9	-25.3

1* Frame lines: 1 5

2* Frame lines: 2 3 4

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:
 - Width (ft) = 75.0
 - Length (ft) = 100.0
 - Eave Height (ft) = 26.0/ 34.3
 - Roof Slope (rise/run) = 1.0
 - Dead Load (psf) = 2.0
 - Collateral Load (psf) = 0.0
 - Roof Live Load (psf) = 20.0
 - Frame Live Load (psf) = 12.0
 - Wind Speed (mph) = 120.0
 - Wind Code = FBC 20 (IBC 18)
 - Exposure = C
 - Closure/Open = C
 - Importance Wind = 1.00
 - Importance Seismic = 1.00
 - Seismic Zone = B
 - Seismic Coeff (Fa/Sa) = 0.14
- Loading conditions are:
 - Dead+Collateral+Live
 - 0.6Dead+0.6Wind_Left1
 - 0.6Dead+0.6Wind_Right1
 - 0.6Dead+0.6Wind_Long1R

BUILDING BRACING REACTIONS

Loc	Line	Col Line	Reactions (k)	Panel Shear (lb/ft)	Note
L_EW	1				(h)
R_EW	5				(h)
B_SW	B	2,3			(o)

(a) Wind bent in bay

(h) Rigid frame at endwall

WIND BENT REACTIONS

		± Reactions									
		Wind(k)		Seismic(k)				Ball(in)		Base_Plate(in)	
Loc	Line	Horz	Vert	Horz	Vert	Qty	Dia	Width	Length	Thick	
F_SW A	2	6.9	18.6	0.3	0.7	4	0.750	10.000	9.000	0.500	
F_SW A	3	6.9	18.6	0.3	0.7	4	0.750	10.000	9.000	0.500	
B_SW B	3	6.4	13.8	0.3	0.6	4	0.750	10.000	9.000	0.500	
B_SW B	2	6.4	13.8	0.3	0.6	4	0.750	10.000	9.000	0.500	

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Total Len (in)	Proj (in)
40	Frame	3/4"	AB-2	15.0	2.00
16	WindCol	3/4"	AB-2	15.0	2.00

Genesis Construction has reviewed the material shown and marked in this submittal for general compliance with construction drawing and specifications.

Reviewed by:

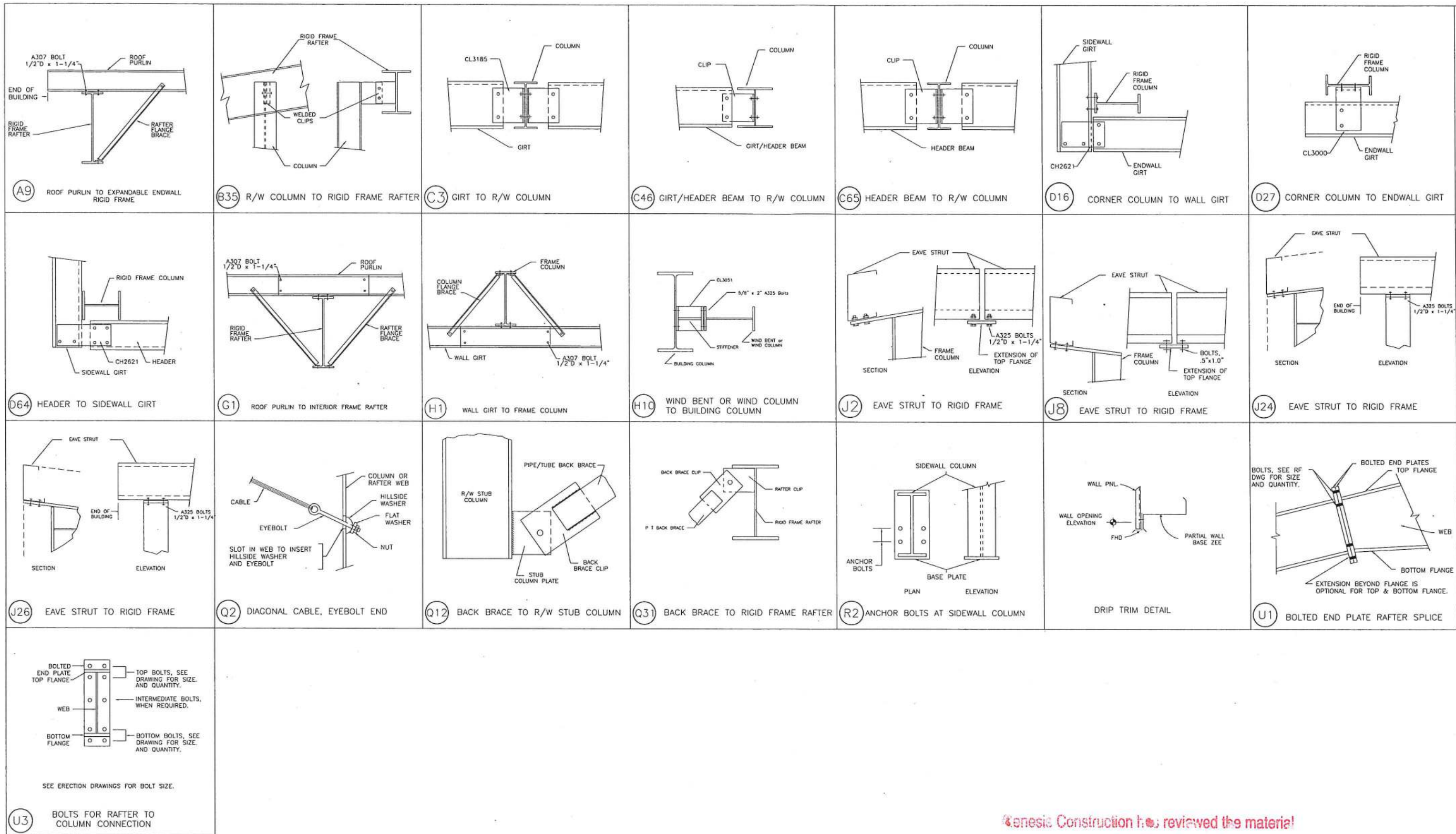
Date: 11/02/2022



JOB NUMBER	TVO1633
DRAWN	YAN
CHECKED	YAN
DATE	10/11/22
SHEET NUMBER	3 OF 7

CUSTOMER	GENESIS CONST. & MANAGEMENT
PROJECT NAME	TOWNHOMES SHED 1
DESCRIPTION	ANCHOR BOLT PLAN, BASE PLATE DETAILS

ISSUE	REVISION	DATE	BY
0		FOR CONST. 10/11/22	YAN

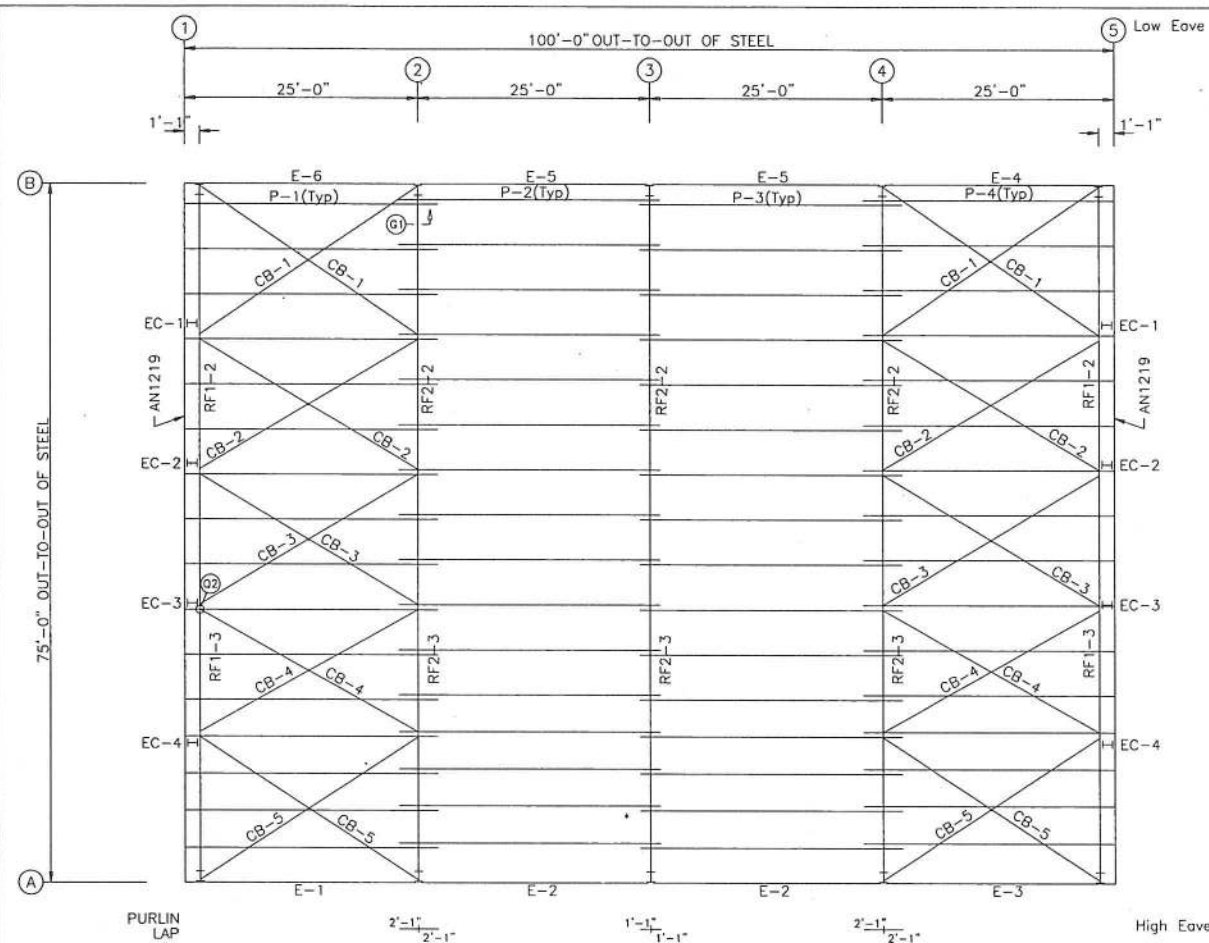


Genesis Construction has reviewed the material shown and marked in this submittal for general compliance with construction drawing and specifications.

Reviewed by: [Signature]
 Date: 11/01/2022

P	PERMIT	10/11/22	YAN
ISSUE	REVISION	DATE	BY
CUSTOMER: GENESIS CONST. & MANAGEMENT			
PROJECT NAME: TOWNHOMES SHED 1			
DESCRIPTION: DETAIL DRAWINGS			

JOB NUMBER		TVO1633
DRAWN	YAN	10/11/22
CHECKED	YAN	10/11/22
SHEET NUMBER		4 OF 7

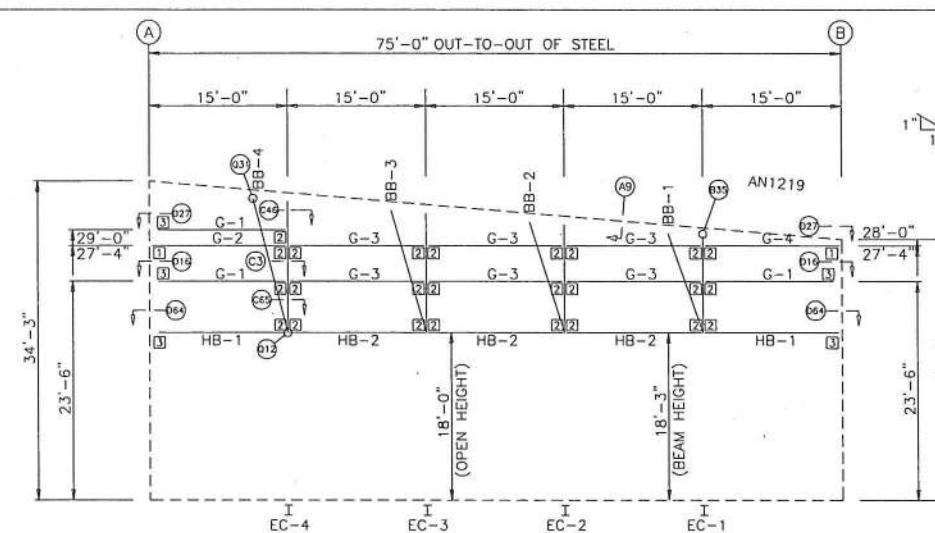


ROOF FRAMING PLAN

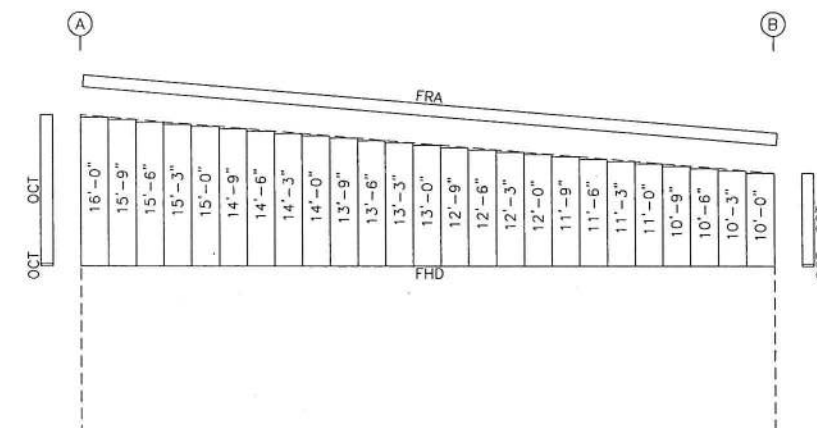
QUAN	MARK	PART	LENGTH
16	P-1	8x25x12	27.083
16	P-2	8x25x14	28.167
16	P-3	8x25x14	28.167
16	P-4	8x25x12	27.083
1	CB-1	ES0814	24.990
1	CB-2	ES0814	24.979
1	CB-3	ES0814	24.990
1	CB-4	ES0814	24.979
1	CB-5	ES0814	24.990
4	CB-1	CAB1	27.250
4	CB-2	CAB1	27.250
4	CB-3	CAB1	27.250
4	CB-4	CAB1	27.250
4	CB-5	CAB2	27.750

36'-4 11/16" (34)
39'-5 11/16" (34)

ROOF SHEETING
PANELS: 26 Ga. R12b
GALVALUME



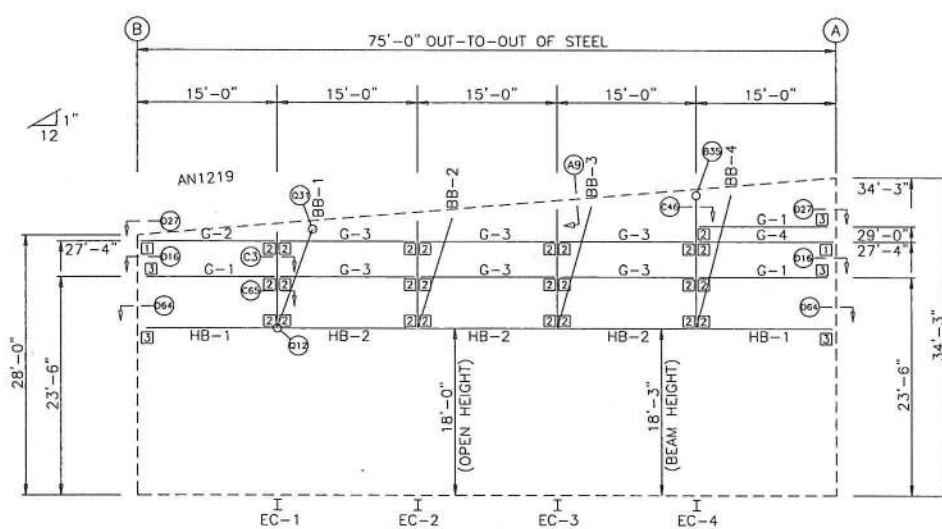
ENDWALL FRAMING: FRAME LINE 5



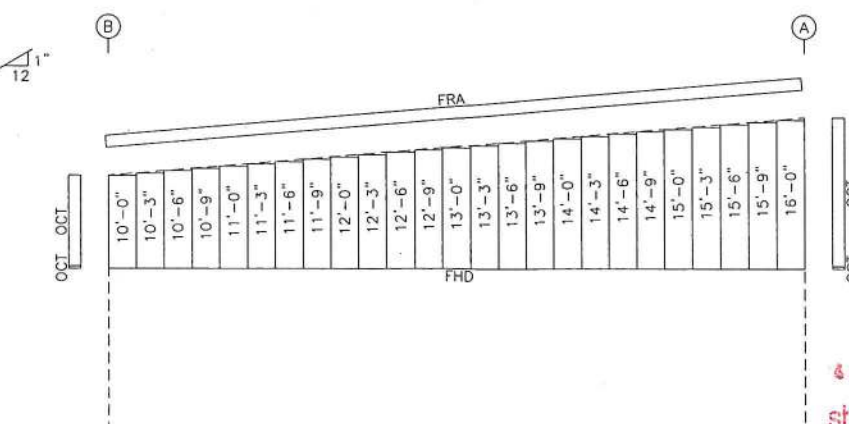
ENDWALL SHEETING & TRIM: FRAME LINE 5
PANELS: 26 Ga. R12a - NEED COLOR

QUAN	MARK	PART	LENGTH
2	HB-1	8x25x16	13.938
2	HB-2	8x25x16	14.250
1	BB-1	P6.625	25.701
1	BB-2	P6.625	26.198
1	BB-3	P6.625	26.745
1	BB-4	P6.625	27.338
1	EC-1	W8x10	10.414
1	EC-2	W8x10	11.664
1	EC-3	W8x10	12.914
1	EC-4	W8x10	14.164
3	G-1	8x25x16	13.938
1	G-2	8x25x16	14.625
1	G-3	8x25x16	14.250
1	G-4	8x25x16	14.625

QUAN	MARK	PART	LENGTH
1	CL3000		
2	CL3185		
3	CH2621		



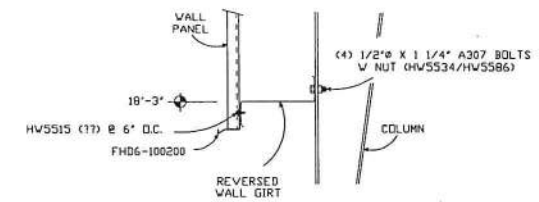
ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 Ga. R12a - NEED COLOR

QUAN	MARK	PART	LENGTH
2	HB-1	8x25x16	13.938
2	HB-2	8x25x16	14.250
1	BB-1	P6.625	25.701
1	BB-2	P6.625	26.198
1	BB-3	P6.625	26.745
1	BB-4	P6.625	27.338
1	EC-1	W8x10	10.414
1	EC-2	W8x10	11.664
1	EC-3	W8x10	12.914
1	EC-4	W8x10	14.164
3	G-1	8x25x16	13.938
1	G-2	8x25x16	14.625
1	G-3	8x25x16	14.250
1	G-4	8x25x16	14.625

QUAN	MARK	PART	LENGTH
1	CL3000		
2	CL3185		
3	CH2621		



WALL GIRTH @ AREA TO BE OPEN TO REMAIN

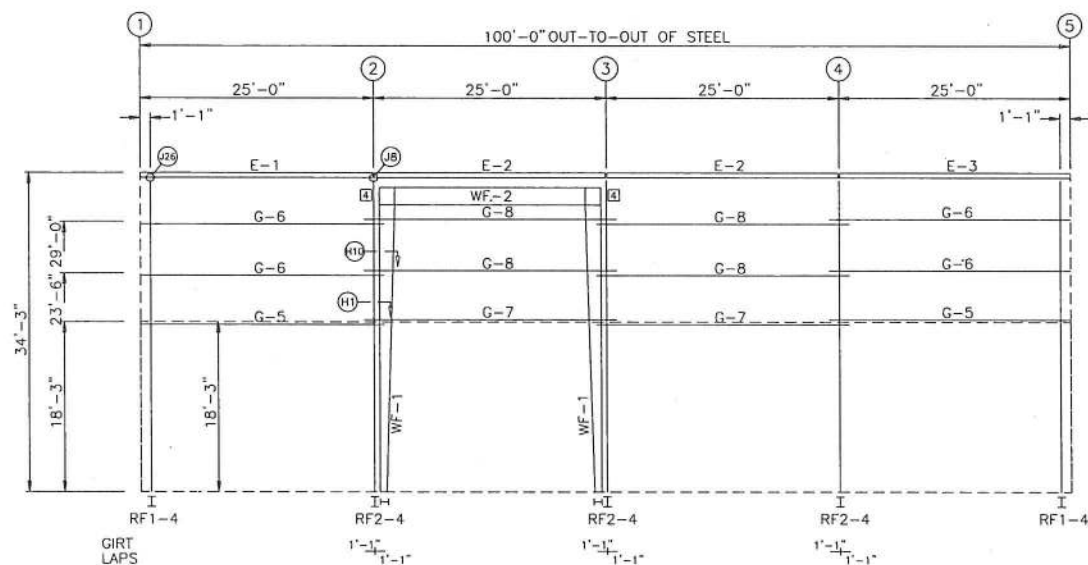
Genesis Construction Inc. is responsible for the design and construction of the materials shown and marked in this submittal. The materials are to be in compliance with construction drawing and specifications.

Reviewed by: Boa
Date: 11/01/2022

ISSUE	REVISION	DATE	BY
P	PERMIT	10/11/22	YAN

JOB NUMBER	TV01633
DRAWN	YAN 10/11/22
CHECKED	YAN 10/11/22
SHEET NUMBER	5 OF 7

CUSTOMER	GENESIS CONST. & MANAGEMENT
PROJECT NAME	TOWNHOMES SHED 1
DESCRIPTION	ROOF FRAMING PLAN ENDWALL ELEVATIONS

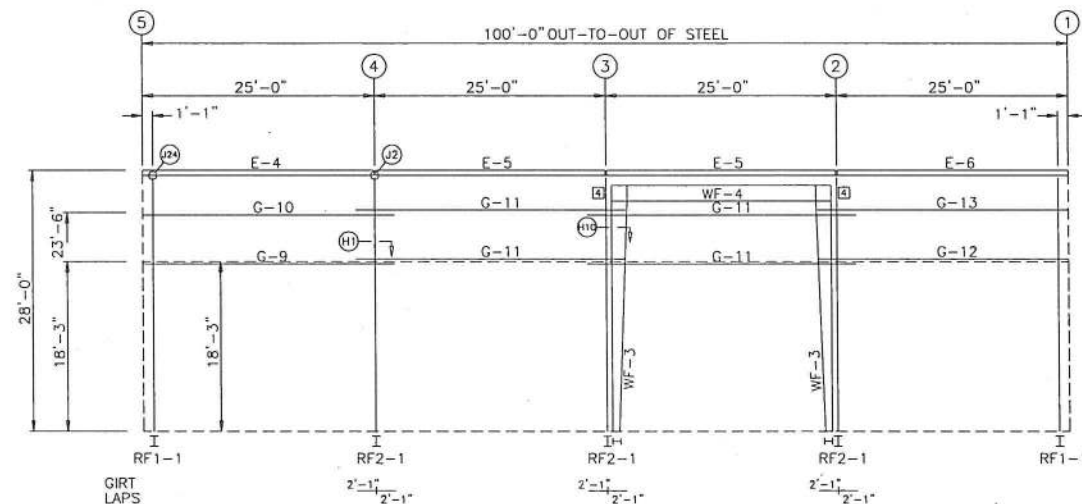


SIDEWALL FRAMING: FRAME LINE A

BOLT TABLE				
FRAME LINE A				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	3/4"	2 1/4"
WF-1 - RF2-4	4	A325	1/2"	1 1/2"

MEMBER TABLE			
FRAME LINE A			
QUAN	MARK	PART	LENGTH
2	WF-1	BEAM	33.248
1	WF-2	B20c06f	20.635
1	E-1	ES0814	24.990
2	E-2	ES0814	24.979
1	E-3	ES0814	24.990
2	G-5	8x25x16	26.083
4	G-6	8x25x14	26.083
2	G-7	8x25x16	27.167
4	G-8	8x25x14	27.167

CONNECTION PLATES			
FRAME LINE A			
ID	QUAN	MARK/PART	
4	2	CL3051	

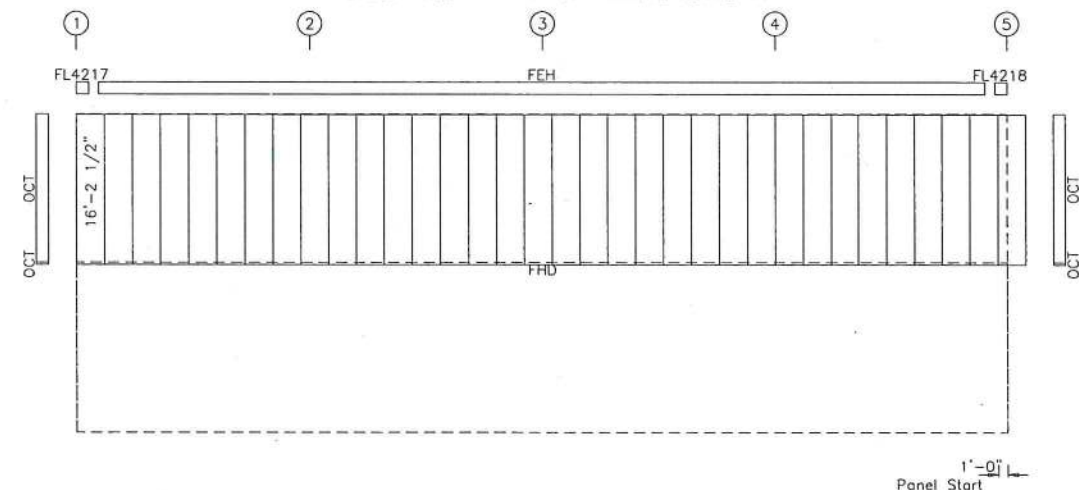


SIDEWALL FRAMING: FRAME LINE B

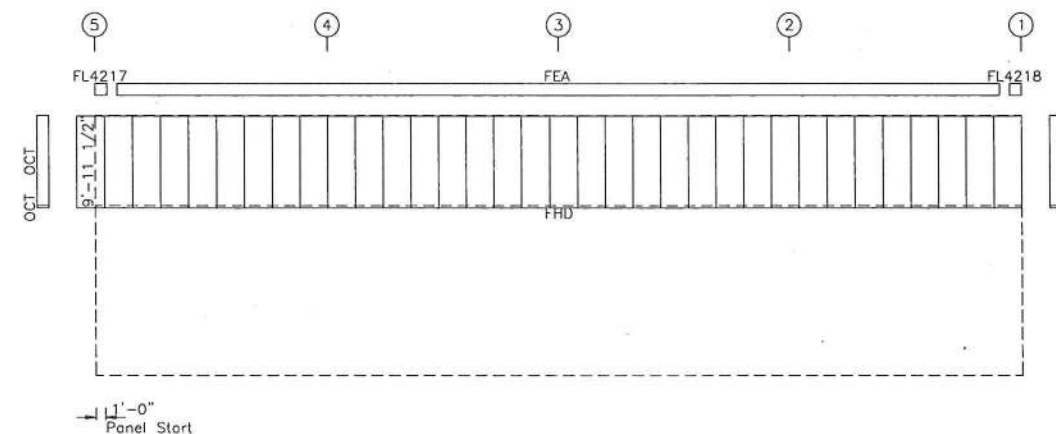
BOLT TABLE				
FRAME LINE B				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-3 - WF-4	8	A325	3/4"	2 1/4"
WF-3 - RF2-1	4	A325	1/2"	1 1/2"

MEMBER TABLE			
FRAME LINE B			
QUAN	MARK	PART	LENGTH
2	WF-3	BEAM	26.998
1	WF-4	B20c06f	20.656
1	E-4	ES0814	24.990
2	E-5	ES0814	24.979
1	E-6	ES0814	24.990
1	G-9	8x25x16	27.083
1	G-10	8x25x15	27.083
4	G-11	8x25x16	29.167
1	G-12	8x25x16	27.083
1	G-13	8x25x15	27.083

CONNECTION PLATES			
FRAME LINE B			
ID	QUAN	MARK/PART	
4	2	CL3051	



SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: 26 Ga. R12a - NEED COLOR



SIDEWALL SHEETING & TRIM: FRAME LINE B
PANELS: 26 Ga. R12a - NEED COLOR

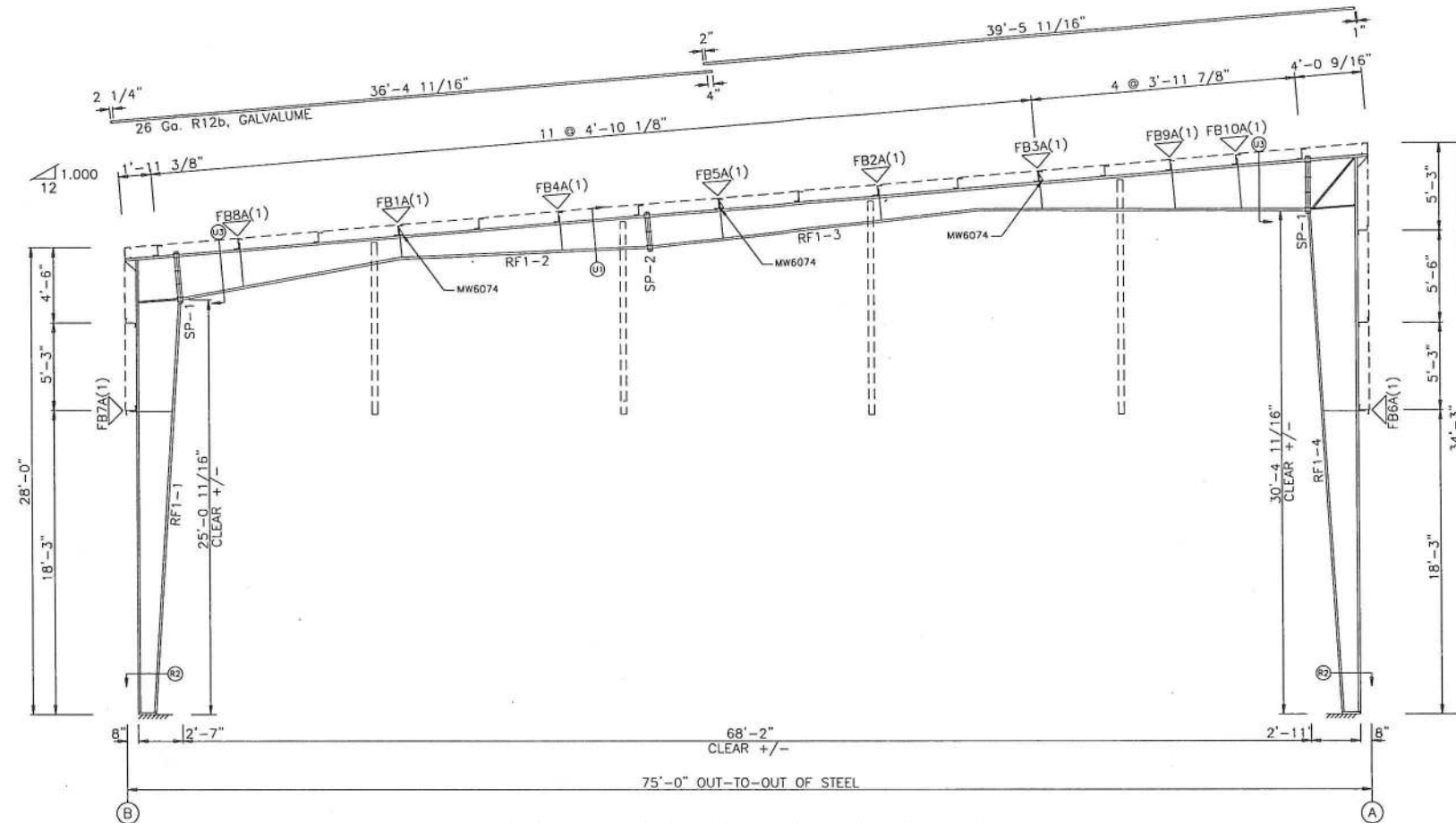
Genesis Construction has reviewed the material shown and marked in this submittal for general compliance with construction drawing and specifications.

Reviewed by: Baco
Date: 11/01/2022

ISSUE	REVISION	DATE	BY
P	PERMIT	10/11/22	YAN

JOB NUMBER	
TV01633	
DATE	
10/11/22	
CHECKED	
YAN	10/11/22
SHEET NUMBER	
6 OF 7	

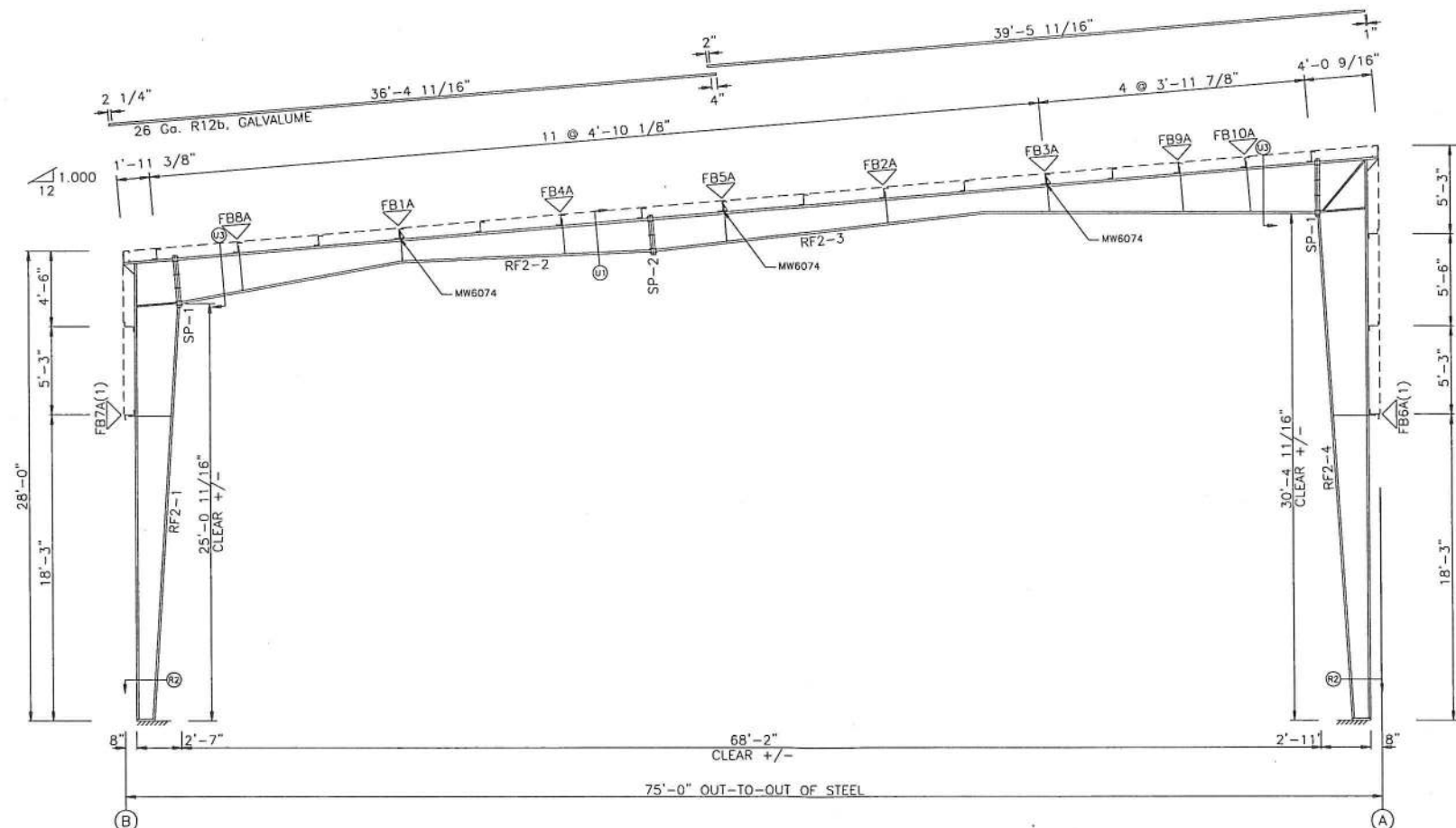
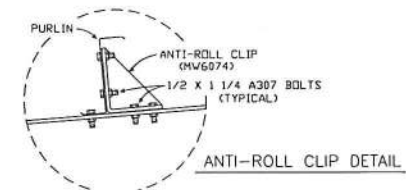
CUSTOMER: GENESIS CONST. & MANAGEMENT	
PROJECT NAME: TOWNHOMES SHED 1	
DESCRIPTION: SIDEWALL ELEVATIONS	



RIGID FRAME ELEVATION: FRAME LINE 1 5

SPLICE BOLT TABLE						
Mark	Qty	Top	Bot	Int	Type	Dia Length
SP-1	4	4	4	4	A325	0.750 2.50
SP-2	4	4	4	2	A325	0.750 2.25

FLANGE BRACES: Both Sides(U.N.)
FBxxA(1)
A - AFB4



RIGID FRAME ELEVATION: FRAME LINE 2 3 4

SPLICE BOLT TABLE						
Mark	Qty	Top	Bot	Int	Type	Dia Length
SP-1	4	4	4	4	A325	0.750 2.50
SP-2	4	4	4	2	A325	0.750 2.25

FLANGE BRACES: Both Sides(U.N.)
FBxxA(1)
A - AFB4

Genesis Construction Inc. reviewed the material shown and marked in this submittal for general compliance with construction drawing and specifications.

Reviewed by: *[Signature]*

Date: 11/01/2022

P	PERMIT	10/11/22	YAN
ISSUE	REVISION	DATE	BY

 2929 INDUSTRIAL AVE. FORT MYERS, FLORIDA 33901	JOB NUMBER	TV01633
	DRAWN	DATE
	YAN	10/11/22
	CHECKED	DATE
	YAN	10/11/22
	SHEET NUMBER	7 OF 7

CUSTOMER: GENESIS CONST. & MANAGEMENT
PROJECT NAME: TOWNHOMES SHED 1
DESCRIPTION: FRAME CROSS SECTIONS