Anchor Rod Drawings

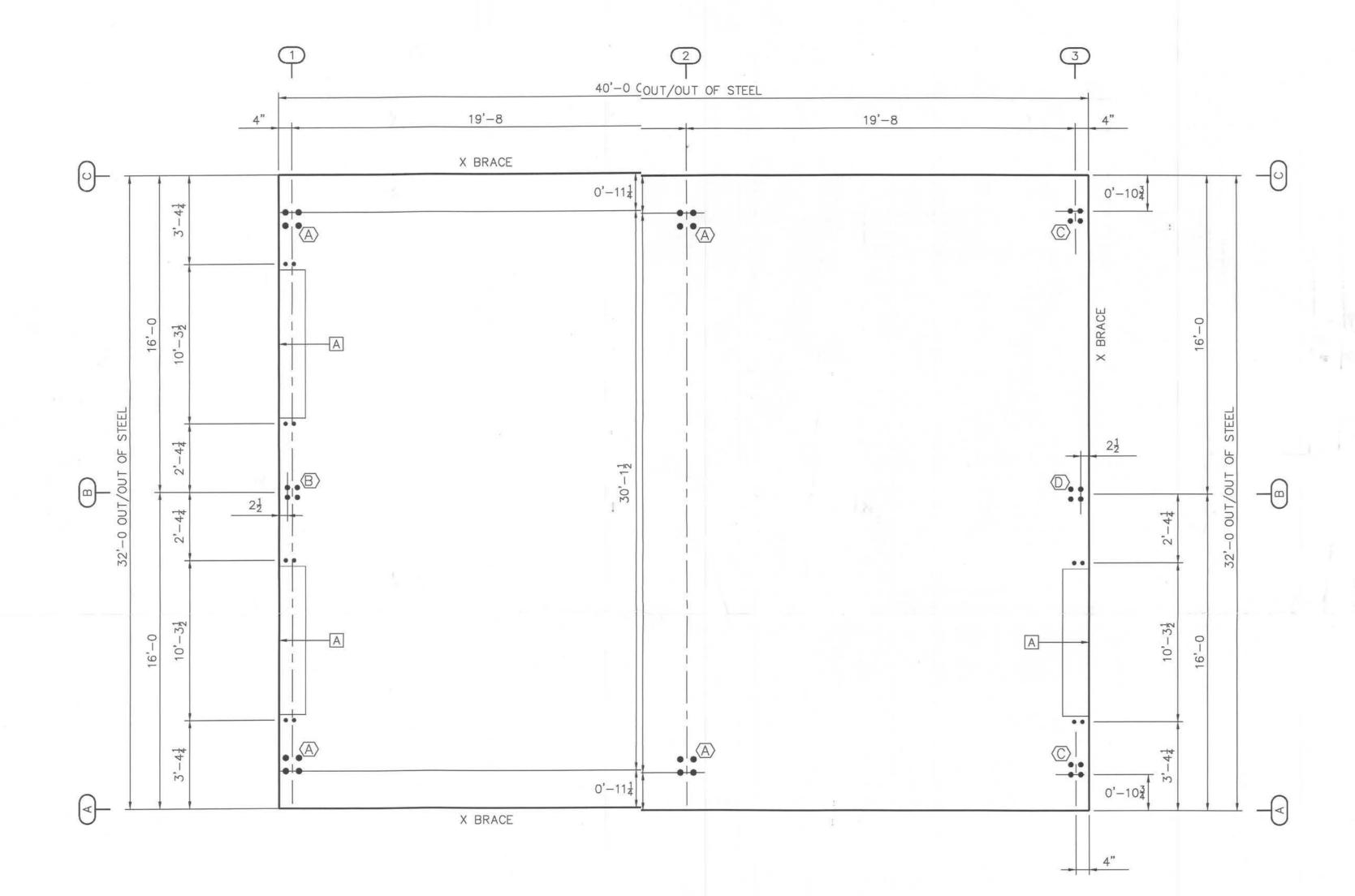
- This drawing is for anchor rod placement only and is not foundation design.
 Foundation must be square and level with all anchor rods true in size, location,
- and projection.

 3) Projection shown must be held to keep threads clear of finished concrete.

 4) This structural design data includes magnitude and location of design loads and support conditions, material properties, and type and size of major structural members necessary to show compliance with the Order Documents at the time of this issue. Any change to building loads or dimensions may change structural member sizes and locations shown. This structural design data will be supersedd and voided by any future mailing.
- 5) Anchor rod size is determined by shear and tension at the bottom of the base plate. The length of the anchor rod and method of load transfer to the foundation are to be determined by the foundation engineer, and are not provided by the
- Anchor rods are ASTM F1554 Gr. 36 material unless noted otherwise.

 3000 psi concrete compressive strength (f'c) is assumed for the purpose of column base plate design unless otherwise noted.

FINISH FLOOR AT ELEVATION 100'-0



ANCHOR FROD SETTING PLAN

	SWC	
EWB	KEY PLAN	EWD

SWA

	ACCESSORY SCHEDULE		
MARK	DESCRIPTION	DETAIL	QUAN.
A	10'-0 X 10'-0 FRAMED OPENINGS	E	3

ANCHOR BOLTS TO BE DESIGNED BY FOUNDATION ENGINEER USING DIAMETERS SHOWN IN THIS TABLE.

ANCHOR ROD DESCRIPTION	QUANTITY
5 "ø DIAMETER X	28
₹ "ø DIAMETER X	16

SYSTEMS

NOT TO SCALE

Drawn by: SJF 1/15/20 Checked by: BKR 1/15/20

Job Number: 17-B-48260

The engineer whose seal appears hereon is an employee for the manufacturer for the

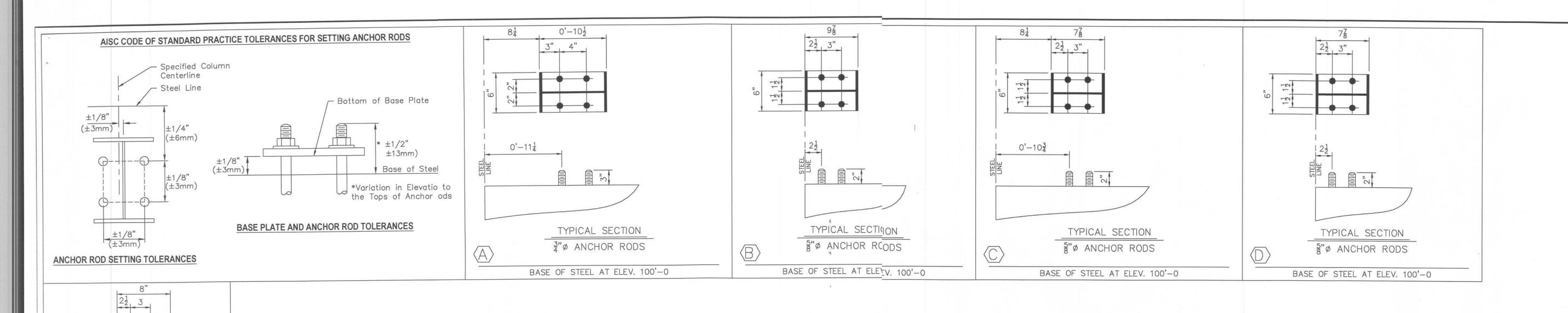
materials described herein. Said

seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this proect.

Vinay Joseph Thottınkal, P.E. Florida P.E. 77970

Sheet Number: F1of 3

Project Engineer: IRV



TYPICAL SECTION

§"ø ANCHOR RODS

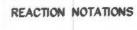
BASE OF STEEL AT ELEV. 100'-0

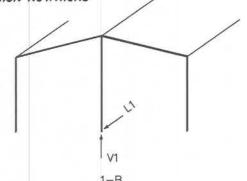
SYSTEMS_® Scale: NOT TO SCALE Drawn by: SJF 1/15/20 Checked by: BKR 1/15,20 Project Engineer: LRV Job Number: 17-B-48260 Sheet Number: F2 of 3 The engineer whose seal appears hereon is an empbyee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

Vinay Joseph Thottunkal, P.E. Florida P.E. 77970



FRAME DES Endwall EW PATH: R:\jobs\Active\Eng\17-B-48260\ver01-lxvega\BLDG-A\v	JOB	NAME: LXVega NAME: 48260A	DATE: 1/15/) FILE: REW3BL[1	PAGE: EW-1
SUPPORT REACTIONS FOR EACH LOAD GROUP NOTE: All reactions are in kips and kip—ft.			1	Π ME: 08: 31: 27





LOAD GROUP REACTION TABLE

COLUMN		1-B	
LOAD GROUP	H1	V1	L1
D	0.	0.2	0.
W+	0.	0.	3.3
W-	0.	0.	-3.6

LOAD GROUP DESCRIPTION

: DEAD LOAD : WIND LOAD AS AN INWARD ACTING PRESSURE W- : WIND LOAD AS AN OUTWARD ACTING SUCTION

NOTES

1) THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.

THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUT (UNLESS NOTED OTHERWISE).

a) A REACTION TABLE IS PROVIDED WITH THE REACTIONS FOR EACH

LOAD GROUP. b) RIGID FRAMES

(1) GABLED BUILDINGS

(a) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE LEFT SIDE OF THE BUILDING, AS SHOWN ON THE ANCHOR ROD DRAWING, FROM THE OUTSIDE OF THE BUILDING. (b) INTERIOR COLUMNS ARE SPACED FROM LEFT SIDE TO RIGHT SIDE. (2) SINGLE SLOPE BUILDINGS

(a) LEFT COLUMN IS THE LOW SIDE COLUMN.

(b) RIGHT COLUMN IS THE HIGH SIDE COLUMN. (c) INTERIOR COLUMNS ARE SPACED FROM LOW SIDE TO HIGH SIDE. c) ENDWALLS

(1) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE WALL FROM THE OUTSIDE. (2) INTERIOR COLUMNS ARE SPACED FROM LEFT TO RIGHT.

d) ANCHOR ROD SIZE IS DETERMINED BY SHEAR AND TENSION AT THE BOTTOM OF THE BASE PLATE. THE LENGTH OF THE ANCHOR ROD AND METHOD OF LOAD TRANSFER TO THE FOUNDATION ARE TO BE DETERMINED BY THE FOUNDATION ENGINEER. e) ANCHOR RODS ARE ASTM F1554 Gr. 36 MATERIAL UNLESS NOTED

OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING. f) X-BRACING (1) ROD BRACING REACTIONS HAVE BEEN INCLUDED IN VALUES SHOWN

IN THE REACTION TABLES. (2) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC

LOADS (RBUPEQ AND RBDWEQ) DO NOT INCLUDE THE AMPLIFICATION FACTOR, Ω_0 . (3) FOR CANADA BUILDING CODE (NBC), WHEN X-BRACING IS PRESENT IN THE SIDEWALL OR ENDWALL, INDIVIDUAL LONGITUDINAL SEISMIC

LOADS (RBUPEQ & RBDWEQ) ARE MULTIPLIED BY FORCE REDUCTION FACTOR, Rd, WHEN SPECIFIED SHORT-PERIOD SPECTRAL ACCELERATION RATIO IEFOSO(0.2) IS GREATER THAN 0.45. 3) REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP

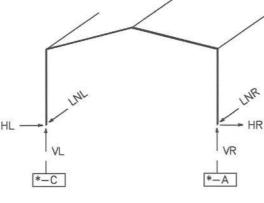
APPLIED TO THE COLUMN. THE FOUNDATION ENGINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE BEARING PRESSURES AND CONCRETE DESIGN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.

a) FOR PROJECTS USING ULTIMATE DESIGN WIND SPEEDS SUCH AS 2012 IBC, 2015 IBC, OR FLORIDA BUILDING CODE, THE WIND LOAD REACTIONS ARE AT A STRENGTH VALUE WITH A LOAD FACTOR OF 1.0. b) FOR IBC CODES, THE SEISMIC REACTIONS PROVIDED ARE AT A STRENGTH LEVEL AND DO NOT CONTAIN THE RHO FACTOR.

c) FOR NBCC CODES, THE SEISMIC REACTIONS PROVIDED DO NOT CONTAIN THE Ra*Ro FACTOR. THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN.

	FRAME ID #1 cs 32./14./24.583	20./110./	USER NAME: LXVega JOB NAME: 48260A	DATE: 1/1/15/20 FILE: frame _{1es_1_2} .fra	PAGE: 1-
*LOCATION: Gridlines: 1 2 NOTES: (1) All reactions are in kips and kip—ft. (2) Primary wind load cases are not concurrer (3) X—bracing reactions (RBPULW and RBUPEQ	t.) are combined withLWI	_ and LEQ groups	s only.	ТІМІ	E: 08: 38: 26

REACTION NOTATIONS



LOAD GROUP REACTION TABLE GRIDLINES * = 1 2

COLUMN		*-C			*-A	
LOAD GROUP	HL	VL	LNL	HR	VR	LNR
DL	0.3	1.2	0.0	-0.3	1.2	0.0
LL	1.2	4.7	0.0	-1.2	4.7	0.0
COLL	0.1	0.2	0.0	-0.1	0.2	0.0
WL1	-2.9	-9.6	0.0	-2.0	-6.6	0.0
WL2	-4.4	-1.6	0.0	-0.5	1.4	0.0
WL3	2.0	-6.6	0.0	2.9	-9.6	0.0
WL4	0.5	1.4	0.0	4.4	-1.6	0.0
LWL1	1.5	-8.2	0.0	-1.0	-7.5	0.0
RBUPLW	0.0	-1.2	-1.7	-0.0	-1.2	-1.7
LWL2	1.0	-7.5	0.0	-1.5	-8.2	0.0
LWL3	0.0	-0.2	0.0	0.6	0.5	0.0
LWL4	-0.6	0.5	0.0	-0.0	-0.2	0.0
RBDWLW	-0.0	1.2	0.0	0.0	1.2	0.0

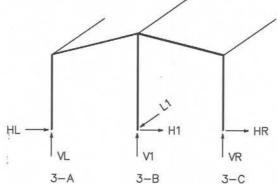
LOAD GROUP DESCRIPTION

: Roof Dead Load : Roof Live Load : Roof Collateral Load : Wind from Left to Right with +GCpi WL1 : Wind from Left to Right with -GCpi WL2 Wind from Right to Left with +GCpi WL3 WL4 Wind from Right to Left with -GCpi LWL1 Windward Corner Left with +GCpi RBUPLW Upward Acting Rod Brace Load from Long. Wind

LWL2 Windward Corner Right with +GCpi Windward Corner Left with -GCpi LWL4 Windward Corner Right with -GCpi RBDWLW : Downward Acting Rod Brace Load from Long. Wind

FRAME DESCRIPTION: USER NAME: LXVega DATE: 1/15/20 PAGE: EW-2 JOB NAME: 48260A FILE: REW4BLDG1 PATH: R:\jobs\Active\Eng\17-B-48260\ver01-lxvega\BLDG-A\run01\ SUPPORT REACTIONS FOR EACH LOAD GROUP NOTE: All reactions are in kips and kip-ft. TIME: 08: 31: 27

REACTION NOTATIONS



LOAD GROUP REACTION TABLE

COLUMN		3-A			3-B			3-C	
LOAD GROUP	HL	VL	LL	H1	V1	L1	HR	VR	LR
D	0.0	0.4	0.	0.	0.7	0.	0.0	0.4	0.
С	0.0	0.0	0.	0.	0.1	0.	0.0	0.0	0.
L	0.0	1.7	0.	0.	3.0	0.0	0.0	1.7	0.
W+	-0.1	-2.9	0.	0.	-4.9	3.3	0.1	-2.9	0.
W—	-0.1	-2.9	0.	0.	-4.9	-3.6	0.1	-2.9	0.
WR	-0.1	-2.9	0.	0.	-3.6	0.0	1.3	-4.2	0.
WL	-0.1	-2.9	0.	-1.2	-6.0	0.0	0.1	-1.8	0.

LOAD GROUP DESCRIPTION

: DEAD LOAD : COLLATERAL LOAD

: LIVE LOAD : WIND LOAD AS AN INWARD ACTING PRESSURE

WIND LOAD AS AN OUTWARD ACTING SUCTION

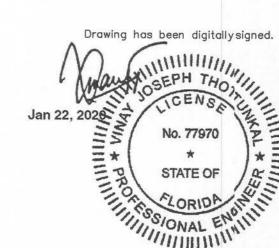
WIND FORCE FROM THE RIGHT WIND FORCE FROM THE LEFT

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er: ETAL BUILDING S K, FL 32064–2 Status:	Scales: APEX METAL BUILDING SYSTEMS LIVE OAK, FL 32064-2 TOTUWING STUTIONS: APEX METAL BUILDING SYSTEMS LIVE OAK, FL 32064-2	Scale: NOT TO SCALE Drawn by: SJE 1/15/20 Checked by: BKK 1/15/20	8600 SOUTH I-35 SERVICE RD	SYSTEMS® (405) 626 2010	(403) 020-2010	Project Name & Location: TRAVIS TUTEN	011 N3005 17 VII 774	LANE CIT, PL 32004 CS		
	Scale: NOT TO SCALE	Scale: NOT TO SCALE Drawn by: SJF 1/15/20 Checked by: BKR 1/15/20			Jan. B. I. Land some a land and the state of	<i>mer:</i> METAL BUILDING			Stutus:	For Approval

Sheet Number: F3 of 3

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is linited to the products designec and manufactured by manufacturer only. The undersigned engneer is not the overall engineer of record for this project.

Vinay Joseph Thottunkal, P.E. lorida P.E. 77970



Builder/Contractor Responsibilities

Drawing Validity — These drawings, supporting structural calculations and design certificatin are based on the order documents as of the date of these drawings. These documents descibe the material supplied by the manufacturer as of the date of these drawings. Any changes to the order documents after the date on these drawings may void these drawings, supporting structual calculations and design certification. The Builder/Contractor is responsible for notifying the building authority of all changes to the order documents which result in changes to the drawings, supporting structural calculations and design certification.

Builder Acceptance of Drawings — Approval of the manufacturer's drawings and design da affirms that the manufacturer has correctly interpreted and applied the requirements of the orde documents and constitutes Builder/Contractor acceptance of the manufacturer's interprettions of the order documents and standard product specifications, including its design, fabricationand quality criteria standards and tolerances. (April 2010 Section 4.4.1)

Code Official Approval — It is the responsibility of the Builder/Contractor to ensure that II project plans and specifications comply with the applicable requirements of any governing buildingauthority. The Builder/Contractor is responsible for securing all required approvals and permits from the appropriate agency as required.

Building Erection — The Builder/Contractor is responsible for all erection of the steel andassociated work in compliance with the Metal Building Manufacturers drawings. Temporary supports, such as temporary guys, braces, false work or other elements required for erection will be determed, furnished and installed by the erector (April 2010 Section 7.10.3) (CSA/S16-09 Section 2).

Discrepancies — Where discrepancies exist between the Metal Building plans and plans forother trades, the Metal Building plans will govern. (April 2010 Section 3.3)

Materials by Others - All interface and compatibility of any materials not furnished by the manufacturer are the responsibility of and to be coordinated by the Builder/Contractor or A/E firm. Unless specific design criteria concerning any interface between materials if furnished as part of the order documents, the manufacturers assumptions will govern.

Modification of the Metal Building from Plans — The Metal Building supplied by the manufcturer has been designed according to the Building Code and specifications and the loads shown on this drawing. Modification of the building configuration, such as removing wall panels or bracs, from that shown on these plans could affect the structural integrity of the building. The Meta Building Manufacturer or a Licensed Structural Engineer should be consulted prior to making any nanges to the building configuration shown on these drawings. The Metal Building Manufacturer will assume no responsibility for any loads applied to the building not indicated on these drawings.

The Metal Building Manufacturer is not responsible for the design, materials and workmanhip of the foundation. Anchor rod plans prepared by the manufacturer are intended to show only lcation, diameter and projection of the anchor rods required to attach the Metal Building Systemto the foundation. It is the responsibility of the end customer to ensure that adequate provisics are made for specifying rod embedment, bearing values, tie rods and or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and othr conditions of the building site. (MBMA 06 Sections 3.2.2 and A3)



For questions regardiling the interpretation of the drawings, materials provided, or assembly of the parts:

• Call 1-800-879-7827 and ask for the "Field Service" department.

• Before or after nnormal hours, you may send an email to <u>OKCSField.Service@StarBuildings.net</u>. Please include the order no., brief description of the question, & contact name and phone number.

ENGINEERING DESIGN CRITERIA

Building Code	FLORIDA BUILDING CODE, 6TH EDITION (2017) Agricultural (Category I)
Roof Dead Load Superimposed	2.12 psf 0.50 psf
(0.50 psf Other) Roof Live Load	20.00 psf reduction allowed
Other Areas	rovided by building manufacturer provided by building manufacturer 21.76 psf pressure -29.14 Fpsf suction 21.76 psf pressure -23.61 Fpsf suction lues required based on a 10 sq ft area.

DEFLECTION CRITERIA

The material supplied by the manufacturer has been designed with the following minimum deflection criteria. The actual deflection may be less depending on actual load and actual member length.

BUILDING DEFLECTION LIMITS..... BLDG-A

Roof Limits		Rafters	Purlins	Panels
Live: Serviceability Wind: Total Gravity: Total Uplift:	L/	180 180 120 N/A	150 180 120 N/A	60 60 60
Frame Limits		Sidesway		
Live: Serviceability Wind: Total Gravity:	H/	60 60 60		
Wall Limits		Limit		
Total Wind Panels: Total Wind Girts: Total Wind EW Columns:		60 90 120		

PROJECT NOTES

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, ASTM A1011 SS, or ASTM A1011 HSLAS with a minimum yield point of 50 ksi. Material properties of hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with a minimum specified yield point of 50 ksi. Hot rolled angles, other than flange braces, conform to ASTM 36 minimum. Hollow structural shapes conform to ASTM A500 grade B, minimum yield point is 42 ksi for round HSS and 46 ksi for rectangular HSS. Material properties of cold-formed light gage steel members conform to the requirements of ASTM A1011 SS Grade 55, ASTM A1011 HSLAS Grade 55 Class 1, ASTM A653 SS Grade 55, or ASTM A653 HSLAS Grade 55 Class 1 with a minimum yield point of 55 ksi. For Canada, material properties conform to CAN/CSA G40, 20/G40, 21 or equivalent.

All bolted joints with A325 Type 1 bolts are specified as snug-tightened joints in accordance with the Specification for Structural Joints Using ASTM A325 or A490 Bolts, December 31, 2009. Pre-tensioning methods, including turn-of-nut, calibrated wrench, twist-off-type tension-control bolts or direct-tension-indicator are NOT required. Installation inspection requirements for Snug Tight Bolts (Specification for Structural Joints Section 9. 1) is suggested.

Design criteria as noted is as given within order documents and is applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the metal building manufacturer nor the certifying engineer declares or attests that the loads as designated are proper for local provisions that may apply or for site specific parameters. The design criteria is supplied by the builder, project owner, or an Architect and/or Engineer of Record for the overall construction project.

This project is designed using manufacturer's standard serviceability criteria. Generally this means that all deflections are within typical performance limits for normal occupancy and standard metal building

The use of the structure is limited to Occupancy Category I for structures representing a low hazard to humans; including agricultural facilities, temporary facilities and/or minor storage facilities. The resulting reduction in applied loads would explicitly exclude most industrial or commercial applications, high human occupancy or post disaster uses. Future use for any category other than Occupancy Category I will require investigation of the structure by a qualified design professional in order to determine any reinforcement that may be required.

This metal building system is designed as Enclosed Building. All exterior components (i.e. doors, windows, vents, etc.) must be designed to withstand the specified wind loading for the design of components and cladding in accordance with the specified building code.

Framed openings, walk doors, and open areas shall be located in the bay and elevation as shown in the erection drawings. The cutting or removal of girts shown on the erection drawings due to the addition of framed openings, walk doors, or open areas not shown may void the design certifications supplied by the metal building manufacturer.

Roof and wall panels have been designed in accordance with section 2222. 4 of the Florida Building Code, 6th Florida (2017) Product approval numbers for the State of Florida, Department of Community Affairs per Product Rule 9B-72:

Panel Walls FL11917 PBR 26 gauge walls 2. Roofing Products FL11868 PBR 26 gauge roofs

Using $7'' \times 7''$ eave gutter with 4×5 downspouts, the roof drainage system has been designed using the method outlined in the MBMA Metal Building Systems Manual. Downspout locations have not been located on these drawings. The downspouts are to be placed on the building sidewalls at a spacing not to exceed 40 feet with the first downspout from both ends of the gutter run within 20 feet of the end. Downspout spacing that does not exceed the maximum spacing will be in compliance with the building code. The gutter and downspout system as provided by the manufacturer is designed to accommodate 10 in/hr rainfall intensity.

The rigid frame at building A Frame Line 1 is designed as a non-expandable rigid frame. Corresponding frame reactions are calculated based upon actual tributary area.

CK,q	Drawing Index	
0	Description	Page
By		
	Cover Sheet	E1
1	Roof Framing BLDGA	E2
1	Roof Sheeting	E3
1	Sidewall BLDGA WALLSWA/WALLSWC	E4
1	Endwall BLDGA WALLEWB/WALLEWD	E5
1	Main Frame Cross Section	E6
Description	Erection Guides	R1-R3
scri	Construction Drawings	R4-R12
De	Trim Profiles	R13

SYSTEMS® (405) 636–2010 Project Name & Location: TRAVIS TUTEN LAKE CITY, FL 32064 US Location For Construction Permit	OK 73149	BUILDING SYSTEMS® (405) 636–2010 Project Name & Location: TRAVIS TUTEN LAKE CITY, FL 32064 US Not For Construction) B600 SOUTH 1–35 SERVICE RD. OKLAHOMA CITY, OK 73149 OKLAHOMA CITY, OK 73149 APPROVED TO THE CITY, FL 32064 US FOR CONSTRUCTION Permit	0		
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appears hereon is an enployee for the manufacturer for the materials described heren. Said seal or certification is linited to the products designed and manufactured by manufacturer only. The undersigned encineer is not the overall engineer of record for this project.

Sheet Number: E1 of 6

The engineer whose seal

Vinay Joseph Thottunkal, P.E. lorida P.E. 77970



Download panel installation manuals from:

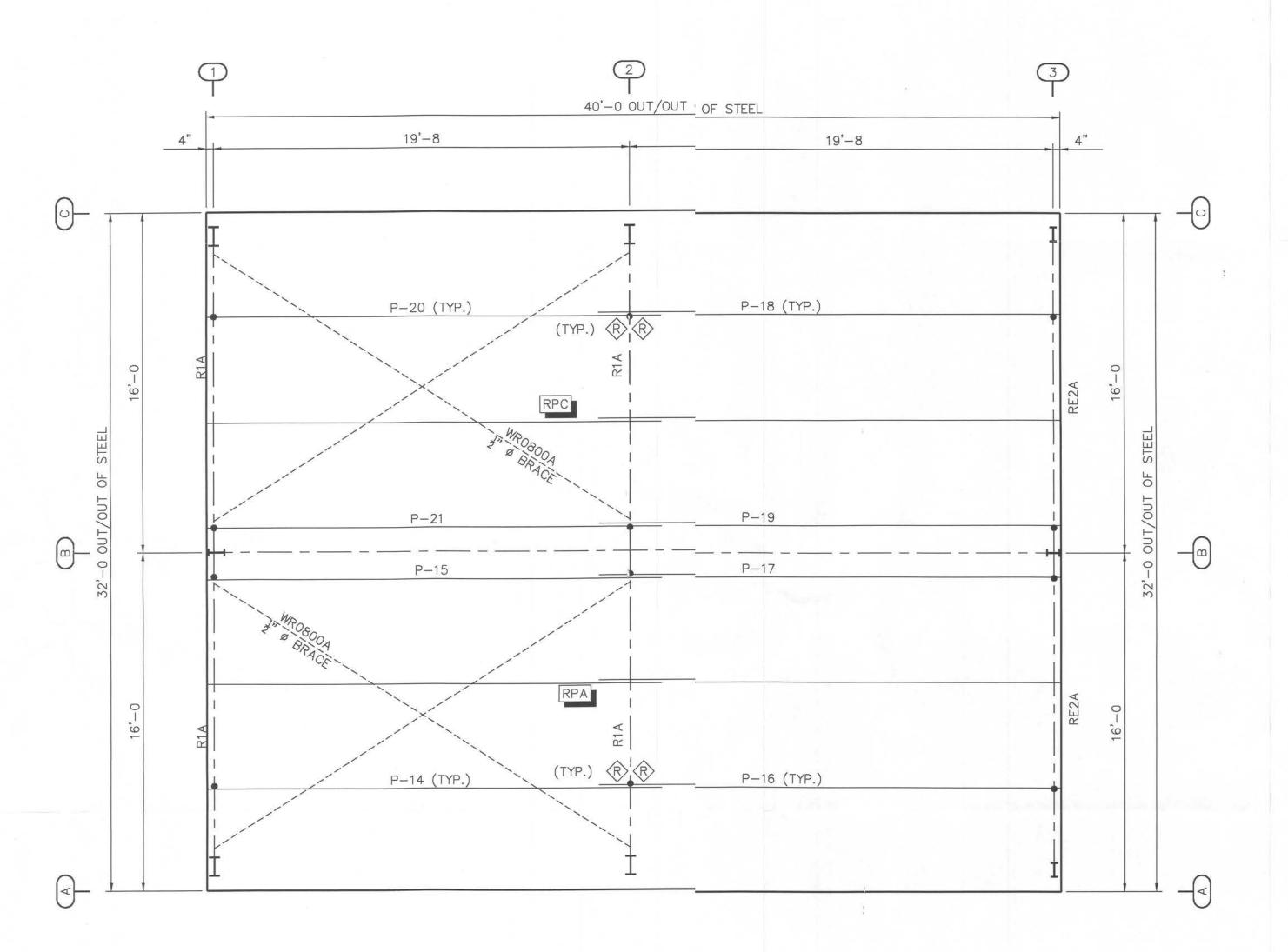
www.ncimanuals.com

Descargue los manuales de instalación del panel desde:

BUILDING DESCRIPTIONS Building ID | Width | Length | Height | Slope Building A 32'-0 40'-0 14'-0 3:12

	1"ø A3	25 BOLT GRIP TABLE	
GRIP	LENGTH	BOLT LENGTH	NOTE: FULL THRED
0 TO 9/16"	1 1/4" F.T.		ENGAGEMENT IS DEMED TO HAVE BEEN MET HEN THE
Over 9/16" TO 1 1/16"	1 3/4" F.T.		END OF THE BOLTIS FLUSH
Over 1 1/16" TO 1 5/16"	2"		WITH THE FACE O THE NUT.
Over 1 5/16" TO 1 9/16"	2 1/4"		
Over 1 9/16" TO 1 13/16"	2 1/2"		REQUIRED ONLY WHE SPECIFIED.
Over 1 13/16" TO 2 1/16"	2 3/4"	I (-RIP I	MAY BE LOCATED UNER HEAD , UNDER NUT, OR ATBOTH AT
LOCATIONS OF BOLTS LONGENOTED ON ERECTION DRAWIN		ADD 5/3	NS NOTED ON ERECTIN DRAWINGS. 32" FOR EACH WASHE TO
F.T. DENOTES FULLY THREAD	ED	MATERIA	L THICKNESS TO DETRMINE GRIP.

DENOTES: CLIP LOCATION
 SC90 AT 8" PURLINS
 SC92 AT 10" PURLINS
 SC94 AT 12" PURLINS



ROOF FRAMING PLAN

EWB' KEY PLAN

SWC

SWA

ZEE SECTION LAP TABLE

SYMBOL LAP LENGTH SYMBOL LAP LENGTH \bigcirc $-0'-0\frac{1}{4}$ \bigcirc $2'-5\frac{3}{4}$ \bigcirc $0'-3\frac{3}{4}$ \bigcirc $3'-1\frac{3}{4}$ \bigcirc $1'-5\frac{3}{4}$ REFER TO CF01122

Jan 22, 2026

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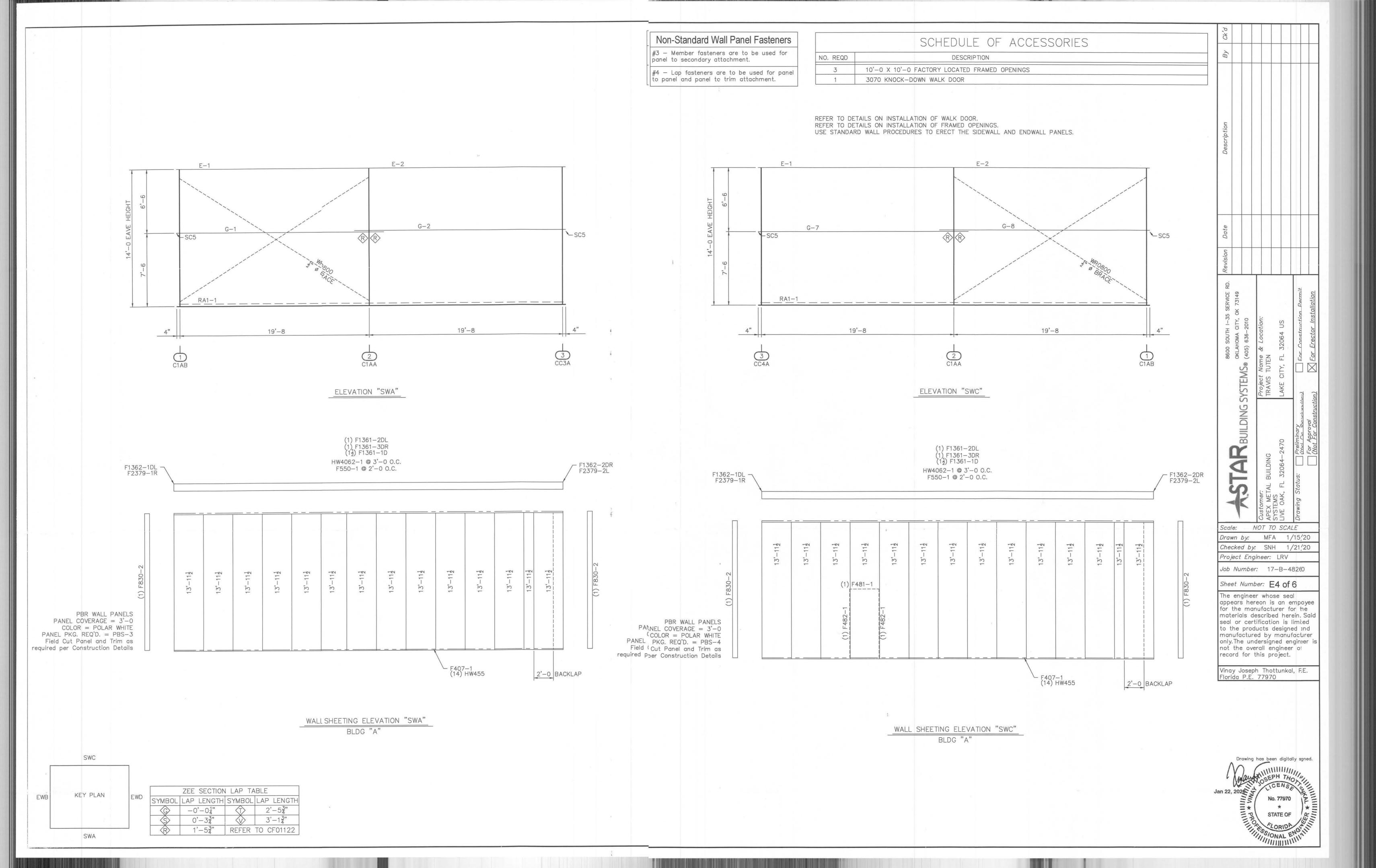
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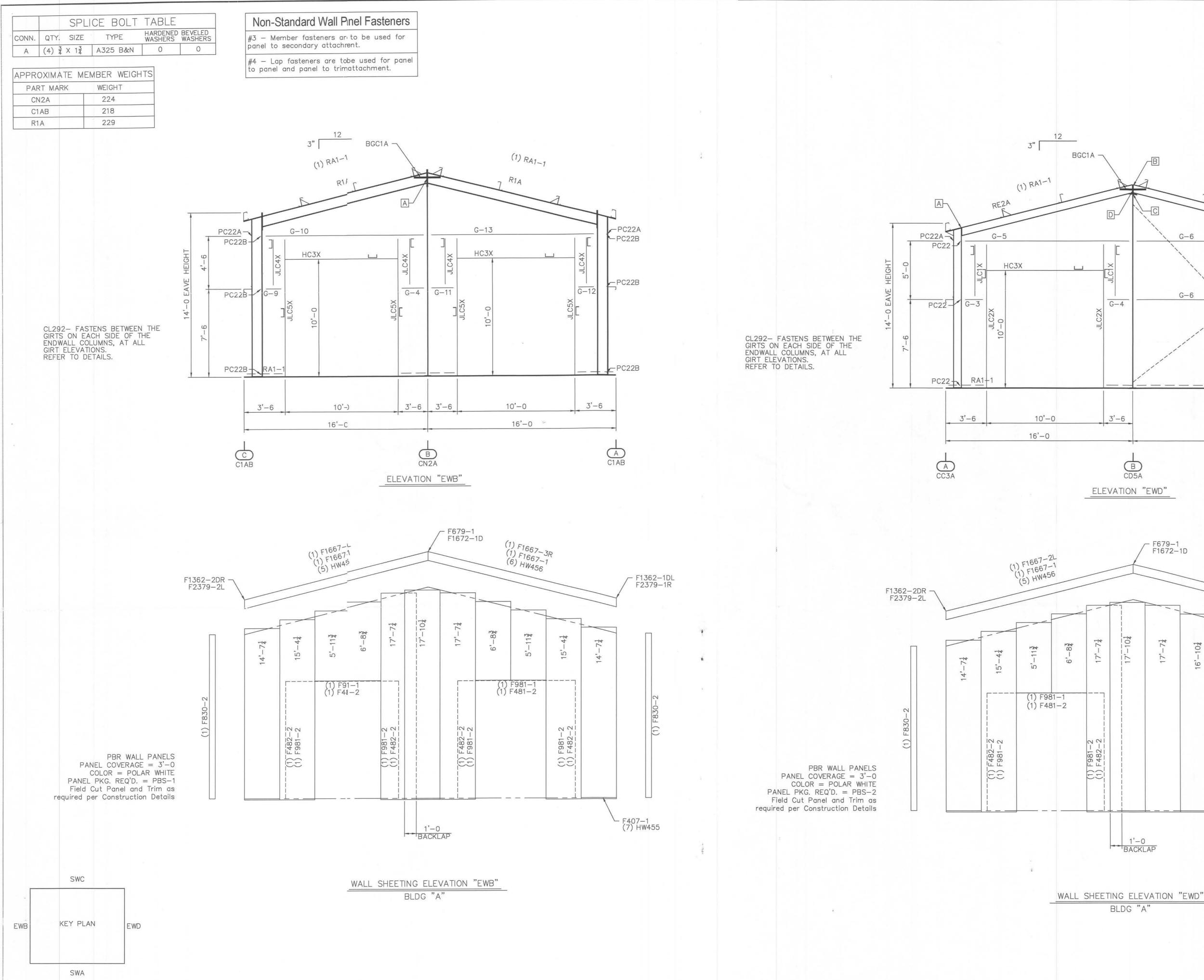
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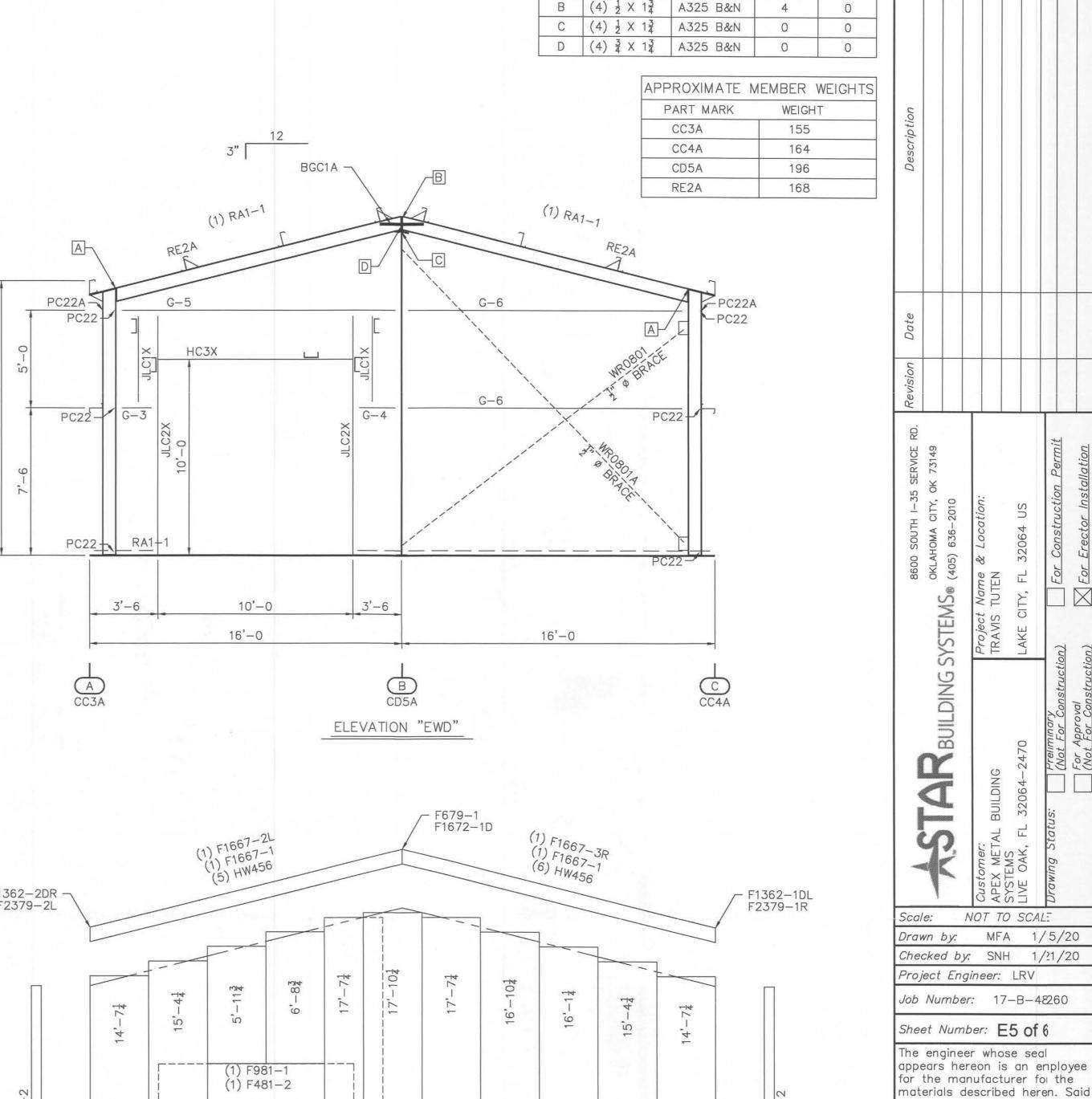
The engineer whose seal appears hereon is an enployee for the manufacturer for the materials described heren. Said seal or certification is linited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

Vinay Joseph Thottunkal, P.E. Florida P.E. 77970

PBR ROOF PANELS ARE TO BE FIELD CUT IF THE PANELS EXTEND OUTSIDE OF THE ROOF PLANE, PANELS ARE NOT TO BE BACK LAPPED. ROOF SHEETING PLANE 2
PANEL TYPE = PBR (CHARCOAL GRAY)
PANEL OVERHANG = 3"
FROM OUTER STEEL **GPR02016** CPR05006 CPR05006 GPR05022 GPR06011 GPR06002 GPR\$6001 (80) HW44041-1 GPR 06000 (4) SA85 CPR05006 Non-Standard Roof Panel Fasteners #58 — Member fasteners are to be used for Scale: NOT TO SCALE panel to secondary attachment. Drawn by: MFA 1/15/20 CPR05006 #4 - Lap fasteners are to be used for panel Checked by: SNH 1/21/20 to panel and panel to trim attachment. \triangle Project Engineer: LRV Job Number: 17-B-48260 Sheet Number: E3 of 6 The engineer whose seal appears hereon is an employee for the manufacturer for the GPR02016 **(** materials described herein. Scio ROOF SHEETING PLAN seal or certification is limited to the products designed and manufactured by manufacture only. The undersigned engineer is not the overall engineer of record for this project. ROOF SHEETING PLANE 1 PANEL TYPE = PBR (CHARCOAL GRAY) PANEL OVERHANG = 3" FROM OUTER STEEL Vinay Joseph Thottunkal, P.E. Florida P.E. 77970 SWC Drawing has been digitally signed F313-1-F797−1 — KEY PLAN EWD F797-1 --- F321-1 DOWNSPOUTS ARE TOBE PLACED AT A SPACING NOT TO EXCEED 40'-0 DOWNSPOUT LAYOUT WITH A DOWNSPOUTWITHIN 20'-0 OF EACH END OF THE GUTTER RUN. [4 REQ'D] SWA







1'-0 BACKLAP

BLDG "A"

SPLICE BOLT TABLE

CONN. QTY. SIZE TYPE

A $(2) \frac{1}{2} \times 1\frac{3}{4}$ A325 B&N

- F407-1 (9) HW455

HARDENED BEVELED WASHERS WASHERS

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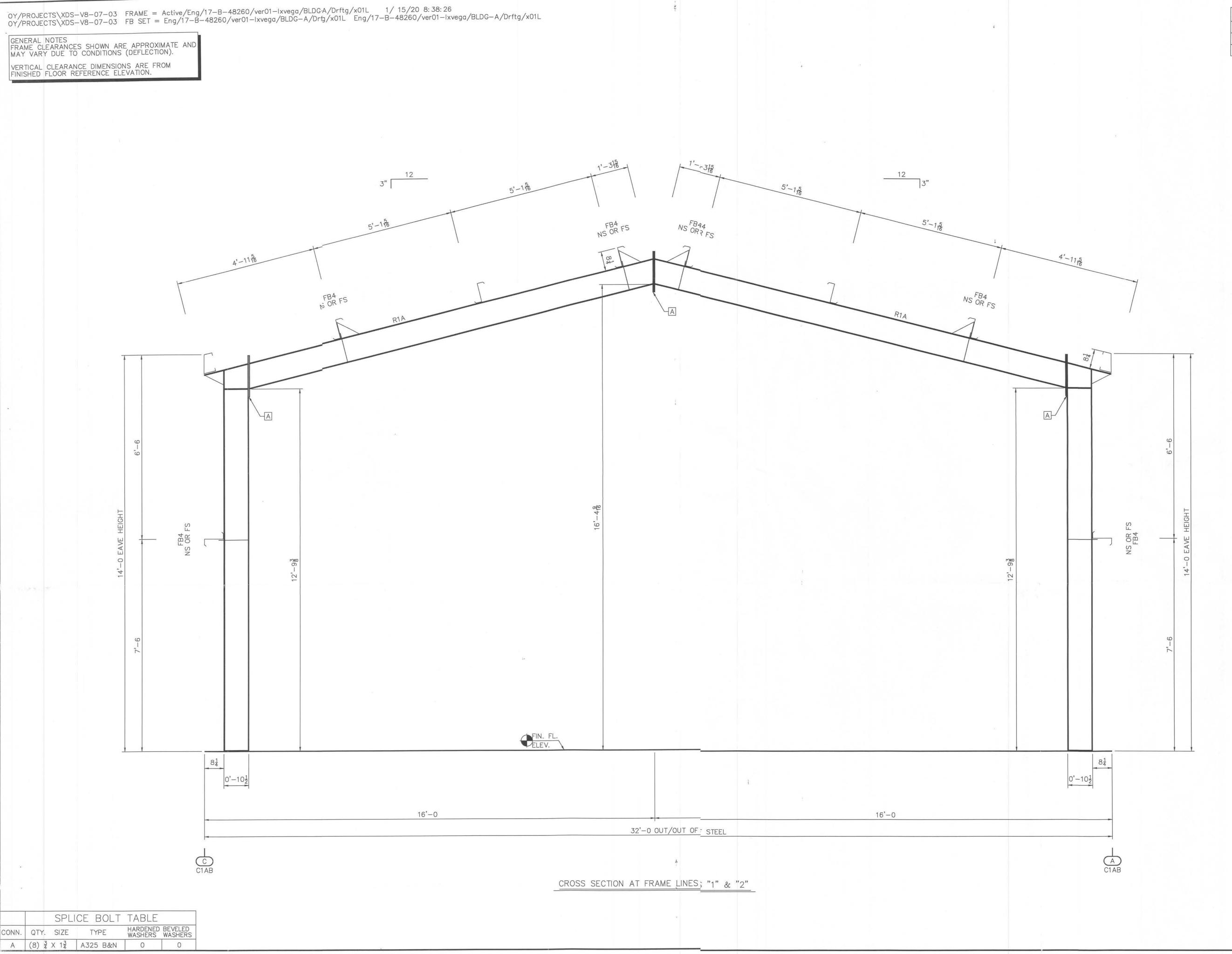
Drawing has been digitally signed.

NOT TO SCALE

seal or certification is limited to the products designed and manufactured by manufacturer

only. The undersigned engineer is not the overall engineer of record for this project.

Vinay Joseph Thottunkal, P.E. Florida P.E. 77970



A (8) ³/₄ X 1³/₄ A325 B&N 0

APPROXIMATE MEMBER WEIGHTS PART MARK WEIGHT R1A 229 218 C1AB

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Revision Date						
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Vinay Joseph Thottun(al, P.E. Florida P.E. 77970

Field Service Procedures

In Order To Give You Prompt Services And Keep Problems To A Minimum, Please Handle Any Shortages Or Back Charges In The Following Manner:

1. Carefully Check Your Packing List While Unloading. 2. Mark Any Items Which Appear To Be Missing And Notify The Field Service Department At The Number Shown In The Title Block As Soon As Possible, Calling Someone Else Could Delay The Proper Response.

In The Event Of An Error, The Customer Must Promptly Make A Written Or Verbal "Initial Claim" to The Manufacturer For The Correction Of Design, Drafting, Bill Of Materials Or Fabrication Error.

- The "Initial Claim" Includes: 1. Description Of The Nature And Extent Of The Errors, Including Quantities. 2. Description Of The Nature And Extent Of Proposed Corrective Work,
 - Including Estimated Man-Hours. 3. Materials To Be Purchased From Other Than the Manufacturer, Including
 - Estimated Quantities and Cost.
 - 4. Maximum Total Cost Of Proposed Corrective Work And Materials To Be Purchased From Other Than The Manufacturer.

Immediately Upon Delivery Of Materials, Quantities Are To Be Verified By The Customer Against Quantities That Are Billed On The Shipping Documents. Neither The Manufacturer Nor The Carrier Is Responsible For Material Shortages Against The Quantities Billed On The Shipping Documents If Such Shortages Are Not Noted On The Shipping Documents When The Material Is Delivered And Acknowledged By The Carrier's Agent. If The Carrier Is The Manufacturer, Claims For Shortages Are To Be Made By The Customer To The Common Carrier. If The Material Quantities Received Are Correct According To The Quantities Billed On The Shipping Documents, But Are Less Than The Quantities Ordered Or The Quantities That Are Necessary To Complete The Metal Building According To The Order Documents, Claim Is To Be Made To The Manufacturer.

DAMAGED OR DEFECTIVE MATERIAL: Damaged Or Defective Material, Regardless Of The Degree Of Damage, Must be Noted On The Shipping Documents By The Customer And Acknowledged By The Carrier's Agent. The Manufacturer Is Not Responsible For Material Damaged In Unloading Of Packages Or Nested Materials, Including, But Not Limited To: Fasteners, Sheet Metal, "C" And "Z" Sections And Covering Panels That Become Wet And/Or Damaged By Water While In The Possession Of Others. Packaged Or Nested Material That Become Wet In Transit Must Be Unpacked, Unstacked And Dried By The Customer. If The Carrier Is The Manufacturer, The Customer Must Make Claim For Damaged Directly To The Manufacturer. If The Carrier Is A Common Carrier, The Customer Must Make The Claim For Damage To The Common Carrier. The Manufacturer Is Not Liable For Any Claim Whatsoever

By Visual Inspection.

The Manufacturer Reserves The Right To Recover Any Material Delivered In Excess Of Those Required By The Order Documents.

Including, But Not Limited To Labor Charges Of Consequential Damages Resulting

From Customer's Use Of Damaged Or Defective Materials That Can Be Detected

OIL CANNING IS NOT A CAUSE FOR REJECTION

Authorization Fo/Corrective Work

Normal Erection Opertions Include The Correction Of Minor Misfits By Amounts Of Reaming, Chipping, Wding Or Cutting And The Drawing Of Elements Into Line Through The Use Of rift Pins. Errors That Cannot Be Corrected By The Foregoing Means Or Which Reque Major Changes In The Member Configuration Should Be Reported ImmediatelyTo The Owner And The Fabricator By The Erector, To Enable Whoever Is Responsib Either To Correct The Error Or Approve The Most Efficient And Economical Metho Of Correction To Be Used By Others. (AISC 303-10, Section 7.14). If Theirror Is The Fault Of The Manufacturer An "Authorization For Corrective Work" Mus Be Issued In Writing By The Manufacturer To Authorize The Corrective Work At ACost Not To Exceed The Maximum Total Cost Set Forth. Alternative CorrectiveWork Other Than That Proposed In The "Initial Claim" May Be Directed By The bnufacturer In The "Authorization Of Corrective Work". Only The Field Service Deprtment May Authorize Corrective Work.

The "Final Claim" In Iriting Must Be Forwarded By The Customer To The Manufacturer Within (0) Days Of The Completion Of The Corrective Work Authorized By The Mnufacturer.

THE "FINAL CLAIM" NST INCLUDE:

- 1. Actual Numbe Of Man-Hours By Dated Of Direct Labor Use On Corrective Work And Acial Hourly Rate Of Pay.
- 2. Taxes And Ingrance On Total Actual Direct Labor.
- 3. Other Direct osts On Actual Direct Labor. 4. Cost Of Mateals (Not Minor Supplies) Authorized By The Manufacturer To Be Purchase From Other Than The Manufacturer, Including Copies Of Paid
- 5. Total Actual irect Cost Of Corrective Work (Sum Of 1, 2, 3, And 4). The "Final ClaimsAre Credited To The Customer By The Manufacturer In The Amount Not'o Exceed The Lesser Of The Maximum Total Cost Set Forth In The "Authrization For Corrective Work" Or The Total Direct Cost Of Corrective Wk.

Cost Of Equipment (ental Or Depreciation), Small Tools, Supervision, Overhead And Profit Are Not abjected To Claims.

SHIPMENT ARRIVAL TIE:

Every Effort Will Be ade To See That The Carrier Arrives At The Jobsite On The Requested Hour. Manfacturer Makes No Warranty And Accepts No Responsibility For Costs Associated With A Shipment Not Arriving At The Requested Time Unless A Separate Agreemet Has Been Made In Writing For A Guaranteed Arrival Time.

Unloading, Handling And Storage

STRUCTURAL:

A Great Amount Of Time And Trouble Can Be Saved Hif The Building Parts Are Unloaded At The Building Site According To A Pre-Arrranged Plan. Proper Location And Handling Of Components Will Eliminate Unnecessarity Handling.

Piece Marks Are Stenciled On The Primary Structural Members At The Lower End, 1'-0" From The End. Inspect All Shipments Prior To RReleasing The Tie-downs For Loads That May Have Shifted During Transit.

Blocking Under Columns And Rafters Protect The Splice Plates And The Slab From Damage During The Unloading Process. It Also Facilitates The Placing Of Slings And Cables Around Members For Later Lifting And Allqows Members To Be Bolted Together Into Sub-assemblies While On The Ground. Extra Care Should Always Be Exercised In The Unloading Operation To Prevent Injurities From Handling Steel And To Prevent Damage To Materials And The Concrete Skyab. If Water Is Allowed To Remain For Extended Periods In Bundles Of Primed Pagets Such As Girts, Purlins, Etc., The Pigment Will Fade And The Paint Will Gradually Soften Reducing Its Bond To The Steel. Therefore, Upon Receipt Of A Job, All Baundles Of Primed Parts Should Be Stored At An Angle To Allow Any Trapped Water To Drain Away And Permit Air Circulation For Drying. Puddles Of Water Shhould Not Be Allowed To Collect And Remain On Columns Or Rafters For Same Reason.

The Coat Of Shop Primer Is Intended To Protect The ! Steel Framing Only For A Short Period Of Exposure To Ordinary Atmospheric Coronditions. The Coat Of Shop Primer Does Not Provide The Uniformity Of Appearance, Or The Durability And Corrosion Resistance Of A Field Applied Finish Coat Off Paint Over Shop Primer.

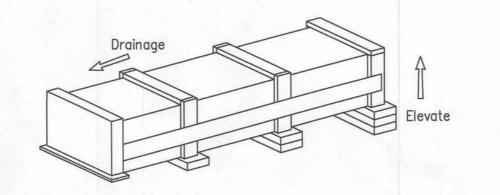
Roof And Wall Panels

Manufacturer's Roof And Wall Panels Include Color Coated, Galvalume, And Galvanized, Provide Excellent Service Under Widely Varied Conditions, All Unloading And Erection Personnel Should Fully Understand That These Panels Are Quality Merchandise, Which Merits Cautious Care And Handling.

UNDER NO CIRCUMSTANCES SHOULD PANELS BE HANDLED ROUGHLY Packages Of Sheets Should Be Lifted Off The Truck With Extreme Care Taken To Ensure That No Damage Occurs To Ends Of The Sheets Or to Side Ribs. The Packages Should Be Stored Off The Ground Sufficiently High To Allow Air Circulation Underneath The Packages. This Avoids Ground Moisture And Deters People From Walking On The Packages. One End Of The Package Should Be Elevated To Encourage Drainage In Case Of Rain. The Manufacturer Exercises Caution During Fabrication An Shipping Operations To Ensure That All Panel Stock Is Kept Dry. However Due To Climatic Conditions, Water Formed By Condensation Of Humid Air Become Trapped Between Sheets. Water Can Also Be Trapped Between The Stacked Sheets When Exposed To Rain. This May Discoloration Caused By Trapped Moisture. The Stain Is Usually Superficial And Has Little Effect On The Appearance Or Service Life Of The Panels As Long As It Not Permitted To Remain On The Panel. However, Moisture In Contact With The Surface Of The panel Over An Extended Period Can Severely Attack The Finish And Reduce The Effective Service Life. See R1-07 Titled "Damage From Condensation Or Trapped Water".

Care Should Always Be Taken When Walking On Panels. Use Safety Lines And Net When Necessary. Panels Are Slippery, Wipe Dry Any Moisture Or Surface Material That Has Puddle From Bundles Stored On A Slope. Dew, Frost, Or Other Forms Of Moisture Greatly Increase The Slipperiness Of The Panels, Always Assume Panel Surface Is Slippery And Act Accordingly, Never Walk Of Step On Skylights Or

Use Wood Blocking To Elevate And Slope The Panels In A Manner That Allows Moisture To Drain. Wood Blocking Placed Between Bundles Will Provide Additional Air Circulation. When Handling Or Uncrating The Panels, Lift Rather Than Slide Them Apart. Burred Edges May Scratch The Coated Surfaces When Sheets Are Slid Over One Another, Never Allow Panels To Be Walked On While On The Ground.



Roof And Wall Panel Damage During Construction

The Quality Of Workmanship In Steel Construction Practices And Handling Methods Used During The Construction Of The Metal Building Can Significantly Affect The Appearance And Performance Of The Building Panels. Panel Damage During Construction Can Be The Result Of Faulty Installation Methods And/or Carelessness.

Overdriven Fasteners Cause Indentations Or Shallow Pockets In The Panel Around The Fastener Head. Rain Water Or Condensation Moisture Combined With Atmospheric Pollutants (principally Sulfur Dioxides) And Dirt Particles Collect In These Pockets. The Combination Of Pollutants And Water Creates Acid Solutions That Will Cause Corrosion Damage To The Panel And Fastener, Rain May Wash Some Pollutants Away, But Moisture In Form Of High Humidity Can Keep These Areas Wet And Continue The Problem. Overdriving The Fastener Also Forces The Sealing Washer From Under The Head Creating A Leak At This Point. Proper Torque Adjustment Of The Screw Gun Or Preferably The Use Of A Depth Gauge Will Eliminate The Problem Of Overdriven Fasteners.

It Is Extremely Important That All Drill Shavings From The Installation Of Panel Fasteners And Fillings From The Saw Cutting Of Panels Be Removed From The Panel Surface, Corrosion Can Occur In A Matter Of Hours When These Shavings Or Fillings Are Not Removed And Are In Contact With Water Or Condensed Moisture. When Panels Are Pre-Drilled Or Cut In The Stack Prior To Erection All Shavings Must Be Cleaned From Both Sides Of The Panel To Prevent Corrosion Of The Panel By These Particles. It Is Imperative That The Roof Be Swept Clean At Least Daily And Certainly At Job Completion. The Final Cleaning Of The Roof Should Be Done Prior To Installing The Gutter So That The Shavings Are Not Deposited Into The Gutter And Left To Corrode. Any Other Foreign Objects Or Debris Left By Construction Personnel Should Also Be Removed From The Roof During The Erection Of The Roof And The Installation Of Such Equipment As Air Condition Units, Etc..

Personnel Walking On The Panel Can Cause Damage. Workmen Should Step Or Walk In The Broad Flat Areas Of The Panel And Avoid Stepping On The Panel Ends And Edges Which Can Be Bent By Careless Handling. If This Damage Is Severe, The Edges Must Be Straighten Prior To Erection Since The Appearance And/or Weather Tightness Of The Panel Could Be Affected. Dragging One Panel Across Another Can Cut Or Abrade The Coating Causing Unsightly Marks On The Panel Surface.

Attempts To Erect Panels During Windy Conditions Should Be Avoided To Prevent Damage And Of Safety Considerations.

Leaving Dirt Piled Against The Exterior Wall Panels At The Foundation Will Cause Panel Damage. This Dirt May Be Wet Or At Least Contain Some Moisture. Mud May Have Splashed Onto The Wall During Construction. Corrosion Damage May Occur Where This Dirt Or Mud Contacts The Panel. In Areas Where Lime Stabilization Of The Soil Is Required, Corrosion Damage From The Soil's Content Will Be Accelerated And Most Likely Be Severe. All Dirt Must Be Removed From The Panel Walls At The Time Of Completion Of Work. Pre-Painted Panels May Require Touch-up If The Coating Has Been Damaged During Handling Or Erection.

The Appearance Of The Building May Be Affected If Damaged Spots Or Scratches Are Located In Highly Visible Places Such As Around Doors, Windows, Etc.. If Damage Is Extensive Then Replacement Of The Entire Panel Should Be Considered.

Types Of Finishes

All Structural Members Of The Metal Building System Not Fabricated Of Corrosion Resistant Material Or Protected By A Corrosion Resistant Coating Are Painted With One Coat Of Shop Primer Meeting The Performance Requirements Of SSPC Paint Specification No.15. The Coat Of Shop Primer Is Intended To Protect The Steel Framing For Only A Short Period Of Exposure To Ordinary Atmospheric Conditions. Shop Primed Steel Which Is Stored In The Field Pending Erection Should Be Kept Free Of The Ground And So Positioned As To Minimize Water Holding Pockets, Dust, Mud And Other Contamination Of The Primer Film. Repairs Of Damaged To Primed Surfaces And/Or Removal Of Foreign Material Due To Improper Field Storage Or Site Conditions Are Not The Responsibility Of The Manufacturer. The Manufacturer Is Not Responsible For Deterioration Of The Shop Coat Of Primer Or Corrosion That May Result From Exposure To Atmospheric And Environmental Conditions, Nor The Compatibility Of The Primer To Any Field Applied Coating. Minor Abrasions To The Shop Coat (Including Galvanizing) Caused By Handling, Loading, Shipping, Unloading And Erection After Painting Or Galvanizing Are Unavoidable. (MBMA 2012, Chapter IV 4.2.4).

Galvalume Is The Trade Name For A Patented Steel Sheet And Coil Product Having A Coating Of Corrosion Resistant Aluminum-Zinc Alloy. The Mixture Is Balanced To Obtain The Coating That Retains The Corrosion Resistance And Heat Reflectivity Of Aluminum And Galvanic Protection Of Zinc. The Best Properties Of Both Aluminum And Zinc Are Combined In This Coating And Offer Added Service Life For The Building.

Using Galvalume Steel As A Substrate, Pre-Painted Steel Is Given An Additional Rust Inhibitor Primer Coat. This Primer Coat Further Increases The Corrosion Resistance. These Coatings Are Applied To The Exterior Surface Of The Panels And A Wash Coat Designed Only For Interior Use, Is Applied On The Opposite Side. Galvalume And Pre-Painted Steel Can Give Excellent Service For Many Years If A Few Rules Concerning Their Care And Maintenance Are Observed. All Of These Finishes Are Equally Subject To Damage And Corrosion When Care Is Not Provided.

PAINT AND COATING MAINTENANCE:

Remove Smudge Marks From Bare Galvalume: Formula 409 Has Proven To Be Somewhat Effective. Lightly Rub With A Clean Cloth And Rinse With Water. Do Not Rub More Than Required To Remove Smudge Marks. No Product Will Remove All Smudge Marks.

Remove Rust Stains: Soft Scrub Without Bleach Has Proven To be Somewhat Effective. Rub With A Soft Cloth And Rinse With Water. Do Not Rub More Than Required To Remove

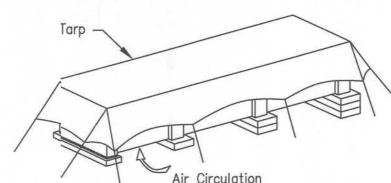
Stain. No Product Will Completely Remove Rust Stains. To Touch-Up Scratches In Paint (Not Bare Metal):

Clean Area To Be Painted With Mild Detergent. Rinse Thoroughly And Dry. Using A Small Artist's Brush, Lightly Apply A Minimal Amount Of Color Matched Touch-Up Pain Required To Fill/Cover The Scratch. Contact The Building Manufacturer For Assistance With Ordering/Purchasing Touch-Up Paint As Needed.

Damage From Ondensation Or Trapped Water

It Is Extremely Important That The Panels Be Monitored For Evidence Or Trapped Water Or Moisture (indensation While Awaiting Erection. High Humidity Conditions With Temperature Cling Will Cause Condensation Between Panels Within The Bundle. Condensatio Can Occur Frequently Near The Sea Coast Or Other Large Bodies Of Water.

If Jobsite Covers Ar Used, They Should Be Tied Away From The Bundle At Corners To Allow Ai Circulation Around The Bundle. This Will Help Prevent Moisture Evaporatin From The Ground Or Building Floor From Condensing On The Panels. Plastic Or Cher Impermeable Covers Are Not Recommended. Immediate Action Is Required I The Panels Are Found To Be Wet From Any Cause. The Bundles Must Be Opned And Each Panel Un-Stacked And Thoroughly Dried On Both Sides. Re-Staring The Panel At A Slight Angle To Each Other To Prevent Nesting Will Allow A Circulation And Assist In Keeping The Panel Dry. In Severe Conditions Large Fas Can Be Used To Circulate Air Between The Un-Stacked Panels And Accelerce Drying. Damage To The panel Coating Occurs When Panels Become Wet And A: Allowed To stay wet. damage Can Occur To Nested Panels Within 24 to 48 Hors. This Damage Shows Corrosion And Discoloration Of The Panel Surface And Commonly Called Wet Storage. Stain, Zinc Oxidation, Or "White Rust".



A Softening Of The aint Film Can Occur With Pre-Painted Steel Under Wet Storage Conditions nd The Durability Of The Panel Finish Substantially Decrease. Bare Galvanized AntGalvalume Panels React More Quickly To Surface Oxidation Since They Lack Th Additional Protection Of Paint. Zinc Coated Or Galvalume Panels Under Norma Exposure Form A Zinc Aluminum Oxide Film On Their Surface Allowing A low Oxidation Process Called "Weathering" To Occur That Inhibits Further Consion. In Nested Bundles Constant Contact Of The Panels With Condensed Or Trappd Water Prevents This Weathering Process.

Rapid Oxidation Of ne Zinc or Zinc Aluminum Coating Can Now Occur And May Lead To "Red Rust"In A Short Time. If Discoloration Or Stains Are Minor A Household Cleaner (The Type Used On Porcelain Sinks And Bathtubs May Be Used To Remove Stins. Wire Brushing Or Abrasive Materials Should be Avoided Since Scratching OrRemoval Of The Coating Could Occur. Panel With Significant Damage Should Be eplaced By The Buyer Prior To Erection.

Safety Commitment

The Builder/Contractor Is Responsible For Applying Annd Observing All Pertinent Safety Rules And OSHA Standards As Applicable.

The Building Manufacturer Has A Commitment To Marznufacture Quality Building Components That Can Be Safely Erected. However Thie Safety Commitment And Job Site Practices Of The Erector Are Beyond The Coontrol Of The Building

It Is Strongly Recommended That Safe Working Condititions And Accident Prevention Practices Be The Top Priority Of Any Job Site.

Local, State And Federal Safety And health Standards, Whether Standard Statuary Or Customary, Should Always Be Followed To Help Enhaure Worker Safety.

Make Sure All Employees Know The Safest And Most; Productive Way Of Erecting A Building. Emergency Procedures Should Be Known To All Employees. Daily Meetings Highlighting Safety Procedures Are Also Recommendeed. The Use Of Hard Hats, Rubber Sole Shoes For Roof Work, Proper Equipment: For Handling Material And Safety Nets Where Applicable Are Recommended

For The Purposes Of Determining Lift Requirements, No Bundle Supplied By The Manufacturer Will Exceed 4,000 Pounds. For Further lynformation Also reference The Bill Of Materials For Individual Member Weights Ohf Structural Members. If Additional Information Is Required Contact The Field Service Department.

Excessive Ice And Snow Removal Should Be Removed From The Roof Immediately To Prevent Damage To Roof And Possible Collapse. Do Not Use Metal Tools To remove The Ice Or Snow As This Can Damage The Paint And/Or Galvalume Coatings. Also Be Careful Around Pipes And Flashing's.

Be Extremely Careful If Your Roof Has Light Transmititting Panels. These Panels Will Not Support A Person's Weight And Will Be Difficult Cor Impossible To See If They Are Covered With Ice Or Snow. See MBMA Low-Rise E Building Systems Manual, Appendix A8 For Details On Snow Removal Procedure's. These Procedures Should Commence When Half Of The Design Roof Snow Loadd Is Realized.

DEBRIS REMOVAL:

Any Foreign Debris Such As Sawdust, Dirt, Leaves, Anizimal Droppings, Etc. Will Cause Corrosion Of The Roof, Gutters, Trim, Etc. If LLeft On The Building Surface For A Long Enough Time. The Roof Should Be Periodically Inspected For Such Conditions And If Found, They Should Be Rectified In 1 A Manner Consistent With These Roof Maintenance Guidelines. Never Allow Treatited Lumber Or Concrete/Mortar/Grout To Come In Contact With Roo of Panels, Especially Galvalume For Extended Periods Of Time.

All High-Strength Shall Be Periodically Be Inspected For Tightness, Particularly In Crane Buildings And After Seismic Or Wind Activity. The Crane Manufacturer Will Specify A Minimum Period But It Should Not Exceed Two Years.

- 1. Keep Roof Free Of Debris And Keep Debris Ouut Of Gutter To Allow Water Quickly Drain From The Roof. 2. Do Not Use Wood Blocking To Hold Equipment off The Panel Seams. This
- Blocks The Flow Of Water And Hold Moisture. 3. Do Not Allow Rooftop AC Units Or Evaporative, Coolers To Drain Onto The
- 4. Anything That Traps Or Holds Moisture On A | Roof Will Cause Premature

Roof Maintenance Guidelines

Urethane Sealan

1. Inspect Roof For Damage After Heavy Storms. 2. Inspect And Reseal As Necessary All Roof Curbs And Other Penetrations With

3. Always Get Manufacturer Approval Before Making Any Modifications To The

4. Repaint Any Areas That Are Susceptible To Rust As Required.

- . When Performing Roof Maintenance, Always Take The Following Precautions: a. Use Fall Protection And Other Safety Protection As Required. b. Do Not Walk On Roof Flashing Such As Gutter, Rake, Hip Or Ridge Flash. c. Do Not Walk On Light Transmitting Panels (LTP's). They Will Not Support A
- d. Guard All LTP's And Roof Openings. e. Step Only In The Panel Flat Directly On Or In Close Proximity To A Supporting Roof Structural.

6. After Other Trades Have Been On The Roof For Any Reason, Inspect The Roof For Damage Caused By Workers Including Chemical Or Solvent Spills, Scratches In The Paint Or Galvalume Coating, Excessive Foot Traffic And Punctures. Make Sure That All Debris Or Scrap Left Behind By Workers Is Removed From The Roof Immediately, Avoid Using Cutoff Saws And Welding Equipment Over The Roof, The Roof Must Adequately Protected.

Keep Foot Traffic To A Minimum. Heavy Foot Traffic Can Cause Ponding On Low Pitched Roofs. This Is Particularly True Just Upslope From The Eave And At Always Walk In The Flat Of The Panel Near A Supporting Roof Structural. Do Not

Walk On Trim Or In Gutters. On Bare Galvalume Roofs, Excessive Foot Traffic May Cause Black Burnish Marks. If Regular Foot Traffic Is Planned For A Roof, Provisions Should Be Made For A Properly Designed And Installed Walkway System. In Order To Limit Access To The

Roof, Roof Hatches Or Access Ladders Should Be Locked At All Times. A Sign Posted At The Access Site Stating That Only Authorized Personnel Are Allowed On The Roof. In Addition A Log Book Should Be Kept Of All Visits To The Roof And The Reason For Such Visits.

Never Allow Your Roof To Come In Contact With, Or Water Runoff From Any Dissimilar Metal Including But Not Limited To: Copper, Lead Or Graphite, This Includes Copper And Arsenic Salts Used In Treated Lumber, Calcium Used In Concrete, Mortar And Grout.

Never Step On Light Transmitting Panels (LTP's) Or Unattended Roof Panels



Panels May Collapse If Not Properly Secured

Roof Panels Must Be Completely Attached To The Purlins And To Panels On Either Side Before They Can Be A Safe Walking Surface. Light Transmitting Panels LTP's) Translucent Panels Can Never Be Considered As A Walking Surface.

Partially Attached Or Unattached Panels Should Never Be Walked On!

Do Not:

1. Step On Rib At Edge Of Panel.

2. Step Near Crease In Rib At Edge Of Panel.

3. Step Within 5 Feet Of Edge On Unsecured Panel.

A Single Roof Panel Must Never Be Used As A Work Platform. An OSHA Approved Runway Should Be Used For Work Platforms. (Consult OSHA Safety And Health Regulations For The Construction Industry). Safety First!

SYSTEMS® BUILDING NOT TO SCALE Scale: Drawn by: MFA 1/16/20 Checked by: SNH 1/21/20 roject Engineer: Job Number: 17-B-48260 Sheet Number: R1 of 13

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materials described herein. Said

for the manufacturer for the

seal or certification is limited

to the products designed ard

manufactured by manufacturer

only. The undersigned engineer is

not the overall engineer of

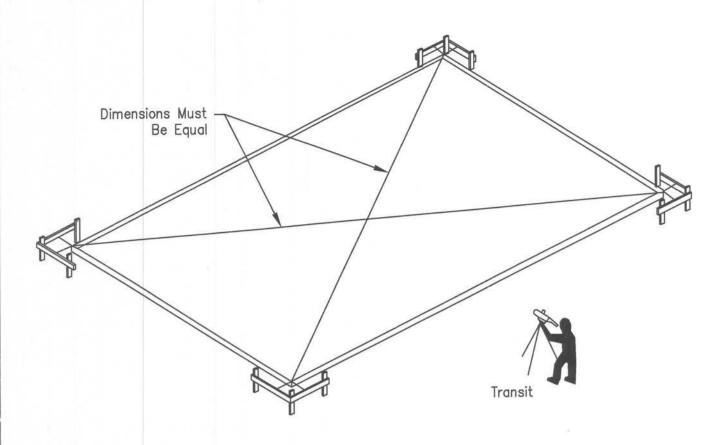
record for this project.

Erection Guide

R1

Building Anchorage

- 1. To Determine That The Foundation Is Square, Measure Diagonal
- Dimensions To Be Sure They Are Of Equal Length.
- 2. To Determine That The Foundation Is Level, Set Up A Transit Or Level And Use A Level Rod To Obtain The Elevation At All Columns.
- 3. Carefully Check The Location Of All Anchor Rods Against The Anchor Rod Setting Plan Furnished By The Manufacturer. All Dimensions Must Be Identical To Assure A Proper Start-up.

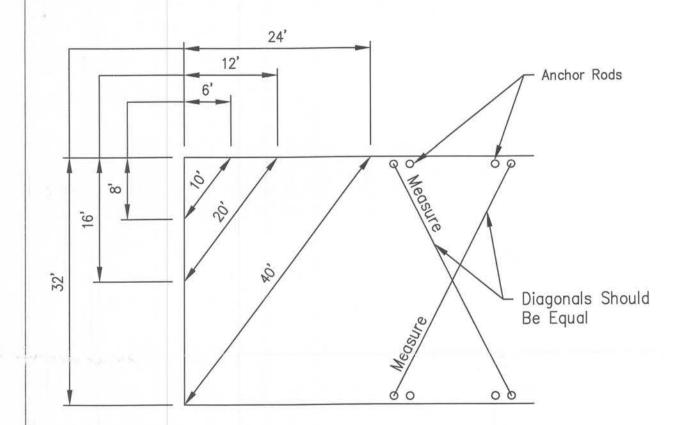


Pre-Erection Notes:

The Following Notes, Procedures And Suggested Recommendations Are Important Parts Of The Pre-Erection Process.

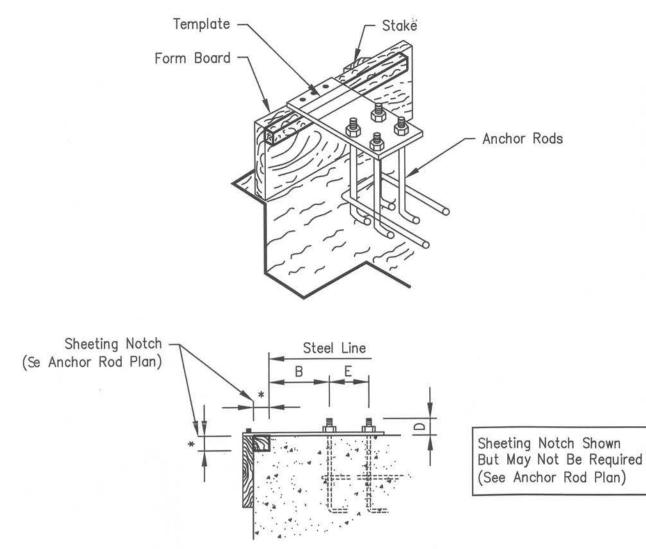
1.) Prior To The Time The Erection Crew Arrives, A Responsible Person Should Check The Job Site For Foundation Readiness, Square, And Accuracy And Anchor Rod Size And Location.

The Drawing Shown Below Indicates A Method Which May Be Used To Check The Foundation And Bolts For Square.

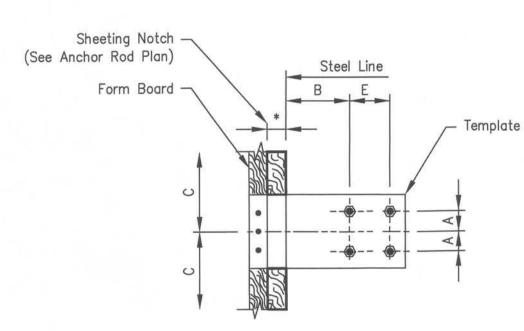


Measure Along Adjacent Sides Of Foundation Using A Pair Of Dimensions Shown. If The Diagonal Distance Between These Points Is As Noted, The Corner Is Square. Diagonal Measurements Between Opposite Anchor Rods Will Indicate If These Bolts Are Set Square.

It is Extremely Important That Anchor Rods Are Placed Accurately And In Accordance With The Anchor Rod Setting Plan. All Anchor Rods Should Be Held In Place With A Template Or Similar Means, So That They Will Remain Plumb And In Correct Location During The Placement Of The Concrete. A Final Check Should Be Made After Completion Of The Concrete Work And Prior To The Steel Installation. This Will Allow Necessary Corrections To Be Made Before Costly Installation Labor And Equipment Arrives.



Projection Of Anchor Rods (D) Given On Anchor Rod Plan



Dimensions A, B, And C Given On Anchor Rod Plan

1" (6 mm)

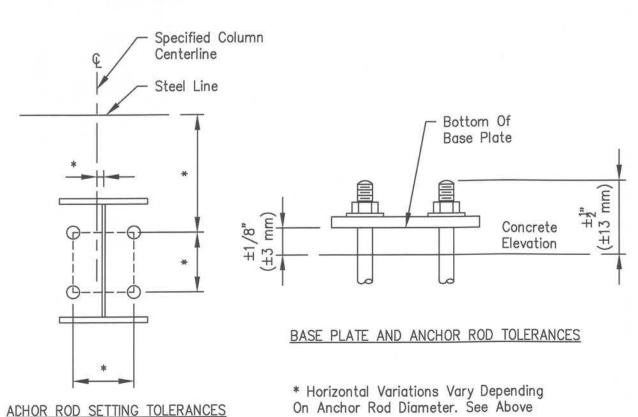
3" (10 mm)

 $\frac{1}{2}$ " (13 mm)

AISC Code Of Standard Practice For Steel Building And Bridges **Folerances For Setting Anchor Rods**

Anchor Rod Diameter, Inches (mm) *Horizontal Variation, Inches (mm)

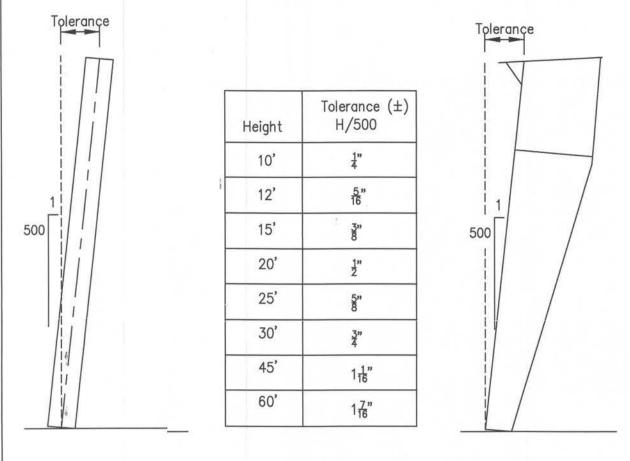
4	and	g" (1	9 And	22 1	nm)
1"	11"	11" (25 3	1 38	mm)
15"	2"	21"	(44 5	50 6.	3 mm)



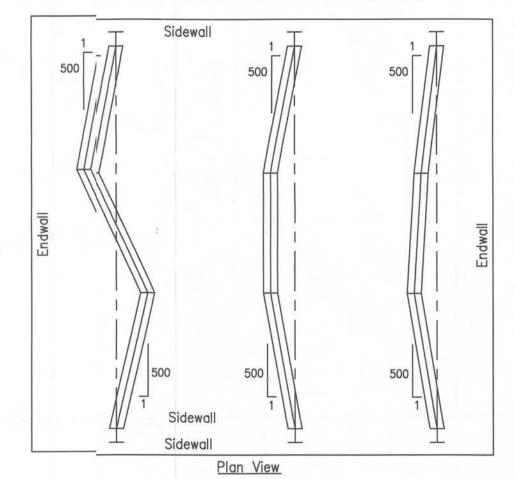
Erection Tolerrances

It is The Responsibility Of The Erector To Determine, Furnish And Install All Temporary Supports Such As Temporary Guys, Beams, Falsework, Cribbing, Or Other Elements Reequired For The Erection Operation (In Accordance With Section 7.10.3 Of ANSI/AISSC 303, Code Of Standard Practice For Steel Building And

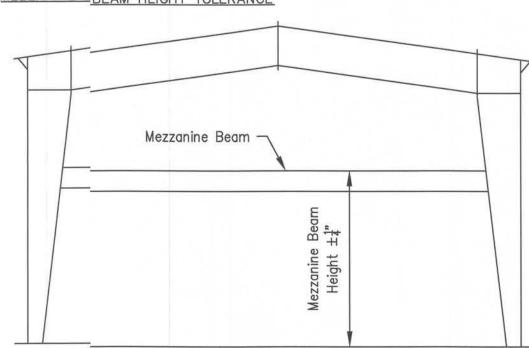
COLUMN ALIGNMENT TOLERANCES



ALIGNMENT T TOLERANCE FOR MEMBERS WITH FIELD SPLICES



MEZZANINE EBEAM HEIGHT TOLERANCE

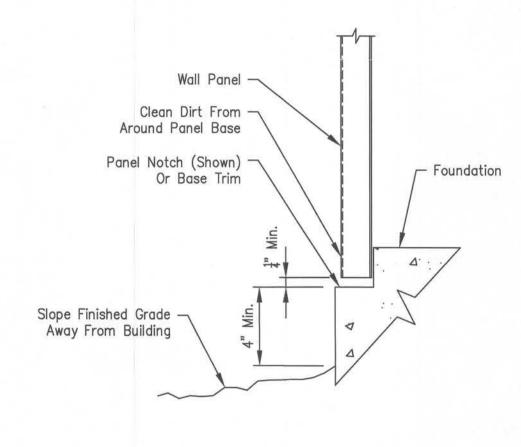


General Erection Notes

- 1.) All Structural Framing Members, Purlins, Girts, Clips, Flange Braces, Bolts, Bracing Systems, Roof And Wall Panels, Etc. Must Be Installed As Shown On
- 2.) It is Extremely Important, Especially During Construction, That Panels At The Eaves, Rakes And Ridges Be Kept Secure.

Panel Cautions And Notes

- To Minimize Potential Of Corrosive Action At The Bottom Edge Of Wall Panels, The Contractor Must Assure That The Following Procedures Are Followed:
- 1.) The Concrete Foundation Should Be Cured For A Minimum Of Seven (7) Days Before Wall Panels Are Installed. (Uncured Concrete Is Highly Alkaline And Metal Panels Can Undergo Varying Degrees Of Corrosive Attack When In Direct Contact With The Concrete.) After The First Week Of The Curing Cycle, The Reaction Between Metallic Coatings On Steel And The Concrete Is Essentially Halted.
- 2.) Top Of Finish Grade At Building To Be A Minimum Of Four (4) Inches Below Bottom Of Panel.
- 3.) Finish Grade Is To Slope Away From Building To Ensure Proper Drainage.
- 4.) Upon Completion Of Finish Grading, All Dirt Is To Be Cleaned From Around Base Of Wall Panel Where It May Have Collected In Panel Notch Or On Base Trim.



Fastener Installation

Correct Fastener Installation Is One Of The Most Critical Steps When Installing Roof/Wall Panels. Drive The Fastener In Until It Is Tight And The Washer Is Firmly Seated. Do Not

A Slight Extrusion Of Neoprene Around The Washer Is A Good Visual Tightness Check. Always Use The Proper Tool To Install Fasteners. A Fastener Driver (Screw Gun) With A RPM Of 1700-2000 Should Be Used For Self-Drilling Screws. A 500-600 RPM Fastener Driver Should Be Used For Self-Tapping Screws. Discard Worn Sockets, These Can Cause The Fastener To Wobble During Installation.

Note: Always Remove Metal Filings From Surface Of Panels At The End Of Each Work Period. Rusting Filings Can Destroy The Paint Finish And Void Any Warranty.



Of Sealing Washer



Of Sealing Washer



Too Loose Compression Of Sealing Washer

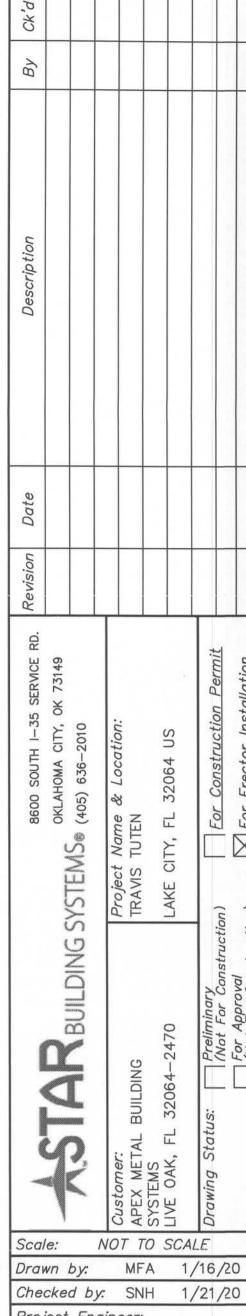
Tape And Tube Sealant

Proper Tape And Tube Sealant Application Is Critical To The Weather Tightness Of A Building. Tape Sealant Should Not Be Stretched When Installed. Apply Only To Clean, Dry Surfaces. Keep Only Enough Sealants On The Roof That Can Be Installed In A Day. During Warm Weather, Store Sealants In A Cool Dry Place. During Cold Weather (below 60°) Sealants Must Be Kept Warm (60°-90°) Until Application. After Tape Sealant Has Been Applied, Keep Protective Paper In Place Until Panel Is Ready To Be Installed.

Important Note

All Details, Recommendations And Suggestions Contained In This Erection Guide Of This Drawings Set Are For General Guidelines Only, And Not Meant To Be All-inclusive. Industry Accepted Installation Practices With Regard To All Areas Not Specifically Discussed In This Section Should Be Followed. Only Experienced, Knowledgeable Installers Familiar With Accepted Practices Should Be Used To Assure A Quality Project.

It is Emphasized That The Manufacturer is Only A Manufacturer Of Metal Building Components And Is Not Engaged In The Installation Of Its Products. Opinions Expressed By The Manufacturer About Installation Practices Noted In The Erection Guide Are Intended To Represent Only A Guide. Both The Quality And Safety Of Installation And The Ultimate Customer Satisfaction With The Completed Building Are Determined By The Experience, Expertise, And Skills Of The Installation Crews, As Well As The Equipment Available For Handling The Materials. Actual Installation Operations, Techniques And Site Conditions Are Beyond The Manufacturers Control.



Scale: NOT TO SCALE	
Drawn by: MFA 1/16,	/20
Checked by: SNH 1/21,	/20
Project Engineer:	

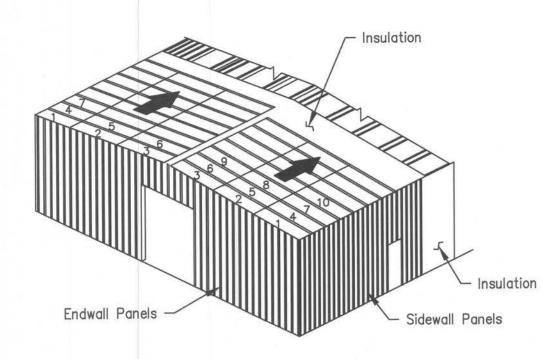
Job Number: 17-B-48260

Sheet Number: R2 of 13

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PBR Roof Panels

For PBR Roofs With Ridge Panels, It Is Recommended That Both Sides Of The Ridge Be Sheeted Simultaneously. This Will Keep The Insulation Covered For The Maximum Amount Of Time And The Panel Ribs Can Be Kept In Proper Alignment For The Ridge Panel. This Is Critical On The PBR Panels So That The Ridge Caps Can Be Properly Installed. Check For Proper Coverage As The Sheeting Progresses.



Install The First Run Of Roof Panels Across The Building From Eave To Eave Or Eave To Ridge. To Allow Proper Installation Of The Rake Trim, The Starting Location For The First Panel Must Be As Shown In The Rake Details Included With The Erection Drawings. When The First Run Is Properly Located And Aligned With The Correct Endlaps And Eave Overhangs, Fasten To Purlins. Roof Panels Should Be Installed So That The Sidelap Is In A Direction Away From Prevailing Wind. Refer To Appropriate Lap Details Included With The Erection Drawings.

Install Remaining Roof Insulation And Panels. To Avoid Accumulative Error Due To Panel Coverage Gain Or Loss, Properly Alian Each Panel Before It Is Fastened. Occasional Checks Should Be Made To Ensure That Correct Panel Coverage Is Maintained, Special Attention Should Be Given To Fastener, Sealant and Closure Requirements. Refer To Details Included With The Erection Drawings.

At Finishing End Of Roof, The Last panels May Require Field Modification For Installation Of Rake Trim. Refer To Rake Details Included With The Erection Drawings. DO NOT BACK LAP THROUGH FASTENED ROOF PANELS.

NOTE: Roof Types And Installation Requirements Will Vary. Refer To The Appropriate Details For Specific Panel Used.

IMPORTANT: Loose Fasteners, Blind Rivets, Drill shavings, Etc.. Must Be Removed From The Roof To Guard Against Corrosion.

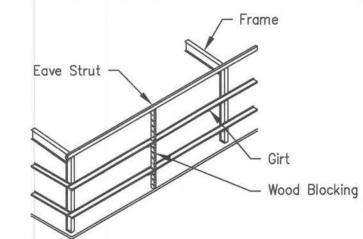
Wall Panels

Proper Horizontal And Vertical Alignment Of Supporting Structure (Girts Or Other Framing) Is The Responsibility Of The Installer. Failure To Align The Secondary members Properly Prior To Wall Installation Can Have A Direct Impact On The Final Appearance And Performance Of The Installed Wall System For Which The Metal Building Manufacturer Is Not Responsible.

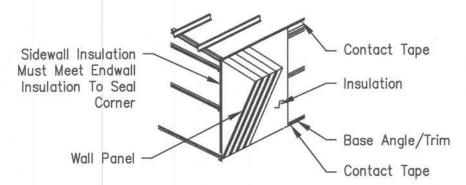
Before Installing Wall Panels, The Girts Must Be Aligned To A Level Position So That There Is No Visible Sag. This Should Be Done Directly Ahead Of Panel

Girt Leveling May Be Accomplished By Standing A Section Of Gable Angle Vertically Against The Outside Girt Flanges At Approximate Mid—bay Location. When Girts Are Level, Attach The Girt Flanges To The Angle With Vise Grip Pliers Or Temporary Screws. Wood Blocking Cut To Fit The Spaces May Also Be Used For Alignment.

Temporary Girt Blocking Is Not Recommended On Concealed Fastener Panels. The Removal Of The Blocks After Panel Installation Can Cause Oil Canning.

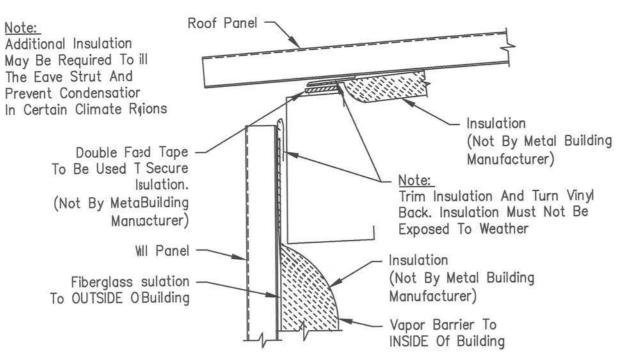


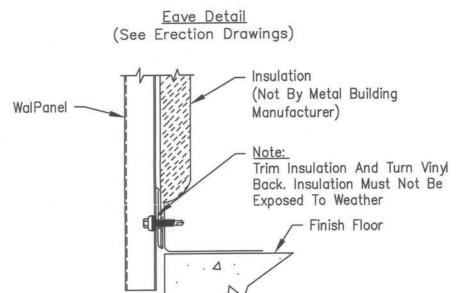
Wall Panel Type And Installation Details Will Vary. Refer To The Erection Drawings And Details For The Specific Panel Used For Your Building.



If Walls Are To Be Insulated With Blanket Insulation Over Girt Flanges, Base And Eave, Place A Continuous Run Of Contact Tape Along The Eave Strut And Base Member.

At The Base, Cut Off The Insulation A Minimum Of 2" Above The Bottom Of The Wall Panel. This Will Prevent The Insulation From Hanging Below The Wall Panel And Wicking Moisture.



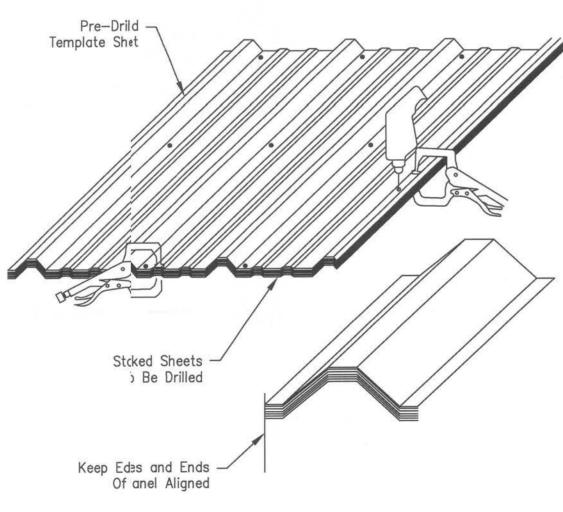


Sidewall Panel Should Be Installed So That The Panel Sidelap Is In A Direction Away From Th Prevailing Wind. Refer To Appropriate Lap Detail Included With Erection Drawigs.)

Base Detail

(See Erection Drawings)

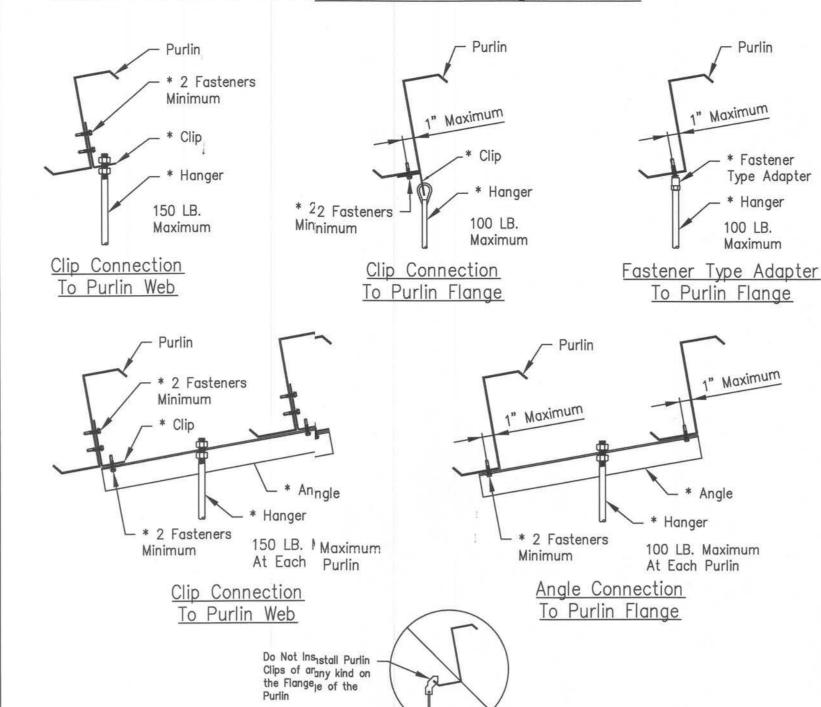
Note: Check Periodially To Ensure That All Panels Are Aligned And Plumb.



Screw Alignment Panel (Through Fastened Panel Only)

After Drilling'anels, It Is Important To Clean Metal Filings Off All Panel Surfaces, Including Beteen Panels That Are Not Installed That Day, To Avoid Rust Stains.

Suggested Method Of Purlin Attachment For Building Accessories

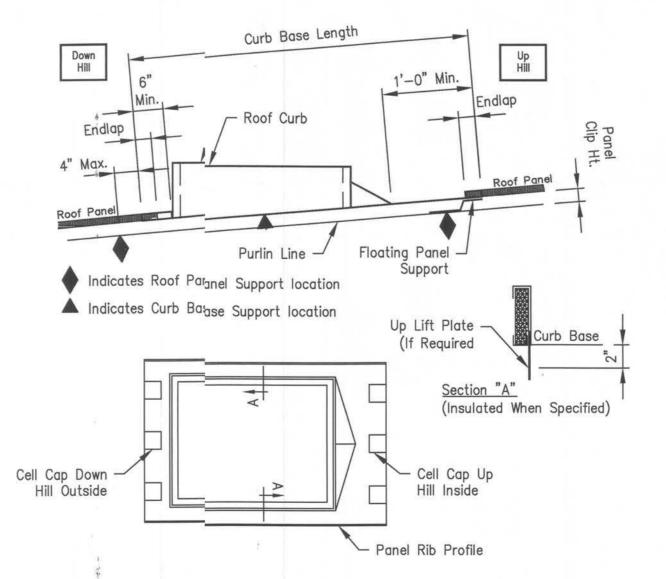


* Denotes Material Not Provided By Metal [Building Manufacturer.

The Total Hanger Load Shall Not Exceed The Design Collateral Load For The Building. Example: 5'-0 (Purlin Spacing) X 5'-0 (Hanger Spacing) X 6 PSF (collateral Load)

= 150 Lbs.See Cover Sheet For Design Collateral Loacid For This Building Note: If The Building Is Designed For O PSSF Collateral Load. Then Adding Any Suspended System (i.e. Duct Work, Pipiping, Lights, Ceilings, Etc.) Will Correspondingly Reduce The Design Live Lobad.

Roof Curbs When Not Supplifed By Building Manufacturer



The Curb Details Shown Illustrate The Building Manufacturers Recommended Curb Style And Installatition Method. It Is The Erector/Installer's Responsibility To Provide The Proper Curb Style And Install Them In Accordance With The Procedures Established By Theese Details. Failure By The Erector/Installer To Follow These Recommendations; May Result In The Curbs Damaging The Roof System Or Excluded From Waarranties.

All Roof Curbs To , Be:

1. .080 Aluminum Or 18 Ga. Stainless Steel (No Galvalume® Or Galvanized). 2. Panel Rib To Poanel Rib (No Flat Skirt Or Lay-Over Curbs). 3. Installed With Down Hill End Over Panel And Up Hill End Under Panel Application For Water Flow Att Panel Splice.

4. Up Lift Preventition For Clip Applied Roof Systems Are Required If:

a. Wind Loadds Exceed 110 MPH. b. Curb Basse Crosses A Purlin.

5. Supported on (4) Sides By Primary Or Secondary Framing. 6. Maximum Single'e Curb Weight Recommended Is 1500 Lbs.

Roof Jack Installation when Not Supplied By Building Manufacturer

 General Installation Notes
 Do Not Use Galvanized Roof Jacks, Lead Hats, Or Other Residential Grade Roof Jacks. These Roof Jacks Do Not Have 20 Year Service Life And In Case Of Lead Hats Will Cause Galvanic Corrosion Of The Roof Panel.

• Use EPDM Rubber Roof Jacks With An Integral Aluminum Band Bonded Into The Perimeter Of The Base. EPDM Roof Jacks Have A Temperature Range From -65°F To 212°F. Use Silicone Roof Jacks For High Temperatures. Silicone Roof Jacks Have A Temperature Range Of -100°F To 437°F.

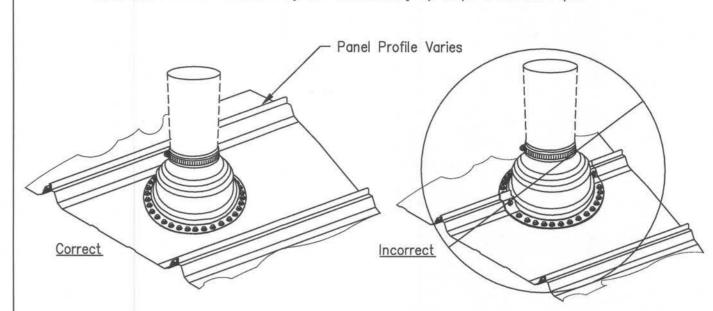
Retrofit Roof Jacks Are Available For Applications In Which The Top Of The Pipe Is Inaccessible, Eliminating The Possibility Of Sliding The Roof Jack Over The Top Of The

• Do Not Use Tube Sealant To Seal The Roof Jack To The Roof Panels. Use Roll Tape Sealer Between The Roof Jack And The Roof Panel And Attach The Roof Jack To The Roof Panel With Fastener #4 $\frac{1}{4}$ - 14 X $\frac{7}{8}$ " LL SD W/washer At 1" O.C. Around The Base Of The Roof Jack. See Table Below For Quantities.

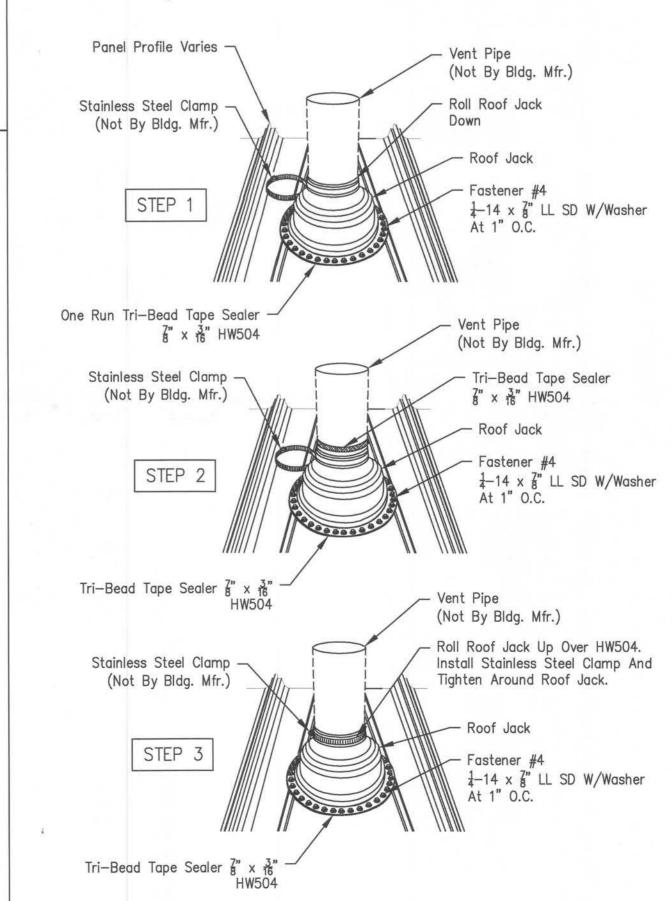
Trim The Top Of The Roof Jack To Fit Over The Pipe, Roll Down The Roof Jack Over The Pipe And Apply Tape Sealer For The Perimeter Of The Roof Jack Base Between The Roof Jack And The Roof Panel. Apply Tape Sealer Around The Pipe And Install A Stainless Steel Clamp (Not By Bldg. Mfr.) Over The Top Of The Roof Jack And Firmly Tighten To Form A Secure Compression Seal.

• If The Pipe Diameter Is So Large To Block The Flow Of Water Down The Roof Panel, A Flat Base Roof Curb Must Be Installed Into The Roof And The Roof Jack Will Be Sealed To The Curb. A Two Piece Curb May Be Required When The Top Of The Pipe Is Inaccessible.

• In Northern Climates, The Pipe Penetration Should Be Protected From Moving Ice Or Snow With A Snow Retention System Immediately Up Slope From The Pipe.



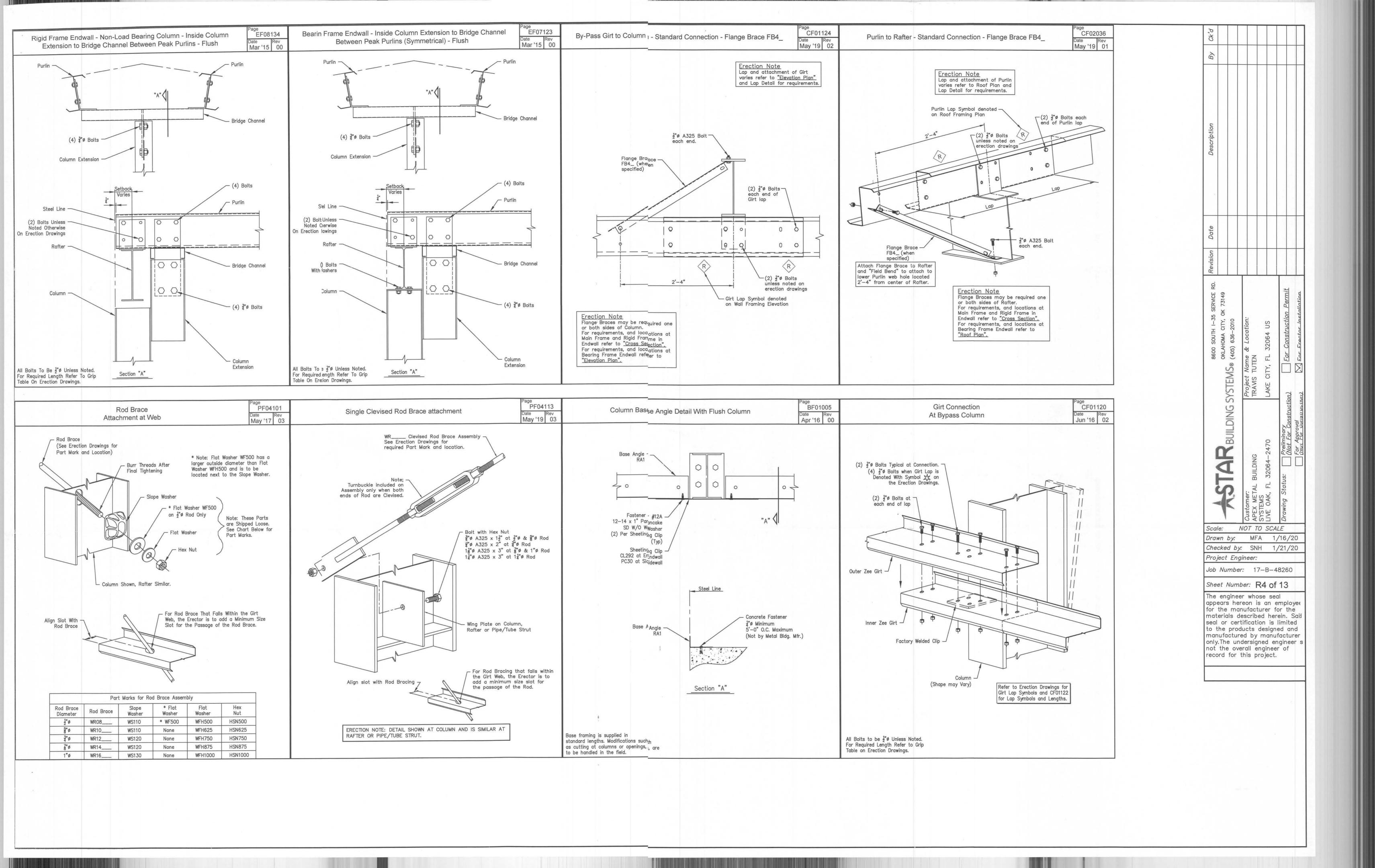
Install Pipe In Center To Allow Base Of Roof Jack To Lay Flat on Panel. Cannot Encompass More Than 75% Of Panel.

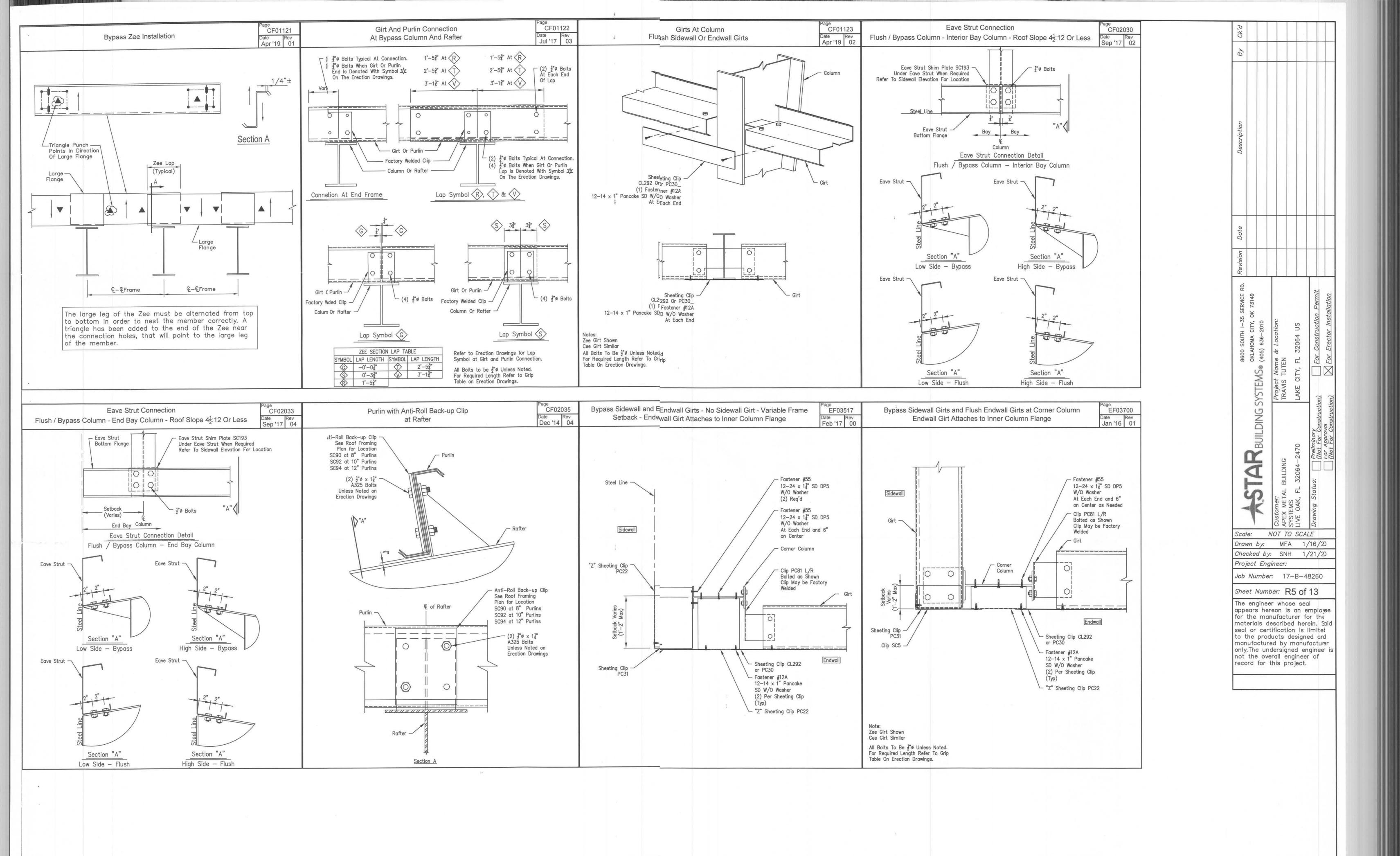


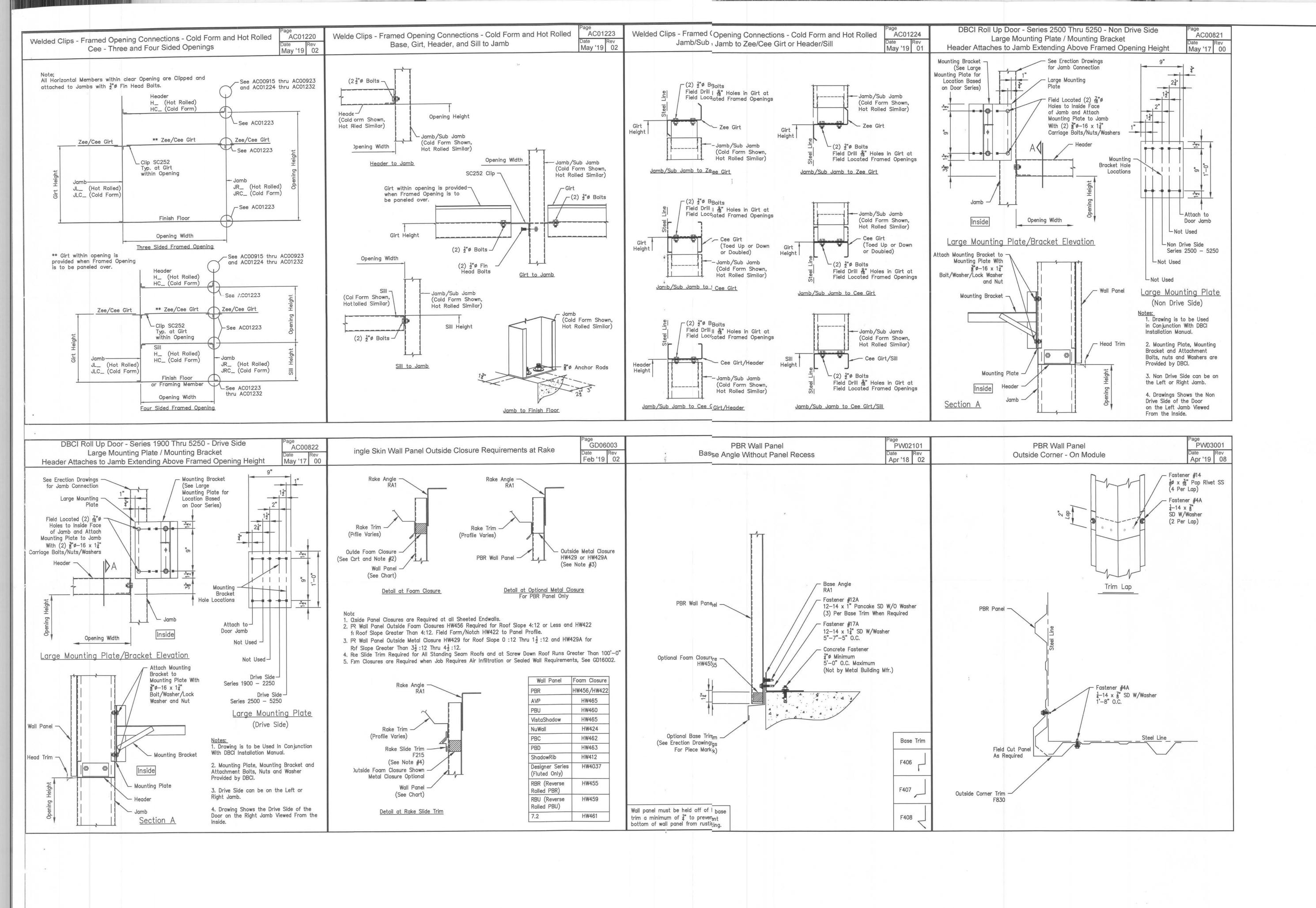
SYSTEMS® BUILDING Scale: NOT TO SCALE Drawn by: MFA 1/16/20 Checked by: SNH 1/21/20 Project Engineer: Job Number: 17-B-48260 Sheet Number: R3 of 13

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record for this project.







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Project Engineer:

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Checked by: SNH 1/21/20

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Sheet Number: R6 of 13

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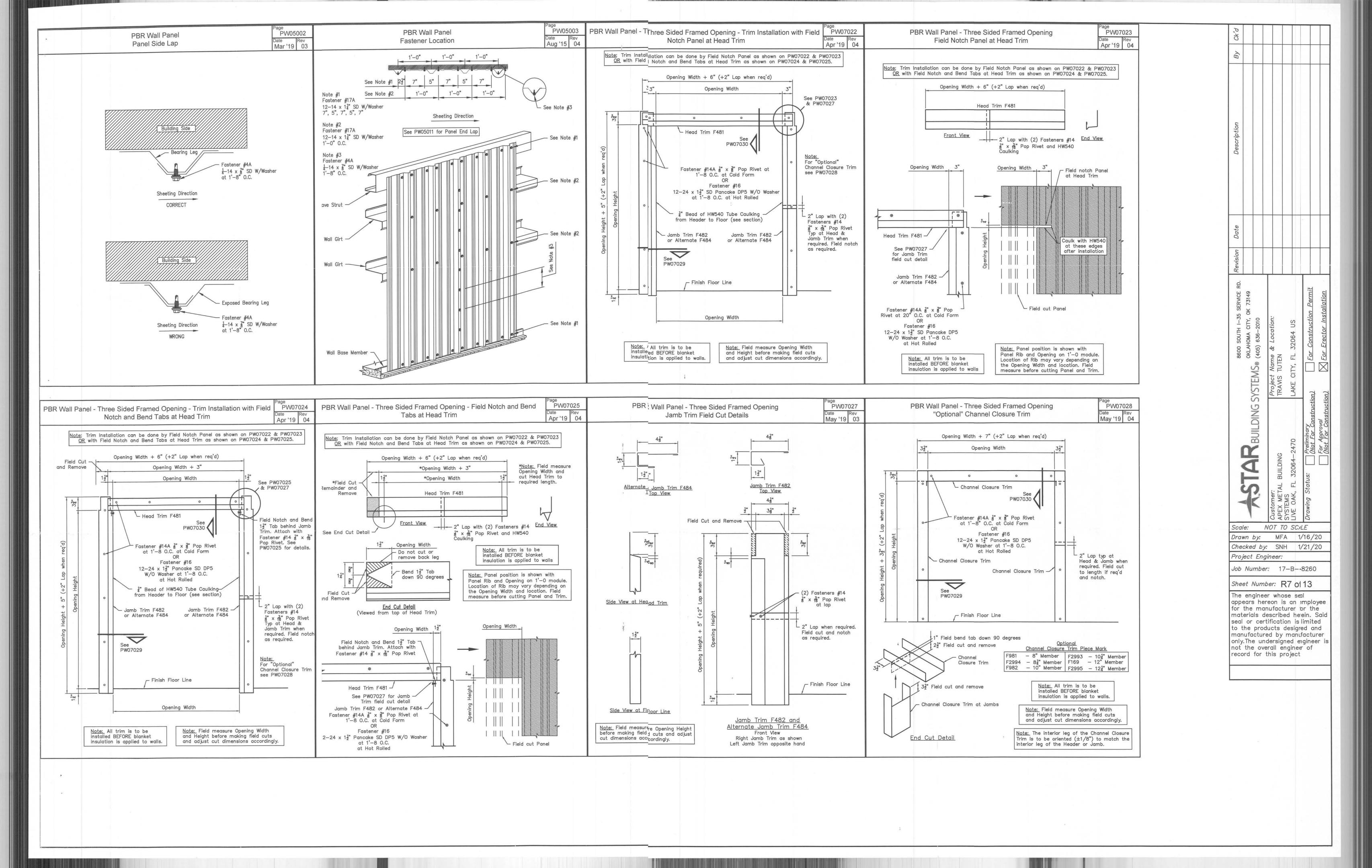
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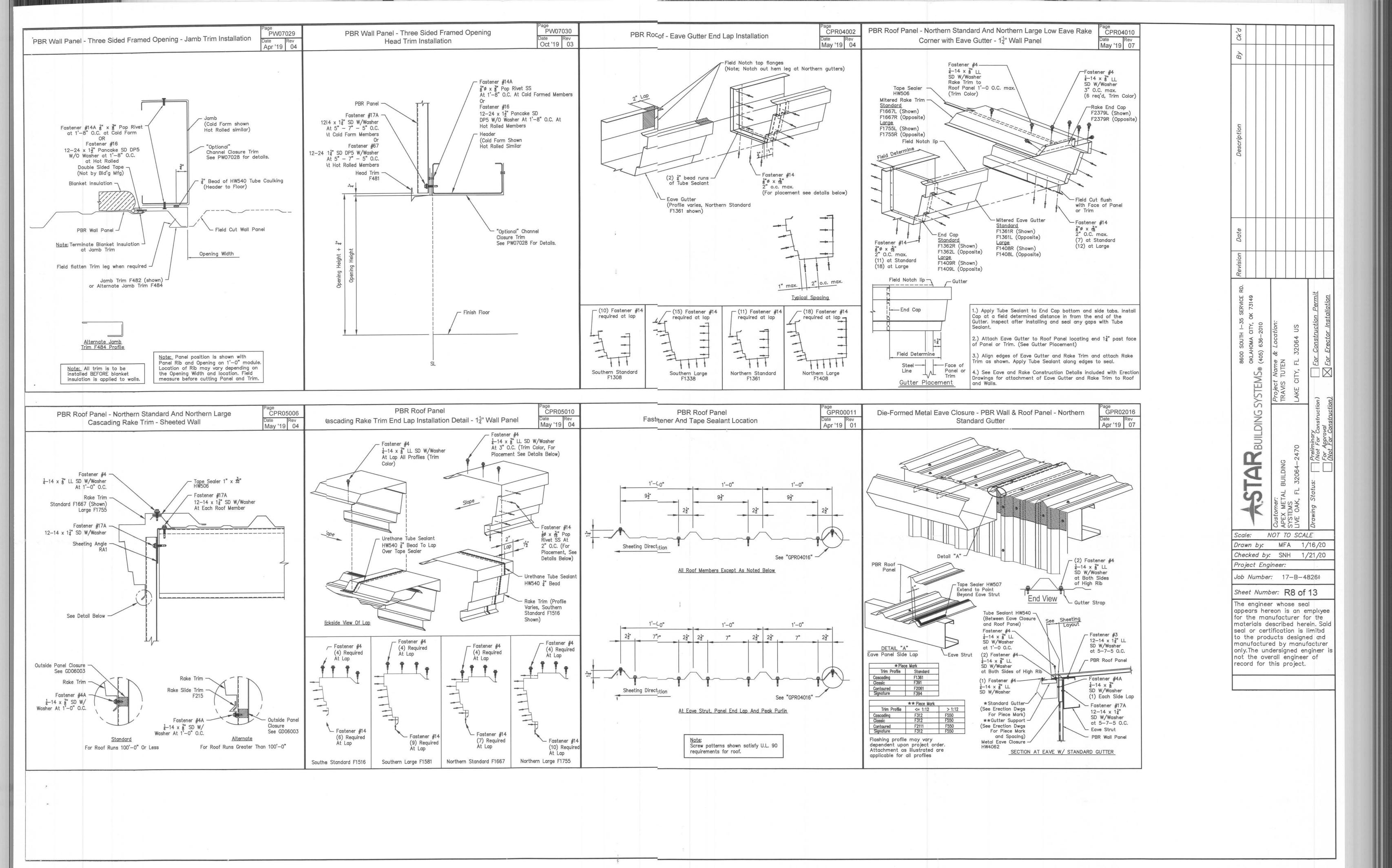
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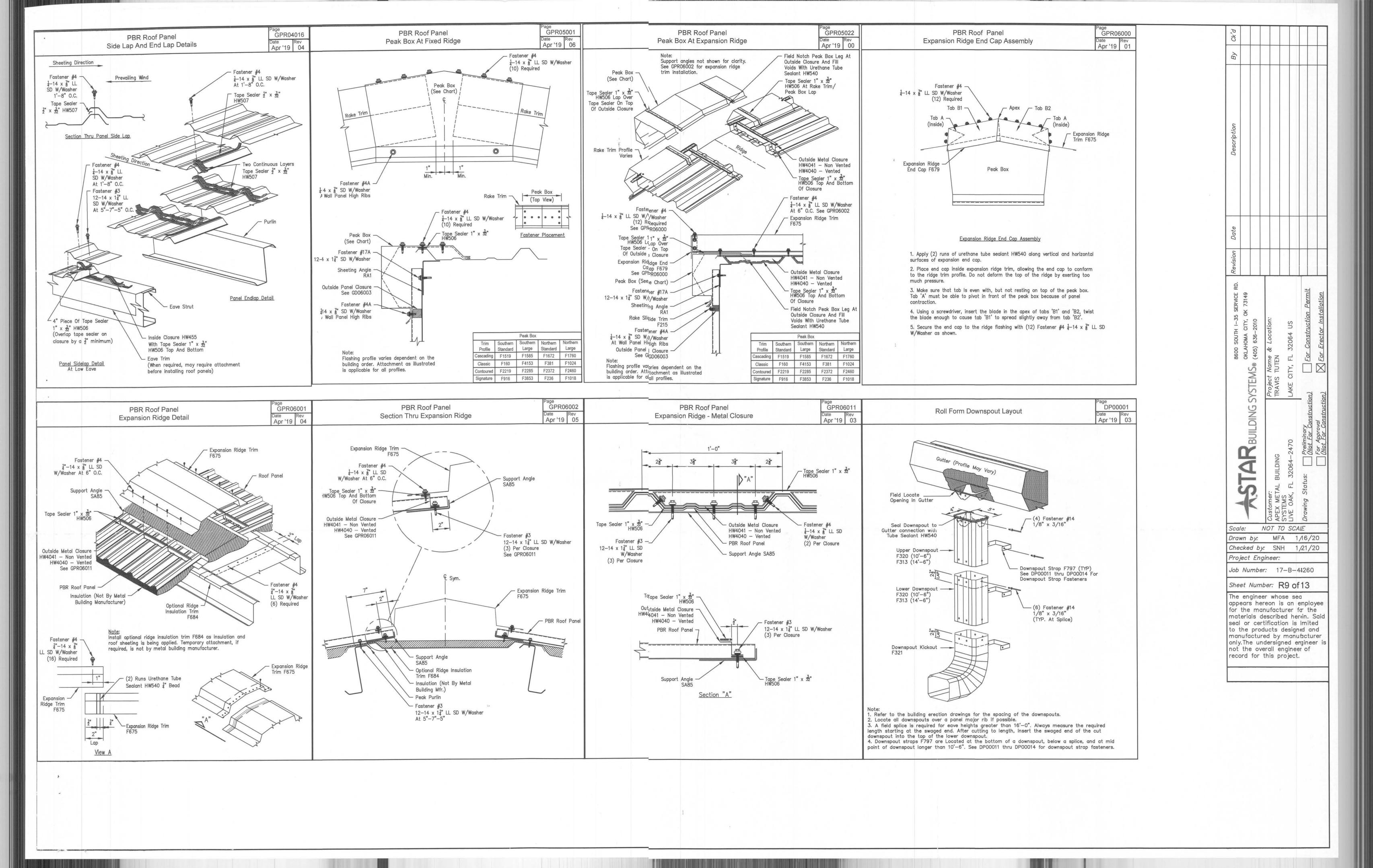
only. The undersigned engineer is

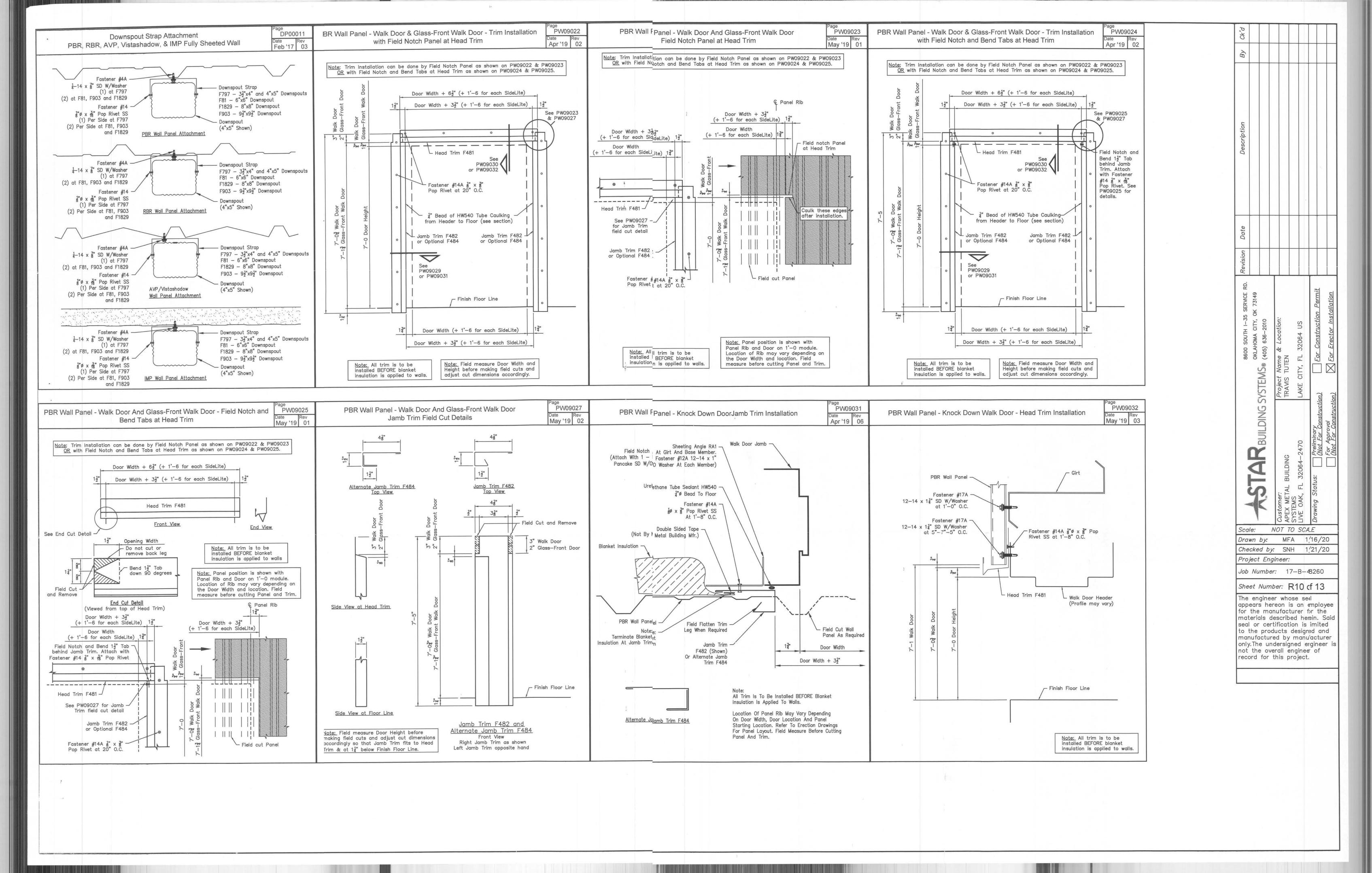
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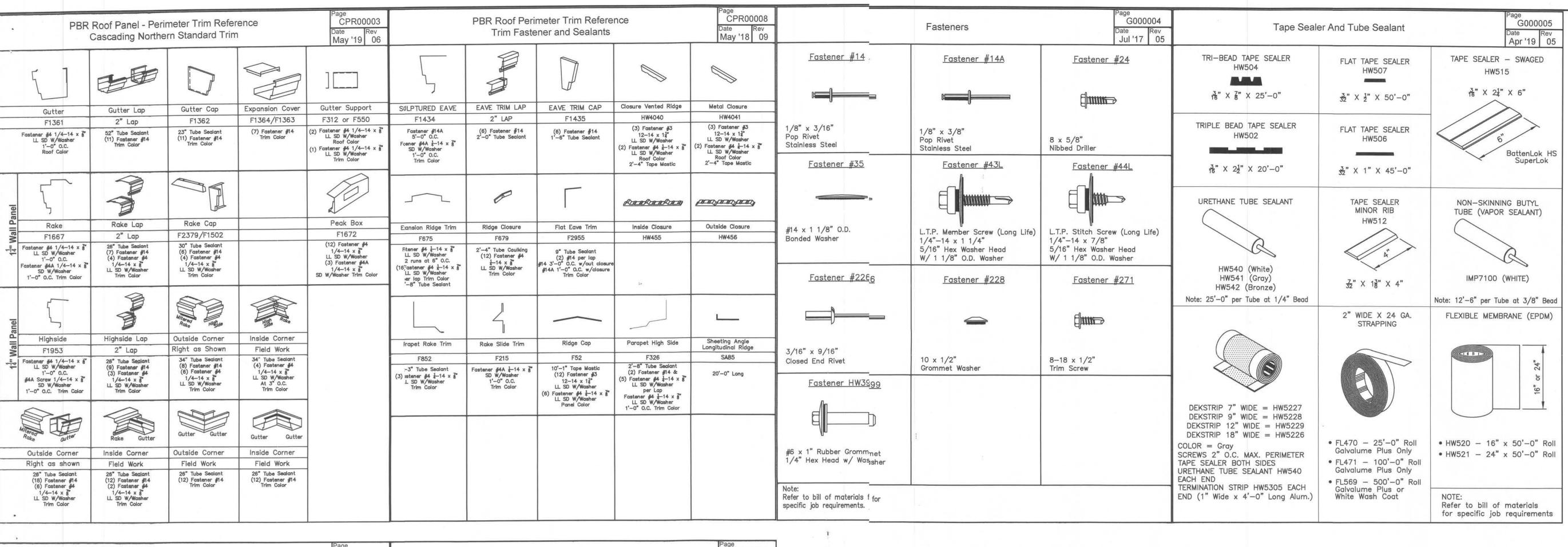
The engineer whose seal





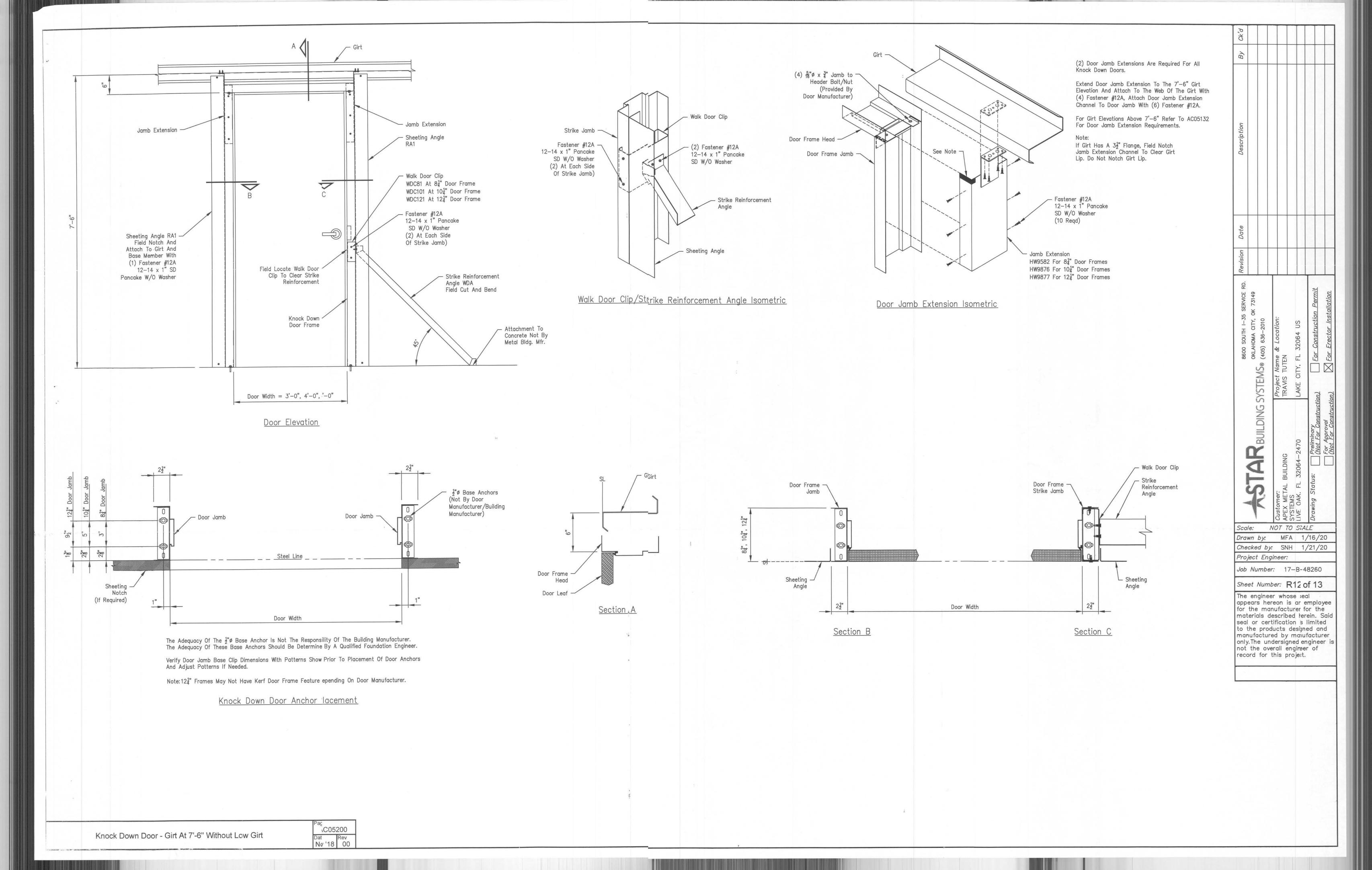


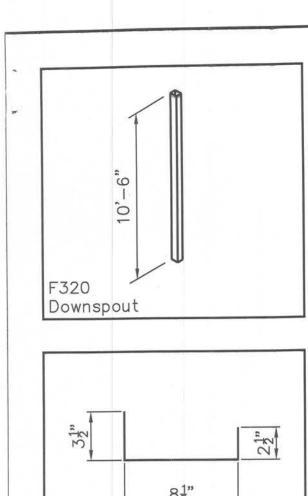


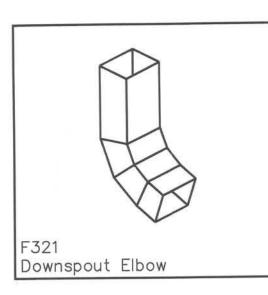


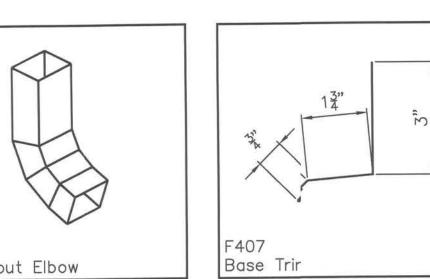
	VP, Vistashadow, RBR, RBU Panel Fasteners	Page G000006 Date Rev May '19 09	,	√arious Fasteners	G000009 Date Rev Nov '16 11
Wall Fasteners Member Screw Fastener #17A 12-14 x 1½" 15" Hex Washer Head w/washer	Roof Fasteners Long Life (Optional at Wall) Member Screw Fastener #3 12-14 x 1‡" 15" Hex Washer Head w/washer		Fastener #17 12-14 x 1" SD W/Washer 5" Hex Head	Fastener #38 1-14 x 7 SD W/O Washer 7 Hex Head	
Member Screw Optional Fastener #17B 12-14 x 1½" 5" Hex Washer Head w/washer Member Screw Optional Fastener #28 12-14 x 2" 5" Hex Washer Head	Member Screw Optional Fastener #3A 12-14 x 1½" The Washer Head W/washer Member Screw Optional Fastener #58 12-14 x 2" The Washer Head		Fitener #55 1:-24 x 1½" SD DP5 W/O Washer ½ Hex Head	Fastener #12A 12-14 x 1" Pancake SD W/O Washer Fastener #70 12-24 x 1½" SD DP5 W/O Washer 5" Hex Head	Fastener #142 1-14 x 1½" SD W/O Washer 56" Hex Head
w/washer	w/washer		Fastener #76 12-14 x 2" SD W/O Washer 56" Hex Head	Fastener #61 12-14 x 1½" SD W/O Washer ½" Hex Head	Fastener #1B 1-14 x 11" SD W/O Washer 5" Hex Head
Stitch Screw Fastener #4A \$\frac{1}{4}-14 \times \frac{7}{8}" #### Hex Washer Head w/washer	Stitch Screw Fastener #4 1—14 x 7° 7° Hex Washer Head w/washer				
	NOTE: Refer to bill of materials for specific job requirements		Fastener #16 12-24 x 1½" Pancake SD DP5 W/O Washer	NOTE: Refer to Bill of Materials for Specific job Requirements	Fastener #46 1-14 x 8" LL ST Type B W/Washer 5" Hex Head

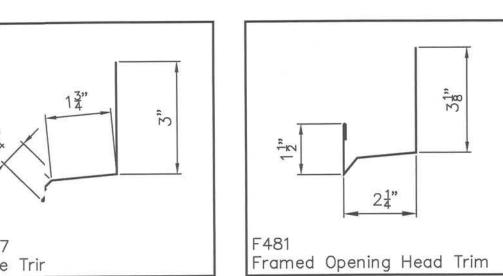
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8600 SOUTH 1-35 SERVICE RD.	OKLAHOMA CITY, OK 73149	DOILDING 3131 LND® (405) 636-2010	Customer:	AL BUILDING		LIVE OAK, FL 32064-2470 LAKE CITY, FL 32064 US	Drawing Status:	(Not For Construction)	Not For Construction)
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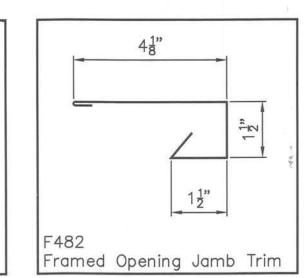


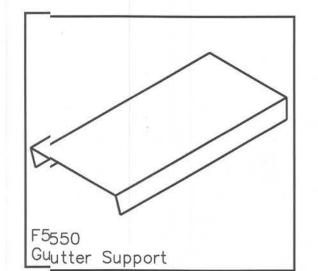


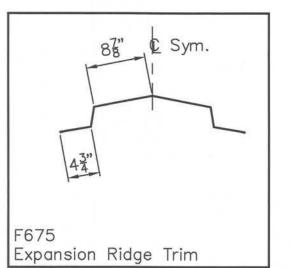


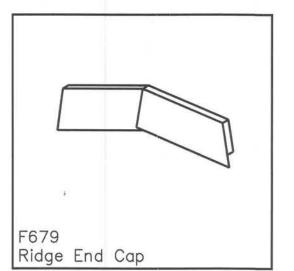


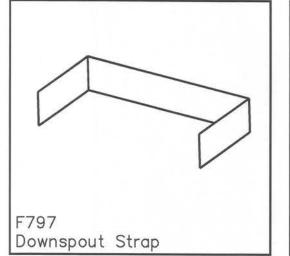


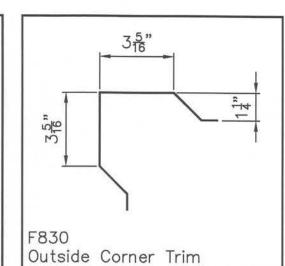


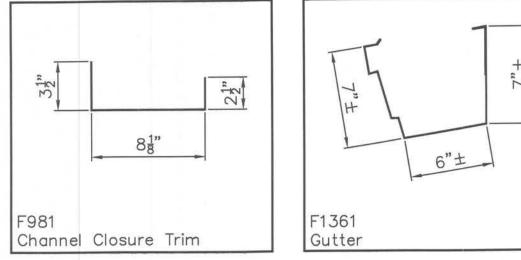


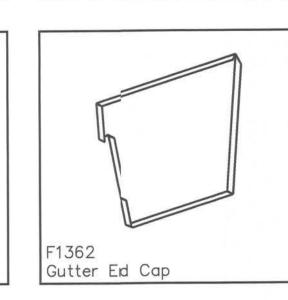


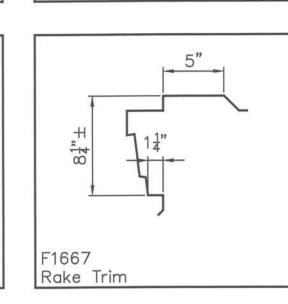


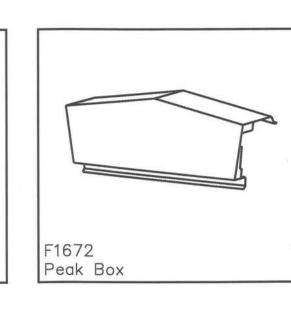


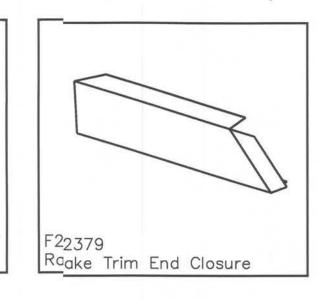












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35"	By	
3,55		
830 Outside Corner Trim	Description	
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		LIVIJ® (405) 6
Customer:	Pro	Project Name & L
APEX METAL BUILDING	TRA	TRAVIS TUTEN
SYSTEMS		
LIVE OAK, FL 32064-2470		LAKE CITY, FL 320
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Scale: NOT TO SCALE

Project Engineer:

Drawn by: MFA 1/16/20 Checked by: SNH 1/21/20

Job Number: 17-B-48260

Sheet Number: R13 of 13

The engineer whose seal appears hereon is an employee for the manufacturer or the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project