STRUCTURAL NOTES

A. GENERAL -----

- 1. NO PROVISION OF ANY REFERENCED STANDARD SPECIFICATION, MANUAL OR CODE (WHETHER OR NOT SPECIFICALLY INCORPORATED BY REFERENCE IN THE CONTRACT DOCUMENTS) SHALL BE EFFECTIVE TO CHANGE THE DUTIES AND RESPONSIBILITIES OF OWNER, CONTRACTOR, ENGINEER, SUPPLIER, OR ANY OF THEIR CONSULTANTS, AGENTS, OR EMPLOYEES FROM THOSE SET FORTH IN THE CONTRACT DOCUMENTS. NOR SHALL IT BE EFFECTIVE TO ASSIGN TO THE STRUCTURAL ENGINEER OR ANY OF THE STRUCTURAL ENGINEER'S CONSULTANTS, AGENTS, OR EMPLOYEES ANY DUTY OR AUTHORITY TO SUPERVISE OR DIRECT THE FURNISHING OR PERFORMANCE OF THE WORK OR ANY DUTY OR AUTHORITY TO UNDERTAKE RESPONSIBILITIES CONTRARY TO THE PROVISIONS OF THE CONTRACT DOCUMENTS.
- 2. CONTRACT DOCUMENTS INCLUDE, BUT ARE NOT LIMITED TO, THE STRUCTURAL DOCUMENTS (DRAWINGS AND SPECIFICATIONS), BUT DO NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR MATERIAL PREPARED AND SUBMITTED BY THE CONTRACTOR.
- 3. REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION OR ASSOCIATION OR TO CODES OF LOCAL OR STATE AUTHORITIES. SHALL MEAN THE LATEST STANDARD, CODE, SPECIFICATION OR TENTATIVE SPECIFICATION ADOPTED AT THE DATE OF TAKING BIDS, UNLESS SPECIFICALLY STATED OTHERWISE.
- 4. CONTRACT DOCUMENTS SHALL GOVERN IN THE EVENT OF A CONFLICT WITH THE CODE OF PRACTICE OR SPECIFICATIONS OF ACI, PCI, AISC, SJI OR OTHER STANDARDS. WHERE A CONFLICT OCCURS WITHIN THE CONTRACT DOCUMENTS, THE STRICTEST REQUIREMENT SHALL GOVERN.
- 5. MATERIAL, WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE REFERENCED BUILDING
- 6. CONTRACTOR SHALL COORDINATE THE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL. MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DOCUMENTS. ARCHITECT/STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS SEE THE ARCHITECTURAL DRAWINGS.
- 7. CONTRACTOR SHALL OBTAIN AND COORDINATE EDGE OF SLAB DIMENSIONS, OPENING LOCATIONS AND DIMENSIONS, DEPRESSED SLAB LOCATIONS AND EXTENTS, SLAB SLOPES, CURB LOCATIONS, AND CMU WALL LOCATIONS. ARCHITECT/STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION.
- 8. CONTRACTOR SHALL VERIFY EXISTING DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS BEFORE STARTING WORK. ARCHITECT/STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY.
- 9. CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, SAFETY, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION. CONTRACTOR HAS SOLE RESPONSIBILITY

TO COMPLY WITH ALL OSHA REGULATIONS.

- 10. THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED FORM. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY THE CONTRACTOR. CONTRACTOR IS RESPONSIBLE FOR CONSTRUCTABILITY ANALYSIS, AND ERECTION PROCEDURES, INCLUDING DESIGN AND ERECTION OF FALSEWORK, TEMPORARY BRACING, ETC.
- 11. REPRODUCTION OF STRUCTURAL DRAWINGS FOR SHOP DRAWINGS IS NOT PERMITTED.
- 12. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE STRUCTURAL ELEMENTS AND CONNECTIONS SHOWN IN THE CONTRACT DOCUMENTS. REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS REGARDING ARRANGEMENT AND SIZES OF MEMBERS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGN LOADS AND CONTRACT DOCUMENT DETAILS. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE ARCHITECT , STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW AND CHECK ALL SUBMITTALS AND SHOP DRAWINGS BEFORE SUBMITTING TO THE STRUCTURAL ENGINEER. REVIEW OF SUBMITTALS OR SHOP DRAWINGS BY THE ARCHITECT STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS. CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE
- 13. WHERE A SECTION OR DETAIL IS SHOWN OR DETAILED FOR ONE CONDITION. IT SHALL APPLY TO ALL SIMILAR AND LIKE CONDITIONS. DETAILS LABELED "TYPICAL" OR "TYP." ON THE STRUCTURAL DRAWINGS APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR. THE CONTRACTOR SHALL CONSIDER ALL OF THE CONTRACT DOCUMENTS IN DETERMINING SIMILAR AND LIKE CONDITIONS.
- 14. USE ONLY DIMENSIONS INDICATED ON THE CONTRACT DOCUMENTS. DO NOT SCALE DRAWINGS OR MEASURE OBJECTS IN ELECTRONIC FILES. NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES.
- 15. THE OWNER SHALL ESTABLISH A PERIODIC MAINTENANCE PROGRAM TO PROTECT THE STRUCTURE FROM DETERIORATION. THE MAINTENANCE PROGRAM IS THE RESPONSIBILITY OF THE OWNER AND SHOULD INCLUDE, BUT IS NOT LIMITED TO, THE FOLLOWING:

PAINTING OF EXPOSED STEEL THAT IS NOT GALVANIZED. INSPECTION AND MAINTENANCE OF PROTECTIVE COATINGS, SEALANTS, CAULKED JOINTS, AND CONTROL JOINTS.

REPAIR OF SPALLS AND CRACKS IN CONCRETE ELEMENTS. REPAIR AND RESTORATION OF CORRODED ELEMENTS. CLEANOUT OF DRAINS INCLUDING ALL TRENCH DRAINS AND SCUPPERS/GUTTERS

16. IN THE EVENT THAT CAD FILES OR BIM FILES ARE PROVIDED TO THE CONTRACTOR, NO WARRANTIES, EXPRESSED OR IMPLIED, ARE MADE WITH RESPECT TO THIS FORM OF THE STRUCTURAL DRAWINGS, INCLUDING ANY IMPLIED WARRANTIES OF MERCHANTABILITY OR FITNESS FOR A PARTICULAR PURPOSE. IT IS UNDERSTOOD THAT THE USER MAKES USE OF THIS FORM OF THE STRUCTURAL DRAWINGS AT THE USER'S SOLE RISK AND THAT THE CAD OR BIM FILES ARE PROVIDED "AS IS" WITHOUT WARRANTIES OF ANY KIND. THE STRUCTURAL ENGINEER HAS NO OBLIGATION TO OR THROUGH THE USER FOR USE OF THIS FORM OF THE DRAWINGS, INCLUDING ANY OBLIGATION OR LIABILITY FOR THE ACCURACY OF THE INFORMATION FURNISHED VIA THE ELECTRONIC FORM. IN ADDITION TO AND NOTWITHSTANDING THE FOREGOING, IN NO EVENT SHALL THE STRUCTURAL ENGINEER BE LIABLE FOR ANY CONSEQUENTIAL OR SPECIAL DAMAGES OR FOR ANY LOSS OF PROFITS SUSTAINED BY USER IN CONNECTION WITH OR ARISING OUT OF THE USE OF CAD OR BIM FORMS OF THE DRAWINGS. IN ALL CASES, THE HARD COPY OF THE STRUCTURAL DRAWINGS

B. CODE/DESIGN CRITERIA -----

- 1. STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE, SEVENTH EDITION (2020)
- 2. GRAVITY LOADS
- 2.1 UNIFORM FLOOR LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):

STORAGE / SHOP

GENERAL AREAS 250 PSF

2.2 UNIFORM ROOF LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):

AND SPECIFICATIONS SUPERSEDES ANY CAD OR BIM FILES.

R00F RAIN INTENSITY, i 4.06 IN/HR

2.3 CONCENTRATED FLOOR LOADS - DISTRIBUTED OVER AN AREA OF 2.5 FEET SQUARE, UNLESS NOTED OTHERWISE: 2000 LB

2.4 DEAD LOADS (IN ADDITION TO STRUCTURE SELF-WEIGHT)

ROOF: COLLATERAL LOAD 13 PSF (MECH. AND OTHER)

3. WIND LOADS: BASIC DESIGN WIND SPEED, V ALLOWABLE STRESS DESIGN WIND SPEED, Vasd 91 MPH **EXPOSURE** RISK CATEGORY

INTERNAL PRESSURE COEFFICIENT, GCpi +/-0.18 4. NO PROVISIONS HAVE BEEN MADE FOR FUTURE HORIZONTAL OR VERTICAL EXPANSION.

C. DEFERRED STRUCTURAL SUBMITTALS

- 1. DEFERRED SUBMITTALS, AS DEFINED BY THE BUILDING CODE, SHALL BE SUBMITTED TO THE BUILDING OFFICIAL BY THE CONTRACTOR. THE DEFERRED SUBMITTALS SHALL BE SIGNED AND SEALED BY A LICENSED ENGINEER IN THE PROJECT STATE.
- 2. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR THE DESIGN OF THE DEFERRED SUBMITTAL COMPONENTS OR THE CONNECTION TO THE STRUCTURE. THE STRUCTURAL DESIGN OF THE COMPONENTS AND THE CONNECTION TO THE STRUCTURE IS DELEGATED TO A SPECIALTY ENGINEER WHO SHALL BE ENGAGED BY THE CONTRACTOR, VENDOR, AND / OR SUPPLIER OF THE COMPONENTS AS PART OF THE DEFERRED SUBMITTAL PROCESS.
- 3. THE CONTRACTOR SHALL SUBMIT THE DEFERRED SUBMITTAL TO THE ARCHITECT STRUCTURAL ENGINEER FOR REVIEW. AFTER REVIEW BY THE ARCHITECT / STRUCTURAL ENGINEER THE CONTRACTOR SHALL SUBMIT THE REVIEWED SUBMITTAL TO THE BUILDING OFFICIAL PER SECTION 107.3 OF THE BUILDING CODE.
- 4. THE ITEMS LISTED BELOW ARE IDENTIFIED AS DEFERRED SUBMITTALS. REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND CIVIL DRAWINGS AND SPECIFICATIONS FOR ADDITIONAL DEFERRED SUBMITTAL COMPONENTS. ALL COSTS ASSOCIATED WITH THE PREPARATION OF THE DEFERRED SUBMITTAL. INCLUDING THE SPECIALTY ENGINEERS DESIGN FEES, SHALL BE THE RESPONSIBILITY OF THE

GROUND IMPROVEMENT

CONTRACTOR

- SHORING AND UNDERPINNING TEMPORARY BRACING / SHORING FOR THE STABILITY OF THE STRUCTURE DURING CONSTRUCTION CONCRETE FORMWORK
- STRUCTURAL STEEL CONNECTIONS AS DEFINED IN STRUCTURAL NOTE SECTION 'H' HANDRAILS AND GUARDRAILS
- EXTERIOR BUILDING SIGNAGE ANCHORAGE OF EXTERIOR ARCHITECTURAL, MECH., ELEC., AND PLUMBING EQUIPMENT ATTACHMENT OF ROOFTOP EQUIPMENT, PIPING, & DUCTWORK TO THE UNDERLYING STRUCTURE PREFABRICATED / PRE-ENGINEERED ARCHITECTURAL ELEMENTS AND STRUCTURES
- 5. DEFERRED SUBMITTAL COMPONENTS SHALL BE DESIGNED FOR THE LOADS AS DEFINED BY THE APPLICABLE BUILDING CODE WITH DESIGN DATA DEFINED IN THE SECTION B OF THE STRUCTURAL NOTES.

D. FOUNDATION

- FOUNDATION DESIGN IS BASED ON THE RECOMMENDATIONS FROM INTERTEK/PSI. _GEOTECHNICAL REPORT NUMBER 07572726 DATED DECEMBER 7, 2021. STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT FROM THOSE ASSUMED FOR DESIGN.
- STRUCTURAL TESTING/INSPECTION AGENCY SHALL CERTIFY THE BEARING MEDIUM. 3. INDIVIDUAL SPREAD FOOTINGS AND CONTINUOUS FOOTINGS SHALL BEAR ON SOIL CAPABLE
- OF SUPPORTING 3,000 PSF. 3.1 RETAINING WALL FOOTINGS FOR WALLS GREATER THAN 15-FEET IN HEIGHT SHALL
- BE BASED ON THE AVERAGE BEARING PRESSURE UNDER THE FOOTING WIDTH AND SHALL BEAR ON SOIL CAPABLE OF SUPPORTING 4,000 PSF THAT IS ALLOWED TO CONSOLIDATE BEFORE BUILDING CONSTRUCTION.
- 4. FINAL FILL HEIGHTS EXCEEDING 10-FEET FROM EXISTING GRADES SHALL BE PLACED TO FULL HEIGHT AND LEFT TO SIT FOR A MINIMUM OF 30 DAYS PRIOR TO BUILDING OR UTILITY CONSTRUCTION.
- 4.1 LOCATE SURVEY POINTS AT APPROXIMATELY 50-FEET APART ALONG THE FULL

LENGTH ON TOP OF THE RETAINING WALL TO BE USED FOR MONITORING POINTS/

- 4.2 WALL ELEVATION MEASUREMENTS AT SURVEY POINTS SHOULD BE TAKEN WEEKLY DURING FILL ACTIVITIES AND THE CONSOLIDATION PAUSE.
- 4.3 REPORT ELEVATION READINGS TO STRUCTURAL ENGINEER ON A REGULAR BASIS
- 5. FOUNDATION WALLS ARE DESIGNED FOR LATERAL PRESSURES DUE TO THE FOLLOWING **EQUIVALENT FLUID DENSITIES:**

WALLS SUPPORTED AT TOP (AT-REST CONDITION): 60 PCF WALLS FREE TO DISPLACE AT TOP (ACTIVE CONDITION): 40 PCF

6. PROOFROLL BUILDING AREAS WITH TWO COMPLETE COVERAGES OF A LOADED DUMP-TRUCK OR SCRAPER. REPLACE SOFT AREAS WITH COMPACTED STRUCTURAL FILL AS REQUIRED BY THE SPECIFICATIONS.

E. REINFORCEMENT

LAPS OF 8".

- 1. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE. 2. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064 AND HAVE MINIMUM SIDE AND END
- 3. SUBMIT SHOP DRAWINGS WHICH ADEQUATELY DEPICT THE REINFORCING BAR SIZES AND PLACEMENT. WRITTEN DESCRIPTION OF REINFORCEMENT WITHOUT ADEQUATE SECTIONS,
- ELEVATIONS, AND DETAILS IS NOT ACCEPTABLE. 4. PROVIDE DOWELS FROM FOUNDATIONS THE SAME SIZE AND NUMBER AS THE VERTICAL WALL OR COLUMN REINFORCING, UNLESS NOTED OTHERWISE.
- 5. PLACE REINFORCEMENT AS FOLLOWS, UNLESS NOTED OTHERWISE:
- 5.1 CAST-IN-PLACE CONCRETE REINFORCEMENT COVER

PERMANENTLY EXPOSED TO EARTH: 3" CLEAR CAST AGAINST THE EARTH

EXPOSED TO EARTH OR WEATHER:

FOR BARS LARGER THAN A NO. 5 BAR 2" CLEAR NO. 5 BARS OR SMALLER 1-1/2" CLEAR

5.2 MASONRY CONCRETE REINFORCEMENT COVER

SHALL BE AS FOLLOWS, UNLESS NOTED OTHERWISE:

MASONRY REINFORCING STEEL SHALL BE PLACED IN THE CENTER OF CMU CELLS, UNLESS NOTED OTHERWISE. IF REINFORCING STEEL IS NOTED TO BE PLACED OTHER THAN THE CENTER OF THE CELL, PLACE BAR TO ACHIEVE 2" CLEAR COVER BETWEEN REINFORCING AND OUTSIDE FACE OF CMU BLOCK.

6. REINFORCEMENT SHALL BE SPLICED ONLY AT LOCATIONS SHOWN OR NOTED IN THE STRUCTURAL DOCUMENTS, EXCEPT REINFORCEMENT MARKED "CONTINUOUS" CAN BE SPLICED AT LOCATIONS DETERMINED BY CONTRACTOR. SPLICES AT OTHER LOCATIONS SHALL BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER. REINFORCING STEEL SPLICES

CONCRETE REINFORCEMENT: MASONRY REINFORCEMENT:

CLASS B TENSION LAP SEE REINFORCING LAP LENGTH SCHEDULE

F. CAST-IN-PLACE CONCRETE

- 1. CONCRETE WORK SHALL CONFORM TO ACI 318 AND CRSI STANDARDS
- NORMAL WEIGHT STRUCTURAL CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES:

	EXPOSURE CLASS	COMPRESSIVE STRENGTH, F'C	W/C RATIO	AGGREGAT SIZE	
FOOTINGS	C1	3,000 PSI	0.55	1 1/2"	
GRADE BEAMS	C1	3,000 PSI	0.55	1"	
PEDESTALS	C1	4,500 PSI	0.50	1"	
FOUNDATION WALLS	C1	4,500 PSI	0.50	1"	
INTERIOR SLABS-ON-GRADE	C1	4,000 PSI	0.48	1"	
EXTERIOR SLABS-ON-GRADE	C1	4,500 PSI	0.45	1"	

ALL NORMAL WEIGHT CONCRETE SHALL BE CONSIDERED TO BE IN EXPOSURE CLASS FO, SO, WO, AND CO ACCORDING TO ACI 318 UNLESS NOTED OTHERWISE ABOVE OR ELSEWHERE ON THE STRUCTURAL DRAWINGS

CONCRETE USED FOR SLAB-ON-GRADE APPLICATIONS SHALL ALSO HAVE A MINIMUM MODULUS OF RUPTURE OF 600 PSI

F. CAST-IN-PLACE CONCRETE (CONT'D.)

- 3. CONCRETE MIX REQUIREMENTS
- 3.1 ALL CONCRETE SHALL BE PROPORTIONED TO COMPLY WITH ACI 318 CHAPTER 19 IN ACCORDANCE WITH THE EXPOSURE CLASS INDICATED. WHERE REQUIREMENTS INDICATED DIFFER FROM REQUIREMENTS OF CHAPTER 19, THE STRICTER REQUIREMENT SHALL APPLY. REFER TO THE SPECIFICATIONS FOR OTHER REQUIREMENTS FOR VARIOUS EXPOSURE CLASSES RELATIVE TO THE CEMENT TYPE, AIR ENTRAINMENT REQUIREMENTS, CHLORIDE ION LIMITS, POZZOLAN LIMITS, AND SHRINKAGE LIMITS.
- 3.2 CONCRETE SHALL BE CONSIDERED EXTERIOR CONCRETE IF THE CONCRETE IS PERMANENTLY EXPOSED TO THE WEATHER OR MOISTURE OR IF IT IS IN AN UNCONDITIONED SPACE IN ITS COMPLETED CONFIGURATION.
- 3.3 ALL CONCRETE SHALL SATISFY BOTH THE SPECIFIED MAXIMUM WATER TO CEMENT RATIO AND THE MINIMUM COMPRESSIVE STRENGTH, F'C, REQUIREMENTS.
- SPECIFICALLY DETAILED. SEE ARCHITECTURAL, CIVIL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION OF SLEEVES, ACCESSORIES, ETC.

4. PIPES OR DUCTS SHALL NOT EXCEED ONE-THIRD THE SLAB OR WALL THICKNESS UNLESS

- 5. REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ORNAMENTS, CLIPS OR GROUNDS REQUIRED TO BE ENCASED IN CONCRETE AND FOR LOCATION OF ALL CONCRETE FINISHES AND SLAB DEPRESSIONS.
- 6. CONSTRUCTION JOINT LOCATIONS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER. NO HORIZONTAL CONSTRUCTION JOINTS ARE PERMITTED EXCEPT THOSE SHOWN ON THE STRUCTURAL DRAWINGS.
- 7. DEFECTIVE AREAS IN CONCRETE INCLUDING, BUT NOT LIMITED TO, HONEY-COMBING, SPALLS, AND CRACKS WITH WIDTHS EXCEEDING 0.01 INCH SHALL BE REPAIRED. EXTENT OF DEFECTIVE AREA TO BE DETERMINED BY THE STRUCTURAL ENGINEER.
- G. CONCRETE MASONRY
- CONCRETE MASONRY WORK SHALL CONFORM TO TMS 402/602, BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES.
- 2. MINIMUM COMPRESSIVE STRENGTH OF CONCRETE MASONRY SHALL BE F'M = 2,000 PSI.
- MORTAR SHALL BE TYPE S AND SHALL COMPLY WITH THE BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY.
- CONCRETE MASONRY UNITS SHALL BE GROUTED WITH 2,500 PSI COARSE GROUT AS SHOWN IN THE STRUCTURAL DOCUMENTS. GROUT SHALL CONFORM TO ASTM C476.
- PROVIDE HORIZONTAL JOINT REINFORCEMENT WITH NO. 9 GAGE LONGITUDINAL WIRES AT 16" VERTICALLY, UNLESS NOTED OTHERWISE. PROVIDE SPECIAL ACCESSORIES FOR CORNERS, INTERSECTIONS, ETC.
- PROVIDE OPEN BOTTOM BEAM BLOCK UNITS WITH 3" DEEP MINIMUM WEB OPENINGS AT HORIZONTAL REINFORCEMENT LOCATIONS. A MINIMUM CLEAR SPACE OF ONE BAR DIAMETER SHALL BE PROVIDED BETWEEN THE REINFORCING BARS AND THE FACE OF MASONRY UNITS.
- PROVIDE CONTRACTION (CONTROL) JOINTS IN ALL CONCRETE MASONRY WALLS AT LOCATIONS APPROVED BY THE ARCHITECT AT A MAXIMUM SPACING OF 1.5 TIMES THE WALL HEIGHT. THE DISTANCE TO THE FIRST CONTRACTION JOINT FROM CORNERS OR
- INTERSECTING WALLS IS LIMITED TO HALF OF THE ABOVE VALUE 8. SUBMIT WRITTEN CONSTRUCTION PROCEDURES PRIOR TO THE START OF MASONRY

L. STRUCTURAL STEEL

- 1. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED ACCORDING TO THE ANSI/AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND THE AISC 303 "CODE OF
- 2. STRUCTURAL STEEL SHALL BE OF THE FOLLOWING GRADE UNLESS NOTED OTHERWISE ON DRAWINGS:

STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".

ASTM A992, GRADE 50 W AND WT-SHAPES SQUARE/RECTANGULAR HSS ASTM A500, GRADE C OR ASTM A1085 OTHER STEEL SHAPES (CHANNELS, ASTM A36

3. BOLTS, ANCHOR RODS, AND HEADED STUDS:

ANGLES, PLATES, AND ROUND ROD)

- 3.1 ALL BOLTS SHALL BE GROUP A OR GROUP B HIGH STRENGTH BOLTS WITH A 3/4" MINIMUM DIAMETER. ALL CONNECTIONS MAY BE BEARING TYPE. BOLT SHEAR STRENGTH SHALL BE DETERMINED IN ACCORDANCE WITH TABLES 7-1 OR 7-3 AS APPLICABLE IN THE AISC "STEEL CONSTRUCTION MANUAL".
- 3.2 ANCHOR RODS SHALL CONFORM TO ASTM F1554, GRADE 36, UNLESS NOTED OTHERWISE.
- 4. CONNECTIONS:
- 4.1 STEEL CONNECTIONS SHALL BE DETAILED BASED ON THE DESIGN INFORMATION PROVIDED IN THE CONTRACT DOCUMENTS. DEVIATION FROM THE CONNECTION DETAILS DEPICTED IN THE CONTRACT DOCUMENTS SHALL NOT BE PERMITTED WITHOUT ADVANCE

WRITTEN PERMISSION FROM THE STRUCTURAL ENGINEER.

- 4.2 THE SERVICES OF A CONNECTION DESIGN SPECIALTY ENGINEER (CDSE) SHALL BE INCLUDED IN THE CONTRACTOR'S SCOPE OF SERVICES. THE CDSE SHALL BE LICENSED IN THE PROJECT STATE. THE CDSE IS RESPONSIBLE FOR REVIEWING THE STEEL SHOP DRAWINGS TO ENSURE THAT ALL CONNECTION DESIGN DETAILS HAVE BEEN CORRECTLY SHOWN ON THE SHOP DRAWINGS; AND, THE CDSE SHALL SUBMIT A SIGNED AND SEALED LETTER, WITH EACH SHOP DRAWING SUBMITTAL, CONFIRMING
- THE ABOVE REVIEW. 4.3 STANDARD SHEAR CONNECTIONS:
- 4.3.1 STANDARD SHEAR CONNECTIONS SHALL BE DETAILED AS DOUBLE-ANGLE, SINGLE-PLATE, SINGLE-ANGLE, OR TEE CONNECTIONS IN ACCORDANCE WITH CONNECTION TABLES IN PART 10 OF THE "STEEL CONSTRUCTION MANUAL". THE CDSE SHALL SUBMIT SIGNED AND SEALED CALCULATIONS FOR ALL SHEAR CONNECTIONS THAT CANNOT BE DETAILED COMPLETELY FROM THE ABOVE MANUAL FOR REVIEW BY THE STRUCTURAL ENGINEER.
- 4.3.2 CONNECTIONS SHALL BE DETAILED TO CARRY A MINIMUM AXIAL LOAD OF 0.05 X THE REQUIRED SHEAR DESIGN LOAD FOR THE CONNECTION
- 4.4 FOR WELDED CONNECTIONS, USE PREQUALIFIED WELDED JOINTS IN ACCORDANCE WITH AISC AND THE STRUCTURAL WELDING CODE OF THE AMERICAN WELDING SOCIETY. "NON-PREQUALIFIED JOINTS" SHALL BE QUALIFIED PRIOR TO FABRICATION.
- 4.5 THE REQUIRED REACTION USING LRFD LOAD COMBINATIONS SHALL BE AS SHOWN ON THE STRUCTURAL DRAWINGS. IF REACTIONS ARE NOT SHOWN: FOR BEAMS W10 AND SMALLER: 12 KIP SHEAR CAPACITY FOR BEAMS W12 AND LARGER: CONTACT THE STRUCTURAL ENGINEER
- 4.6 REVIEW OF THE SHOP DRAWINGS AND/OR CONNECTION CALCULATIONS BY THE STRUCTURAL ENGINEER DOES NOT RELIEVE THE CONTRACTOR AND CDSE OF THE FULL RESPONSIBILITY FOR THE DESIGN AND ADEQUACY OF SUCH CONNECTIONS.
- 5. THE STRUCTURAL STEEL MEMBERS HAVE NOT BEEN DESIGNED TO ACCOMMODATE THE TORSION RESULTING FROM THE ECCENTRIC LOADING OF THE LIGHT GAUGE METAL FRAMING, ETC. SUPPLEMENTAL SECONDARY BRACING SHALL BE DESIGNED AND PROVIDED BY THE SUPPLIER TO ELIMINATE TORSION.
- 6. ALL STEEL EXPOSED TO WEATHER OR MOISTURE SHALL BE GALVANIZED, UNLESS OTHERWISE DIRECTED BY THE ARCHITECT

L. STRUCTURAL STEEL (CONT'D.)

- 7. THE LATERAL LOAD RESISTING SYSTEM INCLUDES STRUCTURAL STEEL, NON-STRUCTURAL STEEL ELEMENTS. AND THE DIAPHRAGM AS INDICATED BELOW. ALL ELEMENTS OF THE LATERAL LOAD RESISTING SYSTEM AND DIAPHRAGM ARE REQUIRED TO BE COMPLETE AS INDICATED AND DETAILED IN THE STRUCTURAL CONTRACT DOCUMENTS TO PROVIDE THE LATERAL STRENGTH AND STABILITY OF THE STEEL STRUCTURE. THE STRUCTURE SHALL BE
- 7.1 THE LATERAL LOAD RESISTING SYSTEM FOR THE STEEL STRUCTURE INCLUDES THE FOLLOWING ELEMENTS AS INDICATED AND DETAILED IN THE STRUCTURAL CONTRACT CANTILEVER COLUMNS

THE FOLLOWING ELEMENTS AS INDICATED AND DETAILED IN THE STRUCTURAL

CONNECTIONS, BASEPLATES, ANCHOR RODS (BOLTS), AND GROUT 7.2 THE LATERAL LOAD RESISTING DIAPHRAGM FOR THE STEEL STRUCTURE INCLUDES

CONSIDERED UNSTABLE UNTIL THESE SYSTEMS AND ELEMENTS ARE COMPLETE.

CONTRACT DOCUMENTS: STEEL ROOF DECK 8. STABILITY BRACING: THE STABILITY OF STRUCTURAL STEEL ELEMENTS INCLUDING INDIVIDUAL HOT-ROLLED STEEL SHAPES IS PROVIDED BY THE FOLLOWING ELEMENTS AS INDICATED AND DETAILED IN THE STRUCTURAL CONTRACT DOCUMENTS. THESE ELEMENTS

TEMPORARY MEANS AND METHODS REQUIRED FOR ERECTION ARE REMOVED.

SHALL BE COMPLETE AS SHOWN IN THE STRUCTURAL CONTRACT DOCUMENTS BEFORE ANY

I. STEEL DECK

STEEL ROOF DECK

- STEEL DECK SHALL BE PLACED OVER MULTIPLE SPANS WHEREVER POSSIBLE. WHERE SINGLE SPAN DECK IS REQUIRED. THE CONTRACTOR SHALL DRAW SPECIFIC ATTENTION TO THOSE LOCATIONS ON THE SHOP DRAWINGS.
- 2. SUBMIT SHOP DRAWINGS SHOWING THE STEEL DECK PROFILE, GAGE, PHYSICAL PROPERTIES AND LAYOUT. THE SUBMITTAL SHALL INCLUDE ALL ACCESSORIES AND INSTALLATION DETAILS. IF DECK OTHER THAN THE BASIS OF DESIGN IS PROVIDED, THE SUBMITTAL SHALL INCLUDE LOAD TABLES DEMONSTRATING THE DECK MEETS OR EXCEEDS THE BASIS OF DESIGN. THE LOAD TABLES SHALL BE IN ACCORDANCE WITH THE STEEL DECK INSTITUTE (SDI) REQUIREMENTS
- 3. ROOF DECK:
- 3.1 THE 1 1/2" TYPE B ROOF DECK BASIS OF DESIGN IS 1.5B ROOF DECK PRODUCED BY VULCRAFT (IAPMO UES ER-0652). OTHER DECK MANUFACTURERS ARE PERMITTED PROVIDED THE FOLLOWING MINIMUM DECK PROPERTIES ARE MET OR EXCEEDED:

GAGE	22	
YIELD STRESS	33	KSI
MOMENT OF INERTIA (POSITIVE BENDING), I(+)	0.155	IN^4/FT
MOMENT OF INERTIA (NEGATIVE BENDING), I(-)	0.178	IN^4/FT
SECTION MODULUS (POSITIVE MOMENT), S(+)	0.169	IN ³ /FT
SECTION MODULUS (NEGATIVE MOMENT), S(-)	0.179	IN ³ /FT

- 3.2 DECK FINISH SHALL BE GALVANIZED G90.
- 3.3 PROVIDE VENTED DECK IN ACCORDANCE WITH SECTION 1917 OF THE FLORIDA BUILDING CODE WHEN USED WITH LIGHTWEIGHT INSULATING CONCRETE ROOFING.

J. METAL BUILDING SYSTEM -----

STRUCTURAL MEMBERS".

- 1. LIGHT-GAGE, COLD FORMED STRUCTURAL MEMBERS AND PANELS SHALL BE DESIGNED IN ACCORDANCE WITH THE AISI "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL
- STRUCTURAL STEEL SECTIONS AND WELDED PLATE MEMBERS SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS: ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN" OR THE AISC "LOAD AND RESISTANCE FACTOR DESIGN SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS"; AND THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
- METAL BUILDING SYSTEM SHALL BE DESIGNED FOR THE LIVE AND WIND LOADS AS PRESCRIBED BY THE BUILDING CODE GIVEN IN THE CODE / DESIGN CRITERIA SECTION ABOVE AND THE LOADS LISTED BELOW. LOAD COMBINATIONS SHALL BE IN ACCORDANCE WITH THE BUILDING CODE AND THE MBMA "LOW RISE BUILDING SYSTEMS
 - 3.1 DEAD LOADS SHALL CONSIST OF THE FOLLOWING:
 - 3.1.1 WEIGHT OF STRUCTURAL FRAME AND ALL OTHER MATERIALS OF THE BUILDING
 - 3.1.2 COLLATERAL DEAD LOAD (NOT TO BE INCLUDED IN LOAD COMBINATIONS
 - INVOLVING WIND UPLIFT) OF 8 PSF. 3.1.3 SUSPENDED ITEMS SUCH AS MECHANICAL EQUIPMENT, PLUMBING, FOLDING

PARTITIONS, ETC. IDENTIFIED IN THE CONTRACT DOCUMENTS.

- DESIGN OF METAL BUILDING SYSTEM SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. SUBMIT SHOP DRAWINGS SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE. REVIEW OF SHOP DRAWINGS SHALL BE FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS AND THE CONTRACTOR'S INTERPRETATION OF THE DESIGN LOADS AND CONTRACT DOCUMENT DETAILS. SUCH REVIEW SHALL NOT RELIEVE THE CONTRACTOR OF FULL
- DESIGN OF METAL BUILDING SYSTEM SHALL INCLUDE THE FOLLOWING:

RESPONSIBILITY FOR THE DESIGN OF THE METAL BUILDING SYSTEM.

- STEEL FRAMES LATERAL LOAD RESISTING SYSTEM (X-BRACING, DIAPHRAGM, ETC.) WIND COLUMNS COLUMN ANCHOR BOLTS (TYPE, NUMBER, DIAMETER, AND EMBEDMENT) ROOF PURLINS
- WALL GIRTS ROOF AND WALL METAL PANELS OPENING FRAMING (DOORS, ROOF VENTS, ETC.)

CANOPY FRAMING

WIND COLUMNS

- METAL BUILDING SYSTEM SHALL MEET THE SERVICEABILITY DEFLECTION AND DRIFT LIMITS AS GIVEN IN THE AISC "STEEL DESIGN GUIDE SERIES 3: SERVICEABILITY
- DESIGN CONSIDERATIONS FOR LOW RISE BUILDINGS", EXCEPT AS MODIFIED BELOW: 6.1 WIND FORCES SHALL BE BASED ON A MEAN RECURRANCE INTEVAL (M.R.I.) OF NO
- LESS THAN 10 YEARS FOR SERVICEABILITY DEFLECTION AND DRIFT LIMITS. 6.2 LATERAL DRIFT/DEFLECTION DUE TO WIND FORCES, SHALL BE LIMITED TO THE FOLLOWING (WHERE H IS THE BUILDING HEIGHT AND L IS THE SPAN LENGTH): **BUILDING FRAME**

H/360

H/360

L/240 WIND GIRTS 7. METAL BUILDING MANUFACTURER SHALL BE CERTIFIED BY THE AISC QUALITY

CERTIFICATION PROGRAM FOR METAL BUILDING SYSTEMS (MB).

8. FOOTINGS HAVE BEEN DESIGNED BASED UPON AN ASSUMED LATERAL LOAD RESISTING SYSTEM LAYOUT AND NO ROTATIONAL FIXITY AT THE BASE OF THE FRAMES (NO BASE COLUMN MOMENTS). METAL BUILDING SHOP DRAWINGS SHALL CLEARLY INDICATE FOUNDATION REACTIONS FOR CODE REQUIRED LOAD COMBINATIONS. UPON RECEIVING THE METAL BUILDING SHOP DRAWINGS, THE STRUCTURAL ENGINEER WILL VERIFY THE FOUNDATION REACTIONS ARE COMPATIBLE WITH THE FOOTING DESIGN AS SHOWN ON THE STRUCTURAL DRAWINGS. FOOTING CONSTRUCTION SHALL NOT BEGIN UNTIL THE STRUCTURAL

ENGINEER HAS RETURNED THE REVIEWED SHOP DRAWINGS AND INDICATED ANY NECESSARY

DRAWING INDEX S010 STRUCTURAL NOTES S011 STRUCTURAL NOTES (CONT.) S201C FOUNDATION PLAN - ANCILLARY - PART C S201D FOUNDATION AND FRAMING PLANS - ANCILLARY - PART D S310 FOUNDATION SECTIONS AND DETAILS S311 FOUNDATION SECTIONS AND DETAILS S320 TYPICAL MASONRY DETAILS S410 ROOF FRAMING SECTIONS AND DETAILS

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General Contractors

5500 Maryland Way, Suite 100 Brentwood, TN 37027

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LAKE CITY MEDICAL CENTER 340 NW Commerce Dr. Lake City, FL 32055

HCA Project No.: 3793800011

Revision		
No.	Date	Description
1	3/2/2022	Amendment 1-1

STRUCTURAL NOTES

STRUCTURAL NOTES CONT.

- I. POST-INSTALLED ANCHORS AND REINFORCING STEEL
- 1. POST-INSTALLED ANCHORS AND REINFORCING STEEL SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER PRIOR TO INSTALLING POST-INSTALLED ANCHORS OR REINFORCING STEEL IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS OR REINFORCING STEEL.
- 2. ANCHORS AND REINFORCING STEEL SHALL BE INSTALLED PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII)
- 3. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE SPECIFIED BELOW SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER ALONG WITH CALCULATIONS THAT ARE SIGNED AND SEALED BY A LICENSED PROFESSIONAL ENGINEER. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING EQUIVALENT PERFORMANCE VALUES (MINIMUM) OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S) AS REQUIRED BY THE BUILDING CODE.
- 4. THE CONTRACTOR SHALL ARRANGE ONSITE INSTALLATION TRAINING BY THE MANUFACTURER FOR EACH PRODUCT TO BE INSTALLED. SUBMIT TO THE STRUCTURAL ENGINEER DOCUMENTATION CONFIRMING TRAINING OF ALL PERSONNEL WHO WILL BE INSTALLING PRODUCTS. TRAINING AND DOCUMENTATION SHALL OCCUR PRIOR TO COMMENCEMENT OF PRODUCT INSTALLATION. INSTALLATION OF ADHESIVE ANCHOR PRODUCTS IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATION RESISTING SUSTAINED TENSION LOADS SHALL BE CONDUCTED BY AN INSTALLER CERTIFIED IN ACCORDANCE WITH THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM. PROOF OF CERTIFICATION SHALL BE MAINTAINED AT THE JOB SITE.
- 5. ANCHOR CAPACITY IS DEPENDENT UPON SPACING BETWEEN ADJACENT ANCHORS, CONCRETE STRENGTH AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS. IF NO SPACING OR EDGE DISTANCES ARE SPECIFIED ON THE STRUCTURAL DRAWINGS, REFER TO APPLICABLE EVALUATION REPORT FOR CRITICAL SPACING AND EDGE DISTANCES.
- 6. EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR OR REINFORCING LOCATIONS. THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TO LOCATE THE POSITION OF THE REINFORCING BARS AT THE LOCATIONS OF THE CONCRETE ANCHORS OR REINFORCING, BY FERROSCAN, GPR, X-RAY, CHIPPING OR OTHER MEANS IN ORDER TO AVOID CONFLICT WITH INSTALLATION. THE CONTRACTOR SHALL NOT DAMAGE ANY REINFORCING STEEL PRIOR TO CONSULTING WITH THE STRUCTURAL ENGINEER.
- 7. UNLESS NOTED OTHERWISE, THE MINIMUM EMBEDMENT OF EXPANSION AND SCREW ANCHORS SHALL BE 6 TIMES THE ANCHOR DIAMETER, THE MINIMUM EMBEDMENT FOR ADHESIVE ANCHORS SHALL BE 12 TIMES THE ANCHOR DIAMETER, AND THE MINIMUM EMBEDMENT FOR ADHESIVE REINFORCING SHALL BE 12 BAR DIAMETERS.
- 8. ADHESIVE ANCHOR INSERT SHALL BE:

AND ICC-ES AC193.

INTERIOR NON-CORROSIVE ENVIRONMENTS: ASTM F1554 Gr. 36 ALL-THREAD ROD EXTERIOR NON-CORROSIVE ENVIRONMENTS: ASTM F1554 Gr. 36 ALL-THREAD ROD HOT-DIPPED GALVANIZED PER ASTM B695, CLASS 65, TYPE I CORROSIVE ENVIRONMENTS: ASTM A193 GR. B8M TYPE 316 ALL-THREAD ROD

- 9. MECHANICAL AND SCREW ANCHORS IN EXTERIOR NON-CORROSIVE ENVIRONMENTS SHALL BE HOT-DIPPED GALVANIZED PER ASTM B695, CLASS 65, TYPE I
- 10. ADHESIVE ANCHORS AND REINFORCING STEEL CAPACITY IS DEPENDENT UPON INSTALLATION CONDITIONS. THE FOLLOWING INSTALLATION CONDITIONS HAVE BEEN ASSUMED:

HOLES DRILLED WITH HAMMER DRILL WITH CARBIDE TIPPED DRILL BIT

CONCRETE CURED FOR A MINIMUM OF 21 DAYS TEMPERATURE CATEGORY B (110 DEG. F LONG TERM AND 130 DEG. F SHORT TERM)

IF THESE CONDITIONS DO NOT EXIST, THE CONTRACTOR SHALL SUBMIT ALTERNATE DRILLING METHODS AND MANUFACTURER BOND DATA FOR THE CONDITIONS BEING INSTALLED.

- 11. POST-INSTALLED ANCHORS AND REINFORCING INSTALLED INTO CONCRETE SHALL BE AS INDICATED BELOW.
- 11.1 MECHANICAL AND SCREW ANCHORS FOR USE IN CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN CRACKED CONCRETE IN ACCORDANCE WITH ACI 355.2

11.1.1 EXPANSION ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.:

SIMPSON STRONG-TIE STRONG-BOLT 2 (ICC ESR-3037) HILTI KWIK BOLT-TZ2 (ICC ESR-4266) DEWALT POWER-STUD+ SD2 (ICC ESR-2502)

11.1.2 SCREW ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.: SIMPSON STRONG-TIE TITEN-HD (ICC ESR-2713)

HILTI KWIK HUS-EZ (ICC ESR-3027) DEWALT SCREW-BOLT+ (ICC ESR-3889

11.1.3 SHALLOW EMBEDMENT ANCHORS (<3/4") SHALL BE ONE OF THE FOLLOWING, U.N.O.:

HILTI HDI-P-TZ (ICC ESR-4236) DEWALT MINI-UNDERCUT+ (ICC ESR-3912)

11.2 ADHESIVE ANCHORS FOR USE IN CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN CRACKED CONCRETE IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308 AND THE ICC-ES EVALUATION SERVICES REPORT SHALL SHOW COMPLIANCE WITH THE BUILDING CODE STATED IN SECTION B OF THESE STRUCTURAL NOTES.

11.2.1 ADHESIVE ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.: SIMPSON STRONG-TIE SET-XP (ICC ESR-2508) HILTI HIT-RE 500 v3 SLOW CURE (ICC ESR-3814) HILTI HIT-HY 200 FAST CURE (ICC ESR-3187) DEWALT PURE110+ (ICC ESR-3298)

RED HEAD EPCON C6+ (ICC ESR-3577)

11.3 ADHESIVE FOR INSTALLING REINFORCING STEEL IN EXISTING CONCRETE SHALL BE QUALIFIED IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308 AND THE ICC-ES EVALUATION SERVICES REPORT SHALL SHOW COMPLIANCE WITH THE BUILDING CODE STATED IN SECTION B OF THESE STRUCTURAL NOTES.

11.3.1 ADHESIVE SHALL BE ONE OF THE FOLLOWING, U.N.O.: SIMPSON STRONG-TIE "SET-XP" (ICC-ES ESR-2508) HILTI HIT-RE 500 v3 SLOW CURE (ICC ESR-3814)

> DEWALT PURE110+ (ICC ESR-3298) RED HEAD EPCON C6+ (ICC ESR-3577)

12. POST-INSTALLED ANCHORS INSTALLED INTO SOLID GROUTED CONCRETE MASONRY CELLS SHALL BE AS INDICATED BELOW. DO NOT INSTALL ANCHORS INTO HOLLOW HEAD JOINTS OR TEE JOINTS BETWEEN MASONRY BLOCKS.

12.1 MECHANICAL AND SCREW ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES ACO1 AND AC106, RESPECTIVELY.

12.1.1 EXPANSION ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.: SIMPSON STRONG-TIE STRONG-BOLT2 (IAPMO-UES ER-240) HILTI KWIK BOLT-TZ2 (ICC ESR-4561)

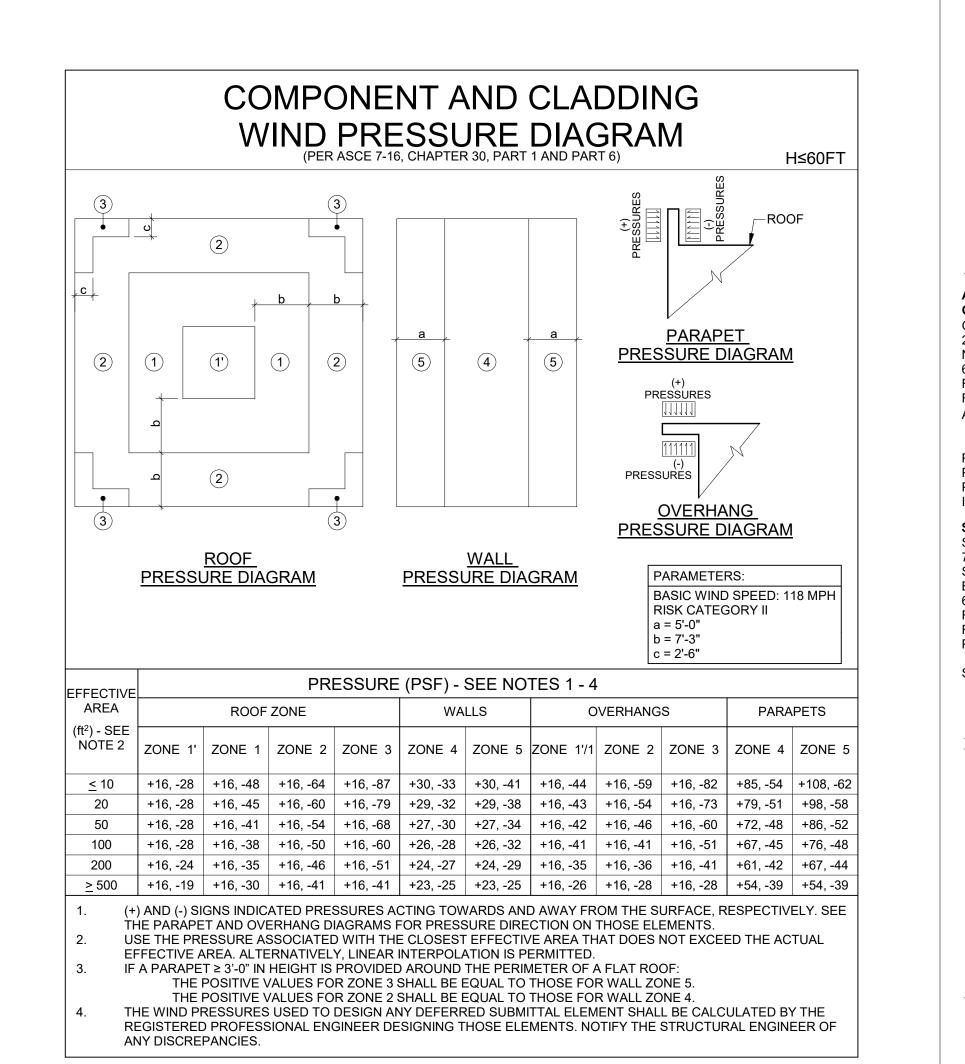
DEWALT POWER-STUD+ SD1 (ICC ESR-2966) 12.1.2 SCREW ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.: SIMPSON STRONG-TIE TITEN-HD (ICC ESR-1056)

12.2 ADHESIVE ANCHORS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC58

HILTI KWIK HUS-EZ (ICC ESR-3056) DEWALT SCREW-BOLT+ (ICC-ES ESR-4042)

DEWALT AC100+ GOLD (ICC ESR-3200)

12.2.1 ADHESIVE ANCHORS SHALL BE ONE OF THE FOLLOWING, U.N.O.: SIMPSON STRONG-TIE "SET-XP" (IAPMO-UES ER-265) HILTI HIT-HY 270 (ICC ESR-4143





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Project Principal: Rob Hamby Plum/Fire P. Designer: Project Architect: Eric Snyder Project Coordinator: Traci Myers Elec. Engineer of Record: Interior Designer: Morgan Black STRUCTURAL Stanley D. Lindsey & Associates, Ltd. I.T/Telecom PM: Michael Henry 750 Old Hickory Blvd, Building 1,

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FL Registry No. 696 EOR: Kelsey Lewis, PE Florida Registration #79384 Proj Mngr: Chris Akers Florida Registration #62025 Landscape AOR: Charlie Johnson Florida Reg #6667402

Shawn Sullivan

Donna Seigal

Paul C. McKinney

Florida Registration #81090

Florida Registration #56569



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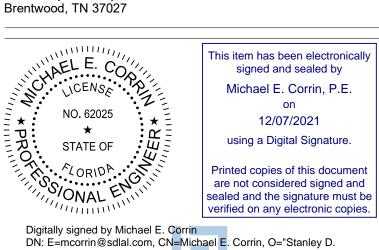
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STRUCTURAL NOTES (CONT.)

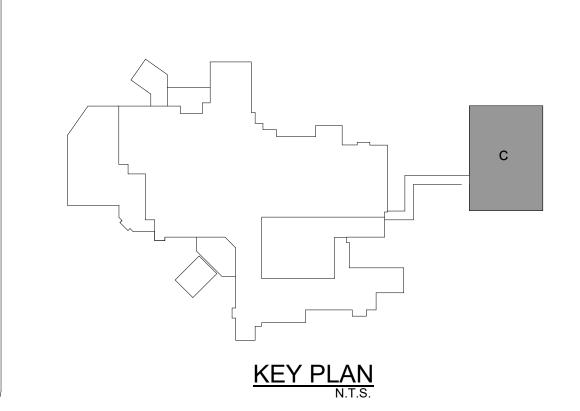
NOTES:

1. REFERENCE ELEVATION 0'-0" = 148.5' (REFER TO CIVIL DRAWINGS). TOP OF FOOTING ELEVATIONS SHALL BE AS FOLLOWS, UNLESS NOTED OTHERWISE ON PLAN: EXTERIOR FOUNDATIONS: 2'-0" BELOW FINISHED GRADE FOUNDATION ELEMENTS ARE CENTERED UNDER COLUMNS, UNLESS NOTED OTHERWISE. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT COORDINATE ALL SLAB DEPRESSIONS AND SLAB STEPS WITH MEP AND ARCHITECTURAL DRAWINGS. CONTRACTOR SHALL FIELD VERIFY EXISTING CONDITIONS AND DIMENSIONS RELATED TO THE NEW STRUCTURE PRIOR TO FABRICATION AND CONSTRUCTION. SHEET AND TYPICAL DETAIL REFERENCES: S0 SERIES STRUCTURAL NOTES S310 FOOTING SCHEDULE 6/S310 FOOTING STEP SLAB ON GRADE 15/S310 GRAADE BEAM DETAIL 2/S311 FILL BEHIND FOUNDAITON WALLS 4/S211 WALL SLEEVE PENETRATIONS MASONRY DETAILS 7/S320 MASONRY LINTEL SCHEDULE 8. MARK AND LEGEND KEY:

AF#.# FOOTING GB# GRADE BEAM

STEP FOOTING STEP

MASONRY LINTEL





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HCA\\\\\Healthcare

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ANCILLARY BUILDING

LAKE CITY MEDICAL CENTER 340 NW Commerce Dr. Lake City, FL 32055 HCA Project No.: 3793800011

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FOUNDATION PLAN -ANCILLARY - PART C

S201C



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MEP Principal: Josh Cartwright

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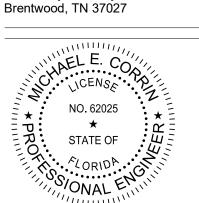


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General Contractors Robins & Morton 5500 Maryland Way, Suite 100



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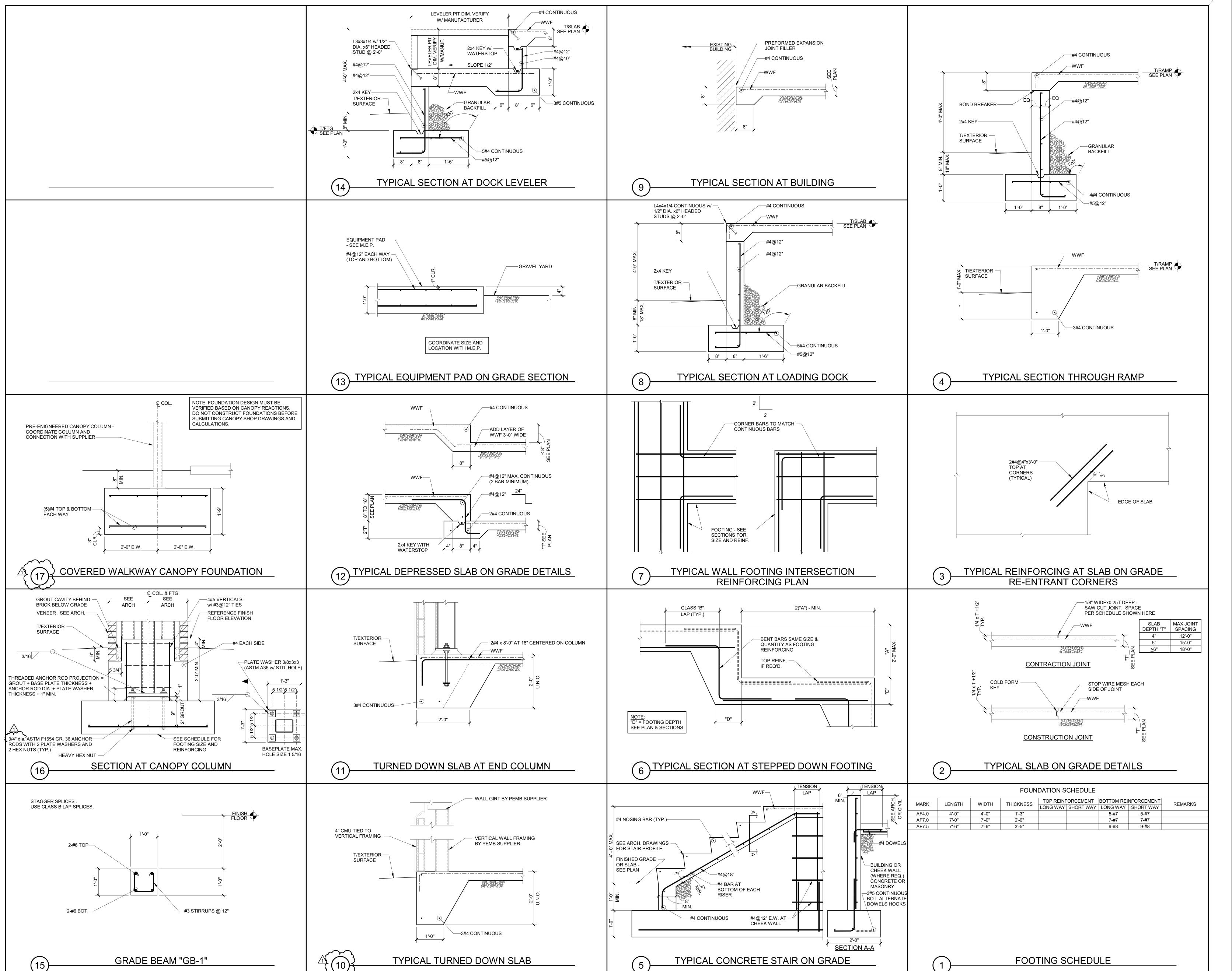
ANCILLARY BUILDING

LAKE CITY MEDICAL CENTER 340 NW Commerce Dr. Lake City, FL 32055 HCA Project No.: 3793800011

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FOUNDATION AND FRAMING PLANS -ANCILLARY - PART D

S201D





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ANCILLARY BUILDING

LAKE CITY MEDICAL CENTER
340 NW Commerce Dr.
Lake City, FL 32055
HCA Project No.: 3793800011

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SECTIONS AND DETAILS

S310

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PROJECT: 45057.00 (DATE: 12/06/2021

-5#5 CONTINUOUS

SECTION AT SCREENWALL

SEE CIVIL FOR SCREENWALL LOCATION

B/FOOTING SEE PLAN

TYPICAL RETAINING WALL

TYPICAL RETAINING WALL



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2619 Centennial Blvd., Suite 200 Tallahassee, FL 32308

HCA Healthcare

HCA Design & Construction One Park Plaza, PO Box 550 Bldg. II, East 3rd Floor Nashville, TN 37203 HCA Design Mgr.: Nicole Hoch HCA Constr. Mgr.: Ben McAlpin

> **General Contractors** Robins & Morton 5500 Maryland Way, Suite 100 Brentwood, TN 37027



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DN: E=mcorrin@sdlal.com, CN=Michael E. Corrin, O="Stanley D. Lindsey & Associates, Ltd.", C=US Reason: I have reviewed this document Date: 2022.03.11 08:14:47-06'00'

ANCILLARY BUILDING

LAKE CITY MEDICAL CENTER 340 NW Commerce Dr. Lake City, FL 32055 HCA Project No.: 3793800011

No.	Date	Description
L	3/2/2022	Amendment 1-1

LOONDATION SECTIONS AND DETAILS

S311

LINE IS 3 INCHES WHEN PRINTED FULL SIZE FULL SHEET SIZE = 30"X42"

SEE ARCH. DRAWINGS FOR EXTERIOR WALL

EDGE OF SLAB

DETAILS

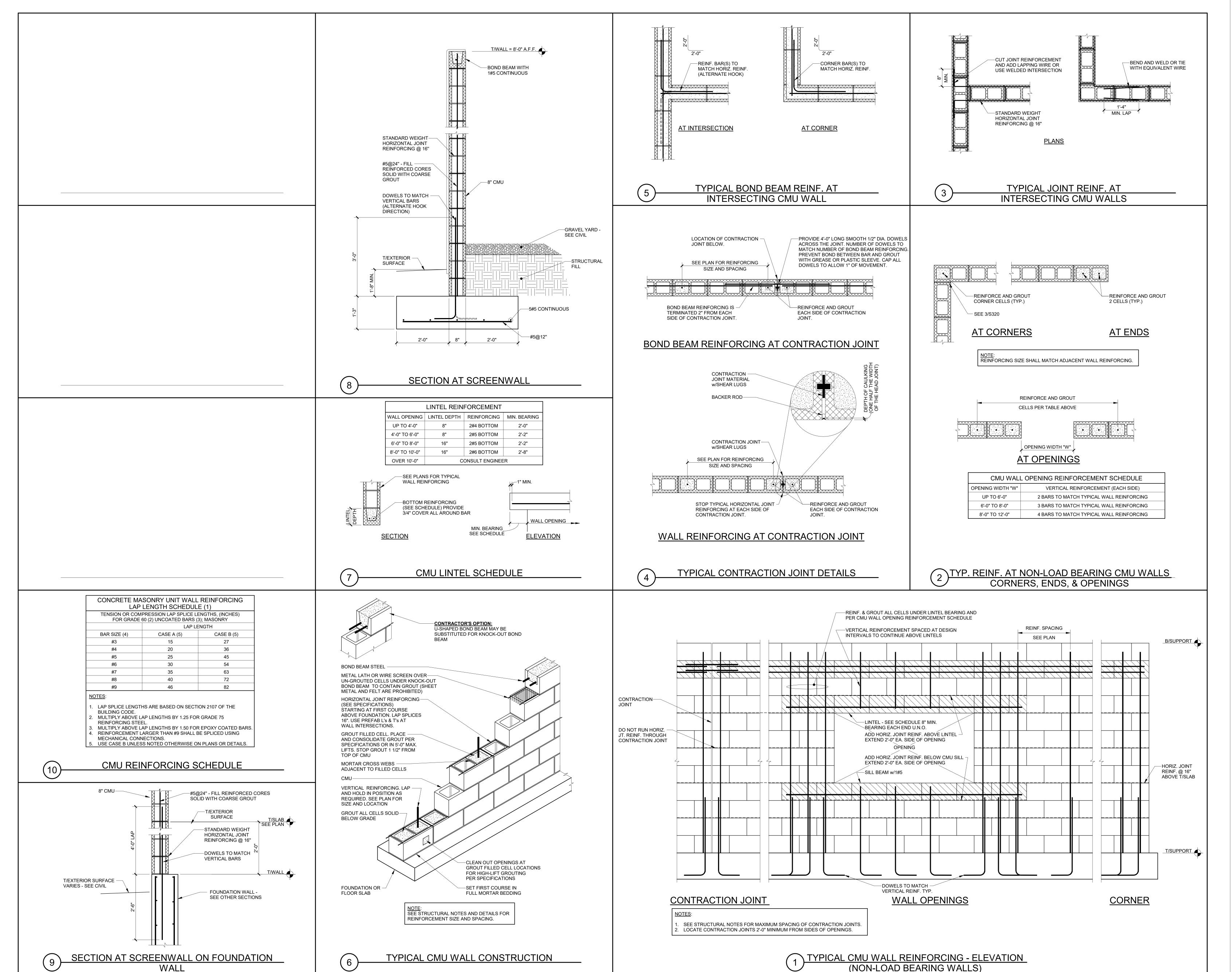
w/ ARCH. AND PEMB DRAWINGS

-C4x5.4 @ 4' - 0" (MAX.)

—EMBED PLATE 1/2"x8"x8" w/ 4-1/2" DIA. x4 3/16" LONG HEADED STUDS

TYPICAL SECTION AT LOW WINDOW SUPPORT

(WINDOWS OVER 8-FEET WIDE)





GreshamSmith.com

ARCHITECT, INTERIOR DESIGN, ME&P GRAPHICS I.C. Thomasson Associates, Inc. 2950 Kraft Drive, Suite 500 Gresham Smith 222 2nd Avenue South, Suite 1400 Nashville, TN 37204 Nashville, TN 37201-2308 615.346.3400 FL Qualifier No. 38970 615.770.8100 FL Registry No. 1276 FL Qualifier No. AR0013420 MEP Principal: Josh Cartwright FL Registry No. RY3806 Architect/Interior Designer of Record: MEP Proj Mngr: Paul C. McKinney Mech Engineer of Record: Eric M. Snyder Shawn Sullivan Florida Registration #96197 Florida Registration #81090 Project Principal: Rob Hamby Plum/Fire P. Designer: Project Architect: Eric Snyder Donna Seigal Project Coordinator: Traci Myers Elec. Engineer of Record: Interior Designer: Morgan Black

Paul C. McKinney STRUCTURAL Florida Registration #56569 Stanley D. Lindsey & Associates, Ltd. I.T/Telecom PM: Michael Henry 750 Old Hickory Blvd, Building 1, **CIVIL & LANDSCAPE** Suite 175 Brentwood, TN 37027 2619 Centennial Blvd., Suite 200 615.320.1735 Tallahassee, FL 32308 FL Qualifier No. 39200 850.553.3500 FL Registry No. 1329 Project Principal/ Engineer: FL Registry No. 696 Mark Hilner Structural Engineer of Record: Michael E. Corrin

FL Qualifier No. 49629 EOR: Kelsey Lewis, PE Florida Registration #79384 Florida Registration #62025 Proj Mngr: Chris Akers Landscape AOR: Charlie Johnson Florida Reg #6667402



I.C. Thomasson Associates, Inc. CONSULTING ENGINEERS

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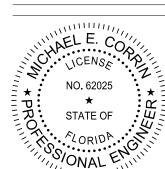


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DETAILS

S320

 $\c G$ BEAM CONTINUOUS 1/4" BENT "Z" — 3/16 / 2@12 MIN. 5" MIN. - COORDINATE w/ARCH. -DWGS. TO ACCOMMODATE ROOFING MATERIAL -ROOF DECK - SEE PLAN FOR DIRECTION 3/8"x8" OUTRIGGER PLATES @ -4'-0". REQUIRED ONLY WHERE Q OF BEAM TO FACE OF BENT PLATE EXCEEDS 8". TYPICAL SECTION AT ROOF 3 SIDES\ TYPICAL/ -3/8" STIFFENER PLATE (NS/FS) 3/16 / 3 -_" CAP PLATE w/ 2-3/4"Ø BOLTS END COLUMN 3 SIDES TYPICAL/ -3/8" STIFFENER PLATE (NS/FS) -3/4" CAP PLATE w/ 4-3/4"Ø BOLTS INTERMEDIATE COLUMN TYPICAL BEAM TO HSS COLUMN CONNECTION (CONTINUOUS/CANTILEVERED BEAMS) 3/16 ---L3-1/2x3-1/2x1/4x0'-5" -L3x3x1/4 SUPPORT -L3x3x1/4 FRAME OPENING SIZE SEE MECH. DWGS TYPICAL ROOF OPENING FRAME G PARTITION 4 - 5/8" DIA. PUDDLE WELDS— -CONNECTION TO -AT SUPPORTS PEMB BY PEMB (36/4 PATTERN) **ENGINEER** - φ OF INTERMEDIATE SUPPORT C4x5.4 @4'-0"— #10 SELF-TAPPING SCREWS— @ 24" AT SIDE LAPS L3x3x1/4 KICKER AT EACH-END OF PARTITION TO PEMB GIRT —5/8" DIA. PUDDLE WELDS -L3x3x1/4 @4'-0" @ 12" AT PERIMETER SUPPORTS L4x4x1/4x0'-6" @4'-0"— - Ç OF END SUPPORT FOLDING PARTITION C8x11.5 CONTINUOUS— WEIGHT OF 100 PLF TYPICAL FOLDING PARTITION WALL HANGER TYPICAL ROOF DECK ATTACHMENT (1 1/2" TYPE "B" ROOF DECK) COORDINATE LOCATIONS W/ ARCH



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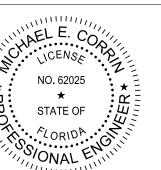


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SECTIONS AND DETAILS

S410